

Alpine, an ITW Company
 6750 Forum Drive, Suite 305
 Orlando, FL 32821
 Phone: (800)755-6001
 www.alpineitw.com

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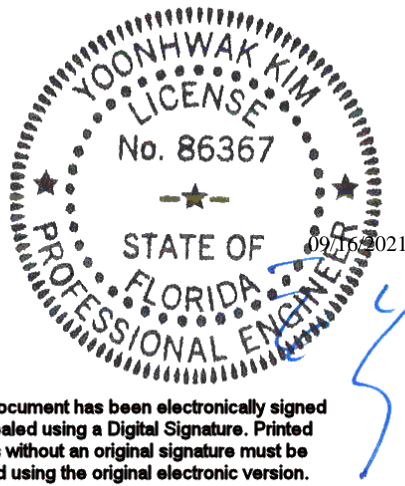
| Site Information: | Page 1: |
|---------------------------------------|---------------------|
| Customer: W. B. Howland Company, Inc. | Job Number: 21-5856 |
| Job Description: Shelley | |
| Address: | |

| Job Engineering Criteria: | |
|---|---|
| Design Code: FBC 7th Ed. 2020 Res. HVHZ | IntelliVIEW Version: 21.01.01A JRef #: 1X8V2150010 |
| Wind Standard: ASCE 7-16 Wind Speed (mph): 120 | Design Loading (psf): 40.00 |
| Building Type: Closed | |

This package contains general notes pages, 123 truss drawing(s) and 7 detail(s).

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 1 | 259.21.1157.39437 | A01 |
| 3 | 259.21.1157.36576 | A03 |
| 5 | 259.21.1157.37951 | A05 |
| 7 | 259.21.1157.39046 | A07 |
| 9 | 259.21.1157.38404 | A09 |
| 11 | 259.21.1157.37889 | A11 |
| 13 | 259.21.1157.38139 | B01 |
| 15 | 259.21.1237.07507 | C01 |
| 17 | 259.21.1157.37982 | C03 |
| 19 | 259.21.1157.36016 | C05 |
| 21 | 259.21.1157.36045 | C07 |
| 23 | 259.21.1157.36873 | C09 |
| 25 | 259.21.1157.38733 | C11 |
| 27 | 259.21.1157.36157 | C13 |
| 29 | 259.21.1157.37873 | CPC02 |
| 31 | 259.21.1157.37390 | CPC04 |
| 33 | 259.21.1157.36779 | CPD01 |
| 35 | 259.21.1157.36404 | CPD03 |
| 37 | 259.21.1157.35779 | CPD05 |
| 39 | 259.21.1157.36187 | CPD07 |
| 41 | 259.21.1157.37187 | D02 |
| 43 | 259.21.1157.37592 | D04 |
| 45 | 259.21.1157.37248 | D06 |
| 47 | 259.21.1157.36624 | D08 |
| 49 | 259.21.1157.37264 | D10 |
| 51 | 259.21.1157.37529 | D12 |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 2 | 259.21.1157.37921 | A02 |
| 4 | 259.21.1157.37780 | A04 |
| 6 | 259.21.1157.38889 | A06 |
| 8 | 259.21.1157.39076 | A08 |
| 10 | 259.21.1157.35750 | A10 |
| 12 | 259.21.1157.39201 | A12 |
| 14 | 259.21.1157.35671 | B02 |
| 16 | 259.21.1157.36452 | C02 |
| 18 | 259.21.1157.35873 | C04 |
| 20 | 259.21.1157.37624 | C06 |
| 22 | 259.21.1157.37232 | C08 |
| 24 | 259.21.1157.37516 | C10 |
| 26 | 259.21.1157.36701 | C12 |
| 28 | 259.21.1157.35937 | CPC01 |
| 30 | 259.21.1157.36717 | CPC03 |
| 32 | 259.21.1157.35312 | CPC05 |
| 34 | 259.21.1157.36655 | CPD02 |
| 36 | 259.21.1157.36420 | CPD04 |
| 38 | 259.21.1157.37467 | CPD06 |
| 40 | 259.21.1157.38264 | D01 |
| 42 | 259.21.1157.36217 | D03 |
| 44 | 259.21.1157.36812 | D05 |
| 46 | 259.21.1157.36968 | D07 |
| 48 | 259.21.1157.36014 | D09 |
| 50 | 259.21.1157.36982 | D11 |
| 52 | 259.21.1157.36436 | D13 |



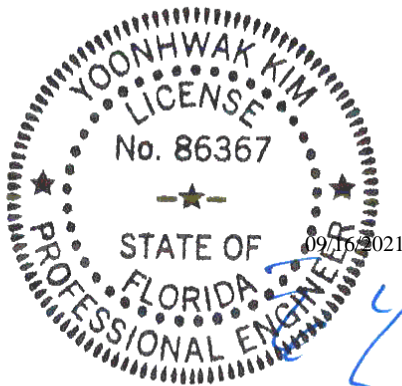
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| Site Information: | Page 2: |
|---------------------------------------|---------------------|
| Customer: W. B. Howland Company, Inc. | Job Number: 21-5856 |
| Job Description: Shelley | |
| Address: | |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 53 | 259.21.1157.38545 | E01 |
| 55 | 259.21.1157.38327 | F01 |
| 57 | 259.21.1157.36936 | F03 |
| 59 | 259.21.1157.38999 | FG01 |
| 61 | 259.21.1157.35420 | G02 |
| 63 | 259.21.1157.38436 | G04 |
| 65 | 259.21.1157.38092 | G06 |
| 67 | 259.21.1157.36248 | G08 |
| 69 | 259.21.1157.39326 | H01 |
| 71 | 259.21.1157.38842 | H03 |
| 73 | 259.21.1157.37843 | J01 |
| 75 | 259.21.1157.39264 | J02HJ |
| 77 | 259.21.1157.39107 | J03HJ |
| 79 | 259.21.1157.38640 | J04HJ |
| 81 | 259.21.1157.38577 | J06 |
| 83 | 259.21.1157.36156 | J07 |
| 85 | 259.21.1157.38686 | J08HJ |
| 87 | 259.21.1157.35390 | J10 |
| 89 | 259.21.1157.37827 | J11 |
| 91 | 259.21.1157.37561 | J12 |
| 93 | 259.21.1157.35421 | J13 |
| 95 | 259.21.1157.35749 | J14 |
| 97 | 259.21.1157.35889 | J15 |
| 99 | 259.21.1157.35483 | J19 |
| 101 | 259.21.1157.38188 | J21 |
| 103 | 259.21.1157.37514 | J27 |
| 105 | 259.21.1157.35326 | J29 |
| 107 | 259.21.1237.12320 | J31 |
| 109 | 259.21.1157.36515 | J33 |
| 111 | 259.21.1157.37076 | J35 |
| 113 | 259.21.1157.35609 | J39 |
| 115 | 259.21.1157.37326 | JG01 |
| 117 | 259.21.1157.36076 | JG03 |
| 119 | 259.21.1157.35701 | V02 |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 54 | 259.21.1157.37279 | E02 |
| 56 | 259.21.1157.35389 | F02 |
| 58 | 259.21.1157.38982 | F04 |
| 60 | 259.21.1157.38187 | G01 |
| 62 | 259.21.1157.39170 | G03 |
| 64 | 259.21.1157.38921 | G05 |
| 66 | 259.21.1157.38217 | G07 |
| 68 | 259.21.1157.38498 | G09 |
| 70 | 259.21.1157.38811 | H02 |
| 72 | 259.21.1157.38936 | H04 |
| 74 | 259.21.1157.37218 | J02 |
| 76 | 259.21.1157.35812 | J03 |
| 78 | 259.21.1157.37842 | J04 |
| 80 | 259.21.1157.35719 | J05 |
| 82 | 259.21.1157.38623 | J06HJ |
| 84 | 259.21.1157.36843 | J08 |
| 86 | 259.21.1157.36374 | J09 |
| 88 | 259.21.1157.38514 | J10HJ |
| 90 | 259.21.1157.38123 | J11HJ |
| 92 | 259.21.1157.38937 | J12HJ |
| 94 | 259.21.1157.38265 | J13HJ |
| 96 | 259.21.1157.38406 | J14HJ |
| 98 | 259.21.1157.39312 | J15HJ |
| 100 | 259.21.1157.37748 | J20 |
| 102 | 259.21.1157.38702 | J26 |
| 104 | 259.21.1157.35718 | J28 |
| 106 | 259.21.1157.38795 | J30 |
| 108 | 259.21.1157.36732 | J32 |
| 110 | 259.21.1157.37139 | J34 |
| 112 | 259.21.1157.36937 | J36 |
| 114 | 259.21.1157.39139 | J40 |
| 116 | 259.21.1157.38046 | JG02 |
| 118 | 259.21.1157.36516 | V01 |
| 120 | 259.21.1157.37046 | V03 |



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| Site Information: | Page 3: |
|---------------------------------------|---------------------|
| Customer: W. B. Howland Company, Inc. | Job Number: 21-5856 |
| Job Description: Shelley | |
| Address: | |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 121 | 259.21.1157.36998 | V04 |
| 123 | 259.21.1157.38467 | V06 |
| 125 | A12030ENC160118 | |
| 127 | GABRST160118 | |
| 129 | PB160160118 | |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 122 | 259.21.1157.36155 | V05 |
| 124 | A12015ENC160118 | |
| 126 | BRCLBSUB0119 | |
| 128 | GBLLETIN0118 | |
| 130 | VALTN160118 | |

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

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Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

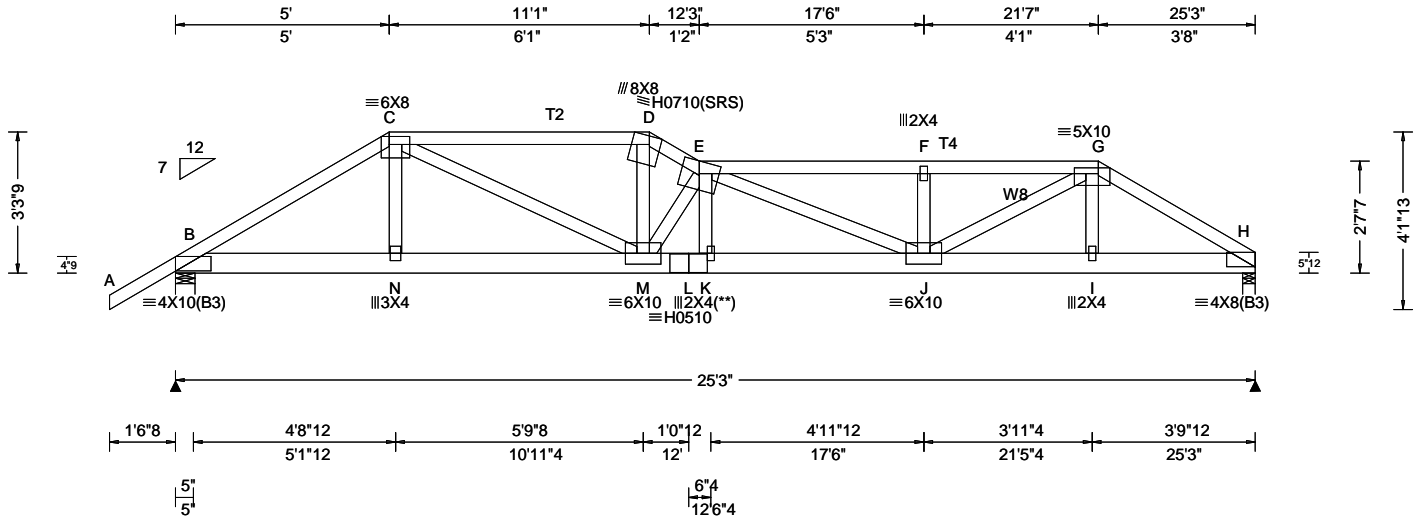
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.



| Loading Criteria (psf) | |
|------------------------|--------|
| TCLL: | 20.00 |
| TCDL: | 10.00 |
| BCLL: | 0.00 |
| BCDL: | 10.00 |
| Des Ld: | 40.00 |
| NCBCLL: | 10.00 |
| Soffit: | 2.00 |
| Load Duration: | 1.25 |
| Spacing: | 24.0 " |

| Wind Criteria | |
|----------------------|----------------|
| Wind Std: | ASCE 7-16 |
| Speed: | 120 mph |
| Enclosure: | Closed |
| Risk Category: | II |
| EXP: | C Kzt: NA |
| Mean Height: | 15.00 ft |
| TCDL: | 5.0 psf |
| BCDL: | 5.0 psf |
| MWFRS Parallel Dist: | 0 to h/2 |
| C&C Dist a: | 3.00 ft |
| Loc. from endwall: | not in 4.50 ft |
| GCp: | 0.18 |
| Wind Duration: | 1.60 |

| Snow Criteria (Pg,Pf in PSF) | |
|------------------------------|----------------------------|
| Pg: | NA Ct: NA CAT: NA |
| Pf: | NA Ce: NA |
| Lu: | NA Cs: NA |
| Snow Duration: | NA |
| Building Code: | FBC 7th Ed. 2020 Res. HVHZ |
| TPI Std: | 2014 |
| Rep Fac: | Varies by Ld Case |
| FT/RT: | 20(0)/10(0) |
| Plate Type(s): | WAVE, HS |

| Defl/CSI Criteria | |
|---------------------------------|-------------------|
| PP Deflection in loc L/defl L/# | |
| VERT(LL): | 0.244 K 999 240 |
| VERT(CL): | 0.495 K 605 180 |
| HORZ(LL): | 0.053 C - - |
| HORZ(TL): | 0.108 C - - |
| Creep Factor: | 2.0 |
| Max TC CSI: | 0.945 |
| Max BC CSI: | 0.494 |
| Max Web CSI: | 0.863 |
| VIEW Ver: | 21.01.01A.0521.20 |

| ▲ Maximum Reactions (lbs) | | | | | | |
|---|-----------------|--------|---------------|-------------|------|-----|
| Gravity | | | | Non-Gravity | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| B | 2374 | -/- | -/- | -/- | /309 | -/- |
| H | 2161 | -/- | -/- | -/- | /233 | -/- |
| Wind reactions based on MWFRS | | | | | | |
| B | Brg Width = 5.5 | | Min Req = 2.0 | | | |
| H | Brg Width = 3.5 | | Min Req = 1.8 | | | |
| Bearings B & H are a rigid surface. | | | | | | |
| Members not listed have forces less than 375# | | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | |
| B - C | 513 -4063 | E - F | 617 -5546 | | | |
| C - D | 634 -5251 | F - G | 617 -5547 | | | |
| D - E | 696 -5846 | G - H | 408 -3687 | | | |

| Maximum Bot Chord Forces Per Ply (lbs) | | | |
|--|------------|--------|-------------|
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| B - N | 3459 -432 | K - J | 6624 -780 |
| N - M | 3486 -428 | J - I | 3138 -336 |
| M - L | 6621 -781 | I - H | 3126 -339 |
| L - K | 6621 -781 | | |

| Maximum Web Forces Per Ply (lbs) | | | |
|----------------------------------|------------|-------|-------------|
| Webs | Tens.Comp. | Webs | Tens. Comp. |
| C - N | 552 0 | E - J | 177 -1171 |
| C - M | 1966 -229 | F - J | 139 -459 |
| M - D | 2265 -108 | J - G | 2744 -321 |
| M - E | 302 -2927 | | |

Lumber

Top chord: 2x4 SP #2; T2,T4 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W8 2x4 SP #2;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 25.25
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 25.25
 TC: 129 lb Conc. Load at 5.06, 7.06, 9.02,11.02
 TC: 91 lb Conc. Load at 17.52,19.52,21.52
 BC: 388 lb Conc. Load at 5.03,11.05
 BC: 90 lb Conc. Load at 7.06, 9.02
 BC: 221 lb Conc. Load at 16.15
 BC: 67 lb Conc. Load at 17.52,19.52
 BC: 229 lb Conc. Load at 21.55

Plating Notes

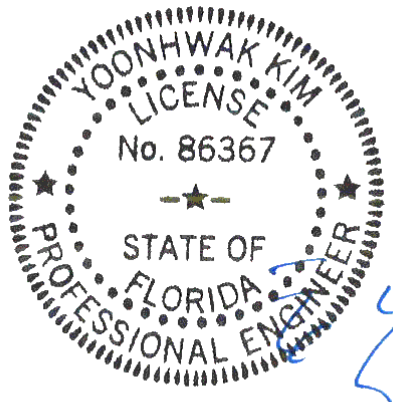
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

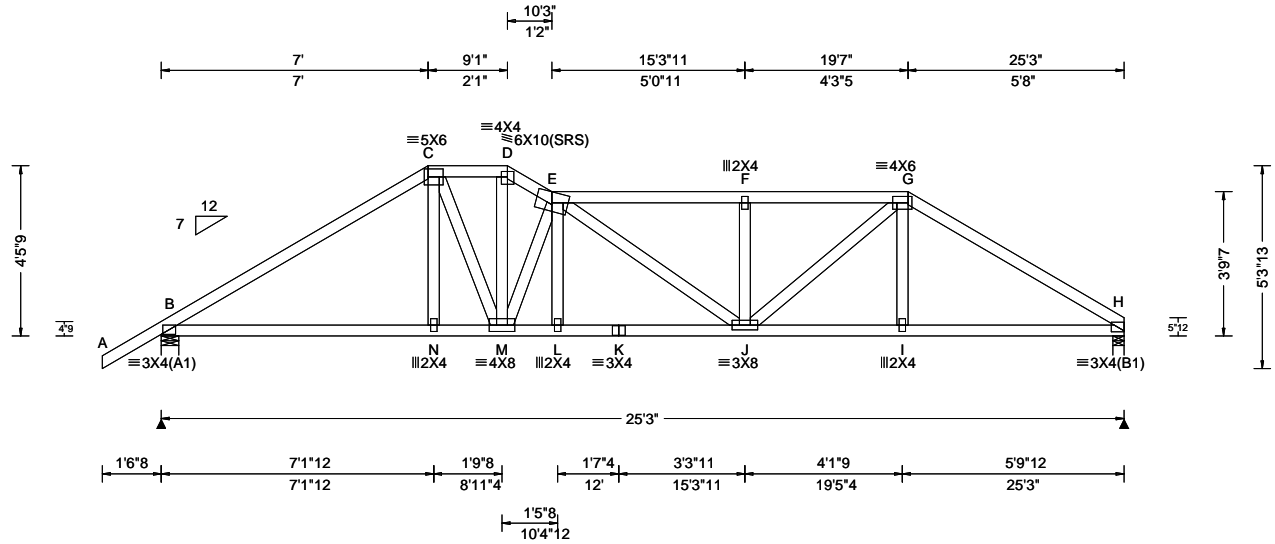
The overall height of this truss excluding overhang is 3-3-9.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

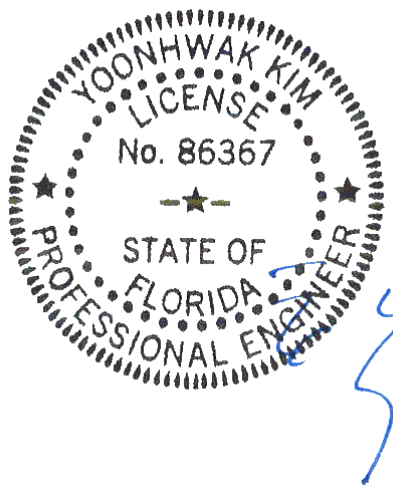




| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.080 F 999 240 VERT(CL): 0.165 F 999 180 HORZ(LL): 0.028 H - - HORZ(TL): 0.058 H - - Creep Factor: 2.0 Max TC CSI: 0.493 Max BC CSI: 0.508 Max Web CSI: 0.352 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1159 - / - / /661 /134 /111 H 1045 - / - / /574 /114 - / - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 H Brg Width = 3.5 Min Req = 1.5 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 530 -1586 E - F 770 -1805 C - D 581 -1408 F - G 770 -1805 D - E 669 -1634 G - H 579 -1611 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - N 1275 -391 K - J 1799 -634 N - M 1279 -388 J - I 1310 -429 M - L 1796 -636 I - H 1306 -431 L - K 1799 -634 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. M - D 807 -331 J - G 638 -329 M - E 511 -1180 |
|--|---|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

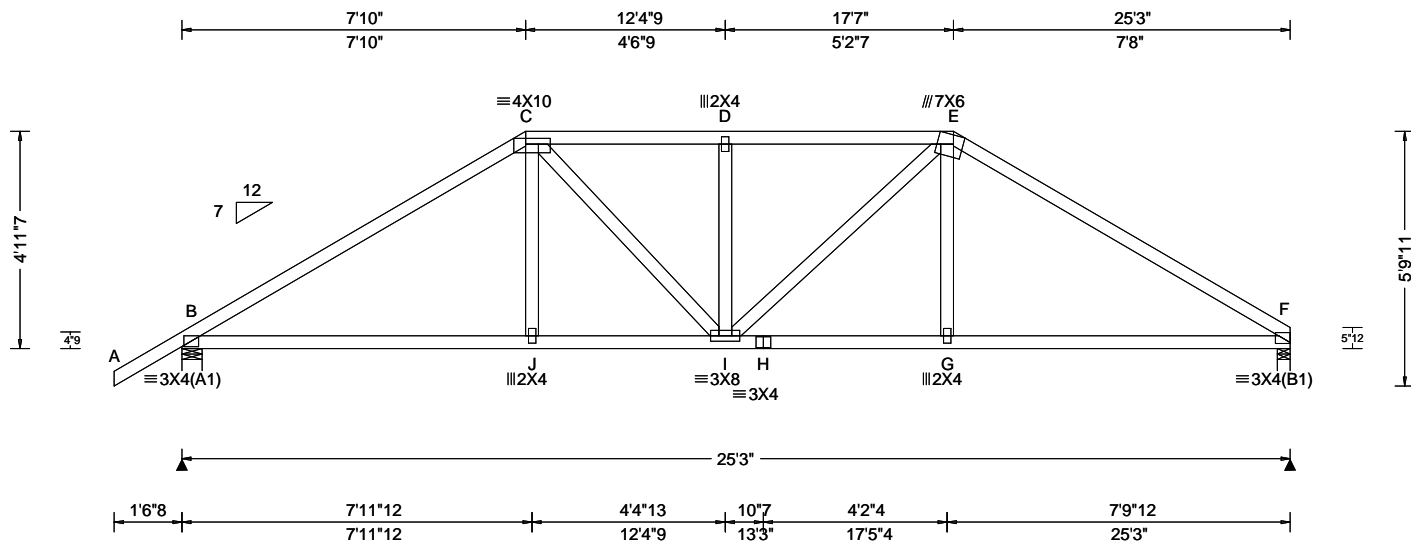
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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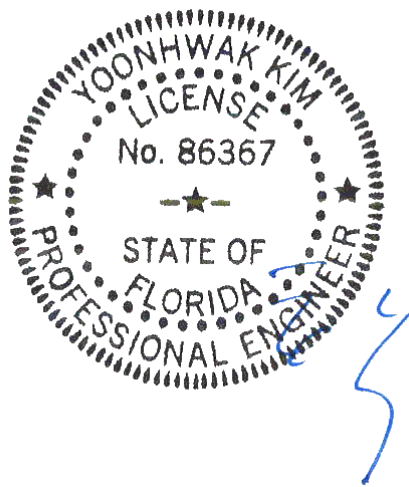
| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.051 D 999 240 VERT(CL): 0.105 D 999 180 HORZ(LL): 0.023 F - - HORZ(TL): 0.047 F - - Creep Factor: 2.0 Max TC CSI: 0.775 Max BC CSI: 0.609 Max Web CSI: 0.157 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1159 - / - / 668 /135 /122 F 1045 - / - / 584 /113 - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 3.5 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 505 -1556 D - E 594 -1397 C - D 594 -1397 E - F 507 -1550 |

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

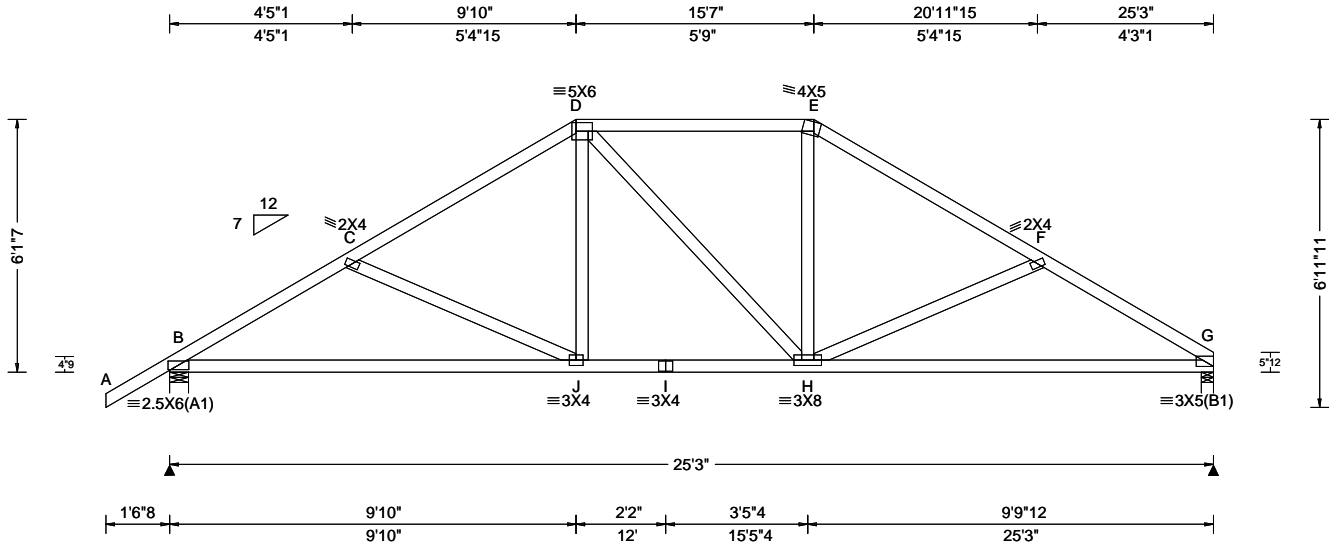
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - J | 1240 -350 | H - G | 1240 -347 |
| J - I | 1245 -348 | G - F | 1235 -350 |
| I - H | 1240 -347 | | |



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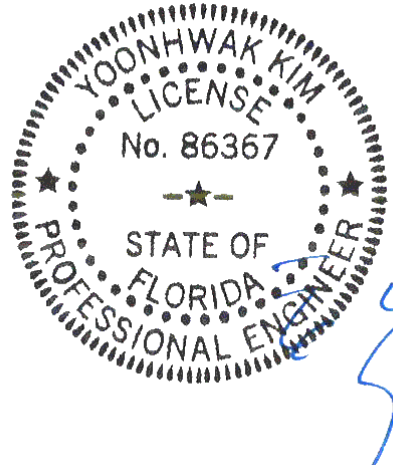


| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.054 J 999 240 VERT(CL): 0.111 J 999 180 HORZ(LL): 0.028 G - - HORZ(TL): 0.057 G - - Creep Factor: 2.0 Max TC CSI: 0.322 Max BC CSI: 0.783 Max Web CSI: 0.229 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1159 /- /- /675 /132 /148 G 1045 /- /- /591 /110 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 G Brg Width = 3.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 421 -1648 E - F 386 -1338 C - D 387 -1348 F - G 423 -1634 D - E 375 -1091 |
|--|---|---|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

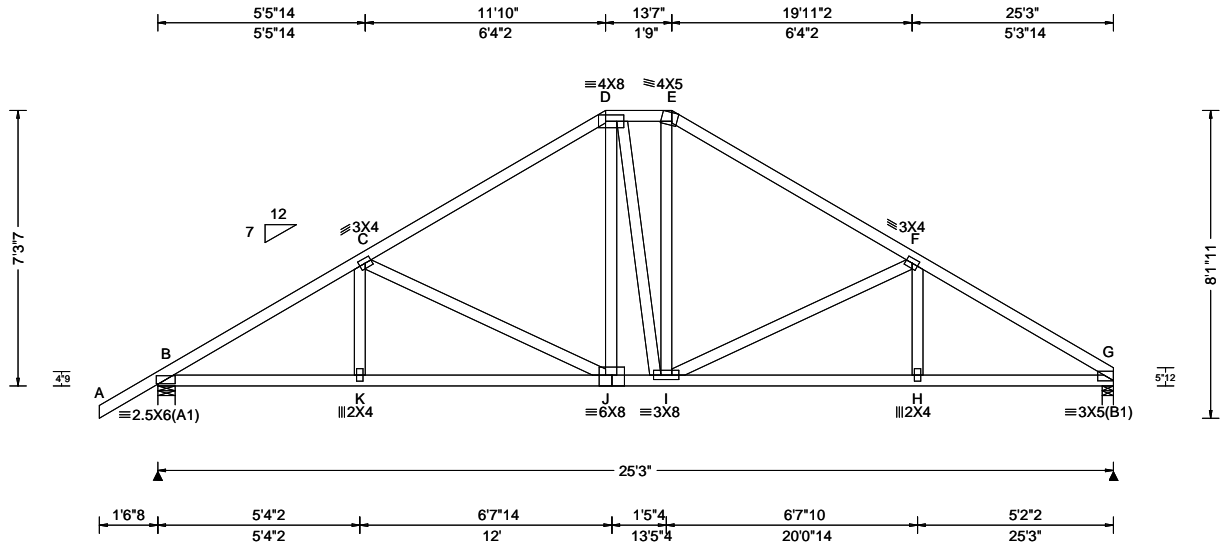
| | | | |
|---|------------|--------|-------------|
| Maximum Bot Chord Forces Per Ply (lbs) | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| B - J | 1369 -307 | I - H | 1091 -202 |
| J - I | 1091 -202 | H - G | 1353 -310 |



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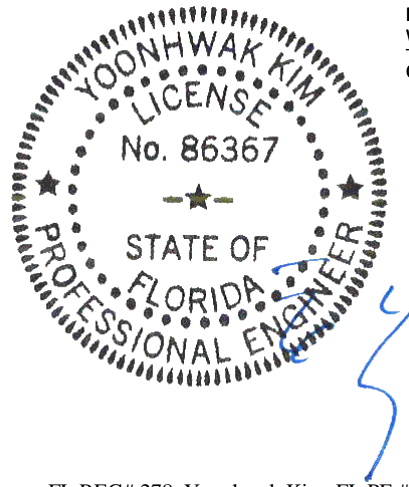
| | | | | |
|--|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.058 J 999 240 VERT(CL): 0.120 J 999 180 HORZ(LL): 0.031 G - - HORZ(TL): 0.064 G - - Creep Factor: 2.0 Max TC CSI: 0.470 Max BC CSI: 0.545 Max Web CSI: 0.496 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1159 /- /- /677 /129 /174 G 1045 /- /- /593 /107 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 G Brg Width = 3.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 261 -1675 E - F 264 -1197 C - D 265 -1202 F - G 263 -1658 D - E 269 -950 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - K 1378 -168 I - H 1360 -173 K - J 1376 -170 H - G 1362 -171 J - I 947 -58 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - J 125 -481 I - F 129 -464 |
|--|--|---|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

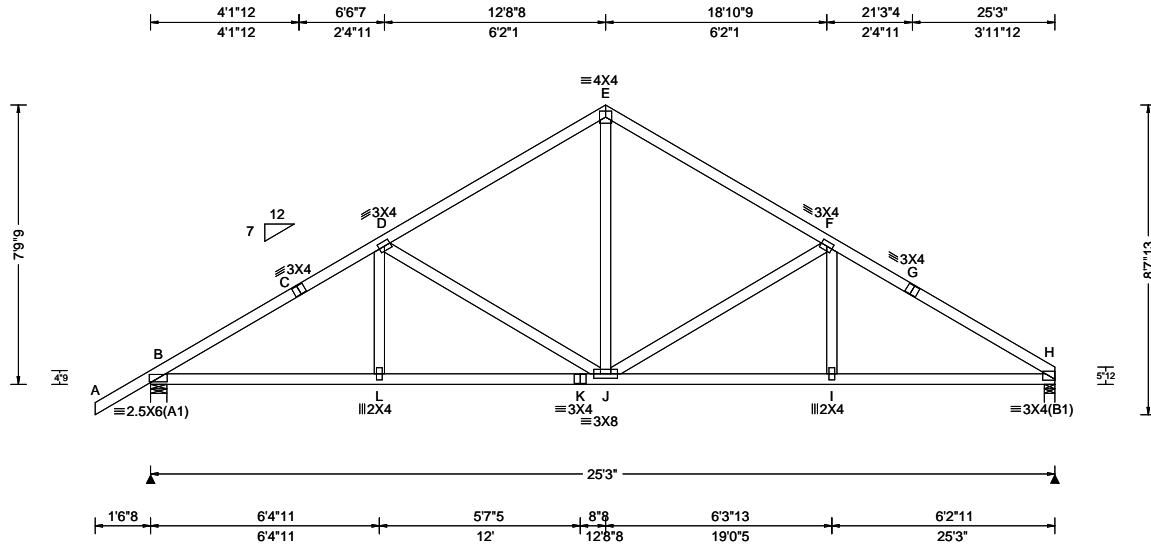
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.056 J 999 240 VERT(CL): 0.116 J 999 180 HORZ(LL): 0.027 H - - HORZ(TL): 0.056 H - - Creep Factor: 2.0 Max TC CSI: 0.395 Max BC CSI: 0.481 Max Web CSI: 0.514 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1159</td> <td>-</td> <td>-</td> <td>/677</td> <td>-</td> <td>/185</td> </tr> <tr> <td>H</td> <td>1045</td> <td>-</td> <td>-</td> <td>/593</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 H Brg Width = 3.5 Min Req = 1.5 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>192 - 1626</td> <td>E - F</td> <td>212 - 1136</td> </tr> <tr> <td>C - D</td> <td>206 - 1506</td> <td>F - G</td> <td>208 - 1493</td> </tr> <tr> <td>D - E</td> <td>212 - 1137</td> <td>G - H</td> <td>194 - 1614</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 1159 | - | - | /677 | - | /185 | H | 1045 | - | - | /593 | - | - | Chords | Tens.Comp. | Chords | Tens. Comp. | B - C | 192 - 1626 | E - F | 212 - 1136 | C - D | 206 - 1506 | F - G | 208 - 1493 | D - E | 212 - 1137 | G - H | 194 - 1614 |
|--|--|---|---|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|------|---|---|------|---|------|---|------|---|---|------|---|---|--------|------------|--------|-------------|-------|------------|-------|------------|-------|------------|-------|------------|-------|------------|-------|------------|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 1159 | - | - | /677 | - | /185 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| H | 1045 | - | - | /593 | - | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 192 - 1626 | E - F | 212 - 1136 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - D | 206 - 1506 | F - G | 208 - 1493 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - E | 212 - 1137 | G - H | 194 - 1614 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

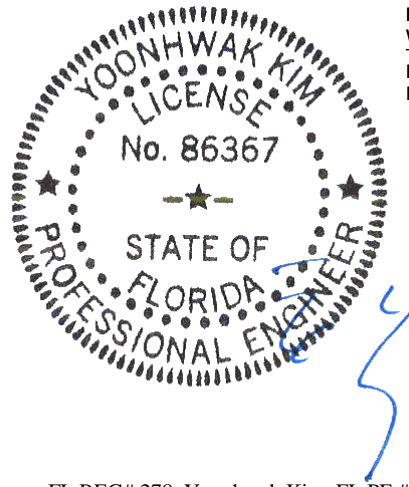
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - L | 1322 - 107 | J - I | 1308 - 111 |
| L - K | 1320 - 108 | I - H | 1310 - 110 |
| K - J | 1320 - 108 | | |

Maximum Web Forces Per Ply (lbs)

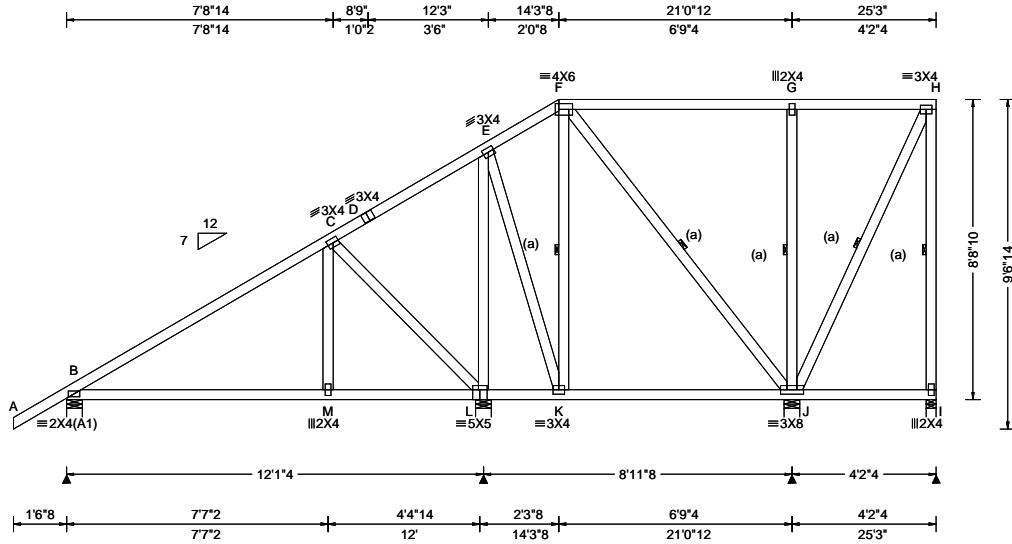
| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| D - J | 133 - 499 | J - F | 137 - 485 |
| E - J | 653 - 78 | | |



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|--|--|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 B 999 240 VERT(CL): 0.021 B 999 180 HORZ(LL): 0.008 B - - HORZ(TL): 0.017 B - - Creep Factor: 2.0 Max TC CSI: 0.631 Max BC CSI: 0.464 Max Web CSI: 0.415 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 637 /- /- /395 /- /197 L 764 /- /- /500 /30 /- J 775 /- /- /379 /110 /- I 81 /-15 /- /30 /13 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 L Brg Width = 5.5 Min Req = 1.5 J Brg Width = 5.5 Min Req = 1.5 I Brg Width = 3.5 Min Req = 1.5 Bearings B, L, J, & I are a rigid surface. Members not listed have forces less than 375# |
|--|--|---|---|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Top Chord Forces Per Ply (lbs)

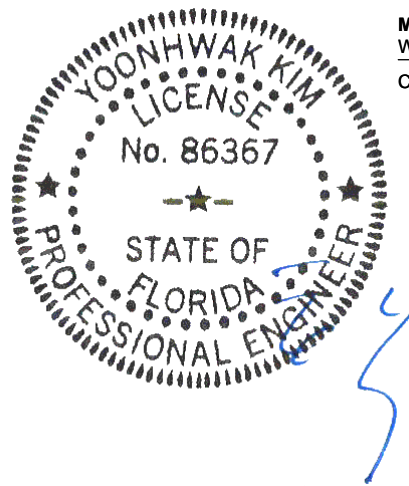
| Chords | Tens.Comp. |
|--------|------------|
| B - C | 0 -581 |

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - M | 408 -154 | M - L | 406 -155 |

Maximum Web Forces Per Ply (lbs)

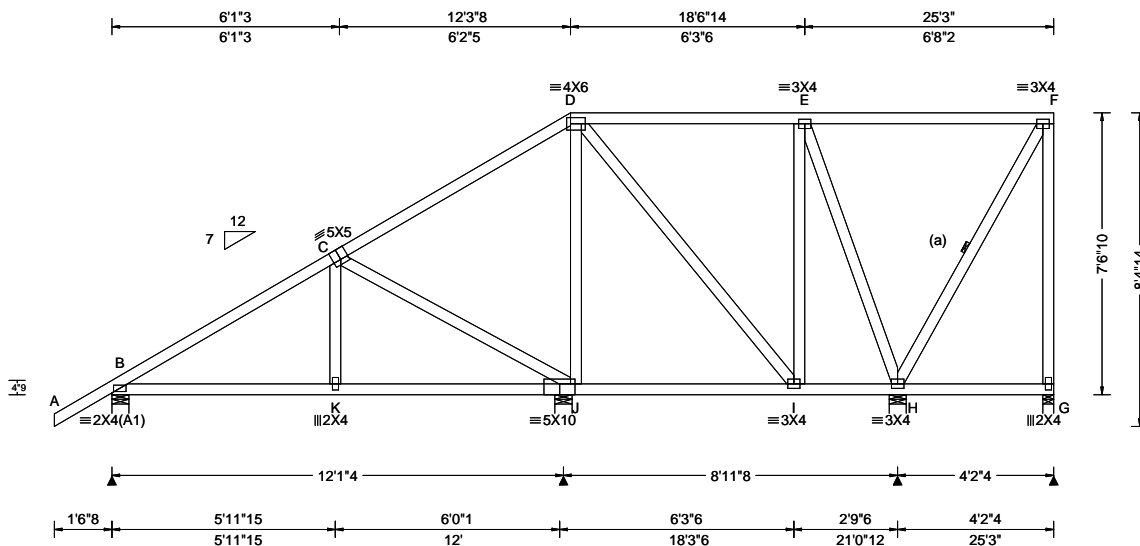
| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| C - L | 134 -538 | G - J | 375 -450 |



FL REG# 278, Yoonhwak Kim, FL PE #86367
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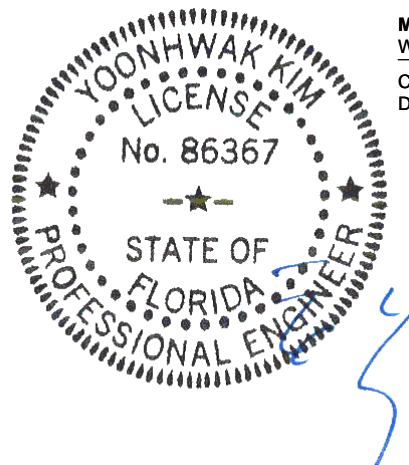


| | | | | | | | | | |
|--|---|---|--|---|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.009 K 999 240 VERT(CL): 0.017 K 999 180 HORZ(LL): 0.003 J - - HORZ(TL): 0.007 J - - Creep Factor: 2.0 Max TC CSI: 0.829 Max BC CSI: 0.322 Max Web CSI: 0.581 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL | | | | | |
| | | | | B 571 /- /- /354 /21 /237 J 948 /- /- /601 /121 /- H 646 /- /- /305 /107 /- G 103 /-30 /- /37 /15 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 J Brg Width = 5.5 Min Req = 1.5 H Brg Width = 5.5 Min Req = 1.5 G Brg Width = 3.5 Min Req = 1.5 Bearings B, J, H, & G are a rigid surface. Members not listed have forces less than 375# | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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Maximum Top Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. |
|--------|------------|
| B - C | 0 -542 |

Maximum Bot Chord Forces Per Ply (lbs)

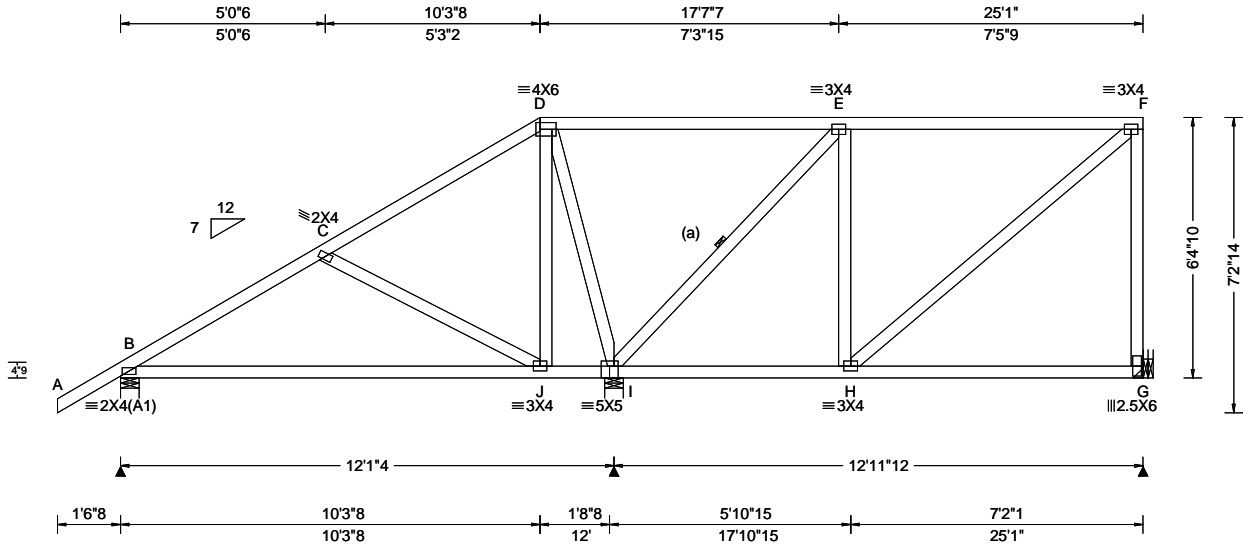
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - K | 401 -152 | K - J | 399 -154 |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| C - J | 151 -564 | E - H | 343 -469 |
| D - J | 244 -534 | | |

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| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.008 E 999 240 VERT(CL): 0.026 B 999 180 HORIZ(LL): 0.005 B - - HORIZ(TL): 0.017 B - - Creep Factor: 2.0 Max TC CSI: 0.854 Max BC CSI: 0.765 Max Web CSI: 0.554 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 556 /- /- /364 /- /202 I 1203 /- /- /645 /229 /- G 496 /- /- /257 /45 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 I Brg Width = 5.5 Min Req = 1.5 G Brg Width = - Min Req = - Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 0 -494 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. B - J 381 -95 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - J 166 -393 I - E 396 -698 D - J 500 0 H - F 397 -160 D - I 237 -719 F - G 256 -438 |
|--|---|---|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

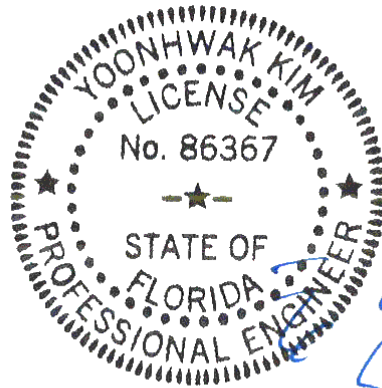
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

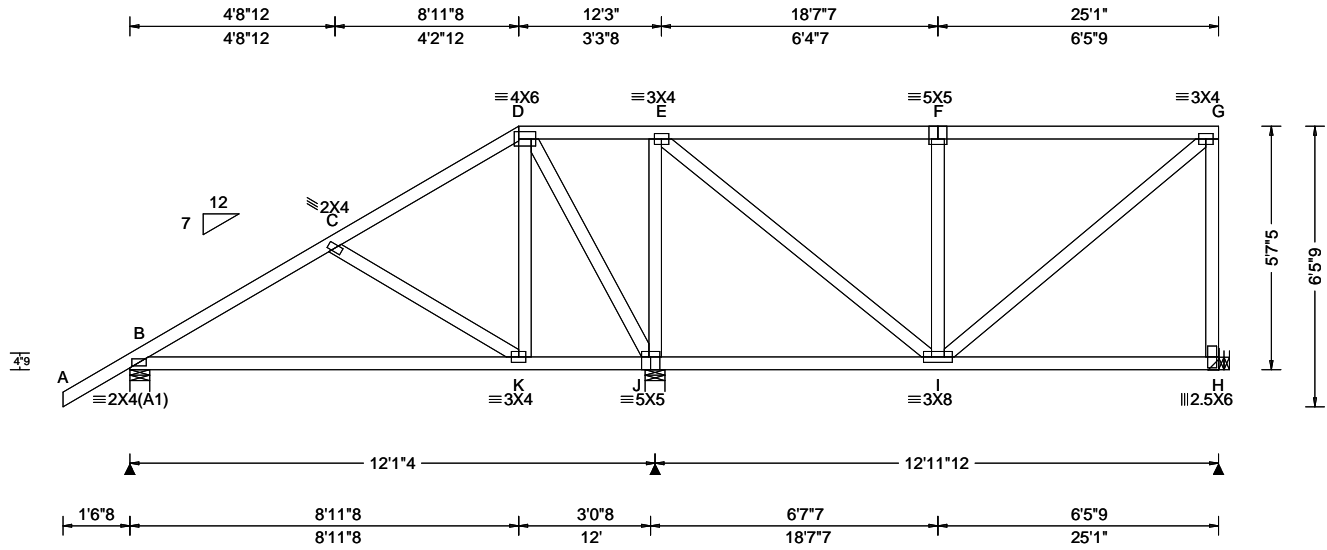
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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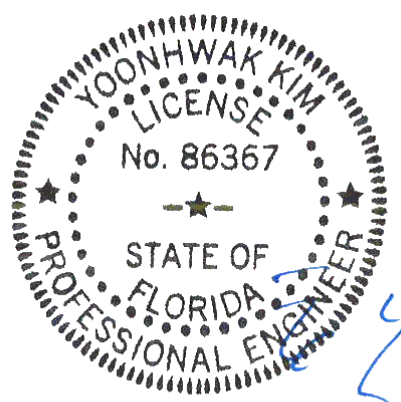


| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.012 F 999 240 VERT(CL): 0.025 F 999 180 HORZ(LL): 0.004 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.554 Max BC CSI: 0.577 Max Web CSI: 0.392 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 545 /- /- /352 /13 /178 J 1200 /- /- /642 /201 /- H 498 /- /- /249 /54 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 J Brg Width = 5.5 Min Req = 1.5 H Brg Width = - Min Req = - Bearings B & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 0 -484 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. D - J 126 -499 F - I 386 -470 J - E 454 -702 I - G 451 -202 E - I 572 -280 G - H 265 -448 |
|--|---|---|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=24'10" uses the following support conditions: 24'10"
Bearing H (24'10", 10') LUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

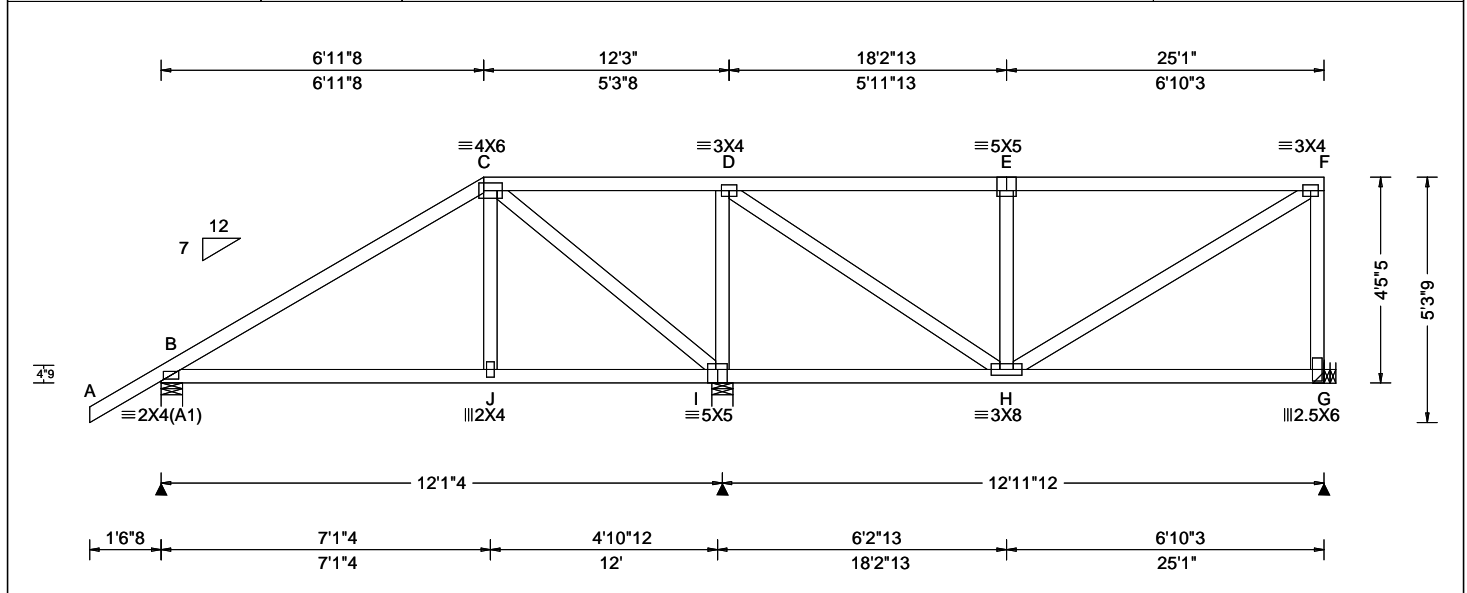
Wind
Wind loads based on MWFRS with additional C&C member design.
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Suite 305
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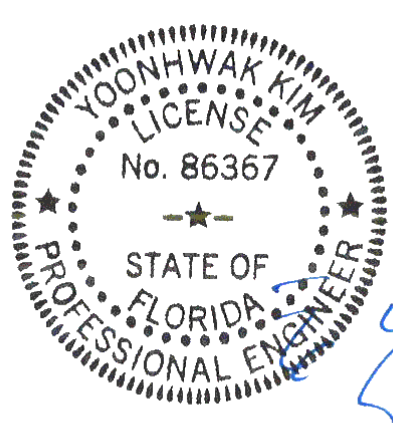


| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.014 E 999 240 VERT(CL): 0.029 E 999 180 HORZ(LL): 0.009 B - - HORZ(TL): 0.019 B - - Creep Factor: 2.0 Max TC CSI: 0.712 Max BC CSI: 0.473 Max Web CSI: 0.378 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>603</td> <td>-</td> <td>-</td> <td>/378</td> <td>/60</td> <td>/143</td> </tr> <tr> <td>I</td> <td>1102</td> <td>-</td> <td>-</td> <td>/562</td> <td>/127</td> <td>-</td> </tr> <tr> <td>G</td> <td>511</td> <td>-</td> <td>-</td> <td>/256</td> <td>/74</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 I Brg Width = 5.5 Min Req = 1.5 G Brg Width = - Min Req = - Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>48 -504</td> <td>E - F</td> <td>231 -459</td> </tr> <tr> <td>D - E</td> <td>231 -459</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - I</td> <td>153 -477</td> <td>E - H</td> <td>382 -474</td> </tr> <tr> <td>I - D</td> <td>472 -712</td> <td>H - F</td> <td>537 -271</td> </tr> <tr> <td>D - H</td> <td>620 -290</td> <td>F - G</td> <td>290 -455</td> </tr> </tbody> </table> </p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 603 | - | - | /378 | /60 | /143 | I | 1102 | - | - | /562 | /127 | - | G | 511 | - | - | /256 | /74 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | B - C | 48 -504 | E - F | 231 -459 | D - E | 231 -459 | | | Webs | Tens.Comp. | Webs | Tens. Comp. | C - I | 153 -477 | E - H | 382 -474 | I - D | 472 -712 | H - F | 537 -271 | D - H | 620 -290 | F - G | 290 -455 |
|--|---|---|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|-----|------|---|------|---|---|------|------|---|---|-----|---|---|------|-----|---|--------|------------|--------|-------------|-------|---------|-------|----------|-------|----------|--|--|------|------------|------|-------------|-------|----------|-------|----------|-------|----------|-------|----------|-------|----------|-------|----------|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 603 | - | - | /378 | /60 | /143 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| I | 1102 | - | - | /562 | /127 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 511 | - | - | /256 | /74 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 48 -504 | E - F | 231 -459 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - E | 231 -459 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | Webs | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - I | 153 -477 | E - H | 382 -474 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| I - D | 472 -712 | H - F | 537 -271 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - H | 620 -290 | F - G | 290 -455 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties
 Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
 Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
 Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
 Bearing at location x=24'10" uses the following support conditions: 24'10"
 Bearing G (24'10", 10) LUS26
 Supporting Member: (2)2x6 SP 2400f-2.0E
 (4) 0.148"x3" nails into supporting member,
 (3) 0.148"x3" nails into supported member.

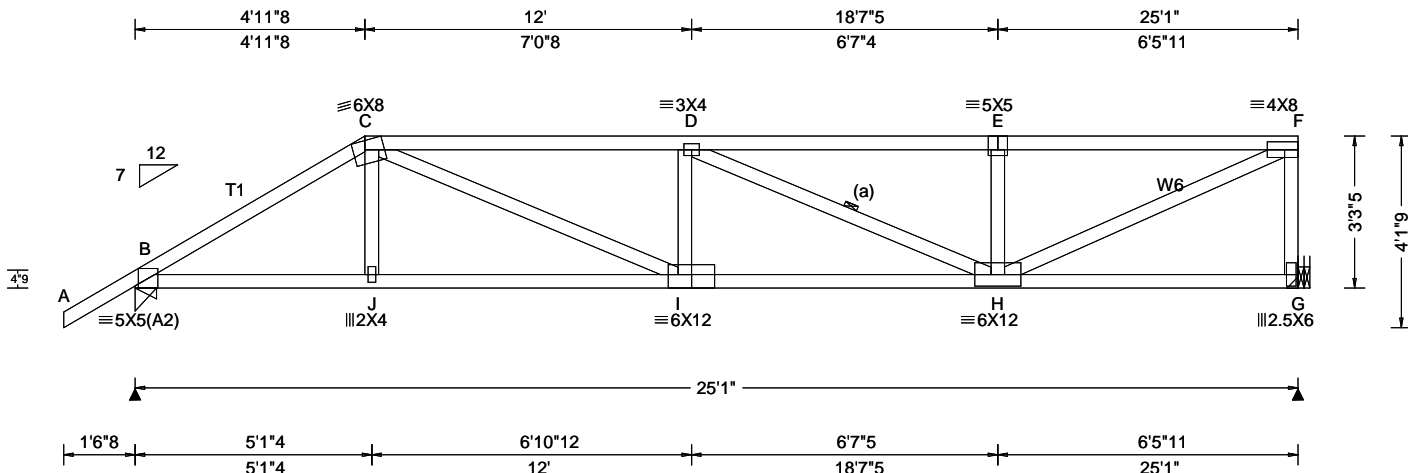
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.184 D 999 240 VERT(CL): 0.371 D 806 180 HORZ(LL): 0.043 C - - HORZ(TL): 0.086 C - - Creep Factor: 2.0 Max TC CSI: 0.632 Max BC CSI: 0.660 Max Web CSI: 0.823 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL B 2036 /- /- /- /303 /- G 2065 /- /- /- /251 /- Non-Gravity Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.7 G Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 492 -3344 D - E 459 -3345 C - D 643 -4276 E - F 459 -3345 |

Lumber
Top chord: 2x4 SP M-31; T1 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W6 2x4 SP #2;

Additional Notes
The overall height of this truss excluding overhang is 3-3-5.

Maximum Bot Chord Forces Per Ply (lbs)

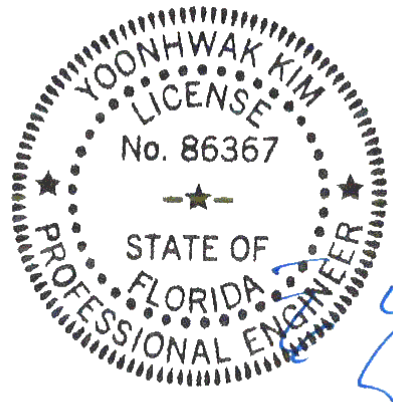
| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - J | 2815 -408 | I - H | 4301 -662 |
| J - I | 2840 -405 | | |

Bracing
(a) Continuous lateral restraint equally spaced on member.

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| C - J | 513 0 | E - H | 197 -609 |
| C - I | 1568 -260 | H - F | 3648 -502 |
| D - H | 222 -1050 | F - G | 240 -1642 |

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 4.96
TC: From 32 plf at 4.96 to 32 plf at 25.08
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 4.99
BC: From 10 plf at 4.99 to 10 plf at 25.08
TC: 129 lb Conc. Load at 5.02, 7.02, 9.02,11.02
13.02,15.02,17.02
TC: 140 lb Conc. Load at 19.02
TC: 118 lb Conc. Load at 21.02
BC: 382 lb Conc. Load at 4.99
BC: 90 lb Conc. Load at 7.02, 9.02,11.02,13.02
15.02,17.02,21.02
BC: 94 lb Conc. Load at 19.02
BC: 215 lb Conc. Load at 23.02
BC: 263 lb Conc. Load at 24.52



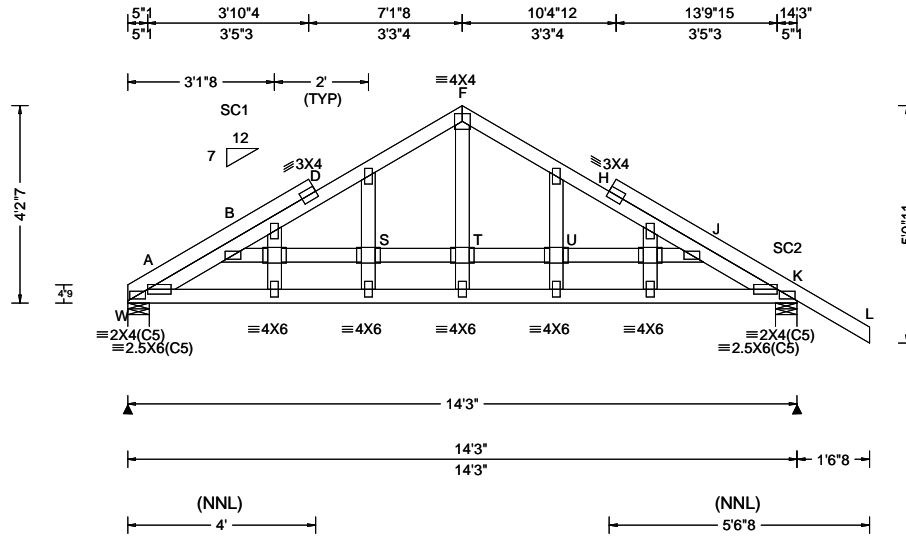
Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|---|---|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.031 D 999 240 VERT(CL): 0.066 D 999 180 HORZ(LL): 0.015 D - - HORZ(TL): 0.031 D - - Creep Factor: 2.0 Max TC CSI: 0.271 Max BC CSI: 0.387 Max Web CSI: 0.196 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ /R- /Rh /Rw /U /RL W 447 /- /- /- /236 /- K 496 /- /- /- /311 /- Non-Gravity Wind reactions based on MWFRS W Brg Width = 5.5 Min Req = 1.5 K Brg Width = 5.5 Min Req = 1.5 Bearings W & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - D 297 -529 F - H 185 -389 D - F 186 -382 H - K 330 -493 |

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 32 plf at 0.00 to 32 plf at 13.83
TC: From 63 plf at 13.83 to 63 plf at 15.79
BC: From 10 plf at 0.00 to 10 plf at 14.25
BC: From 5 plf at 14.25 to 5 plf at 15.79
TC: -9 lb Conc. Load at 1.40, 3.40, 5.40, 7.13, 8.85, 10.85, 12.85
BC: 65 lb Conc. Load at 1.36
BC: 13 lb Conc. Load at 3.40, 5.40, 7.13, 8.85, 10.85
BC: 27 lb Conc. Load at 12.89

Additional Notes
See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x4.
The overall height of this truss including overhang is 4-2-7.

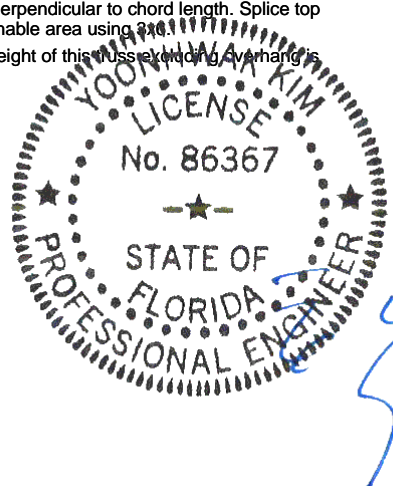
Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp.
A - K 722 -347

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
S - T 191 -388 T - U 195 -382

Plating Notes
All plates are 2X4 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

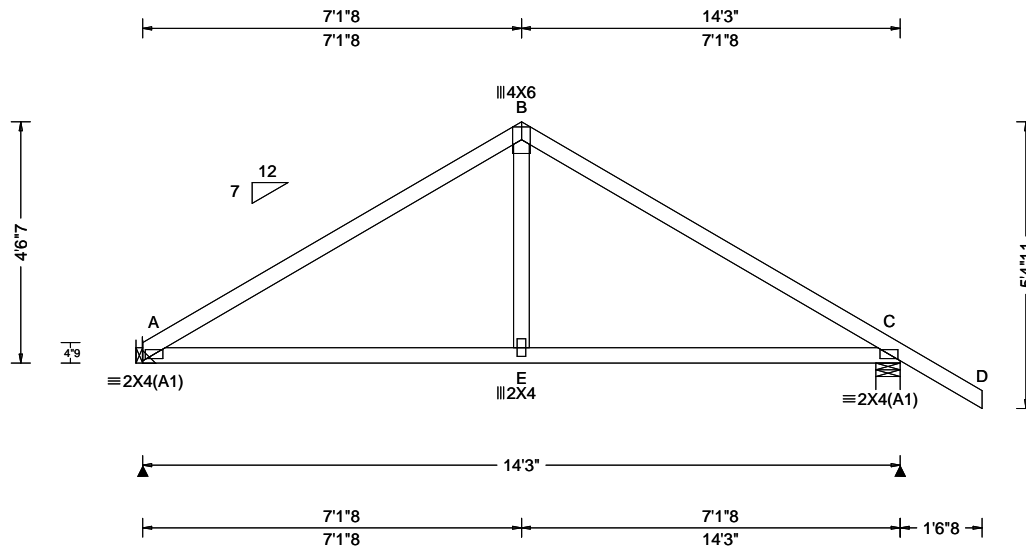
Wind
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Wind loading based on both gable and hip roof types.



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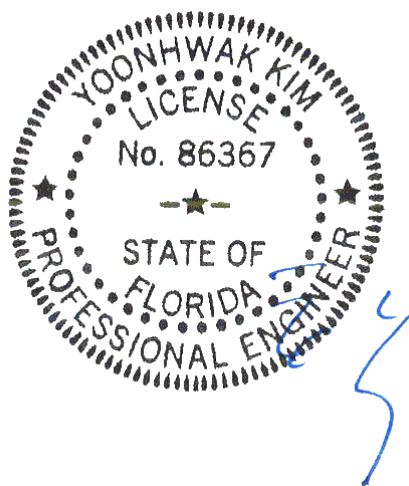


| | | | | |
|--|--|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.010 A 999 240 VERT(CL): 0.020 A 999 180 HORZ(LL): 0.008 A - - HORZ(TL): 0.016 A - - Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.495 Max Web CSI: 0.122 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 585 -/- /- /335 /57 /115 C 705 -/- /- /419 /81 -/ Wind reactions based on MWFRS A Brg Width = - Min Req = - C Brg Width = 5.5 Min Req = 1.5 Bearing C is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 286 -748 B - C 281 -752 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - E 564 -73 E - C 564 -73 |
|--|--|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

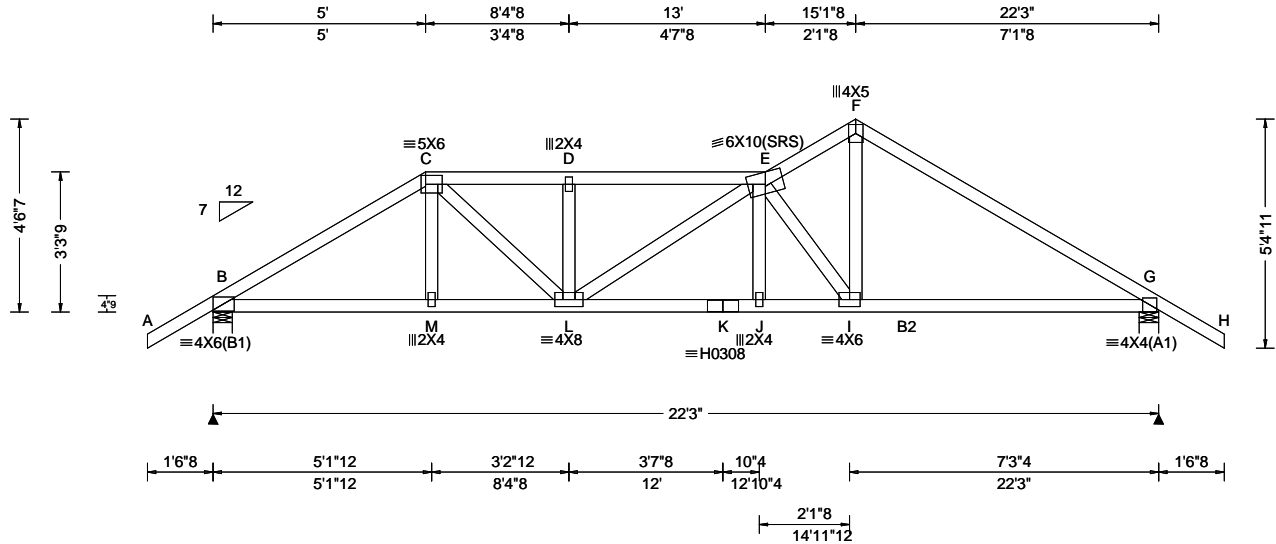
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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|--|--|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.120 D 999 240 VERT(CL): 0.241 D 999 180 HORZ(LL): 0.035 G - - HORZ(TL): 0.070 G - - Creep Factor: 2.0 Max TC CSI: 0.621 Max BC CSI: 0.741 Max Web CSI: 0.632 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1925 - / - / - / 254 - / - G 1414 - / - / - / 180 - / - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.6 G Brg Width = 5.5 Min Req = 1.7 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 398 -3077 E - F 233 -2019 C - D 417 -3358 F - G 260 -2069 D - E 417 -3358 |
|--|--|---|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 7.94
TC: From 63 plf at 7.94 to 63 plf at 23.79
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 7.94
BC: From 20 plf at 7.94 to 20 plf at 22.25
BC: From 5 plf at 22.25 to 5 plf at 23.79
TC: 129 lb Conc. Load at 5.06
TC: 140 lb Conc. Load at 7.06
BC: 388 lb Conc. Load at 5.03
BC: 94 lb Conc. Load at 7.06
BC: 650 lb Conc. Load at 7.94

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

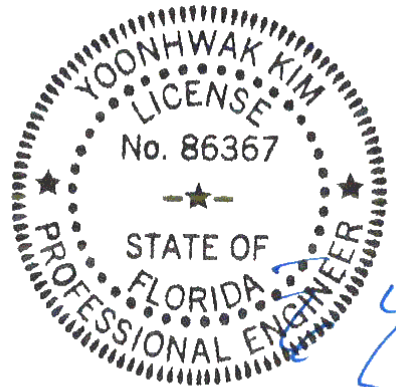
The overall height of this truss excluding overhang is 4-6-7.
WIND LOAD CASE MODIFIED!

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - M | 2572 -321 | K - J | 2758 -326 |
| M - L | 2597 -320 | J - I | 2759 -327 |
| L - K | 2758 -326 | I - G | 1690 -195 |

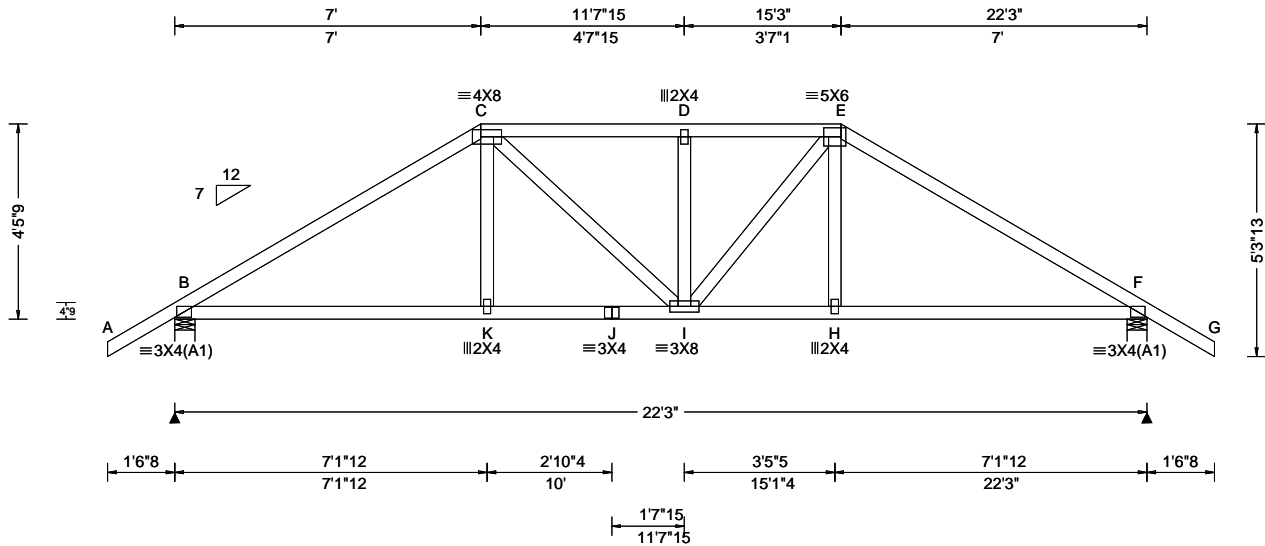
Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| C - M | 503 0 | E - I | 232 -1835 |
| C - L | 1018 -129 | I - F | 1660 -136 |
| L - E | 714 -108 | | |



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|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.039 D 999 240 VERT(CL): 0.080 D 999 180 HORZ(LL): 0.018 F - - HORZ(TL): 0.036 F - - Creep Factor: 2.0 Max TC CSI: 0.514 Max BC CSI: 0.487 Max Web CSI: 0.117 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1030 - / - / 598 /120 /128 F 1030 - / - / 598 /120 - / - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens.Comp. B - C 462 -1345 D - E 535 -1187 C - D 535 -1187 E - F 460 -1341 |
|--|---|---|--|---|

Lumber

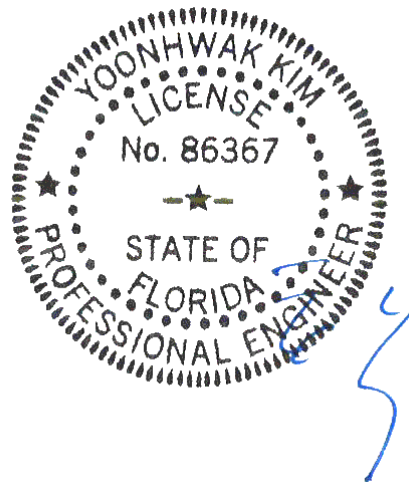
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

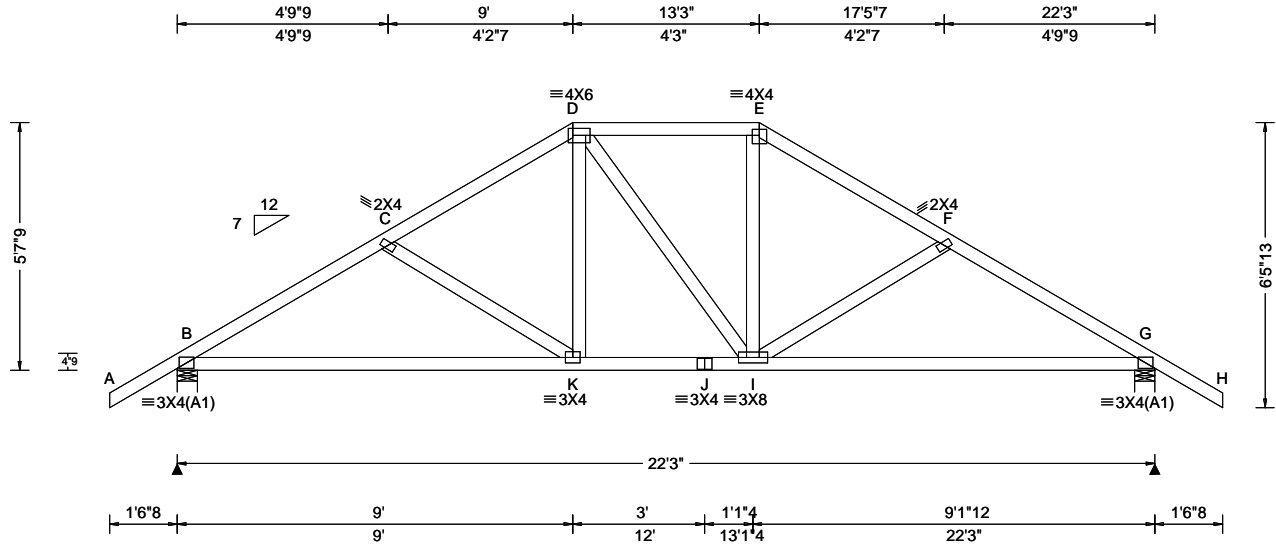
| Chords | Tens.Comp. | Chords | Tens.Comp. |
|--------|------------|--------|------------|
| B - K | 1069 -287 | I - H | 1069 -289 |
| K - J | 1074 -285 | H - F | 1065 -291 |
| J - I | 1074 -285 | | |



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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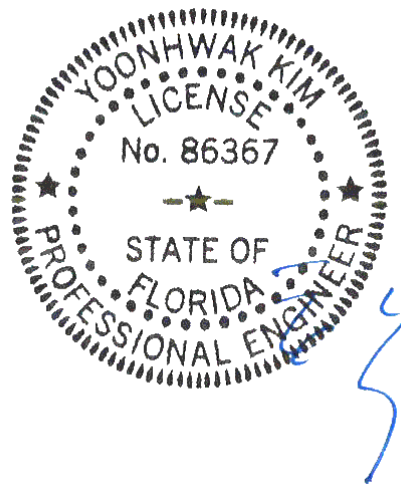
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCcp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.039 K 999 240 VERT(CL): 0.079 K 999 180 HORZ(LL): 0.019 G - - HORZ(TL): 0.038 G - - Creep Factor: 2.0 Max TC CSI: 0.223 Max BC CSI: 0.678 Max Web CSI: 0.143 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1030</td> <td>-</td> <td>-</td> <td>/604</td> <td>/117</td> <td>/154</td> </tr> <tr> <td>G</td> <td>1030</td> <td>-</td> <td>-</td> <td>/604</td> <td>/117</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 G Brg Width = 5.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>347 -1387</td> <td>E - F</td> <td>324 -1126</td> </tr> <tr> <td>C - D</td> <td>325 -1131</td> <td>F - G</td> <td>348 -1386</td> </tr> <tr> <td>D - E</td> <td>311 -919</td> <td></td> <td></td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 1030 | - | - | /604 | /117 | /154 | G | 1030 | - | - | /604 | /117 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | B - C | 347 -1387 | E - F | 324 -1126 | C - D | 325 -1131 | F - G | 348 -1386 | D - E | 311 -919 | | |
|--|--|---|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|------|---|---|------|------|------|---|------|---|---|------|------|---|--------|------------|--------|-------------|-------|-----------|-------|-----------|-------|-----------|-------|-----------|-------|----------|--|--|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 1030 | - | - | /604 | /117 | /154 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 1030 | - | - | /604 | /117 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 347 -1387 | E - F | 324 -1126 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - D | 325 -1131 | F - G | 348 -1386 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - E | 311 -919 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

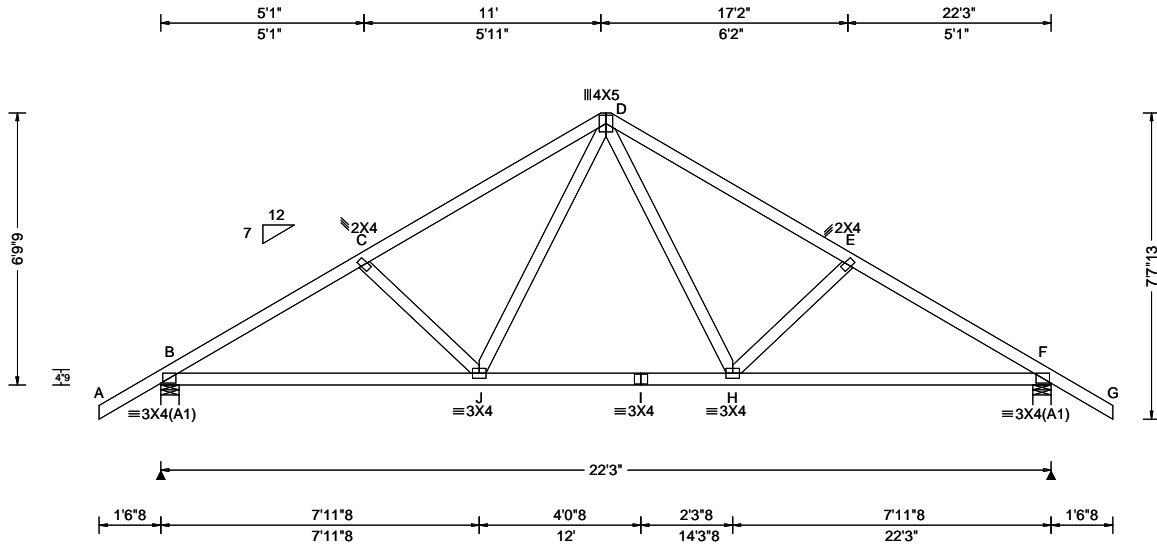
Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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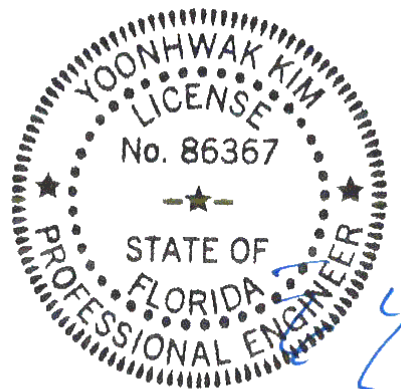


| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.042 H 999 240 VERT(CL): 0.085 H 999 180 HORZ(LL): 0.018 F - - HORZ(TL): 0.037 F - - Creep Factor: 2.0 Max TC CSI: 0.386 Max BC CSI: 0.551 Max Web CSI: 0.157 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1030 /- /- /605 /112 /180 F 1030 /- /- /605 /112 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 196 -1408 D - E 196 -1185 C - D 196 -1185 E - F 196 -1408 |
|--|---|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

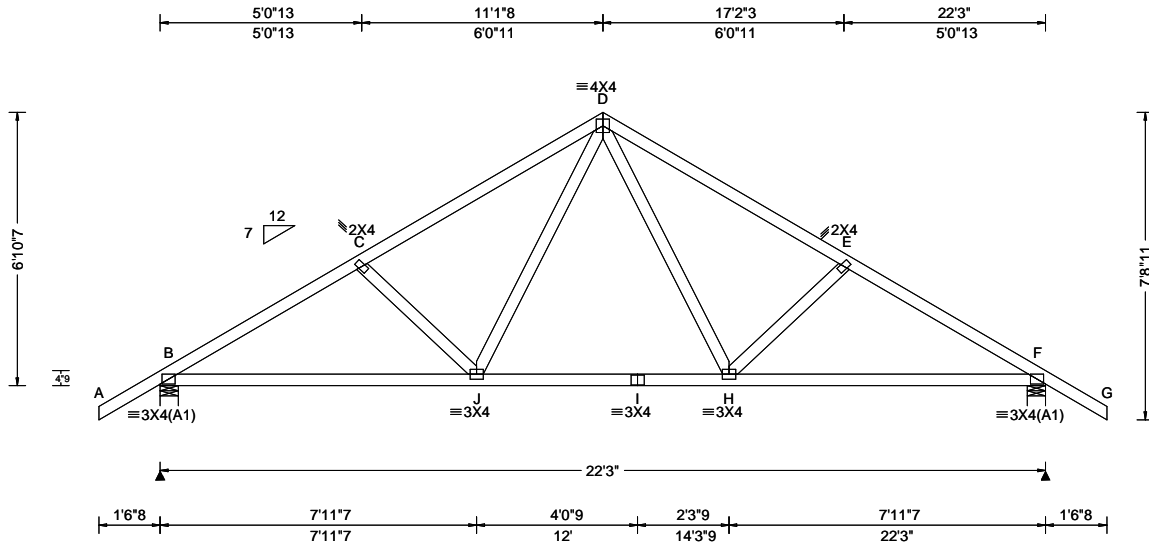
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

| | | | |
|---|------------|--------|-------------|
| Maximum Bot Chord Forces Per Ply (lbs) | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| B - J | 1155 -80 | I - H | 774 0 |
| J - I | 774 0 | H - F | 1155 -82 |
| Maximum Web Forces Per Ply (lbs) | | | |
| Webs | Tens.Comp. | Webs | Tens. Comp. |
| J - D | 412 -34 | D - H | 412 -34 |



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| | | | | |
|--|--|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.049 H 999 240 VERT(CL): 0.095 H 999 180 HORZ(LL): 0.022 F - - HORZ(TL): 0.043 F - - Creep Factor: 2.0 Max TC CSI: 0.397 Max BC CSI: 0.550 Max Web CSI: 0.169 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1079 /- /- /605 /- /181 F 1079 /- /- /605 /- /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 199 -1511 D - E 198 -1289 C - D 198 -1289 E - F 199 -1511 |
|--|--|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

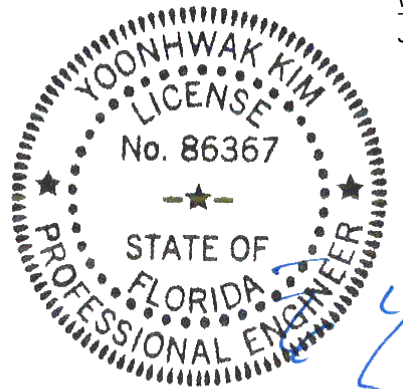
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - J | 1241 -78 | I - H | 840 0 |
| J - I | 840 0 | H - F | 1242 -85 |

Maximum Web Forces Per Ply (lbs)

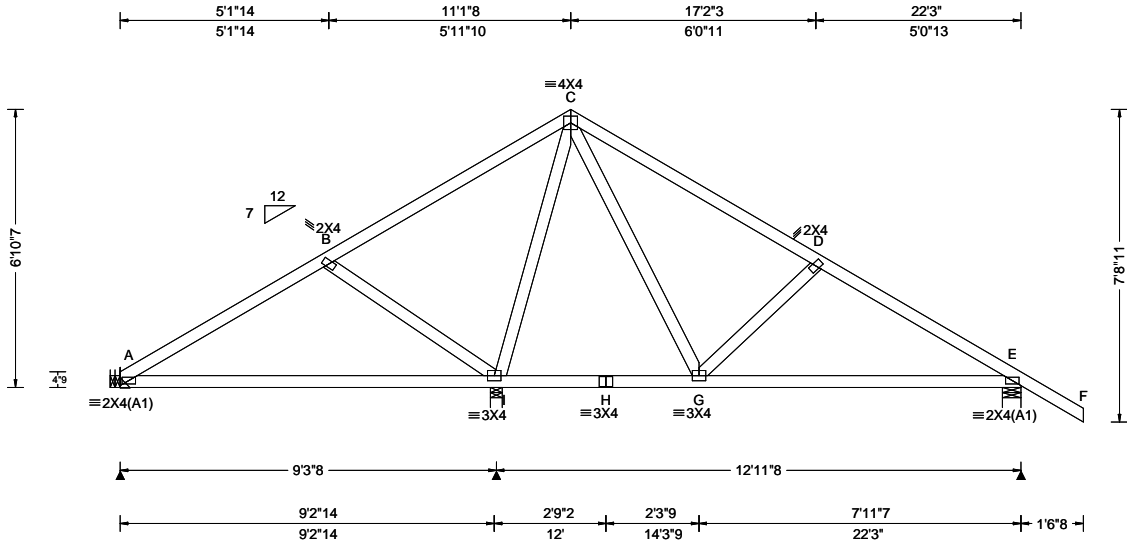
| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| J - D | 444 -33 | D - H | 444 -33 |



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| | | | | |
|--|---|---|---|---|
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|--|---|---|---|---|

Lumber

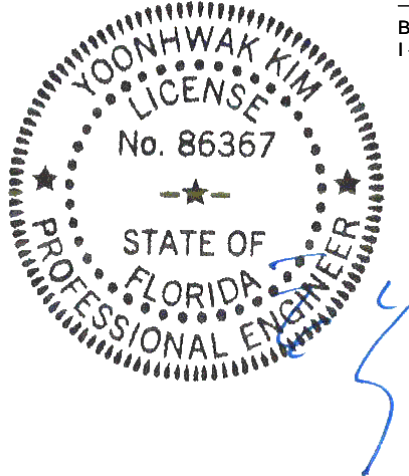
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

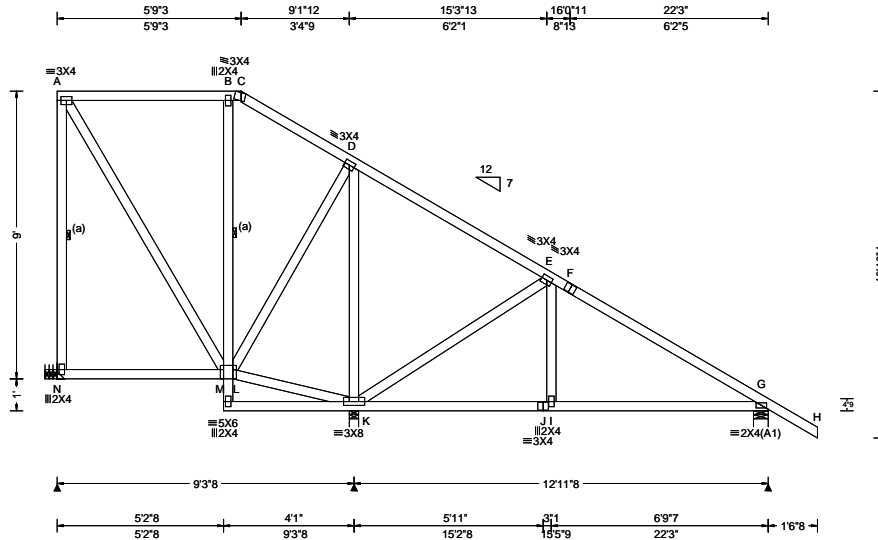
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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|--|--|---|--|---|
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|--|--|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

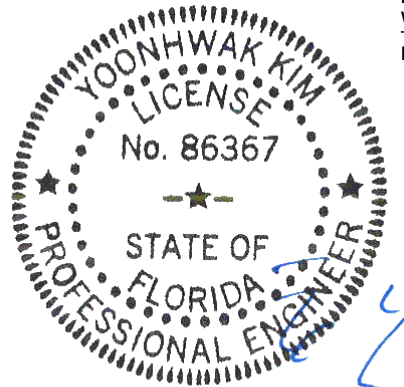
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

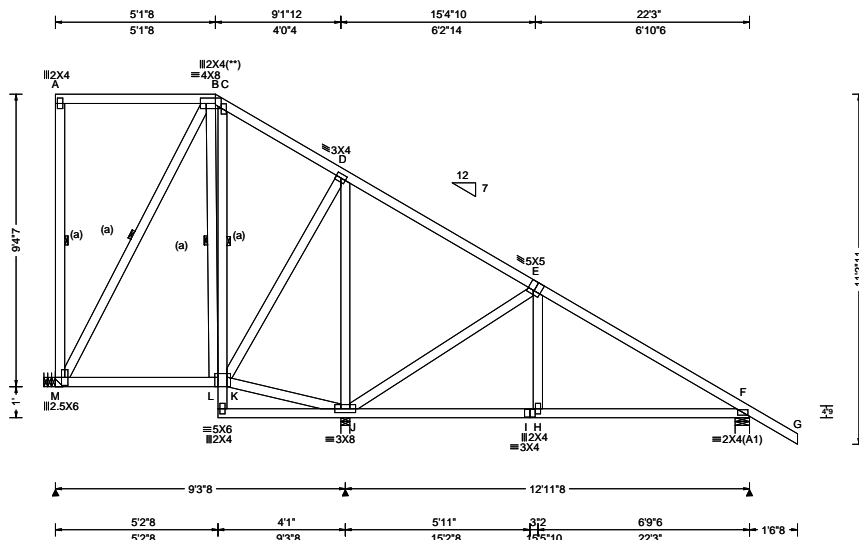
Refer to DWG PB160160118 for piggyback details.



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| | | | | |
|--|--|---|---|---|
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.007 H 999 240 VERT(CL): 0.015 H 999 180 HORZ(LL): 0.005 F - - HORZ(TL): 0.011 F - - Creep Factor: 2.0 Max TC CSI: 0.494 Max BC CSI: 0.379 Max Web CSI: 0.675 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity M 354 /- /- /187 /107 /233 J 1017 /- /- /638 /- /- F 612 /- /- /394 /- /- Wind reactions based on MWFRS M Brg Width = - Min Req = - J Brg Width = 3.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. |
|--|--|---|---|---|

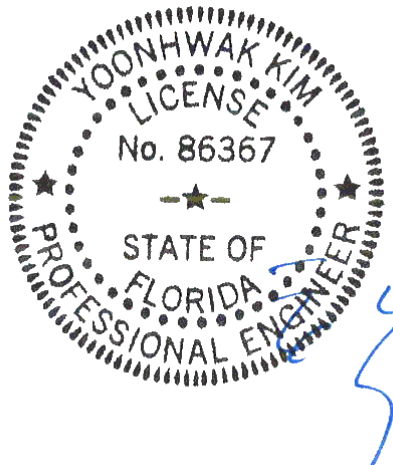
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

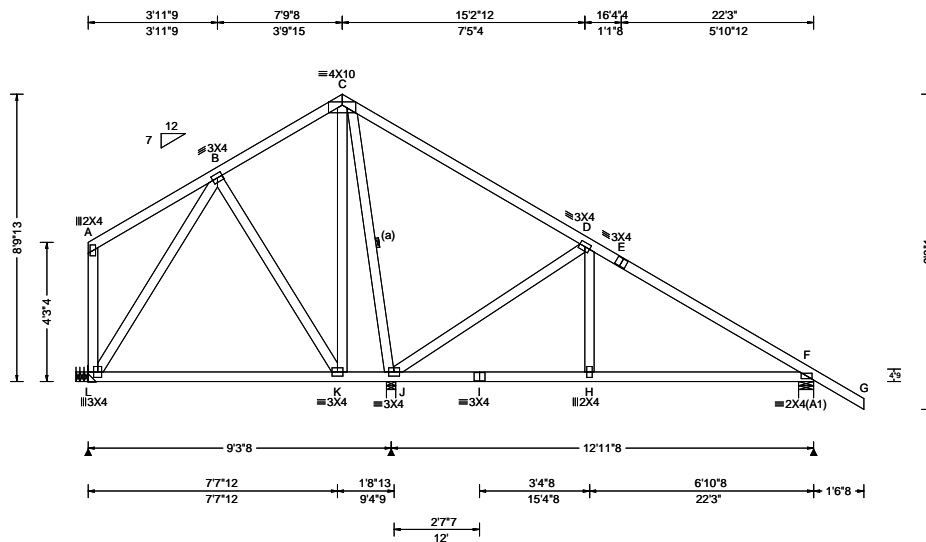
Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|--|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 H 999 240 VERT(CL): 0.018 H 999 180 HORZ(LL): 0.006 F - - HORZ(TL): 0.012 F - - Creep Factor: 2.0 Max TC CSI: 0.780 Max BC CSI: 0.535 Max Web CSI: 0.661 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity L 464 /- /- /194 /- /199 J 1008 /- /- /540 /- /- F 631 /- /- /399 /- /- Wind reactions based on MWFRS L Brg Width = - Min Req = - J Brg Width = 3.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings J & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 86 -451 E - F 80 -628 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. J - I 466 0 H - F 469 0 I - H 466 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. K - C 385 -11 J - D 181 -641 C - J 16 -674 |
|--|---|---|---|--|

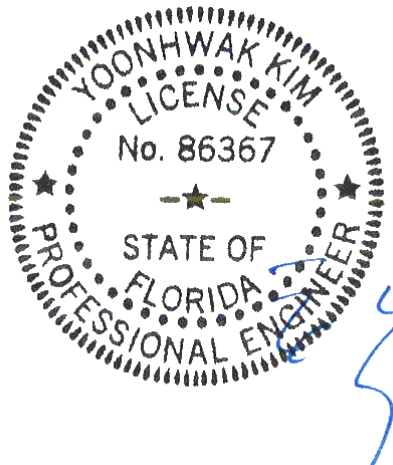
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

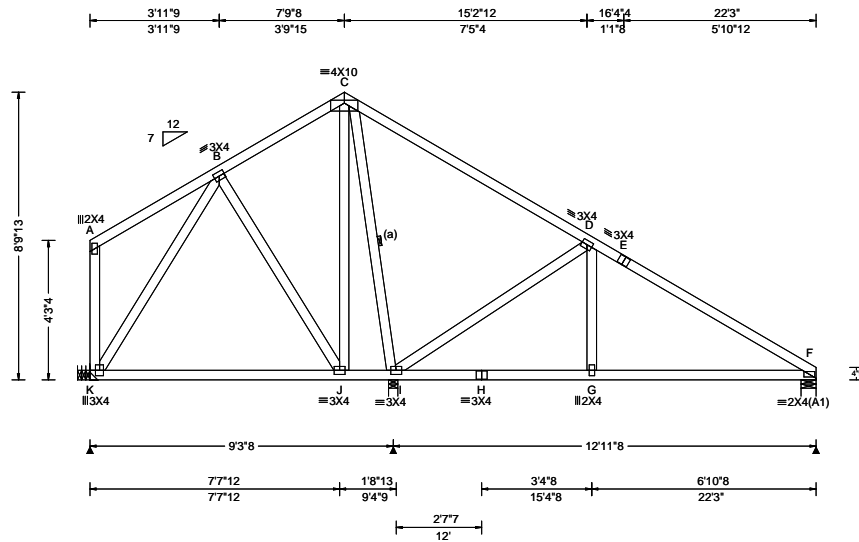
Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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09/16/2021

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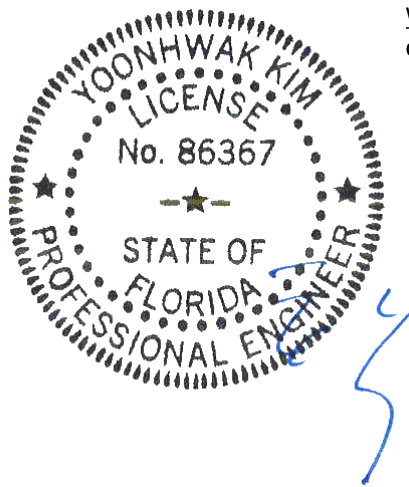
| | | | | |
|--|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.008 G 999 240 VERT(CL): 0.017 G 999 180 HORZ(LL): 0.006 F - - HORZ(TL): 0.012 F - - Creep Factor: 2.0 Max TC CSI: 0.789 Max BC CSI: 0.475 Max Web CSI: 0.681 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity K 393 - / - / 198 - / 185 I 965 - / - / 527 - / - F 522 - / - / 323 - / - Wind reactions based on MWFRS K Brg Width = - Min Req = - I Brg Width = 3.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings I & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 93 -470 E - F 87 -647 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. I - H 486 -9 G - F 489 -8 H - G 486 -9 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - I 9 -557 I - D 188 -660 |
|--|--|---|---|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

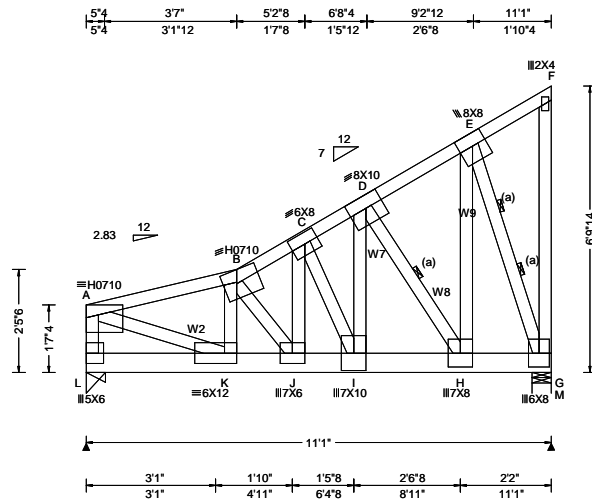


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09/16/2021

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2 Complete Trusses Required



| | | | | | | | | | |
|---|--|--|--|--|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): HS, WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.114 J 999 240 VERT(CL): 0.229 J 581 180 HORZ(LL): -0.063 F - - HORZ(TL): 0.126 F - - Creep Factor: 2.0 Max TC CSI: 0.919 Max BC CSI: 0.517 Max Web CSI: 0.925 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 9108 /- /- /- /206 /- M 8668 /- /- /- /349 /- Wind reactions based on MWFRS L Brg Width = 5.5 Min Req = 3.8 M Brg Width = 5.5 Min Req = 3.6 Bearings L & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 173 -6531 C - D 182 -5496 B - C 220 -6628 D - E 80 -2006 | | | | | |
| | | | | Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. K - J 6452 -180 I - H 4408 -146 J - I 5549 -180 H - G 1512 -57 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - L 116 -3902 C - I 64 -2048 A - K 6729 -174 I - D 6088 -167 K - B 110 -775 D - H 158 -5161 B - J 0 -1336 H - E 5023 -169 J - C 2429 -70 E - G 165 -4375 | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W2,W7,W9 2x4 SP M-31;
 W8 2x4 SP #2;

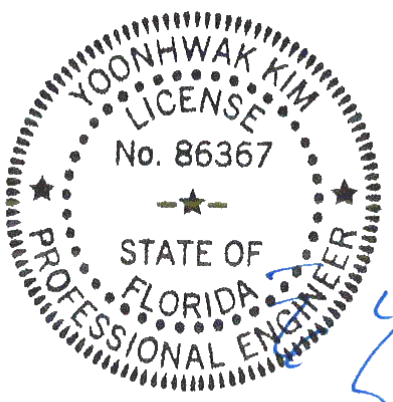
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 2.00" o.c. (Each Row)
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 61 plf at 0.00 to 61 plf at 3.59
 TC: From 63 plf at 3.59 to 63 plf at 11.08
 BC: From 20 plf at 0.00 to 20 plf at 11.08
 BC: 2224 lb Conc. Load at 1.52, 3.52
 BC: 2065 lb Conc. Load at 5.06
 BC: 2068 lb Conc. Load at 5.52
 BC: 6777 lb Conc. Load at 6.58
 BC: 511 lb Conc. Load at 7.06
 BC: 498 lb Conc. Load at 9.06
 BC: 496 lb Conc. Load at 10.40

Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The maximum concentrated load is 6778#
 The overall height of this truss excluding overhang is 6-9-14.



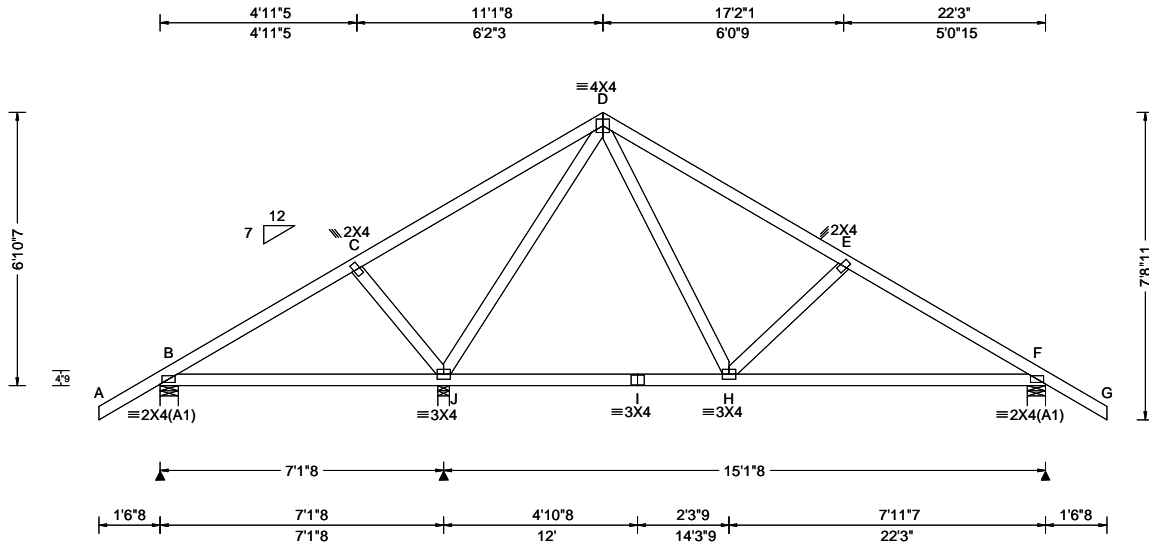
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| | | | | |
|---|---|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.018 E 999 240 VERT(CL): 0.036 E 999 180 HORZ(LL): 0.008 D - - HORZ(TL): 0.015 D - - Creep Factor: 2.0 Max TC CSI: 0.538 Max BC CSI: 0.504 Max Web CSI: 0.875 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 348 /- /- /215 /- /181 J 1151 /- /- /572 /- /- F 726 /- /- /460 /- /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 J Brg Width = 3.5 Min Req = 1.5 F Brg Width = 5.5 Min Req = 1.5 Bearings B, J, & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

| | | | | | |
|-------|-----|------|-------|-----|------|
| D - E | 101 | -627 | E - F | 106 | -864 |
|-------|-----|------|-------|-----|------|

Maximum Bot Chord Forces Per Ply (lbs)

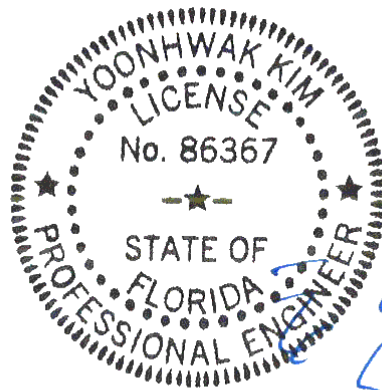
Chords Tens.Comp.

| | | |
|-------|-----|----|
| H - F | 694 | -9 |
|-------|-----|----|

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

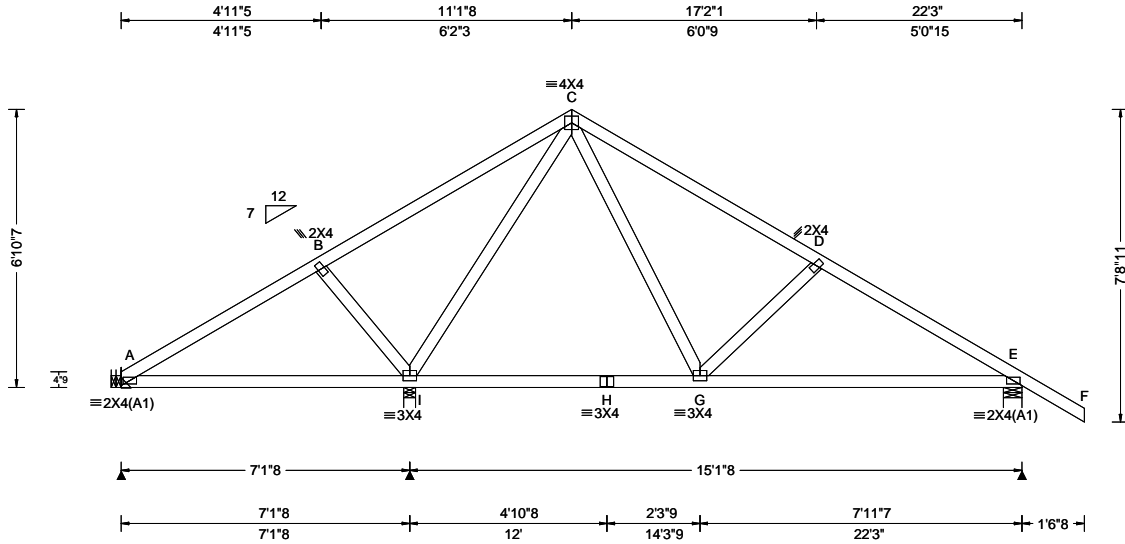
| | | | | | |
|-------|-----|------|-------|-----|-----|
| J - D | 124 | -760 | D - H | 522 | -46 |
|-------|-----|------|-------|-----|-----|



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| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.018 D 999 240 VERT(CL): 0.036 D 999 180 HORZ(LL): 0.008 C - - HORZ(TL): 0.016 C - - Creep Factor: 2.0 Max TC CSI: 0.601 Max BC CSI: 0.503 Max Web CSI: 0.828 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL A 249 /- /- /141 /- /167 I 1136 /- /- /560 /- /- E 730 /- /- /460 /- /- Non-Gravity Wind reactions based on MWFRS A Brg Width = - Min Req = - I Brg Width = 3.5 Min Req = 1.5 E Brg Width = 5.5 Min Req = 1.5 Bearings I & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

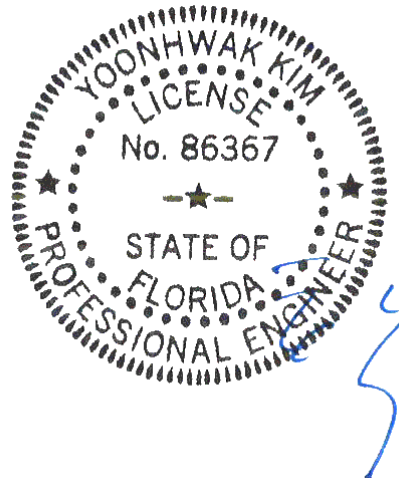
(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

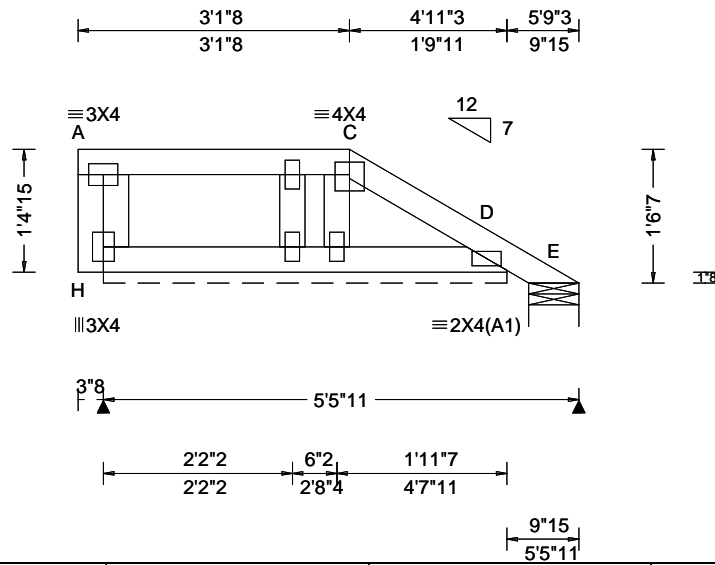
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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|--|--|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.77 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): -0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.084 Max BC CSI: 0.019 Max Web CSI: 0.069 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 77 /- /- /51 /21 /7 E 8 /- /- /2 /1 /- Wind reactions based on MWFRS H Brg Width = 55.7 Min Req = - E Brg Width = 6.9 Min Req = 1.5 Bearings H & E are a rigid surface. Members not listed have forces less than 375# |
|--|--|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

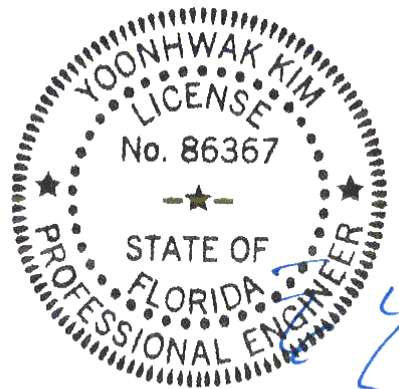
Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

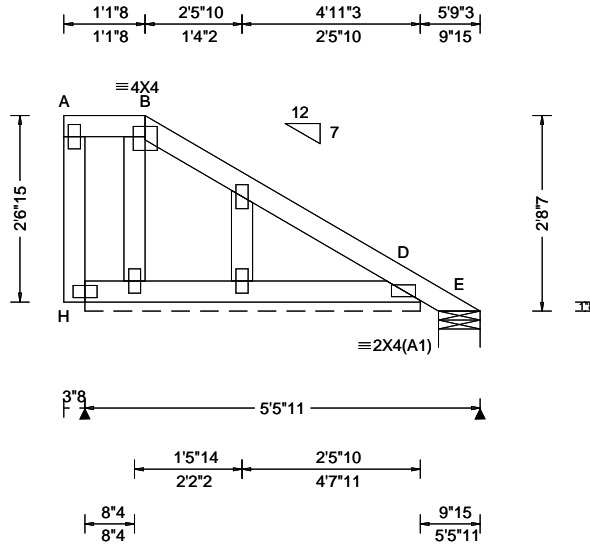
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FL REG# 278, Yoonhwak Kim, FL PE #86367
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|--|--|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.35 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.039 Max BC CSI: 0.020 Max Web CSI: 0.032 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 78 /- /- /59 /16 /13 E 0 /0 /- /3 /6 /- Wind reactions based on MWFRS H Brg Width = 55.7 Min Req = - E Brg Width = 6.9 Min Req = 1.5 Bearings H & E are a rigid surface. Members not listed have forces less than 375# |
|--|--|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

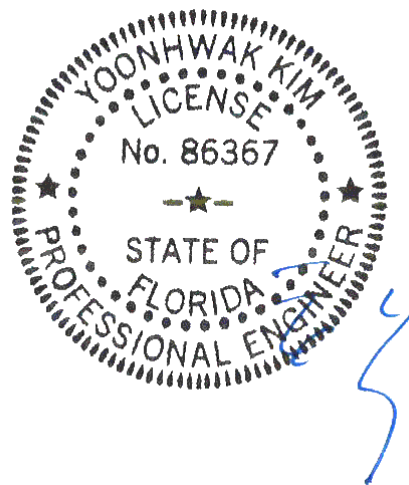
Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

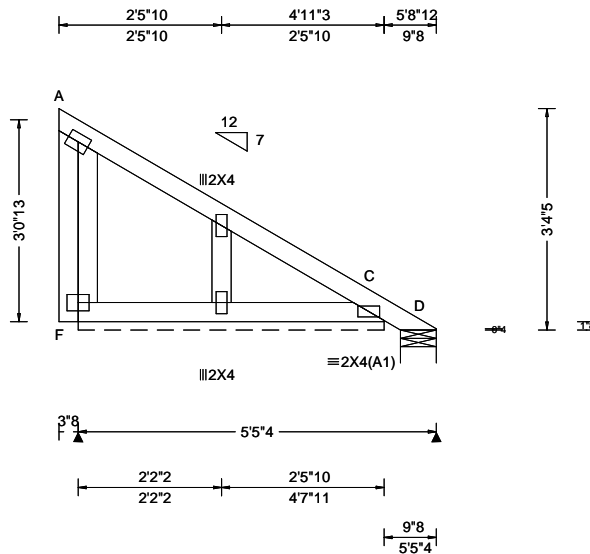
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|--|--|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.69 ft TC DL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.000 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): 0.000 A - - HORZ(TL): 0.000 A - - Creep Factor: 2.0 Max TC CSI: 0.073 Max BC CSI: 0.024 Max Web CSI: 0.058 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F* 77 /- /- /62 /12 /16 D 5 /- /- /2 /2 /- Wind reactions based on MWFRS F Brg Width = 55.7 Min Req = - D Brg Width = 6.5 Min Req = 1.5 Bearings F & D are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Loading

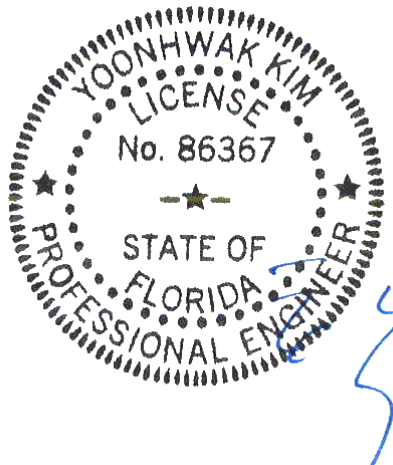
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

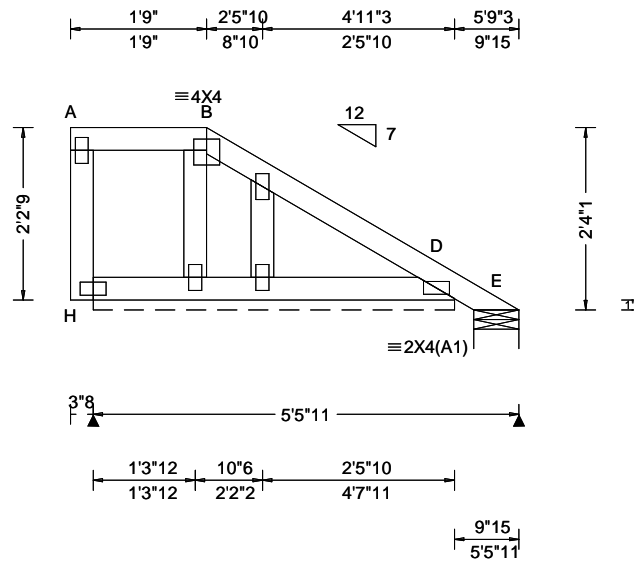
See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
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| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.91 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): -0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.039 Max BC CSI: 0.021 Max Web CSI: 0.019 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 79 /- /- /58 /13 /10 E 1 /- /- /5 /1 /- Wind reactions based on MWFRS H Brg Width = 55.7 Min Req = - E Brg Width = 6.9 Min Req = 1.5 Bearings H & E are a rigid surface. Members not listed have forces less than 375# |
|--|---|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

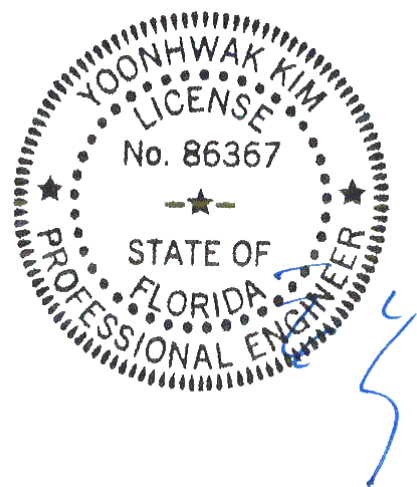
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

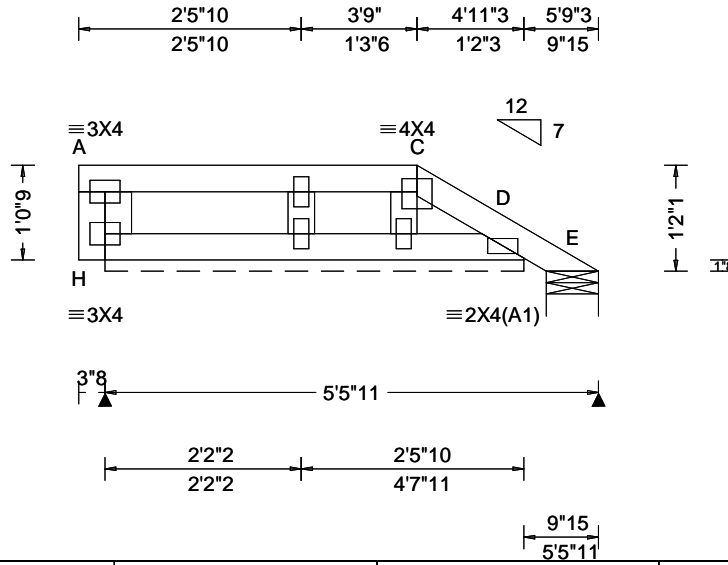
See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
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|---|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.33 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): -0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.068 Max BC CSI: 0.017 Max Web CSI: 0.029 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 75 /- /- /49 /16 /5 E 19 /- /- /16 /3 /- Wind reactions based on MWFRS H Brg Width = 55.7 Min Req = - E Brg Width = 6.9 Min Req = 1.5 Bearings H & E are a rigid surface. Members not listed have forces less than 375# |
|---|--|---|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

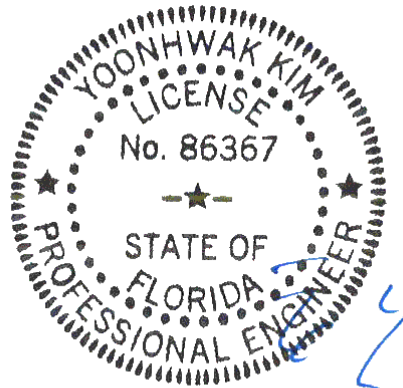
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

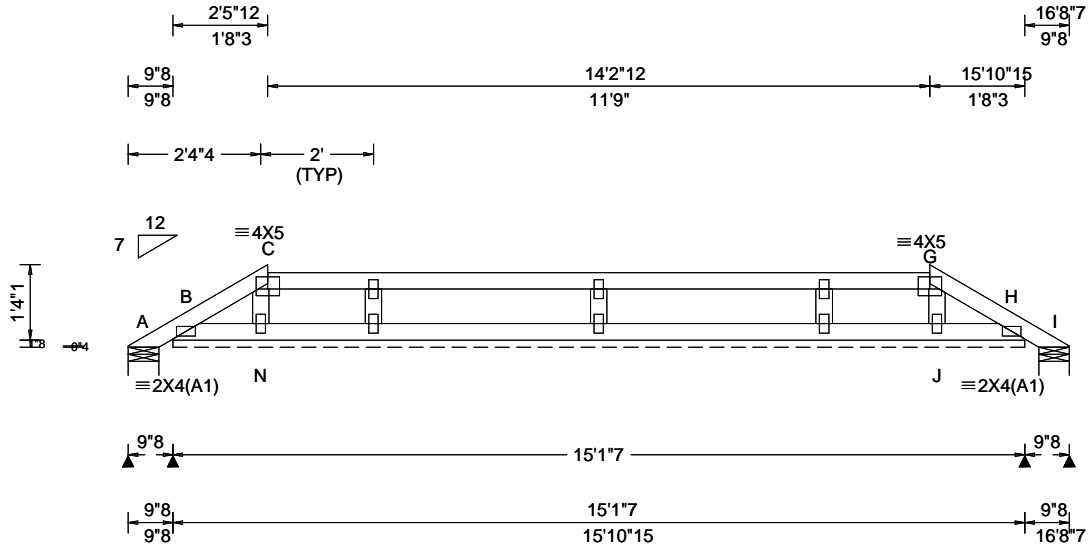
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|---|---|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.74 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.000 E 999 240 VERT(CL): 0.000 E 999 180 HORZ(LL): 0.000 H - - HORZ(TL): 0.000 H - - Creep Factor: 2.0 Max TC CSI: 0.181 Max BC CSI: 0.048 Max Web CSI: 0.051 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 12 /- /- /23 /13 /30 B* 70 /- /- /45 /13 /- I 12 /- /- /12 /1 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

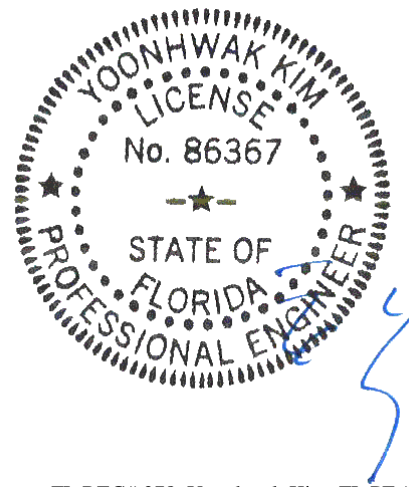
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

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Wind loading based on both gable and hip roof types.

Additional Notes

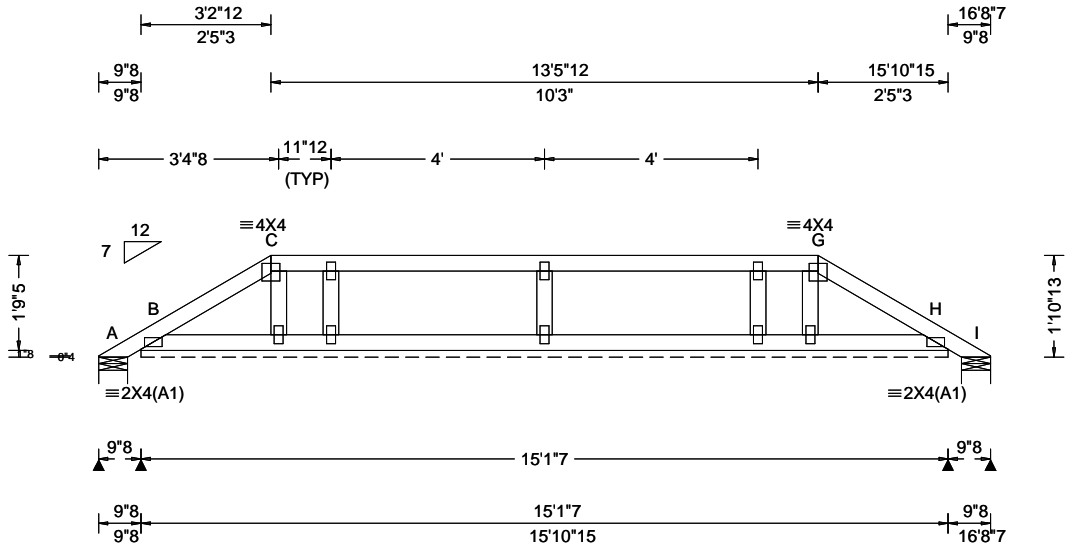
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| | | | | |
|---|--|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.96 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.001 H 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.182 Max BC CSI: 0.049 Max Web CSI: 0.052 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-6 /- /25 /25 /41 B* 73 /- /- /47 /11 /- I - /-6 /- /4 /4 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

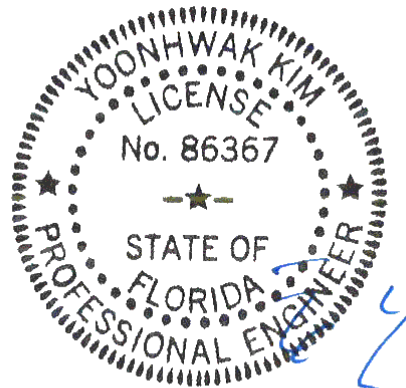
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

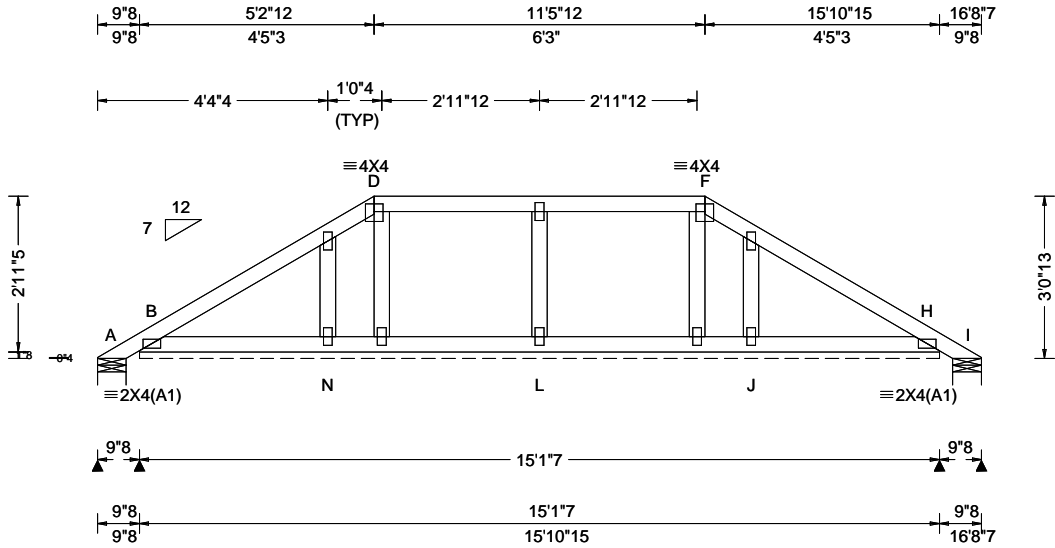
See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
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FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|---|--|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.55 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.40 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.001 H - - Creep Factor: 2.0 Max TC CSI: 0.139 Max BC CSI: 0.040 Max Web CSI: 0.075 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-34 /- /46 /67 /69 B* 76 /- /- /49 /14 /- I - /-34 /- /12 /33 /- N /-100 L /-237 J /-101 Non-Gravity Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

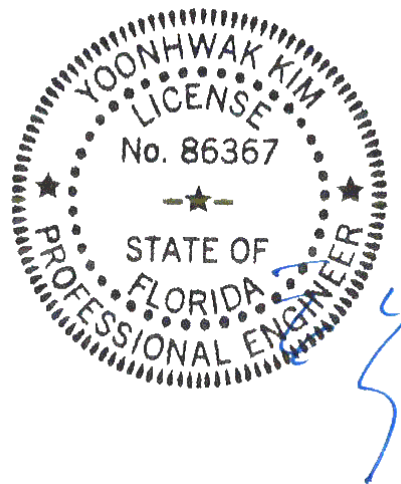
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

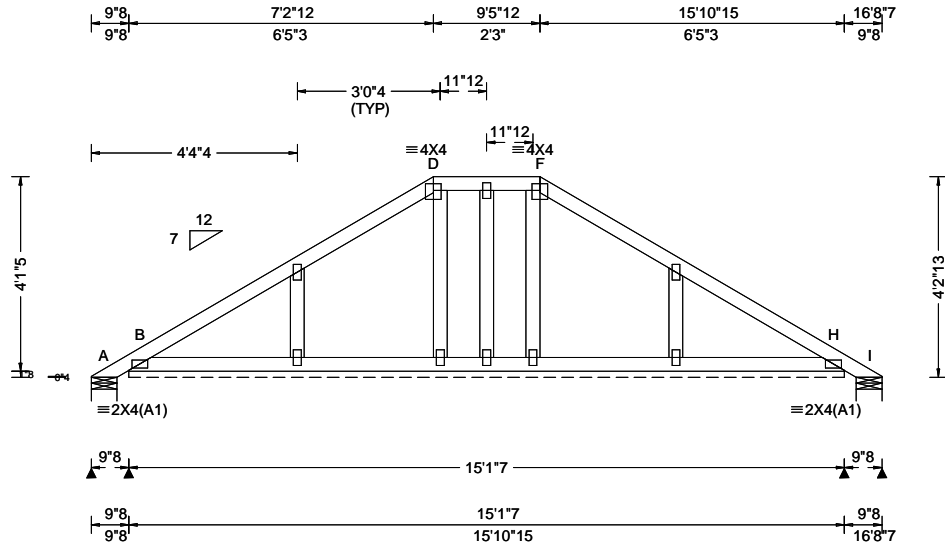
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| | | | | |
|---|--|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.13 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.001 H 999 180 HORZ(LL): 0.001 G - - - HORZ(TL): 0.001 H - - - Creep Factor: 2.0 Max TC CSI: 0.122 Max BC CSI: 0.048 Max Web CSI: 0.044 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-24 /- /59 /70 /97 B* 75 /- /- /52 /6 /- I - /-24 /- /11 /22 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

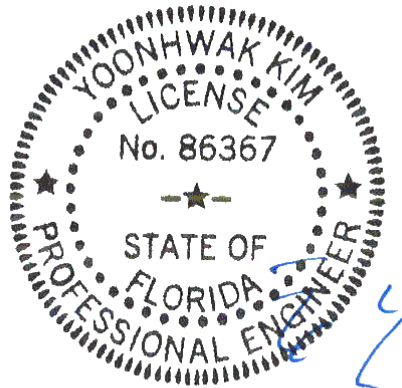
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

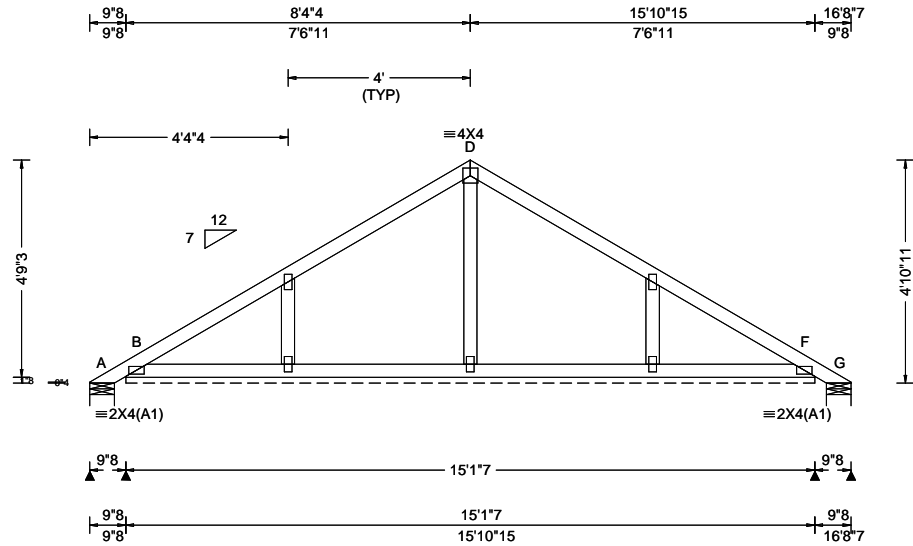
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| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.46 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.001 E - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.204 Max BC CSI: 0.056 Max Web CSI: 0.069 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-9 /- /64 /68 /112 B* 73 /- /- /52 /5 /- G - /-9 /- /8 /12 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - G Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

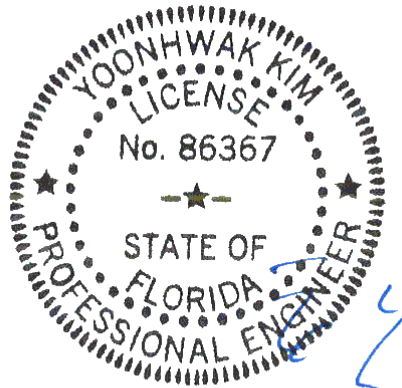
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

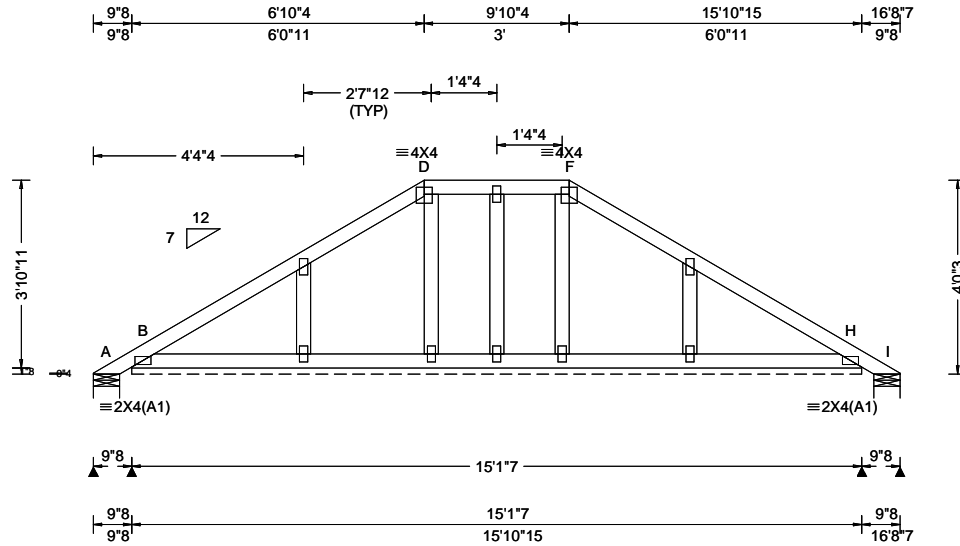
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|---|--|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.02 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.001 H 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.001 H - - Creep Factor: 2.0 Max TC CSI: 0.107 Max BC CSI: 0.045 Max Web CSI: 0.041 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-27 /- /58 /71 /93 B* 76 /- /- /52 /6 /- I - /-27 /- /12 /25 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

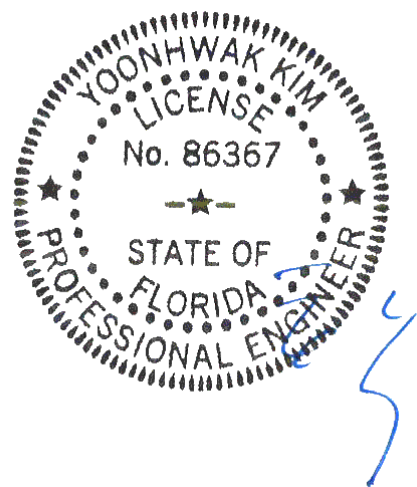
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

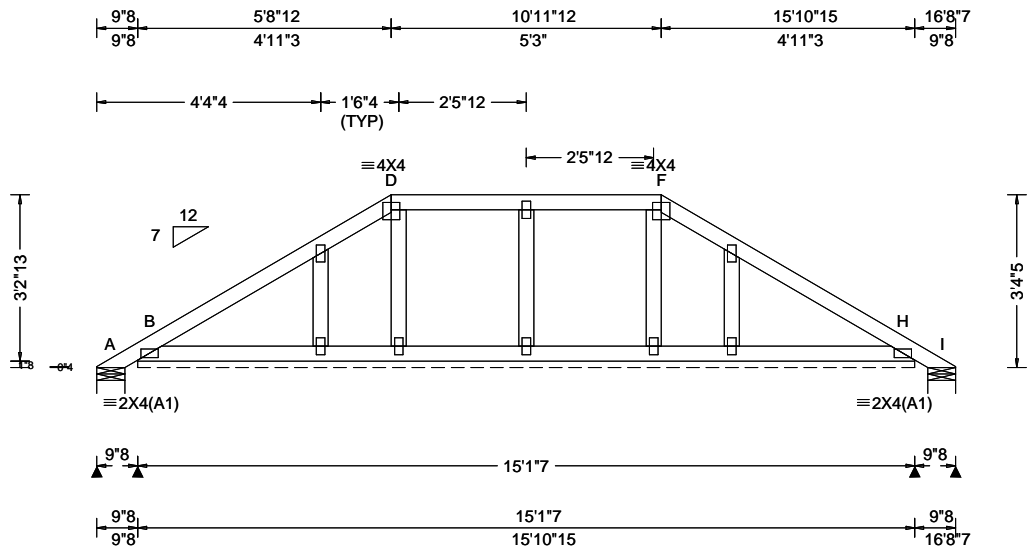
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|---|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.69 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.002 H 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.001 H - - Creep Factor: 2.0 Max TC CSI: 0.095 Max BC CSI: 0.040 Max Web CSI: 0.048 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-32 /- /51 /66 /77 B* 76 /- /- /51 /6 /- I - /-32 /- /12 /28 /- Wind reactions based on MWFRS A Brg Width = 6.5 Min Req = 1.5 B Brg Width = 181 Min Req = - I Brg Width = 6.5 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# |
|---|---|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

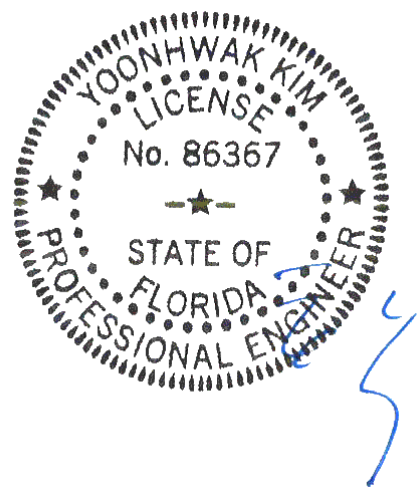
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Wind

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Wind loading based on both gable and hip roof types.

Additional Notes

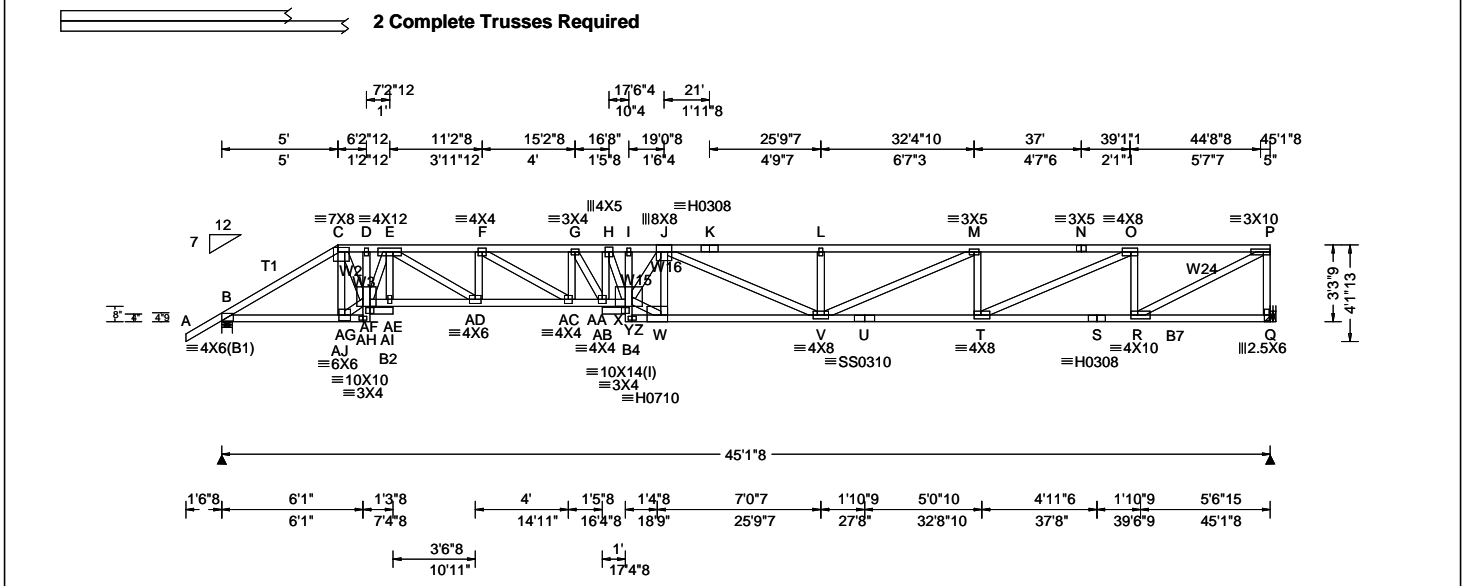
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| Loading Criteria (psf) | |
|------------------------|--------|
| TCLL: | 20.00 |
| TCDL: | 10.00 |
| BCLL: | 0.00 |
| BCDL: | 10.00 |
| Des Ld: | 40.00 |
| NCBCLL: | 0.00 |
| Soffit: | 2.00 |
| Load Duration: | 1.25 |
| Spacing: | 24.0 " |

| Wind Criteria | |
|----------------------|-----------|
| Wind Std: | ASCE 7-16 |
| Speed: | 120 mph |
| Enclosure: | Closed |
| Risk Category: | II |
| EXP: | C Kzt: NA |
| Mean Height: | 15.00 ft |
| TCDL: | 5.0 psf |
| BCDL: | 5.0 psf |
| MWFRS Parallel Dist: | 0 to h/2 |
| C&C Dist a: | 4.51 ft |
| Loc. from endwall: | Any |
| GCpi: | 0.18 |
| Wind Duration: | 1.60 |

| Snow Criteria (Pg,Pf in PSF) | |
|------------------------------|----------------------------|
| Pg: | NA Ct: NA CAT: NA |
| Pf: | NA Ce: NA |
| Lu: | NA Cs: NA |
| Snow Duration: | NA |
| Building Code: | FBC 7th Ed. 2020 Res. HVHZ |
| TPI Std: | 2014 |
| Rep Fac: | No |
| FT/RT: | 20(0)/10(0) |
| Plate Type(s): | WAVE, HS, 18SS |

| Defl/CSI Criteria | |
|---------------------------------|-------------------|
| PP Deflection in loc L/defl L/# | |
| VERT(LL): | 0.842 AB 640 240 |
| VERT(CL): | 1.699 AB 317 180 |
| HORZ(LL): | 0.171 C - - |
| HORZ(TL): | 0.345 C - - |
| Creep Factor: | 2.0 |
| Max TC CSI: | 0.697 |
| Max BC CSI: | 0.896 |
| Max Web CSI: | 0.903 |
| VIEW Ver: | 21.01.01A.0521.20 |

| ▲ Maximum Reactions (lbs) | | | | | |
|---|-----------------|--------|---------------|-----|--------|
| Gravity | | | Non-Gravity | | |
| Loc | R+ | /R- | /Rh | /Rw | /U /RL |
| B | 3459 | - | - | - | /535 - |
| Q | 3519 | - | - | - | /555 - |
| Wind reactions based on MWFRS | | | | | |
| B | Brg Width = 5.5 | | Min Req = 1.5 | | |
| Q | Brg Width = - | | Min Req = - | | |
| Bearing B is a rigid surface. | | | | | |
| Members not listed have forces less than 375# | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | |
| B - C | 459 -2957 | I - J | 1258 -8261 | | |
| C - D | 632 -4089 | J - K | 933 -6356 | | |
| D - E | 640 -4143 | K - L | 933 -6356 | | |
| E - F | 924 -5904 | L - M | 933 -6356 | | |
| F - G | 1104 -7140 | M - N | 785 -5194 | | |
| G - H | 1148 -7476 | N - O | 785 -5194 | | |
| H - I | 1269 -8335 | O - P | 450 -2917 | | |

Lumber

Top chord: 2x4 SP M-31; T1 2x4 SP #2;
 Bot chord: 2x4 SP M-31; B2,B4,B7 2x4 SP #2;
 Webs: 2x4 SP #3; W2,W3,W16,W24 2x4 SP #2;
 W15 2x4 SP M-31;

Nailnote

Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 5.00
 TC: From 32 plf at 5.00 to 32 plf at 45.12
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 5.03
 BC: From 10 plf at 5.03 to 10 plf at 45.12
 TC: 129 lb Conc. Load at 5.06,31.06,33.06,35.06
 37.06,39.06,41.06,43.06,44.56
 TC: 76 lb Conc. Load at 7.06,17.06
 TC: 48 lb Conc. Load at 9.06,11.06,13.06,15.06
 TC: 140 lb Conc. Load at 19.06,21.06,23.06,25.06
 27.06,29.06
 BC: 388 lb Conc. Load at 5.03
 BC: 115 lb Conc. Load at 7.06,17.06
 BC: 133 lb Conc. Load at 9.06,11.06,13.06,15.06
 BC: 94 lb Conc. Load at 19.06,21.06,23.06,25.06
 27.06,29.06
 BC: 90 lb Conc. Load at 31.06,33.06,35.06,37.06
 39.06,41.06,43.06,44.56

Plating Notes

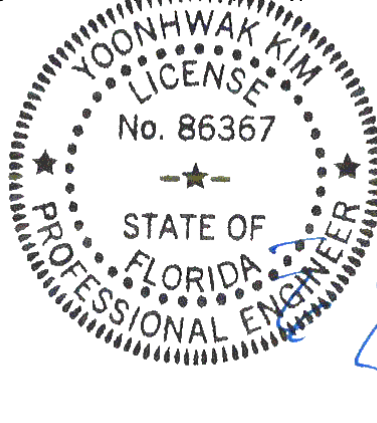
All plates are 2X4 except as noted.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

| Maximum Bot Chord Forces Per Ply (lbs) | | | |
|--|------------|--------|-------------|
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| B -AJ | 2499 -383 | W - V | 6277 -945 |
| AG-AF | 4437 -694 | V - U | 5273 -803 |
| AF-AD | 4384 -685 | U - T | 5273 -803 |
| AD-AC | 6006 -942 | T - S | 3047 -478 |
| AC-AA | 7218 -1116 | S - R | 3047 -478 |
| AA -X | 7802 -1196 | | |

| Maximum Web Forces Per Ply (lbs) | | | |
|----------------------------------|------------|-------|-------------|
| Webs | Tens.Comp. | Webs | Tens. Comp. |
| C -AJ | 330 -1836 | H - X | 1727 -242 |
| C -AG | 3575 -568 | X - W | 6970 -1039 |
| AJ-AG | 2956 -437 | X - J | 3724 -591 |
| AG -E | 107 -577 | W - J | 589 -3511 |
| E -AF | 106 -634 | V - M | 1190 -142 |
| E -AD | 1800 -284 | M - T | 185 -815 |
| AD -F | 143 -828 | T - O | 2369 -339 |
| F -AC | 1340 -192 | O - R | 281 -1349 |
| AC -G | 99 -631 | R - P | 3281 -507 |
| G -AA | 579 -71 | P - Q | 294 -1675 |
| AA -H | 258 -1783 | | |

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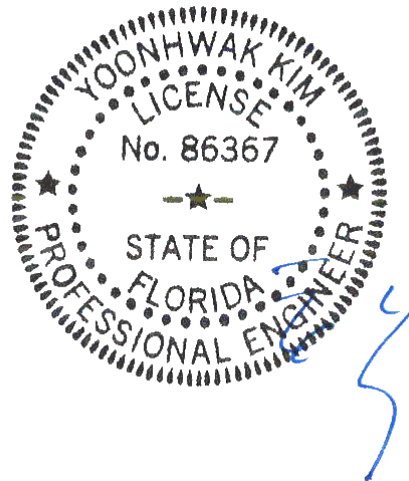


| | | | | | | |
|----------------|------|--------|---------------------|--------------------------|-------------------|-------|
| SEQN: 396635 / | HIPM | Ply: 2 | Job Number: 21-5856 | Cust: R215 | JRef: 1X8V2150010 | T28 / |
| FROM: | | Qty: 1 | Shelley | DrwNo: 259.21.1157.38264 | | |
| Page 2 of 2 | | | Truss Label: D01 | / YK | 09/16/2021 | |

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 3-3-9.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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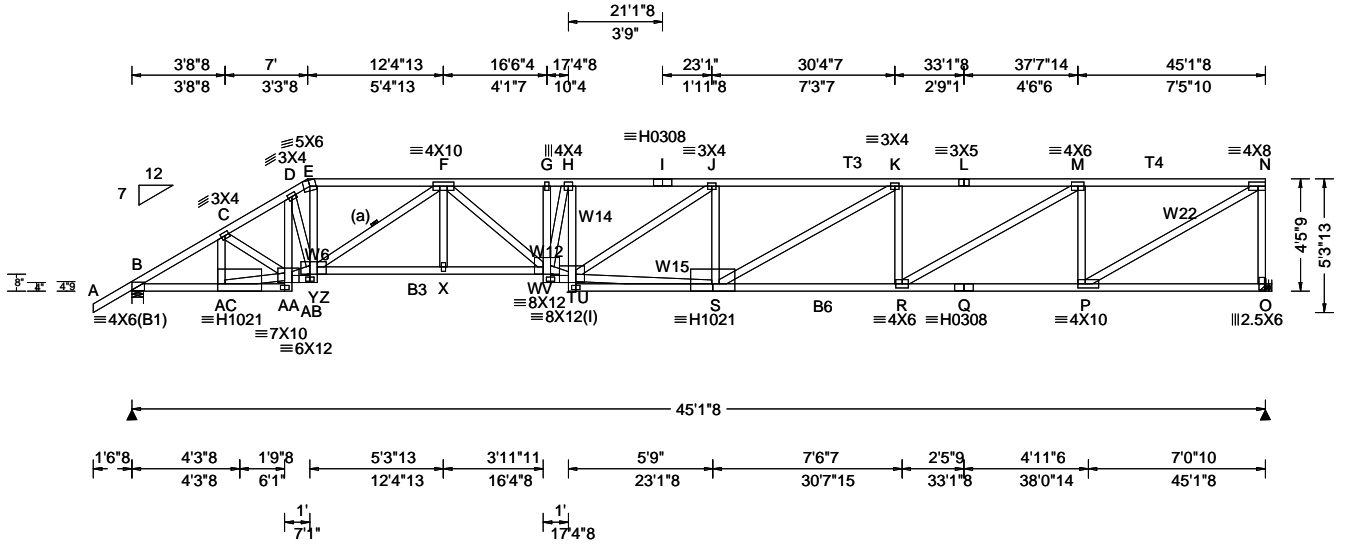
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6750 Forum Drive
Suite 305
Orlando FL, 32821



| Loading Criteria (psf) | |
|------------------------|-------|
| TCLL: | 20.00 |
| TCDL: | 10.00 |
| BCLL: | 0.00 |
| BCDL: | 10.00 |
| Des Ld: | 40.00 |
| NCBCLL: | 10.00 |
| Soffit: | 2.00 |
| Load Duration: | 1.25 |
| Spacing: | 24.0" |

| Wind Criteria | |
|----------------------|----------------|
| Wind Std: | ASCE 7-16 |
| Speed: | 120 mph |
| Enclosure: | Closed |
| Risk Category: | II |
| EXP: | C Kzt: NA |
| Mean Height: | 15.00 ft |
| TCDL: | 5.0 psf |
| BCDL: | 5.0 psf |
| MWFRS Parallel Dist: | h/2 to h |
| C&C Dist a: | 4.51 ft |
| Loc. from endwall: | not in 6.50 ft |
| GCpi: | 0.18 |
| Wind Duration: | 1.60 |

| Snow Criteria (Pg, Pf in PSF) | |
|-------------------------------|----------------------------|
| Pg: | NA Ct: NA CAT: NA |
| Pf: | NA Ce: NA |
| Lu: | NA Cs: NA |
| Snow Duration: | NA |
| Building Code: | FBC 7th Ed. 2020 Res. HVHZ |
| TPI Std: | 2014 |
| Rep Fac: | Yes |
| FT/RT: | 20(0)/10(0) |
| Plate Type(s): | WAVE, HS |

| Defl/CSI Criteria | |
|---------------------------------|-------------------|
| PP Deflection in loc L/defl L/# | |
| VERT(LL): | 0.477 H 999 240 |
| VERT(CL): | 0.986 H 547 180 |
| HORZ(LL): | 0.130 E - - |
| HORZ(TL): | 0.269 E - - |
| Creep Factor: | 2.0 |
| Max TC CSI: | 0.710 |
| Max BC CSI: | 0.934 |
| Max Web CSI: | 0.969 |
| VIEW Ver: | 21.01.01A.0521.20 |

| ▲ Maximum Reactions (lbs) | | | | | |
|---|-----------------|--------|---------------|-------|-----------|
| Gravity | | | Non-Gravity | | |
| Loc | R+ | /R- | /Rh | /Rw | /U /RL |
| B | 1989 | - | - | /1087 | /241 /145 |
| O | 1868 | - | - | /922 | /236 - |
| Wind reactions based on MWFRS | | | | | |
| B | Brg Width = 5.5 | | Min Req = 2.3 | | |
| O | Brg Width = - | | Min Req = - | | |
| Bearing B is a rigid surface. | | | | | |
| Members not listed have forces less than 375# | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | |
| B - C | 741 -3164 | H - I | 1391 -5378 | | |
| C - D | 928 -3557 | I - J | 1391 -5378 | | |
| D - E | 1001 -3696 | J - K | 1249 -5086 | | |
| E - F | 922 -3339 | K - L | 1054 -4451 | | |
| F - G | 1518 -5815 | L - M | 1054 -4451 | | |
| G - H | 1512 -5791 | M - N | 710 -2785 | | |

Lumber

Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
 Bot chord: 2x4 SP #2; B3,B6 2x4 SP M-31;
 Webs: 2x4 SP #3; W6,W14,W22 2x4 SP #2; W12, W15 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.
 (l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

| Maximum Bot Chord Forces Per Ply (lbs) | | | |
|--|------------|--------|-------------|
| Chords | Tens.Comp. | Chords | Tens. Comp. |
| B - AC | 2651 -712 | S - R | 4506 -1075 |
| Y - X | 4783 -1297 | R - Q | 2887 -744 |
| X - V | 4784 -1297 | Q - P | 2887 -744 |
| U - S | 444 -103 | | |

| Maximum Web Forces Per Ply (lbs) | | | |
|----------------------------------|------------|-------|-------------|
| Webs | Tens.Comp. | Webs | Tens. Comp. |
| C - AC | 212 -580 | V - T | 5580 -1439 |
| C - AA | 488 -160 | H - T | 476 -1529 |
| AC - AA | 2511 -673 | T - S | 4681 -1161 |
| AA - D | 204 -642 | J - S | 277 -613 |
| AA - Y | 3123 -849 | S - K | 672 -202 |
| D - Y | 573 -213 | K - R | 299 -789 |
| E - Y | 1559 -339 | R - M | 1821 -440 |
| Y - F | 457 -1710 | M - P | 495 -1463 |
| F - V | 1333 -313 | P - N | 3222 -821 |
| V - H | 1257 -370 | N - O | 536 -1809 |

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| | | | |
|--------------------------------------|--------------------------|--|--|
| SEQN: 9751 / FROM: Page 2 of 2 | HIPM Ply: 1 Qty: 1 | Job Number: 21-5856 Shelley Truss Label: D02 | Cust: R215 JRef:1X8V2150010 T96 / DrwNo: 259.21.1157.37187 / YK 09/16/2021 |
|--------------------------------------|--------------------------|--|--|

Hangers / Ties

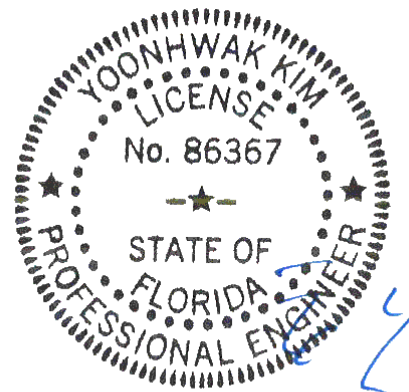
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=44'10"8 uses the following support conditions: 44'10"8

Bearing O (44'10"8, 10') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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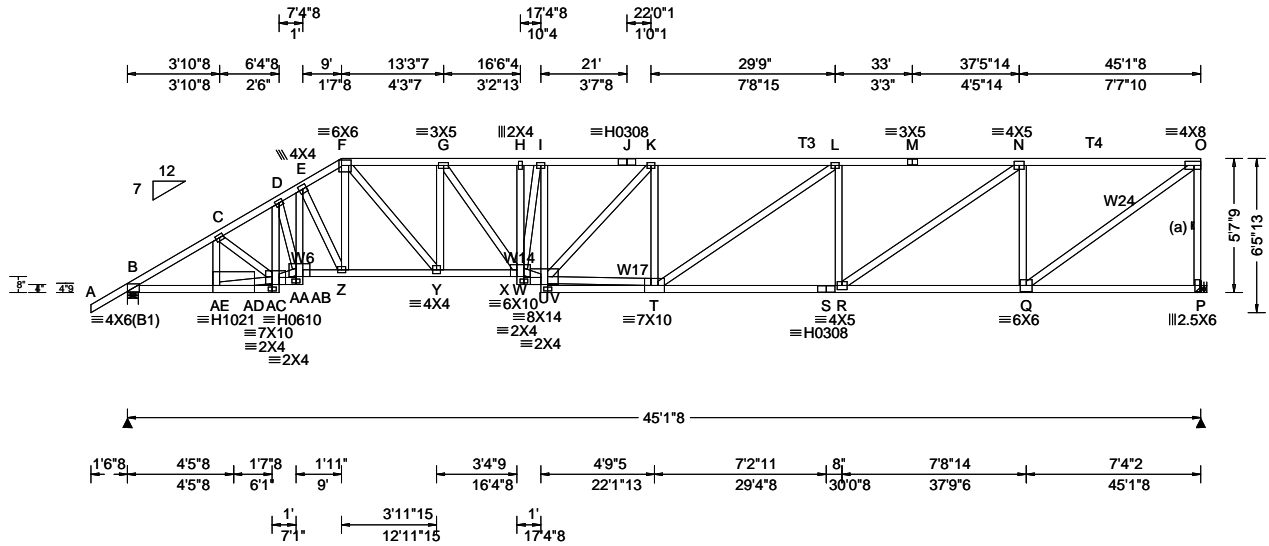
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6750 Forum Drive
Suite 305
Orlando FL, 32821



| Loading Criteria (psf) | |
|------------------------|--------|
| TCCL: | 20.00 |
| TCDL: | 10.00 |
| BCLL: | 0.00 |
| BCDL: | 10.00 |
| Des Ld: | 40.00 |
| NCBCLL: | 10.00 |
| Soffit: | 2.00 |
| Load Duration: | 1.25 |
| Spacing: | 24.0 " |

| Wind Criteria | |
|----------------------|----------------|
| Wind Std: | ASCE 7-16 |
| Speed: | 120 mph |
| Enclosure: | Closed |
| Risk Category: | II |
| EXP: | C Kzt: NA |
| Mean Height: | 15.00 ft |
| TCDL: | 5.0 psf |
| BCDL: | 5.0 psf |
| MWFRS Parallel Dist: | h/2 to h |
| C&C Dist a: | 4.51 ft |
| Loc. from endwall: | not in 6.50 ft |
| GCp: | 0.18 |
| Wind Duration: | 1.60 |

| Snow Criteria (Pg,Pf in PSF) | |
|------------------------------|-------------------|
| Pg: | NA Ct: NA CAT: NA |
| Pf: | NA Ce: NA |
| Lu: | NA Cs: NA |
| Snow Duration: | NA |
| Building Code: | |
| FBC 7th Ed. 2020 Res. HVHZ | |
| TPI Std: 2014 | |
| Rep Fac: Yes | |
| FT/RT:20(0)/10(0) | |
| Plate Type(s): | |
| WAVE, HS | |

| Defl/CSI Criteria | |
|---------------------------------|-------------------|
| PP Deflection in loc L/defl L/# | |
| VERT(LL): | 0.343 I 999 240 |
| VERT(CL): | 0.708 I 762 180 |
| HORZ(LL): | 0.115 Q - - |
| HORZ(TL): | 0.237 Q - - |
| Creep Factor: | 2.0 |
| Max TC CSI: | 0.717 |
| Max BC CSI: | 0.973 |
| Max Web CSI: | 0.992 |
| VIEW Ver: | 21.01.01A.0521.20 |

| ▲ Maximum Reactions (lbs) | | | | | |
|---|-----------------|-------|---------------|-------------|-------|
| Gravity | | | Non-Gravity | | |
| Loc | R+ / R- | / Rh | / Rw | / U | / RL |
| B | 1989 | - / - | / 1110 | / 235 | / 179 |
| P | 1868 | - / - | / 929 | / 239 | - / - |
| Wind reactions based on MWFRS | | | | | |
| B | Brg Width = 5.5 | | Min Req = 2.3 | | |
| P | Brg Width = - | | Min Req = - | | |
| Bearing B is a rigid surface. | | | | | |
| Members not listed have forces less than 375# | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) | | | | | |
| Chords | Tens.Comp. | | Chords | Tens. Comp. | |
| B - C | 705 | -3167 | I - J | 1089 | -4085 |
| C - D | 882 | -3530 | J - K | 1089 | -4085 |
| D - E | 987 | -3758 | K - L | 1010 | -3977 |
| E - F | 889 | -3314 | L - M | 864 | -3565 |
| F - G | 1008 | -3671 | M - N | 864 | -3565 |
| G - H | 1159 | -4307 | N - O | 566 | -2223 |
| H - I | 1155 | -4293 | | | |

Lumber

Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W6,W14,W17,W24 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

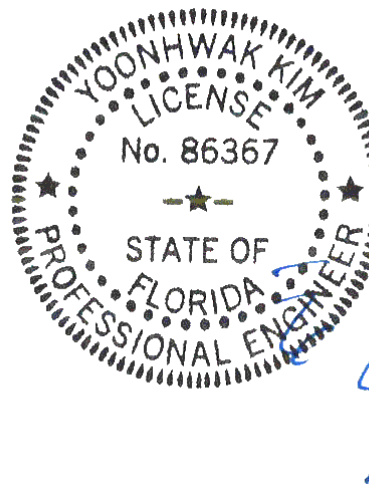
All plates are 3X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

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FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

| Maximum Bot Chord Forces Per Ply (lbs) | | | | | |
|--|------------|-------|--------|-------------|------|
| Chords | Tens.Comp. | | Chords | Tens. Comp. | |
| B - AE | 2653 | -715 | T - S | 3606 | -881 |
| AA - Z | 3233 | -890 | S - R | 3606 | -881 |
| Z - Y | 2857 | -789 | R - Q | 2303 | -593 |
| Y - W | 3742 | -1032 | | | |

| Maximum Web Forces Per Ply (lbs) | | | | | |
|----------------------------------|------------|-------|-------|-------------|-------|
| Webs | Tens.Comp. | | Webs | Tens. Comp. | |
| C - AE | 214 | -589 | W - I | 847 | -269 |
| C - AC | 482 | -150 | W - U | 4208 | -1117 |
| AE - AC | 2515 | -677 | I - U | 353 | -1027 |
| AC - D | 239 | -763 | U - T | 3714 | -947 |
| AC - AA | 3134 | -860 | K - T | 265 | -526 |
| D - AA | 507 | -149 | T - L | 454 | -158 |
| AA - E | 969 | -253 | L - R | 305 | -750 |
| E - Z | 223 | -843 | R - N | 1554 | -388 |
| F - Z | 479 | -85 | N - Q | 497 | -1452 |
| F - Y | 1250 | -350 | Q - O | 2749 | -700 |
| Y - G | 358 | -1055 | O - P | 537 | -1809 |
| G - W | 979 | -230 | | | |

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Hangers / Ties

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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

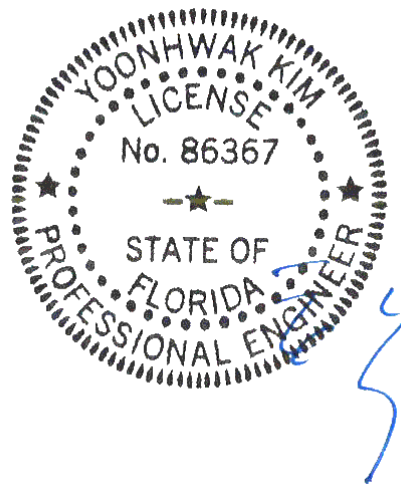
Bearing at location x=44'10"8 uses the following support conditions: 44'10"8

Bearing P (44'10"8, 10') HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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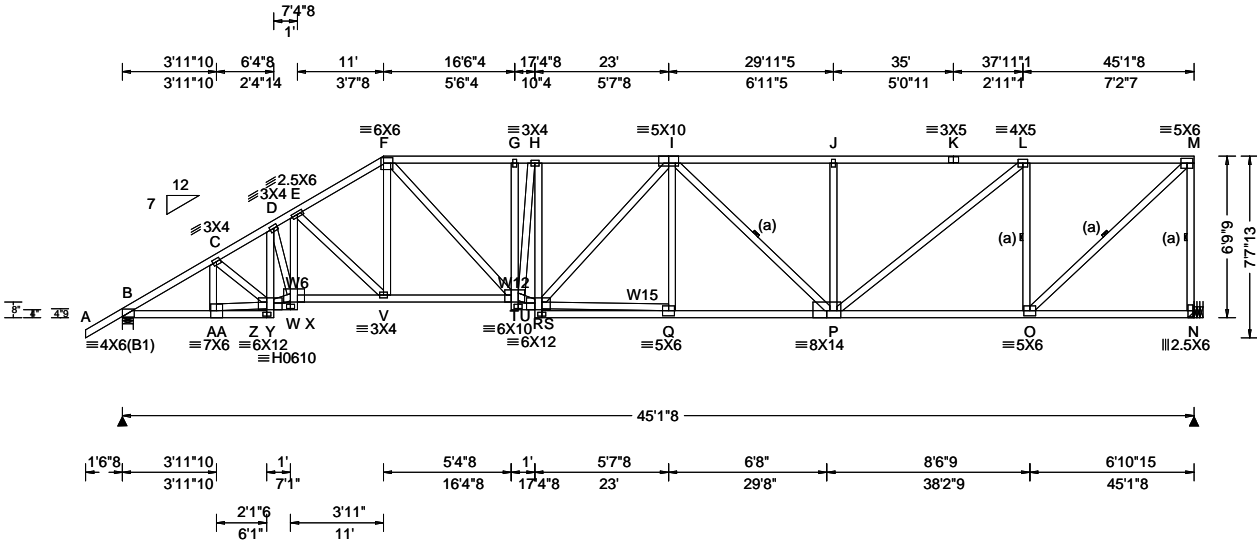
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6750 Forum Drive
Suite 305
Orlando FL, 32821



| | | | | | | | | | |
|--|--|---|---|--|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.51 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.275 G 999 240 VERT(CL): 0.567 G 951 180 HORZ(LL): 0.101 O - - HORZ(TL): 0.210 O - - Creep Factor: 2.0 Max TC CSI: 0.979 Max BC CSI: 0.930 Max Web CSI: 0.962 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1989 - / - / /1132 /228 /214 N 1868 - / - / /937 /243 -/ Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 2.3 N Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. | | | | | |
| | | | | B - C 665 -3169 H - I 911 -3311 C - D 840 -3528 I - J 730 -2941 D - E 940 -3749 J - K 730 -2941 E - F 816 -3128 K - L 730 -2941 F - G 958 -3452 L - M 444 -1742 G - H 955 -3443 | | | | | |

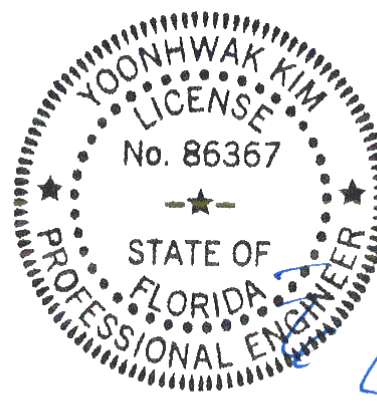
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W6,W12,W15 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B-AA | 2655 -717 | Q - P | 3260 -846 |
| W - V | 3235 -890 | P - O | 1808 -467 |
| V - T | 2663 -737 | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|--------|------------|-------|-------------|
| C-AA | 219 -609 | T - H | 657 -218 |
| C - Y | 492 -151 | T - R | 3435 -931 |
| AA - Y | 2525 -681 | H - R | 334 -884 |
| Y - D | 236 -763 | R - Q | 3014 -788 |
| Y - W | 3136 -858 | I - P | 161 -441 |
| D - W | 552 -170 | J - P | 204 -467 |
| W - E | 759 -179 | P - L | 1463 -378 |
| E - V | 213 -800 | L - O | 505 -1470 |
| F - V | 457 -27 | O - M | 2392 -610 |
| F - T | 1132 -337 | M - N | 534 -1815 |

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Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

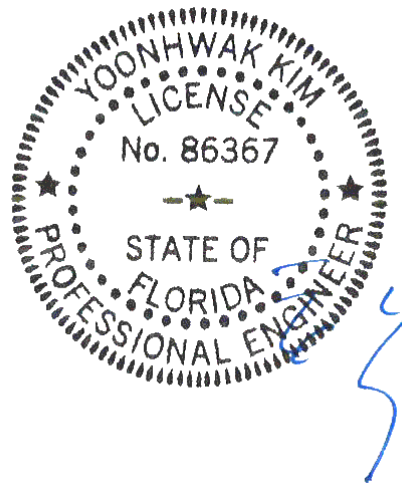
Bearing at location x=44'10"8 uses the following support conditions: 44'10"8

Bearing N (44'10"8, 10') HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

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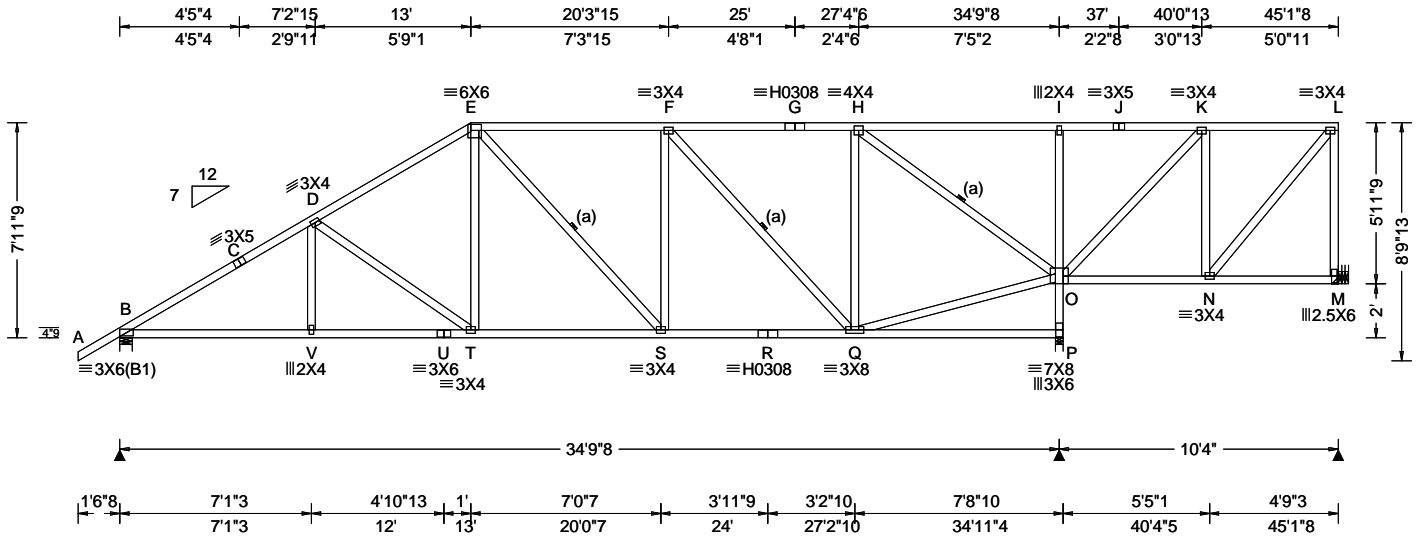
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6750 Forum Drive
Suite 305
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| | | | | | | | | | |
|--|--|---|---|--|---|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.51 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.083 T 999 240 VERT(CL): 0.172 T 999 180 HORZ(LL): 0.033 P - - HORZ(TL): 0.068 P - - Creep Factor: 2.0 Max TC CSI: 0.698 Max BC CSI: 0.650 Max Web CSI: 0.911 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1456 /- /- /891 /137 /249 P 2342 /- /- /1216 /323 /- M 231 /-125 /- /52 /24 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.7 P Brg Width = 3.5 Min Req = 2.8 M Brg Width = - Min Req = - Bearings B & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. | | | | | |
| | | | | Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; | Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types. | ▲ Maximum Reactions (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 368 -2157 G - H 195 -789 C - D 384 -2027 H - I 642 -176 D - E 408 -1683 I - J 642 -177 E - F 386 -1353 J - K 642 -177 F - G 195 -789 | | | |

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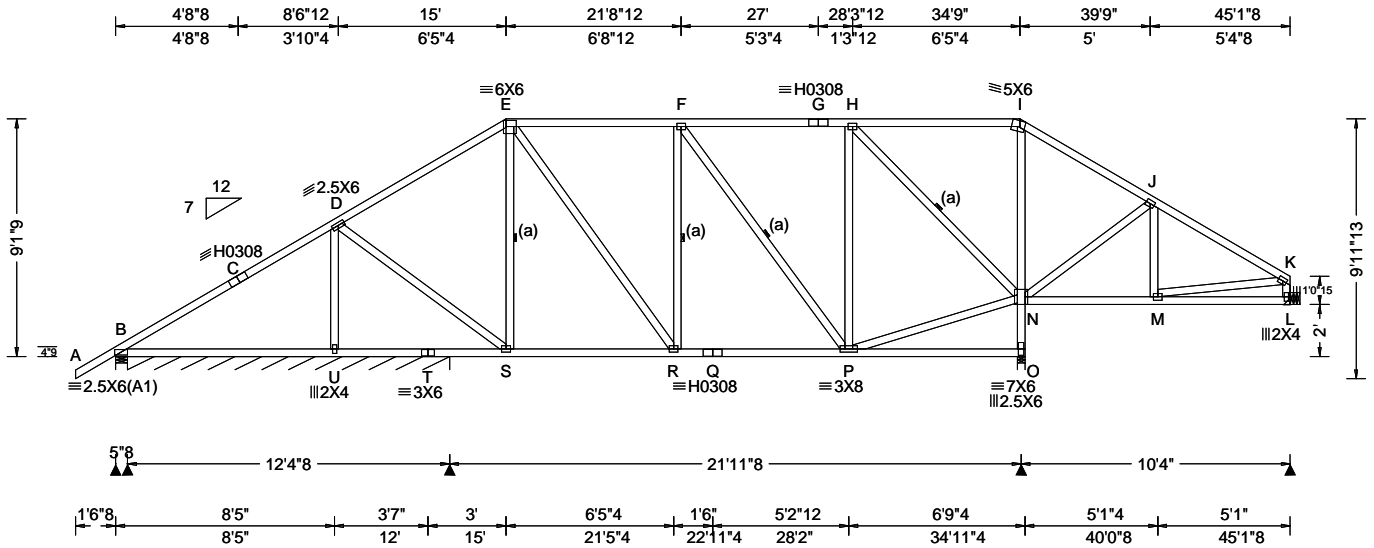
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FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | |
|---|--|--|--|
| Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - V 1767 -481 T - S 1373 -393 V - U 1765 -482 S - R 1347 -388 U - T 1765 -482 R - Q 1347 -388 | | | |
| Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. D - T 127 -486 H - O 459 -1742 E - T 477 -11 I - O 195 -414 F - Q 289 -844 O - P 626 -2278 Q - H 619 -76 O - K 221 -787 Q - O 805 -205 | | | |

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|--|---|---|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|---|------|----|-----|---|---|-----|----|---|---|------|---|---|------|-----|---|---|-----|---|---|------|---|---|--------|------------|--------|-------------|-------|---------|-------|----------|-------|----------|-------|----------|-------|----------|--|--|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| B | 565 | - | - | /342 | - | /216 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B* | 103 | - | - | /64 | /2 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| O | 1723 | - | - | /911 | /62 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| L | 346 | - | - | /218 | - | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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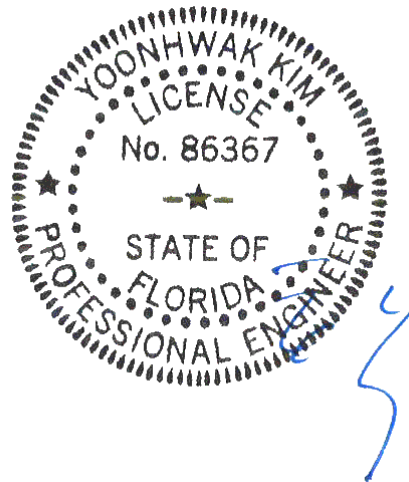
Lumber
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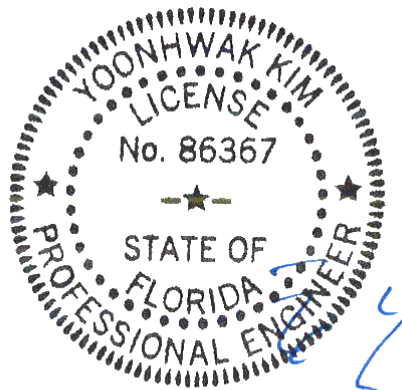
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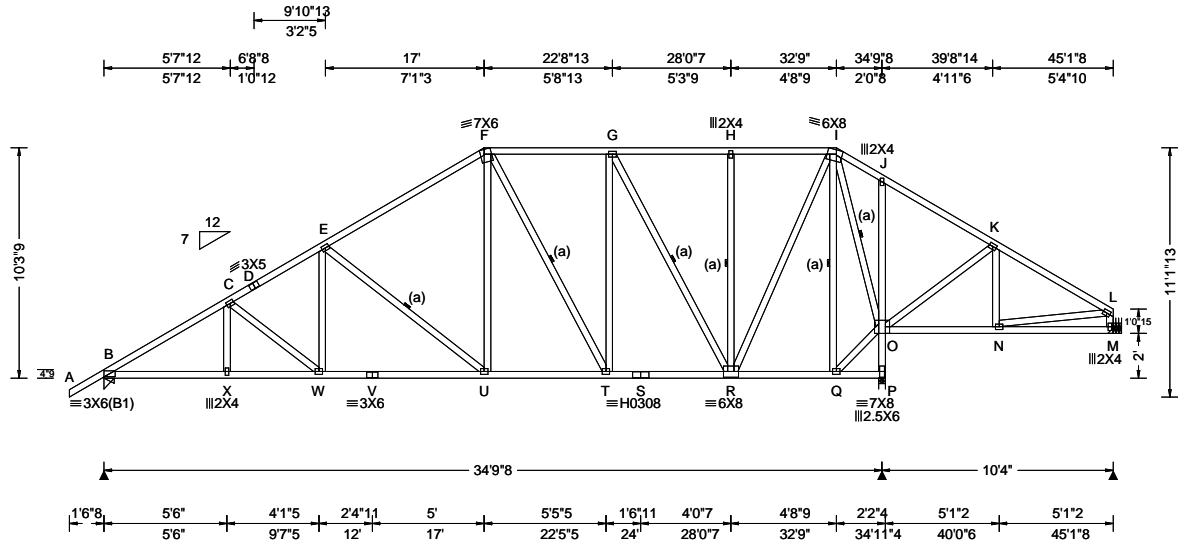
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6750 Forum Drive
Suite 305
Orlando FL, 32821



| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.51 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.087 W 999 240 VERT(CL): 0.181 W 999 180 HORZ(LL): 0.033 Q - - HORZ(TL): 0.070 Q - - Creep Factor: 2.0 Max TC CSI: 0.554 Max BC CSI: 0.674 Max Web CSI: 0.737 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL B 1504 - / - / - /926 /13 /244 P 2144 - / - / - /1108 /26 /- M 298 - / - / - /193 /36 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.8 P Brg Width = 3.5 Min Req = 2.5 M Brg Width = - Min Req = - Bearings B & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |
| | | | | B - C 393 -2276 G - H 310 -644 C - D 404 -2000 H - I 310 -644 D - E 422 -1964 I - J 389 0 E - F 419 -1478 J - K 436 -26 F - G 399 -1031 |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

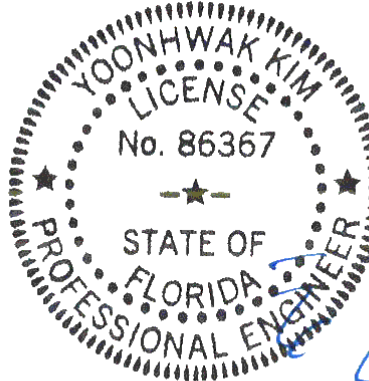
All plates are 3X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

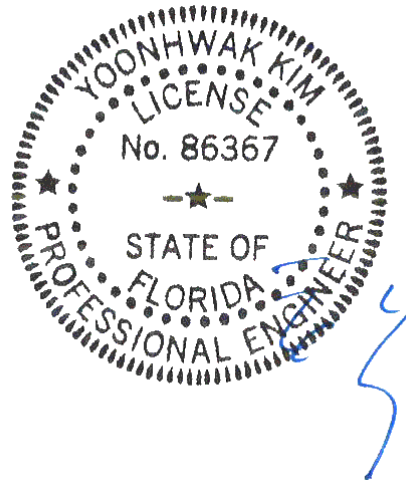
Bearing at location x=44'10"8 uses the following support conditions: 44'10"8

Bearing M (44'10"8, 12') LUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting member,

(3) 0.148"x3" nails into supported member.



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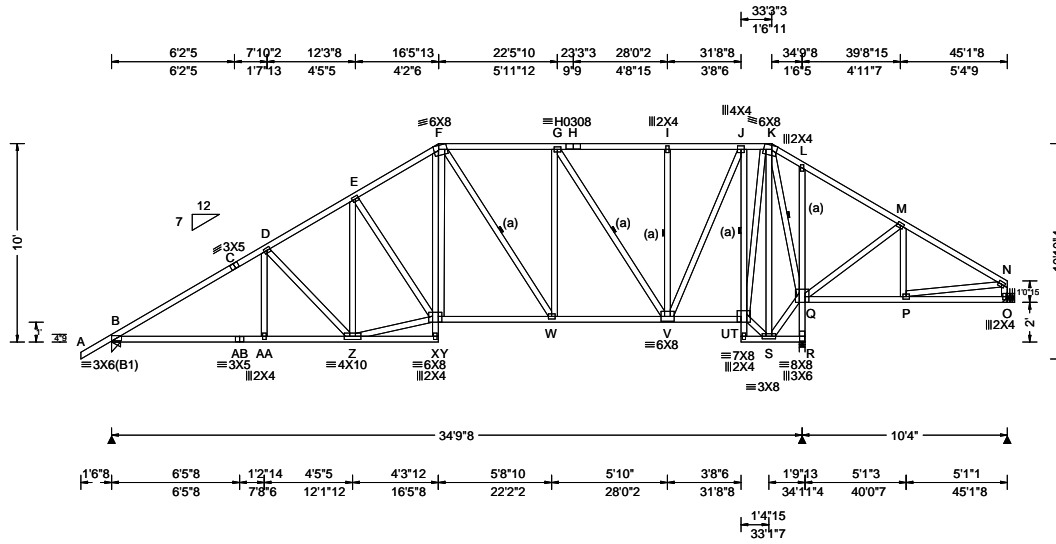
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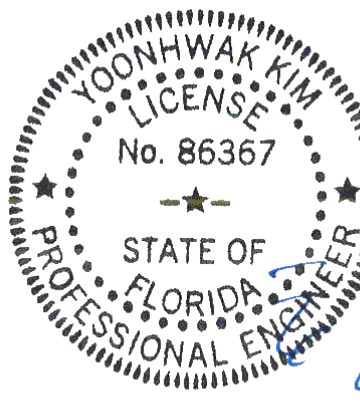
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|---|---|---|--|---|-------------|-------|-----------|-------|-----------|-------|-----------|-------|-----------|-------|-----------|------|-----|------|------------|------|-------------|-------|----------|-------|-----------|-------|-----------|-------|-----------|-------|---------|-------|-----------|------------|--------|-------------|-----------|-----------|-----------|----------|----------|-----------|-----------|----------|-------|-----------|-------|----------|-------|-----------|-------|--------|-------|-----------|-------|---------|
| | | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 1582 | - | - | /919 | /15 | /237 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R | 2351 | - | - | /1130 | /31 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| O | 267 | -18 | - | /188 | /53 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| C - D | 416 -2186 | H - I | 329 -803 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| B -AB | 1945 -350 | X - W | 1509 -235 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| AB-AA | 1945 -350 | W - V | 1345 -225 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| AA -Z | 1943 -352 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | Webs | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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| V - J | 1397 -301 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



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|---------------------------------------|--------------------------|--|--|
| SEQN: 17388 / FROM: Page 2 of 2 | HIPS Ply: 1 Qty: 4 | Job Number: 21-5856 Shelley Truss Label: D08 | Cust: R215 JRef:1X8V2150010 T21 / DrwNo: 259.21.1157.36624 / YK 09/16/2021 |
|---------------------------------------|--------------------------|--|--|

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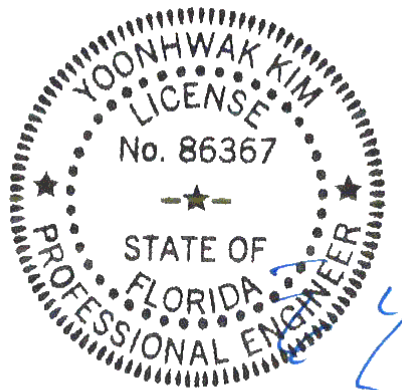
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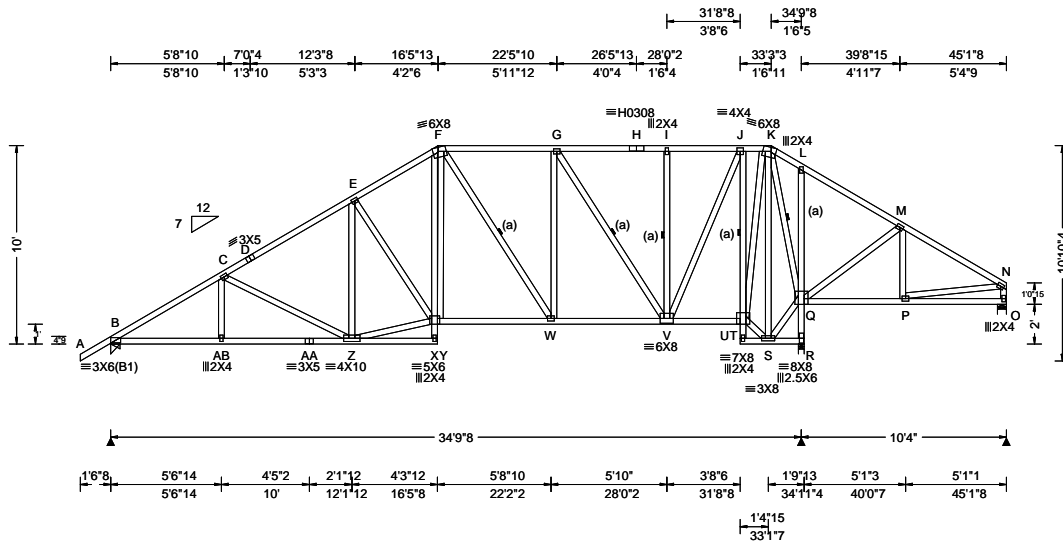
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| | | | | |
|--|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.51 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.097 X 999 240 VERT(CL): 0.202 X 999 180 HORZ(LL): 0.047 S - - HORZ(TL): 0.098 S - - Creep Factor: 2.0 Max TC CSI: 0.511 Max BC CSI: 0.652 Max Web CSI: 0.582 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1490 - / - / 915 / 15 / 237 R 2199 - / - / 1146 / 33 / - O 280 - / -13 / - / 182 / 57 / - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.8 R Brg Width = 3.5 Min Req = 2.6 O Brg Width = 5.5 Min Req = 1.5 Bearings B, R, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 410 -2286 G - H 325 -693 C - D 397 -1825 H - I 325 -693 D - E 428 -1781 I - J 325 -693 E - F 488 -1628 K - L 442 0 F - G 436 -1170 L - M 514 -14 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B -AB 1899 -373 X - W 1327 -232 AB-AA 1897 -374 W - V 1159 -223 AA-Z 1897 -374 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - Z 121 -463 J - T 353 -1246 Z - X 1527 -280 T - K 1283 -300 X - F 732 -108 K - Q 374 -1668 W - G 412 0 Q - R 459 -2175 G - V 227 -875 Q - M 121 -463 V - J 1232 -303 |
|--|---|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Wind

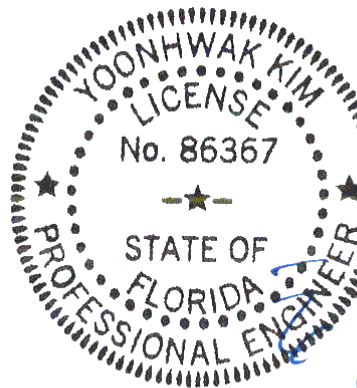
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

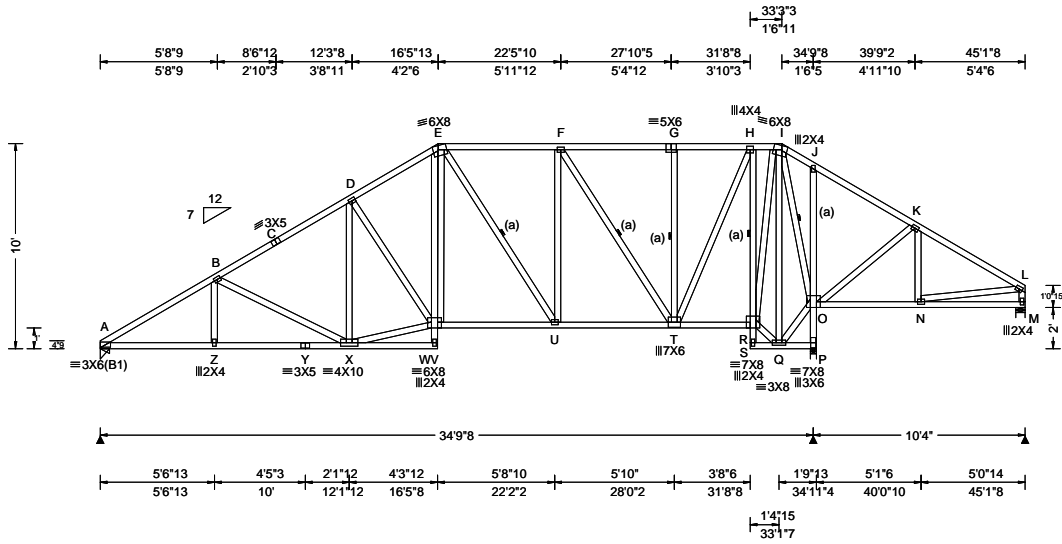
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FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | | | | | | |
|--|---|---|---|---|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.19 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.51 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.114 V 999 240 VERT(CL): 0.223 V 999 180 HORZ(LL): 0.055 Q - - HORZ(TL): 0.109 Q - - Creep Factor: 2.0 Max TC CSI: 0.518 Max BC CSI: 0.643 Max Web CSI: 0.646 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1470 - / - / - /832 /8 /222 P 2375 - / - / - /1147 /35 /- M 258 -/34 - / - /181 /56 /- Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.7 P Brg Width = 3.5 Min Req = 2.8 M Brg Width = 5.5 Min Req = 1.5 Bearings A, P, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. | | | | | |
| | | | | A - B 433 -2481 F - G 326 -790 B - C 415 -2006 G - H 326 -790 C - D 436 -1913 I - J 479 0 D - E 494 -1831 J - K 551 -13 E - F 441 -1354 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - Z 2072 -396 V - U 1503 -237 Z - Y 2070 -397 U - T 1338 -227 Y - X 2070 -397 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - X 131 -482 H - R 356 -1401 X - V 1696 -286 R - I 1434 -302 V - E 821 -110 I - O 374 -1837 U - F 473 0 O - P 461 -2351 F - T 231 -1028 O - K 120 -467 T - H 1404 -305 | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Loading

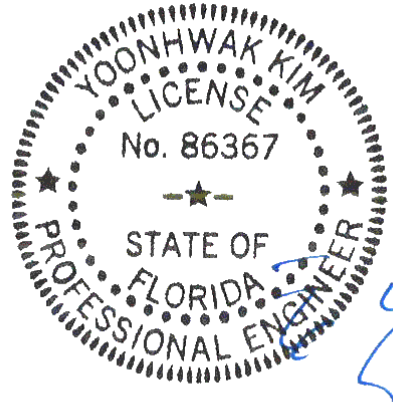
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

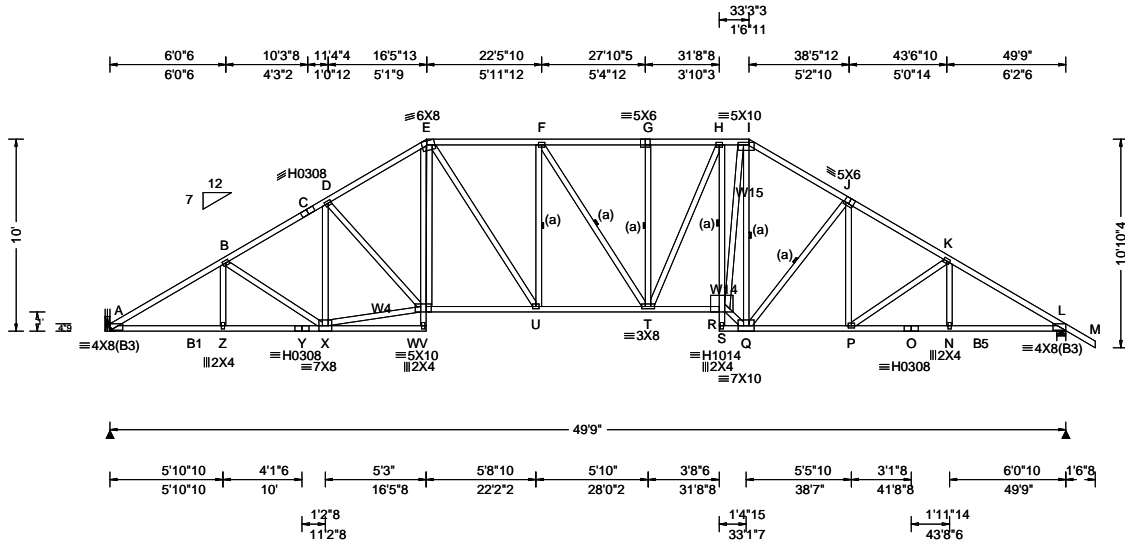
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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| | | | | | | | | | |
|--|---|---|---|---|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.98 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.344 G 999 240 VERT(CL): 0.662 G 897 180 HORZ(LL): 0.175 L - - HORZ(TL): 0.336 L - - Creep Factor: 2.0 Max TC CSI: 0.684 Max BC CSI: 0.988 Max Web CSI: 0.864 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 2224 -/- /- /1187 -/ /256 L 2340 -/- /- /1271 -/ -/ Wind reactions based on MWFRS A Brg Width = - Min Req = - L Brg Width = 5.5 Min Req = 1.9 Bearing L is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. | | | | | |
| | | | | A - B 690 -3883 G - H 764 -3209 B - C 701 -3520 H - I 720 -3047 C - D 707 -3382 I - J 711 -3072 D - E 769 -3515 J - K 700 -3515 E - F 771 -3265 K - L 668 -3849 F - G 764 -3209 | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B1,B5 2x4 SP M-31;
 Webs: 2x4 SP #3; W4,W14,W15 2x4 SP #2;

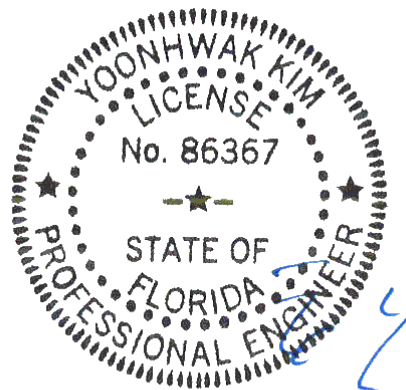
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
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Hangers / Ties

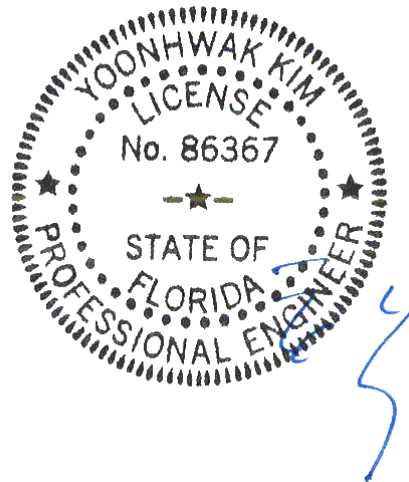
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

- Bearing A (0', 10') HUS26
- Supporting Member: (2)2x6 SP 2400f-2.0E
- (14) 0.162"x3.5" nails into supporting member,
- (4) 0.162"x3.5" nails into supported member.



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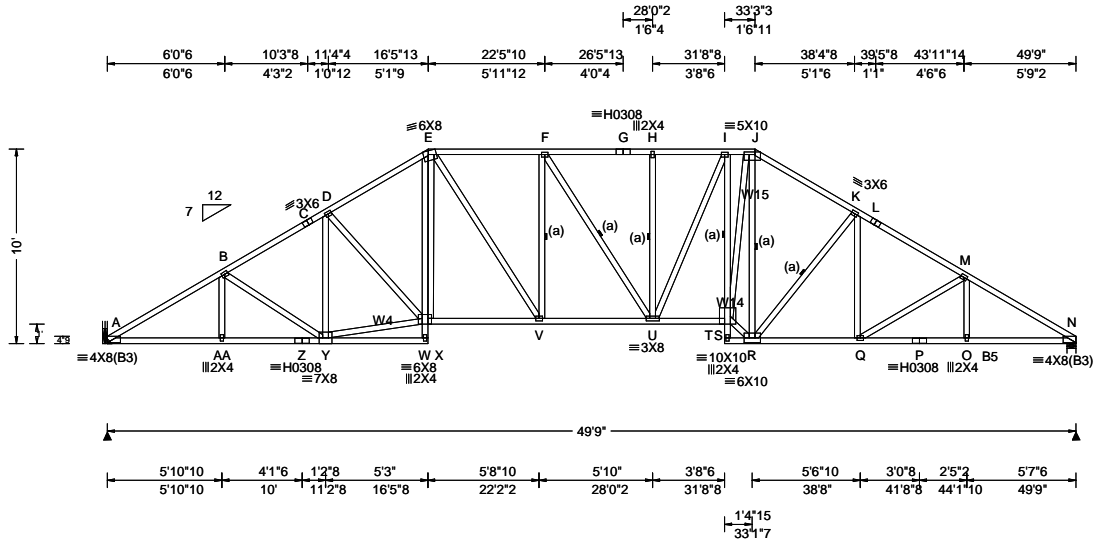
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6750 Forum Drive
Suite 305
Orlando FL, 32821



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|--|---|---|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.19 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.98 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.293 H 999 240 VERT(CL): 0.608 H 975 180 HORZ(LL): 0.151 N - - HORZ(TL): 0.314 N - - Creep Factor: 2.0 Max TC CSI: 0.533 Max BC CSI: 0.975 Max Web CSI: 0.778 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 2068 - / - / /1187 - / /230 N 2069 - / - / /1188 - / - Wind reactions based on MWFRS A Brg Width = - Min Req = - N Brg Width = 5.5 Min Req = 1.7 Bearing N is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |
| | | | | A - B 692 -3589 H - I 776 -2894 B - C 703 -3216 I - J 731 -2758 C - D 709 -3078 J - K 718 -2798 D - E 776 -3166 K - L 707 -3073 E - F 778 -2911 L - M 702 -3219 F - G 776 -2894 M - N 689 -3585 G - H 776 -2894 |

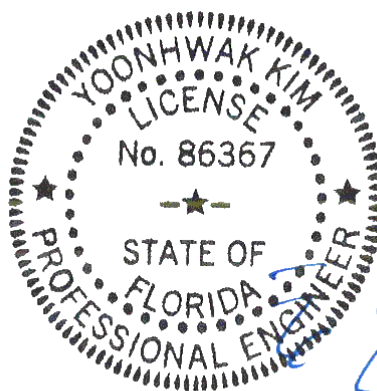
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B5 2x4 SP M-31;
 Webs: 2x4 SP #3; W4,W14,W15 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

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 09/16/2021

| | | | | | |
|---|------|------|-----|------|-------|
| Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. | | | | | |
| A-AA | 3015 | -528 | U-S | 2777 | -438 |
| AA-Z | 3014 | -529 | R-Q | 2699 | -444 |
| Z-Y | 3014 | -529 | Q-P | 3013 | -530 |
| W-V | 2651 | -406 | P-O | 3013 | -530 |
| V-U | 2922 | -485 | O-N | 3015 | -528 |
| Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. | | | | | |
| Y-W | 2736 | -447 | S-J | 2659 | -502 |
| W-E | 808 | -103 | R-J | 261 | -1563 |
| E-V | 477 | -171 | R-K | 149 | -578 |
| S-R | 3133 | -468 | | | |

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Hangers / Ties

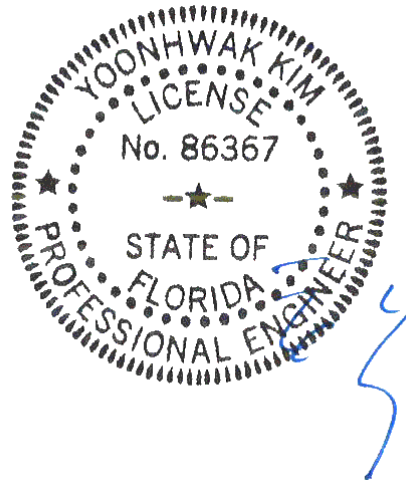
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Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

- Bearing A (0', 10') HUS26
- Supporting Member: (2)2x6 SP 2400f-2.0E
- (14) 0.162"x3.5" nails into supporting member,
- (4) 0.162"x3.5" nails into supported member.



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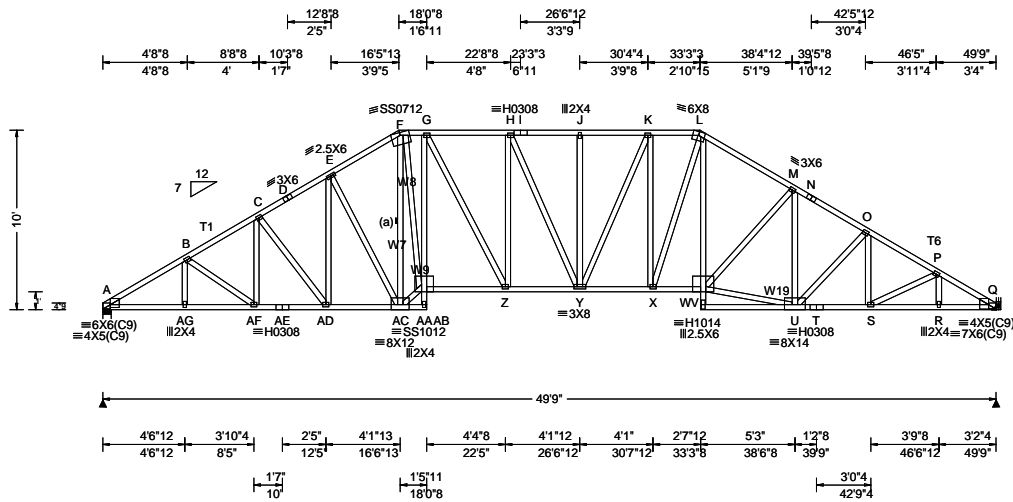
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6750 Forum Drive
Suite 305
Orlando FL, 32821

2 Complete Trusses Required



| | | | | |
|---|--|--|---|---|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.19 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.98 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS, 18SS | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.394 H 999 240 VERT(CL): 0.777 H 764 180 HORZ(LL): 0.191 Q - - HORZ(TL): 0.375 Q - - Creep Factor: 2.0 Max TC CSI: 0.676 Max BC CSI: 0.812 Max Web CSI: 0.824 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL A 6033 - / - / - / - / - / - Q 6777 - / - / - / - / - / - Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 2.5 Q Brg Width = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. |
| | | | | A - B 16 -5184 I - J 44 -4524 B - C 13 -4986 J - K 44 -4524 C - D 14 -4599 K - L 65 -4346 D - E 6 -4573 L - M 103 -4899 E - F 20 -4159 M - N 90 -5050 F - G 14 -4222 N - O 105 -5099 G - H 27 -4476 O - P 118 -5619 H - I 44 -4524 P - Q 125 -5859 |

Lumber
 Top chord: 2x4 SP #2; T1,T6 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3; W7 2x4 SP #2; W8,W9,
 W19 2x4 SP M-31;
 Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 1 Row @ 7.50" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

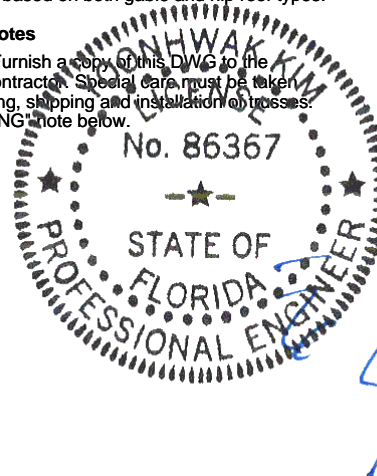
Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 16.49
 TC: From 32 plf at 16.49 to 32 plf at 33.26
 TC: From 63 plf at 33.26 to 63 plf at 49.75
 BC: From 10 plf at 0.00 to 10 plf at 49.75
 BC: 249 lb Conc. Load at 0.56, 2.56, 4.56, 6.56
 BC: 362 lb Conc. Load at 8.56, 10.56, 12.56, 14.56
 16.56
 BC: 414 lb Conc. Load at 18.56, 20.56, 22.56, 24.56
 26.56, 28.56, 30.56
 BC: 354 lb Conc. Load at 32.56
 BC: 464 lb Conc. Load at 34.56, 36.56, 38.56, 40.56
 42.56, 44.56, 46.56
 BC: 393 lb Conc. Load at 48.56

Plating Notes
 All plates are 3X4 except as noted.

Hangers / Ties
 (J) Hanger Support Required, by others

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation processes. See "WARNING" note below.



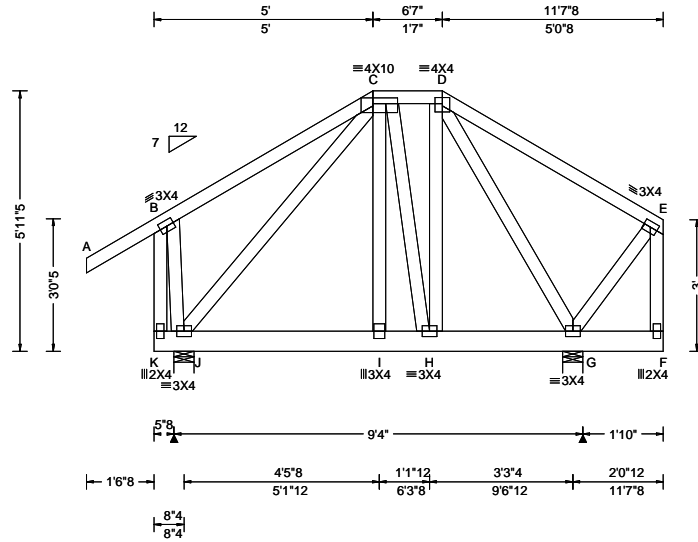
FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.03 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.012 I 999 240 VERT(CL): 0.020 I 999 180 HORZ(LL): 0.005 E - - HORZ(TL): 0.008 E - - Creep Factor: 2.0 Max TC CSI: 0.519 Max BC CSI: 0.079 Max Web CSI: 0.650 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 906 /- /- /- /104 /- G 1016 /- /- /- /75 /- Wind reactions based on MWFRS J Brg Width = 5.5 Min Req = 1.5 G Brg Width = 5.5 Min Req = 1.5 Bearings J & G are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. J - I 387 -15 I - H 400 -15 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 6.58
TC: From 63 plf at 6.58 to 63 plf at 11.62
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 6.55
BC: From 20 plf at 6.55 to 20 plf at 11.62
TC: 6 lb Conc. Load at 5.03
TC: -290 lb Conc. Load at 6.55
BC: 536 lb Conc. Load at 5.03
BC: 664 lb Conc. Load at 6.55

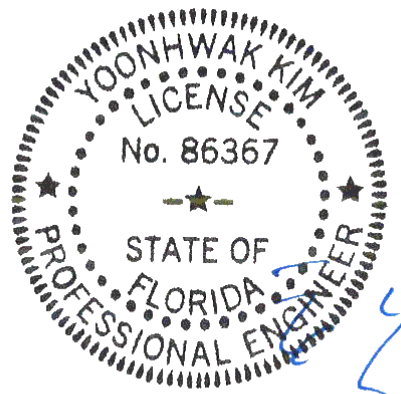
Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-11-5.

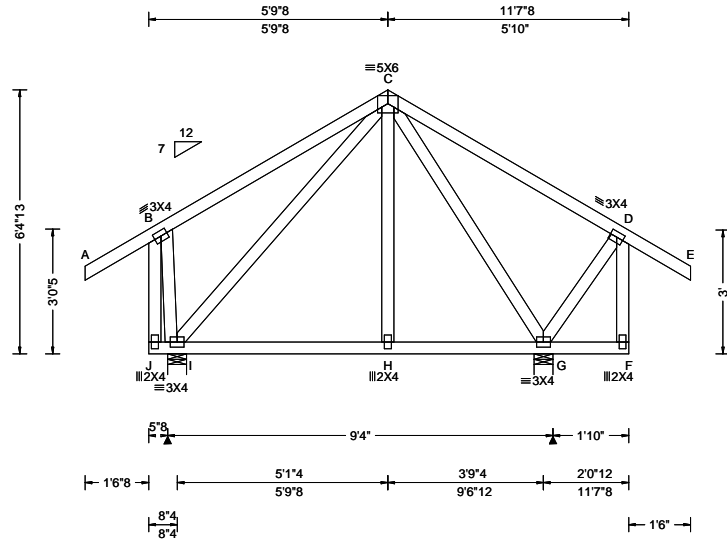
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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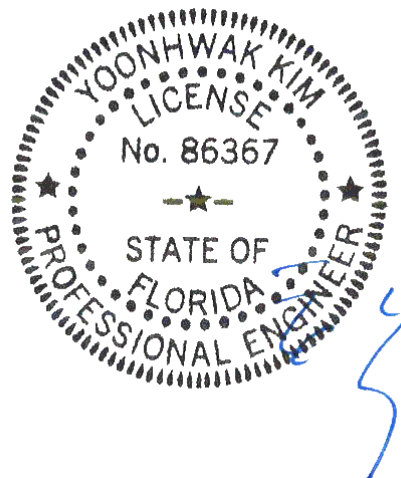
| | | | | |
|---|--|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.26 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.003 H 999 240 VERT(CL): 0.006 H 999 180 HORZ(LL): 0.003 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.567 Max BC CSI: 0.166 Max Web CSI: 0.434 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 527 - / - / 306 / 79 / 118 G 675 - / - / 421 / 96 / - Wind reactions based on MWFRS I Brg Width = 5.5 Min Req = 1.5 G Brg Width = 5.5 Min Req = 1.5 Bearings I & G are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - G 113 -451 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

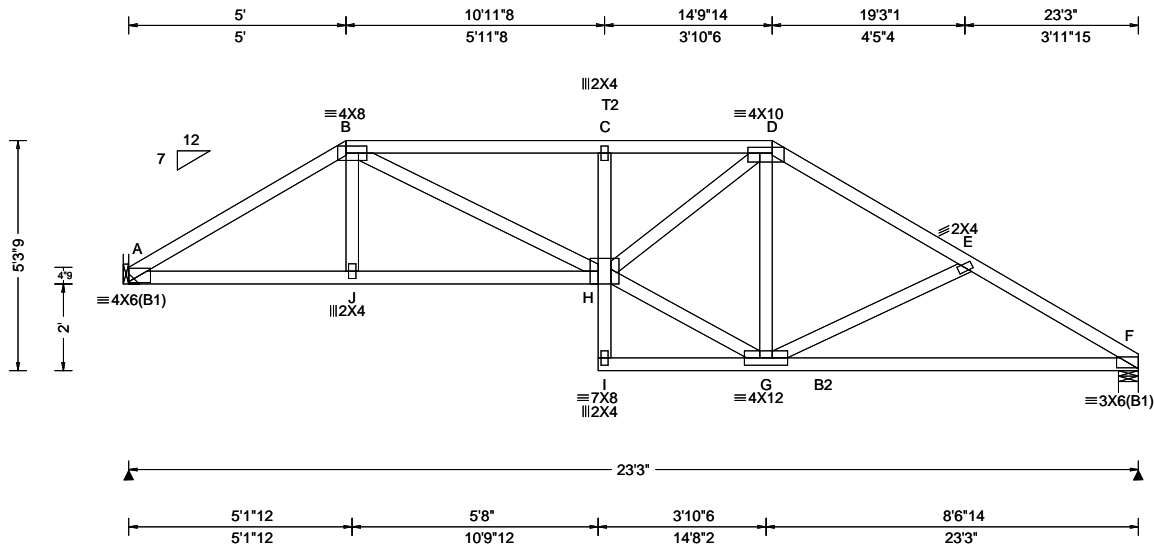
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



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|--|---|---|--|--|--|--|--|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.130 C 999 240 VERT(CL): 0.253 C 999 180 HORZ(LL): 0.054 F - - HORZ(TL): 0.105 F - - Creep Factor: 2.0 Max TC CSI: 0.421 Max BC CSI: 0.711 Max Web CSI: 0.809 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1664 -/- /- /- /209 -/ F 1359 -/- /- /- /155 -/ Wind reactions based on MWFRS A Brg Width = - Min Req = - F Brg Width = 5.5 Min Req = 1.6 Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 374 -2876 D - E 240 -2026 B - C 434 -3404 E - F 307 -2270 C - D 433 -3383 | | | | | |
| | | | | Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - J 2417 -306 G - F 1904 -246 J - H 2451 -312 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - J 622 -109 H - G 1907 -206 B - H 1059 -136 H - D 2123 -307 C - H 148 -538 G - D 146 -701 | | | | | |

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
 Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 5.00
 TC: From 32 plf at 5.00 to 32 plf at 14.82
 TC: From 63 plf at 14.82 to 63 plf at 23.25
 BC: From 20 plf at 0.00 to 20 plf at 5.00
 BC: From 10 plf at 5.00 to 10 plf at 14.73
 BC: From 20 plf at 14.73 to 20 plf at 23.25
 TC: -147 lb Conc. Load at 5.00
 TC: 129 lb Conc. Load at 5.06, 7.06, 9.06
 TC: 156 lb Conc. Load at 11.06,13.06,14.73
 BC: 532 lb Conc. Load at 5.00
 BC: 90 lb Conc. Load at 7.06, 9.06
 BC: 26 lb Conc. Load at 11.06,13.06,14.73

Hangers / Ties

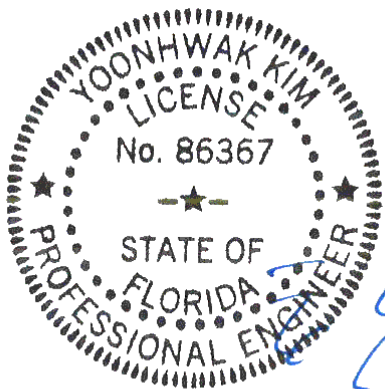
(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3'-3-9.

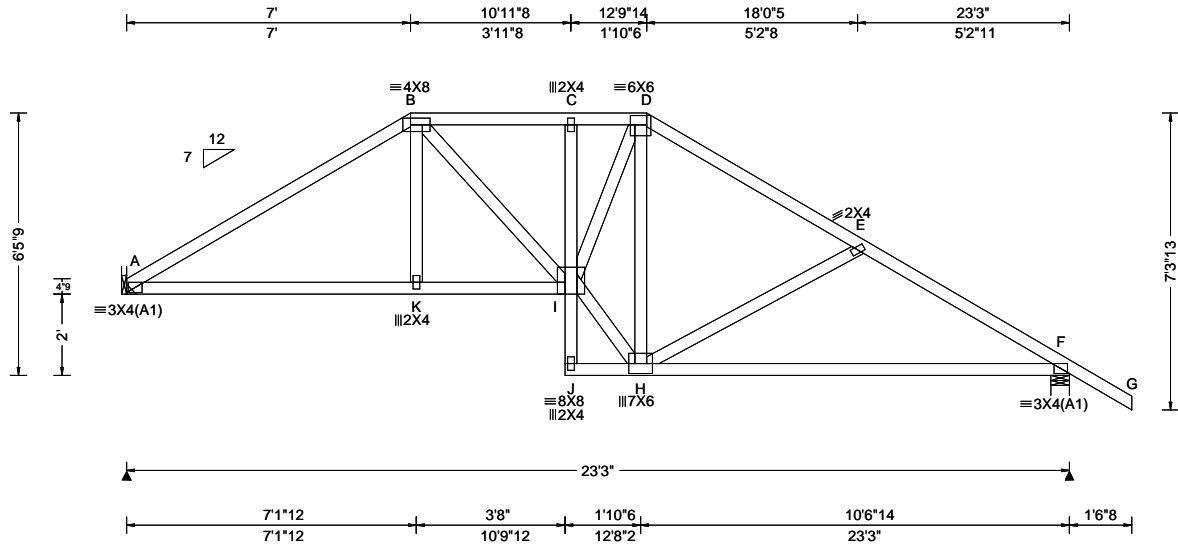


FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | | |
|---------------------|------|------------------|--|---|
| SEQN: 10002 / FROM: | SPEC | Ply: 1 Qty: 1 | Job Number: 21-5856 Shelley Truss Label: F02 | Cust: R215 JRef: 1X8V2150010 T81 / DrwNo: 259.21.1157.35389 / YK 09/16/2021 |
|---------------------|------|------------------|--|---|



| | | | | |
|--|---|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.058 C 999 240 VERT(CL): 0.119 C 999 180 HORZ(LL): 0.033 F - - HORZ(TL): 0.069 F - - Creep Factor: 2.0 Max TC CSI: 0.603 Max BC CSI: 0.924 Max Web CSI: 0.566 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 962 /- /- /530 /98 /148 F 1076 /- /- /653 /127 /- Wind reactions based on MWFRS A Brg Width = - Min Req = - F Brg Width = 5.5 Min Req = 1.5 Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 411 -1449 D - E 359 -1139 B - C 468 -1380 E - F 400 -1464 C - D 466 -1373 |
|--|---|---|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

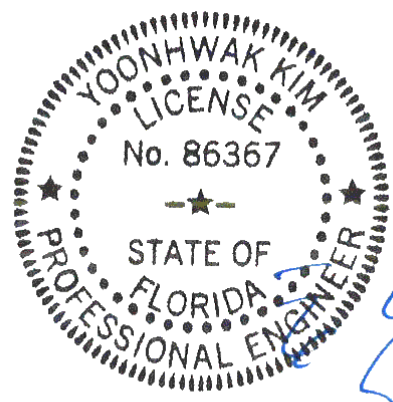
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 12') LUS26
Supporting Member: (1)2x6 SP 2400f-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

Wind

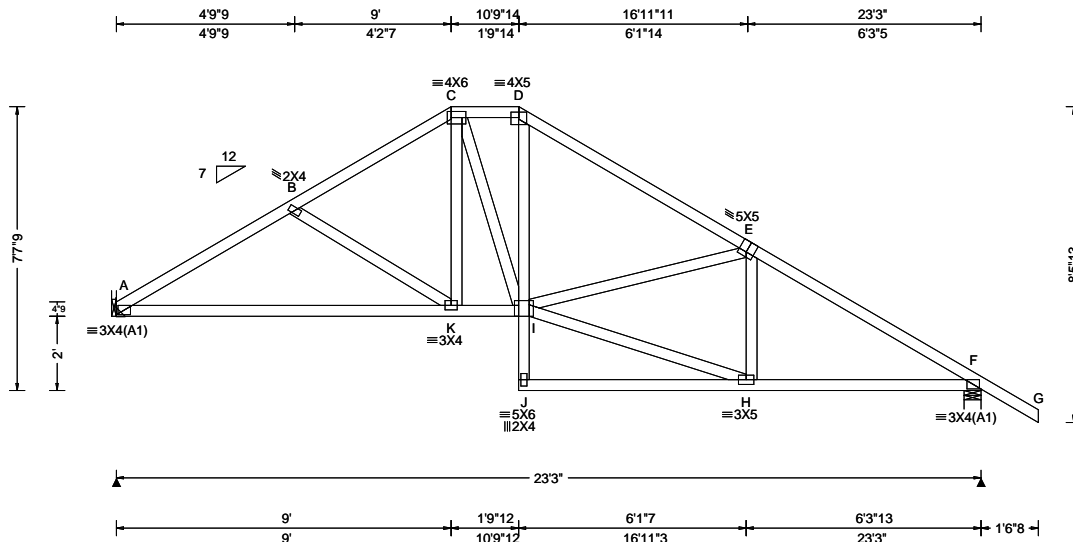
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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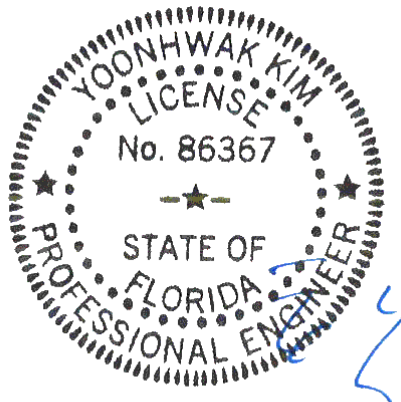


| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.049 I 999 240 VERT(CL): 0.101 I 999 180 HORZ(LL): 0.025 F - - HORZ(TL): 0.051 F - - Creep Factor: 2.0 Max TC CSI: 0.410 Max BC CSI: 0.703 Max Web CSI: 0.476 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>962</td> <td>-</td> <td>-</td> <td>/534</td> <td>/94</td> <td>/173</td> </tr> <tr> <td>F</td> <td>1076</td> <td>-</td> <td>-</td> <td>/656</td> <td>/124</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Width = - Min Req = - F Brg Width = 5.5 Min Req = 1.5 Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>284 - 1503</td> <td>D - E</td> <td>265 - 1351</td> </tr> <tr> <td>B - C</td> <td>251 - 1233</td> <td>E - F</td> <td>250 - 1482</td> </tr> <tr> <td>C - D</td> <td>267 - 1073</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - K</td> <td>1245 - 97</td> <td>H - F</td> <td>1201 - 117</td> </tr> <tr> <td>K - I</td> <td>998 0</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>I - H</td> <td>1251 - 124</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 962 | - | - | /534 | /94 | /173 | F | 1076 | - | - | /656 | /124 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | A - B | 284 - 1503 | D - E | 265 - 1351 | B - C | 251 - 1233 | E - F | 250 - 1482 | C - D | 267 - 1073 | | | Chords | Tens.Comp. | Chords | Tens. Comp. | A - K | 1245 - 97 | H - F | 1201 - 117 | K - I | 998 0 | | | Webs | Tens.Comp. | I - H | 1251 - 124 |
|--|--|---|---|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|-----|------|---|------|---|---|------|------|---|--------|------------|--------|-------------|-------|------------|-------|------------|-------|------------|-------|------------|-------|------------|--|--|--------|------------|--------|-------------|-------|-----------|-------|------------|-------|-------|--|--|------|------------|-------|------------|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 962 | - | - | /534 | /94 | /173 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| F | 1076 | - | - | /656 | /124 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - B | 284 - 1503 | D - E | 265 - 1351 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 251 - 1233 | E - F | 250 - 1482 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - D | 267 - 1073 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A - K | 1245 - 97 | H - F | 1201 - 117 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| K - I | 998 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Webs | Tens.Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| I - H | 1251 - 124 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties
 Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
 Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
 Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
 Bearing at location x=0' uses the following support conditions: 0'
 Bearing A (0', 12') LUS26
 Supporting Member: (1)2x6 SP 2400f-2.0E
 (4) 0.148"x3" nails into supporting member,
 (3) 0.148"x3" nails into supported member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

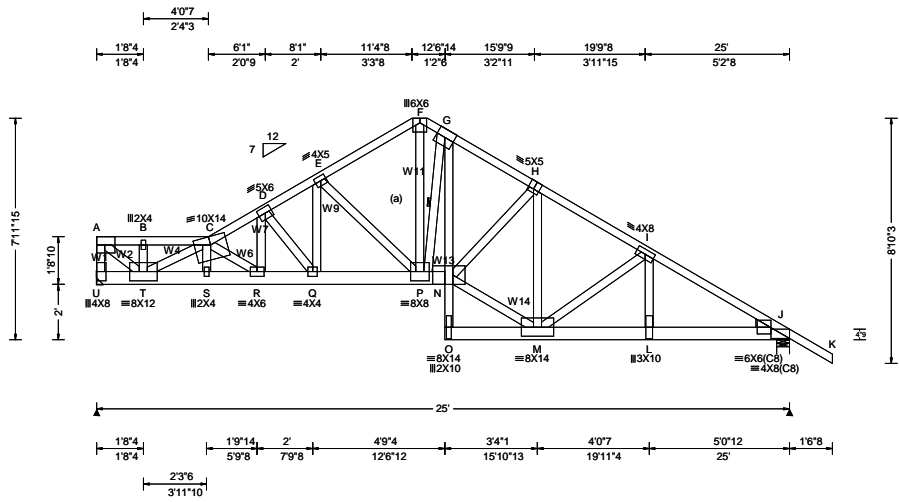


FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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2 Complete Trusses Required



Loading Criteria (psf)

TCLL: 20.00
 TCCL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 0.00
 Soffit: 2.00
 Load Duration: 1.25
 Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 120 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCCL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 3.00 ft
 Loc. from endwall: not in 9.00 ft
 GCcpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res. HVHZ
 TPI Std: 2014
 Rep Fac: No
 FT/RT:20(0)/10(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.232 N 999 240
 VERT(CL): 0.465 N 641 180
 HORZ(LL): 0.075 J - -
 HORZ(TL): 0.151 J - -
 Creep Factor: 2.0
 Max TC CSI: 0.453
 Max BC CSI: 0.682
 Max Web CSI: 0.782

VIEW Ver: 21.01.01A.0521.20

Maximum Reactions (lbs)

| Loc | Gravity | | | Non-Gravity | | |
|-----|---------|-----|-----|-------------|------|-----|
| | R+ | /R- | /Rh | /Rw | /U | /RL |
| U | 6573 | - | - | - | 857 | - |
| J | 7879 | - | - | - | 1077 | - |

Wind reactions based on MWFRS
 U Brg Width = - Min Req = -
 J Brg Width = 5.5 Min Req = 3.3
 Bearing J is a rigid surface.
 Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - B | 514 - 3954 | F - G | 555 - 4351 |
| B - C | 514 - 3954 | G - H | 673 - 5213 |
| C - D | 852 - 6534 | H - I | 715 - 5450 |
| D - E | 697 - 5457 | I - J | 988 - 7273 |
| E - F | 579 - 4515 | | |

Lumber

Top chord: 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W1,W4,W6,W7,W9,W13 2x4 SP #2;
 W2,W11,W14 2x4 SP M-31;
 Rt Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 32 plf at 0.00 to 32 plf at 19.88
 TC: From 63 plf at 19.88 to 63 plf at 26.54
 BC: From 10 plf at 0.00 to 10 plf at 19.88
 BC: From 20 plf at 19.88 to 20 plf at 25.00
 BC: From 5 plf at 25.00 to 5 plf at 26.54
 BC: 267 lb Conc. Load at 1.19, 3.19, 5.19, 5.94
 BC: 2032 lb Conc. Load at 1.69
 BC: 298 lb Conc. Load at 7.94
 BC: 346 lb Conc. Load at 9.94
 BC: 231 lb Conc. Load at 11.94
 BC: 1868 lb Conc. Load at 13.94, 15.94, 17.94
 BC: 3519 lb Conc. Load at 19.88

Plating Notes

All plates are 7X8 except as noted.

Hangers / Ties

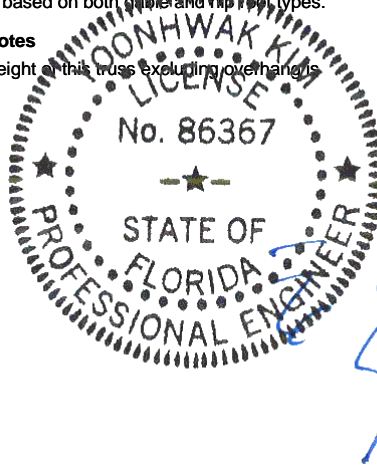
(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-11-15.

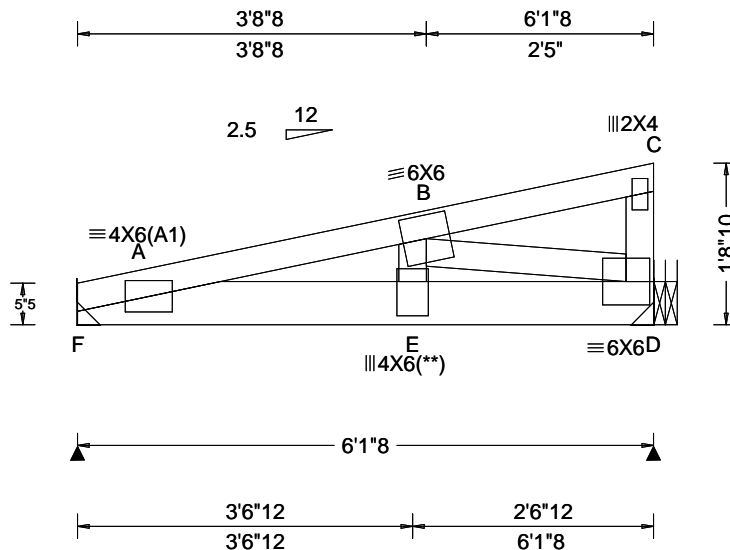


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| | | | | |
|--|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.047 A 999 240 VERT(CL): 0.092 A 782 180 HORZ(LL): 0.008 A - - HORZ(TL): 0.015 A - - Creep Factor: 2.0 Max TC CSI: 0.323 Max BC CSI: 0.702 Max Web CSI: 0.737 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL F 1987 /- /- /- /256 /- D 2032 /- /- /- /236 /- Wind reactions based on MWFRS F Brg Width = - Min Req = - D Brg Width = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 450 -3671 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - E 3605 -440 E - D 3377 -417 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. E - B 1646 -166 B - D 439 -3548 |
|--|--|---|---|---|

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

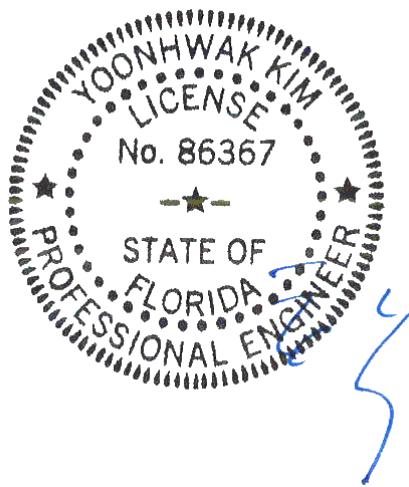
Special Loads
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 6.13
BC: From 10 plf at 0.00 to 10 plf at 6.13
BC: 1664 lb Conc. Load at 1.56
BC: 962 lb Conc. Load at 3.56, 5.56

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

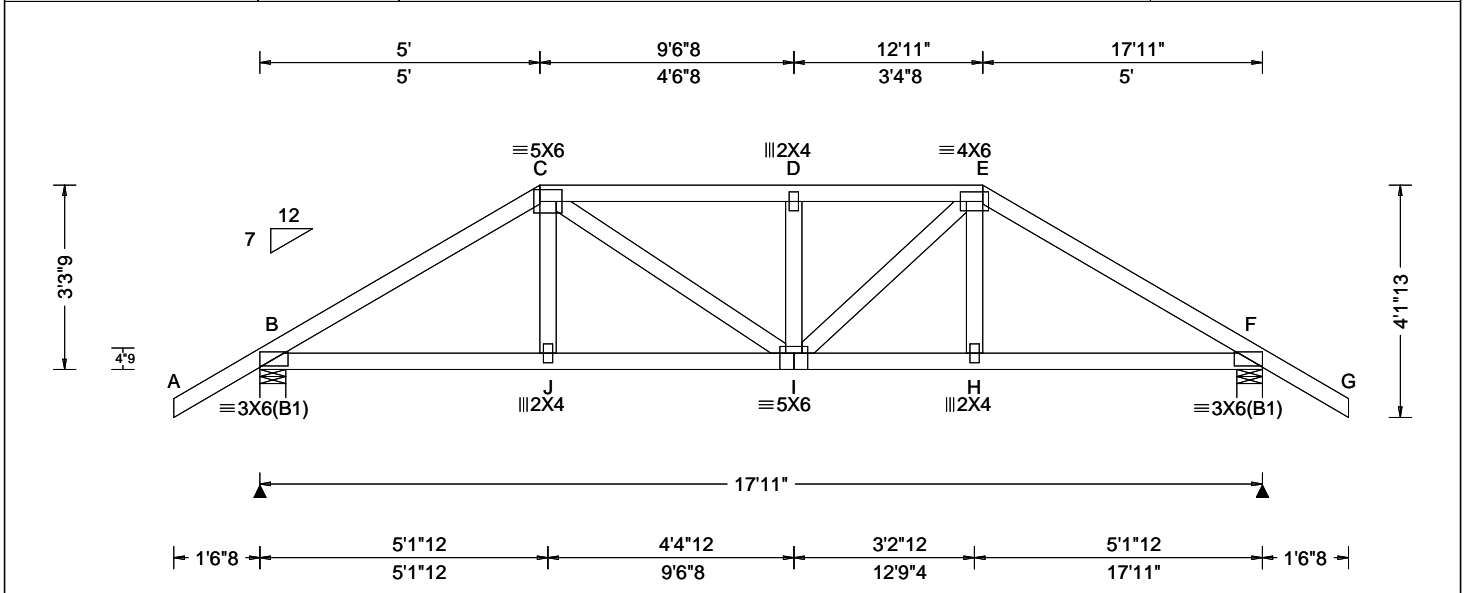
Additional Notes
The overall height of this truss excluding overhang is 1-8-10.



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| | | | | | | | | | |
|--|---|---|--|---|--|--|--|--|--|
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.075 D 999 240 VERT(CL): 0.150 D 999 180 HORZ(LL): 0.030 F - - HORZ(TL): 0.060 F - - Creep Factor: 2.0 Max TC CSI: 0.566 Max BC CSI: 0.740 Max Web CSI: 0.220 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1532 /- /- /- /223 /- F 1532 /- /- /- /223 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.8 F Brg Width = 5.5 Min Req = 1.8 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 339 -2351 D - E 351 -2384 C - D 351 -2384 E - F 337 -2329 | | | | | |
| | | | | Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - J 1953 -272 I - H 1951 -266 J - I 1977 -270 H - F 1930 -268 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - J 478 0 I - E 578 -113 C - I 488 -97 H - E 430 0 D - I 168 -412 | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

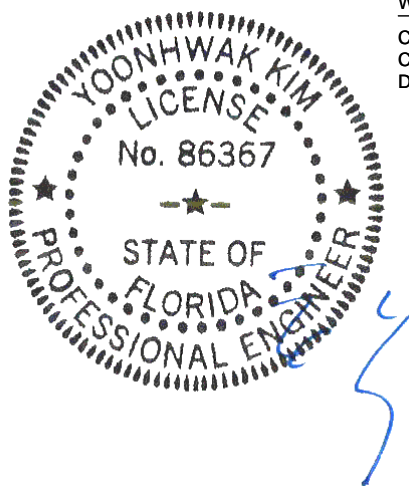
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 5.00
 TC: From 32 plf at 5.00 to 32 plf at 12.92
 TC: From 63 plf at 12.92 to 63 plf at 19.46
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 5.03
 BC: From 10 plf at 5.03 to 10 plf at 12.89
 BC: From 20 plf at 12.89 to 20 plf at 17.92
 BC: From 5 plf at 17.92 to 5 plf at 19.46
 TC: 129 lb Conc. Load at 5.06, 7.06, 8.96,10.85
 12.85
 BC: 388 lb Conc. Load at 5.03,12.89
 BC: 90 lb Conc. Load at 7.06, 8.96,10.85

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

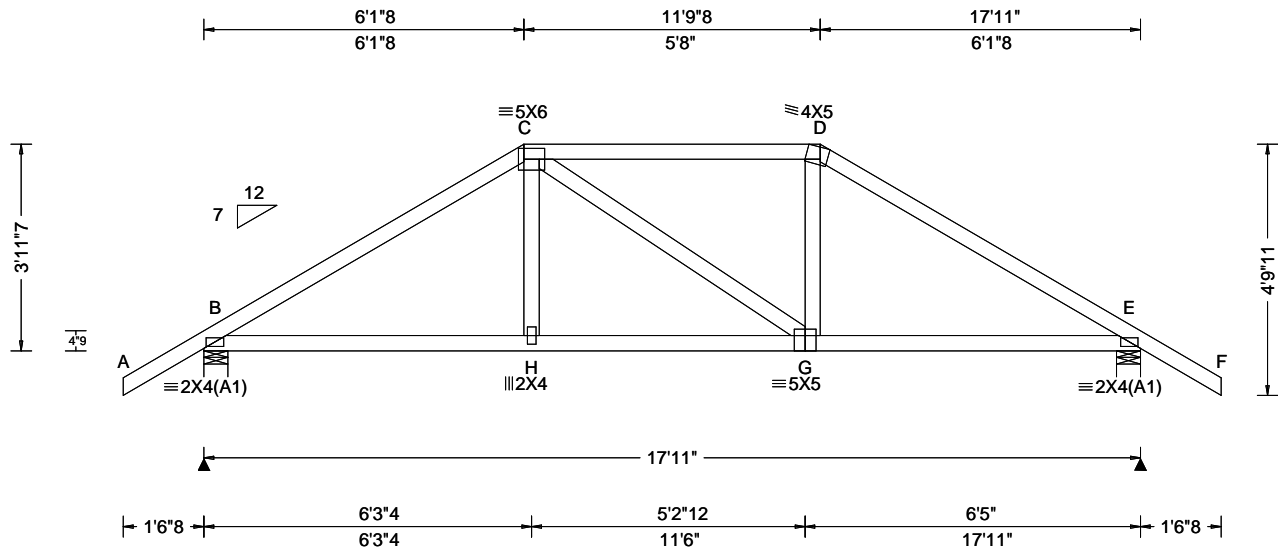
The overall height of this truss excluding overhang is 3-3-9.



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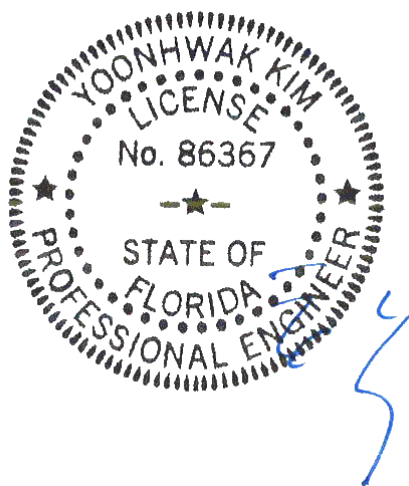




| | | | | |
|--|--|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.022 H 999 240 VERT(CL): 0.045 H 999 180 HORZ(LL): 0.011 E - - HORZ(TL): 0.022 E - - Creep Factor: 2.0 Max TC CSI: 0.318 Max BC CSI: 0.374 Max Web CSI: 0.088 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 849 - / - / /500 /99 /117 E 849 - / - /500 /99 - /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 E Brg Width = 5.5 Min Req = 1.5 Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 375 -1041 D - E 376 -1038 C - D 367 -827 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - H 819 -228 G - E 817 -236 H - G 824 -226 |
|--|--|---|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

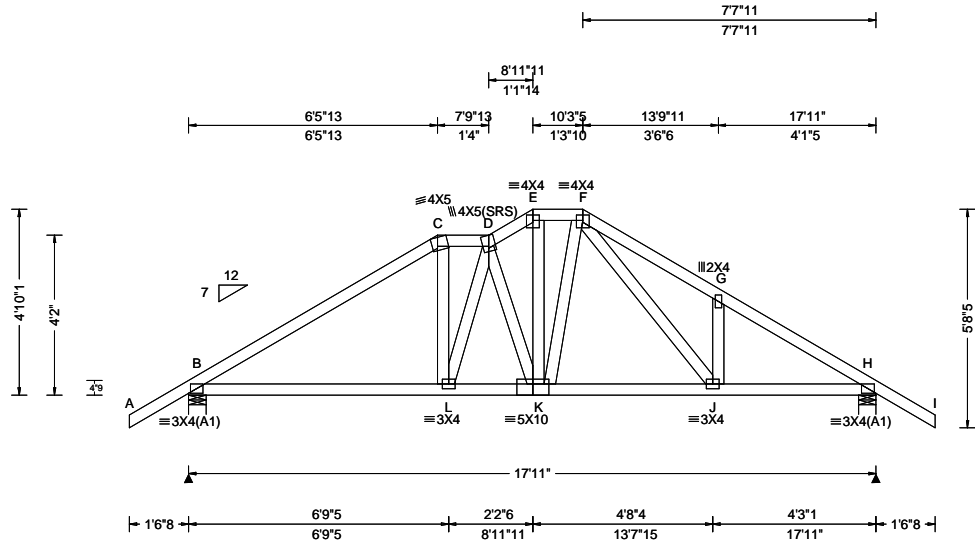
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|---|--|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.027 D 999 240 VERT(CL): 0.055 D 999 180 HORZ(LL): 0.012 H - - HORZ(TL): 0.025 H - - Creep Factor: 2.0 Max TC CSI: 0.489 Max BC CSI: 0.427 Max Web CSI: 0.134 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 884 /- /- /502 /106 /136 H 890 /- /- /503 /107 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 H Brg Width = 5.5 Min Req = 1.5 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 244 -1082 E - F 235 -772 C - D 251 -859 F - G 290 -1149 D - E 256 -881 G - H 224 -1181 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - L 849 -112 K - J 743 -79 L - K 870 -106 J - H 959 -119 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

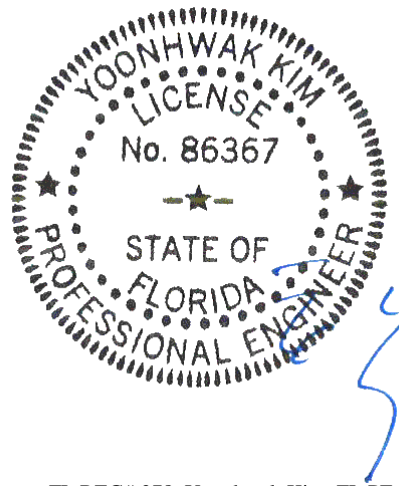
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 19.46
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 17.92
BC: From 5 plf at 17.92 to 5 plf at 19.46
BC: 75 lb Conc. Load at 9.63

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

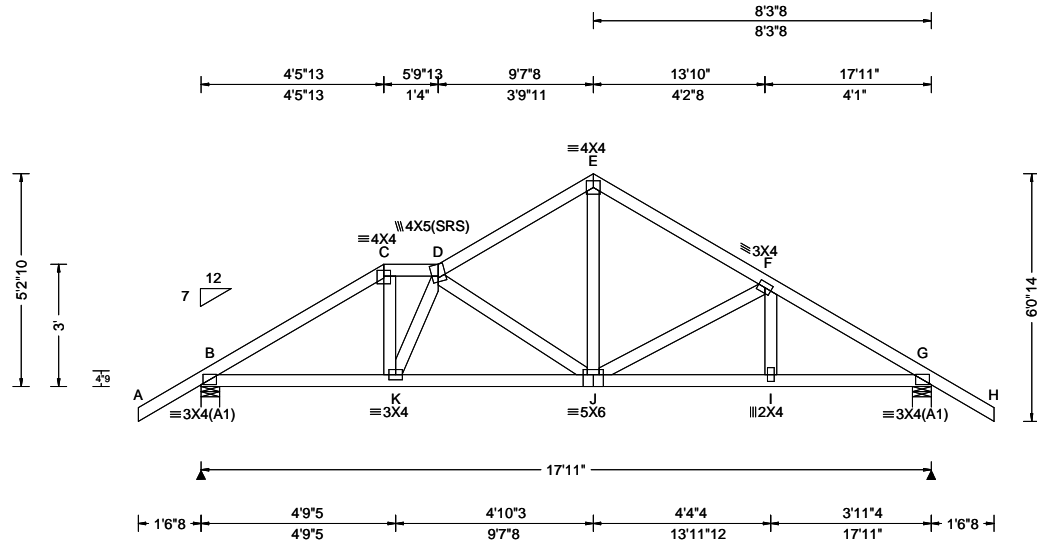
The overall height of this truss excluding overhang is 4-10-1.
WIND LOAD CASE MODIFIED!



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.033 J 999 240 VERT(CL): 0.067 J 999 180 HORZ(LL): 0.015 G - - HORZ(TL): 0.031 G - - Creep Factor: 2.0 Max TC CSI: 0.302 Max BC CSI: 0.473 Max Web CSI: 0.216 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>884</td> <td>-</td> <td>-</td> <td>/500</td> <td>/106</td> <td>/145</td> </tr> <tr> <td>G</td> <td>890</td> <td>-</td> <td>-</td> <td>/502</td> <td>/107</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 G Brg Width = 5.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>208 -1148</td> <td>E - F</td> <td>173 -908</td> </tr> <tr> <td>C - D</td> <td>212 -964</td> <td>F - G</td> <td>158 -1182</td> </tr> <tr> <td>D - E</td> <td>173 -895</td> <td></td> <td></td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 884 | - | - | /500 | /106 | /145 | G | 890 | - | - | /502 | /107 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | B - C | 208 -1148 | E - F | 173 -908 | C - D | 212 -964 | F - G | 158 -1182 | D - E | 173 -895 | | |
|--|--|---|---|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|------|------|---|-----|---|---|------|------|---|--------|------------|--------|-------------|-------|-----------|-------|----------|-------|----------|-------|-----------|-------|----------|--|--|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 884 | - | - | /500 | /106 | /145 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 890 | - | - | /502 | /107 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Chords | Tens.Comp. | Chords | Tens. Comp. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B - C | 208 -1148 | E - F | 173 -908 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C - D | 212 -964 | F - G | 158 -1182 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D - E | 173 -895 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

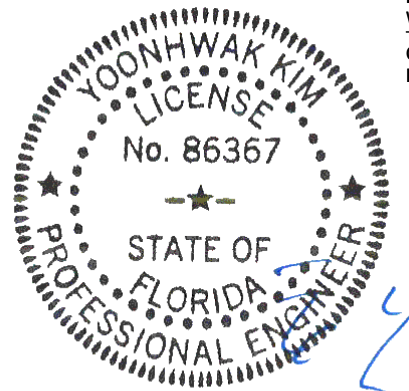
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 19.46
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 17.92
 BC: From 5 plf at 17.92 to 5 plf at 19.46
 BC: 75 lb Conc. Load at 9.63

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-2-10.
 WIND LOAD CASE MODIFIED!



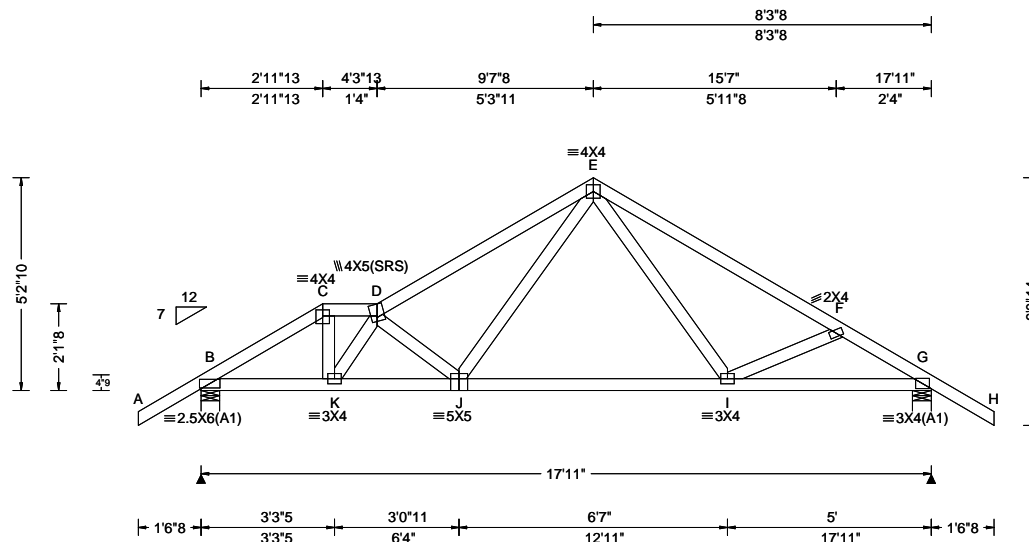
FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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6750 Forum Drive
 Suite 305
 Orlando FL, 32821



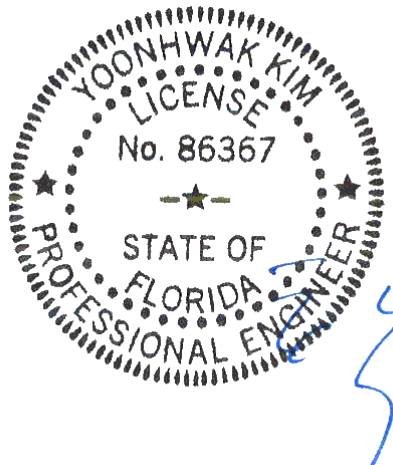
| | | | | |
|--|--|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.054 J 999 240 VERT(CL): 0.108 J 999 180 HORZ(LL): 0.020 G - - HORZ(TL): 0.040 G - - Creep Factor: 2.0 Max TC CSI: 0.504 Max BC CSI: 0.667 Max Web CSI: 0.343 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1296 /- /- /- /149 /- G 970 /- /- /- /118 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 G Brg Width = 5.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 185 -1834 E - F 115 -1253 C - D 156 -1661 F - G 173 -1428 D - E 154 -1606 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - K 1519 -143 J - I 843 -96 K - J 1931 -195 I - G 1193 -142 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K 899 -78 D - J 109 -814 K - D 79 -614 J - E 867 -33 |
|--|--|---|---|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 19.46
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 17.92
BC: From 5 plf at 17.92 to 5 plf at 19.46
BC: 492 lb Conc. Load at 3.02
BC: 75 lb Conc. Load at 9.73

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

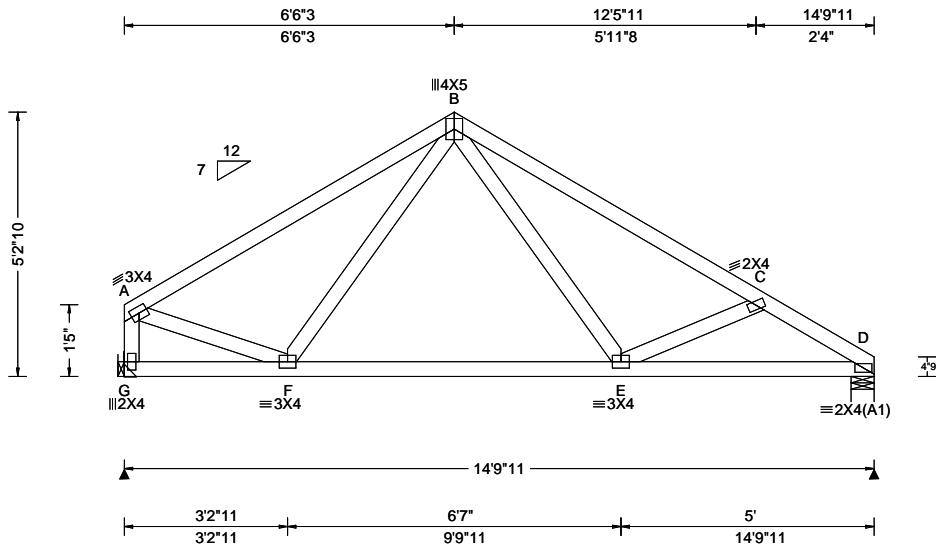
Additional Notes
The overall height of this truss excluding overhang is 5-2-10.
WIND LOAD CASE MODIFIED!



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| | | | | |
|---|---|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.019 E 999 240 VERT(CL): 0.038 E 999 180 HORZ(LL): 0.008 D - - HORZ(TL): 0.016 D - - Creep Factor: 2.0 Max TC CSI: 0.713 Max BC CSI: 0.501 Max Web CSI: 0.201 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 650 /- /- /332 /68 /106 D 657 /- /- /357 /69 /- Wind reactions based on MWFRS G Brg Width = - Min Req = - D Brg Width = 5.5 Min Req = 1.5 Bearing D is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 112 -698 C - D 221 -1100 B - C 167 -905 |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at 0.00 to 63 plf at 14.81
BC: From 20 plf at 0.00 to 20 plf at 14.81
BC: 75 lb Conc. Load at 6.84

Hangers / Ties

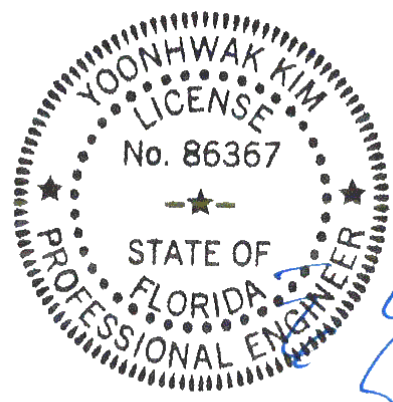
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

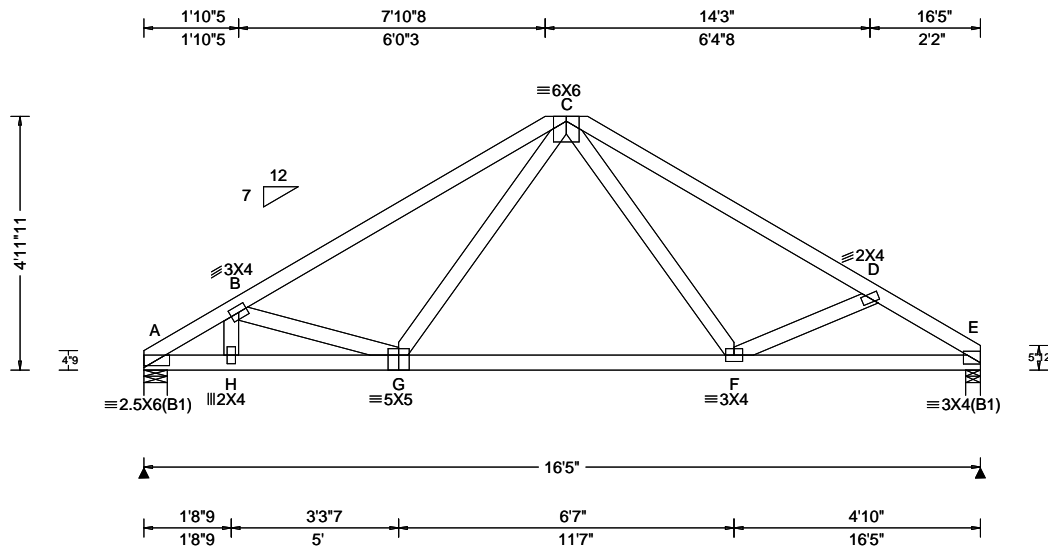
The overall height of this truss excluding overhang is 5'-2-10.
WIND LOAD CASE MODIFIED!



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| | | | | |
|--|--|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.030 G 999 240 VERT(CL): 0.062 G 999 180 HORZ(LL): 0.013 E - - HORZ(TL): 0.027 E - - Creep Factor: 2.0 Max TC CSI: 0.586 Max BC CSI: 0.282 Max Web CSI: 0.196 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1143 -/- /- /119 -/ E 765 -/- /- /80 -/ Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 E Brg Width = 3.5 Min Req = 1.5 Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 214 -1835 C - D 106 -1083 B - C 127 -1244 D - E 166 -1257 |
|--|--|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at 0.00 to 63 plf at 16.42
BC: From 20 plf at 0.00 to 20 plf at 16.42
BC: 468 lb Conc. Load at 1.71
BC: 75 lb Conc. Load at 8.29

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

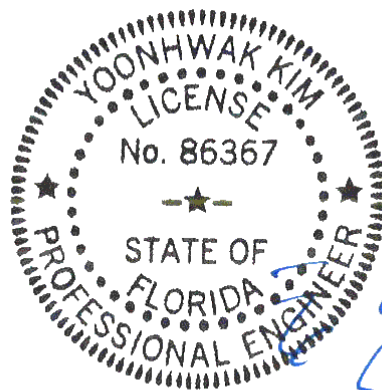
The overall height of this truss excluding overhang is 4-11-11.
WIND LOAD CASE MODIFIED!

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| A - H | 1588 -192 | G - F | 714 -89 |
| H - G | 1571 -192 | F - E | 1051 -139 |

Maximum Web Forces Per Ply (lbs)

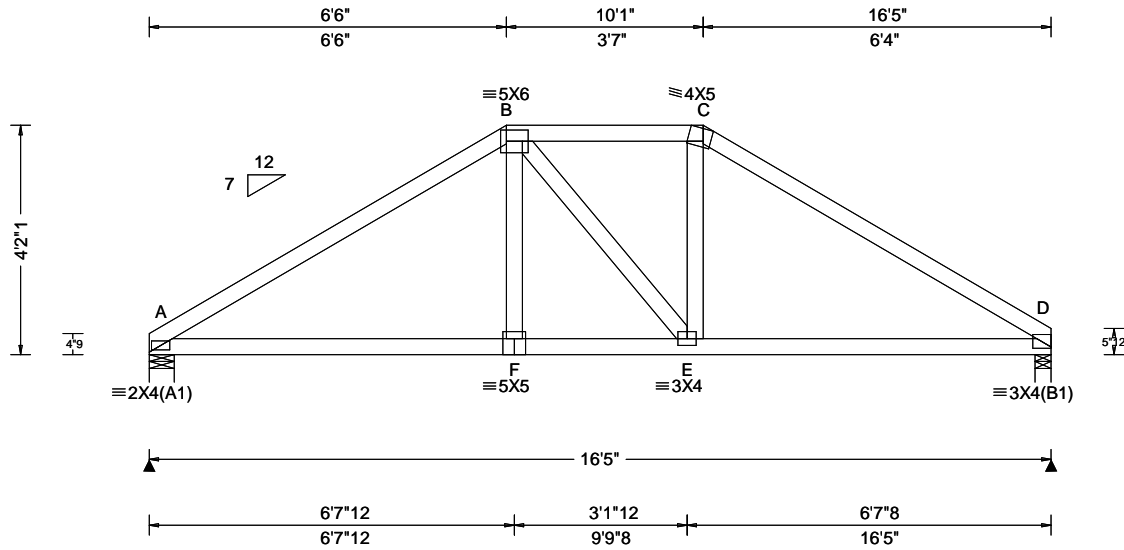
| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| B - G | 118 -658 | G - C | 514 0 |



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|--|--|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.013 F 999 240 VERT(CL): 0.027 F 999 180 HORZ(LL): 0.008 D - - HORZ(TL): 0.016 D - - Creep Factor: 2.0 Max TC CSI: 0.496 Max BC CSI: 0.426 Max Web CSI: 0.077 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 683 - / - / 387 / 2 / 83 D 682 - / - / 384 / 1 / - Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 D Brg Width = 3.5 Min Req = 1.5 Bearings A & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 298 -929 C - D 294 -916 B - C 294 -714 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - F 721 -194 E - D 706 -178 F - E 725 -192 |
|--|--|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

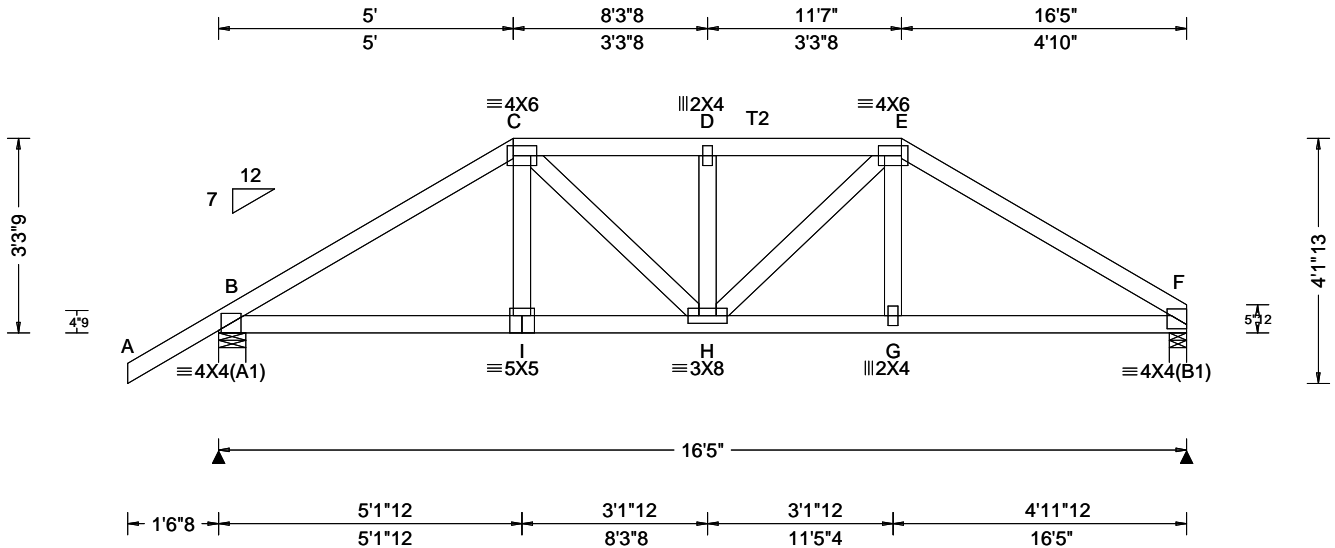
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | | |
|---|---|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.056 D 999 240 VERT(CL): 0.111 D 999 180 HORZ(LL): 0.025 F - - HORZ(TL): 0.050 F - - Creep Factor: 2.0 Max TC CSI: 0.468 Max BC CSI: 0.820 Max Web CSI: 0.203 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1441 -/- /- /- /217 -/ F 1453 -/- /- /- /210 -/ Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.7 F Brg Width = 3.5 Min Req = 1.7 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 326 -2165 D - E 327 -2097 C - D 327 -2097 E - F 325 -2042 |

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

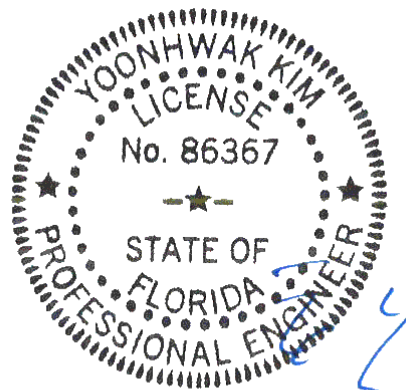
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 11.58
TC: From 63 plf at 11.58 to 63 plf at 16.42
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.15
BC: From 10 plf at 5.15 to 10 plf at 16.42
TC: 129 lb Conc. Load at 5.06, 7.06, 8.29, 9.54
11.54
BC: 388 lb Conc. Load at 5.03
BC: 90 lb Conc. Load at 7.06, 8.29, 9.54, 11.54
BC: 175 lb Conc. Load at 13.54, 15.54

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

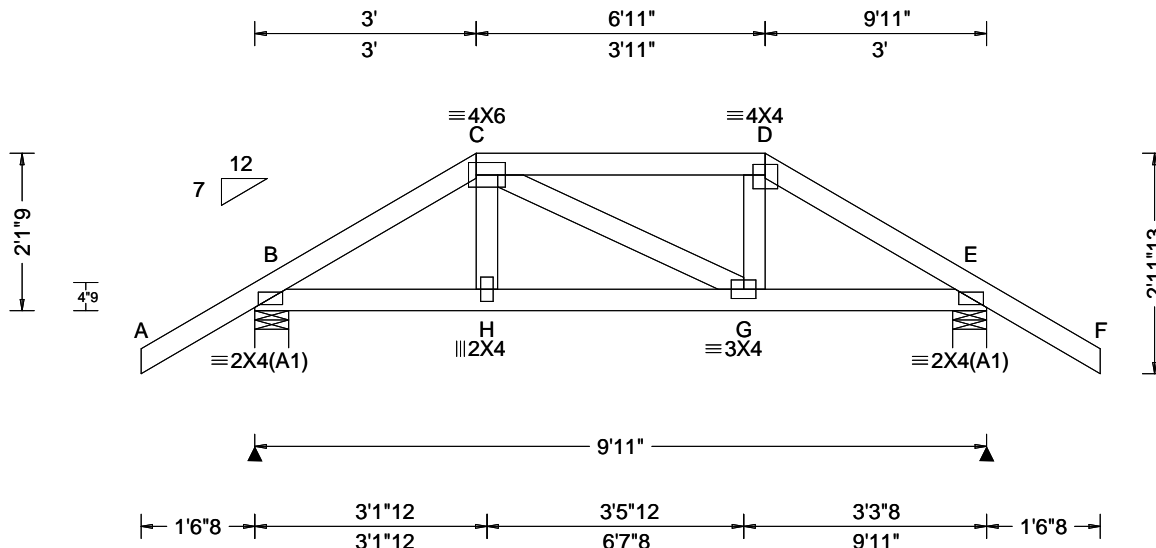
The overall height of this truss excluding overhang is 3-3-9.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|--|--|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 G 999 240 VERT(CL): 0.019 G 999 180 HORZ(LL): 0.005 E - - HORZ(TL): 0.011 E - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.224 Max Web CSI: 0.034 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 501 /- /- /- /76 /- E 501 /- /- /- /76 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 E Brg Width = 5.5 Min Req = 1.5 Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 89 -636 D - E 91 -635 C - D 64 -531 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - H 518 -69 G - E 517 -71 H - G 524 -66 |
|--|--|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

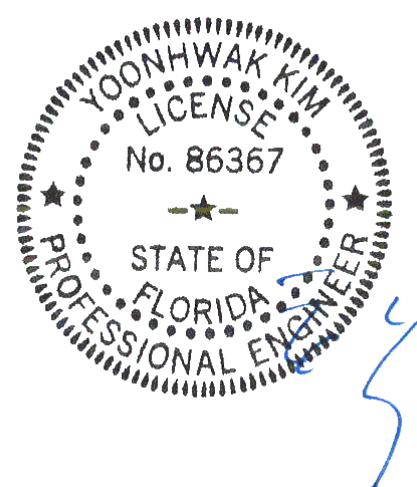
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 42 plf at -1.54 to 42 plf at 3.00
TC: From 21 plf at 3.00 to 21 plf at 6.92
TC: From 42 plf at 6.92 to 42 plf at 11.46
BC: From 3 plf at -1.54 to 3 plf at 0.00
BC: From 13 plf at 0.00 to 13 plf at 3.03
BC: From 7 plf at 3.03 to 7 plf at 6.89
BC: From 13 plf at 6.89 to 13 plf at 9.92
BC: From 3 plf at 9.92 to 3 plf at 11.46
TC: 93 lb Conc. Load at 3.03, 6.89
TC: 41 lb Conc. Load at 4.40, 5.52
BC: 42 lb Conc. Load at 3.03, 6.89
BC: 33 lb Conc. Load at 4.40, 5.52

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

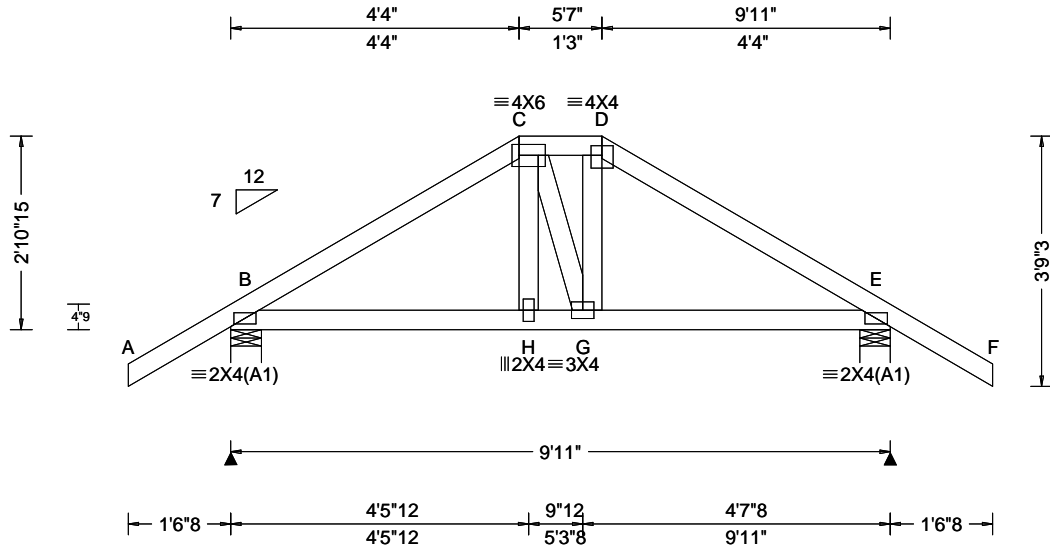
The overall height of this truss excluding overhang is 2-1-9.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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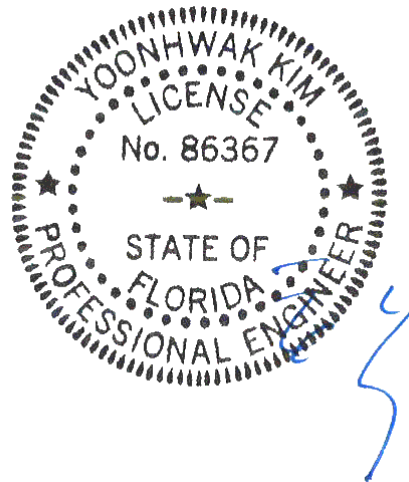
| | | | | |
|---|--|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.004 H 999 240 VERT(CL): 0.008 H 999 180 HORZ(LL): 0.002 E - - HORZ(TL): 0.003 E - - Creep Factor: 2.0 Max TC CSI: 0.123 Max BC CSI: 0.118 Max Web CSI: 0.026 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 345 /- /- /212 /41 /63 E 345 /- /- /212 /41 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 E Brg Width = 5.5 Min Req = 1.5 Bearings B & E are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

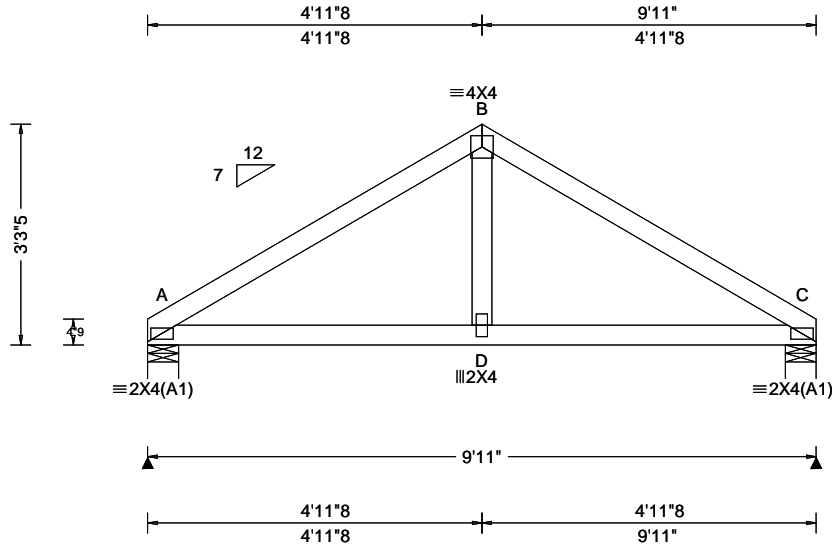
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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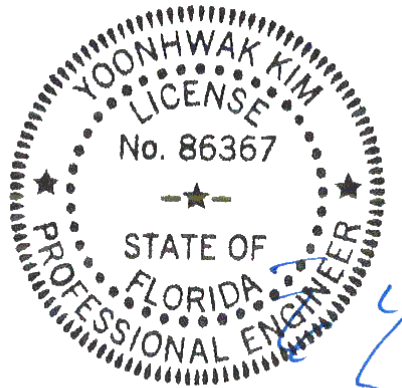
| | | | | |
|---|--|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.003 D 999 240 VERT(CL): 0.006 D 999 180 HORZ(LL): 0.002 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.154 Max BC CSI: 0.163 Max Web CSI: 0.055 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 275 /- /- /156 /27 /42 C 275 /- /- /156 /27 /- Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 C Brg Width = 5.5 Min Req = 1.5 Bearings A & C are a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

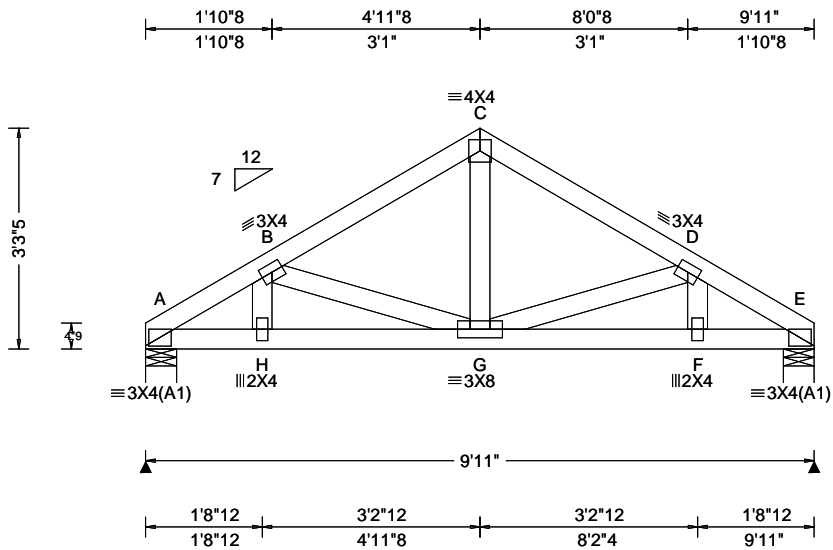
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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09/16/2021

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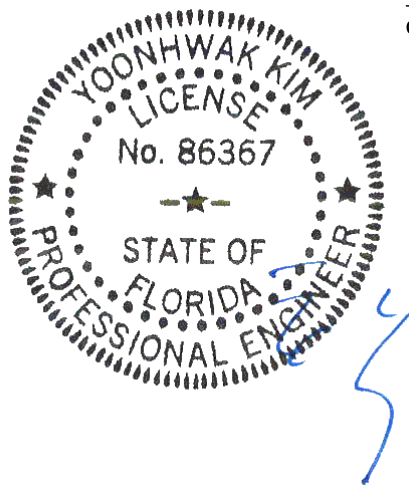


| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.016 G 999 240 VERT(CL): 0.032 G 999 180 HORZ(LL): 0.007 E - - HORZ(TL): 0.014 E - - Creep Factor: 2.0 Max TC CSI: 0.141 Max BC CSI: 0.582 Max Web CSI: 0.239 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 722 /- /- /14 /- /- E 681 /- /- /10 /- /- Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 E Brg Width = 5.5 Min Req = 1.5 Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 0 -1157 C - D 0 -818 B - C 0 -817 D - E 0 -1120 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - H 964 0 G - F 919 0 H - G 949 0 F - E 932 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - G 627 0 |
|--|---|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 42 plf at 0.00 to 42 plf at 9.92
BC: From 13 plf at 0.00 to 13 plf at 9.92
BC: 213 lb Conc. Load at 1.73, 3.73, 5.73, 7.73

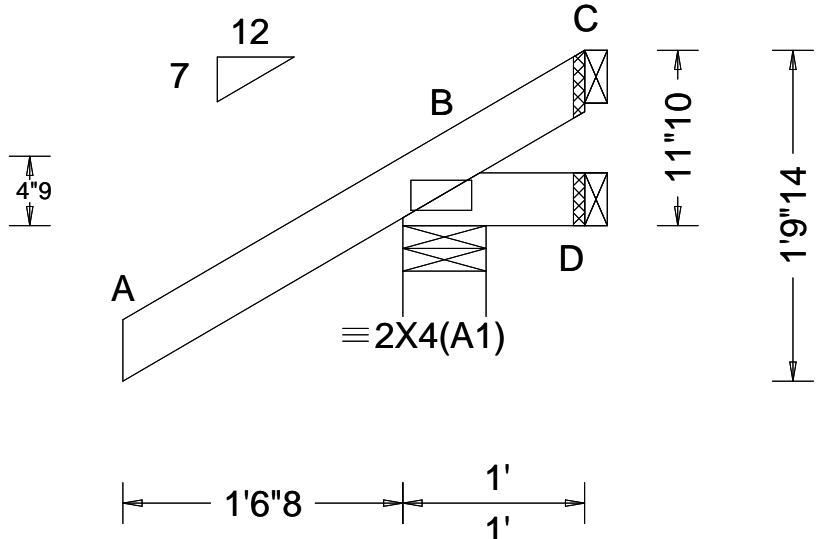
Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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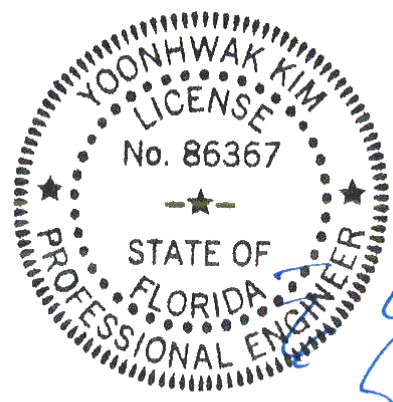
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp1: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.216 Max BC CSI: 0.035 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>265</td> <td>/-</td> <td>/-</td> <td>/197</td> <td>/48</td> <td>/38</td> </tr> <tr> <td>D</td> <td>5</td> <td>/-17</td> <td>/-</td> <td>/12</td> <td>/13</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-60</td> <td>/-</td> <td>/30</td> <td>/53</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 265 | /- | /- | /197 | /48 | /38 | D | 5 | /-17 | /- | /12 | /13 | /- | C | - | /-60 | /- | /30 | /53 | /- |
|--|---|---|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|-----|-----|---|---|------|----|-----|-----|----|---|---|------|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 265 | /- | /- | /197 | /48 | /38 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 5 | /-17 | /- | /12 | /13 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /-60 | /- | /30 | /53 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

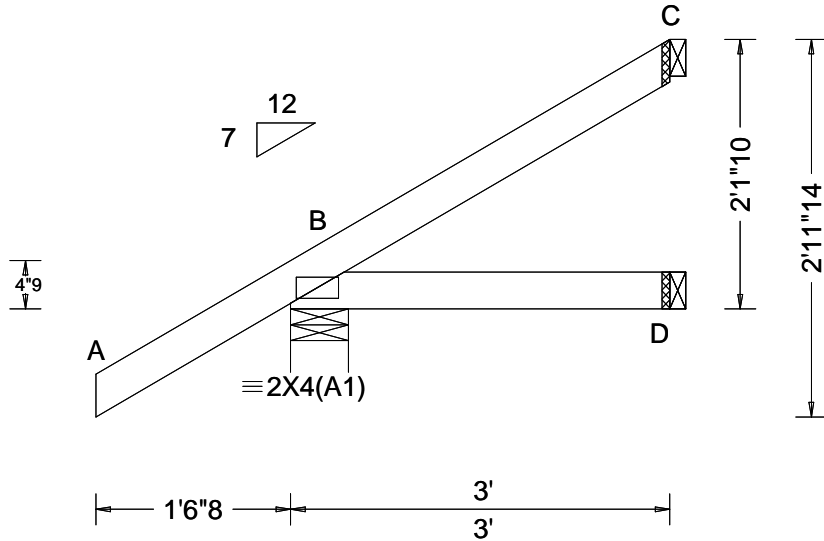
Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



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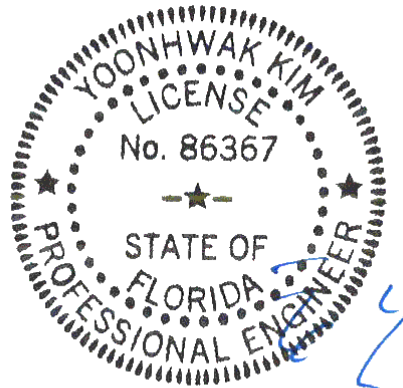
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|--------------------------------|---|-----|-----|-------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: | Wind Std: | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| 20.00 | ASCE 7-16 | Pf: NA Ce: NA | VERT(LL): NA | B | 269 | - | - | /184 | /23 | /73 |
| TCDL: 10.00 | Speed: 120 mph | Lu: NA Cs: NA | VERT(CL): NA | D | 50 | - | - | /31 | - | - |
| BCLL: 0.00 | Enclosure: Closed | Snow Duration: NA | HORZ(LL): 0.001 B - - | C | 62 | - | - | /35 | /31 | - |
| BCDL: 10.00 | Risk Category: II | Building Code: | HORZ(TL): 0.001 B - - | Wind reactions based on MWFRS | | | | | | |
| Des Ld: 40.00 | EXP: C Kzt: NA | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | B Brg Width = 5.5 Min Req = 1.5 | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | TPI Std: 2014 | Max TC CSI: 0.216 | D Brg Width = 1.5 Min Req = - | | | | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.066 | C Brg Width = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Bearing B is a rigid surface. | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | Members not listed have forces less than 375# | | | | | | |
| | C&C Dist a: 3.00 ft | WAVE | | | | | | | | |
| | Loc. from endwall: Any | | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

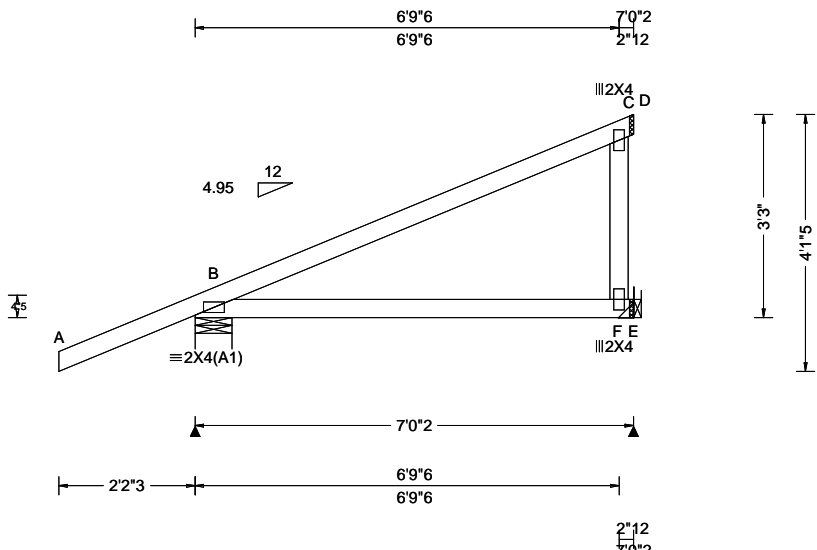
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| | | | | |
|---|---|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 B - - HORZ(TL): 0.011 B - - Creep Factor: 2.0 Max TC CSI: 0.586 Max BC CSI: 0.254 Max Web CSI: 0.189 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 311 /- /- /- /48 /- E 292 /- /- /- /27 /- Wind reactions based on MWFRS B Brg Width = 7.1 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|---|---|--|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

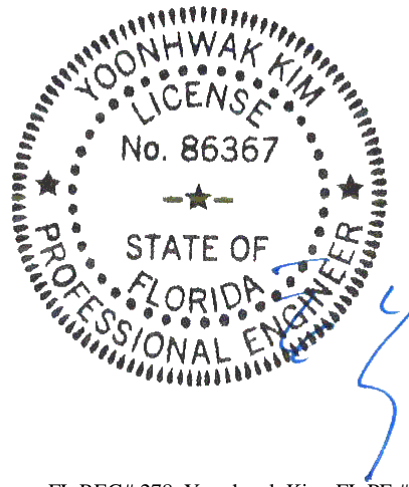
Hipjack supports 4-11-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

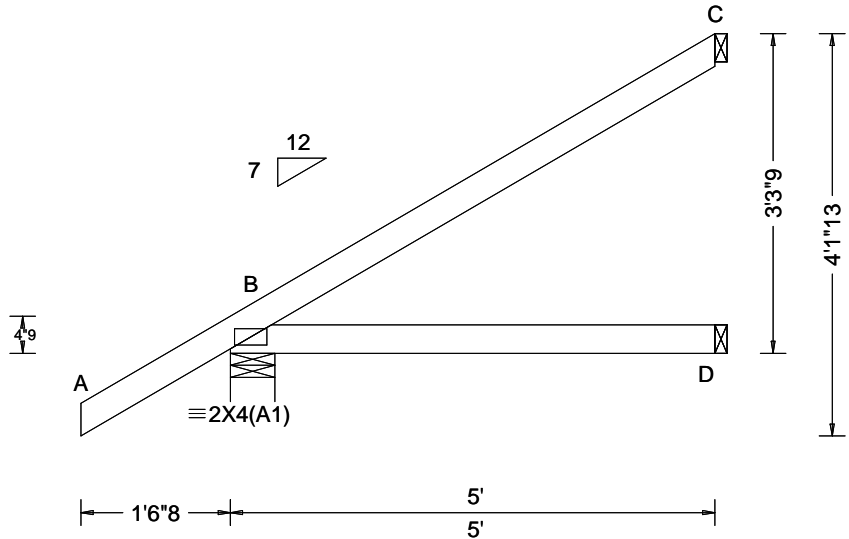
The overall height of this truss excluding overhang is 3-3-0.



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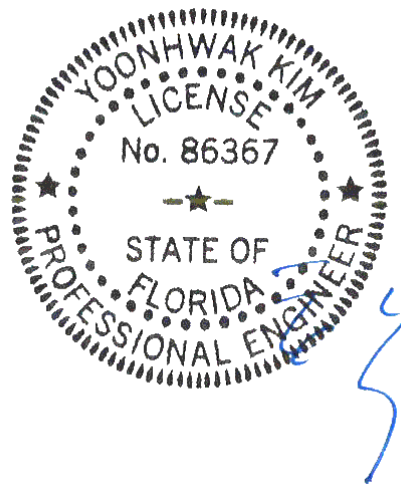
| | | | | |
|---|---|--|--|---|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.007 B - - Creep Factor: 2.0 Max TC CSI: 0.318 Max BC CSI: 0.238 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 338 /- /- /223 /18 /109 D 90 /- /- /51 /- /- C 129 /- /- /78 /58 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

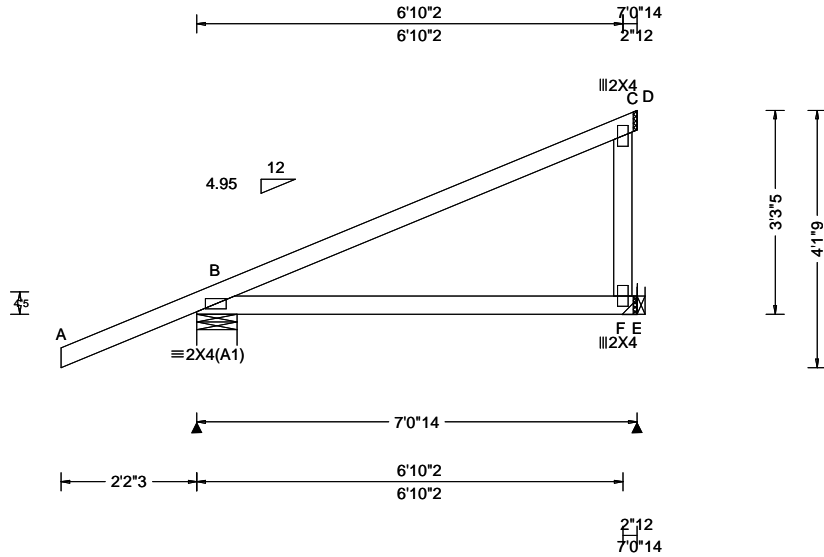
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 B - - HORZ(TL): 0.011 B - - Creep Factor: 2.0 Max TC CSI: 0.605 Max BC CSI: 0.263 Max Web CSI: 0.197 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>313</td> <td>-</td> <td>-</td> <td>-</td> <td>/48</td> <td>-</td> </tr> <tr> <td>E</td> <td>298</td> <td>-</td> <td>-</td> <td>-</td> <td>/28</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 313 | - | - | - | /48 | - | E | 298 | - | - | - | /28 | - |
|---|---|--|--|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|-----|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 313 | - | - | - | /48 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 298 | - | - | - | /28 | - | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

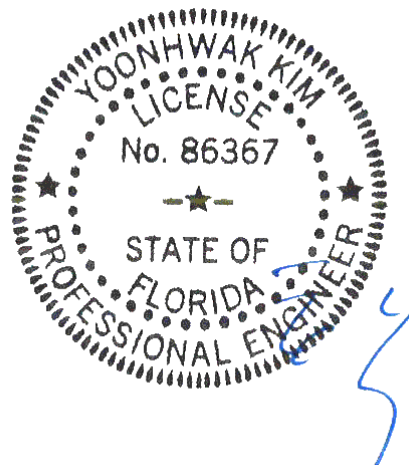
Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

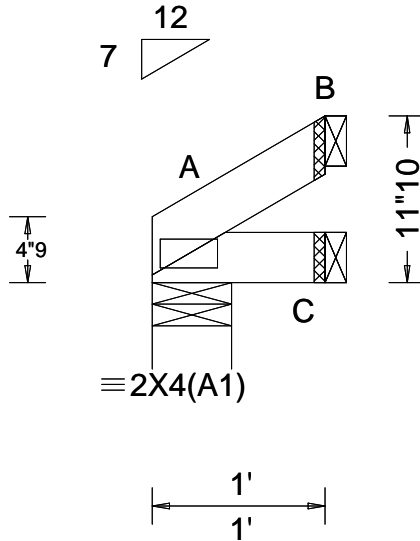
The overall height of this truss excluding overhang is 3-3-5.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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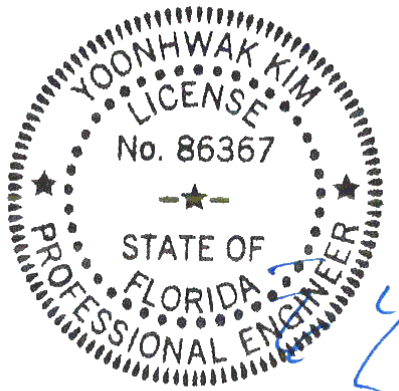
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|--------------------------------|---|----|-----|-------------|-----|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | A | 49 | /- | /- | /29 | /- | /16 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | C | 16 | /- | /- | /10 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 A - - | B | 24 | /- | /- | /15 | /11 | /- |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.000 A - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | A Brg Width = 5.5 Min Req = 1.5 | | | | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.012 | C Brg Width = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.006 | B Brg Width = 1.5 Min Req = - | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Bearing A is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: Any | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

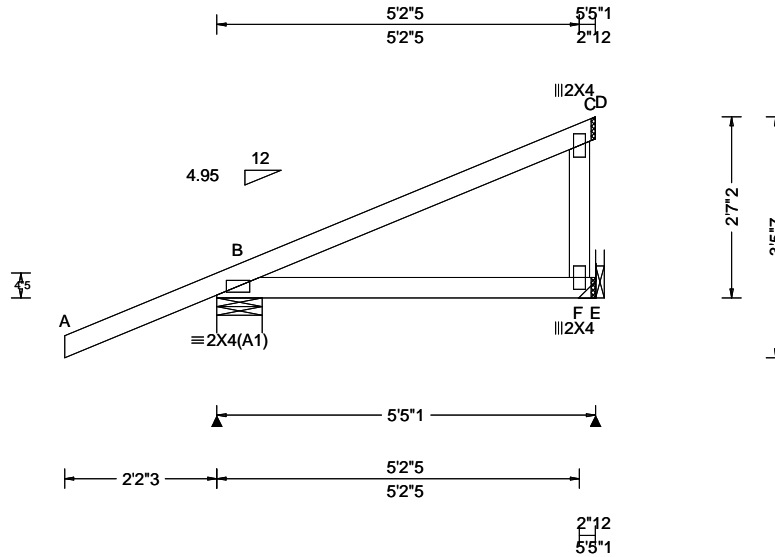
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.306 Max BC CSI: 0.089 Max Web CSI: 0.056 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>217</td> <td>-</td> <td>-</td> <td>-</td> <td>/35</td> <td>-</td> </tr> <tr> <td>E</td> <td>162</td> <td>-</td> <td>-</td> <td>-</td> <td>/14</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 217 | - | - | - | /35 | - | E | 162 | - | - | - | /14 | - |
|---|---|--|--|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|-----|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 217 | - | - | - | /35 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 162 | - | - | - | /14 | - | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

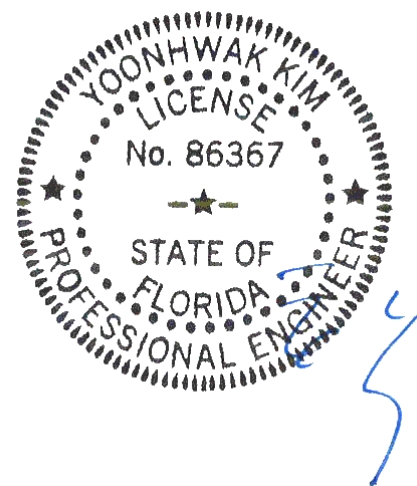
Hipjack supports 3-10-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

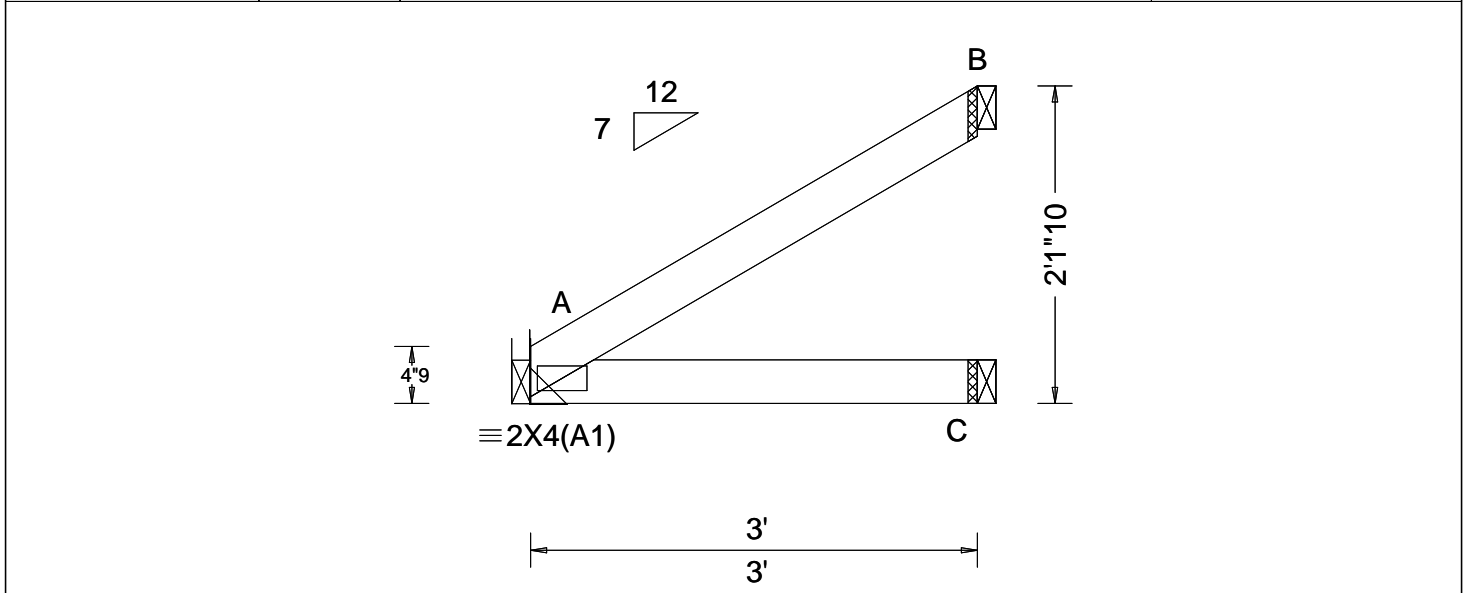
The overall height of this truss excluding overhang is 2-7-2.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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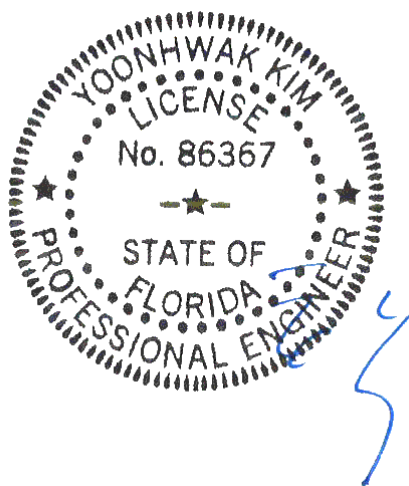




| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.119 Max BC CSI: 0.082 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 130 /- /- /78 /- /51 C 55 /- /- /32 /- /- B 83 /- /- /52 /36 /- Wind reactions based on MWFRS A Brg Width = - Min Req = - C Brg Width = 1.5 Min Req = - B Brg Width = 1.5 Min Req = - Members not listed have forces less than 375# |
|--|---|---|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

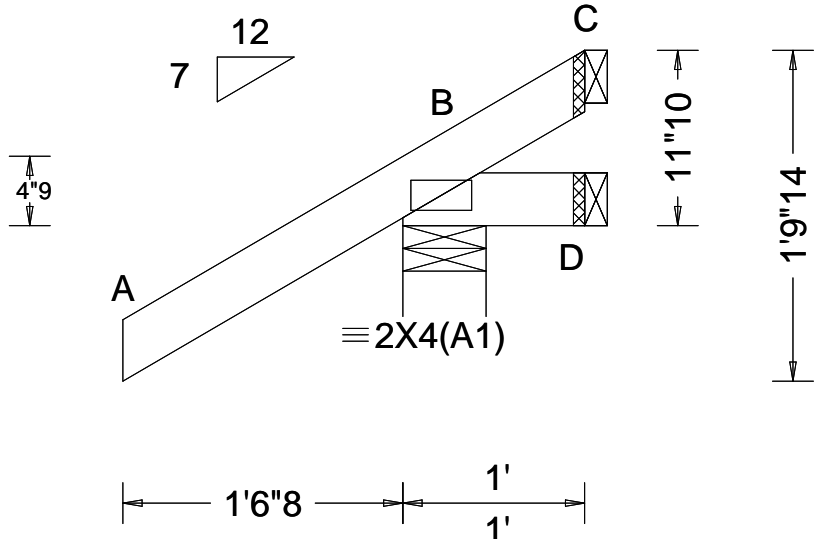
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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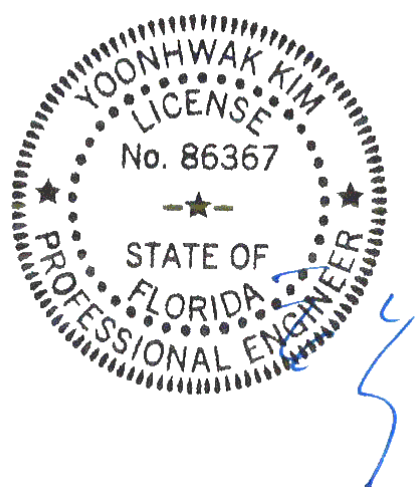
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 16.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.144 Max BC CSI: 0.023 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>177</td> <td>/-</td> <td>/-</td> <td>/132</td> <td>/32</td> <td>/26</td> </tr> <tr> <td>D</td> <td>3</td> <td>/-11</td> <td>/-</td> <td>/8</td> <td>/9</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-40</td> <td>/-</td> <td>/20</td> <td>/36</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 177 | /- | /- | /132 | /32 | /26 | D | 3 | /-11 | /- | /8 | /9 | /- | C | - | /-40 | /- | /20 | /36 | /- |
|--|--|---|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|-----|-----|---|---|------|----|----|----|----|---|---|------|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 177 | /- | /- | /132 | /32 | /26 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 3 | /-11 | /- | /8 | /9 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /-40 | /- | /20 | /36 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

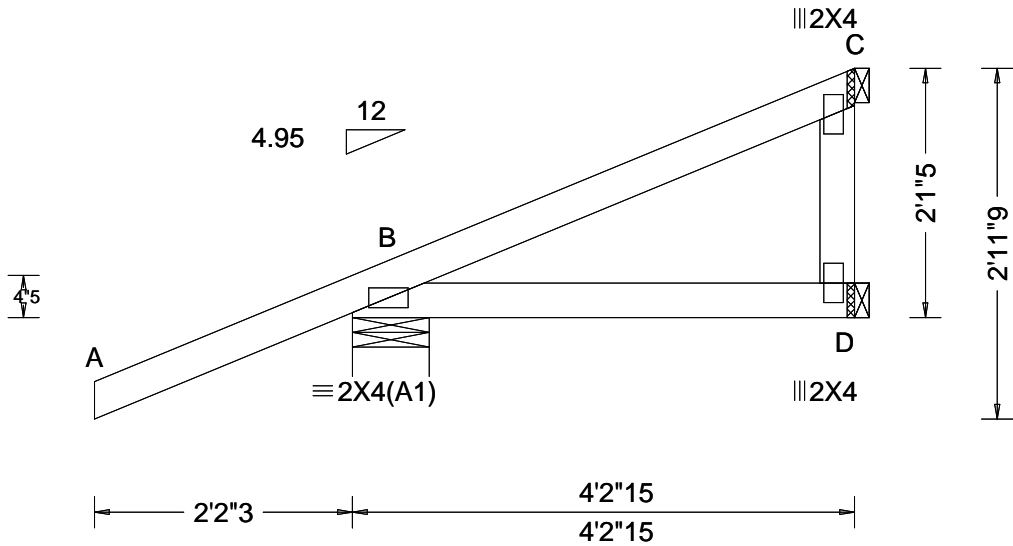
Wind loads based on MWFRS with additional C&C member design.
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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg, Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|-------------------------------|--------------------------------|---|-----|-----|-------------|-----|-----|----|
| | | | | Gravity | | | Non-Gravity | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | B | 233 | /- | /- | /- | /42 | /- |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | D | 9 | /- | /- | /15 | /- | /- |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | C | 52 | /- | /- | /- | /16 | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.002 B - - | Wind reactions based on MWFRS | | | | | | |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.004 B - - | B Brg Width = 7.8 Min Req = 1.5 | | | | | | |
| NCBCLL: 0.00 | Mean Height: 15.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | D Brg Width = 1.5 Min Req = - | | | | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.458 | C Brg Width = 1.5 Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: No | Max BC CSI: 0.153 | Bearing B is a rigid surface. | | | | | | |
| Spacing: 16.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT: 20(0)/10(0) | Max Web CSI: 0.007 | Members not listed have forces less than 375# | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | | | | | | | |
| | Loc. from endwall: NA | WAVE | | | | | | | | |
| | GCp1: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

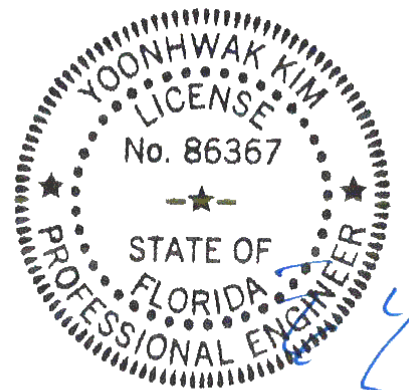
Hipjack supports 3-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

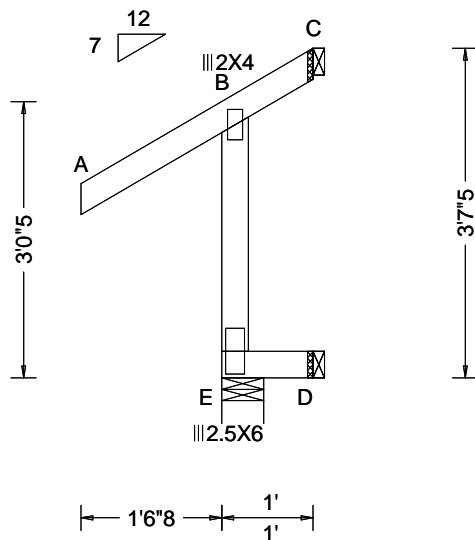
The overall height of this truss excluding overhang is 2-1-5.



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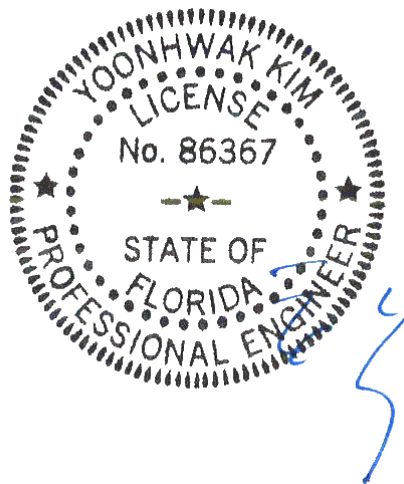
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|--|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|-----|----|---|----|----|----|-----|----|----|---|---|------|----|-----|-----|-----|
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.87 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp1: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.222 Max BC CSI: 0.010 Max Web CSI: 0.114 VIEW Ver: 21.01.01A.0521.20 | <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>227</td> <td>/-</td> <td>/-</td> <td>/189</td> <td>/69</td> <td>/-</td> </tr> <tr> <td>D</td> <td>20</td> <td>/-</td> <td>/-</td> <td>/10</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-49</td> <td>/-</td> <td>/47</td> <td>/67</td> <td>/41</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS E Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp.</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | E | 227 | /- | /- | /189 | /69 | /- | D | 20 | /- | /- | /10 | /- | /- | C | - | /-49 | /- | /47 | /67 | /41 |
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 227 | /- | /- | /189 | /69 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 20 | /- | /- | /10 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /-49 | /- | /47 | /67 | /41 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

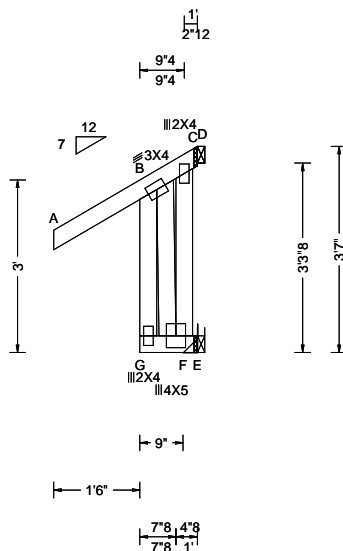
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|--|--|---|-------------|-----|---------|--|--|-------------|--|--|-----|----|-----|-----|-----|----|-----|---|-----|----|-----|------|-----|-----|---|---|----|-----|----|----|-----|
| TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.85 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.000 G 999 240 VERT(CL): 0.001 G 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.209 Max BC CSI: 0.008 Max Web CSI: 0.099 VIEW Ver: 21.01.01A.0521.20 | <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2"></th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>185</td> <td>/-</td> <td>/62</td> <td>/143</td> <td>/26</td> <td>/55</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-</td> <td>/62</td> <td>/-</td> <td>/-</td> <td>/33</td> </tr> </tbody> </table> Wind reactions based on MWFRS E Brg Width = - Min Req = - C Brg Width = 1.5 Min Req = - Members not listed have forces less than 375# | | | Gravity | | | Non-Gravity | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | E | 185 | /- | /62 | /143 | /26 | /55 | C | - | /- | /62 | /- | /- | /33 |
| | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 185 | /- | /62 | /143 | /26 | /55 | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /- | /62 | /- | /- | /33 | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

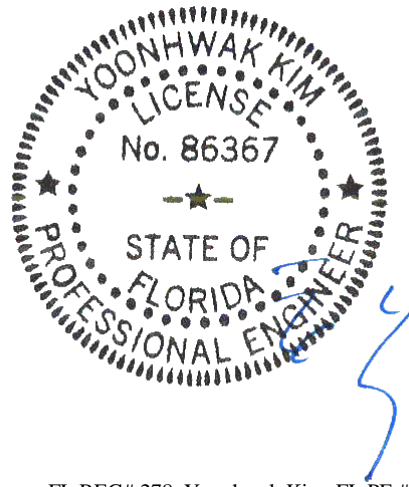
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

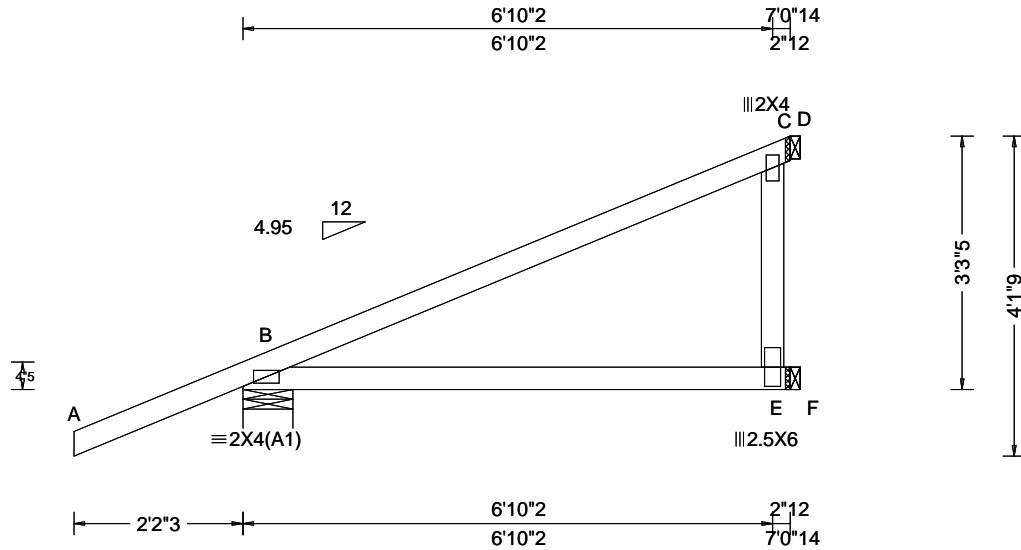
***** BEARING ANALOG MODIFIED! *****



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp1: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.595 Max BC CSI: 0.301 Max Web CSI: 0.195 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>317</td> <td>-</td> <td>-</td> <td>-</td> <td>/48</td> <td>-</td> </tr> <tr> <td>E</td> <td>442</td> <td>-</td> <td>-</td> <td>-</td> <td>/205</td> <td>-</td> </tr> <tr> <td>C</td> <td>-</td> <td>-147</td> <td>-</td> <td>/178</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 317 | - | - | - | /48 | - | E | 442 | - | - | - | /205 | - | C | - | -147 | - | /178 | - | - |
|---|--|--|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|------|---|---|---|------|---|------|---|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 317 | - | - | - | /48 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 442 | - | - | - | /205 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | -147 | - | /178 | - | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

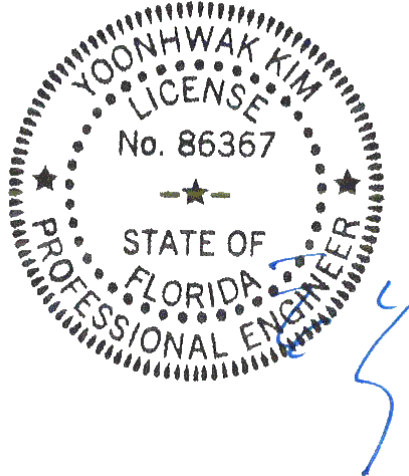
Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

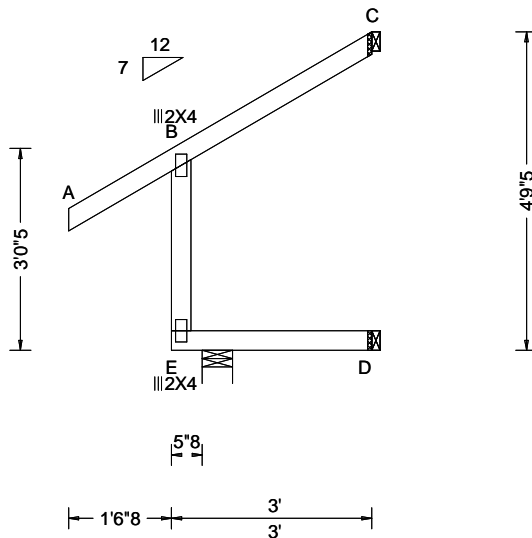
The overall height of this truss excluding overhang is 3-3-5.



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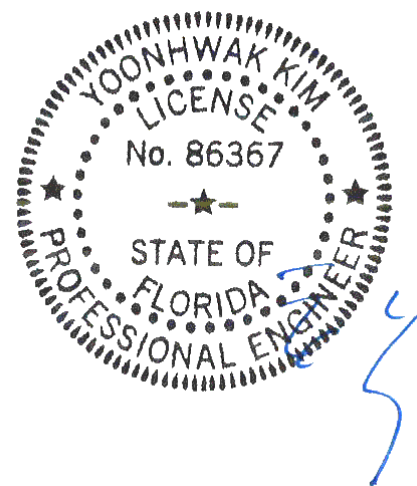
| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.45 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.016 B 509 240 VERT(CL): 0.031 B 261 180 HORZ(LL): 0.009 B - - HORZ(TL): 0.018 B - - Creep Factor: 2.0 Max TC CSI: 0.193 Max BC CSI: 0.313 Max Web CSI: 0.090 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 332 /- /- /273 /97 /- D 16 /-46 /- /37 /32 /- C 68 /- /- /41 /15 /81 Wind reactions based on MWFRS E Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

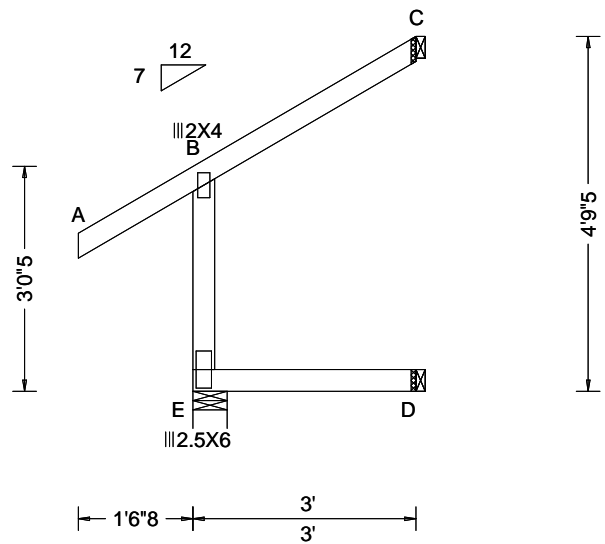
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.



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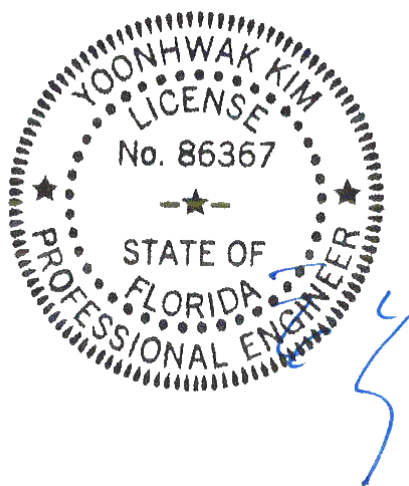
| | | | | |
|---|--|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.45 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.251 Max BC CSI: 0.098 Max Web CSI: 0.111 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 256 /- /- /204 /77 /- D 60 /- /- /30 /- /- C 68 /- /- /41 /15 /81 Wind reactions based on MWFRS E Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

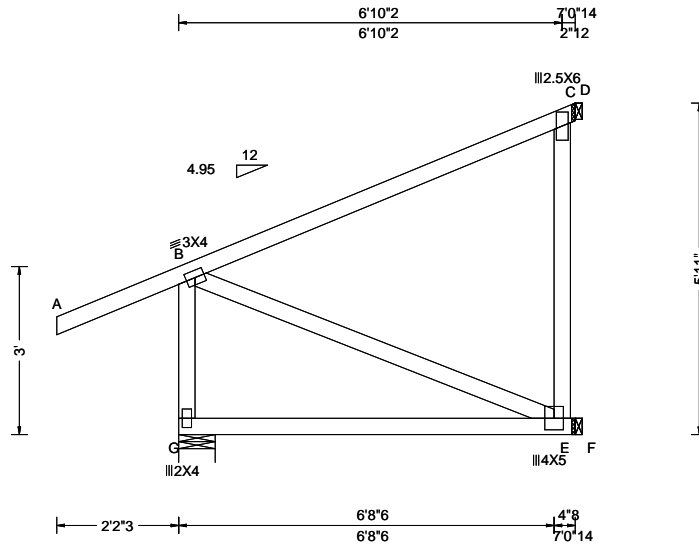
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.01 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.011 C 999 180 HORZ(LL): 0.003 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.702 Max BC CSI: 0.335 Max Web CSI: 0.218 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>310</td> <td>-</td> <td>-</td> <td>-</td> <td>/61</td> <td>-</td> </tr> <tr> <td>E</td> <td>436</td> <td>-</td> <td>-</td> <td>-</td> <td>/284</td> <td>-</td> </tr> <tr> <td>C</td> <td>-</td> <td>-/135</td> <td>-</td> <td>/243</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS G Brg Width = 7.8 Min Req = 1.5 E Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | G | 310 | - | - | - | /61 | - | E | 436 | - | - | - | /284 | - | C | - | -/135 | - | /243 | - | - |
|---|---|--|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|------|---|---|---|-------|---|------|---|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 310 | - | - | - | /61 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 436 | - | - | - | /284 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | -/135 | - | /243 | - | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

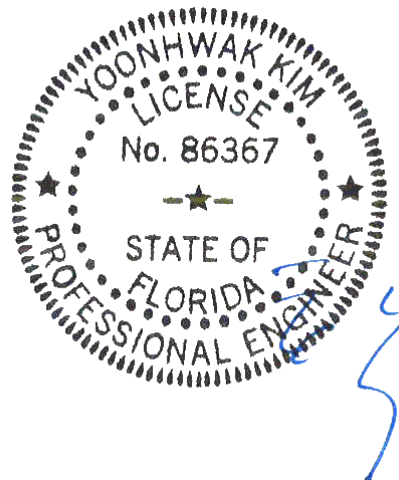
Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

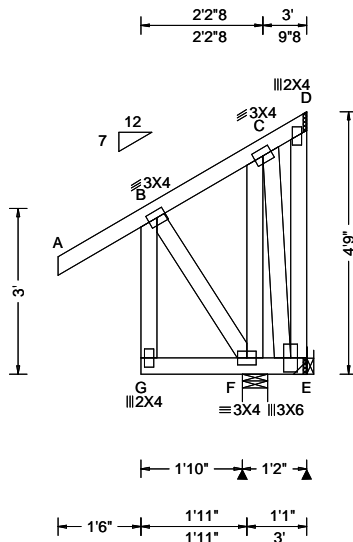
The overall height of this truss excluding overhang is 5-11-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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| | | | | |
|--|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.006 G 999 240 VERT(CL): 0.012 G 999 180 HORZ(LL): -0.012 D - - HORZ(TL): 0.023 D - - Creep Factor: 2.0 Max TC CSI: 0.177 Max BC CSI: 0.068 Max Web CSI: 0.161 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 697 /- /- /490 /42 /80 E - /-367 /- /30 /250 /- Wind reactions based on MWFRS F Brg Width = 5.5 Min Req = 1.5 E Brg Width = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. F - C 0 -483 |
|--|---|---|---|--|

Lumber

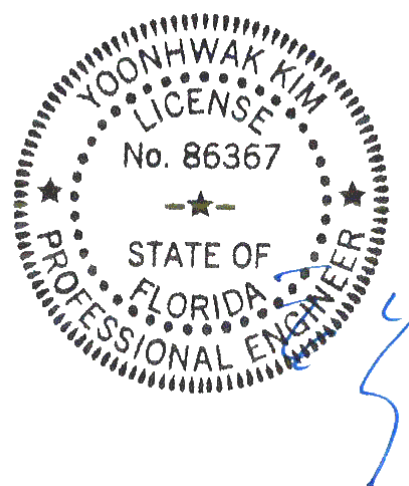
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes

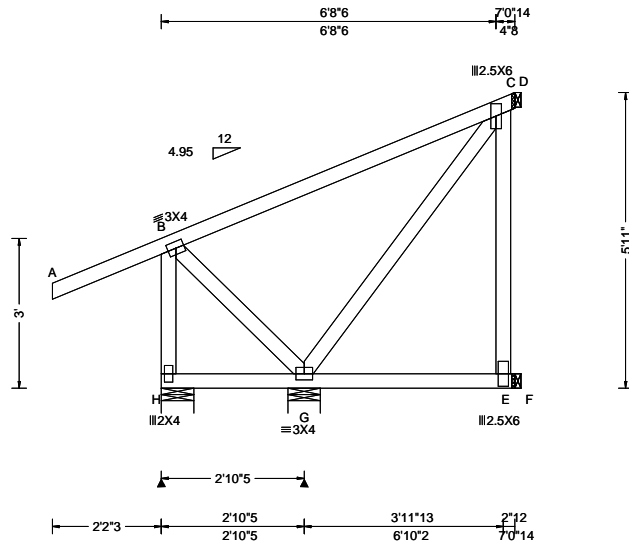
Negative reaction(s) of -367# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.



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| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.01 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.004 C 999 240 VERT(CL): 0.009 C 999 180 HORZ(LL): 0.003 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.674 Max BC CSI: 0.181 Max Web CSI: 0.322 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 61 /- /- /- /14 /- G 418 /- /- /- /80 /- E 564 /- /- /- /218 /- C - /-432 /- /210 /- /- Wind reactions based on MWFRS H Brg Width = 7.8 Min Req = 1.5 G Brg Width = 7.8 Min Req = 1.5 E Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearings H & G are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 249 -529 |
|---|---|--|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

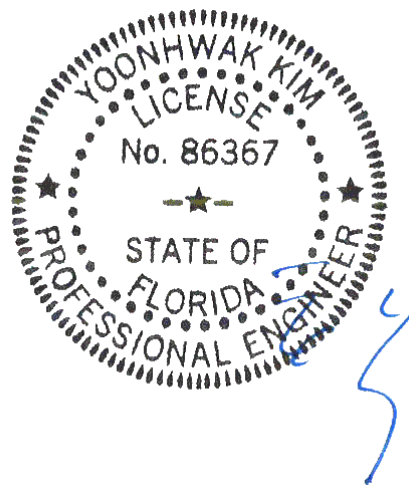
Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

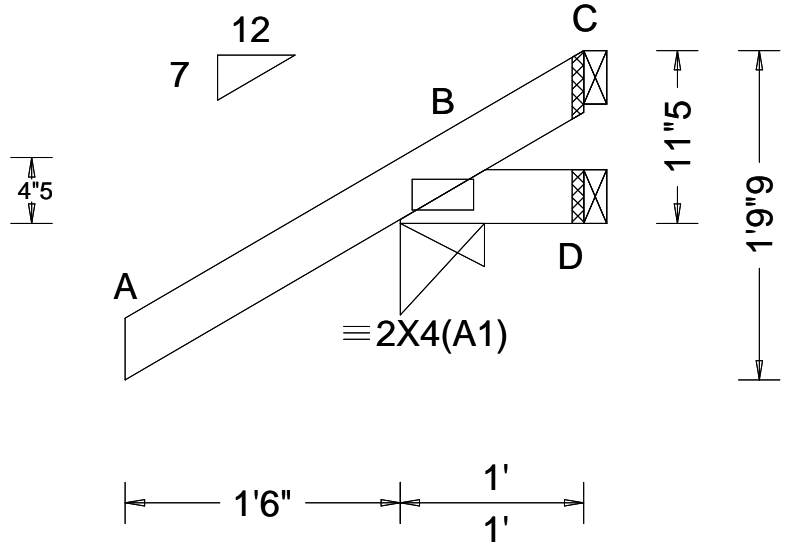
Negative reaction(s) of -432# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
The overall height of this truss excluding overhang is 5-11-0.



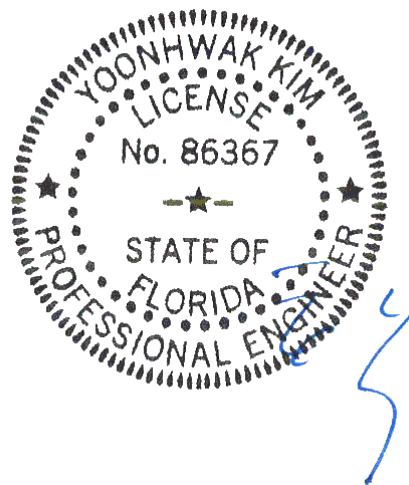
FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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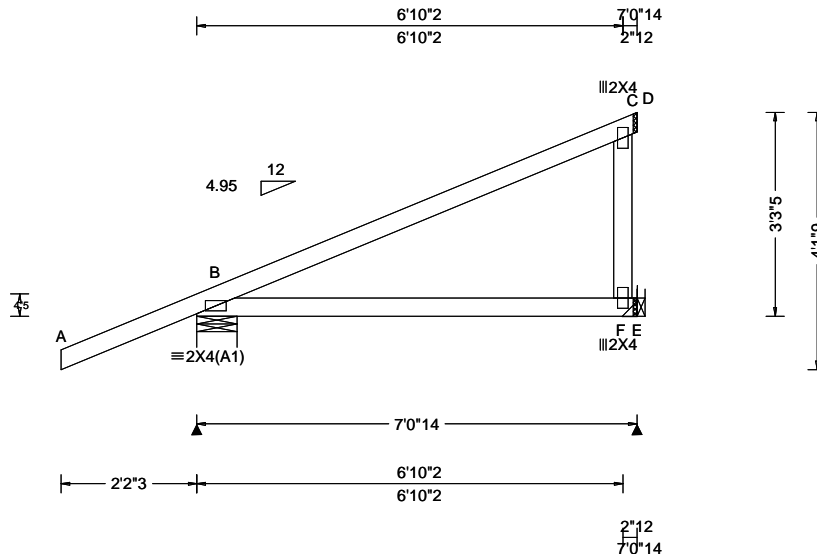
| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.203 Max BC CSI: 0.032 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>257</td> <td>/-</td> <td>/-</td> <td>/191</td> <td>/46</td> <td>/38</td> </tr> <tr> <td>D</td> <td>5</td> <td>/-17</td> <td>/-</td> <td>/13</td> <td>/13</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-55</td> <td>/-</td> <td>/28</td> <td>/49</td> <td>/-</td> </tr> </tbody> </table> Non-Gravity B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | B | 257 | /- | /- | /191 | /46 | /38 | D | 5 | /-17 | /- | /13 | /13 | /- | C | - | /-55 | /- | /28 | /49 | /- |
|--|--|---|---|--|-----|-----|-----|-----|-----|-----|-----|---|-----|----|----|------|-----|-----|---|---|------|----|-----|-----|----|---|---|------|----|-----|-----|----|
| | | | | Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | |
| B | 257 | /- | /- | /191 | /46 | /38 | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 5 | /-17 | /- | /13 | /13 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | - | /-55 | /- | /28 | /49 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | | |
|---|---|--|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 B - - HORZ(TL): 0.011 B - - Creep Factor: 2.0 Max TC CSI: 0.605 Max BC CSI: 0.263 Max Web CSI: 0.197 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 313 /- /- /- /48 /- E 298 /- /- /- /28 /- Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|---|---|--|---|---|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

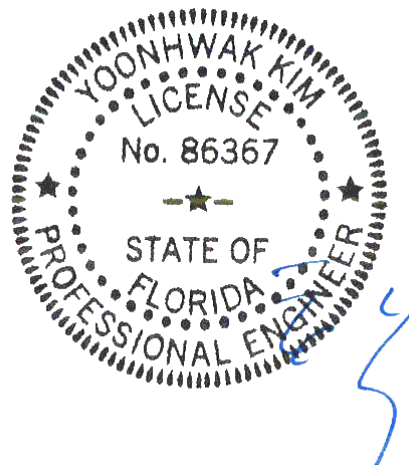
Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

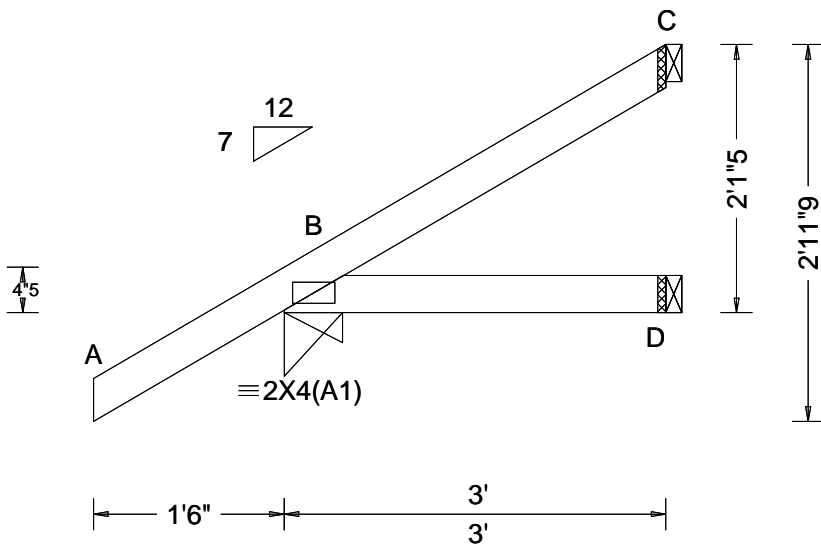
The overall height of this truss excluding overhang is 3-3-5.



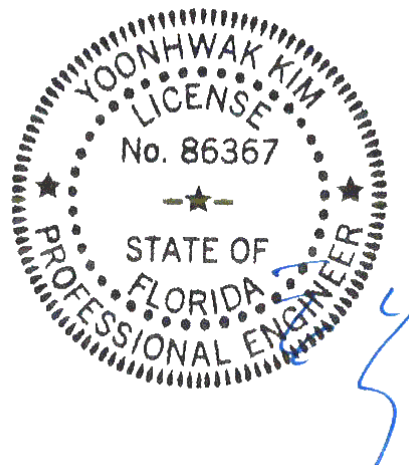
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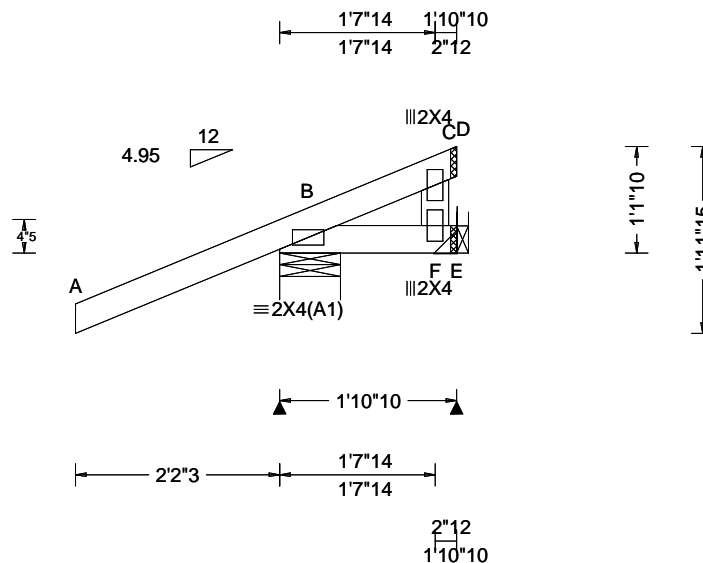
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.163 Max BC CSI: 0.065 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>265</td> <td>-</td> <td>-</td> <td>/180</td> <td>/22</td> <td>/73</td> </tr> <tr> <td>D</td> <td>50</td> <td>-</td> <td>-</td> <td>/31</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>63</td> <td>-</td> <td>-</td> <td>/36</td> <td>/31</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 265 | - | - | /180 | /22 | /73 | D | 50 | - | - | /31 | - | - | C | 63 | - | - | /36 | /31 | - |
|--|---|---|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|-----|-----|---|----|---|---|-----|---|---|---|----|---|---|-----|-----|---|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 265 | - | - | /180 | /22 | /73 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 50 | - | - | /31 | - | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 63 | - | - | /36 | /31 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.134 Max BC CSI: 0.020 Max Web CSI: 0.019 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>159</td> <td>-</td> <td>-</td> <td>-</td> <td>/42</td> <td>-</td> </tr> <tr> <td>E</td> <td>52</td> <td>-</td> <td>-</td> <td>-</td> <td>/67</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 159 | - | - | - | /42 | - | E | 52 | - | - | - | /67 | - |
|--|---|---|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|----|---|---|---|-----|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 159 | - | - | - | /42 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 52 | - | - | - | /67 | - | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

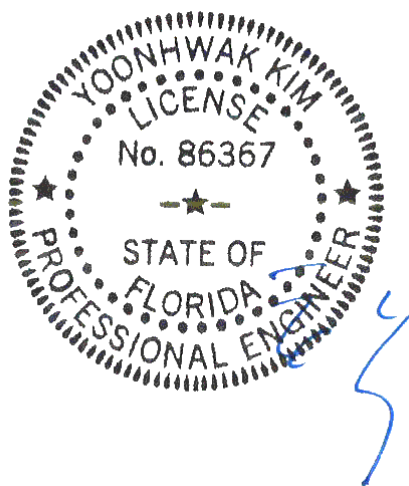
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From -0 plf at -2.18 to 33 plf at 0.00
 TC: From 35 plf at 0.00 to 64 plf at 1.89
 BC: From 4 plf at 0.00 to 6 plf at 1.89
 TC: -1 lb Conc. Load at 1.48
 BC: 21 lb Conc. Load at 1.48

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

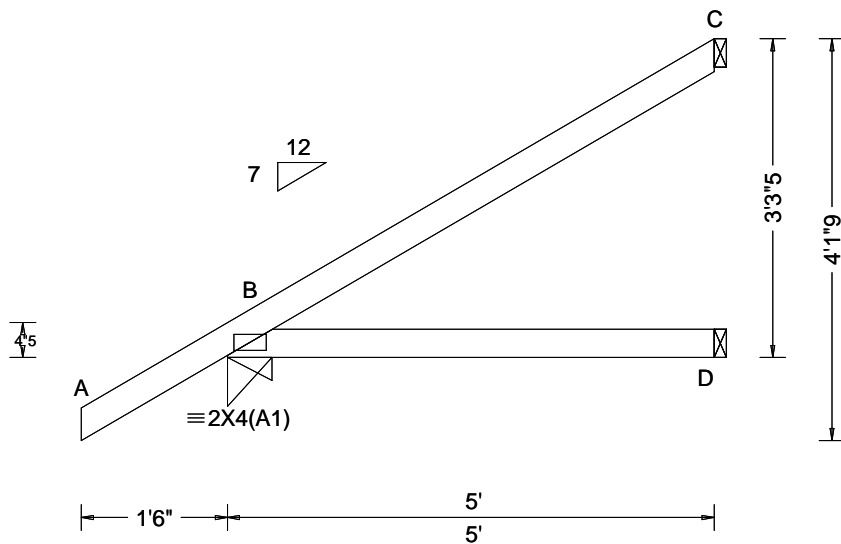
The overall height of this truss excluding overhang is 1-1-10.
 WIND LOAD CASE MODIFIED!



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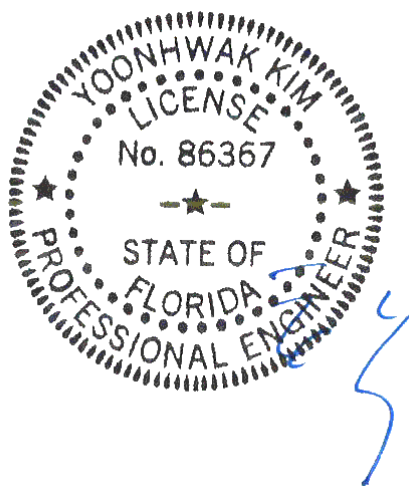




| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.319 Max BC CSI: 0.237 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 335 /- /- /220 /17 /108 D 90 /- /- /51 /- /- C 129 /- /- /79 /57 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

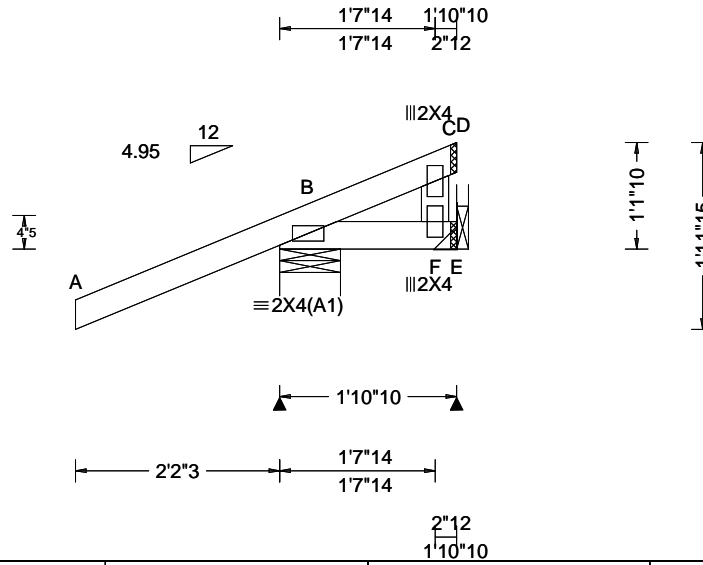
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| | | | | |
|--|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.134 Max BC CSI: 0.020 Max Web CSI: 0.030 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 159 /- /- /- /53 /- E 14 /- /- /- /113 /- Wind reactions based on MWFRS B Brg Width = 7.8 Min Req = 1.5 E Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|---|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

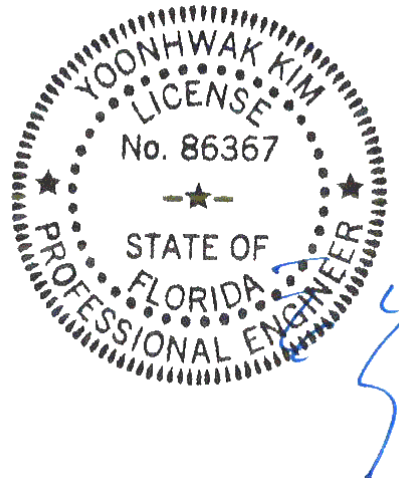
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From -0 plf at -2.18 to 33 plf at 0.00
TC: From 35 plf at 0.00 to 64 plf at 1.89
BC: From 4 plf at 0.00 to 6 plf at 1.89
TC: -49 lb Conc. Load at 1.48
BC: 9 lb Conc. Load at 1.48

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

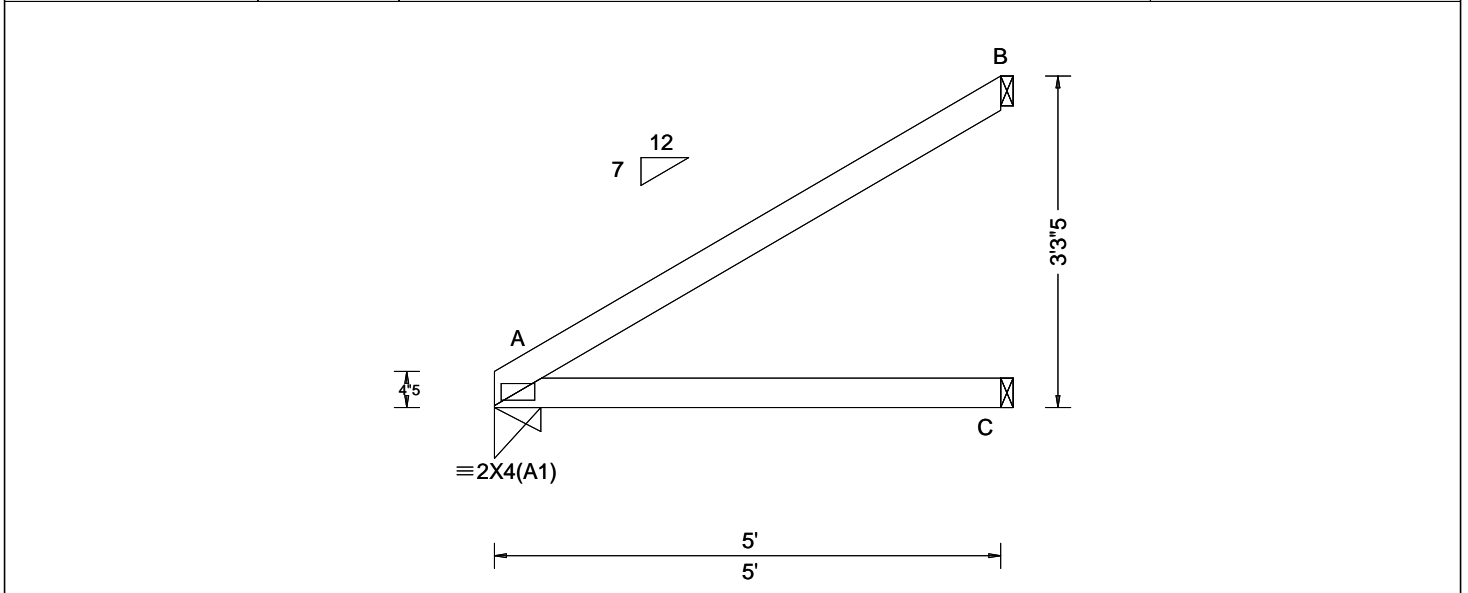
The overall height of this truss excluding overhang is 1-1-10.
WIND LOAD CASE MODIFIED!



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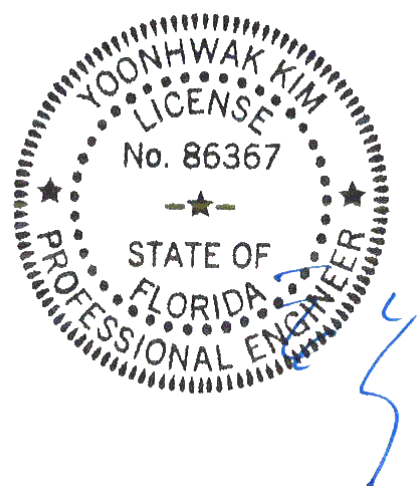




| | | | | |
|--|--|---|--|--|
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|--|--|---|--|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

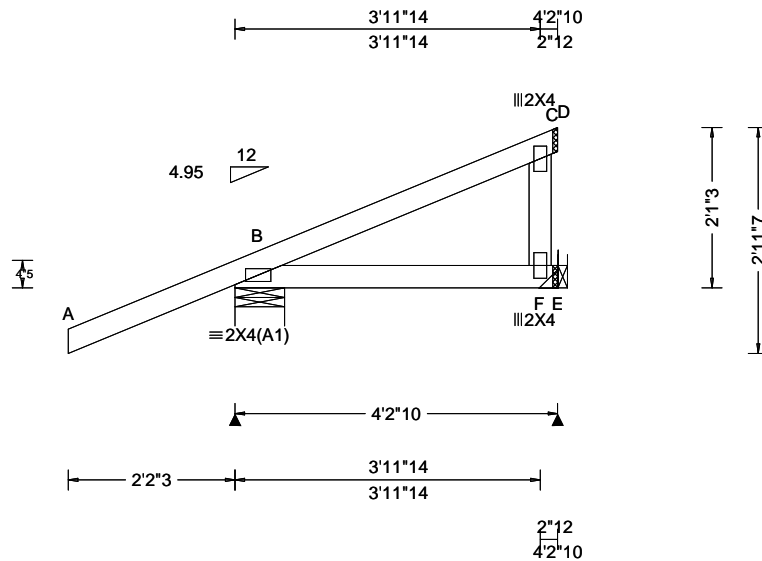
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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|---|---|--|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|----|-----|----|---|----|----|----|----|----|----|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 226 | /- | /- | /- | /41 | /- | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 63 | /- | /- | /- | /1 | /- | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Loading

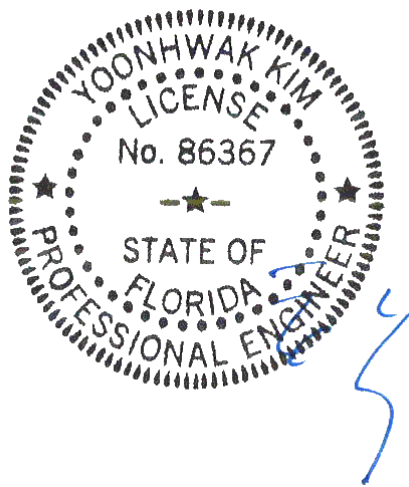
Hipjack supports 2-11-13 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

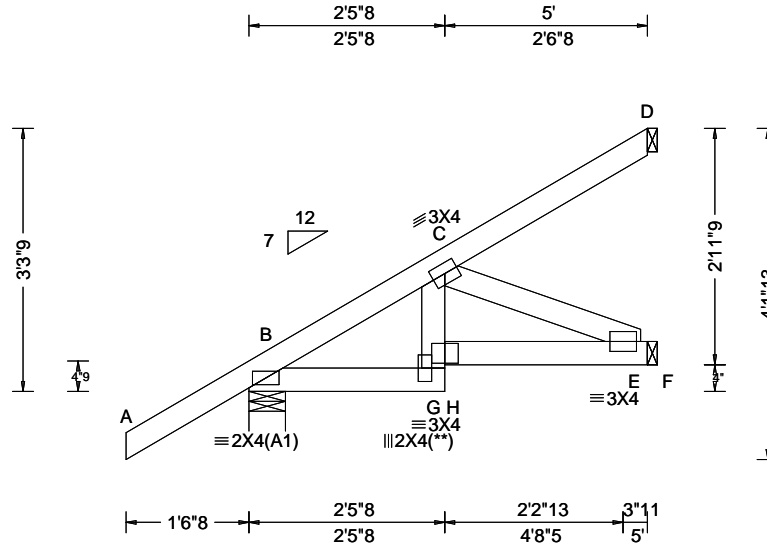
The overall height of this truss excluding overhang is 2-1-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 G 999 240 VERT(CL): 0.008 G 999 180 HORZ(LL): 0.002 F - - HORZ(TL): 0.005 F - - Creep Factor: 2.0 Max TC CSI: 0.172 Max BC CSI: 0.110 Max Web CSI: 0.178 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>338</td> <td>-</td> <td>-</td> <td>/223</td> <td>/18</td> <td>/109</td> </tr> <tr> <td>E</td> <td>115</td> <td>-</td> <td>-</td> <td>/80</td> <td>/10</td> <td>-</td> </tr> <tr> <td>D</td> <td>76</td> <td>-</td> <td>-</td> <td>/47</td> <td>/32</td> <td>-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | B | 338 | - | - | /223 | /18 | /109 | E | 115 | - | - | /80 | /10 | - | D | 76 | - | - | /47 | /32 | - |
|--|---|---|--|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|------|-----|------|---|-----|---|---|-----|-----|---|---|----|---|---|-----|-----|---|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 338 | - | - | /223 | /18 | /109 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 115 | - | - | /80 | /10 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 76 | - | - | /47 | /32 | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

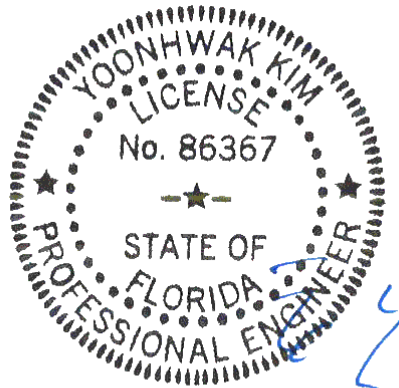
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

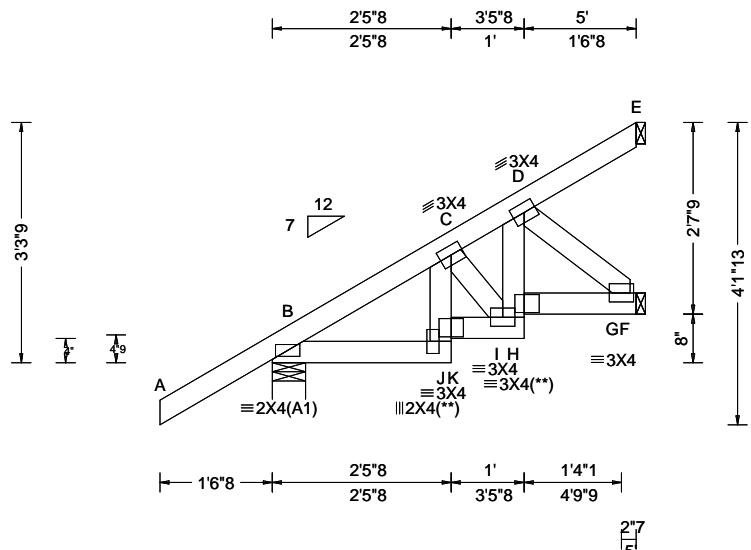
Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 J 999 240 VERT(CL): 0.008 J 999 180 HORZ(LL): 0.003 G - - HORZ(TL): 0.006 G - - Creep Factor: 2.0 Max TC CSI: 0.172 Max BC CSI: 0.069 Max Web CSI: 0.164 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 338 - / - / 223 /18 /109 F 133 - / - / 96 /22 - E 48 - / - / 30 /21 - Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 1.5 Min Req = - E Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|---|

Lumber

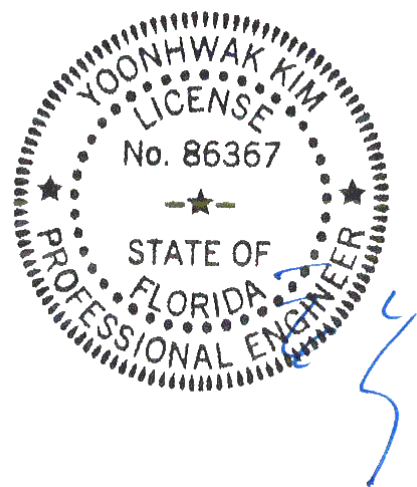
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

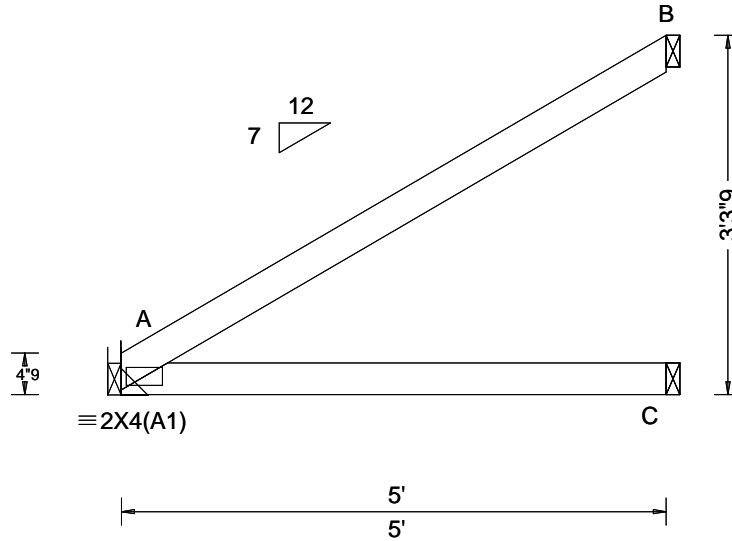
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.377 Max BC CSI: 0.260 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>213</td> <td>/-</td> <td>/-</td> <td>/129</td> <td>/-</td> <td>/63</td> </tr> <tr> <td>C</td> <td>94</td> <td>/-</td> <td>/-</td> <td>/55</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>B</td> <td>141</td> <td>/-</td> <td>/-</td> <td>/88</td> <td>/37</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 213 | /- | /- | /129 | /- | /63 | C | 94 | /- | /- | /55 | /- | /- | B | 141 | /- | /- | /88 | /37 | /- |
|--|--|---|---|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|----|-----|---|----|----|----|-----|----|----|---|-----|----|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 213 | /- | /- | /129 | /- | /63 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 94 | /- | /- | /55 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 141 | /- | /- | /88 | /37 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Wind reactions based on MWFRS A Brg Width = - Min Req = - C Brg Width = 1.5 Min Req = - B Brg Width = 1.5 Min Req = - Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

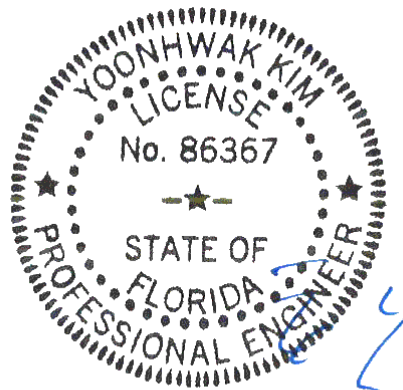
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

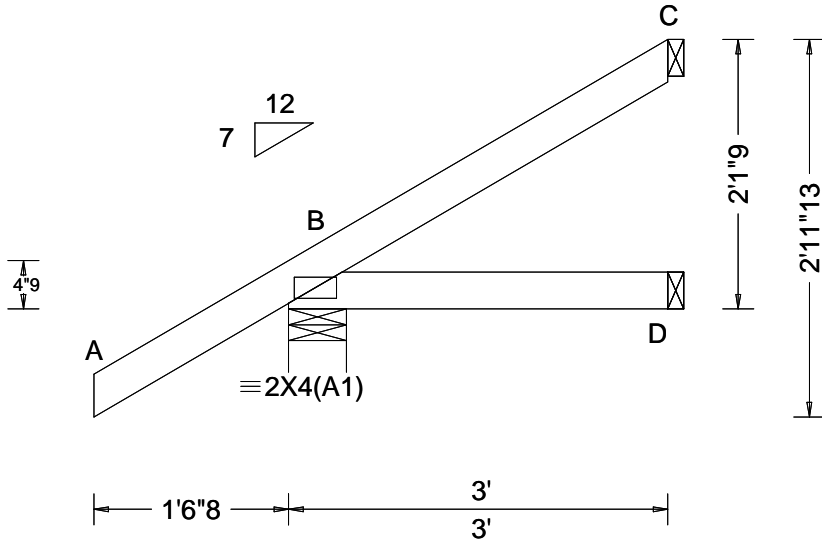
Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



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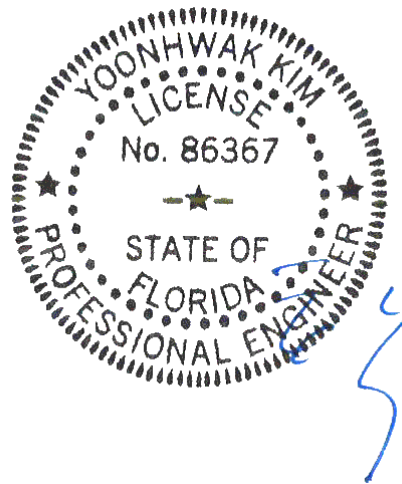
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-----------------------------------|------------------------------|--------------------------------|---|-----------------|-----|---------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | B | 179 | - | - | /123 | /15 | /49 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | D | 33 | - | - | /21 | - | - |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.000 B - - | C | 41 | - | - | /23 | /21 | - |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.001 B - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | B | Brg Width = 5.5 | | Min Req = 1.5 | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.115 | D | Brg Width = 1.5 | | Min Req = - | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.044 | C | Brg Width = 1.5 | | Min Req = - | | | |
| Spacing: 16.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Bearing B is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: not in 4.50 ft | WAVE | | | | | | | | |
| | GCp1: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.




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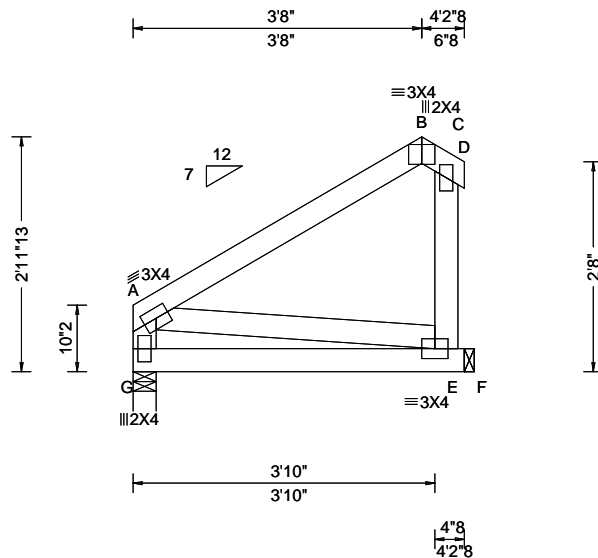
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6750 Forum Drive
Suite 305
Orlando FL, 32821



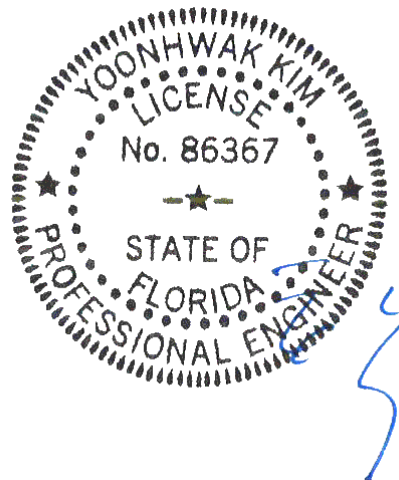
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|--|--|--|---|---------|-----|-------------|--|--|--|-----|----|-----|-----|-----|----|-----|---|-----|---|---|------|---|-----|---|-----|---|---|------|-----|---|
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| Gravity | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| G | 175 | - | - | /102 | - | /46 | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 175 | - | - | /115 | /11 | - | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

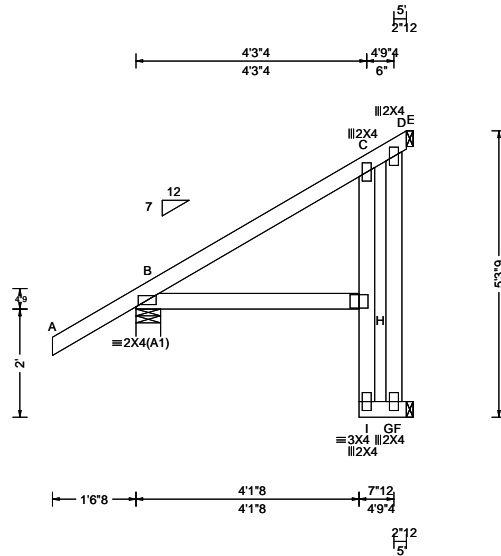
Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 09/16/2021

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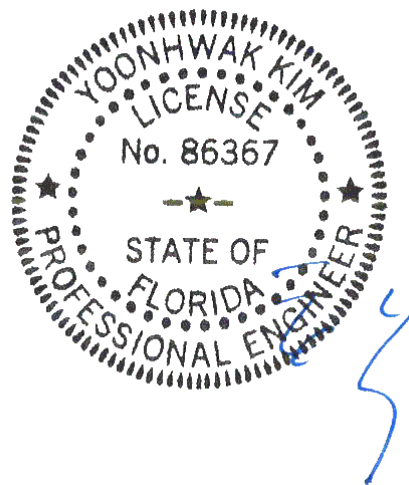
| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp1: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.024 H 999 240 VERT(CL): 0.047 H 999 180 HORZ(LL): 0.015 C - - HORZ(TL): 0.030 C - - Creep Factor: 2.0 Max TC CSI: 0.358 Max BC CSI: 0.169 Max Web CSI: 0.150 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 338 /- /- /223 /- /78 F 26 /- /- /19 /1 /- D 156 /- /- /107 /18 /- Wind reactions based on MWFRS B Brg Width = 5.5 Min Req = 1.5 F Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

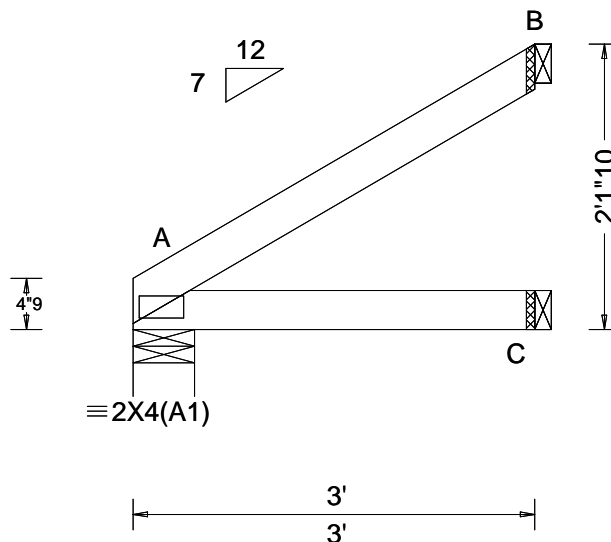
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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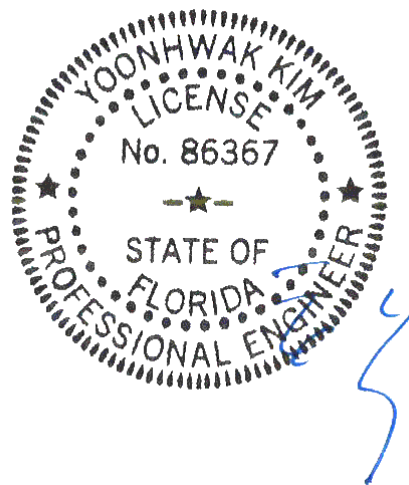
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|-----|-----|-------------|-----|-----|-----|--|--|--|
| | | | | Gravity | | | Non-Gravity | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | A | 131 | - | - | /79 | - | /51 | | | |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | C | 55 | - | - | /32 | - | - | | | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | B | 83 | - | - | /52 | /36 | - | | | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.001 A - - | Wind reactions based on MWFRS | | | | | | | | | |
| Des Ld: 40.00 | EXP: C Kzt: NA | | HORZ(TL): 0.003 A - - | A Brg Width = 5.5 Min Req = 1.5 | | | | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | Building Code: | Creep Factor: 2.0 | C Brg Width = 1.5 Min Req = - | | | | | | | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | FBC 7th Ed. 2020 Res. HVHZ | Max TC CSI: 0.144 | B Brg Width = 1.5 Min Req = - | | | | | | | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | TPI Std: 2014 | Max BC CSI: 0.081 | Bearing A is a rigid surface. | | | | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | Rep Fac: Yes | Max Web CSI: 0.000 | Members not listed have forces less than 375# | | | | | | | | | |
| | C&C Dist a: 3.00 ft | FT/RT:20(0)/10(0) | VIEW Ver: 21.01.01A.0521.20 | | | | | | | | | | |
| | Loc. from endwall: Any | Plate Type(s): | | | | | | | | | | | |
| | GCp: 0.18 | WAVE | | | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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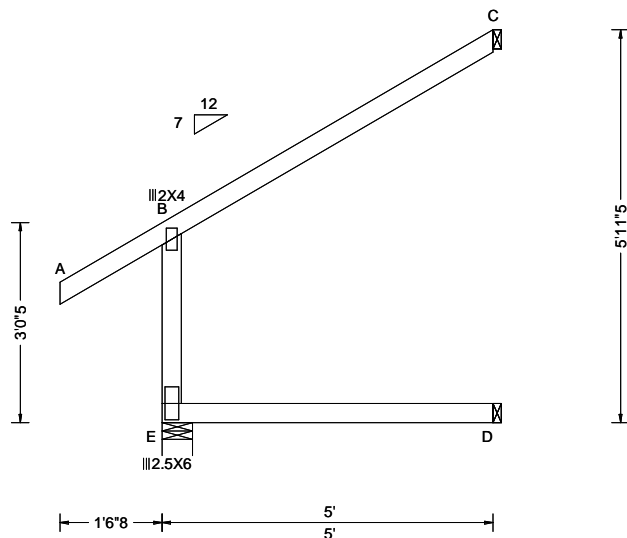
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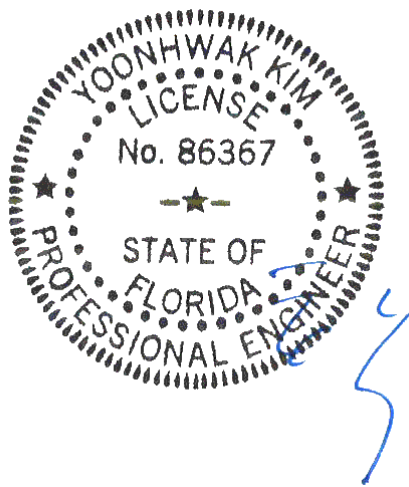
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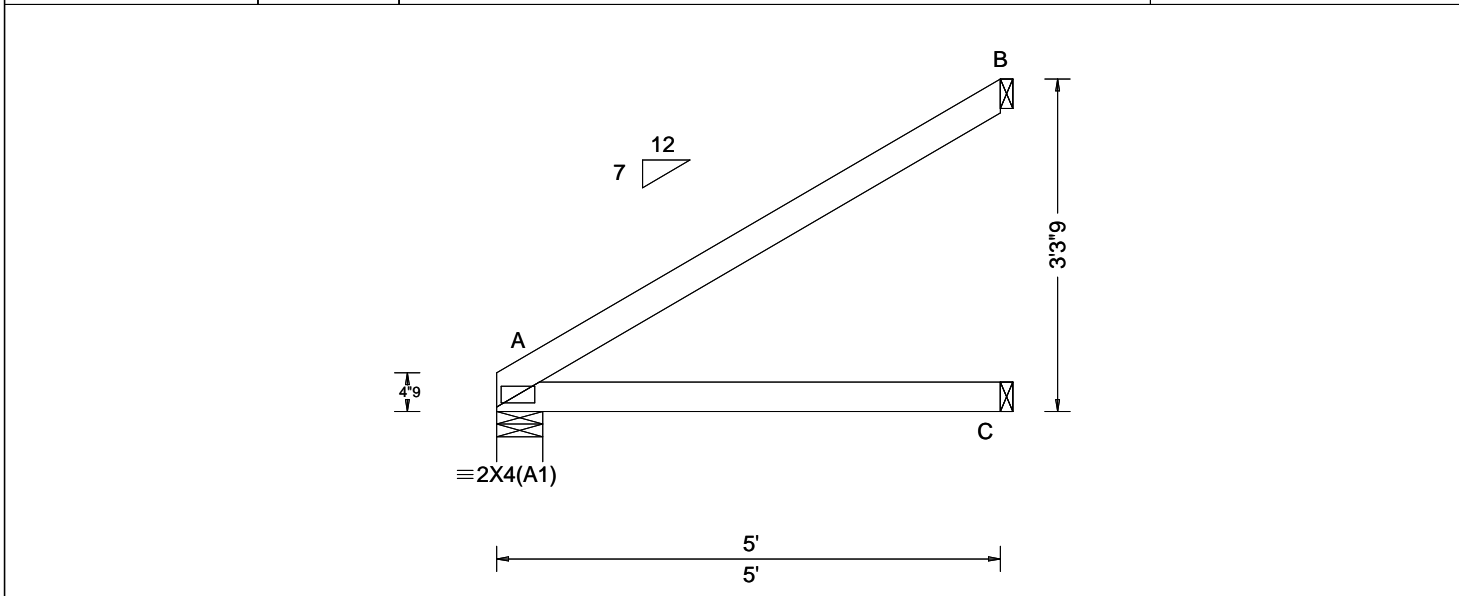
| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.03 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp1: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.388 Max BC CSI: 0.298 Max Web CSI: 0.109 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>328</td> <td>/-</td> <td>/-</td> <td>/254</td> <td>/100</td> <td>/-</td> </tr> <tr> <td>D</td> <td>100</td> <td>/-</td> <td>/-</td> <td>/50</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>142</td> <td>/-</td> <td>/-</td> <td>/58</td> <td>/8</td> <td>/122</td> </tr> </tbody> </table> Wind reactions based on MWFRS E Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375# | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | E | 328 | /- | /- | /254 | /100 | /- | D | 100 | /- | /- | /50 | /- | /- | C | 142 | /- | /- | /58 | /8 | /122 |
|---|--|---|--|---|------|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|------|----|---|-----|----|----|-----|----|----|---|-----|----|----|-----|----|------|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| E | 328 | /- | /- | /254 | /100 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 100 | /- | /- | /50 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 142 | /- | /- | /58 | /8 | /122 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Wind Wind loads based on MWFRS with additional C&C member design. Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



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| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.375 Max BC CSI: 0.258 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>214</td> <td>/-</td> <td>/-</td> <td>/129</td> <td>/-</td> <td>/63</td> </tr> <tr> <td>C</td> <td>94</td> <td>/-</td> <td>/-</td> <td>/55</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>B</td> <td>140</td> <td>/-</td> <td>/-</td> <td>/87</td> <td>/37</td> <td>/-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 214 | /- | /- | /129 | /- | /63 | C | 94 | /- | /- | /55 | /- | /- | B | 140 | /- | /- | /87 | /37 | /- |
|--|--|---|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|------|----|-----|---|----|----|----|-----|----|----|---|-----|----|----|-----|-----|----|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 214 | /- | /- | /129 | /- | /63 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| C | 94 | /- | /- | /55 | /- | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| B | 140 | /- | /- | /87 | /37 | /- | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
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Lumber

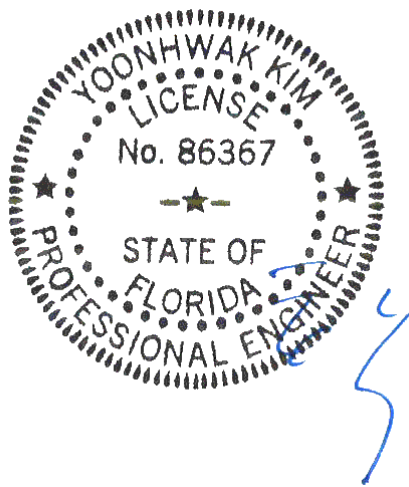
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

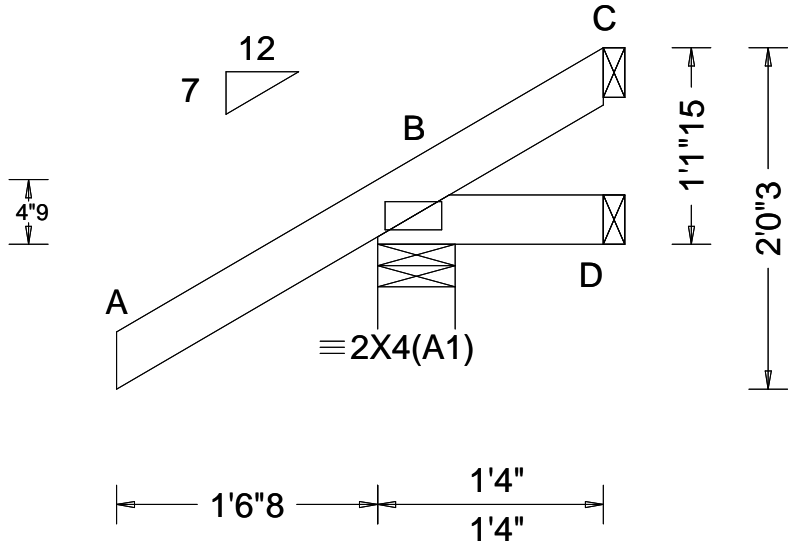
The overall height of this truss excluding overhang is 3-3-9.



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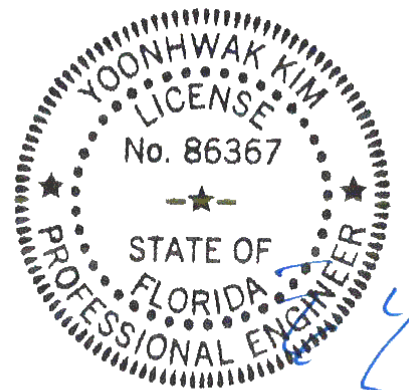
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-------------------------------|------------------------------|--------------------------------|---|-----------------|------|-------------|---------------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | B | 247 | /- | /- | /180 | /36 | /44 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | D | 13 | /-8 | /- | /15 | /9 | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.000 B - - | C | - | /-24 | /- | /24 | /31 | /- |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.001 B - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | B | Brg Width = 5.5 | | | Min Req = 1.5 | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.216 | D | Brg Width = 1.5 | | | Min Req = - | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.043 | C | Brg Width = 1.5 | | | Min Req = - | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Bearing B is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: Any | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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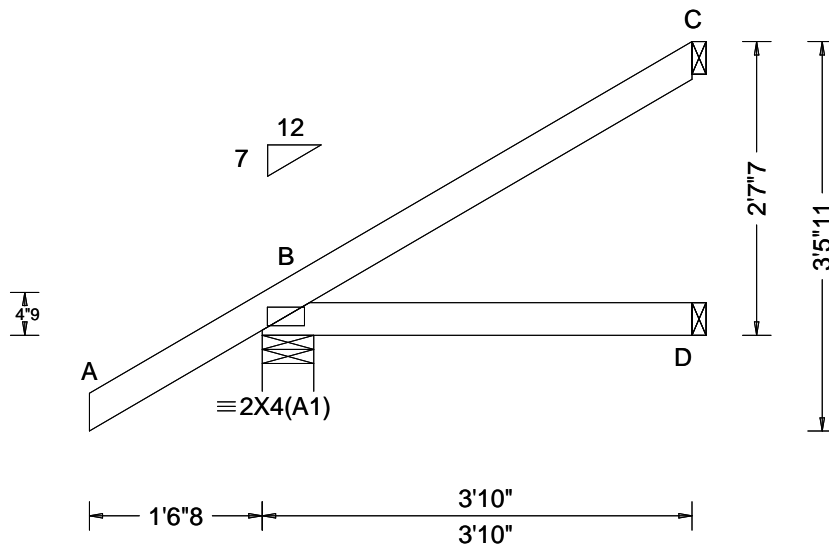
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6750 Forum Drive
Suite 305
Orlando FL, 32821



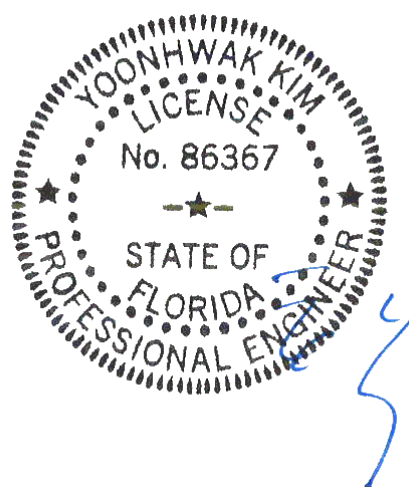
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | |
|------------------------|-----------------------------------|------------------------------|--------------------------------|---|-----------------|-----|---------------|------|-----|-----|
| | | | | Gravity | | | Non-Gravity | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | Loc | R+ | /R- | /Rh | /Rw | /U | /RL |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | B | 296 | /- | /- | /199 | /2 | /63 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | D | 67 | /- | /- | /39 | /- | /- |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.001 B - - | C | 91 | /- | /- | /54 | /27 | /- |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.002 B - - | Wind reactions based on MWFRS | | | | | | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | B | Brg Width = 5.5 | | Min Req = 1.5 | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.172 | D | Brg Width = 1.5 | | Min Req = - | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.125 | C | Brg Width = 1.5 | | Min Req = - | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: h to 2h | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Bearing B is a rigid surface. | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | Members not listed have forces less than 375# | | | | | | |
| | Loc. from endwall: not in 9.00 ft | WAVE | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

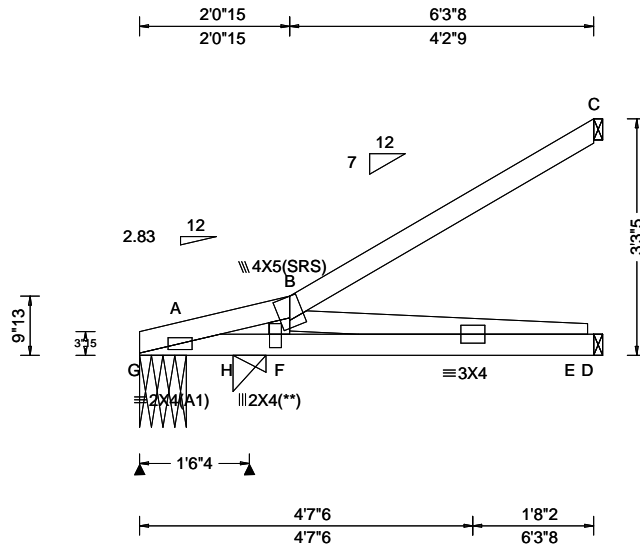
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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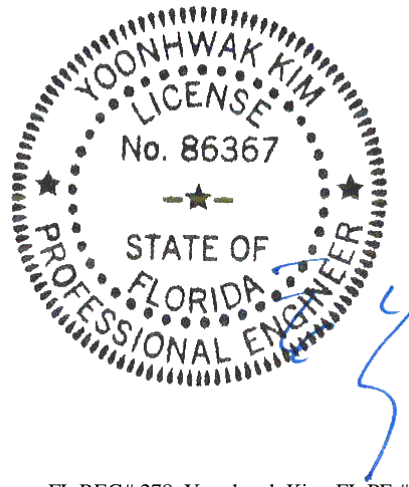


| | | | | |
|--|---|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 B 999 240 VERT(CL): 0.008 B 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.261 Max BC CSI: 0.196 Max Web CSI: 0.057 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 30 /-38 /- /19 /20 /62 H 344 /- /- /211 /- /- D 89 /- /- /52 /- /- C 118 /- /- /72 /32 /- Wind reactions based on MWFRS G Brg Width = 7.8 Min Req = 1.5 H Brg Width = 5.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearings G & H are a rigid surface. Members not listed have forces less than 375# |
|--|---|---|---|--|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

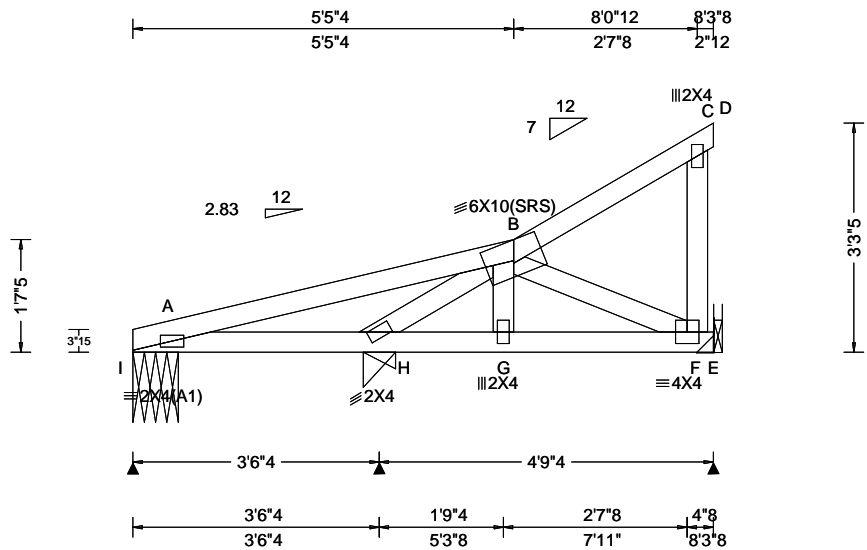
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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| | | | | |
|--|---|---|--|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp1: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.011 A 999 240 VERT(CL): 0.020 A 999 180 HORZ(LL): 0.003 A - - HORZ(TL): 0.006 A - - Creep Factor: 2.0 Max TC CSI: 0.221 Max BC CSI: 0.230 Max Web CSI: 0.071 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity I 168 - / - / 81 / 14 / 58 H 292 - / - / 165 - / - E 215 - / - / 137 / 16 - Wind reactions based on MWFRS I Brg Width = 7.8 Min Req = 1.5 H Brg Width = 5.5 Min Req = 1.5 E Brg Width = - Min Req = - Bearings I & H are a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|---|

Lumber

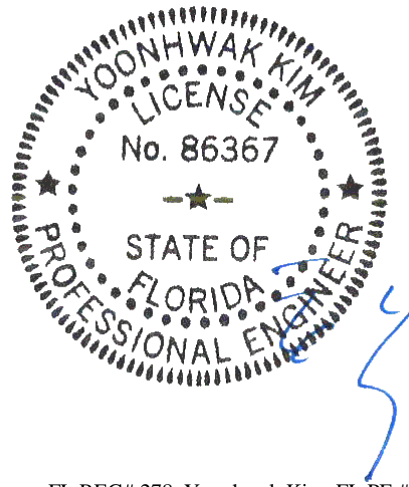
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

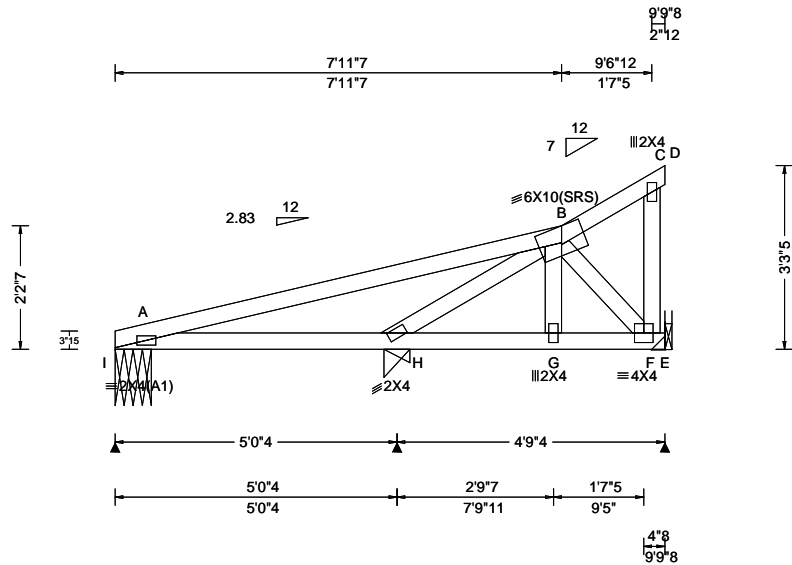
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| | | | | |
|---|--|--|--|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.034 A 999 240 VERT(CL): 0.070 A 841 180 HORIZ(LL): 0.010 A - - HORIZ(TL): 0.020 A - - Creep Factor: 2.0 Max TC CSI: 0.548 Max BC CSI: 0.496 Max Web CSI: 0.082 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 270 /- /- /128 /27 /61 H 261 /- /- /155 /- /- E 263 /- /- /147 /18 /- Wind reactions based on MWFRS I Brg Width = 7.8 Min Req = 1.5 H Brg Width = 5.5 Min Req = 1.5 E Brg Width = - Min Req = - Bearings I & H are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - F 256 -392 |

Lumber

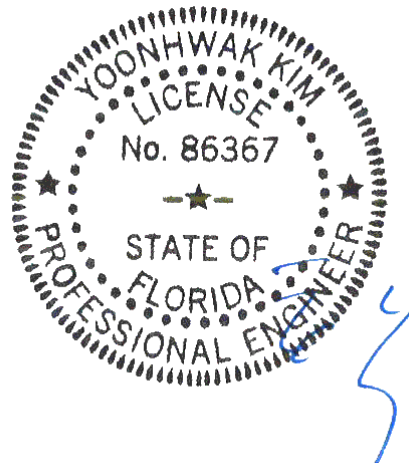
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

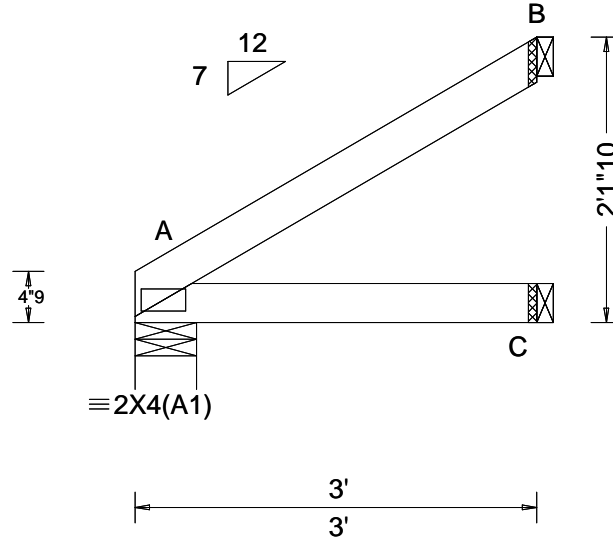
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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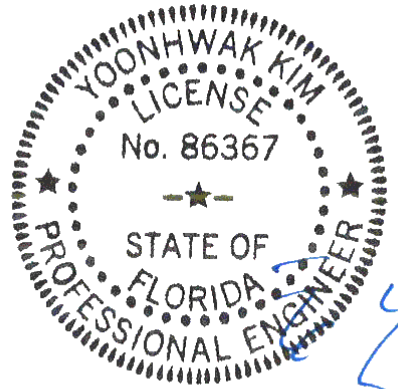
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg, Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | | | | | | | | | |
|------------------------|-----------------------------------|-------------------------------|--------------------------------|---|-----|-----|---------------|-----|-----|-----|--|--|--|
| | | | | Gravity | | | Non-Gravity | | | | | | |
| Loc | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | |
| TCLL: 20.00 | Wind Std: ASCE 7-16 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/def L/# | A | 131 | /- | /- | /79 | /- | /51 | | | |
| TCDL: 10.00 | Speed: 120 mph | Pf: NA Ce: NA | VERT(LL): NA | C | 55 | /- | /- | /32 | /- | /- | | | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | B | 83 | /- | /- | /52 | /36 | /- | | | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.001 A - - | Wind reactions based on MWFRS | | | | | | | | | |
| Des Ld: 40.00 | EXP: C Kzt: NA | Building Code: | HORZ(TL): 0.003 A - - | A Brg Width = 5.5 | | | Min Req = 1.5 | | | | | | |
| NCBCLL: 10.00 | Mean Height: 0.00 ft | FBC 7th Ed. 2020 Res. HVHZ | Creep Factor: 2.0 | C Brg Width = 1.5 | | | Min Req = - | | | | | | |
| Soffit: 2.00 | TCDL: 5.0 psf | TPI Std: 2014 | Max TC CSI: 0.118 | B Brg Width = 1.5 | | | Min Req = - | | | | | | |
| Load Duration: 1.25 | BCDL: 5.0 psf | Rep Fac: Yes | Max BC CSI: 0.081 | Bearing A is a rigid surface. | | | | | | | | | |
| Spacing: 24.0 " | MWFRS Parallel Dist: > 2h | FT/RT:20(0)/10(0) | Max Web CSI: 0.000 | Members not listed have forces less than 375# | | | | | | | | | |
| | C&C Dist a: 3.00 ft | Plate Type(s): | VIEW Ver: 21.01.01A.0521.20 | | | | | | | | | | |
| | Loc. from endwall: not in 4.50 ft | WAVE | | | | | | | | | | | |
| | GCp: 0.18 | | | | | | | | | | | | |
| | Wind Duration: 1.60 | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

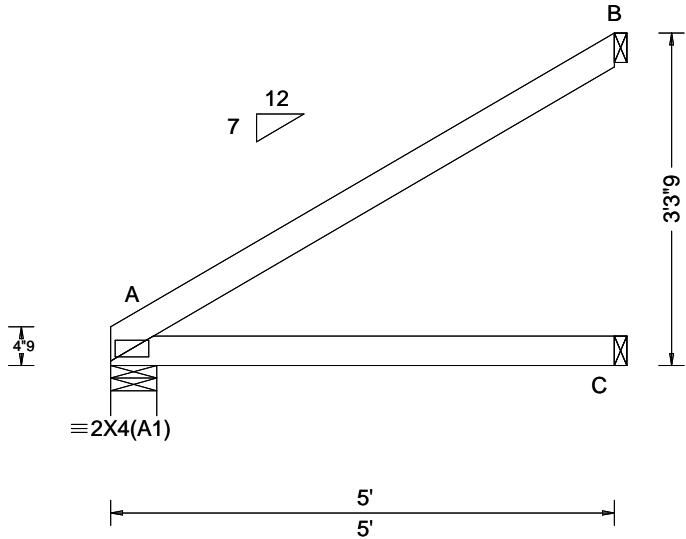
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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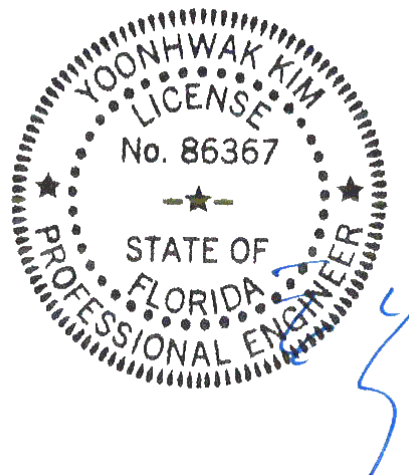
| | | | | |
|--|---|---|--|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.375 Max BC CSI: 0.258 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 214 /- /- /131 /- /88 C 94 /- /- /55 /- /- B 140 /- /- /87 /60 /- Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 C Brg Width = 1.5 Min Req = - B Brg Width = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|--|--|

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

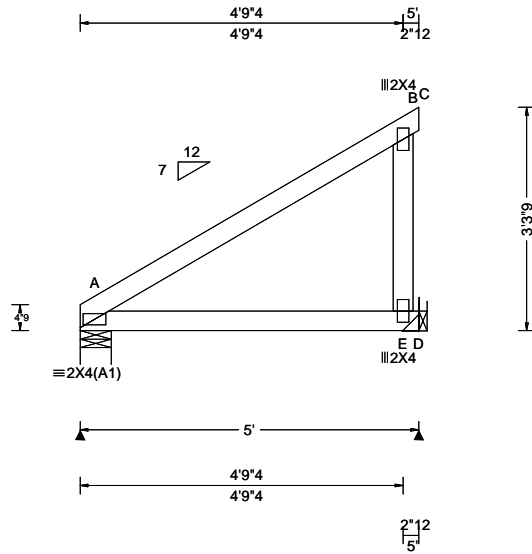
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Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.016 A - - HORZ(TL): 0.031 A - - Creep Factor: 2.0 Max TC CSI: 0.391 Max BC CSI: 0.782 Max Web CSI: 0.323 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>885</td> <td>-</td> <td>-</td> <td>-</td> <td>103</td> <td>-</td> </tr> <tr> <td>D</td> <td>650</td> <td>-</td> <td>-</td> <td>-</td> <td>79</td> <td>-</td> </tr> </tbody> </table> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 885 | - | - | - | 103 | - | D | 650 | - | - | - | 79 | - |
|--|--|---|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|-----|---|---|-----|---|---|---|----|---|
| | | | | Loc | | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | |
| R+ | /R- | /Rh | /Rw | | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 885 | - | - | - | 103 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 650 | - | - | - | 79 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 D Brg Width = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3;

Special Loads

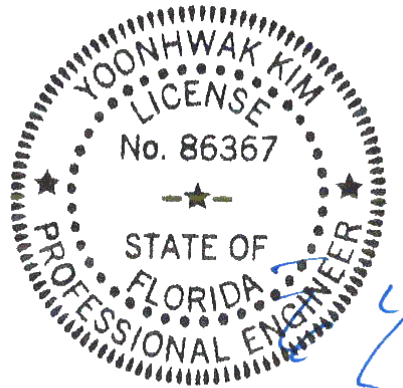
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 5.00
 BC: From 10 plf at 0.00 to 10 plf at 5.00
 BC: 585 lb Conc. Load at 1.06, 3.06

Hangers / Ties

(J) Hanger Support Required, by others

Wind

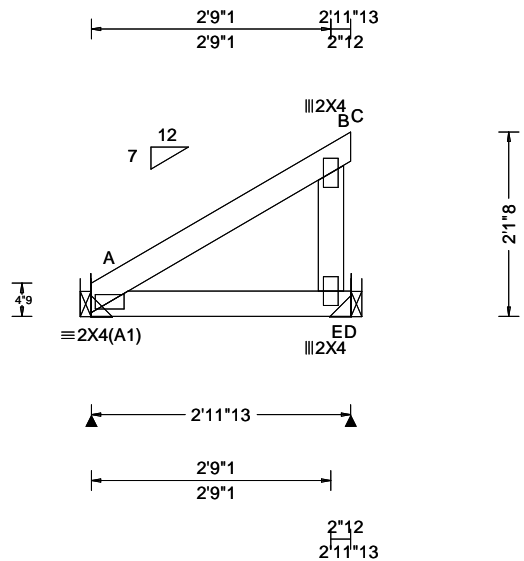
Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 A - - HORZ(TL): 0.011 A - - Creep Factor: 2.0 Max TC CSI: 0.368 Max BC CSI: 0.967 Max Web CSI: 0.104 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>468</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/49</td> <td>/-</td> </tr> <tr> <td>D</td> <td>429</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/45</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Width = - Min Req = - D Brg Width = - Min Req = - Members not listed have forces less than 375# | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 468 | /- | /- | /- | /49 | /- | D | 429 | /- | /- | /- | /45 | /- |
|--|---|---|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|----|----|----|-----|----|---|-----|----|----|----|-----|----|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 468 | /- | /- | /- | /49 | /- | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 429 | /- | /- | /- | /45 | /- | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

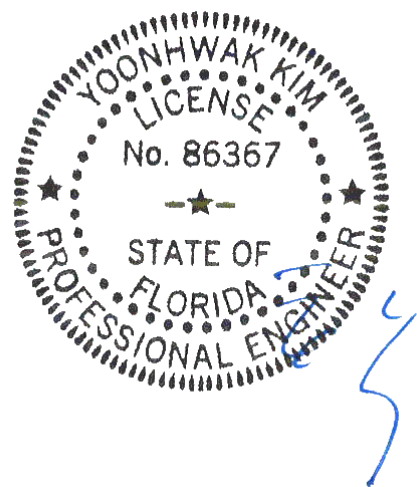
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 2.98
 BC: From 20 plf at 0.00 to 20 plf at 2.98
 BC: 650 lb Conc. Load at 1.44

Hangers / Ties

(J) Hanger Support Required, by others
 (H2) = (J) Special hanger required (1)2x4 SP #2 supporting member.

Wind

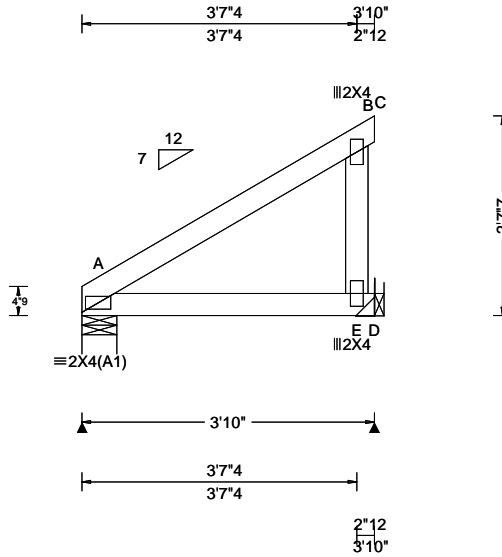
Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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| Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 A - - HORZ(TL): 0.007 A - - Creep Factor: 2.0 Max TC CSI: 0.251 Max BC CSI: 0.365 Max Web CSI: 0.057 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>228</td> <td>-</td> <td>-</td> <td>-</td> <td>14</td> <td>-</td> </tr> <tr> <td>D</td> <td>221</td> <td>-</td> <td>-</td> <td>-</td> <td>14</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Width = 5.5 Min Req = 1.5 D Brg Width = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#</p> | Loc | Gravity | | | Non-Gravity | | | R+ | /R- | /Rh | /Rw | /U | /RL | A | 228 | - | - | - | 14 | - | D | 221 | - | - | - | 14 | - |
|--|--|---|---|---|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|-----|---|---|---|----|---|---|-----|---|---|---|----|---|
| Loc | Gravity | | | Non-Gravity | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | R+ | /R- | /Rh | /Rw | /U | /RL | | | | | | | | | | | | | | | | | | | | | | | | | |
| A | 228 | - | - | - | 14 | - | | | | | | | | | | | | | | | | | | | | | | | | | |
| D | 221 | - | - | - | 14 | - | | | | | | | | | | | | | | | | | | | | | | | | | |

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

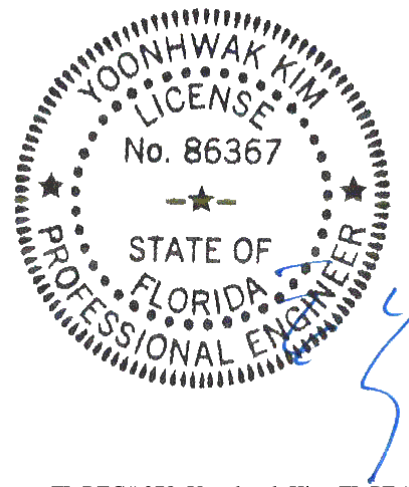
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 3.83
 BC: From 20 plf at 0.00 to 20 plf at 3.83
 BC: 130 lb Conc. Load at 1.92

Hangers / Ties

(J) Hanger Support Required, by others

Wind

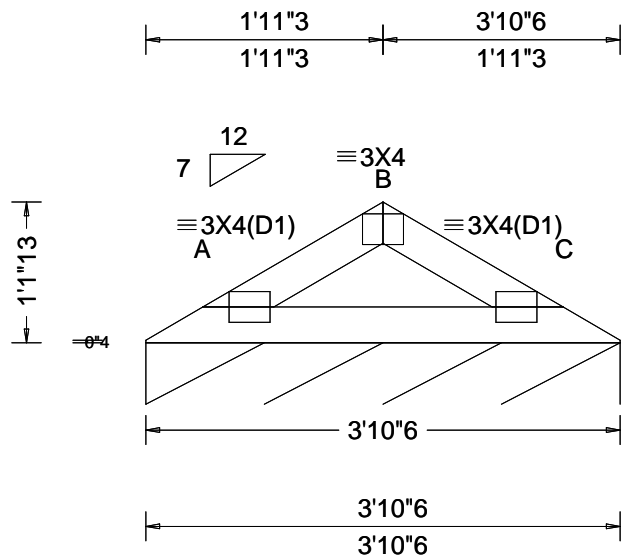
Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
|---|---|--|--|--|
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.97 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.006 C 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.070 Max BC CSI: 0.093 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 82 /- /- /38 /2 /5 Wind reactions based on MWFRS C Brg Width = 46.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |

Lumber

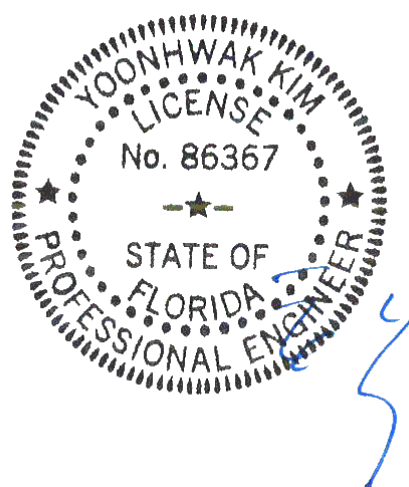
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

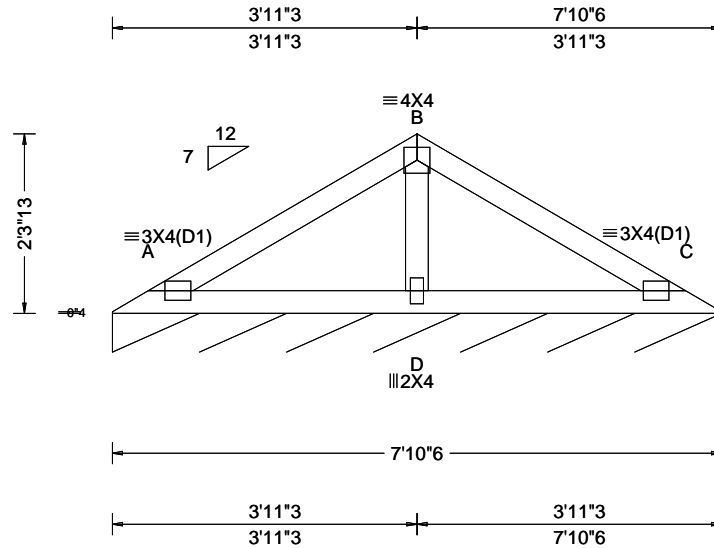
See DWG VALTN160118 for valley details.



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| | | | | |
|--|---|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.006 C 999 240 VERT(CL): 0.013 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.190 Max BC CSI: 0.166 Max Web CSI: 0.070 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /41 /5 /6 Wind reactions based on MWFRS C Brg Width = 94.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |
|--|---|---|---|---|

Lumber

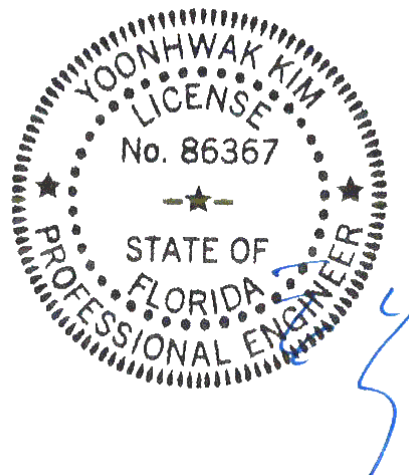
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

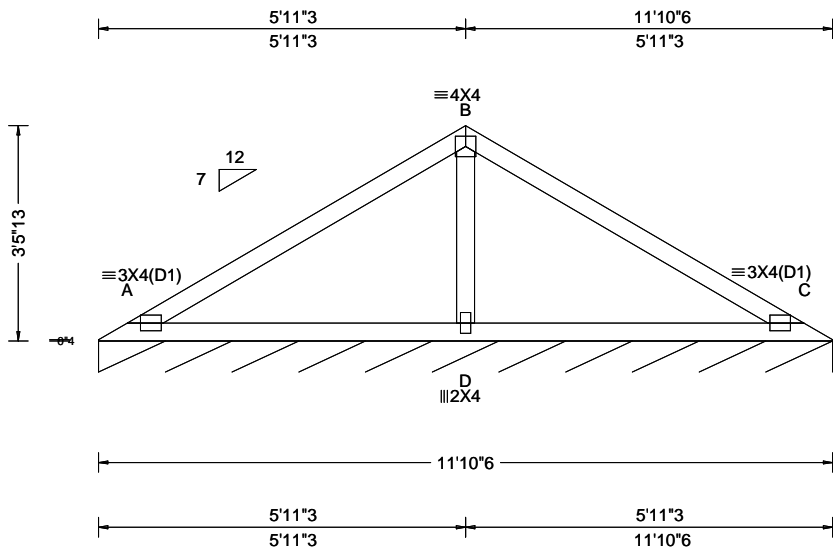
See DWG VALTN160118 for valley details.



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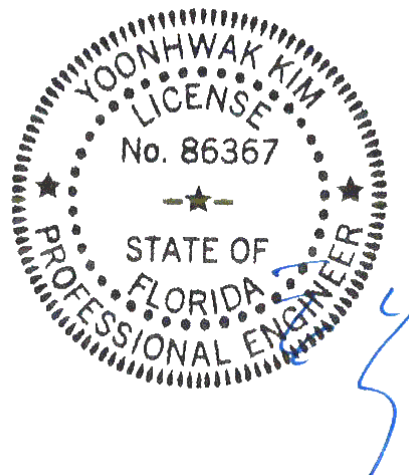


| | | | | |
|--|---|---|---|---|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.81 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.022 C 999 240 VERT(CL): 0.046 C 999 180 HORZ(LL): -0.010 C - - HORZ(TL): 0.021 C - - Creep Factor: 2.0 Max TC CSI: 0.500 Max BC CSI: 0.414 Max Web CSI: 0.176 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 83 /- /- /42 /6 /6 Wind reactions based on MWFRS C Brg Width = 142 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 484 -170 B - C 484 -170 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 298 -708 |
|--|---|---|---|---|

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

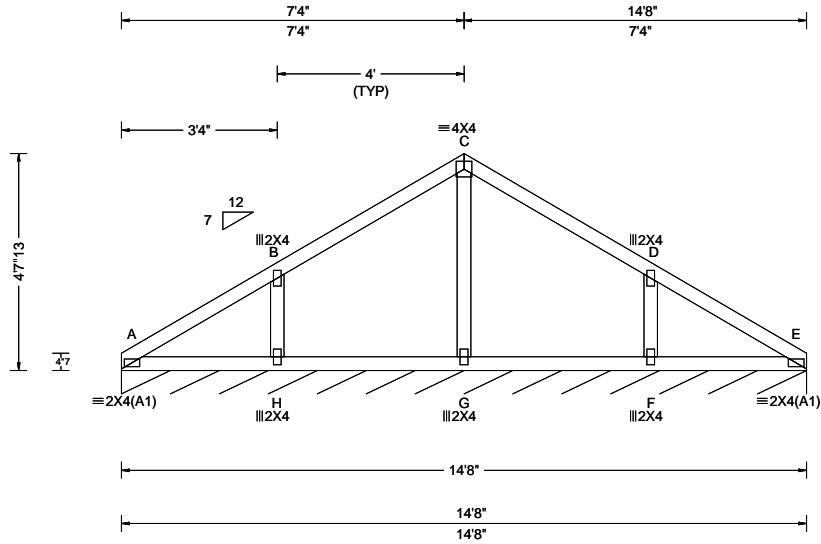
Additional Notes
See DWG VALTN160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/16/2021

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| | | | | |
|---|--|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.26 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.001 E 999 240 VERT(CL): 0.002 E 999 180 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.227 Max BC CSI: 0.113 Max Web CSI: 0.061 VIEW Ver: 21.01.01A.0521.20 | Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 83 /- /- /44 /8 /7 Wind reactions based on MWFRS E Brg Width = 176 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |

Lumber

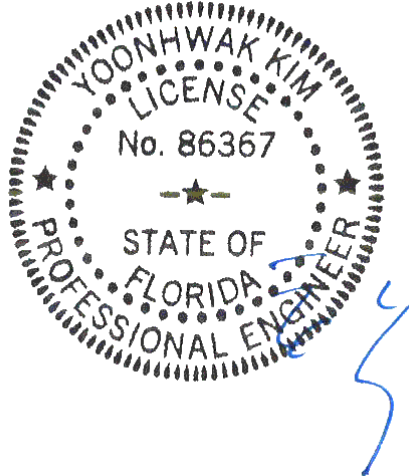
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

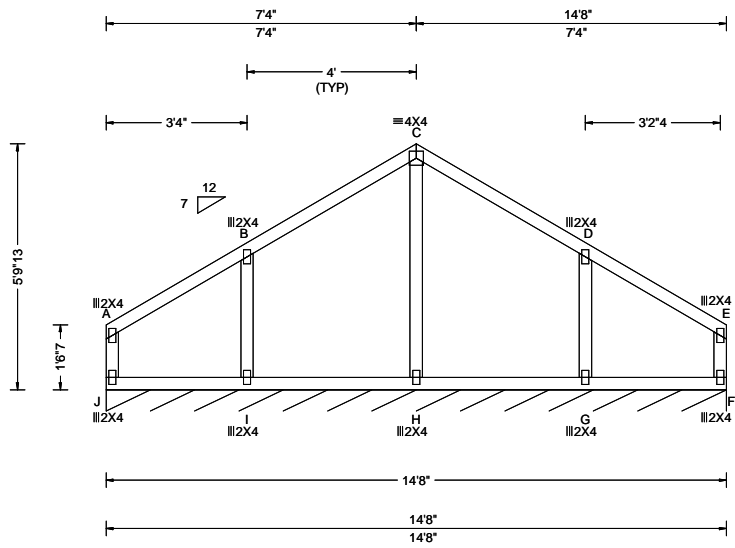
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| | | | | |
|--|--|---|---|--|
| Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.26 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60 | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.033 A - - HORZ(TL): 0.060 A - - Creep Factor: 2.0 Max TC CSI: 0.206 Max BC CSI: 0.103 Max Web CSI: 0.122 VIEW Ver: 21.01.01A.0521.20 | ▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F* 83 /- /- /44 /8 /7 Wind reactions based on MWFRS F Brg Width = 176 Min Req = - Bearing J is a rigid surface. Members not listed have forces less than 375# |
|--|--|---|---|--|

Lumber

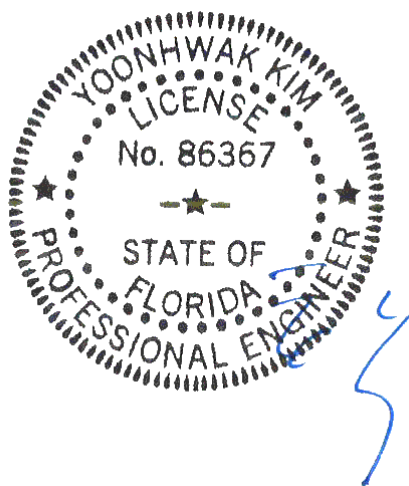
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

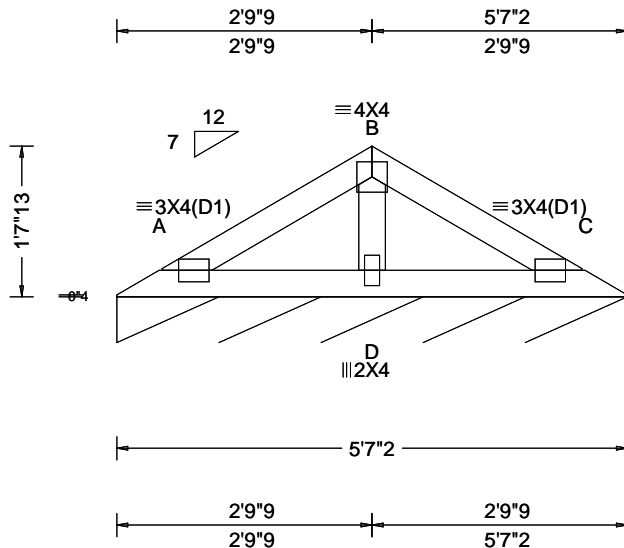
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| | | | | |
|---|---|--|---|--|
| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs), or *=PLF |
| TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/def L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.004 A 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.082 Max BC CSI: 0.074 Max Web CSI: 0.039 VIEW Ver: 21.01.01A.0521.20 | Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /40 /4 /5 Non-Gravity Wind reactions based on MWFRS C Brg Width = 67.1 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# |

Lumber

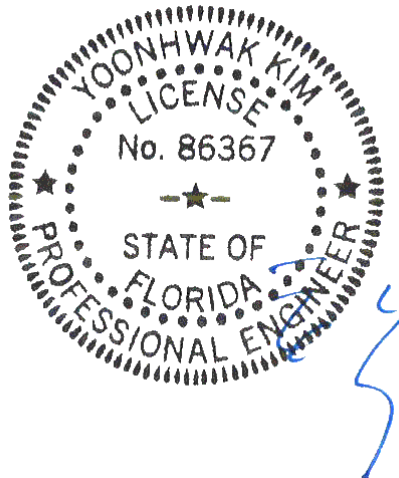
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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09/16/2021

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Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

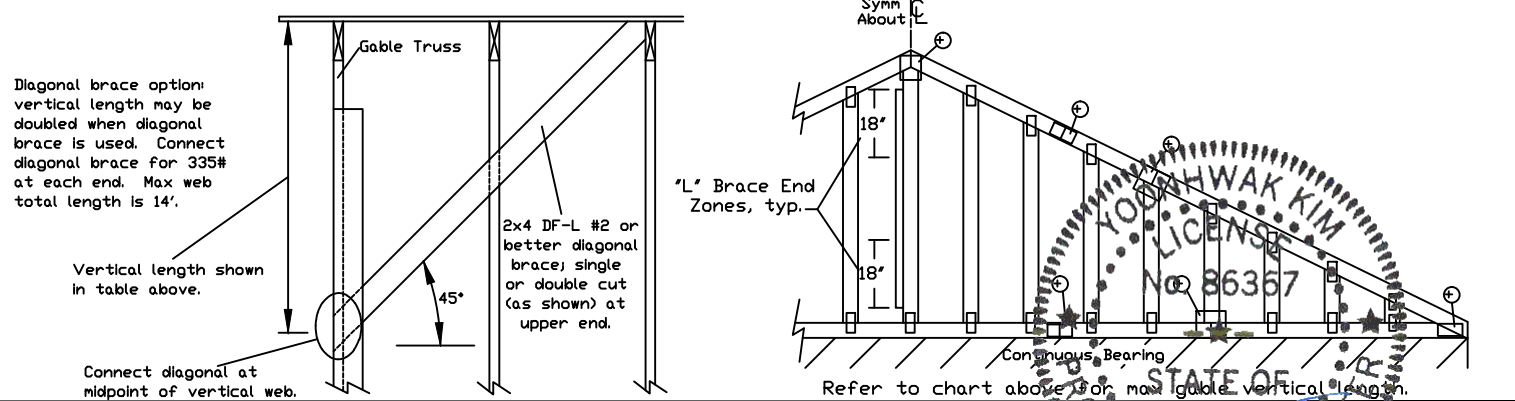
| Max Gable Vertical Length | 2x4 Gable Vertical | | Brace Grade | No Braces | (1) 1x4 'L' Brace * | | (1) 2x4 'L' Brace * | | (2) 2x4 'L' Brace ** | | (1) 2x6 'L' Brace * | | (2) 2x6 'L' Brace ** | |
|---------------------------|--------------------|---------|-------------|-----------|---------------------|---------|---------------------|---------|----------------------|---------|---------------------|---------|----------------------|---------|
| | Spacing | Species | | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| | 24" o.c. | SPF | #1 / #2 | #1 / #2 | 4' 10" | 8' 2" | 8' 6" | 9' 8" | 10' 1" | 11' 6" | 12' 0" | 14' 0" | 14' 0" | 14' 0" |
| #3 | | | | 4' 7" | 7' 9" | 8' 3" | 9' 7" | 9' 11" | 11' 5" | 11' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| Stud | | | | 4' 7" | 7' 8" | 8' 2" | 9' 7" | 9' 11" | 11' 5" | 11' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| HF | | | #1 | 4' 7" | 6' 7" | 7' 0" | 8' 10" | 9' 5" | 11' 5" | 11' 10" | 13' 10" | 14' 0" | 14' 0" | 14' 0" |
| | | | #2 | 4' 7" | 7' 8" | 8' 2" | 9' 7" | 9' 11" | 11' 5" | 11' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | | Standard | 4' 7" | 6' 7" | 7' 0" | 8' 10" | 9' 5" | 11' 5" | 11' 10" | 13' 10" | 14' 0" | 14' 0" | 14' 0" |
| SP | | #1 | #1 | 5' 0" | 8' 4" | 8' 7" | 9' 10" | 10' 2" | 11' 8" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | | #2 | 4' 10" | 8' 2" | 8' 6" | 9' 8" | 10' 1" | 11' 6" | 12' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | | #3 | 4' 8" | 7' 0" | 7' 5" | 9' 3" | 9' 11" | 11' 5" | 11' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | DFL | #1 | 4' 8" | 7' 0" | 7' 5" | 9' 3" | 9' 11" | 11' 5" | 11' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | | #2 | 4' 7" | 6' 2" | 6' 7" | 8' 2" | 8' 9" | 11' 1" | 11' 10" | 12' 10" | 13' 9" | 14' 0" | 14' 0" |
| | | | Standard | 4' 7" | 6' 2" | 6' 7" | 8' 2" | 8' 9" | 11' 1" | 11' 10" | 12' 10" | 13' 9" | 14' 0" | 14' 0" |
| 16" o.c. | SPF | #1 / #2 | #1 / #2 | 5' 6" | 9' 5" | 9' 9" | 11' 1" | 11' 6" | 13' 2" | 13' 9" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #3 | 5' 3" | 9' 3" | 9' 9" | 10' 11" | 11' 4" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| | | | Stud | 5' 3" | 9' 3" | 9' 7" | 10' 11" | 11' 4" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| | | HF | #1 | 5' 3" | 8' 1" | 8' 7" | 10' 10" | 11' 4" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #2 | 5' 9" | 9' 6" | 9' 10" | 11' 3" | 11' 8" | 13' 4" | 13' 10" | 14' 0" | 14' 0" | 14' 0" | |
| | | | Standard | 5' 3" | 8' 1" | 8' 7" | 10' 10" | 11' 4" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| | SP | #1 | #1 | 5' 9" | 9' 6" | 9' 10" | 11' 3" | 11' 8" | 13' 4" | 13' 10" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #2 | 5' 6" | 9' 5" | 9' 9" | 11' 1" | 11' 6" | 13' 2" | 13' 9" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #3 | 5' 5" | 8' 6" | 9' 1" | 11' 0" | 11' 5" | 13' 1" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | |
| | | DFL | #1 | 5' 5" | 8' 6" | 9' 1" | 11' 0" | 11' 5" | 13' 1" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #2 | 5' 3" | 7' 6" | 8' 0" | 10' 0" | 10' 9" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| | | | Standard | 5' 3" | 7' 6" | 8' 0" | 10' 0" | 10' 9" | 13' 0" | 13' 7" | 14' 0" | 14' 0" | 14' 0" | |
| 12" o.c. | SPF | #1 / #2 | #1 / #2 | 6' 1" | 10' 4" | 10' 8" | 12' 2" | 12' 8" | 13' 2" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #3 | 5' 9" | 10' 2" | 10' 7" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | Stud | 5' 9" | 10' 2" | 10' 7" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | HF | #1 | 5' 9" | 9' 4" | 9' 11" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 6' 4" | 10' 6" | 10' 10" | 12' 4" | 12' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | Standard | 5' 9" | 9' 4" | 9' 11" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | SP | #1 | #1 | 6' 1" | 10' 4" | 10' 8" | 12' 2" | 12' 8" | 13' 2" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 6' 1" | 10' 4" | 10' 8" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #3 | 5' 11" | 9' 10" | 10' 6" | 12' 1" | 12' 7" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | DFL | #1 | 5' 11" | 9' 10" | 10' 6" | 12' 1" | 12' 7" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 5' 9" | 9' 4" | 9' 11" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | Standard | 5' 9" | 9' 4" | 9' 11" | 12' 0" | 12' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |

Bracing Group Species and Grades:

| Group A: | | | |
|-------------------|----------|------------------|----------|
| Spruce-Pine-Fir | | Hem-Fir | |
| #1 / #2 | Standard | #2 | Stud |
| #3 | Stud | #3 | Standard |
| Douglas Fir-Larch | | Southern Pine*** | |
| #3 | | #3 | |
| Stud | | Stud | |
| Standard | | Standard | |
| Group B: | | | |
| Hem-Fir | | | |
| #1 & Btr | | | |
| #1 | | | |
| Douglas Fir-Larch | | Southern Pine*** | |
| #1 | | #1 | |
| #2 | | #2 | |

1x4 Braces shall be SRB (Stress-Rated Board).
***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.
Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).
Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.




Attach 'L' braces with 10d (0.128"x3.0" min) nails.
* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
'L' bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate Sizes | |
|--|------------|
| Vertical Length | No Splice |
| Less than 4' 0" | 1X4 or 2X3 |
| Greater than 4' 0", but less than 11' 6" | 2X4 |
| Greater than 11' 6" | 3X4 |

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



514 Earth City Expressway
Suite 242
Earth City, MO 63045

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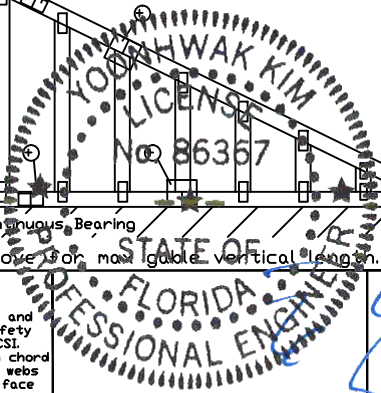
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REF ASCE7-16-GAB12015
DATE 01/26/2018
DRWG A12015ENC160118

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



Yoonhwak Kim, FL PE #86367

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

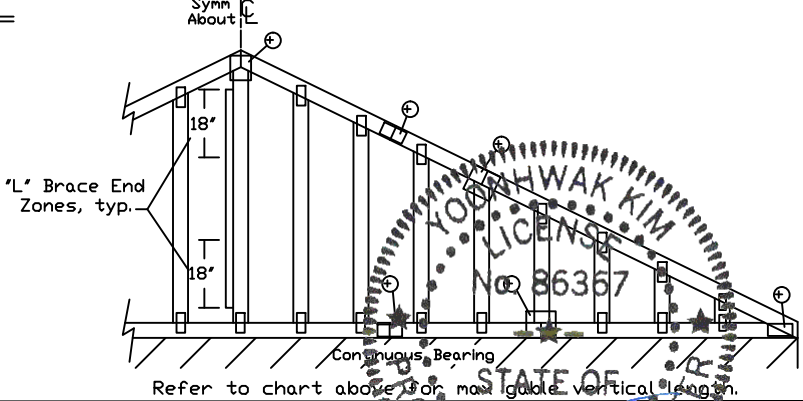
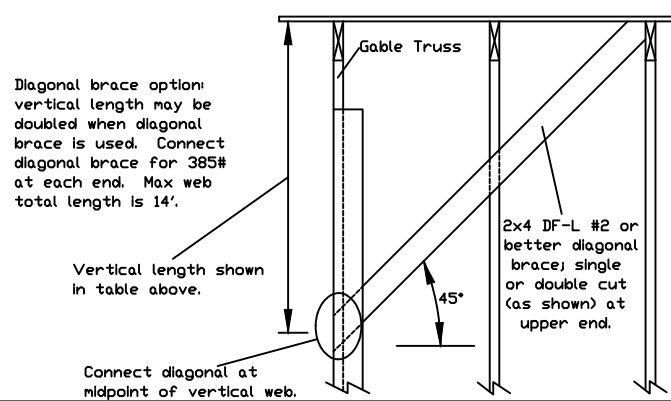
| Max Gable Vertical Length | 2x4 Gable Vertical Spacing | | Brace Grade | No Braces | (1) 1x4 'L' Brace * | | (1) 2x4 'L' Brace * | | (2) 2x4 'L' Brace ** | | (1) 2x6 'L' Brace * | | (2) 2x6 'L' Brace ** | | |
|---------------------------|----------------------------|----------|-------------|-----------|---------------------|---------|---------------------|---------|----------------------|---------|---------------------|---------|----------------------|---------|---------|
| | Species | Grade | | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A |
| | 24" o.c. | SPF | #1 / #2 | HF | #1 / #2 | 4' 7" | 7' 10" | 8' 1" | 9' 3" | 9' 7" | 11' 0" | 11' 5" | 14' 0" | 14' 0" | 14' 0" |
| #3 | | | | | 4' 4" | 7' 2" | 7' 8" | 9' 1" | 9' 5" | 10' 10" | 11' 4" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| Stud | | | | | 4' 4" | 7' 2" | 7' 7" | 9' 1" | 9' 5" | 10' 10" | 11' 4" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| Standard | | | #1 | 4' 4" | 6' 2" | 6' 7" | 8' 2" | 8' 9" | 10' 10" | 11' 4" | 12' 10" | 13' 9" | 14' 0" | 14' 0" | |
| | | | #2 | 4' 10" | 7' 11" | 8' 2" | 9' 4" | 9' 8" | 11' 1" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #3 | 4' 7" | 7' 10" | 8' 1" | 9' 3" | 9' 7" | 11' 0" | 11' 5" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| SP | | DFL | #1 | 4' 6" | 6' 6" | 6' 11" | 8' 7" | 9' 2" | 10' 11" | 11' 4" | 13' 6" | 14' 0" | 14' 0" | | |
| | | | #2 | 4' 6" | 6' 6" | 6' 11" | 8' 7" | 9' 2" | 10' 11" | 11' 4" | 13' 6" | 14' 0" | 14' 0" | | |
| | | | Standard | 4' 4" | 5' 9" | 6' 1" | 7' 7" | 8' 2" | 10' 4" | 11' 1" | 11' 11" | 12' 10" | 14' 0" | 14' 0" | |
| | | SPF | #1 / #2 | 5' 3" | 8' 11" | 9' 3" | 10' 7" | 11' 0" | 12' 7" | 13' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | | #3 | 5' 0" | 8' 10" | 9' 3" | 10' 5" | 10' 10" | 12' 5" | 12' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | | Stud | 5' 0" | 8' 9" | 9' 2" | 10' 5" | 10' 10" | 12' 5" | 12' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| SP | DFL | #1 | 5' 0" | 7' 6" | 8' 0" | 10' 1" | 10' 9" | 12' 5" | 12' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | #2 | 5' 6" | 9' 1" | 9' 5" | 10' 8" | 11' 1" | 12' 8" | 13' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | #3 | 5' 3" | 8' 11" | 9' 3" | 10' 7" | 11' 0" | 12' 7" | 13' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | Standard | #1 | 5' 1" | 7' 11" | 8' 5" | 10' 6" | 10' 11" | 12' 6" | 13' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | #2 | 5' 0" | 7' 0" | 7' 5" | 9' 4" | 10' 0" | 12' 5" | 12' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | #3 | 5' 0" | 7' 0" | 7' 5" | 9' 4" | 10' 0" | 12' 5" | 12' 11" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| 12" o.c. | SPF | #1 / #2 | HF | #1 / #2 | 5' 9" | 9' 10" | 10' 2" | 11' 7" | 12' 1" | 12' 7" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | | | #3 | 5' 6" | 9' 8" | 10' 1" | 11' 6" | 11' 11" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | | | Stud | 5' 6" | 9' 8" | 10' 1" | 11' 6" | 11' 11" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | |
| | | Standard | #1 | 5' 6" | 8' 8" | 9' 3" | 11' 6" | 11' 11" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 6' 0" | 10' 0" | 10' 4" | 11' 9" | 12' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #3 | 5' 9" | 9' 10" | 10' 2" | 11' 7" | 12' 1" | 13' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | SP | DFL | #1 | 5' 8" | 9' 2" | 9' 9" | 11' 6" | 12' 0" | 13' 9" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 5' 8" | 9' 2" | 9' 9" | 11' 6" | 12' 0" | 13' 9" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | Stud | 5' 8" | 9' 2" | 9' 9" | 11' 6" | 12' 0" | 13' 9" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | Standard | #1 | 5' 6" | 8' 1" | 8' 7" | 10' 9" | 11' 6" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #2 | 5' 6" | 8' 1" | 8' 7" | 10' 9" | 11' 6" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |
| | | | #3 | 5' 6" | 8' 1" | 8' 7" | 10' 9" | 11' 6" | 13' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" | | |

Bracing Group Species and Grades:

| Group A: | | | |
|-------------------|----------|------------------|----------|
| Spruce-Pine-Fir | | Hem-Fir | |
| #1 / #2 | Standard | #2 | Stud |
| #3 | Stud | #3 | Standard |
| Douglas Fir-Larch | | Southern Pine*** | |
| #3 | Stud | #3 | Standard |
| Group B: | | | |
| Hem-Fir | | | |
| #1 & Btr | | | |
| #1 | | | |
| Douglas Fir-Larch | | Southern Pine*** | |
| #1 | Stud | #1 | Standard |
| #2 | Stud | #2 | Standard |

1x4 Braces shall be SRB (Stress-Rated Board).
***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.
Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).
Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

*'L' bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate Sizes | |
|--|------------|
| Vertical Length | No Splice |
| Less than 4' 0" | 1X4 or 2X3 |
| Greater than 4' 0", but less than 11' 6" | 2X4 |
| Greater than 11' 6" | 3X4 |

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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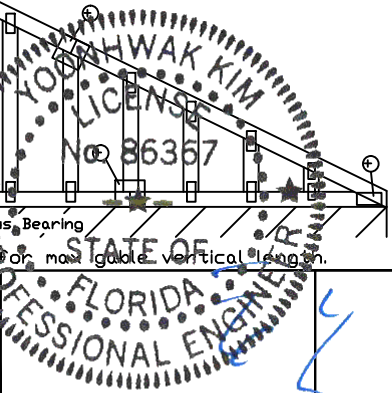
IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING. FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 150A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this Job's general notes page and these web sites: www.alpineitw.com; www.tpinst.org; www.sbcindustry.org; www.icd.com



| | |
|----------------------|-------------------|
| REF | ASCE7-16-GAB12030 |
| DATE | 01/26/2018 |
| DRWG | A12030ENC160118 |
| MAX. TOT. LD. 60 PSF | |
| MAX. SPACING 24.0" | |

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

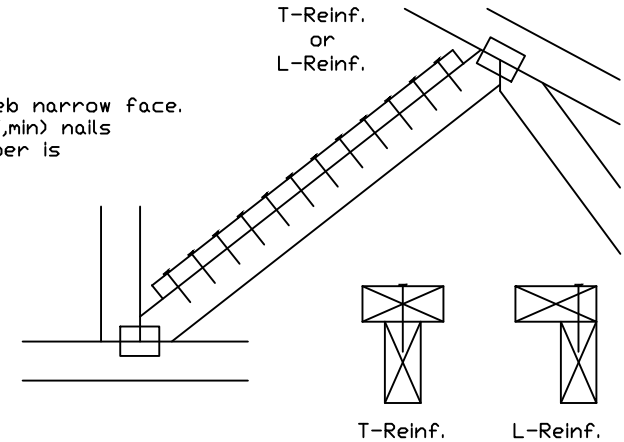
| Web Member Size | Specified CLR Restraint | Alternative Reinforcement T- or L- Reinf. | Scab Reinf. |
|-----------------|-------------------------|---|-------------|
| 2x3 or 2x4 | 1 row | 2x4 | 1-2x4 |
| 2x3 or 2x4 | 2 rows | 2x6 or 2x4 | 2-2x4 |
| 2x6 | 1 row | 2x4 | 1-2x6 |
| 2x6 | 2 rows | 2x6 | 2-2x4(⊗) |
| 2x8 | 1 row | 2x6 | 1-2x8 |
| 2x8 | 2 rows | 2x6 | 2-2x6(⊗) |

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

⊗ Center scab on wide face of web. Apply (1) scab to each face of web.

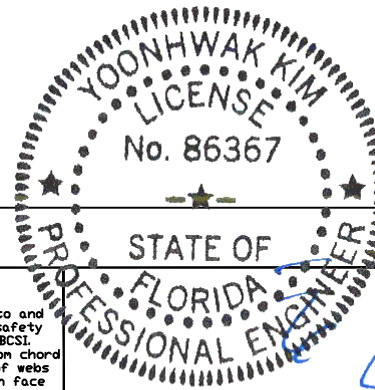
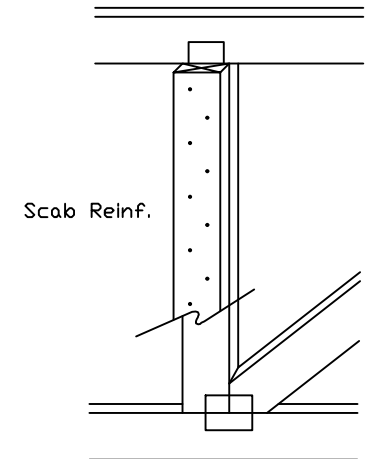
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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| | | |
|-----------|-----|-------------------|
| TC LL | PSF | REF CLR Subst. |
| TC DL | PSF | DATE 01/02/19 |
| BC DL | PSF | DRWG BRCLBSUB0119 |
| BC LL | PSF | |
| TOT. LD. | PSF | |
| DUR. FAC. | | |
| SPACING | | |

Yoonhwak Kim, FL PE #86367

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

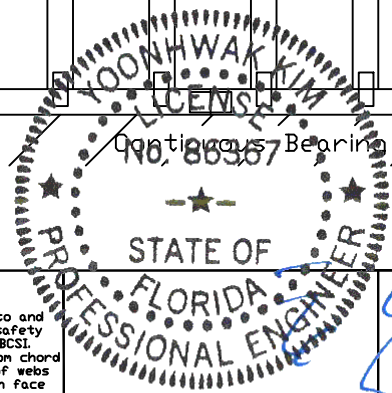
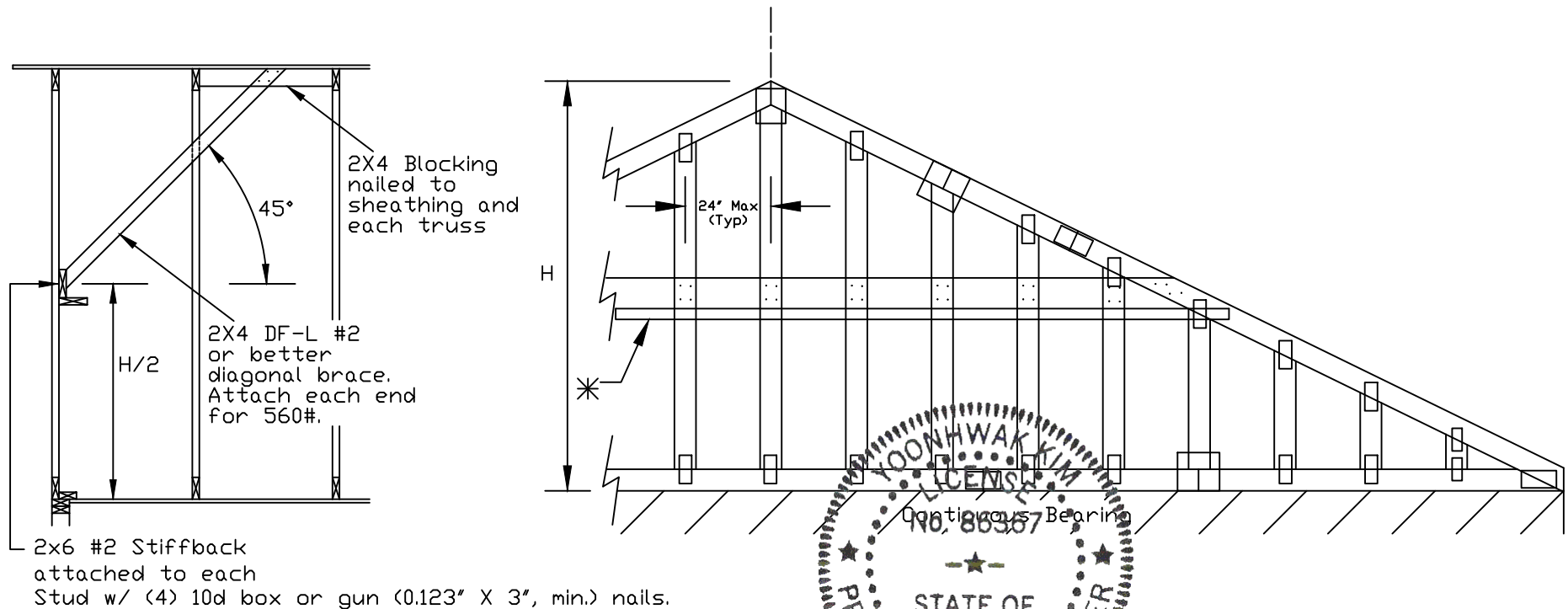
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



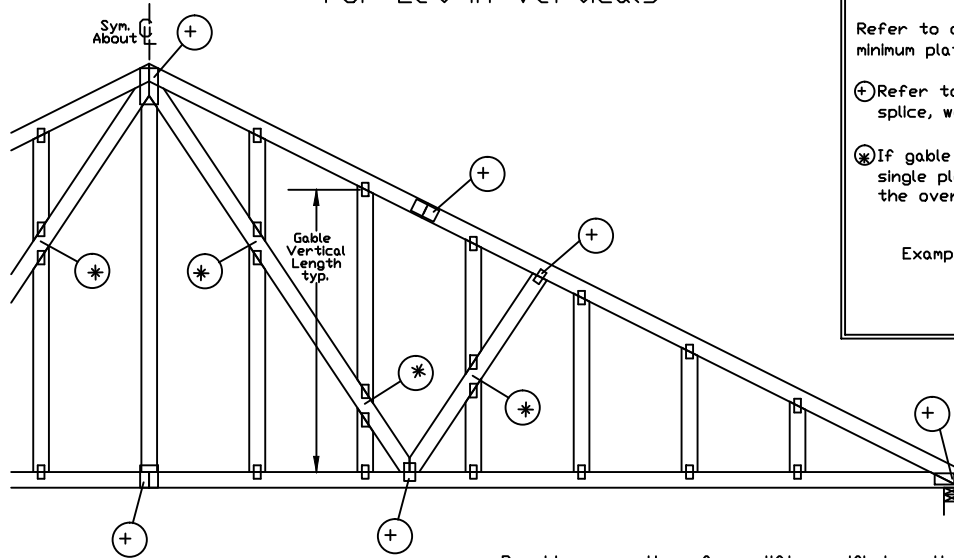
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For more information see this Job's general notes page and these web sites: 16/2021
ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

Yoonhwak Kim, FL PE #86367

MAX. TOT. LD. 60 PSF
MAX. SPACING

| | |
|------|--------------|
| REF | GE WHALER |
| DATE | 01/02/2018 |
| DRWG | GABRST160118 |

Gable Detail For Let-in Verticals

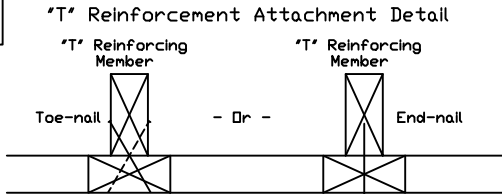


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with
End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PE1D100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PE1D100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PE1D100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PE1D100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

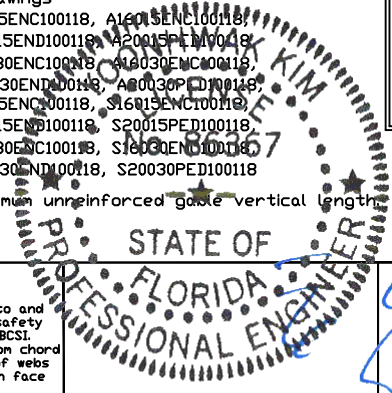
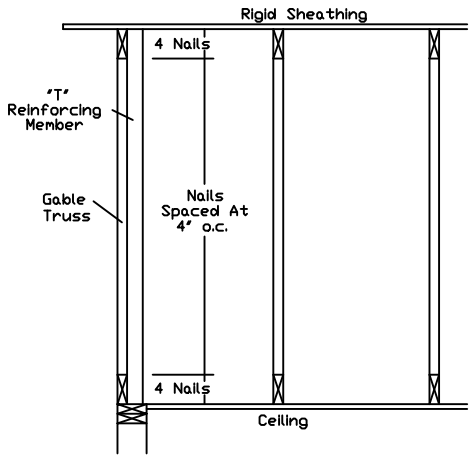
To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.
"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

| "T" Reinf. Mbr. Size | "T" Increase |
|----------------------|--------------|
| 2x4 | 30 % |
| 2x6 | 20 % |

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



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| | |
|---------------|--------------|
| REF | LET-IN VERT |
| DATE | 01/02/2018 |
| DRWG | GBLLETIN0118 |
| MAX. TOT. LD. | 60 PSF |
| DUR. FAC. | ANY |
| MAX. SPACING | 24.0" |

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

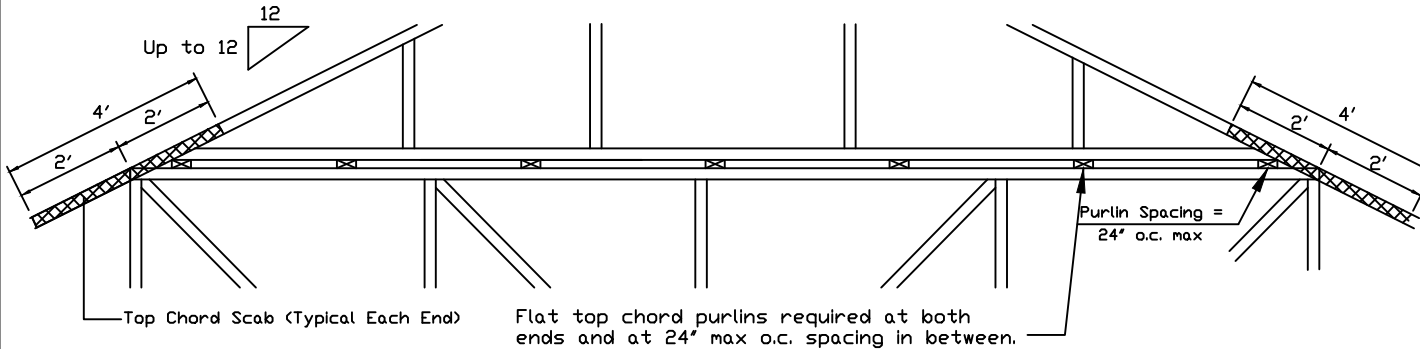
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

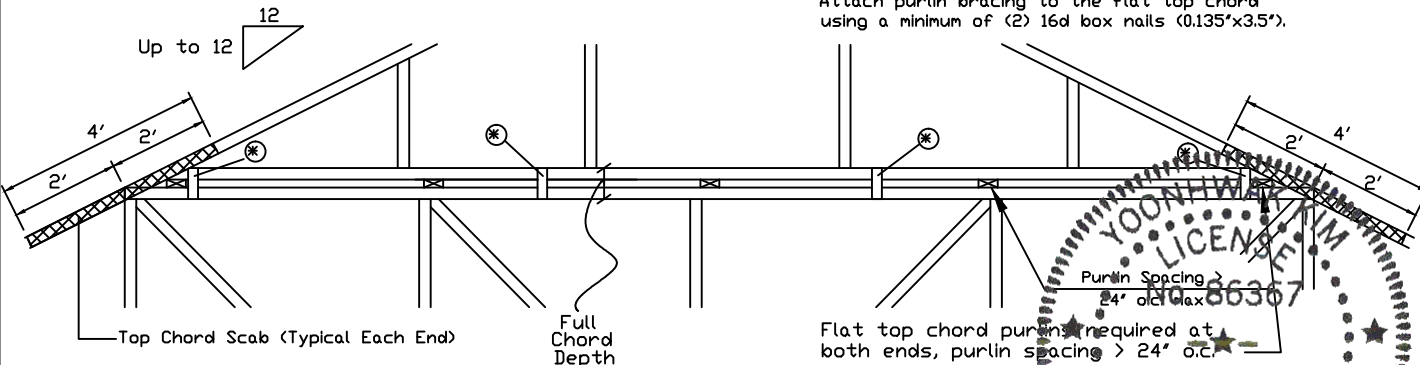


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox
 Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
 One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING. FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

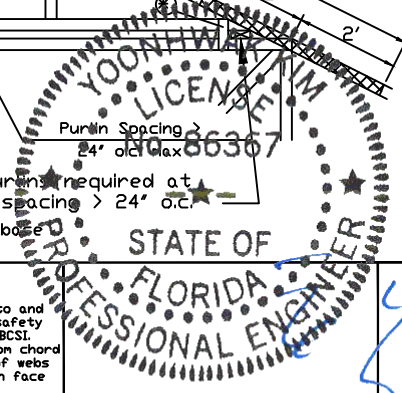
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites: www.alpineitw.com; www.tpinst.org; www.sbcindustry.org; www.iccsafe.org



13723 Riverport Drive
 Suite 200
 Maryland Heights, MO 63043



REF PIGGYBACK
 DATE 01/02/2018
 DRWG PB160160118

SPACING 24.0"

Yoonhwak Kim, FL PE #86367

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

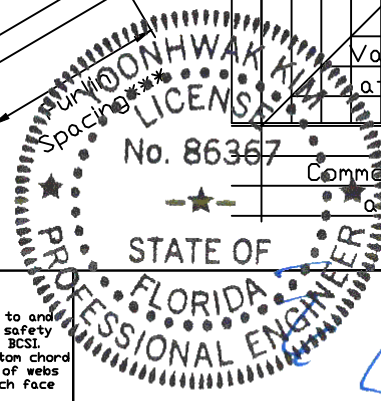
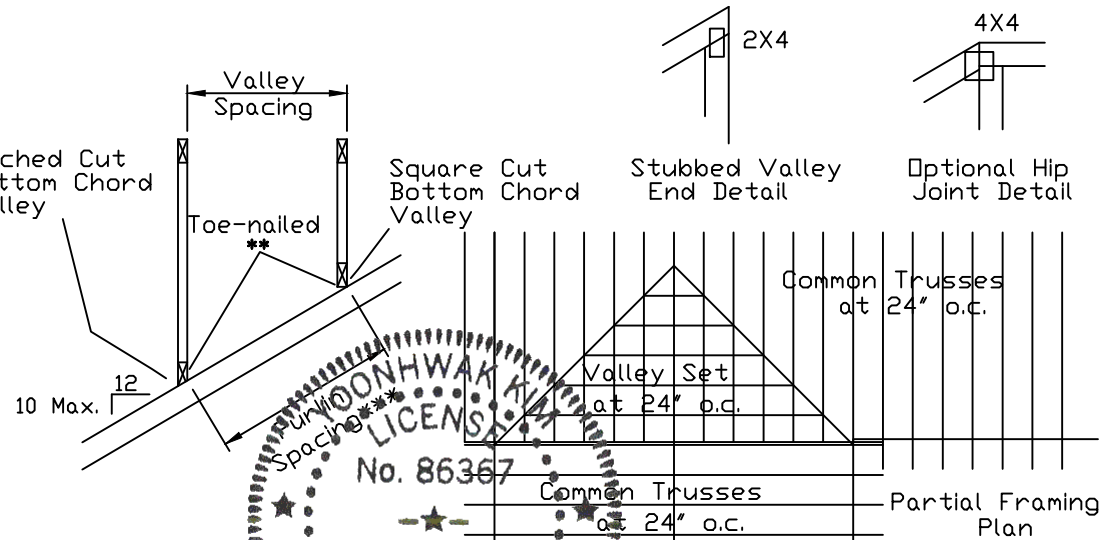
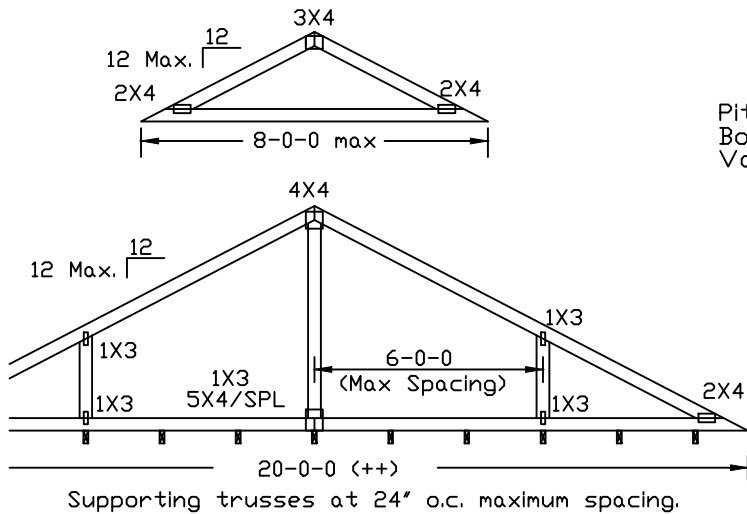
Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



ALPINE
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| | | | | | |
|----------|-----------|------|--------|------|---------------|
| TC LL | 30 | 30 | 40PSF | REF | VALLEY DETAIL |
| TC DL | 20 | 15 | 7PSF | DATE | 01/26/2018 |
| BC DL | 10 | 10 | 10 PSF | DRWG | VALTN160118 |
| BC LL | 0 | 0 | 0 PSF | | |
| TOT. LD. | 60 | 55 | 57PSF | | |
| DUR.FAC. | 1.25/1.33 | 1.15 | 1.15 | | |
| SPACING | 24.0" | | | | |