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GENERAL REQUIREMENTS

1. THE GENERAL STRUCTURAL NOTES EMPLOY THE FOLLOWING DEFINITIONS AND ABBREVIATIONS:
2. CONTRACT DOCUMENTS – THE LATEST SET OF DRAWINGS, SPECIFICATIONS, AND RECORDED ADDENDA AND AMENDMENTS ISSUED FOR BID OR CONSTRUCTION.
3. LICENSED PROFESSIONAL ENGINEER – AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED AND QUALIFIED TO PERFORM THE WORK REQUIRED.
4. STRUCTURAL ENGINEER OF RECORD – LICENSED PROFESSIONAL ENGINEER WHO IS IN RESPONSIBLE CHARGE FOR THE PREPARATION, REVIEW, DATING, SEALING, AND ISSUING OF STRUCTURAL ENGINEERING DOCUMENTS FOR ENGINEERING SERVICE OR CREATIVE WORK.
5. DELEGATED ENGINEER – A LICENSED PROFESSIONAL ENGINEER WHO PROVIDES SERVICES OR CREATIVE WORK REGARDING A PORTION OF THE ENGINEERING PROJECT. THE DELEGATED ENGINEER IS THE ENGINEER OF RECORD FOR THAT PORTION OF THE ENGINEERING PROJECT. TYPICALLY, DELEGATED ENGINEERS FALL INTO ONE OF THE FOLLOWING CATEGORIES:
- a. AN INDEPENDENT CONSULTANT
- b. AN EMPLOYEE OR OFFICE OF AN ENTITY SUPPLYING COMPONENTS TO A FABRICATOR OR CONTRACTOR
- c. AN EMPLOYEE OR OFFICE OF A FABRICATOR OR CONTRACTOR
6. DELEGATED ENGINEERING DOCUMENTS – ENGINEERING DOCUMENTS THAT ARE PREPARED BY A DELEGATED ENGINEER.
7. DESIGN TEAM – DESIGN PROFESSIONALS INCLUDING THE ARCHITECT, STRUCTURAL ENGINEER, CIVIL ENGINEER, MEP ENGINEER, AND ANY OTHER CONSULTANT THAT ISSUES CONTRACT DOCUMENTS.
8. CONTRACTOR – GENERAL CONTRACTOR, CONSTRUCTION MANAGER, DESIGN-BUILDER, OR ANY OTHER ENTITY CONTRACTED BY THE OWNER TO PERFORM THE WORK.
9. SHOP DRAWINGS – DRAWINGS DEPICTING INSTALLATION MEANS AND METHODS AND CATALOG INFORMATION ON STANDARD PRODUCTS. SHOP DRAWINGS SHALL BE PREPARED BASED ON ENGINEERING DIRECTION CONTAINED IN CONTRACT DOCUMENTS BY A CONTRACTOR, FABRICATOR, MANUFACTURER, OR LICENSED PROFESSIONAL ENGINEER, FOR INCORPORATION INTO THE PROJECT.
10. ESTABLISHED CHANNELS – AT THE ONSET OF THE PROJECT, ARCHITECT, OWNER, AND CONTRACTOR SHALL ESTABLISH DESIRED LINES OF COMMUNICATION BETWEEN ALL PROJECT PARTIES. THESE AGREED UPON LINES OF COMMUNICATION ARE THE ESTABLISHED CHANNELS.
11. GENERAL STRUCTURAL NOTES ARE APPLICABLE TO THE STRUCTURAL DESIGN AND CONSTRUCTION OF THE ENTIRE PROJECT AND THIS ARE APPLICABLE TO EVERY SHEET WITHIN THIS SET.
12. WHERE A DETAIL, TYPICAL DESIGN, SECTION, TYPICAL SECTION, OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL CONDITIONS, UNLESS NOTED OTHERWISE.
13. ISOMETRIC VIEWS ARE FOR VISUALIZATION PURPOSES ONLY AND DO NOT CONVEY ALL OF THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
14. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT BETWEEN THESE DRAWINGS AND ANY OTHER CONTRACT DOCUMENT OR APPLICABLE CODE OR STANDARD OF PRACTICE DURING BUILDING, THE PROVISION RESULTING IN THE GREATER COST APPLIES. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT DURING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT A REQUEST FOR CLARIFICATION TO THE DESIGN TEAM, WHO WILL PROVIDE A WRITTEN RESPONSE IN RETURN.
15. SPECIFICATIONS HAVE BEEN ISSUED ON THIS PROJECT BY THE STRUCTURAL ENGINEER OF RECORD AND ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS.
16. SEE SPECIFICATIONS FOR MATERIALS TESTING REQUIREMENTS.
17. THE CONTRACTOR SHALL SUPERVISE AND DIRECT ALL WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE QUALITY AND CORRECTNESS OF THE WORK.
18. THE BUILDING SHALL BE SOLELY RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH OTHER TRADES INCLUDING, BUT NOT LIMITED TO, ARCHITECTURAL, CIVIL, AND MEP FOR FLOOR SLAB STEPS, SLOPES AND CURBS, FLOOR SLAB FINISH, OPENINGS IN STRUCTURAL FLOORS, ROOFS AND WALLS, ETC.
19. THE BUILDING HAS BEEN DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD TO RESIST THE CODE REQUIRED VERTICAL AND LATERAL FORCES IN ITS FULLY COMPLETED CONDITION. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING, SHORING, AND OTHER CONSTRUCTION SUPPORTS NECESSARY TO ENSURE THE BUILDING'S STABILITY AND SAFETY THROUGHOUT THE DURATION OF CONSTRUCTION. FURTHER, THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO PROVIDE THE ANALYSIS AND DESIGN NECESSARY TO DETERMINE POTENTIALLY OVERLOADED, UNSTABLE, OR HAZARDOUS CONDITIONS THAT MAY OCCUR AT ANY STAGE DURING CONSTRUCTION.
20. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE CONTRACT DOCUMENTS AND SHOP DRAWINGS.
21. THE CONTRACTOR SHALL NOT EMPLOY CONSTRUCTION MEANS OR METHODS THAT MAY DAMAGE UTILITIES, ADJACENT BUILDINGS, OR PROPERTY. DOCUMENTATION OF ADJACENT CONDITIONS PRIOR TO CONSTRUCTION IS RECOMMENDED. FURTHER, THE CONTRACTOR SHALL EITHER ADEQUATELY CONFINE THE SITE OR PROTECT ADJACENT PROPERTY FROM DAMAGE.
22. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROJECT SAFETY AND OSHA REQUIREMENTS. SHOULD THE STRUCTURAL ENGINEER OF RECORD NOTIFY THE CONTRACTOR OF A POTENTIALLY UNSAFE CONDITION, IT IS SOLELY AS A COURTESY FROM ONE PROFESSIONAL TO ANOTHER. IT SHOULD NOT BE INTERPRETED AS THE STRUCTURAL ENGINEER OF RECORD ASSUMING ANY RESPONSIBILITY FOR PROJECT SAFETY.
23. ALL STRUCTURES REQUIRE MAINTENANCE TO EXTEND LIFE SPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE, BUT NOT BE LIMITED TO: PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANT JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALINE OR OTHER HARSH CHEMICAL ENVIRONMENT.
24. THE USE OF DE-CARCINICALS ON ANY EXPOSED STRUCTURAL ELEMENT IS DISCOURAGED AND WILL ACCELERATE DETERIORATION OF STRUCTURAL ELEMENTS.
25. THE BUILDING OWNER SHALL NOT ALTER OR MODIFY ANY STRUCTURAL ELEMENT WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER. FURTHER, THE BUILDING OWNER SHALL NOT RENOVATE, REPURPOSE, ADD-ON TO, OR OTHERWISE MODIFY THE EXISTING STRUCTURAL SYSTEMS WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER.
26. CONTRACT DRAWINGS MAJOR OPENINGS IN FLOORS AND WALLS AND DO NOT NECESSARILY SHOW ALL OPENINGS REQUIRED. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS BETWEEN ALL DISCIPLINES AND TRADES. ADDITIONAL OPENINGS, BLOCKOUTS, AND SLEEVES MAY BE REQUIRED AND SHALL BE CONSTRUCTED USING THE FOLLOWING DETAILS AND REQUIREMENTS WITHIN THE CONTRACT DOCUMENTS. OPENINGS REQUIRED, BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
27. THE CONTRACTOR SHALL COORDINATE PIPING AND CONDUIT EMBEDDED IN OR ATTACHED TO SLABS, SLABS-ON-DECK, BEAMS, AND COLUMNS. ANY REQUIRED MODIFICATIONS TO STRUCTURAL MEMBERS OR THEIR REINFORCEMENT AS A RESULT OF EMBEDMENT OR ATTACHMENT SHALL BE SUBMITTED TO THE DESIGN TEAM FOR THEIR REVIEW. SEE GENERAL STRUCTURAL NOTES SECTION "DESIGN CRITERIA" FOR LIMITATIONS OF MEP LOADING ON STRUCTURAL SYSTEMS.
28. THE STRUCTURAL ENGINEER OF RECORD'S ROLE DURING CONSTRUCTION
1. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT ASSUME CONTROL OF, OR RESPONSIBILITY FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, PROJECT SAFETY, THE ACTS AND OMISSIONS OF THE CONTRACTOR, OR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. STRUCTURAL ENGINEER OF RECORD SHALL NOT HAVE AUTHORITY TO STOP THE WORK OR AUTHORIZE CHANGES TO ANY CONTRACT SUM.
3. PERIODIC SITE VISITS BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER OF RECORD ARE SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS LIMITED OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS AND THAT OBSERVATIONS ARE QUALITATIVE, NOT QUANTITATIVE. THIS LIMITED INFORMATION WILL BE USED TO ADVISE THE OWNER/CONTRACTOR/ARCHITECT OF POTENTIAL DEFICIENCIES.
4. CLARIFICATION OF POSITION OF STRUCTURALLY FRAMING ELEMENTS
1. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS, DO NOT SCALE ANY DIMENSIONS.
2. IF NOT INDICATED ON DRAWINGS, ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS.
3. CENTER LINES OF FOUNDATIONS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTION, UNLESS NOTED OTHERWISE.
4. CENTER LINES OF COLLINGS, GRADE BEAMS, AND WALLS SHALL COINCIDE WITH CENTER LINES OF FOUNDATIONS, UNLESS NOTED OTHERWISE.
5. CENTER LINES OF FRAMING MEMBERS SHALL COINCIDE WITH COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.
6. ELEVATIONS SHOWN ARE TO TOP OF FOUNDATIONS, SLABS, OR BEAMS, UNLESS NOTED OTHERWISE.
7. SEE ARCHITECTURAL, CIVIL, MEP, AND VERTICAL TRANSPORTATION CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION RELATING TO THE COORDINATION OF STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO:
1. CIVIL
- a. SITTING OF BUILDING GRID LINES WITH RESPECT TO CITY BENCHMARKS
- b. SITE PREPARATION
- c. BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
- d. SITE ELEMENTS OUTSIDE OF BUILDING ENVELOPE
- e. NEW AND EXISTING SITE UTILITIES
2. ARCHITECTURAL
- a. PLAN DIMENSIONS AND PROJECT DATUM
- b. SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
- c. WATERPROOFING AND DAMP PROOFING DETAILS
- d. SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
- e. EMBEDMENTS, INSERTS, BLOCKOUTS, ETC.
- f. CONCRETE FINISHES AND TOPPING SLABS
- g. CONCRETE CURBS AND HOUSEKEEPING PADS
- h. INTERIOR NON-STRUCTURAL MASONRY PARTITIONS
- i. LIFE SAFETY, FIRE RATING
- j. OPERABLE PARTITIONS
3. MEP
- a. PIPE AND DUCT SIZES FOR OPENING AND SLEEVE COORDINATION
- b. FLOOR DRAINS
- c. UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS
- d. EQUIPMENT CURBS
- e. CONDUITS AND EMBEDMENTS IN WALLS AND SLABS
4. THIS BUILDING DOES NOT QUALIFY AS A THRESHOLD BUILDING PER CHAPTER 553.71 OF THE FLORIDA STATUTES. HOWEVER, THE OWNER HAS ELECTED TO PERFORM SPECIAL INSPECTIONS ABOVE AND BEYOND THOSE REQUIRED BY THE AUTHORITY HAVING JURISDICTION. SEE SPECIAL INSPECTION PLAN FOR SPECIFIC INSPECTION REQUIREMENTS.

ELECTRONIC DATA/REPRODUCTION

1. ALL INFORMATION CONTAINED IN THE ELECTRONIC FILES OF THE CONTRACT DOCUMENTS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT, OR THE COMPLETION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
2. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, DIRECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS DOES NOT CONSTITUTE THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN.
3. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTORS RESPONSIBILITY FOR PROPER CHECKING OF DIMENSIONS AND ELEMENT SIZES AND LOCATIONS IN THE ELECTRONIC FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.
4. WHEN USED FOR THE PREPARATION OF SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBSECTION SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.

SUBMITTALS

1. REFER TO DIVISION 01 OF SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT.
2. SUBMIT SPECIFIC COMPONENTS SUCH AS COLUMNS, FOUNDATIONS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
3. TEN WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT, FOR REVIEW AND COMMENT BY THE STRUCTURAL ENGINEER OF RECORD, A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE STRUCTURAL ENGINEER OF RECORD. THE STRUCTURAL ENGINEER OF RECORD SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWINGS SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE STRUCTURAL ENGINEER OF RECORD WILL RETURN THE SHOP DRAWING ITEMS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE REPRODUCIBLE SHOP DRAWING.
4. THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR SHALL STAMP EACH SUBMITTAL, VERIFYING THAT THE FOLLOWING IS ADDRESSED:
1. THE SUBMITTAL IS REQUESTED.
2. THE SUBMITTAL IS BASED ON THE LATEST DESIGN.
3. THE SUBMITTAL IS CLEARLY CLOUDDED FOR ALL THE DIFFERENCES FROM THE CONTRACT DOCUMENTS ON THE DESIGN INTENT.
4. THE SUBMITTAL IS CLEARLY CLOUDDED FOR ALL CHANGES AND ADDITION FROM PREVIOUS SUBMITTAL.
5. THE ARCHITECTS AND STRUCTURAL ENGINEER OF RECORD'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
6. THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
7. THE SUBMITTAL IS COMPLETE.
8. THE SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.
9. THE STRUCTURAL ENGINEER OF RECORDS REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT.
10. THE STRUCTURAL ENGINEER OF RECORD SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS.
11. FOR THE COMPONENTS DESIGNED BY A DELEGATED ENGINEER, PROVIDE SHOP DRAWINGS, DESIGN CALCULATIONS, AND A COVER LETTER SIGNED AND SEALED BY THE DELEGATED ENGINEER. LETTER SHALL INDICATE THAT THE SHOP DRAWINGS ARE IN CONFORMANCE WITH THE DELEGATED ENGINEER'S CALCULATIONS.
12. DEFERRED SUBMITTALS ARE MANUFACTURER OR CONTRACTOR DESIGNED COMPONENTS PER THE CONTRACT DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE DESIGN DOCUMENTS FOR DEFERRED SUBMITTAL. ITEMS SHALL BE SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD, WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE CONTRACTOR SHALL SUBMIT THESE REVIEWED DEFERRED SUBMITTAL DOCUMENTS TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD FOR REVIEW. THESE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN TEAM HAS REVIEWED AND THE BUILDING OFFICIAL HAS APPROVED. SEE BELOW FOR THE LIST OF DEFERRED SUBMITTALS.
13. THE FOLLOWING SUBMITTALS ARE REQUIRED TO BE SUBMITTED FOR STRUCTURAL ENGINEER OF RECORD REVIEW AS OUTLINED IN THE SPECIFICATIONS:

013000	CONCRETE FORMWORK	(SS, CALC)
032000	CONCRETE REINFORCEMENT LAYOUT	(SS)
033000	CONCRETE MIX DESIGNS	(CALC, TA)
033000	CONCRETE CONSTRUCTION JOINT LAYOUT	(SS)
034713	TILT-UP CONCRETE PANEL	(OF, SS, CALC)
034713	TILT-UP CONCRETE PANEL LIFTING	(OF, SS, CALC)
051000	STRUCTURAL STEEL	(S)
051000	STRUCTURAL STEEL CONNECTIONS	(OF, S, CALC)
052100	STEEL JOISTS, BRIDGING, AND CONNECTIONS	(OF, SS, CALC)
052100	STEEL COMPOSITE DECK	(S)
052100	HANDRAIL, GUARDRAIL, RAILING	(SS, CALC, REC)
054000	COLD-FORMED METAL FRAMING USED FOR EXTERIOR	(SS, CALC, SEE ARCH)
323139	DEWATERING	(SS, REC)

- S = SHOP DRAWING REQUIRED
- OF = DEFERRED SUBMITTAL
- SS = SIGNED AND SEALED SHOP DRAWINGS PREPARED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED
- CALC = SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED
- REC = ITEMS SUBMITTED FOR RECORD ONLY AND WILL NOT HAVE STRUCTURAL ENGINEER OF RECORD SHOP DRAWING STAMP AFFIXED
- GEN = ITEMS SUBMITTED TO CONSTRUCTION GEOTECHNICAL ENGINEER FOR THEIR REVIEW
- TA = ITEMS SUBMITTED TO OWNER'S TESTING AGENCY FOR THEIR REVIEW

GOVERNING CODES & STANDARDS

BUILDING CODE:	FBC 2023	FLORIDA BUILDING CODE: BUILDING
STANDARDS:	ASCE 7	AMERICAN SOCIETY OF CIVIL ENGINEERS: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
	ACI 318	AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
	AISC 360	AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
	AWS D1.1	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL
	AWS D1.3	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL
	AWS D1.4	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - REINFORCING STEEL
	ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
	AIS S100	NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS

DESIGN CRITERIA

1. STRUCTURE LOCATION:
LONGITUDE: -82.6574
LATITUDE: 30.2034
2. LOADING:
SUPERIMPOSED DEAD & LIVE LOADS:
ALL DEAD LOADS LISTED BELOW ARE IN ADDITION TO THE STRUCTURE'S SELF-WEIGHT.
3. ROOF:
TYPICAL
10 PSF
20 PSF REDUCIBLE
4. FLOOR:
OFFICE
20 PSF
CORRIDORS
10 PSF
STAIRS
10 PSF
STORAGE
20 PSF
MECHANICAL ROOM
150 PSF
5. RAIN LOAD:
DESIGN RAINFALL: 4.44" (HOUR (100-YEAR, 1-HOUR RAINFALL))
RAINFALL AT LOWEST POINT OF ROOF SHALL NOT POND DURING DESIGN RAINFALL AND SHALL NOT EXCEED 3.75" DURING DESIGN RAINFALL
DESIGN RAIN LOAD, R: 20 PSF
6. SNOW LOAD:
GROUND SNOW LOAD: 0 PSF
WIND LOAD:
ULTIMATE DESIGN WIND SPEED, V₁₀₀: 26 MPH
NOMINAL DESIGN WIND SPEED, V₁₀₀: 175 MPH
RISK CATEGORY: IV
WIND EXPOSURE CATEGORY: C
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: +0.18
COMPONENTS & CLADDING DESIGN PRESSURES: SEE WIND PRESSURE DIAGRAMS
7. TORNADO LOAD:
INTERNAL PRESSURE COEFFICIENT: +0.18
WIND EXPOSURE CATEGORY: C
EFFECTIVE PLAN AREA: 4,000.00 SF
ULTIMATE TORNADO SPEED, V₁₀₀: 118 MPH
PER 20.2.2 OF ASCE 7, DESIGN FOR TORNADO LOADS SHALL NOT BE REQUIRED FOR STRUCTURES IN EXPOSURE C WITH A V₁₀₀ ≤ 80.5 V₁₀₀.
8. SEISMIC LOAD:
RISK CATEGORY: IV
SEISMIC IMPORTANCE FACTOR, I: 1.5
FIRST SUBMITTAL:
1. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_s: 0.087 g
2. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S₁: 0.051 g
3. SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{0.1}: 0.262 g
4. SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{0.2}: 0.082 g
5. SEISMIC DESIGN CATEGORY: A
6. SEISMIC DESIGN RESISTING SYSTEM: NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
9. HANDRAIL AND GUARDRAILS:
CONCENTRATED AND DISTRIBUTED LOADS ARE TO BE APPLIED AT THE HANDRAIL OR TOP RAIL IN ANY DIRECTION. CONCENTRATED AND DISTRIBUTED LOADS ARE NOT TO BE APPLIED CONCURRENTLY.
10. DISTRIBUTED LOAD:
20 PLF
11. FUTURE EXPANSION:
THE COMPONENTS HAVE BEEN MADE FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION OF THE STRUCTURE.
12. SERVICEABILITY:
1. DEFLECTION LIMITS: TOTAL DEFLECTION UNLESS OTHERWISE SPECIFIED SHALL BE LIMITED TO THE FOLLOWING:
a. ROOF MEMBERS:
i. TOTAL LOAD DEFLECTION: L/240
ii. TOTAL DEAD LOAD DEFLECTION: L/360
b. FLOOR MEMBERS:
i. TOTAL LOAD DEFLECTION: L/240
ii. TOTAL DEAD LOAD DEFLECTION: L/360
2. DRIFT LIMITS:
a. INTERSTORY DRIFT: L/600
b. TOTAL STRUCTURE DRIFT: L/600

EARTHWORK & FOUNDATIONS

1. GEOTECHNICAL INVESTIGATION REPORT
1. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT AS FOLLOWS:
a. REPORT TITLE: GEOTECHNICAL ENGINEERING REPORT-PROJECT #41- TERRACON PROJECT NO. E0215160
b. PREPARED BY: TERRACON CONSULTANTS, INC.
c. DATED: APRIL 21, 2022
2. THE GEOTECHNICAL INVESTIGATION REPORT IS AVAILABLE TO THE CONTRACTOR UPON REQUEST TO THE OWNER. THE INFORMATION HEREIN MAY BE USED BY THE CONTRACTOR FOR HIS GENERAL REFERENCE ONLY. THE GEOTECHNICAL INVESTIGATION REPORT RECOMMENDATIONS SHALL SUPERSEDE THE MINIMUM CRITERIA STATED IN THE STRUCTURAL GENERAL NOTES.
3. SHALLOW FOUNDATIONS
1. FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
2. FOUNDATION SIZES AND REINFORCEMENT ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 2500 PSF FOR COLUMNS AND 2,000 PSF FOR WALLS.
3. FOUNDATIONS SHALL BEAR A MINIMUM OF 2'-0" BELOW ADJACENT EXTERIOR GRADE.
4. FOUNDATIONS SHALL BEAR ON COMPACTED STRUCTURAL FILL, NATURAL SOILS, OR ROCK PREPARED PER THE GEOTECHNICAL INVESTIGATION REPORT.
5. PRIOR TO PLACEMENT OF CONCRETE, A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY SOILS CONFORMANCE TO THE RECOMMENDATIONS AND ASSUMPTIONS IN THE GEOTECHNICAL INVESTIGATION REPORT. ALL ADVERSE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD.
6. SOILS BELOW FOUNDATIONS NOT MEETING DESIGN BEARING PRESSURE SHALL BE REMEDIATED PER THE GEOTECHNICAL INVESTIGATION REPORT AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATIONS.
7. CENTER ALL FOUNDATIONS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, UNLESS NOTED OTHERWISE.
8. TOP OF FOUNDATION ELEVATIONS PROVIDED ON THE CONTRACT DRAWINGS ARE FOR PURPOSE OF THE CONTRACT AND SHALL BE ADJUSTED, AS REQUIRED, AT THE TIME OF EXCAVATION TO BEAR ON PROPERLY PREPARED SUPPORT SUBGRADE (PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS).
9. EARTHWORK AND EXCAVATION
1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING, BUT NOT LIMITED TO: LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS.
2. EXCAVATION SHALL NOT OCCUR WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROTECTED AGAINST SETTLEMENT.
3. THE EXTENT OF SUBGRADE PREPARATION SHALL EXTEND A MINIMUM OF 5'-0" BEYOND THE BUILDING PERIMETER.
4. THE CONTRACTOR SHALL PROVIDE A SUBGRADE BENEATH THE SLAB-ON-GROUND PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
5. UNLESS NOTED IN THE GEOTECHNICAL INVESTIGATION REPORT, COMPACT FILL TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER SHALL NOT EXCEED 4" LOOSE THICKNESS. COMPACT PRIOR TO THE PLACEMENT OF THE NEXT LAYER. COMPACT FILL MEET ALL RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT.
6. PLACEMENT OF FILL AND COMPACTION SHALL BE MONITORED AND ACCEPTED BY A RETAINED TESTING AGENCY. PERFORM A MINIMUM OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-6938) FOR EVERY 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.
7. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF THE CONSTRUCTION DEWATERING SYSTEMS REQUIRED FOR THE EXCAVATION. AT A MINIMUM, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING SITE.
8. THE CONTRACTOR SHALL SUBMIT CONSTRUCTION DEWATERING PLAN TO THE GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO BEGINNING EXCAVATION.
9. THE CONTRACTOR SHALL INSTALL ALL NECESSARY DEWATERING SYSTEMS.

1. RETAINING WALLS SHALL BE DESIGNED FOR THE FOLLOWING:
a. LATERAL EARTH PRESSURE AT REST: 55 PSF
b. ACTIVE EARTH PRESSURE: 37 PSF
c. PASSIVE EARTH PRESSURE RESISTANCE: 330 PSF
d. SOIL FRICTION FACTOR: 0.35
2. RETAINING WALL DESIGN IS BASED ON DRAINAGE BACKFILL WITH NO BUILDUP OF WATER. THE CONTRACTOR SHALL PROVIDE A DRAINAGE SYSTEM IN ALL BACKFILL CONDITIONS (SEE CIVIL ARCHITECTURAL DRAWINGS FOR DRAINAGE SPECIFICATIONS).
3. DO NOT BACKFILL AGAINST CANTILEVERED RETAINING WALLS UNTIL CONCRETE COMPRESSIVE STRENGTH, F_c, REACHES ITS 28 DAY DESIGN STRENGTH. DO NOT BACKFILL PRIOR TO WATERPROOFING AND INSPECTION.

SLAB-ON-GROUND

1. THE SLAB-ON-GROUND HAS BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
2. SLAB THICKNESSES AND REINFORCEMENT ARE BASED ON A MODULUS OF SUBGRADE REACTION OF 150 PCF PER THE GEOTECHNICAL INVESTIGATION REPORT.
3. SUBGRADE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
4. FOR INTERIOR SLABS, PLACE A 15-MIL MINIMUM VAPOR RETARDER BETWEEN THE SOIL AND BOTTOM OF SLAB. SEE CAST-IN-PLACE CONCRETE SPECIFICATIONS FOR APPROVED VAPOR RETARDER PRODUCTS/MANUFACTURERS. DO NOT USE VAPOR RETARDERS ON EXTERIOR SLABS. SEE ARCHITECTURAL CONTRACT DOCUMENTS FOR PROJECT SPECIFIC REQUIREMENTS.
5. IF THE SLAB-ON-GROUND HAS BEEN DESIGNATED AS A STRUCTURAL SLAB-ON-GROUND IN THE CONTRACT DOCUMENTS, NO SAW CUTTING OF THE SLAB IS PERMITTED.
6. CONTROL JOINTS SHALL BE CUT INTO THE SURFACE OF THE SLAB, IN EACH DIRECTION. SEE THE TYPICAL SAW CUT JOINT DETAIL FOR TIE, DEPTH, AND SPACING OF JOINT REQUIREMENTS UNLESS NOTED OTHERWISE. CONTROL JOINTS SHALL BE CONSTRUCTED SUCH THAT THE AREA CONTAINED BY THE CONTROL JOINTS HAS A MAXIMUM RATIO OF LONG SIDE TO SHORT SIDE OF 1.5 TO 1 UNLESS NOTED OTHERWISE. DO NOT CONSTRUCT CONTROL JOINTS SUCH THAT L-SHAPED SLAB PANELS ARE CREATED.
7. COLUMN SOLUTION JOINTS SHALL BE CONSTRUCTED PER THE TYPICAL COLUMN ISOLATION JOINT DETAIL IN ORDER TO PROVIDE ADEQUATE SPACE FOR COLUMN INSTALLATION.
8. CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. SLAB CONSTRUCTION JOINTS SHALL BE DOWNEDRAUGHTS.
9. WHERE SPECIFIED ON PLAN, WELDED WIRE REINFORCEMENT SHALL BE INSTALLED. WELDED WIRE REINFORCEMENT SHALL BE PROPERLY CHAIRED SUCH THAT IT IS LOCATED AT A DEPTH OF 1 1/2" FROM THE TOP OF SLAB.
10. REFERENCE ARCHITECTURAL AND MEP DOCUMENTS FOR SLAB FINISHES AND SLOPES NOT REFERENCED ON THE STRUCTURAL DOCUMENTS. THE MINIMUM SLAB THICKNESS SPECIFIED IN THE CONTRACT DOCUMENTS MUST BE MAINTAINED.
11. REFERENCE ARCHITECTURAL DOCUMENTS FOR VAPOR RETARDER AND SLAB AND CONTROL JOINT SEALANT REQUIREMENTS.
12. CONDUITS SHALL NOT BE PLACED WITHIN THE SLAB. CONDUITS SHALL BE PLACED BENEATH THE SLAB.

CONCRETE REINFORCEMENT

1. ALL CONCRETE REINFORCEMENT SHALL BE INSTALLED IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
2. ALL REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
3. WHERE WELDED ARE INDICATED FOR REINFORCING STEEL ON THE DRAWINGS, REINFORCING STEEL SHALL BE A706, GRADE 60 UNLESS OTHERWISE NOTED.
4. WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE MATERIAL REQUIREMENTS OF ASTM A1064.
5. ALL 90°, 135°, AND 180° HOOKED REINFORCEMENT SPECIFIED AND GRAPHICALLY DEPICTED IN THE CONTRACT DOCUMENTS SHALL BE DETAILED IN ACCORDANCE WITH ACI 318 STANDARD HOOK GEOMETRY FOR DEFORMED BARS IN TENSION AND FOR STIRRUPS, TIES, AND HOOPS.
6. FOR EVERY VERTICAL OR HORIZONTAL BAR DISCONTINUED BY AN OPENING, ONE BAR (MINIMUM OF 2 BARS) SHALL BE ADDED AT SIDE OF OPENING (HALF TO EACH SIDE, TYPICAL).
7. FOR CONCRETE CLEAR COVER TO REINFORCEMENT SEE SCHEDULE ON SHEET S-003 UNLESS NOTED OTHERWISE. CLEAR COVER IN PARENTHESES () DENOTES CLEAR COVER WHEN THE AS-BUILT APPLICATION IS EXPOSED TO WEATHER.
8. ALL LAP SPICES SHALL BE CLASS B TENSION LAP SPICES IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE. SEE LAP SPICE SCHEDULE ON SHEET S-003 FOR LAP SPICE LENGTHS. UNLESS NOTED AS CONTINUOUS, REINFORCEMENT SHALL ONLY BE SPICED AT LOCATIONS SHOWN ON THE CONTRACT DOCUMENTS. SPICES AT NON-SPECIFIED LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
9. A MINIMUM LAP SPICE OF 8" SHALL BE PROVIDED AT ALL END AND SIDE LAP CONDITIONS FOR WELDED WIRE REINFORCEMENT UNLESS NOTED OTHERWISE.
10. MECHANICAL SPICES ARE REQUIRED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. MECHANICAL SPICES ARE ALSO REQUIRED TO SPICE ALL REINFORCING STEEL AND HIR BARS. LONGER CHAIR PLUS THE SHORT TERM DEFLECTION OF WOOD STRUCTURAL MEMBERS SHALL BE CALCULATED IN ACCORDANCE WITH THE AWS NDS. IT IS PERMITTED TO ESTIMATE THE CREEP-COMPONENT OF THE LONG-TERM DEFLECTION AS THE IMMEDIATE MECHANICAL SPICES SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
11. THE USE OF WELDED SPICES IS PROHIBITED UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL SUBMIT THE LOCATION OF WELDED SPICES TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. IF APPROVED, WELDED SPICES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.
12. DOWELS SHALL MATCH SIZE AND SPACING OF PRIMARY REINFORCEMENT UNLESS NOTED OTHERWISE.
13. THE TYPICAL DETAIL LAYOUT SHALL BE USED FOR ALL REINFORCEMENT DETAILS AND CONNECTIONS.
14. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCEMENT BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
15. CONCRETE
1. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
2. FOR CONCRETE MIXTURE REQUIREMENTS SEE SCHEDULE ON SHEET S-003.
3. THE USE OF RECYCLED CONCRETE IS PROHIBITED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
4. NORMAL WEIGHT CONCRETE SHALL BE USED FOR ALL CONCRETE MEMBERS UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE SHALL HAVE A CURED DENSITY OF 145 PCF ±5 PCF. WHERE LIGHT WEIGHT CONCRETE IS SPECIFIED, THE CURED DENSITY SHALL BE 112 PCF ±5 PCF.
5. EACH MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER AND THE INTENDED LOCATION OF PLACEMENT ON THE SPECIFIC PROJECT SHALL BE CLEARLY STATED.
6. ALL PROPOSED CONSTRUCTION METHODS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN BEAMS, WALLS, AND SLABS UNLESS SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS OR BY WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD. FOR MILD REINFORCED MEMBERS, CONSTRUCTION JOINTS SHALL OCCUR WITHIN THE MIDDLE THIRD OF A MEMBER'S SPAN. ALL APPROVED CONSTRUCTION JOINTS SHALL BE INDICATED, DIMENSIONED, AND DETAILED ON THE CONCRETE REINFORCEMENT SHOP DRAWINGS.
7. GRIDERS, BEAMS, HAUNCHES, DROP PANELS, DROP CAPS, AND CAPITALS SHALL BE POURED MONOLITHICALLY AS PART OF THE SLAB SYSTEM UNLESS NOTED OTHERWISE.
8. PROVIDE A 1/4" INCH CHAMFER AT EXPOSED CORNERS OF BEAMS, WALLS, ETC. UNLESS NOTED OTHERWISE.
9. CONCRETE CORING AND INSTALLATION OF DRILLED ANCHORS IS NOT PERMITTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
10. REFER TO THE CONTRACT DOCUMENTS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL COORDINATE BETWEEN THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS TO FURNISH DIMENSIONED DRAWINGS THAT LOCATE AND SIZE ALL SLAB EDGES, OPENINGS, AND PENETRATIONS. THESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
11. EMBEDDED CONDUITS, PIPES, AND SLEEVES
1. THE OUTSIDE DIAMETER OF CONDUITS, PIPES, AND SLEEVES SHALL NOT EXCEED ONE-THIRD THE THICKNESS OF THE SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED. EMBEDMENTS SHALL NOT SIGNIFICANTLY REDUCE THE CAPACITY OF THE MEMBERS THEY PENETRATE.
2. THE MINIMUM CLEAR COVER FOR CONDUITS, PIPES, AND SLEEVES SHALL BE 1 1/2" FOR CONCRETE EXPOSED TO EARTH OR WEATHER AND 1/2" FOR CONCRETE NOT EXPOSED TO EARTH OR WEATHER.
3. ALUMINUM EMBEDMENTS AND EMBEDMENTS MADE OF ANY OTHER MATERIAL HARMFUL TO THE CONCRETE OR REINFORCEMENT ARE PROHIBITED.
4. EMBEDMENTS NOT SHOWN ON THE CONTRACT DOCUMENTS SHALL BE DESIGNED TO RESIST THE EFFECTS OF MATERIAL, PRESSURE, AND TEMPERATURE THAT THEY WILL BE SUBJECTED TO. THE WORK SHALL BE COORDINATED AMONGST ALL CONSTRUCTION TRADES.
5. THE CONTENTS OF EMBEDDED PIPES SHALL NOT FLOW UNTIL THE CONCRETE HAS REACHED ITS SPECIFIED DESIGN STRENGTH.
6. CONDUITS, PIPES, AND SLEEVES SHALL BE PLACED BETWEEN TOP AND BOTTOM LAYERS OF REINFORCEMENT IN SLABS AND BETWEEN INNER AND OUTER LAYERS OF REINFORCEMENT IN WALLS.
7. EMBEDDED ITEMS SHALL BE FABRICATED AND INSTALLED SUCH THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS SPECIFIED LOCATION IS NOT REQUIRED.

STRUCTURAL DRAWING LIST	
SHEET NUMBER	SHEET NAME
C-S001	GENERAL NOTES
C-S002	GENERAL NOTES
C-S003	GENERAL SCHEDULES
C-S004	SPECIAL INSPECTION PLAN
C-S101	FOUNDATION PLAN
C-S102	ROOF FRAMING PLAN
C-S201	FOUNDATION SECTIONS & DETAILS
C-S202	FOUNDATION SECTIONS & DETAILS
C-S203	FOUNDATION SECTIONS & DETAILS
C-S301	FRAMING SECTIONS & DETAILS
C-S302	FRAMING SECTIONS & DETAILS
C-S303	FRAMING SECTIONS & DETAILS

ABBREVIATIONS

ADDL	ADJACENT	KSJ	KIPS PER SQUARE INCH
ADJ	ADJACENT	NS	LENGTH
AF	ABOVE FINISHED FLOOR	L	LENGTH
ALT	ALTERNATE	LL(S)	LONG SIDE
APPROX	APPROXIMATE	LL	LIVE LOAD
ARCH	ARCHITECT OR ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
ASD	ALLOWABLE STRESS DESIGN	LLV	LONG LEG VERTICAL
B	BOTTOM OF	LONG	LONGITUDINAL
B/B	BACK-TO-BACK	LFRD	LOAD RESISTANCE FACTORED DESIGN
BLDG	BUILDING	LSH	LONG SIDE HORIZONTAL
BLKG	BLOCKING	LSV	LONG SIDE VERTICAL
BP	BASE PLATE	LTS	LAP TENSION SPICE
BRG	BEARING	LWC	LIGHT WEIGHT CONCRETE
BTM	BOTTOM		
BWN	BETWEEN	M	MOMENT
C	COMPRESSION	MAX	MAXIMUM
CONST	CONSTRUCTION	MC	MOMENT CONNECTION(S)
CIP	CAST-IN-PLACE	MECH	MECHANICAL
CONJ	CONTROL JOINT	MEP	MECHANICAL, ELECTRICAL, PLUMBING
CJP	COMPLETE JOINT PENETRATION	MFR	MANUFACTURER
CLR	CLEAR LINE	MID	MIDDLE
CMU	CONCRETE MASONRY UNIT	MIN	MINIMUM
COL	COLUMN	MISC	MISCELLANEOUS
CONC	CONCRETE	NTC	NOT IN CONTRACT
CONNS	CONNECTION(S)	NIS	NOT TO SCALE
CONSTR	CONSTRUCTION	NWC	NORMAL WEIGHT CONCRETE
COORD	COORDINATE		

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TILT-UP CONCRETE PANELS

- A. DESIGN, CONSTRUCTION, AND ERECTION OF TILT-UP CONCRETE PANELS AND THEIR CONNECTIONS TO ONE ANOTHER SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS, ACI 318 AND ACI 308.
- B. CONTRACTOR SHALL RETAIN A DELEGATED ENGINEER FOR THE DESIGN OF ALL TILT-UP CONCRETE PANELS, COLUMNS, AND ROOFS AS DETAILED IN THE CONTRACT DOCUMENTS.
- C. DELEGATED ENGINEER SHALL REFER TO ARCHITECTURAL DRAWINGS FOR PANEL SIZES AND LAYOUT. PROVIDE WALL JOINTS TO ALLOW FOR THE PROPER EXPANSION AND CONTRACTION OF WALLS. ALL PANEL EDGES SHALL BE CHAMFERED, UNLESS NOTED OTHERWISE.
- D. TILT-UP CONCRETE PANELS SHALL BE DESIGNED TO RESIST APPLICABLE GRAVITY AND WIND LOADS. TILT-UP CONCRETE PANELS SHALL BE DESIGNED WITH ADEQUATE CAPACITY TO SPAN BETWEEN BRACING LOCATIONS. BRACING FOR TILT-UP CONCRETE PANELS SHALL NOT BE REMOVED UNTIL ALL PERMANENT CONNECTIONS ARE COMPLETED AND HAVE BEEN INSPECTED BY THE TESTING AGENCY.
- E. DELEGATED ENGINEER SHALL PROVIDE ALL ENGINEERING AND DETAILING OF THE TILT-UP CONCRETE PANELS, PANEL-TO-PANEL CONNECTIONS, PANEL-TO-STRUCTURAL STEEL COLUMN CONNECTIONS, AND PANEL-TO-FOUNDATION CONNECTIONS TO RESIST THE LOADS SPECIFIED IN THE GENERAL NOTES, WIND LOADING DIAGRAMS, FRAMING PLANS, AND DETAILS.
- G. DELEGATED ENGINEER TO DESIGN ALL HARDWARE FOR LIFTING AND BRACING AS REQUIRED. ADD ADDITIONAL REINFORCING AS REQUIRED BY DESIGN. DELEGATED ENGINEER SHALL PROPERLY CONSIDER STRESSES INDUCED DURING THE LIFTING PROCESS AND SHALL PROVIDE ANY ADDITIONAL REINFORCING REQUIRED FOR LIFTING AND ERECTION STRESSES.
- H. TILT-UP CONCRETE PANELS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT THE TIME OF ERECTION.
- I. DELEGATED ENGINEER SHALL PROVIDE COMPLETE FABRICATION AND ERECTION DRAWINGS. DRAWINGS AND CALCULATIONS SHALL BE PROVIDED TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD FOR REVIEW. DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY DELEGATED ENGINEER PRIOR TO REVIEW BY STRUCTURAL ENGINEER OF RECORD.
- J. CONTRACTOR SHALL REMOVE AND REPAIR ALL INSERTS AFTER BRACING HAS BEEN REMOVED. REPAIR SHALL MEET ALL ARCHITECTURAL REQUIREMENTS FOR FINISHES.

POST-INSTALLED ANCHORS

- A. POST-INSTALLED ANCHORS SHALL INCLUDE MECHANICAL, SCREW, AND ADHESIVE ANCHORS OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE STRUCTURAL DRAWINGS.
- B. MECHANICAL ANCHORS (EXPANSION ANCHORS/EXPANSION BOLTS) INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:
 1. KWIK BOLT TZ ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. STRONG-BOLT Z ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
- C. MECHANICAL ANCHORS (EXPANSION ANCHORS/EXPANSION BOLTS) INTO EXISTING CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:
 1. KWIK BOLT 3 ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. WEDGE-ALL ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
- D. POWER-STUD- SD1 ANCHORS MANUFACTURED BY DEWALT
- E. SCREW ANCHORS INTO EXISTING CONCRETE AND CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:
 1. KWIK HUS EZ ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. TITEN 40 ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
 3. SCREW-BOLT+ ANCHORS MANUFACTURED BY DEWALT
- F. ADHESIVE ANCHORS (EPOXY ANCHORS/DRILL & EPOXY) INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:
 1. HIT-HY200 EPOXY ADHESIVE WITH HAS ROD MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. PURE10+ EPOXY ADHESIVE MANUFACTURED BY DEWALT WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD
- G. ADHESIVE ANCHORS (EPOXY ANCHORS/DRILL & EPOXY) INTO EXISTING CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:
 1. HIT-HY200 EPOXY ADHESIVE WITH HAS ROD MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. AT-AP ADHESIVE MANUFACTURED BY SIMPSON STRONGTIE COMPANY WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD
 3. ACT10+ GOLD MANUFACTURED BY DEWALT WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD
- H. ADHESIVE FOR ANCHORING REINFORCING BARS (REBAR) INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:
 1. HIT-HY200 EPOXY ADHESIVE MANUFACTURED BY HILTI FASTENING SYSTEMS
 2. PURE10+ EPOXY ADHESIVE MANUFACTURED BY DEWALT
- I. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
- J. SUBSTITUTION REQUESTS FOR ALTERNATIVE PRODUCTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER. CALCULATIONS SHALL SHOW THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE REFERENCED BUILDING CODE.
- K. ALTERNATIVE PRODUCTS SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL SHALL HAVE A VALID RESEARCH REPORT, ALSO KNOWN AS EVALUATION REPORT, INDICATING COMPLIANCE WITH APPROPRIATE ACCEPTANCE CRITERIA REQUIRED BY THE REFERENCED BUILDING CODE FOR THE INTENDED LOAD TYPE AND USE (E.G. WIND, SEISMIC, SUSTAINED TENSION, ETC.). ALTERNATIVE PRODUCTS SUBMITTED SHALL INDICATE THAT THE ANCHOR IS PERMITTED FOR RESISTING LOADS IN CRACKED CONCRETE. RESEARCH REPORTS SHALL BE ISSUED BY A SOURCE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- L. LOCATE, BY NON-DESTRUCTIVE MEANS, ALL EXISTING REINFORCEMENT, AND AVOID DURING INSTALLATION OF ANCHORS. IF EXISTING REINFORCEMENT LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD IMMEDIATELY.
- M. HOLES SHALL BE DRILLED AND CLEANED, AND ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN INJECTABLE ADHESIVE MATCHING THE ADJACENT CONCRETE COMPRESSIVE STRENGTH.
- N. HOT DIPPED GALVANIZED STEEL ANCHORS SHALL BE USED AT ALL EXTERIOR LOCATIONS AND WHERE SPECIFICALLY INDICATED ON THE DRAWINGS.
- O. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HOLLOW CORE MASONRY. IF INSTALLATION INTO HOLLOW CORE MASONRY IS DESIRED, SUBMIT ALTERNATIVE PRODUCT FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD.
- P. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HEAD JOINTS.
- Q. IN ADDITION TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, THE FOLLOWING GUIDELINES SHALL BE FOLLOWED FOR INSTALLATION OF ADHESIVE ANCHORS:
 1. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT THE TIME OF INSTALLATION UNLESS HIGHER STRENGTH IS REQUIRED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
 2. ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, AND DURING DRY CONDITIONS.
 3. ADHESIVE ANCHORS SHALL BE INSTALLED IN HOLES PREDRILLED WITH A CARBIDE TIPPED DRILL BIT.
 4. ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE RANGE SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, BUT NOT OUTSIDE OF THE DESIGN TEMPERATURE RANGE. LOADS SHALL NOT BE APPLIED TO ADHESIVE ANCHORS UNTIL THE FULL CURING TIME ASSOCIATED WITH THE INSTALLATION TEMPERATURE HAS ELAPSED.
- R. INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT.
- S. SPECIAL INSPECTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE ANCHOR MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND/OR EVALUATION REPORTS, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.
- T. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO RESIST SUSTAINED TENSILE LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.
- U. CONTINUOUS INSPECTION SHALL BE PROVIDED FOR ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSILE LOADS.
- V. HOLE DRILLING AND INSTALLATION OF ADHESIVE ANCHORS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN CONCRETE DRY CONDITION.

STRUCTURAL STEEL

- A. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS:
 1. ROLLED SHAPES AND CHANNELS: ASTM A572 OR A992, MIN. YIELD STRENGTH 50 KSI
 2. ANGLES FOR TRUSSES AND BRACES: ASTM A36
 3. MISCELLANEOUS ANGLES: ASTM A36
 4. HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE C, MIN. YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS
- B. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:
 1. ANGLES: ASTM A36
 2. WTS: ASTM A992
 3. PLATES: ASTM A572, MIN. YIELD STRENGTH 50 KSI
 4. BOLTS: ASTM A325
 5. NUTS: ASTM A663
 6. WASHERS: ASTM F436
 7. ANCHOR RODS: ASTM F1554 GRADE 36 WITH WELDBILITY SUPPLEMENT S1
 8. WELD ELECTRODES: MATCH FILLET METAL TO BASE METAL, PER AWS D1
- C. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION. CANTILEVERED BEAMS WITH NATURAL MILL CAMBER SHALL BE ERECTED SUCH THAT THE CAMBER IS ORIENTED DOWNWARD (OR CONCAVE UP).
- E. SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE SER IN WRITING.
- F. FOR STEEL MEMBERS AND EMBEDEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH.
- G. PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1 1/8" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.
- H. SHOW ALL COPIES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.
- I. FIELD MODIFICATIONS OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.
- J. WHERE BEAM SHEAR IS NOT NOTED, DESIGN FOR 10K.
- K. ALL CONNECTIONS SHALL BE DESIGNED FOR THE SPECIFIED SHEAR, MOMENT, AND AXIAL LOADS ON THE DRAWINGS. THE CONNECTIONS SHALL BE DESIGNED FOR LOAD REVERSAL. ALL CONNECTIONS FORCES SPECIFIED ON PLAN ARE ULTIMATE LEVEL FORCES UNLESS NOTED OTHERWISE.
- L. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, AND FABRICATION OF ALL STEEL FRAMING CONNECTIONS UNLESS SPECIFICALLY NOTED AS COMPLETELY DESIGNED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL DESIGN THE CONNECTIONS, SUBMIT SIGN AND SEALED CALCULATIONS TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO STARTING FABRICATION.

CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATIONS AND THE BUILDING CODE. CONNECTIONS SHALL BE CAPABLE OF RESISTING VERTICAL AND HORIZONTAL LOADS LISTED ON THE DRAWINGS. CONNECTION DESIGN SHALL PROVIDE AN ADEQUATE LOAD PATH TO TRANSFER THE LOADS FROM EACH MEMBER THROUGH THE CONNECTION INTO THE SUPPORTING MEMBER, AND SHALL CONSIDER THE EFFECTS OF THE FORCES ON EACH MEMBER. PROVIDE STIFFENER PLATES, WEB DOUBLER PLATES, FLANGE CONTINUITY PLATES, ETC., AS REQUIRED. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS.

STEEL CONNECTION DETAILS SHOW GENERAL CRITERIA FOR DESIGN AND DETAILING, AND ARE NOT INTENDED TO SHOW COMPLETE CONNECTION CONFIGURATIONS OR OTHER SPECIFIC INFORMATION THAT ARE THE RESPONSIBILITY OF THE CONNECTION DESIGN ENGINEER. ALTERNATIVE CONNECTION CONFIGURATION MAY BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL. CONNECTIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS ARE TO BE FABRICATED AS SHOWN.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 3/4 INCH DIAMETER A325 BOLTS IN SINGLE SHEAR UNLESS NOTED OTHERWISE. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE KRCS. BOLTS IN CONNECTIONS SHALL BE INSTALLED WITH FULL PRETENSION EXCEPT WHERE "SNUG-TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON THE DRAWINGS, WHERE CONNECTIONS ARE NOTED AS NOT TO BE TIGHTENED. THE CONTRACTOR MAY NOT ALTER THE CRITERIA FOR SNUG-TIGHT BOLTS. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE INSTALLED USING TURN-OF-NUT PRETENSIONING, TWIST-OFF TYPE TENSION CONTROL, BOLT PRETENSIONING, OR DIRECT-TENSION INDICATOR (DTI) PRETENSIONING. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.

WELDING

- A. ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION PER AMERICAN WELDING SOCIETY D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WELDING PROCEDURE SPECIFICATION SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.
- B. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI).
- C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPIES, SURFACE ROUGHNESS VALUES, TAPERS, AND TRANSITIONS OF UNEQUAL PARTS.
- D. WELDING SHALL BE DONE BY WELDERS WITH CURRENT AMERICAN WELDING SOCIETY CERTIFICATION.
- E. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMERICAN WELDING SOCIETY D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.
- F. ALL WELD SIZES SHALL BE THE LARGER OF: THE SIZE REQUIRED BY THE CONNECTION FORCES, MINIMUM SIZE PER AWS/D1.1 OR 3/16 INCH MINIMUM FILLET WELD, UNLESS NOTED OTHERWISE.
- G. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE FORCE OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- H. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED BY THE TESTING FIRM.
- I. GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- J. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATES LESS THAN OR EQUAL TO 1/4-INCH THICK SHALL BE MAGNETIC PARTICLE TESTED.
- K. REDUCTION IN JOINTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.
- L. RUN-OUT TAB SHALL BE USED AT ALL BEVEL AND FULL PENETRATION WELDS. RUN-OUT TABS SHALL BE REMOVED BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.
- M. WHERE REQUIRED BY DETAIL, REMOVE WELD BACKING BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.
- N. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- O. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARTER V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
 1. ASTM A660M HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2 INCHES AND FULL-UP HEAVY SHAPES WITH PLATES EXCEEDING 2 INCHES IN THICKNESS: 20 FT-LB AT 70° FAHRENHEIT.
 2. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDETS, BRACES, ETC.): 20 FT-LB AT 70° FAHRENHEIT.
 3. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50° FAHRENHEIT: 20 FT-LB AT SERVICE TEMPERATURE + 20° FAHRENHEIT; 40° FAHRENHEIT MAXIMUM.
 4. WELD METAL: 20 FT-LB AT 20° FAHRENHEIT AND 40 FT-LB AT 70° FAHRENHEIT.
 5. TESTING IS TO BE IN ACCORDANCE WITH ASTM A660M, SUPPLEMENTARY REQUIREMENT S30, CHARTER V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES - ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A673 FOR PLATES, AT ANY PERMITTED LOCATIONS. WELD METAL SHALL BE TESTED IN ACCORDANCE WITH ASTM E23. STANDARD METHODS FOR NOTCHED BAR IMPACT TESTING OF METALLIC MATERIALS FOR WELD METAL.

STEEL JOISTS

- A. MANUFACTURE AND ERECT ALL STRUCTURAL STEEL JOISTS, JOIST GIRDETS, AND BRIDGING IN ACCORDANCE WITH SPECIFICATIONS SECTION 052100, SPECIFICATIONS OF THE STEEL JOIST INSTITUTE AND ALL OSHA REQUIREMENTS.
- B. JOIST MANUFACTURER SHALL DESIGN JOISTS PER LISTED DESIGN CRITERIA AND ANY ADDITIONAL LOADING SHOWN ON PLAN OR IN DETAILS. AT A MINIMUM, JOIST SHALL BE DESIGNED PER STEEL JOIST INSTITUTE (SJI) LOAD TABLES.
- C. JOISTS SHALL BE EQUALLY SPACED BETWEEN COLUMN LINES OR OTHER SPECIFICALLY LOCATED FRAMING MEMBERS UNLESS NOTED OTHERWISE.
- D. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR NET UPLIFT (NEGATIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO THE APPLICABLE BUILDING CODE LISTED IN DESIGN CRITERIA FOR LOAD COMBINATIONS.
 1. DEAD LOAD (MIN) = 10 PSF
- E. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR ADDITIONAL DOWN FORCE RESULTING FROM WIND (POSITIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS.
 1. DEAD LOAD MAXIMUM = 30 PSF
- F. JOIST MANUFACTURER SHALL DESIGN AND DETAIL ALL BRIDGING PER SJI REQUIREMENTS AND CLEARLY INDICATE LOCATION ON STEEL JOIST ERECTION DRAWINGS.
 1. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR JOISTS NOT BRACED BY STEEL ROOF DECK.
 2. BOTTOM CHORD OF ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT (COMPONENTS & CLADDING) SHOWN ON DIAGRAM ON S-03, UNLESS NOTED OTHERWISE.
- G. AN ALLOWABLE STRESS INCREASE FOR LOAD COMBINATIONS INCLUDING WIND IS NOT PERMITTED.
- H. JOIST MANUFACTURER SHALL CAMBER JOISTS PER SJI CRITERIA.
- I. PROVIDE MINIMUM BEARING PER SJI REQUIREMENTS AND CONNECT TO STEEL SUPPORT AS FOLLOWS:
 1. K-SERIES: 3/16" x 1" FILLET WELD EACH SIDE
 2. LH AND DLH SERIES: 1/2" x 2" FILLET WELD EACH SIDE
- J. PROVIDE STANDARD DEPTH OF BEARING FOR ALL JOISTS AS SHOWN BELOW UNLESS NOTED IN DRAWINGS:
 1. K-SERIES = 2 1/2"
 2. LH = 5"
 3. JOIST GIRDER = 7 1/4"
 4. WHERE STEEL JOIST OR JOIST GIRDER SLOPE EXCEEDS 1/4 INCH PER FOOT, PROVIDE SLOPED BEARING AS NOTED IN SLOPED SEAT REQUIREMENTS OF SJI.
- K. JOIST MANUFACTURER SHALL DESIGN AND DETAIL FIELD BOLTING FOR ERECTION PER SJI REQUIREMENTS.

STEEL DECK GENERAL REQUIREMENTS

- A. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE DIVISION 05 SPECIFICATIONS.
- B. STEEL DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION), UNLESS NOTED OTHERWISE.
- C. THE CONTRACTOR SHALL COORDINATE SLAB/DECK OPENING SIZES AND LOCATIONS PER ARCHITECTURAL AND MEP CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE OPENING SUPPORT FRAMING AND/OR REINFORCEMENT AS REQUIRED PER TYPICAL DETAILS AND SUBMIT PROPOSED SLAB/DECK OPENINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD.
- D. SHOP DRAWINGS SHALL BE SUBMITTED INDICATING:
 1. MATERIAL STRENGTH
 2. SECTION PROPERTIES
 3. DECK GAGE LAYOUT
 4. FASTENER TYPE
 5. CONNECTION PATTERN
 6. CLOSURE ANGLES
- E. THE CAPACITY OF THE DECK SHALL BE BASED ON CURRENT ICC-ES EVALUATION REPORTS.

COMPOSITE STEEL FLOOR DECK

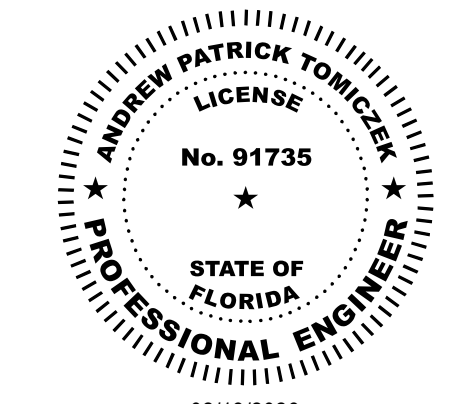
- A. STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 3/4 INCH DIAMETER A325 BOLTS IN SINGLE SHEAR UNLESS NOTED OTHERWISE. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE KRCS. BOLTS IN CONNECTIONS SHALL BE INSTALLED WITH FULL PRETENSION EXCEPT WHERE "SNUG-TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON THE DRAWINGS, WHERE CONNECTIONS ARE NOTED AS NOT TO BE TIGHTENED. THE CONTRACTOR MAY NOT ALTER THE CRITERIA FOR SNUG-TIGHT BOLTS. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE INSTALLED USING TURN-OF-NUT PRETENSIONING, TWIST-OFF TYPE TENSION CONTROL, BOLT PRETENSIONING, OR DIRECT-TENSION INDICATOR (DTI) PRETENSIONING. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.
- B. WHERE COMPOSITE BEAM DESIGNATIONS OCCUR ON FRAMING PLANS THROUGH THE CALLING OUT OF A NUMBER OF REQUIRED STEEL HEADED STUD ANCHORS, FLOOR AND BEAM ARE DESIGNED TO ACT COMPOSITELY BY USE OF SHEAR STUDS ON THE BEAM TOP FLANGE.
 1. SEE DISTRIBUTED STEEL HEADED STUD ANCHOR PLACEMENT DETAIL FOR PLACEMENT REQUIREMENTS.
 2. IF LONGITUDINAL STEEL HEADED STUD ANCHOR SPACING EXCEEDS 12 INCHES, PLUG WELD DECK TO BEAM BETWEEN STUDS.
- C. COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:
 1. AT BUTTED ENDS: AT 12 INCHES ON CENTER
 2. AT PERIMETER EDGES OF BUILDING: AT 12 INCHES ON CENTER
 3. INTERMEDIATE SUPPORTS: AT 12 INCHES ON CENTER
 4. SIDE LAPS: AT MID-SPAN OR AT 3'-0" ON CENTER, WHICHEVER IS SMALLER. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUTTON PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.
- D. CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL.
- E. DESIGN AND PROVIDE FLASHING AND CLOSURE PLATES AT WALL ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. COORDINATE ALL CLOSURES WITH ELEVATOR, STAIR, ESCALATOR, AND OTHER ARCHITECTURAL DETAILS.
- F. SHEAR CONNECTORS (SHEAR STUDS) SHALL BE TYPE B AND CONFORM TO AWS D1.1, ASTM A108, AND ASTM A29 SPECIFICATIONS FOR 1010 THROUGH 1020 MILD STEELS. MINIMUM YIELD STRENGTH = 51 KSI AND MINIMUM ULTIMATE TENSILE STRENGTH = 65 KSI.
- G. ALL STEEL HEADED STUD ANCHORS SHALL BE 3/4 INCH DIAMETER + (FLUTE HEIGHT + 1 INCH) AFTER WELDING, UNLESS NOTED OTHERWISE. MINIMUM CLEARANCE FROM TOP OF STUD TO TOP OF SLAB IS 1 INCH.
- H. DISTRIBUTE STEEL HEADED STUD ANCHORS UNIFORMLY OVER BEAM SPAN UNLESS NOTED OTHERWISE ON PLAN. MAXIMUM SPACING OF 34 INCH HEADED STUD ANCHORS SHALL NOT EXCEED 12 INCHES ON CENTER (ONE STUD PER FOOT) UNLESS NOTED OTHERWISE ON PLAN.
- I. THE COMPOSITE STEEL FLOOR DECK INSTALLATION, WHEN COMPLETE, SHOULD BE READY TO RECEIVE CONCRETE. THE COMPOSITE STEEL FLOOR DECK HAS BEEN DESIGNED TO SUPPORT CONSTRUCTION LOADS UNSHORED, UNLESS NOTED OTHERWISE. IF ANY SHORING IS USED, IT SHALL BE DESIGNED AND SUPPLIED BY THE CONTRACTOR AND SHALL BE SHOWN ON THE SHOP DRAWINGS. SHORING SHALL BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO PLACING CONCRETE.
- J. COMPOSITE STEEL FLOOR DECKS ARE TO BE POURED LEVEL TO COLUMNS OR AS INDICATED ON THE DRAWINGS.
- K. MAINTAIN INDICATED MINIMUM SLAB THICKNESS OVER BEAMS THROUGHOUT SPAN. ELEVATED SLAB SHALL BE PLACED WITH MINIMUM THICKNESS THROUGHOUT WITH NO MORE THAN 1/2" OF ADDITIONAL THICKNESS IN ANY LOCATION TO HELP WITH THE LEVELING OF THE SLAB.
- L. COMPOSITE STEEL FLOOR DECK HANGER TAB LOADS SHALL NOT EXCEED 80 POUNDS PER HANGER TAB. IN ADDITION, LOADS ON HANGER TABS SHALL BE DISTRIBUTED IN SUCH A MANNER THAT THE TRIBUTARY LOADS FOR EACH HANGER TAB SHALL NOT EXCEED 5 POUNDS PER SQUARE FOOT. DO NOT HANG MEP EQUIPMENT AND PIPING DIRECTLY FROM STEEL DECK.



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ISSUE FOR
CONSTRUCTION DOCUMENTS

ISSUE DATE
2/16/2026

REVISIONS
NO. REASON DATE

PROJECT TEAM
PRINCIPAL IN CHARGE
ANDREW P. TOMICZEK
PROJECT MANAGER
ANDREW P. TOMICZEK
DESIGN TEAM
JEZERINAC GROUP

COLUMBIA STORM PROCESSING & SERVICE CENTER - BUILDING C - FLEET GARAGE
COUNTY ROAD 25A, LAKE CITY, FL

PROJECT NO.
814C632800
SHEET TITLE
GENERAL NOTES

SHEET NUMBER
C-S002



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TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

F

CONCRETE MIXTURE REQUIREMENTS								
APPLICATION	EXPOSURE CLASS	f _c	TEST AGE	MODULUS OF ELASTICITY	MAXIMUM W/C/M	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE	MAXIMUM CONCRETE WEIGHT
FOOTINGS	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
EXTERIOR SLAB-ON-GROUND	F1	4500 PSI	28 DAYS	3824 KSI	0.55	4.5% ± 1.5%	1"	150 PCF
SLAB-ON-GROUND	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
ELEVATED SLABS ON METAL DECK	F0	4000 PSI	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
TILT-WALLS	F0	4000 PSI (MIN)	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
NOTES: 1. EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE. 2. WATER/CEMENT RATIO SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX DESIGN. THERE IS NO MAXIMUM WATER/CEMENT RATIO REQUIREMENT FOR THE EXPOSURE CLASSIFICATION ASSOCIATED WITH THIS APPLICATION. MAXIMUM WATER/CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE. 3. THERE IS NO MANDATORY TARGET AIR CONTENT FOR THIS APPLICATION. THE CONTRACTOR MAY CHOOSE TO ADD AIR ENTRAINMENT TO IMPROVE THE WORKABILITY AND FINISHING PROPERTIES OF THE MIX. AIR CONTENT SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX.								

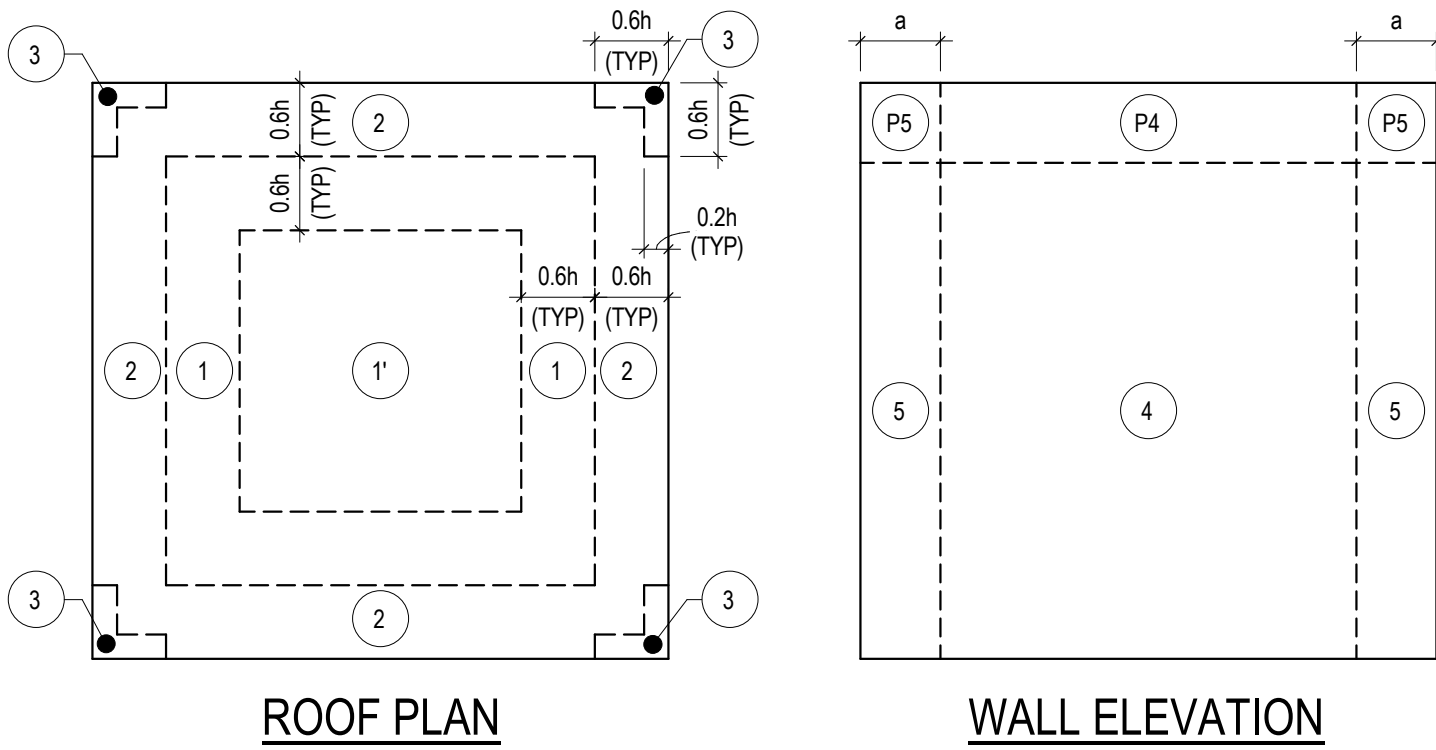
E

COMPONENTS & CLADDING EXTERNAL PRESSURE LOADS (PSF)										
EFFECTIVE WIND AREA (FT²)	LOCATION PER ASCE 7-16: FIGURE 30.3-1, 30.3-2A, 30.11-1B									
	1	1'	2	3	4	5	P4	P5	CANOPY	
<10	52 -205	52 -129	52 -270	52 -388	118 -127	118 -157	388	486	98 -88	
20	49 -192	49 -129	49 -254	49 -330	113 -123	113 -147	367	443	90 -88	
50	45 -172	45 -129	45 -229	45 -286	106 -116	106 -133	335	392	79 -75	
100	41 -159	41 -129	41 -213	41 -254	101 -111	101 -123	314	355	71 -65	
200	41 -147	41 -101	41 -194	41 -216	95 -112	95 -112	289	311		
500	41 -129	41 -80	41 -172	41 -172	88 -98	88 -98	260	260		
>1000		41 -63								

NOTES:

- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
- a = 3'-0": SEE ROOF PLAN MAP BELOW FOR LOCATION OF a-ZONES. WALL a-ZONE LOCATIONS TO MATCH ROOF a-ZONES.
- 0.6h = 18'-0": SEE ROOF PLAN MAP BELOW FOR LOCATION OF ROOF PRESSURE ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - P4/5 SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF 207 PSF AND VERTICAL VALUE OF 163 PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUs, AHUs, AND SCREEN WALLS.
- ROOF# : DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
- ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

D



C

B

A

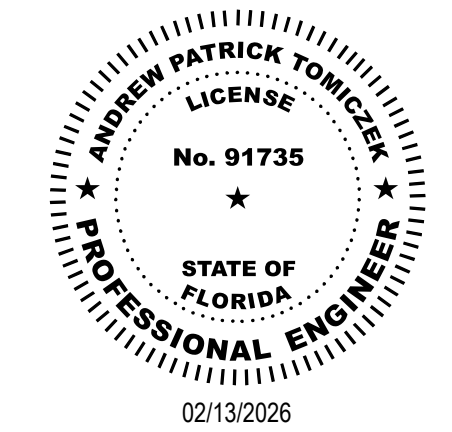
CAST-IN-PLACE CONCRETE (NON-PRESTRESSED) CLEAR COVER SCHEDULE			
APPLICATION	BOTTOM	TOP	SIDES
FOUNDATIONS	3"	2"	3"
SLAB-ON-GROUND	SEE DETAILS	SEE DETAILS	3"
RETAINING WALLS	N/A	N/A	2"
WALLS	N/A	N/A	¾" (2")
COLUMNS	N/A	N/A	1½" (2")
ELEVATED SLABS	¾" (2")	¾" (2")	¾" (2")
BEAMS	1½" (2")	1½" (2")	1½" (2")

DEVELOPMENT LENGTH SCHEDULE (INCHES)							
BAR SIZE	MIN BAR SPACING (INCHES) (MAX OF db + 1" OR 2db)	TENSION				COMPRESSION	
		NOTED AS Ld ON DRAWINGS, f _c (psi)	NOTED AS Ld ON DRAWINGS, f _c (psi)	NOTED AS Ld ON DRAWINGS, f _c (psi)	NOTED AS Ld ON DRAWINGS, f _c (psi)	NOTED AS Ld ON DRAWINGS, f _c (psi)	NOTED AS Ld ON DRAWINGS, f _c (psi)
#3	1,375	17	15	13	9	8	7
#4	1,500	22	19	17	11	10	9
#5	1,625	28	24	22	14	12	11
#6	1,750	33	29	26	17	15	13
#7	1,875	48	42	38	20	17	15
#8	2,000	55	48	43	22	19	17
#9	2,375	62	54	48	25	22	20
#10	2,625	70	61	54	28	25	22
#11	2,875	78	67	60	31	27	24

FOOTING LAP SPlice LENGTH SCHEDULE (INCHES)				
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)		
		f _c = 4,000 PSI	f _c = 5,000 PSI	f _c = 6,000 PSI
#4	1,500	33	25	29
#5	1,875	41	31	36
#6	2,250	49	37	44
#7	2,625	71	54	63
#8	3,000	81	62	72
#9	3,500	91	70	81
#10	3,875	102	79	92
#11	4,250	114	87	102

WALL REINFORCEMENT - HORIZONTAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4,000 PSI	f _c = 5,000 PSI	f _c = 6,000 PSI	
#4	1,500	33	30	27	15
#5	1,875	40	36	34	19
#6	2,250	48	44	40	23
#7	2,625	105	95	87	27
#8	3,000	121	108	99	30
#9	3,500	137	122	112	34
#10	3,875	153	138	125	39
#11	4,250	170	152	139	43

WALL REINFORCEMENT - VERTICAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4,000 PSI	f _c = 5,000 PSI	f _c = 6,000 PSI	
#4	2,000	25	23	21	15
#5	2,125	31	28	26	19
#6	2,250	37	34	31	23
#7	2,375	54	49	45	27
#8	2,500	62	56	51	30
#9	2,875	70	63	57	34
#10	3,250	118	106	96	39
#11	3,625	131	117	107	43



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NO. REASON DATE

PROJECT TEAM
PRINCIPAL IN CHARGE
ANDREW TOMICZEK
PROJECT MANAGER
ANDREW TOMICZEK
DESIGN TEAM
JEZERINAC GROUP

COLUMBIA STORM
PROCESSING & SERVICE
CENTER - BUILDING C -
FLEET GARAGE
COUNTY ROAD 25A, LAKE
CITY, FL

PROJECT NO.
814C632800

SHEET TITLE
GENERAL SCHEDULES

SHEET NUMBER
C-S003



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IG Project #: 22-01-004-01

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MATERIAL SPECIFICATIONS.



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FLEET GARAGE
COUNTY ROAD 25A, LAKE
CITY, FL

PROJECT NO.	814C632800
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SPECIAL INSPECTION
PLAN

SHEET NUMBER

C-S004

A. PRIOR TO STARTING WITH THEIR WORK, THE SPECIAL INSPECTOR, AND THEIR AUTHORIZED REPRESENTATIVE, IF APPLICABLE, SHALL ATTEND A PRE-CONSTRUCTION MEETING FOR THE PURPOSE OF BECOMING FAMILIAR WITH THE SPECIFIC STRUCTURAL COMPONENTS AND SYSTEM WHICH THEY WILL BE RESPONSIBLE FOR INSPECTING.

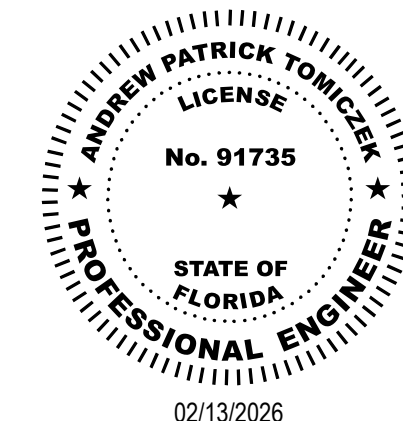
B. THE SPECIAL INSPECTOR SHALL INSPECT ALL STRUCTURAL SYSTEMS NOTED BELOW AND IN COMPLIANCE WITH THOSE LISTED UNDER THE "GENERAL" HEADING.

C. THE SPECIAL INSPECTOR SHALL VISIT THE SITE AT SUCH FREQUENCY TO SATISFY THEMSELVES THAT THEIR REPRESENTATIVE'S INSPECTIONS CONFORM WITH THE REQUIREMENTS OF THIS PLAN.

D. THE SPECIAL INSPECTOR SHALL DEDICATE THEIR ENTIRE TIME ON-SITE TO THE REQUIREMENTS OF THIS PLAN AND SHALL NOT PERFORM OTHER WORK, SUCH AS MATERIAL TESTING.

A. THE STRUCTURAL INSPECTOR SHALL:

1. BE FAMILIAR WITH PROOF TESTING REQUIREMENT AND PLAN; SEE GENERAL NOTES UNDER "POST-INSTALLED ANCHORS".
2. VERIFY THAT FEE OWNER'S TESTING AGENCY HAS TESTED ALL REQUIRED ANCHORS PER THE APPROVED POST-INSTALLED ANCHOR TEST PLAN.
3. VERIFY THAT ANY FAILED ANCHORS HAVE BEEN REPLACED.
4. VERIFY THAT THE TESTING AGENCY HAS SUBMITTED SIGNED AND SEALED TEST REPORTS TO THE STRUCTURAL ENGINEER OF RECORD.



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PROJECT MANAGER ANDREW TOMICZEK
DESIGN TEAM JEZERINAC GROUP

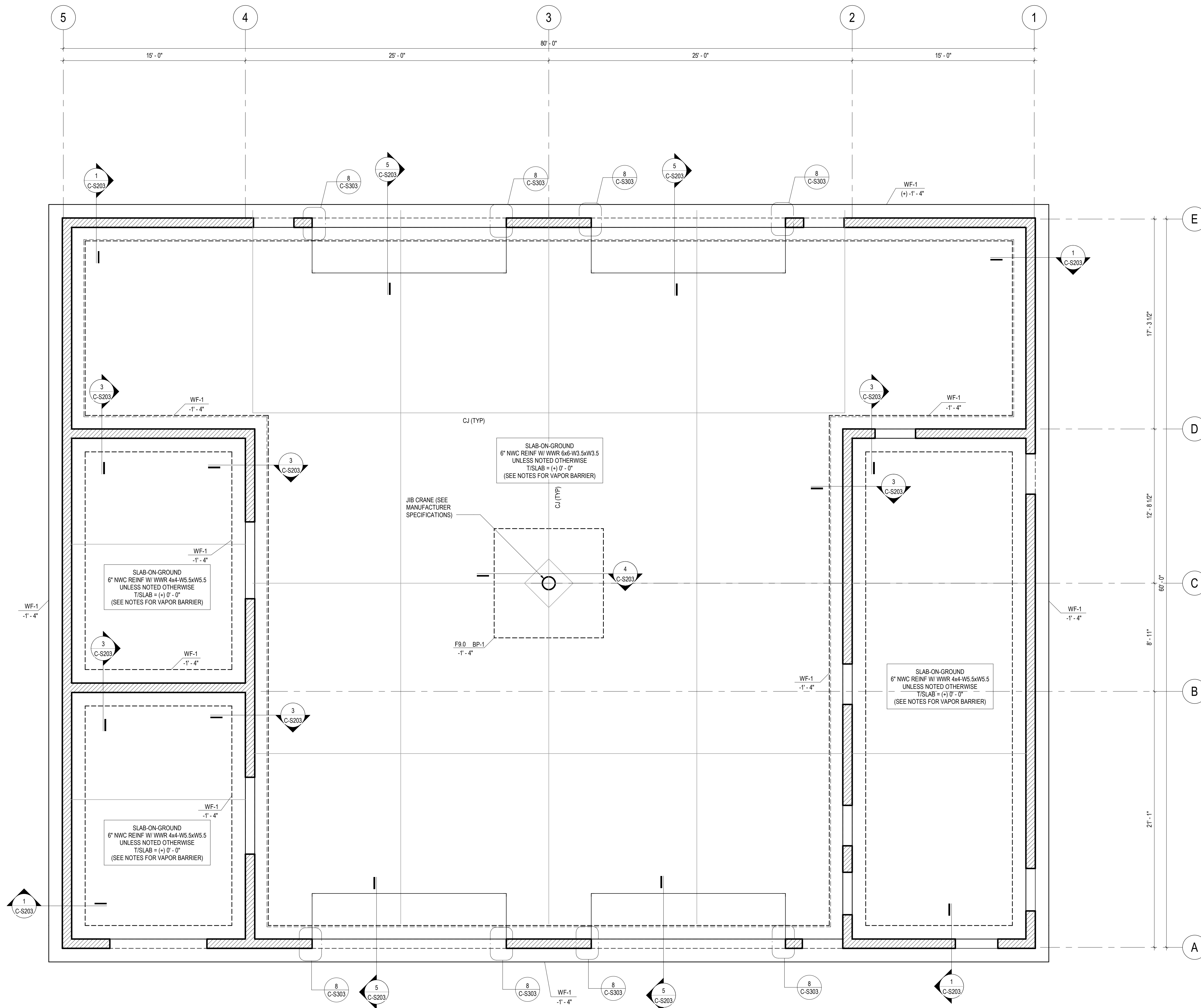
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FOUNDATION PLAN

SHEET NUMBER

C-S101



1 FOUNDATION PLAN

 $1/4'' = 1'-0''$

NOTES

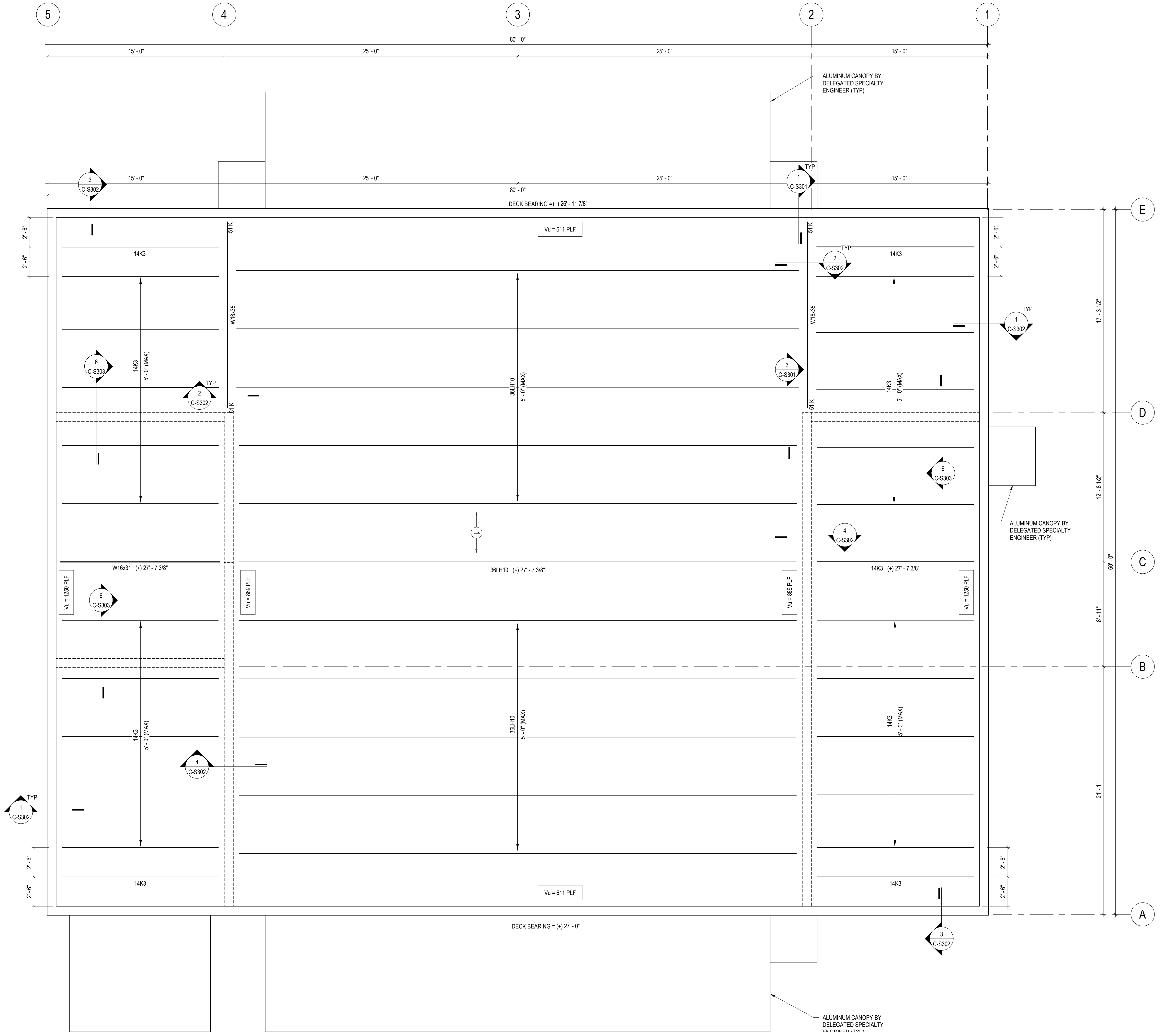
- PF: DENOTES ISOLATED FOOTING MARK (SEE SCHEDULE ON 4-C 23003).
 WFF: DENOTES CONTINUOUS FOOTING MARK (SEE SCHEDULE ON 2-C 20303).
 CJ: DENOTES SLAB-ON-GROUND CONTROL JOINT. SPACING SHALL NOT EXCEED 16'-0" UNLESS NOTED OTHERWISE.
 [] FOR RE-INTEGRATION CORNER BARS, SEE 5-C 52014 & 6-C 52014.
 [] PLACE FINISH (SLAB, VACUUM, OR FORM) TOLERANCE SHALL, NOT COMPACTED SHALL (SEE ARCHITECT) FOR CONCRETE FINISH, SEE ARCHITECT.
 [] DENOTES 9" 14" 14" CONCRETE PANEL.
 PIPING MUST PASS OVER ALL FOOTINGS. SEE SPEC FOR STANDARD DETAILS OF PIPING PASSING UNDER WALL FOOTINGS AND TRENCHES ADJACENT TO FOOTINGS. PIPING PASSING UNDER FOOTINGS MUST BE IN PLACE AND INSPECTED BEFORE FOOTINGS ARE PREPARED OR FOOTING MUST STEP BELOW PIPE. PIPING SHALL BE PROTECTED BY 1" MINIMUM THICKNESS OF CONCRETE OR 4" MINIMUM THICKNESS OF G.C. SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NOTED. FORWARD ANY CHANGES REQUIRED TO FOUNDATION LOCATIONS TO STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.



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ROOF FRAMING PLAN

- 1/4" = 1'-0"
- NOTES:
- 1 DENOTES 3" NORMAL WEIGHT CONCRETE ON 1 1/2" Z22 GAGE CORRUGATED COMPOSITE DECK REINFORCED WITH WWR 646-W1.4xW1.4
 - GC TO COORDINATE OPEN WEB STEEL JOIST BRIDGING AND BRACING REQUIREMENTS PER OPEN WEB STEEL JOIST SUPPLIER AND SJI
 - 1/5" STEEL - VARIES (SEE PLAN)
 - JOIST SPACING SHALL NOT EXCEED 5'-0" UNLESS NOTED OTHERWISE.
 - PROVIDE 5" JOIST SEAT FOR K-SERIES JOISTS.
 - Vu = XXX PLF DENOTES IN-PLACE UNFACTORED ULTIMATE LEVEL WIND SHEAR APPLIED TO CONCRETE TILT WALL AT ROOF LEVEL

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PROJECT MANAGER
ANDREW TOMICZEK

DESIGN TEAM
JEZERINAC GROUP

PROPOSED BY

COLUMBIA STORM PROCESSING & SERVICE CENTER - BUILDING C - FLEET GARAGE
COUNTY ROAD 25A, LAKE CITY, FL

PROJECT NO.

814C632800

SHEET TITLE

ROOF FRAMING PLAN

SHEET NUMBER

C-S102

JEZERINAC GROUP

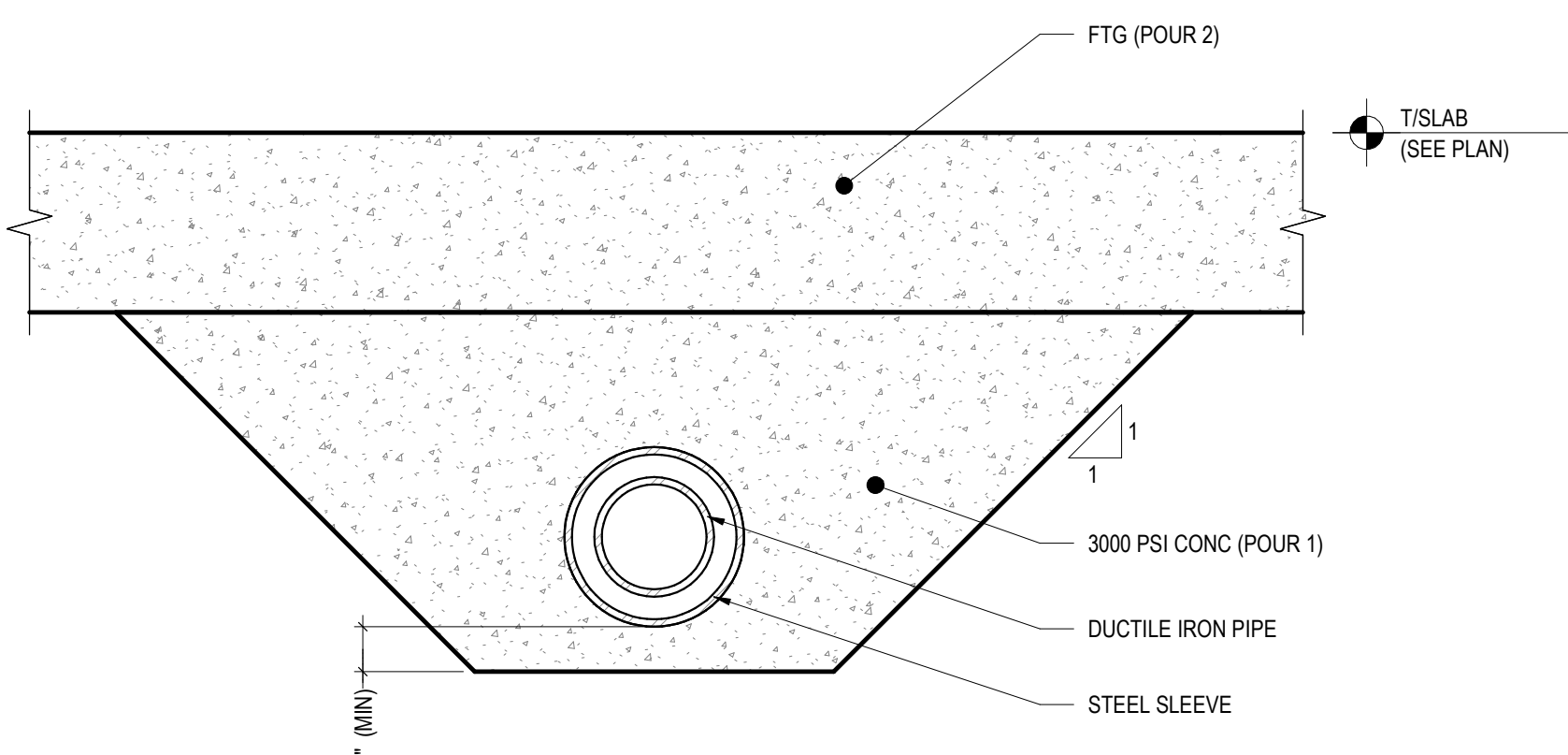
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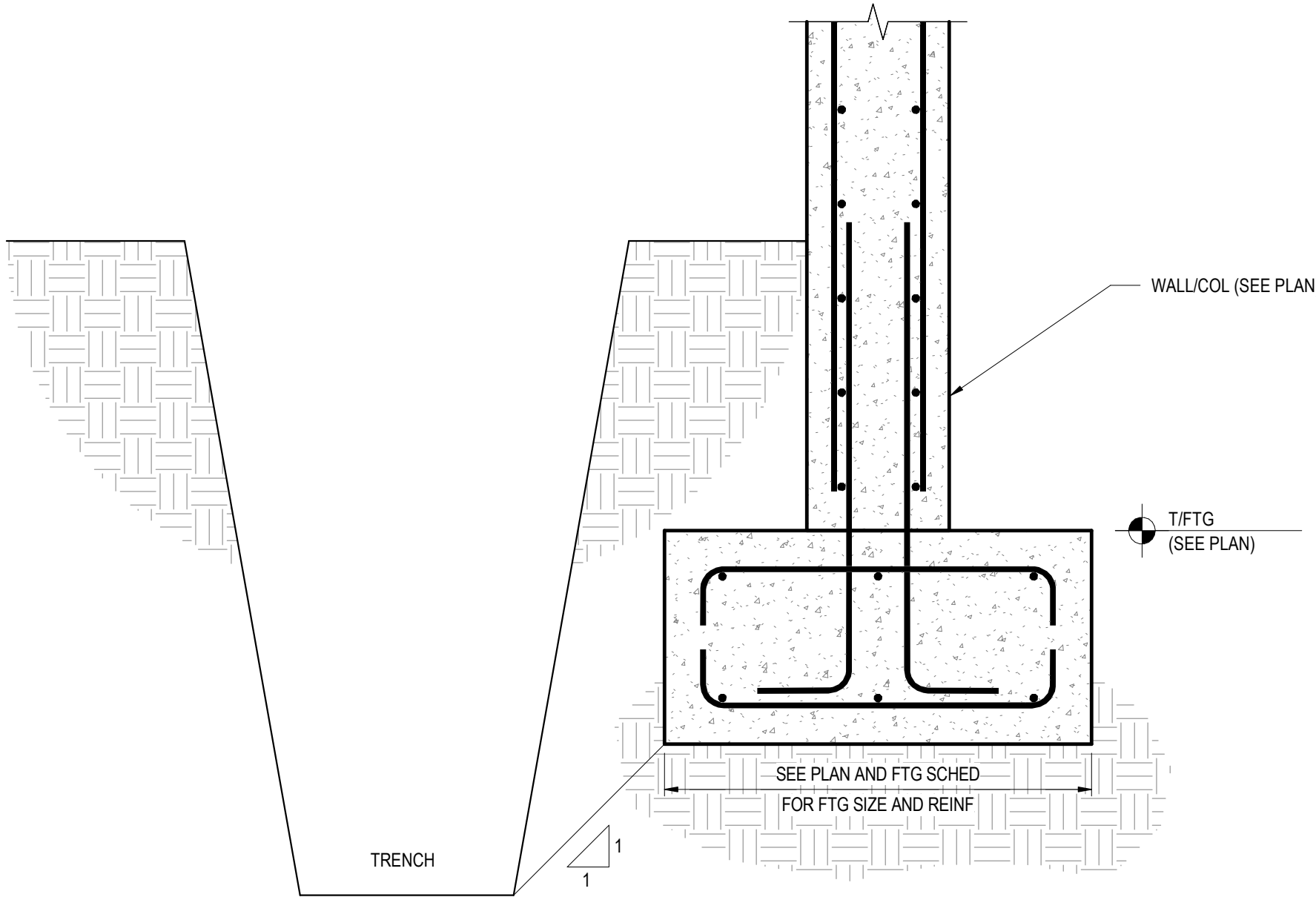
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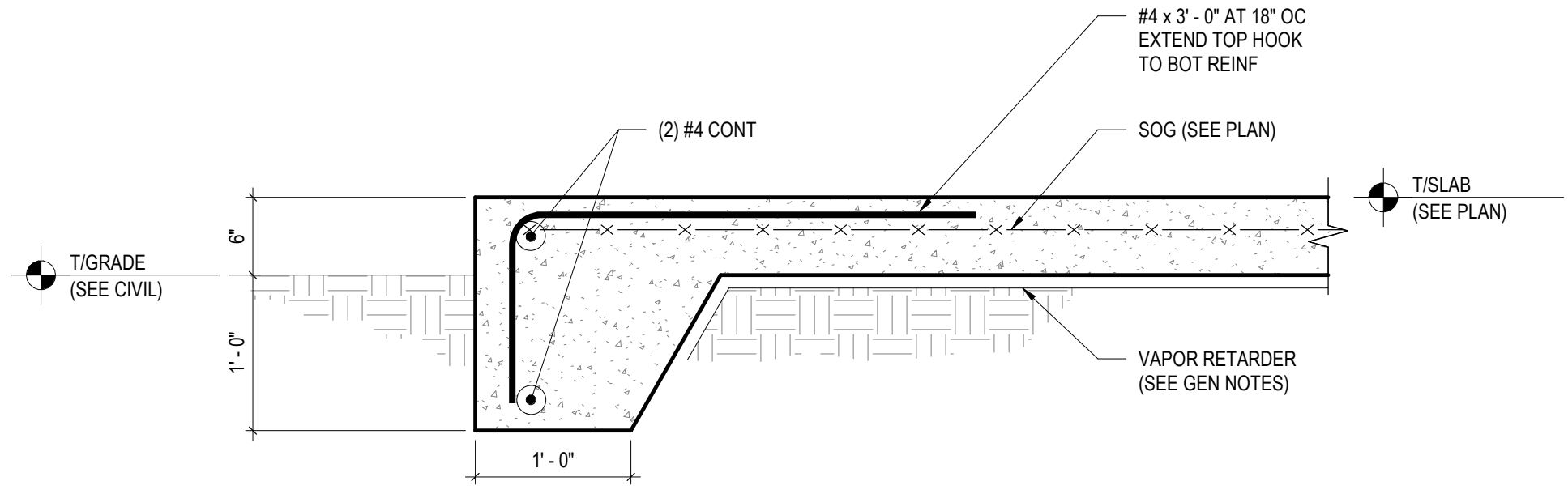
TYPICAL PIPE UNDER FOOTING

1
1" = 1'-0"



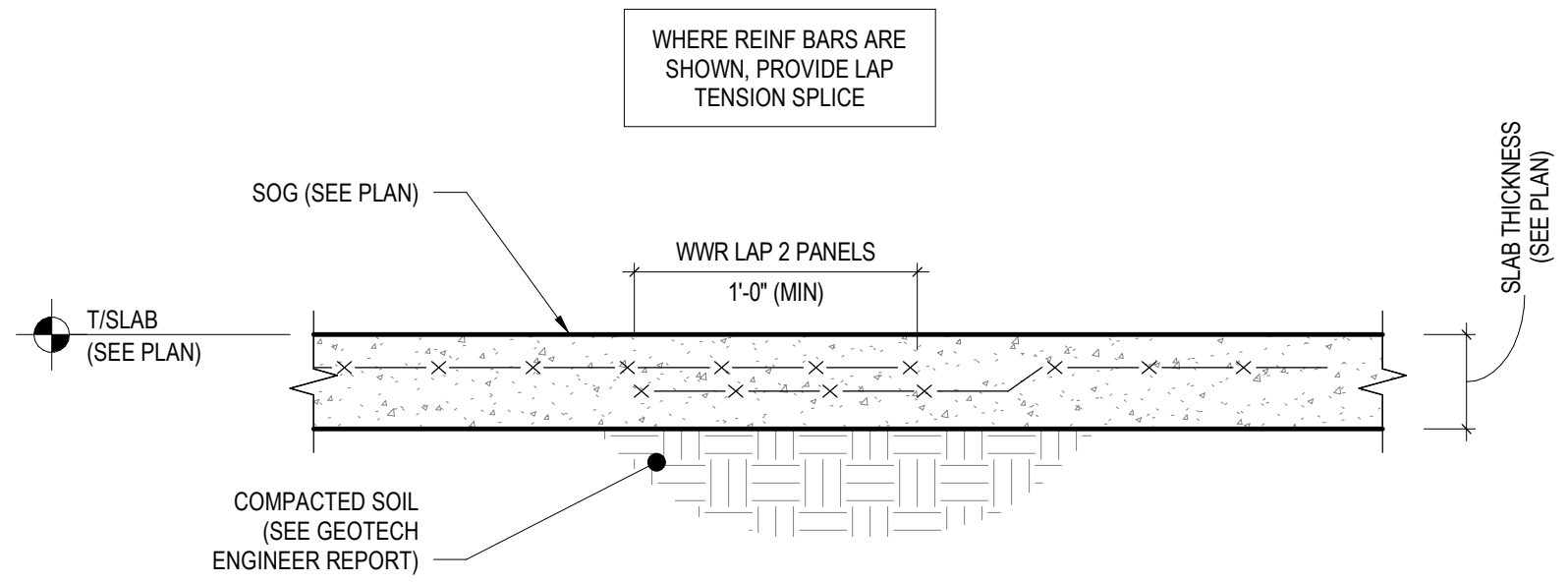
TYPICAL FOOTING ADJACENT TO TRENCH

2
1" = 1'-0"

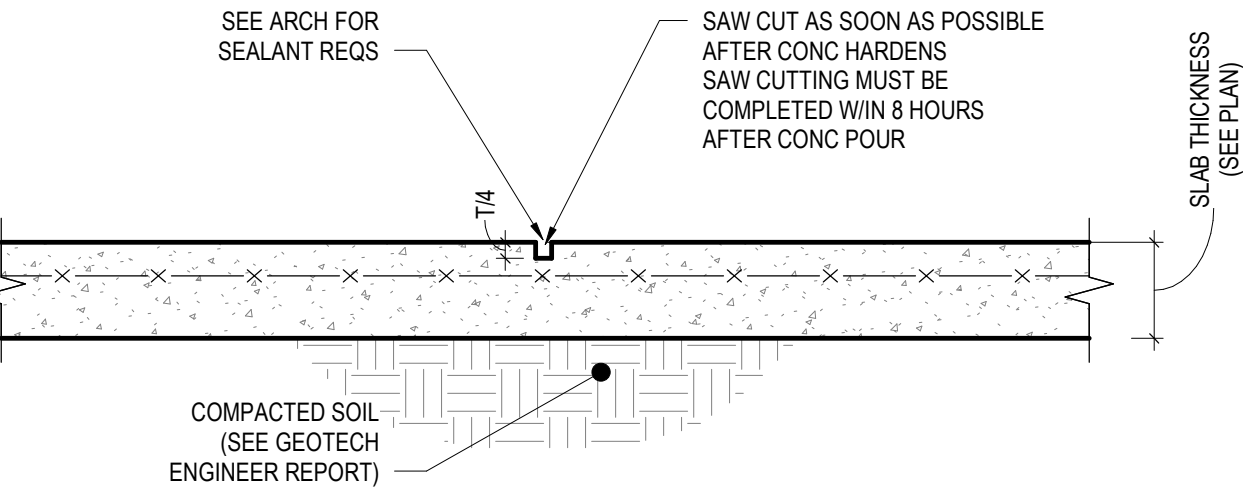


TYPICAL SLAB EDGE

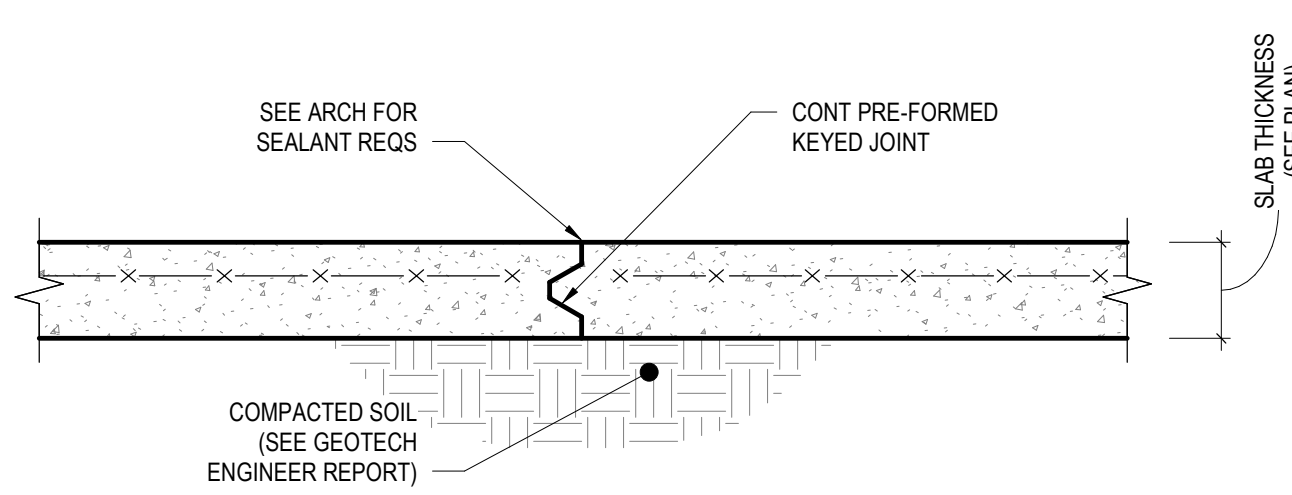
3
1" = 1'-0"



SLAB-ON-GROUND



SAW CUT JOINT



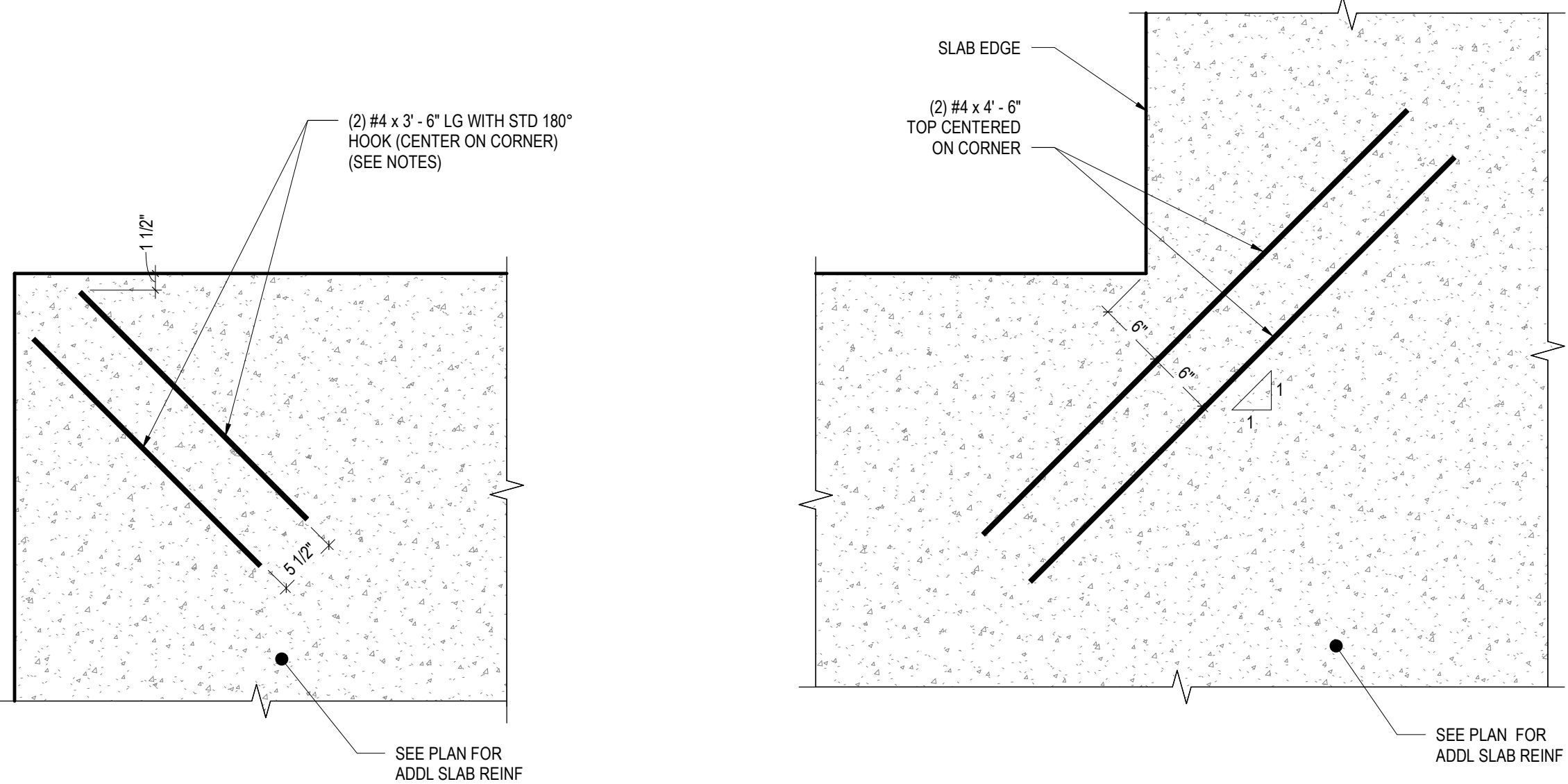
CONSTRUCTION JOINT

NOTES:

1. WHERE NOT INDICATED ON PLAN PROVIDE JOINTS AT COLUMN CENTER LINES AND BETWEEN COLUMN CENTER LINES WITH SPACING OF JOINTS NOT TO EXCEED 36 TIMES THE SLAB THICKNESS

TYPICAL SLAB-ON-GROUND DETAILS

4
1" = 1'-0"

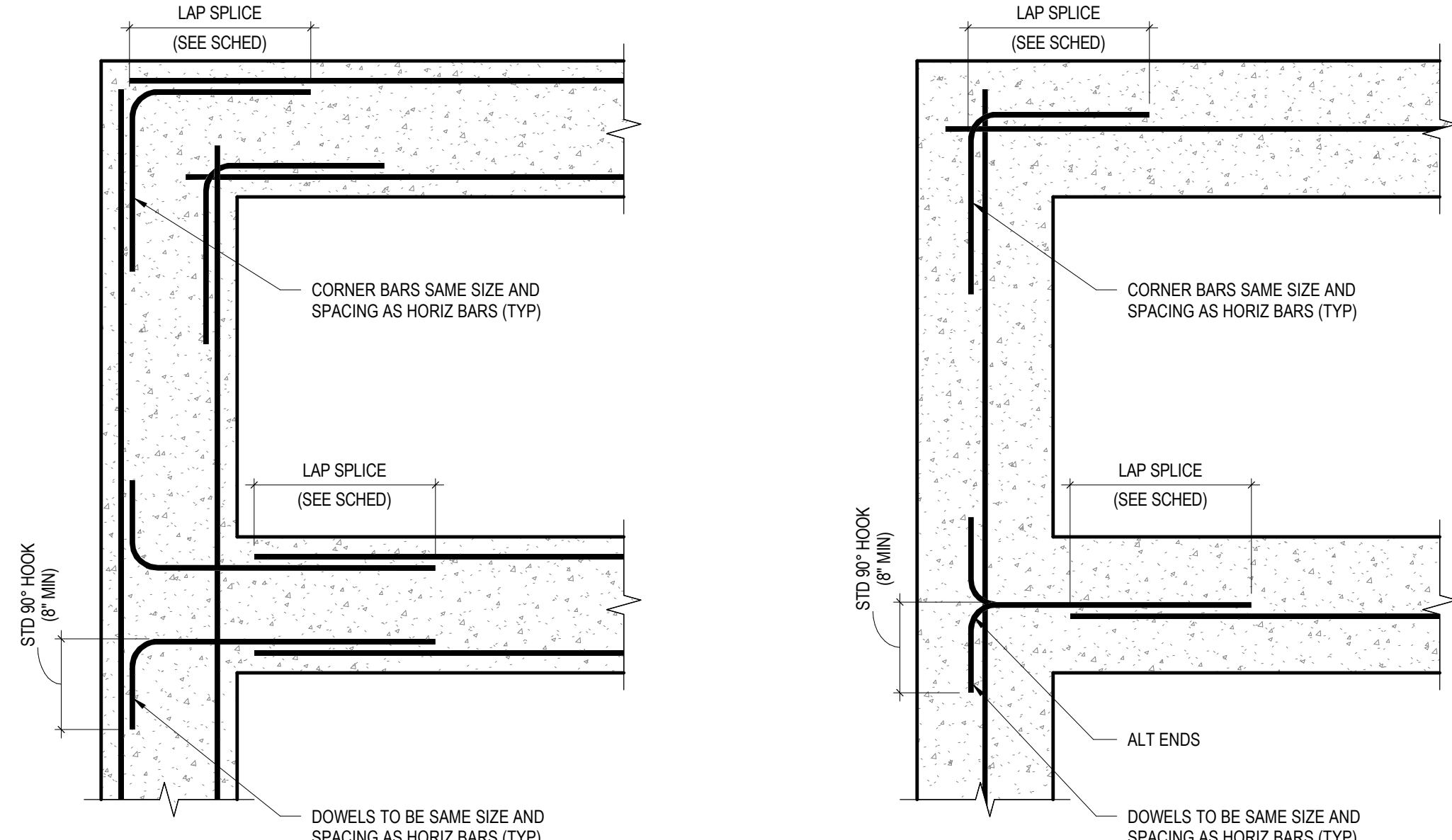


TYPICAL ADDITIONAL SLAB CORNER REINFORCEMENT

NOTES:

1. INSTALL BELOW TOP LAYER OF SLAB REINFORCING.

5
1" = 1'-0"



TYPICAL CORNER BARS FOR CONCRETE WALL AND FOOTING CONSTRUCTION

6
1" = 1'-0"

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PROJECT TEAM

PRINCIPAL IN CHARGE: ANDREW P. TOMICZEK

PROJECT MANAGER: ANDREW P. TOMICZEK

DESIGN TEAM: JEZERINAC GROUP

COLUMBIA STORM PROCESSING & SERVICE CENTER - BUILDING C - FLEET GARAGE

COUNTY ROAD 25A, LAKE CITY, FL

PROJECT NO. 814C632800

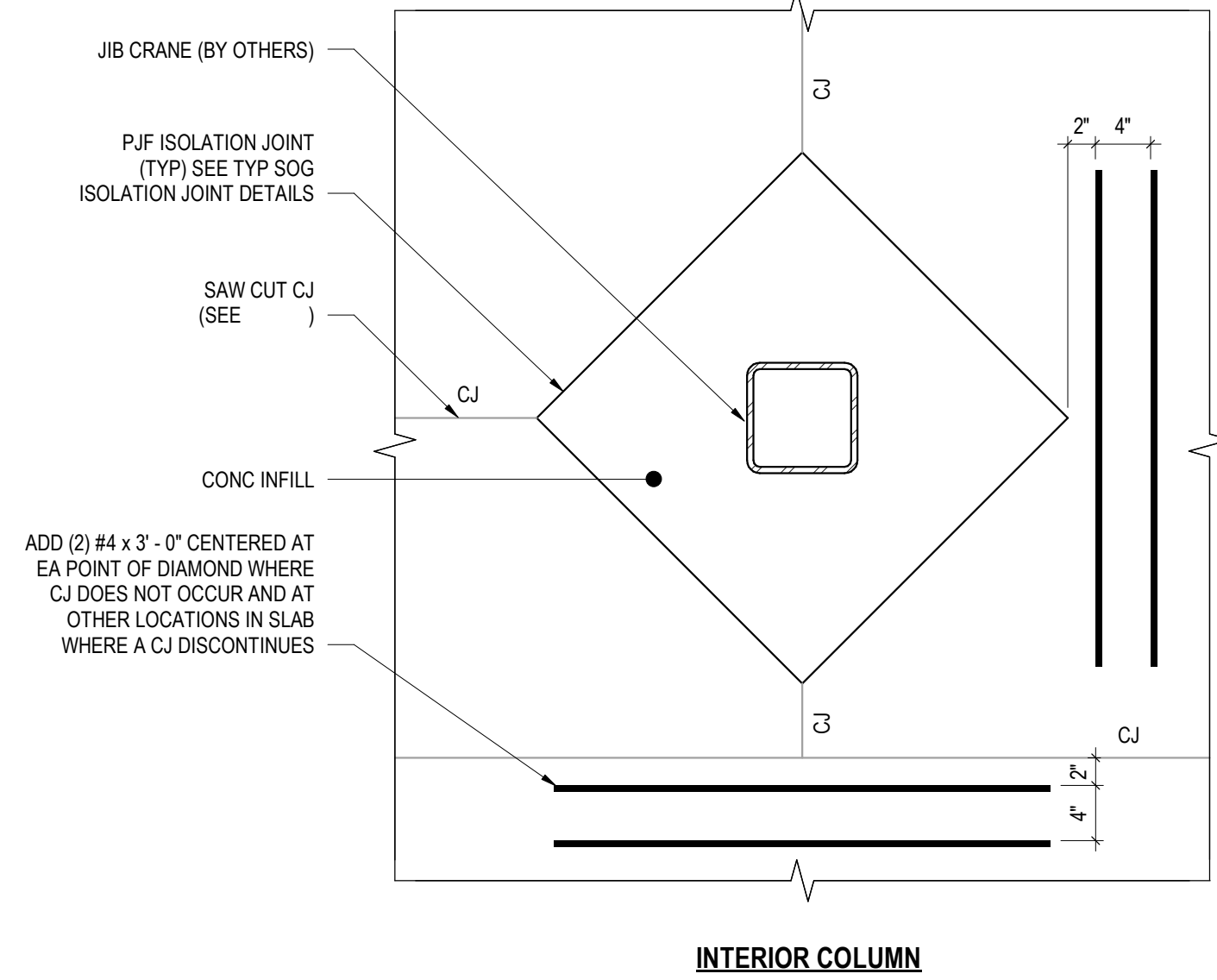
SHEET TITLE: FOUNDATION SECTIONS & DETAILS

SHEET NUMBER: C-S201

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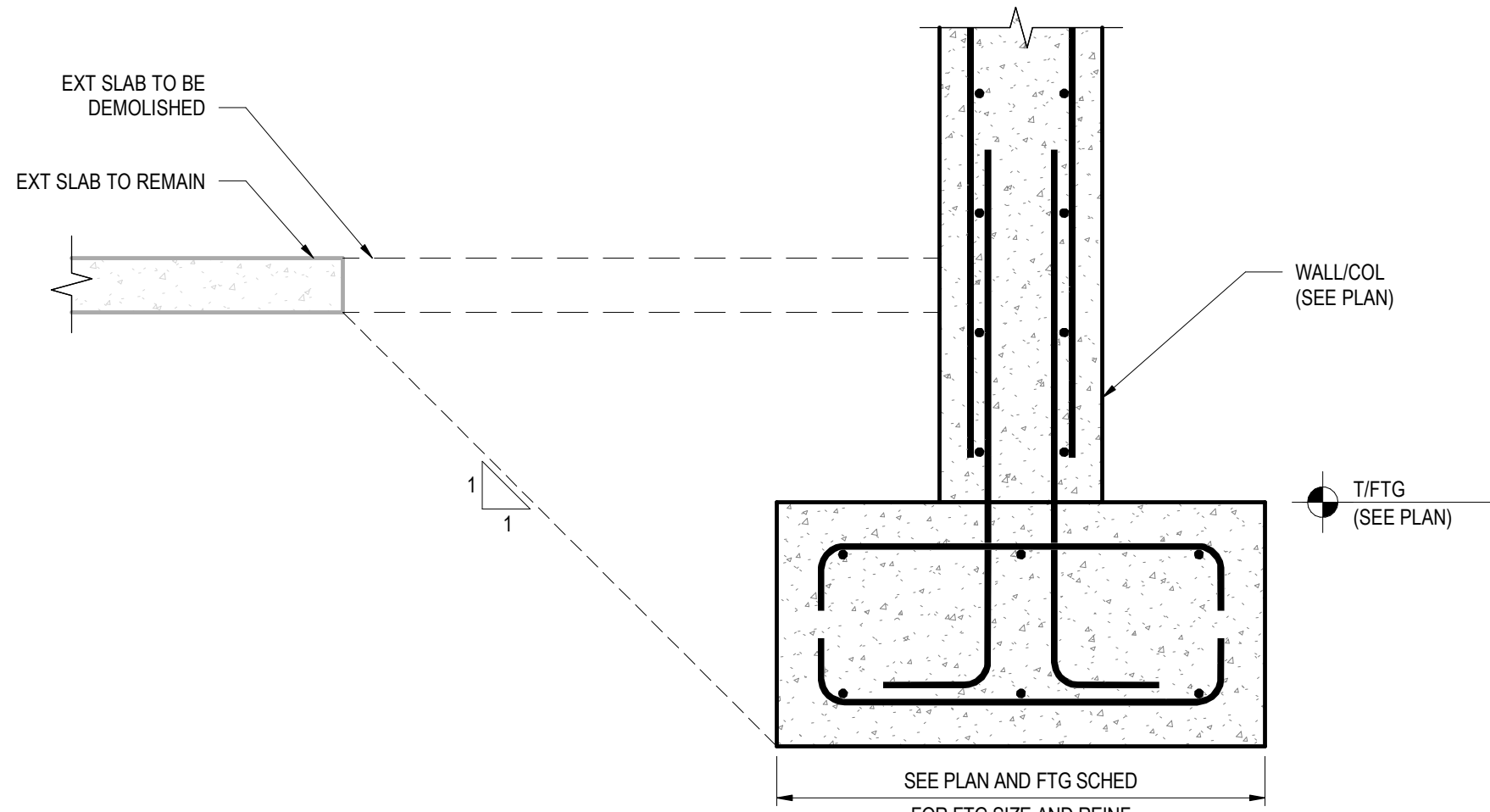
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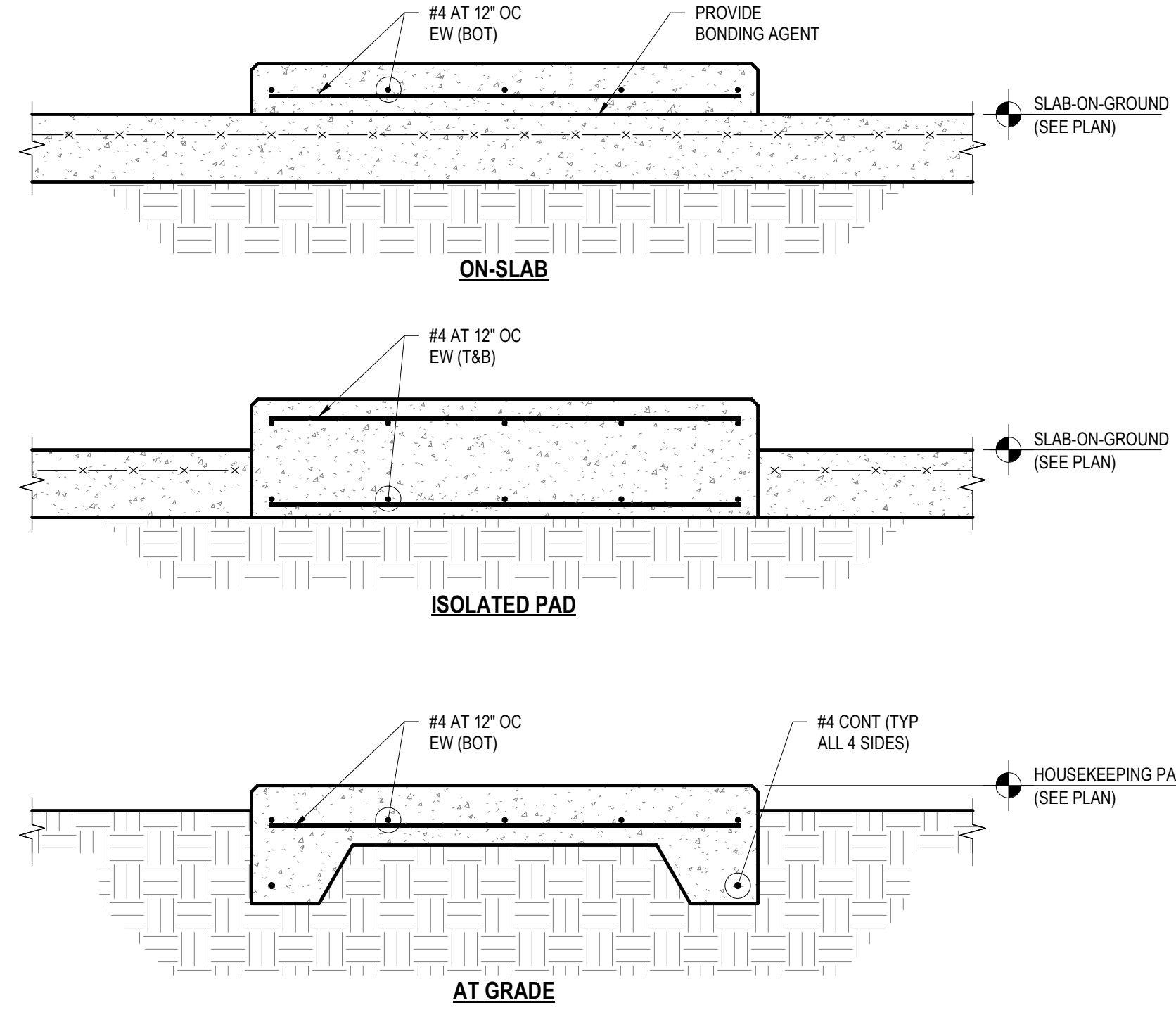


- NOTES:
1. SEE TYPICAL SLAB-ON-GROUND DETAILS FOR JOINT SPACING REQUIREMENT.
 2. CONCRETE INFILL BETWEEN COLUMN AND ISOLATION JOINT TO BE POURED AFTER ALL THE SLABS SUPPORTED BY THE COLUMN HAVE BEEN POURED.

TYPICAL SLAB-ON-GROUND ISOLATION/CONTROL JOINT



TYPICAL FOOTING ADJACENT TO EXTERIOR SLAB



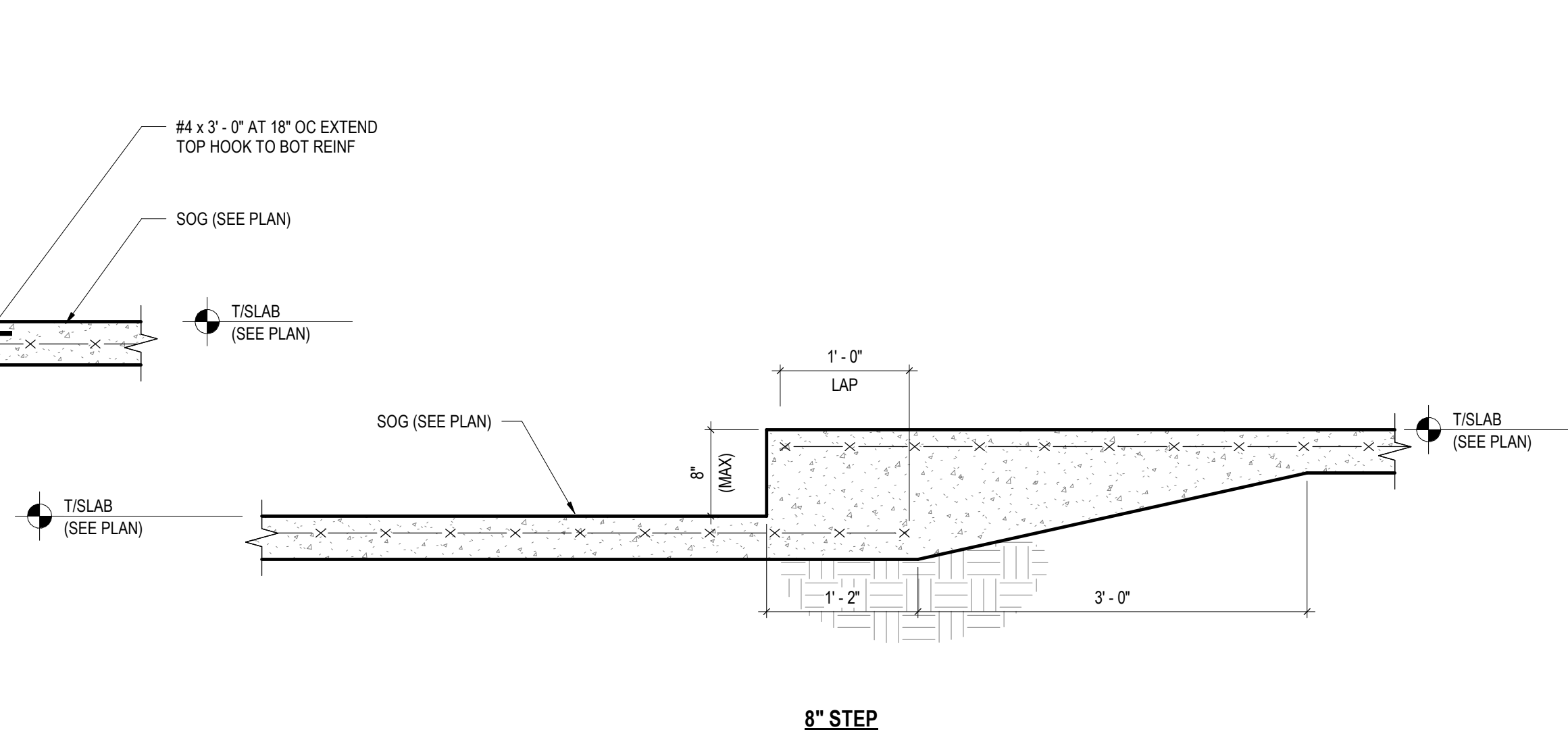
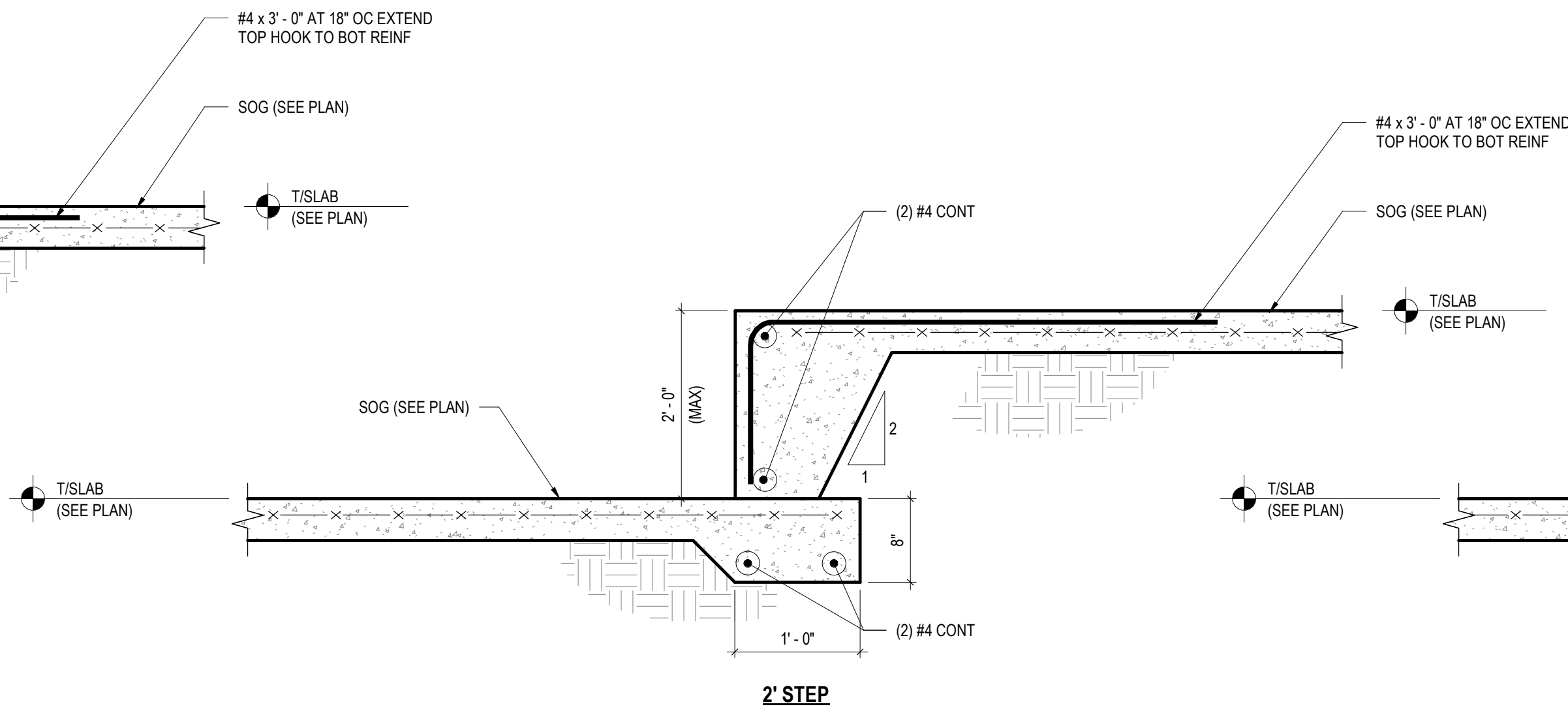
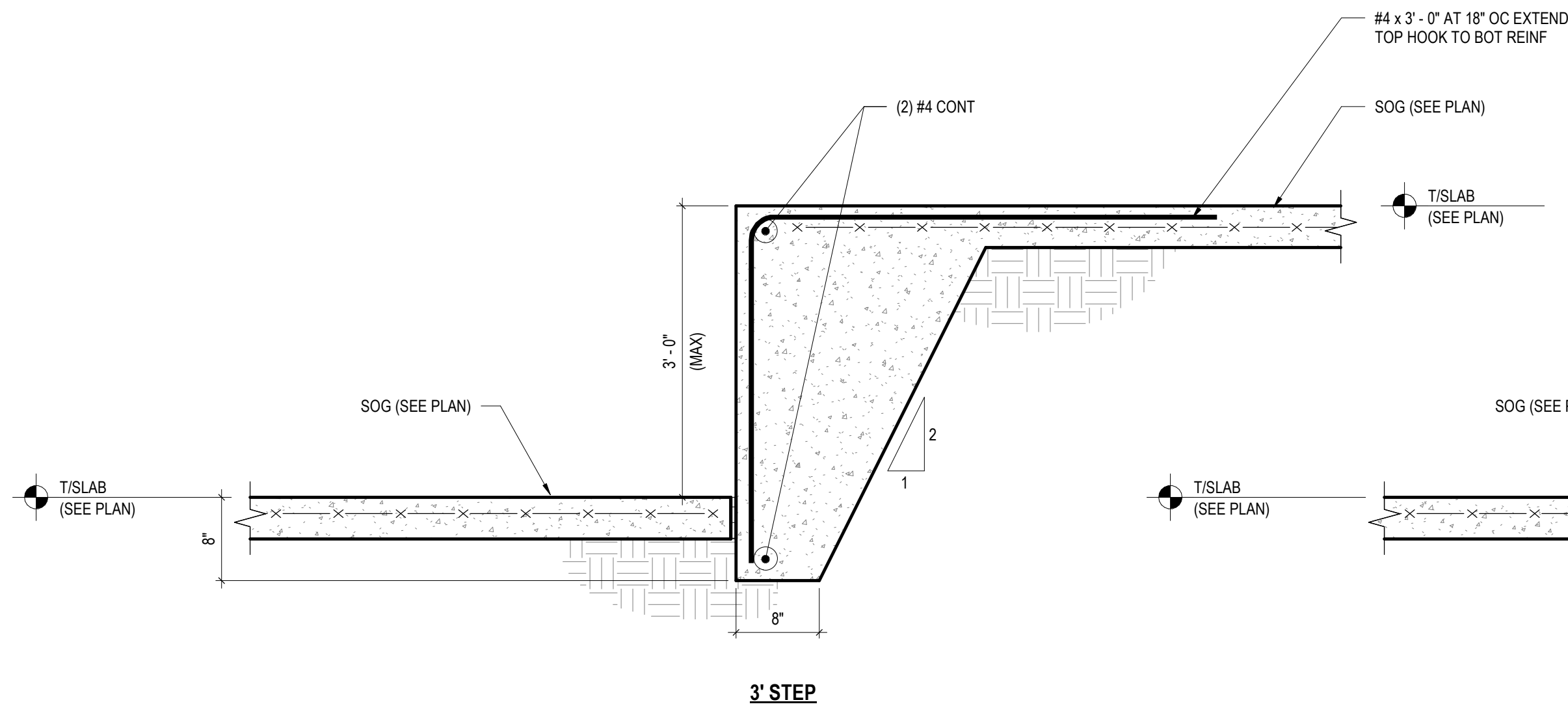
- NOTES:
1. SEE ARCHITECTURAL AND MEP DRAWINGS FOR SIZE AND LOCATION OF HOUSEKEEPING PADS.

HOUSEKEEPING PAD DETAILS

1 DETAIL
1" = 1'-0"

2 SECTION
1" = 1'-0"

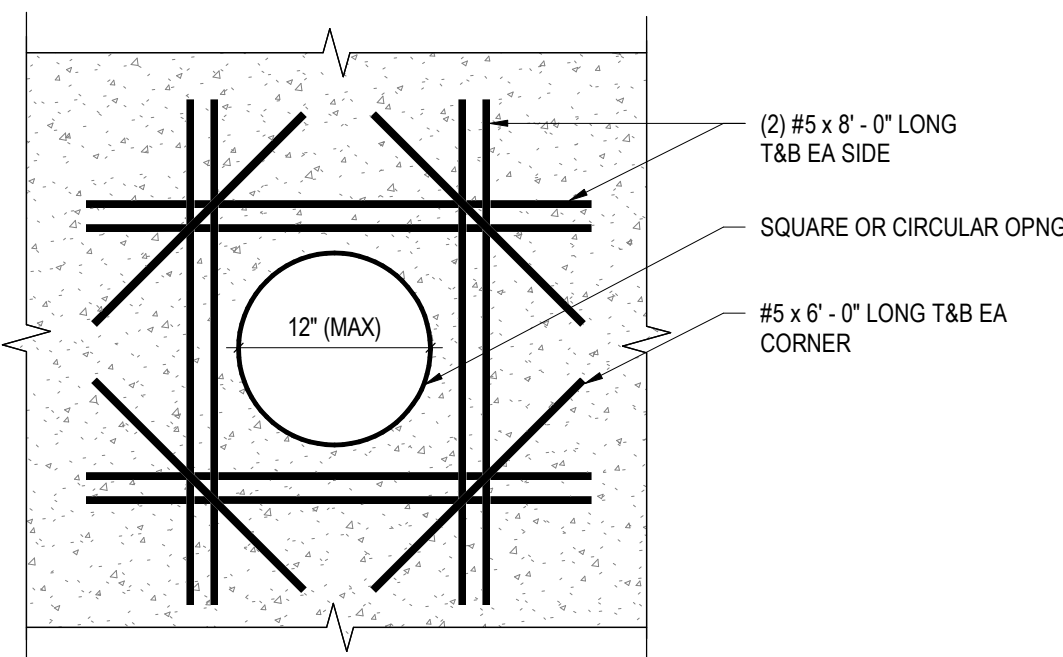
3 DETAIL
3/4" = 1'-0"



- NOTES:
1. COORDINATE ALL SLAB STEPS WITH ARCHITECT

TYPICAL SLAB STEP DETAILS

4 DETAIL
1" = 1'-0"



TYPICAL REINFORCEMENT AT WALL PIPE PENETRATIONS

5 DETAIL
1" = 1'-0"

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COLUMBIA STORM PROCESSING & SERVICE CENTER - BUILDING C - FLEET GARAGE
COUNTY ROAD 25A, LAKE CITY, FL

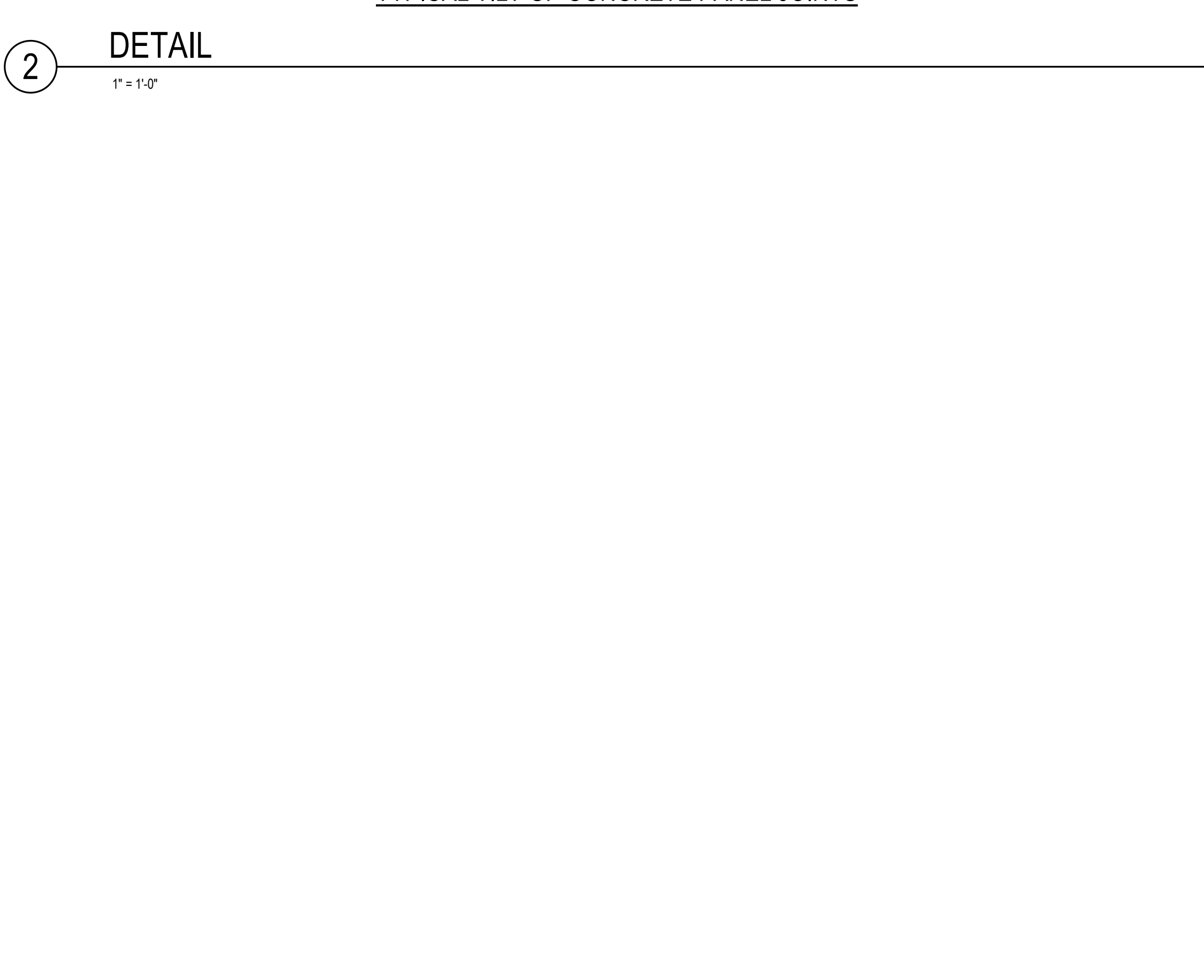
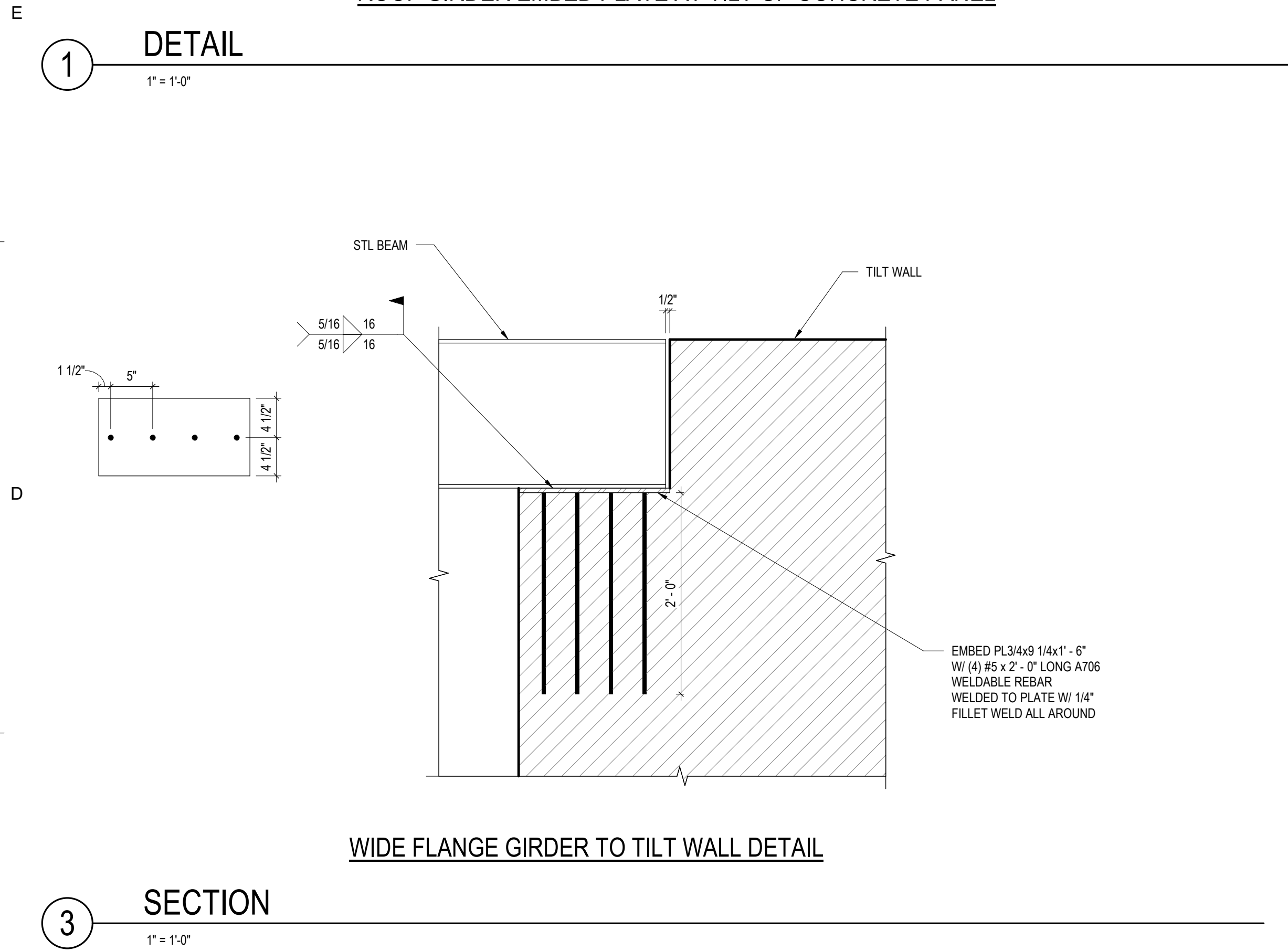
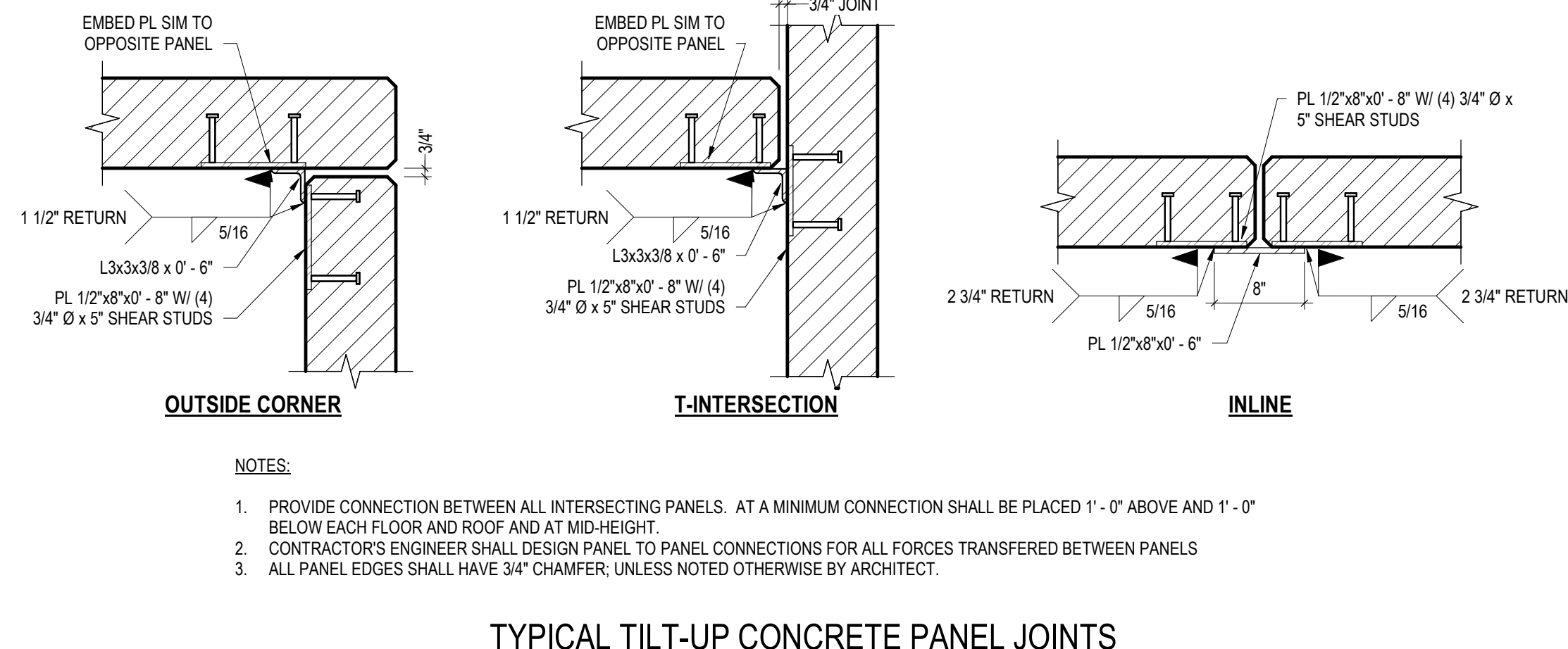
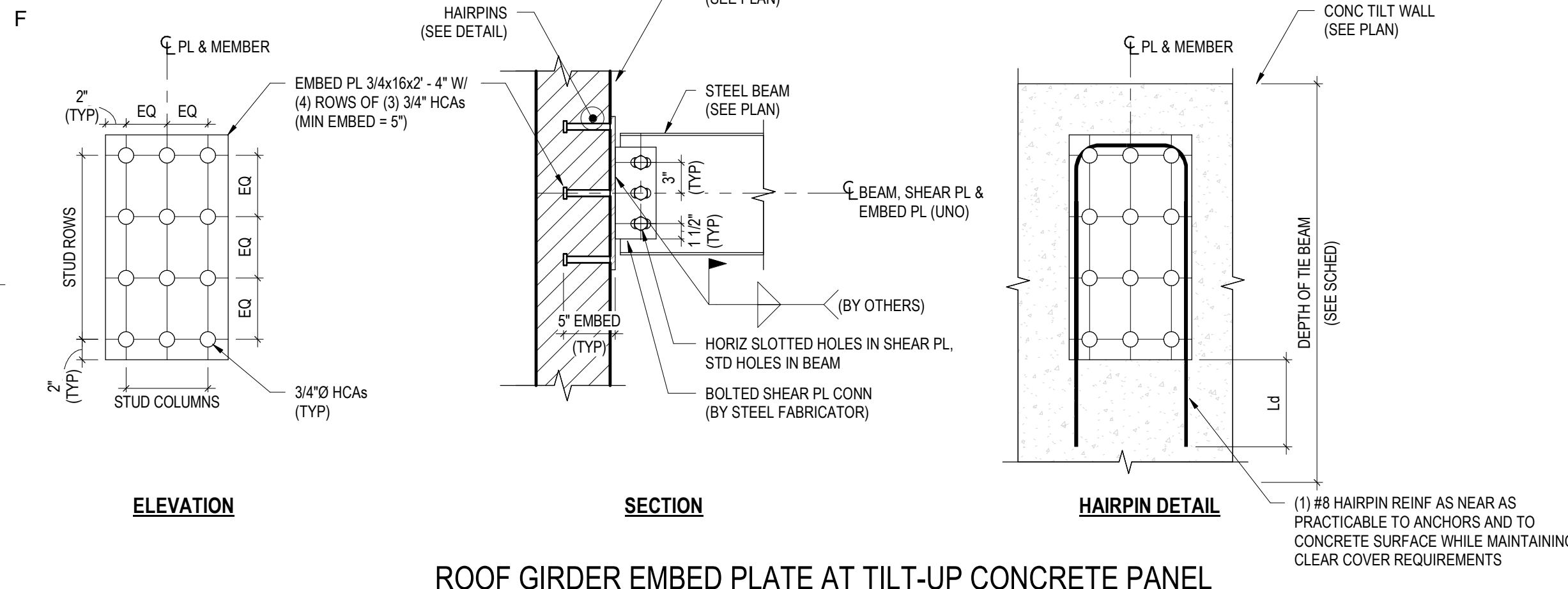
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SHEET TITLE
FOUNDATION SECTIONS & DETAILS

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C-S202

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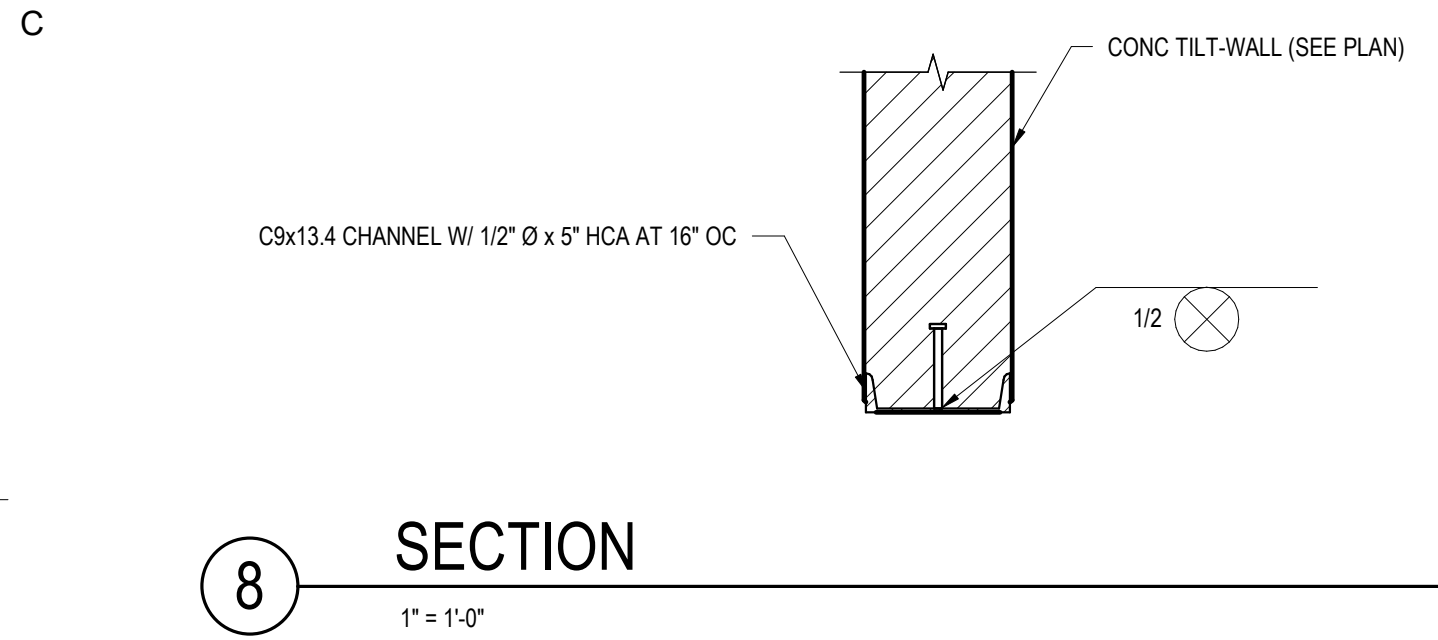
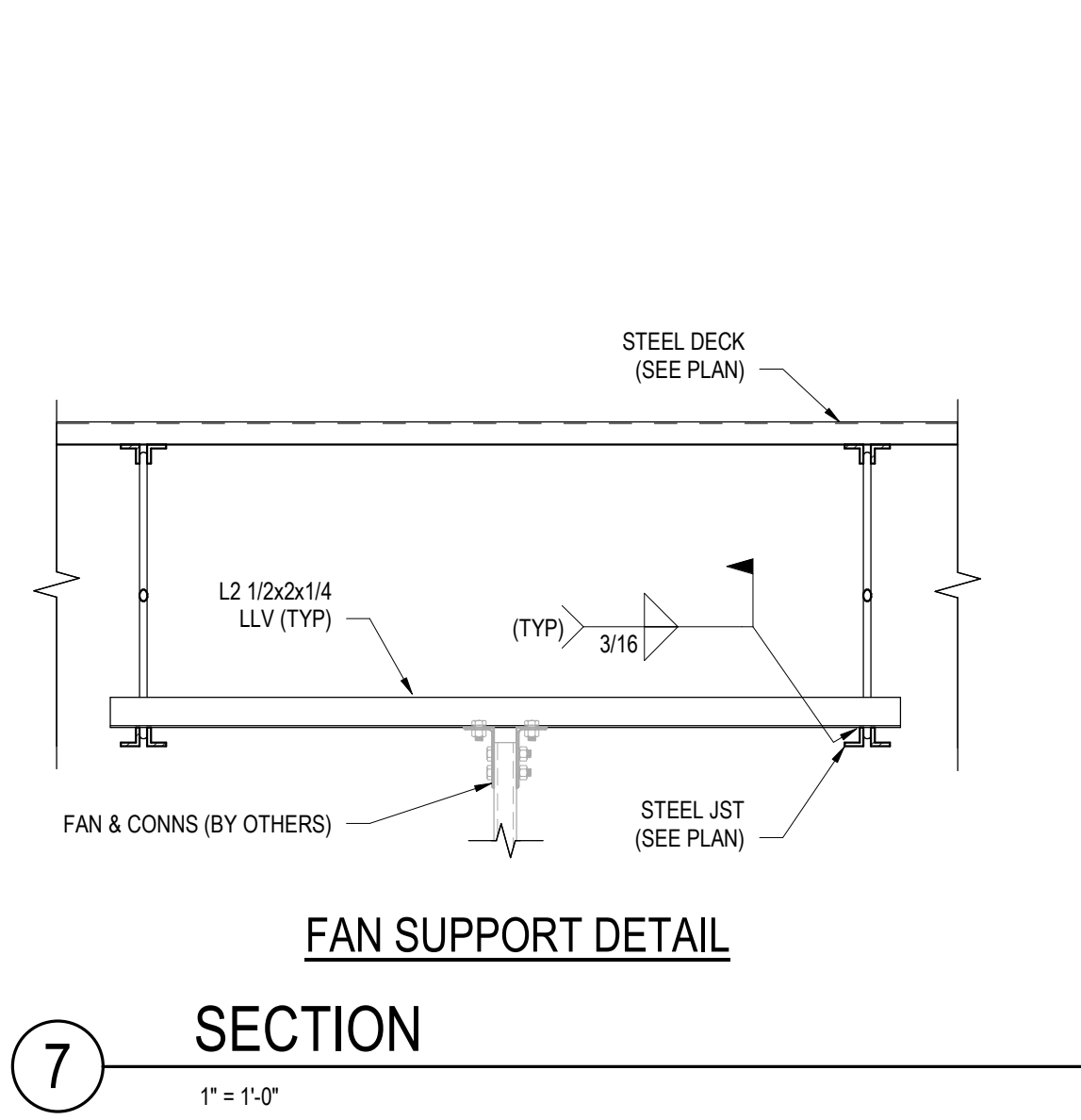
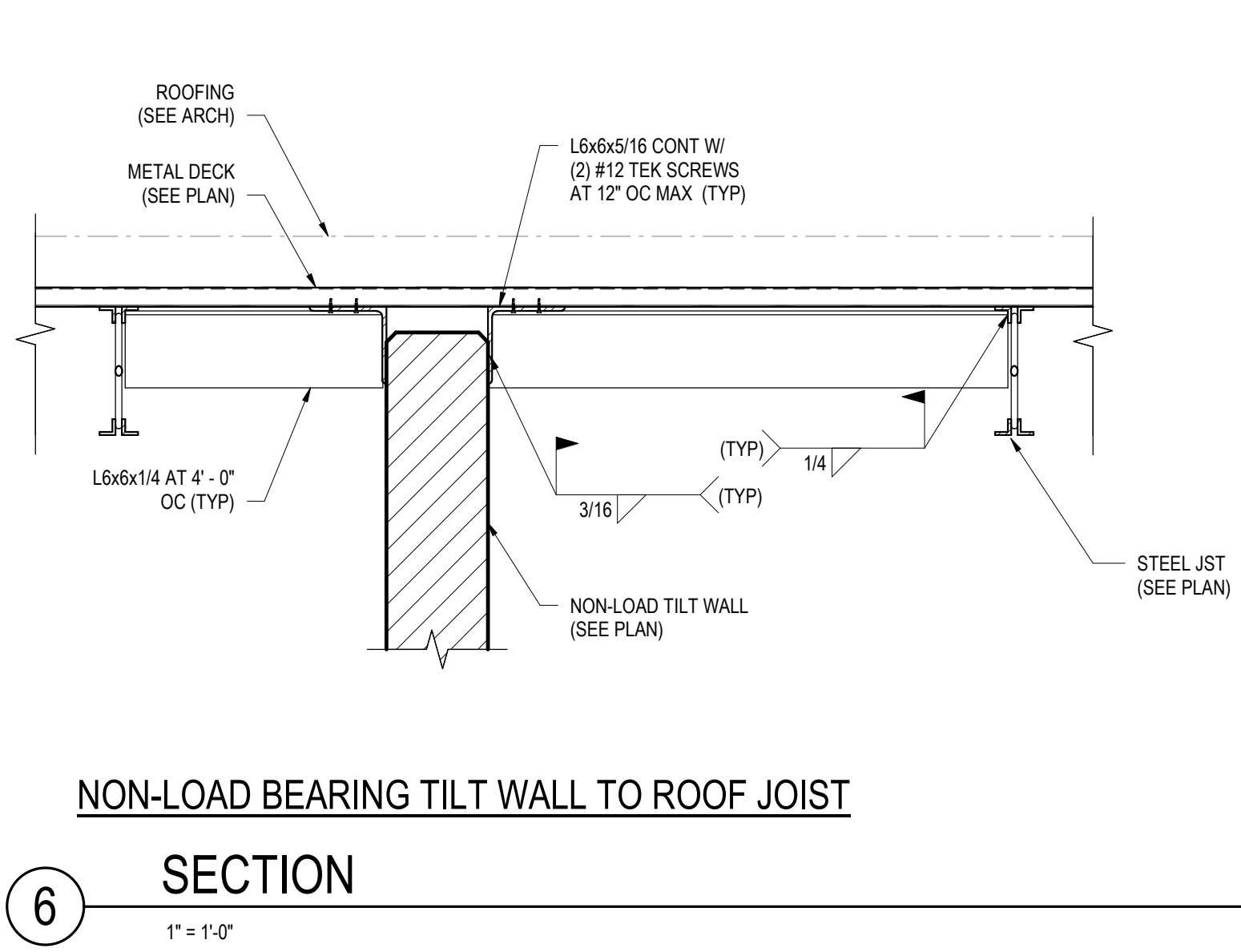
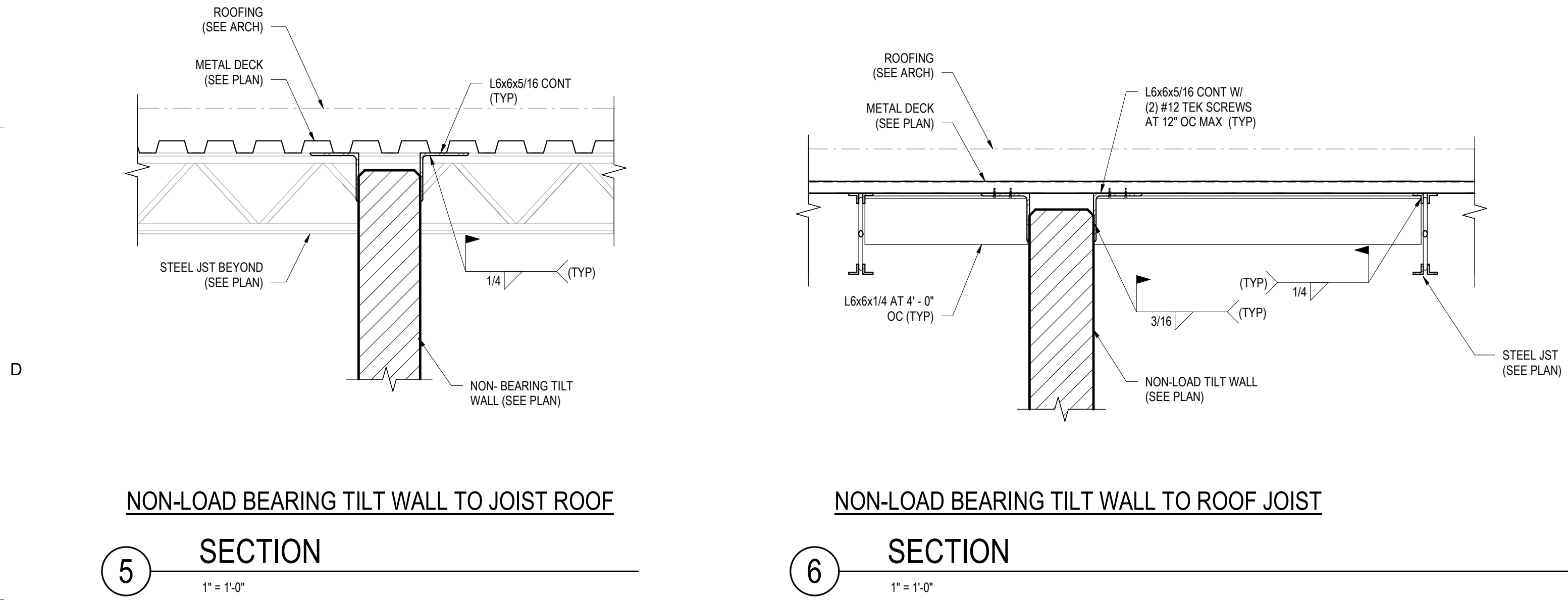
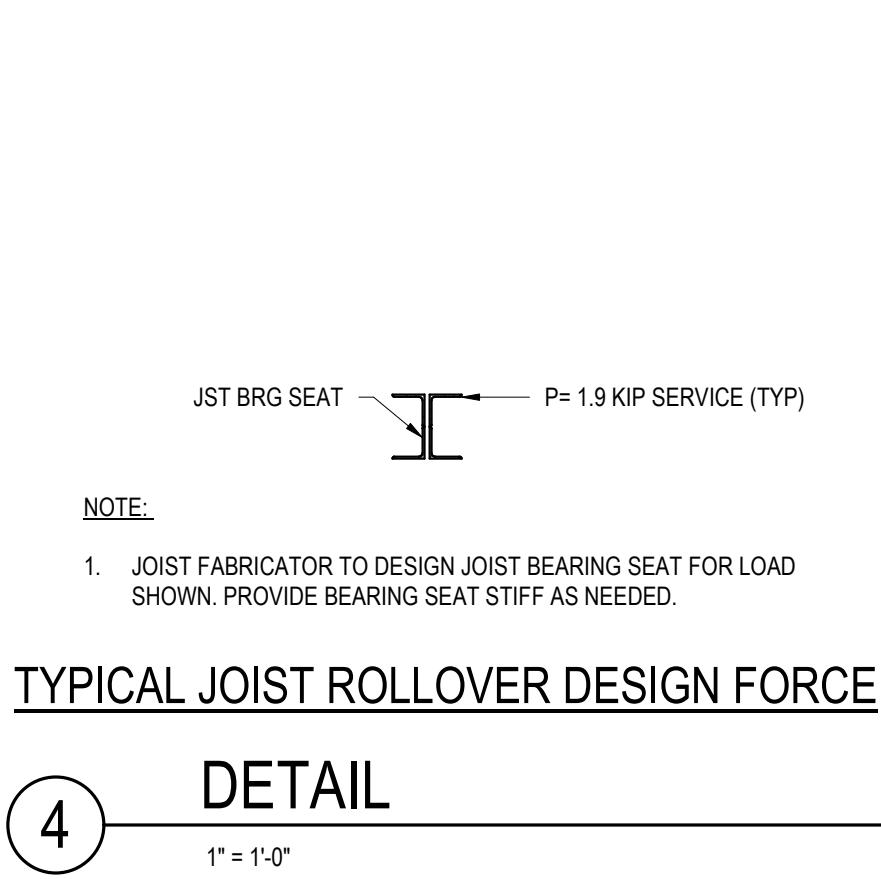
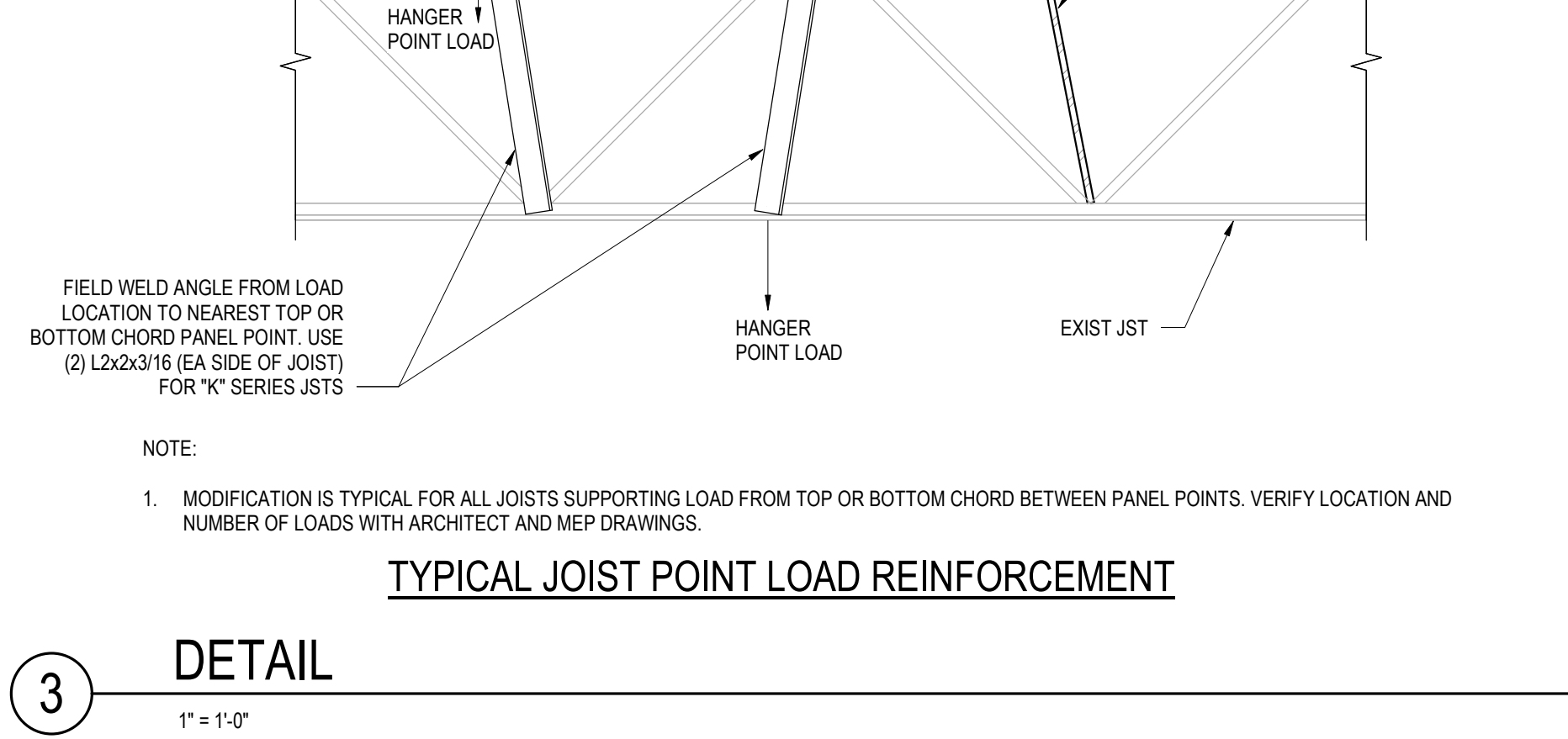
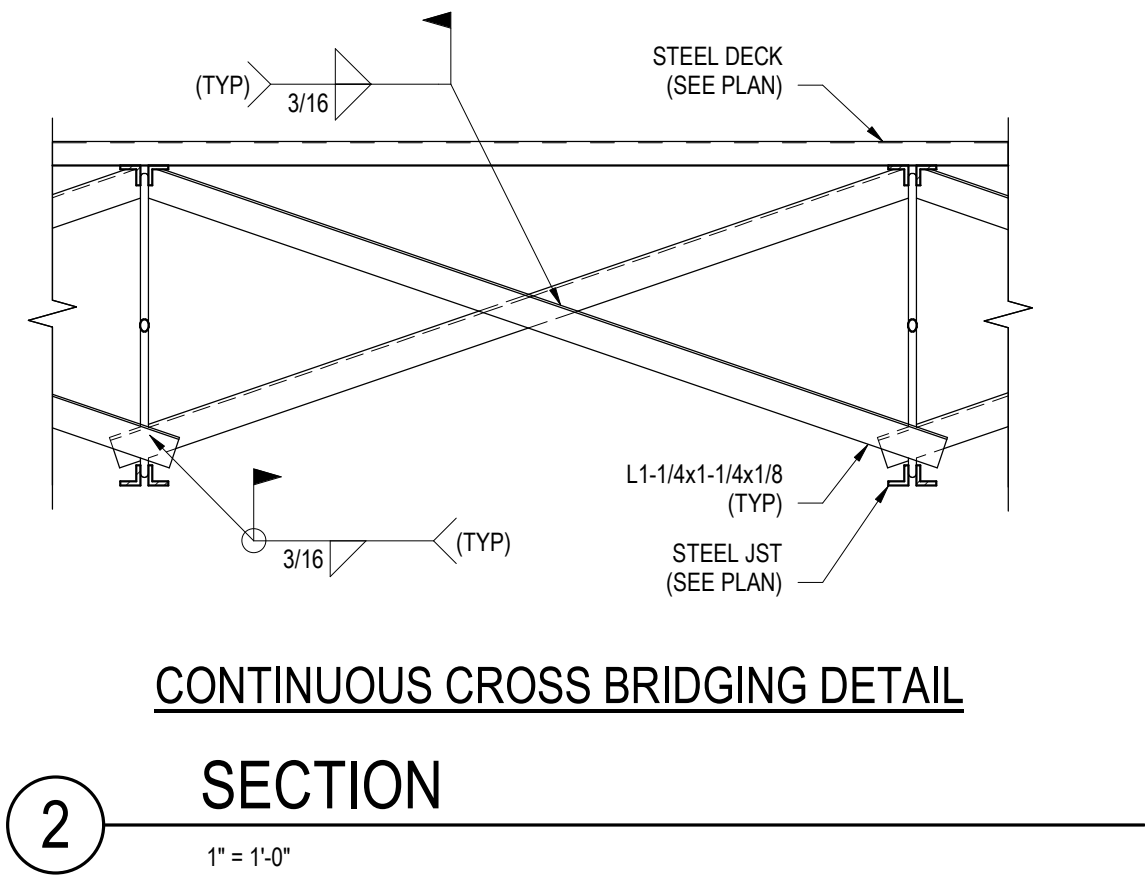
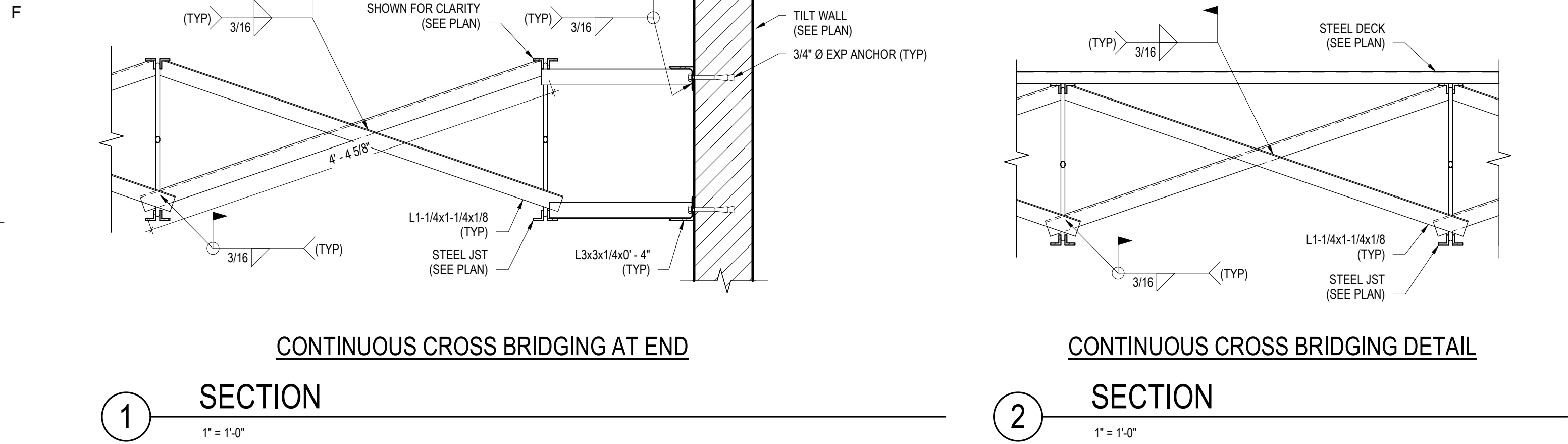
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SHEET TITLE: FRAMING SECTIONS & DETAILS

SHEET NUMBER: C-S301

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COLUMBIA STORM PROCESSING & SERVICE CENTER - BUILDING C - FLEET GARAGE

COUNTY ROAD 25A, LAKE CITY, FL

PROJECT NO.

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SHEET TITLE

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