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A

- GENERAL REQUIREMENTS**
- A. THE GENERAL STRUCTURAL NOTES EMPLOY THE FOLLOWING DEFINITIONS AND ABBREVIATIONS:
- CONTRACT DOCUMENTS – THE LATEST SET OF DRAWINGS, SPECIFICATIONS, AND RECORDED ADDENDA AND AMENDMENTS ISSUED FOR BID OR CONSTRUCTION.
 - LICENSED PROFESSIONAL (STRUCTURAL) ENGINEER – AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED AND QUALIFIED TO PERFORM THE WORK REQUIRED.
 - STRUCTURAL ENGINEER OF RECORD – LICENSED PROFESSIONAL ENGINEER WHO IS IN RESPONSIBLE CHARGE FOR THE PREPARATION, SIGNING, DATING, SEALING, AND ISSUING OF STRUCTURAL ENGINEERING DOCUMENTS FOR ENGINEERING SERVICE OR CREATIVE WORK.
 - DELEGATED ENGINEER – A LICENSED PROFESSIONAL ENGINEER WHO PROVIDES SERVICES OR CREATIVE WORK REGARDING A PORTION OF THE ENGINEERING PROJECT. THE DELEGATED ENGINEER IS THE ENGINEER OF RECORD FOR THAT PORTION OF THE ENGINEERING PROJECT. TYPICALLY, DELEGATED ENGINEERS FALL INTO ONE OF THE FOLLOWING CATEGORIES:
 - AN INDEPENDENT CONSULTANT
 - AN EMPLOYEE OR OFFICER OF AN ENTITY SUPPLYING COMPONENTS TO A FABRICATOR OR CONTRACTOR
 - AN EMPLOYEE OR OFFICER OF A FABRICATOR OR CONTRACTOR
 - DELEGATED ENGINEERING DOCUMENTS – ENGINEERING DOCUMENTS THAT ARE PREPARED BY A DELEGATED ENGINEER
 - DESIGN TEAM – DESIGN PROFESSIONALS INCLUDING THE ARCHITECT, STRUCTURAL ENGINEER, CIVIL ENGINEER, MEP ENGINEER, AND ANY OTHER CONSULTANT THAT ISSUES CONTRACT DOCUMENTS
 - CONTRACTOR – GENERAL CONTRACTOR, CONSTRUCTION MANAGER, DESIGN BUILDER, OR ANY OTHER ENTITY CONTRACTED BY THE OWNER TO PERFORM THE WORK
 - SHOP DRAWINGS – DRAWINGS DEPICTING INSTALLATION MEANS AND METHODS AND CATALOG INFORMATION ON STANDARD PRODUCTS. SHOP DRAWINGS SHALL BE PREPARED BASED ON ENGINEERING DIRECTION CONTAINED IN CONTRACT DOCUMENTS BY A CONTRACTOR, FABRICATOR, MANUFACTURER, OR LICENSED PROFESSIONAL ENGINEER, FOR INCORPORATION INTO THE PROJECT
 - ESTABLISHED CHANNELS – AT THE ONSET OF THE PROJECT, ARCHITECT, OWNER, AND CONTRACTOR SHALL ESTABLISH DESIRED LINES OF COMMUNICATION BETWEEN ALL PROJECT PARTIES. THESE AGREED UPON LINES OF COMMUNICATION ARE THE ESTABLISHED CHANNELS.
- B. GENERAL STRUCTURAL NOTES ARE APPLICABLE TO THE DESIGN AND CONSTRUCTION OF THE ENTIRE PROJECT AND THIS ARE APPLICABLE TO EVERY SHEET WITHIN THIS SET
- C. WHERE A DETAIL, TYPICAL DETAIL, SECTION, TYPICAL SECTION, OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL SIMILAR OR LIKE CONDITIONS, UNLESS NOTED OTHERWISE.
- D. ISOMETRIC VIEWS ARE FOR VISUALIZATION PURPOSES ONLY AND DO NOT CONVEY ALL OF THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- E. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT BETWEEN THESE DRAWINGS AND ANY OTHER CONTRACT DOCUMENT OR APPLICABLE CODE OR STANDARD OF PRACTICE DURING BIDDING, THE PROVISION RESULTING IN THE GREATER COST APPLIES. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT DURING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST FOR CLARIFICATION TO THE DESIGN TEAM, WHO WILL PROVIDE A WRITTEN RESPONSE IN RETURN.
- F. SPECIFICATIONS HAVE BEEN ISSUED ON THIS PROJECT BY THE STRUCTURAL ENGINEER OF RECORD AND ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS.
- G. SEE SPECIFICATIONS FOR MATERIALS TESTING REQUIREMENTS.
- H. THE CONTRACTOR SHALL SUPERVISE AND DIRECT ALL WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE QUALITY AND CORRECTNESS OF THE WORK.
- I. THE BUILDING SHALL BE SOLELY RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH OTHER TRADES INCLUDING, BUT NOT LIMITED TO, ARCHITECTURAL, CIVIL, AND MEP FOR FLOOR SLAB STEPS, SLOPES AND CURBS, FLOOR SLAB FINISH, OPENINGS IN STRUCTURAL FLOORS, ROOFS AND WALLS, ETC.
- J. THE BUILDING HAS BEEN DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD TO RESIST THE CODE REQUIRED VERTICAL AND LATERAL FORCES IN ITS FULLY COMPLETED CONDITION. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING, SHORING, AND OTHER CONSTRUCTION SUPPORTS NECESSARY TO ENSURE THE BUILDING'S STABILITY AND SAFETY THROUGHOUT THE DURATION OF CONSTRUCTION. FURTHER, THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO PROVIDE THE ANALYSIS AND DESIGN NECESSARY TO DETERMINE POTENTIALLY OVERLOADED, UNSTABLE, OR HAZARDOUS CONDITIONS THAT MAY OCCUR AT ANY STAGE DURING CONSTRUCTION. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE CONTRACT DOCUMENTS AND SHOP DRAWINGS.
- L. THE CONTRACTOR SHALL EMPLOY CONSTRUCTION MEANS OR METHODS THAT MAY DAMAGE UTILITIES, ADJACENT BUILDINGS, OR PROPERTY. DOCUMENTATION OF ADJACENT CONDITIONS PRIOR TO CONSTRUCTION IS RECOMMENDED. FURTHER, THE CONTRACTOR SHALL EITHER ADEQUATELY CONFINE THE SITE OR PROTECT ADJACENT PROPERTY FROM DAMAGE.
- M. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROJECT SAFETY AND OSHA REQUIREMENTS. SHOULD THE STRUCTURAL ENGINEER OF RECORD NOTIFY THE CONTRACTOR OF A POTENTIALLY UNSAFE CONDITION, IT IS SOLELY AS A COURTESY FROM ONE PROFESSIONAL TO ANOTHER. IT SHOULD NOT BE INTERPRETED AS THE STRUCTURAL ENGINEER OF RECORD ASSUMING ANY RESPONSIBILITY FOR PROJECT SAFETY.
- N. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFE SPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE, BUT NOT BE LIMITED TO: PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CALLED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALINE OR OTHER HARSH CHEMICAL ENVIRONMENT.
- O. THE USE OF DE-ICING CHEMICALS ON ANY EXPOSED STRUCTURAL ELEMENT IS DISCOURAGED AND WILL ACCELERATE DETERIORATION OF STRUCTURAL ELEMENTS.
- P. THE BUILDING OWNER SHALL NOT ALTER OR MODIFY ANY STRUCTURAL ELEMENT WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER. FURTHER, BUILDING OWNER SHALL NOT RENOVATE, REPAIR, ADD-ON TO, OR OTHERWISE MODIFY THE EXISTING STRUCTURAL SYSTEMS WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER.
- Q. CONTRACT DRAWINGS SHOW MAJOR OPENINGS IN FLOORS AND WALLS AND DO NOT NECESSARILY SHOW ALL OPENINGS REQUIRED. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS BETWEEN ALL DISCIPLINES AND TRADES. ADDITIONAL DRAININGS, BLOCKOUTS, AND SLEEVES MAY BE REQUIRED AND SHALL BE CONSTRUCTED USING THE TYPICAL DETAILS AND/OR REQUIREMENTS WITHIN THE CONTRACT DOCUMENTS. OPENINGS REQUIRED, BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- R. THE CONTRACTOR SHALL COORDINATE PIPING AND CONDUIT EMBEDDED IN OR ATTACHED TO SLABS, SLAB-ON-DECK, BEAMS, AND COLUMNS. ANY REQUIRED MODIFICATIONS TO STRUCTURAL MEMBERS OR THEIR REINFORCEMENT AS A RESULT OF EMBEDMENT OR ATTACHMENT MUST BE SUBMITTED TO THE DESIGN TEAM FOR THEIR REVIEW. SEE GENERAL STRUCTURAL NOTES SECTION "DESIGN CRITERIA" FOR LIMITATIONS OF MEP LOADING ON STRUCTURAL SYSTEMS.
- S. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE INSTALLATION OF ALL CONDUITS, PIPING, ELECTRICAL GANG BOXES, MOUNTING PLATES, DOOR FRAMES, ETCETERA, ASSOCIATED WITH THE INSTALLATION OF OTHER WORK IN THE FLOORS, WALLS, COLUMNS, FOUNDATIONS, ETCETERA.
- T. THE STRUCTURAL ENGINEER OF RECORD'S ROLE DURING CONSTRUCTION
- THE STRUCTURAL ENGINEER OF RECORD SHALL NOT ASSUME CONTROL OF, OR RESPONSIBILITY FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, PROJECT SAFETY, THE ACTS AND OMISSIONS OF THE CONTRACTOR, OR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS
 - STRUCTURAL ENGINEER OF RECORD SHALL NOT HAVE AUTHORITY TO STOP THE WORK OR AUTHORIZE CHANGES TO ANY CONTRACT SUM
 - PERIODIC SITE VISITS BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER OF RECORD ARE SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS LIMITED OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS AND THAT OBSERVATIONS ARE QUALITATIVE, NOT QUANTITATIVE. THIS LIMITED INFORMATION WILL BE USED TO ADVISE THE OWNER/CONTRACTOR/ARCHITECT OF POTENTIAL DEFICIENCIES.
- U. CLARIFICATION OF POSITION OF STRUCTURALLY FRAMING ELEMENTS
- USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS, DO NOT SCALE ANY DIMENSIONS.
 - IF NOT INDICATED ON DRAWINGS, ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS.
 - CENTER LINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTION, UNLESS NOTED OTHERWISE.
 - CENTER LINES OF FOOTINGS, GRADE BEAMS, AND WALLS SHALL COINCIDE WITH CENTER LINES OF FOUNDATIONS, UNLESS NOTED OTHERWISE.
 - CENTER LINES OF FRAMING MEMBERS SHALL COINCIDE WITH COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.
 - ELEVATIONS SHOWN ARE TO TOP OF FOUNDATIONS, SLABS, OR BEAMS, UNLESS NOTED OTHERWISE.
- V. SEE ARCHITECTURAL, CIVIL, MEP, AND VERTICAL TRANSPORTATION CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION RELATING TO THE COORDINATION OF STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO:
- CIVIL
 - SITING OF BUILDING GRID LINES WITH RESPECT TO CITY BENCHMARKS
 - SITE PREPARATION
 - BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
 - SITE ELEMENTS OUTSIDE OF BUILDING ENVELOPE
 - NEW AND EXISTING SITE UTILITIES
 - ARCHITECTURAL
 - PLAN DIMENSIONS AND PROJECT DATUM
 - SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
 - WATERPROOFING AND DAMP PROOFING DETAILS
 - SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
 - EMBEDMENTS, INSERTS, BLOCKOUTS, ETC.
 - CONCRETE FINISHES AND TOPPING SLABS
 - CONCRETE CURBS AND HOUSEKEEPING PADS
 - INTERIOR NON-STRUCTURAL MASONRY PARTITIONS
 - LIFE SAFETY, FIRE RATING
 - METAL PAN STAIRS AND SUPPORTS
 - OPERABLE PARTITIONS
 - MEP
 - PIPE AND DUCT SIZES FOR OPENING AND SLEEVE COORDINATION
 - FLOOR DRAINS
 - UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS
 - EQUIPMENT CURBS
 - CONDUITS AND EMBEDMENTS IN WALLS AND SLABS
 - VERTICAL TRANSPORTATION
 - INSERTS, HANGERS, TRENCHES, PITS, CONDUITS IN WALLS AND SLAB
- W. THIS BUILDING DOES NOT QUALIFY AS A THRESHOLD BUILDING PER CHAPTER 553.71 OF THE FLORIDA STATUTES. HOWEVER, THE OWNER HAS ELECTED TO PERFORM SPECIAL INSPECTIONS ABOVE AND BEYOND THOSE REQUIRED BY THE AUTHORITY HAVING JURISDICTION. SEE SPECIAL INSPECTION PLAN FOR SPECIFIC INSPECTION REQUIREMENTS.
- X. OWNER SHALL PROVIDE MATERIAL AND CONCRETE TESTING ENGINEER. CONTRACTOR SHALL COORDINATE. REFER TO SPECIAL INSPECTIONS PLAN.

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- ELECTRONIC DATA/REPRODUCTION**
- A. ALL INFORMATION CONTAINED IN THE ELECTRONIC FILES OF THE CONTRACT DOCUMENTS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT, OR THE COMPLETION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
- B. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZE, AND QUANTITIES.
- C. DIMENSIONS AND ELEMENT SIZES AND LOCATIONS IN THE ELECTRONIC FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.
- D. WHEN USED FOR THE PREPARATION OF SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACTOR SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.
- SUBMITTALS**
- A. REFER TO DIVISION 01 OF SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT.
- B. SUBMIT SPECIFIC COMPONENTS SUCH AS COLUMNS, FOUNDATIONS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
- C. TEN WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT, FOR REVIEW AND COMMENT BY THE STRUCTURAL ENGINEER OF RECORD, A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE STRUCTURAL ENGINEER OF RECORD. THE STRUCTURAL ENGINEER OF RECORD SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWING SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE STRUCTURAL ENGINEER OF RECORD WILL RETURN THE SHOP DRAWING ITEMS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE REPRODUCIBLE SHOP DRAWING.
- D. THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR SHALL STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED:
- THE SUBMITTAL IS REQUESTED.
 - THE SUBMITTAL IS BASED ON THE LATEST DESIGN.
 - THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL THE DIFFERENCES FROM THE CONTRACT DOCUMENTS ON THE FIRST SUBMITTAL.
 - THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL CHANGES AND ADDITION FROM PREVIOUS SUBMITTAL.
 - THE ARCHITECT'S AND STRUCTURAL ENGINEER OF RECORD'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
 - THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
 - THE SUBMITTAL IS COMPLETE.
 - THE SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.
- E. THE STRUCTURAL ENGINEER OF RECORD'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT.
- F. THE STRUCTURAL ENGINEER OF RECORD SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS.
- G. FOR THE COMPONENTS DESIGNED BY A DELEGATED ENGINEER, PROVIDE SHOP DRAWINGS, DESIGN CALCULATIONS, AND A COVER LETTER SIGNED AND SEALED BY THE DELEGATED ENGINEER. LETTER SHALL INDICATE THAT THE SHOP DRAWINGS ARE IN CONFORMANCE WITH THE DELEGATED ENGINEER'S CALCULATIONS. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDITIONAL REQUIREMENTS.
- H. DEFERRED SUBMITTALS ARE MANUFACTURER OR CONTRACTOR DESIGNED COMPONENTS PER THE CONTRACT DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD, WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE CONTRACTOR SHALL SUBMIT THESE REVIEWED DEFERRED SUBMITTAL DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL. THESE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN TEAM HAS REVIEWED AND THE BUILDING OFFICIAL HAS APPROVED. SEE BELOW FOR THE LIST OF DEFERRED SUBMITTALS.
- I. A SUBMITTAL SHOWING THE COORDINATED PANELS WITH MEP SLEEVES ALONG WITH THE FOLLOWING SUBMITTALS ARE REQUIRED TO BE SUBMITTED FOR STRUCTURAL ENGINEER OF RECORD REVIEW AS OUTLINED IN THE SPECIFICATIONS:
- | | | |
|--------|---|---------------------------|
| 031000 | CONCRETE FORMWORK | (SS, CALC) |
| 032000 | CONCRETE REINFORCEMENT LAYOUT | (S) |
| 033000 | CONCRETE MIX DESIGNS | (CALC, TA) |
| 033000 | CONCRETE CONSTRUCTION JOINT LAYOUT | (S) |
| 034713 | TILT-UP CONCRETE PANEL | (DF, SS, CALC) |
| 034713 | TILT-UP CONCRETE PANEL LIFTING | (DF, SS, CALC) |
| 051200 | STRUCTURAL STEEL | (S) |
| 051200 | STRUCTURAL STEEL CONNECTIONS | (DF, S, CALC) |
| 051200 | SHEAR STUD LAYOUT | (S) |
| 052100 | STEEL JOISTS, BRIDGING, AND CONNECTIONS | (DF, SS, CALC) |
| 053100 | STEEL COMPOSITE DECK | (S) |
| 053100 | STEEL ROOF DECK | (S) |
| 133419 | METAL BUILDING
HANDRAIL, GUARDRAIL, RAILING | (DF, SS, CALC) (SEE ARCH) |
| 142000 | ELEVATOR
SURVEY OF STRUCTURAL STEEL ERECTION | (SS, CALC, REC) |
| 312319 | DEWATERING | (SS) |
| | | (GEO, REC) |
- S = SHOP DRAWING REQUIRED
DF = DEFERRED SUBMITTAL
SS = SIGNED AND SEALED SHOP DRAWINGS PREPARED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED
CALC = SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED
REC = ITEMS SUBMITTED FOR RECORD ONLY AND WILL NOT HAVE STRUCTURAL ENGINEER OF RECORD SHOP DRAWING STAMP AFFIXED
GEO = ITEMS SUBMITTED TO CONSTRUCTION GEOTECHNICAL ENGINEER FOR THEIR REVIEW.
TA = ITEMS SUBMITTED TO OWNER'S TESTING AGENCY FOR THEIR REVIEW.
- GOVERNING CODES & STANDARDS**
- | | | |
|----------------|----------|--|
| BUILDING CODE: | FBC 2023 | FLORIDA BUILDING CODE, BUILDING |
| STANDARDS: | ASCE 7 | AMERICAN SOCIETY OF CIVIL ENGINEERS: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES |
| | ACI 318 | AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE |
| | AISC 360 | AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS |
| | AWS D1.1 | AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL |
| | AWS D1.3 | AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - SHEET STEEL |
| | AWS D1.4 | AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - REINFORCING STEEL |
| | ASTM | AMERICAN SOCIETY FOR TESTING AND MATERIALS |

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- DESIGN CRITERIA**
- A. STRUCTURE LOCATION:
LONGITUDE: -80.531
LATITUDE: 27.630
- B. LOADING:
- SUPERIMPOSED DEAD & LIVE LOADS:
ALL DEAD LOADS LISTED BELOW ARE IN ADDITION TO THE STRUCTURE'S SELF-WEIGHT.

ROOF:	DEAD LOAD	LIVE LOAD
TYPICAL	10 PSF	20 PSF REDUCIBLE
FLOOR:		
OFFICE	20 PSF	80 PSF REDUCIBLE
CORRIDORS	10 PSF	100 PSF NON-REDUCIBLE
STAIRS	10 PSF	100 PSF NON-REDUCIBLE
STORAGE	20 PSF	125 PSF NON-REDUCIBLE
MECHANICAL ROOM	150 PSF	125 PSF NON-REDUCIBLE
 - RAIN LOAD:
DESIGN RAINFALL: 4.44"/HOUR (100-YEAR, 1-HOUR RAINFALL)
RAINFALL AT LOWEST POINT OF ROOF SHALL NOT POND DURING DESIGN RAINFALL AND SHALL NOT EXCEED 3 1/8" DURING DESIGN RAINFALL
DESIGN RAIN LOAD, R: 20 PSF
 - SNOW LOAD:
GROUND SNOW LOAD, P_g: 0 PSF
 - WIND LOAD:
ULTIMATE DESIGN WIND SPEED, V_{ult}: 226 MPH
MINIMUM DESIGN WIND SPEED, V_{min}: 115 MPH
RISK CATEGORY: IV
WIND EXPOSURE CATEGORY: C
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: +/- 0.18
COMPONENTS & CLADDING DESIGN PRESSURES: SEE WIND PRESSURE DIAGRAMS
 - TORNADO LOAD:
RISK CATEGORY: IV
EFFECTIVE PLAN AREA: LESS THAN 40,000 FT²
ULTIMATE TORNADO SPEED, V_{wt}: 73 MPH
V_{wt} 73 MPH < 0.6 V_{wt} = 136 MPH
PER 32.2.2 OF ASCE 7, STRUCTURES WITH A V_{wt} < 0.6 V_{wt} ARE EXEMPT FROM TORNADO LOAD CONSIDERATIONS.
 - SEISMIC LOAD:
RISK CATEGORY: IV
SEISMIC IMPORTANCE FACTOR, I_p: 1.5
SITE CLASS: D
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_s: 0.052 g
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S₁: 0.029 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{0.2}: 0.055 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{0.1}: 0.047 g
SEISMIC DESIGN CATEGORY: A
SEISMIC FORCE RESISTING SYSTEM: NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE DESIGN BASE SHEAR: 2 KIPS
- C. HANDRAIL AND GUARDRAIL LOADS:
CONCENTRATED AND DISTRIBUTED LOADS ARE TO BE APPLIED AT THE HANDRAIL OR TOP RAIL IN ANY DIRECTION. CONCENTRATED AND DISTRIBUTED LOADS ARE NOT TO BE APPLIED CONCURRENTLY.
CONCENTRATED LOAD 200 LB
DISTRIBUTED LOAD 50 PLF
- D. FUTURE EXPANSION:
NO PROVISIONS HAVE BEEN MADE FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION OF THE STRUCTURE.
- E. SERVICEABILITY:
- DEFLECTION LIMITS: TOTAL LOAD DEFLECTION ONLY APPLIES TO THE DEFLECTION DUE TO THE CREEP COMPONENT OF LONG-TERM DEAD LOAD DEFLECTION PLUS THE SHORT-TERM DEFLECTION. LONG-TERM DEFLECTION OF WOOD STRUCTURAL MEMBERS SHALL BE CALCULATED IN ACCORDANCE WITH THE AWC NDS. IT IS PERMITTED TO ESTIMATE THE CREEP-COMPONENT OF THE LONG-TERM DEFLECTION AS THE IMMEDIATE DEAD LOAD DEFLECTION.
 - ROOF MEMBERS
 - TOTAL LOAD DEFLECTION: L/240
 - TRANSITORY LOAD DEFLECTION: L/360
 - FLOOR MEMBERS
 - TOTAL LOAD DEFLECTION: L/240
 - LIVE LOAD DEFLECTION: L/360
 - DRIFT LIMITS
 - INTERSTORY DRIFT: L/600
 - TOTAL STRUCTURE DRIFT: L/600
- EARTHWORK & FOUNDATIONS**
- A. GEOTECHNICAL INVESTIGATION REPORT
- FOUNDATION DESIGN IS BASED ON THE PRESUMPTIVE LOAD BEARING VALUES OF SOIL IN THE BUILDING CODE.
- B. SHALLOW FOUNDATIONS
- FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
 - FOUNDATION SIZES AND REINFORCEMENT ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 2000 PSF PER THE PRESUMPTIVE LOAD BEARING VALUES OF SOIL IN THE BUILDING CODE.
 - FOUNDATIONS SHALL BEAR A MINIMUM OF 1'-4" BELOW ADJACENT EXTERIOR GRADE.
 - FOUNDATIONS SHALL BEAR ON COMPACTED STRUCTURAL FILL, NATURAL SOILS, OR ROCK PREPARED PER THE GEOTECHNICAL INVESTIGATION REPORT.
 - PRIOR TO PLACEMENT OF CONCRETE, A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY SOILS CONFORMANCE TO THE RECOMMENDATIONS AND ASSUMPTIONS IN THE GEOTECHNICAL INVESTIGATION REPORT. ALL ADVERSE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT/ STRUCTURAL ENGINEER OF RECORD.
 - SOILS BELOW FOUNDATIONS NOT MEETING DESIGN BEARING PRESSURE SHALL BE REMEDIATED PER THE GEOTECHNICAL INVESTIGATION REPORT AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATIONS.
 - CENTER ALL FOUNDATIONS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, UNLESS NOTED OTHERWISE.
 - TOP OF FOUNDATION ELEVATIONS PROVIDED ON THE CONTRACT DRAWINGS ARE FOR PURPOSE OF THE CONTRACT AND SHALL BE ADJUSTED, AS REQUIRED, AT THE TIME OF EXCAVATION TO BEAR ON PROPERLY PREPARED SUPPORT SUBGRADE (PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS).
- C. EARTH-WORK AND EXCAVATION
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING, BUT NOT LIMITED TO: LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS.
 - EXCAVATION SHALL NOT OCCUR WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROTECTED AGAINST SETTLEMENT.
 - THE EXTENT OF SUBGRADE PREPARATION SHALL EXTEND A MINIMUM OF 5'-0" BEYOND THE BUILDING PERIMETER.
 - THE CONTRACTOR SHALL PROVIDE A SUBGRADE BENEATH THE SLAB-ON-GROUND PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
 - UNLESS NOTED IN THE GEOTECHNICAL INVESTIGATION REPORT, COMPACT FILL TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER SHALL NOT EXCEED 8" LOOSE THICKNESS. COMPACT PRIOR TO THE PLACEMENT OF THE NEXT LAYER. COMPACTION SHALL MEET ALL RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT.
 - PLACEMENT OF FILL AND COMPACTION SHALL BE MONITORED AND ACCEPTED BY A RETAINED TESTING AGENCY. PERFORM A MINIMUM OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-6938) FOR EVERY 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.
 - THE CONTRACTOR SHALL DETERMINE THE EXTENT OF THE CONSTRUCTION DEWATERING SYSTEMS REQUIRED FOR THE EXCAVATION. AT A MINIMUM, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING SITE.
 - THE CONTRACTOR SHALL SUBMIT CONSTRUCTION DEWATERING PLAN TO THE GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO BEGINNING EXCAVATION.
 - THE CONTRACTOR SHALL INSTALL ALL NECESSARY DEWATERING SYSTEMS.
- D. RETAINING WALL
- RETAINING WALLS SHALL BE DESIGNED FOR THE FOLLOWING:
 - LATERAL EARTH PRESSURE AT REST: 83.45 PSF PER FOOT OF DEPTH
 - ACTIVE EARTH PRESSURE: 67.22 PSF PER FOOT OF DEPTH
 - PASSIVE EARTH PRESSURE RESISTANCE: 180 PSF PER FOOT OF DEPTH
 - SOIL FRICTION FACTOR: 0.55
 - RETAINING WALL DESIGN IS BASED ON DRAINED BACKFILL WITH NO BUILDUP OF WATER. THE CONTRACTOR SHALL PROVIDE A DRAINAGE SYSTEM IN ALL BACKFILL CONDITIONS (SEE CIVIL/ ARCHITECTURAL DRAWINGS FOR DRAINAGE SPECIFICATIONS).
 - DO NOT BACKFILL AGAINST CANTILEVERED RETAINING WALLS UNTIL CONCRETE COMPRESSIVE STRENGTH, F_c, REACHES ITS 28 DAY DESIGN STRENGTH. DO NOT BACKFILL PRIOR TO WATERPROOFING AND INSPECTION.

3

- SLAB-ON-GROUND**
- A. THE SLAB-ON-GROUND HAS BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
- B. SLAB THICKNESS AND REINFORCEMENT ARE BASED ON A MODULUS OF SUBGRADE REACTION OF 150 PCI PER THE GEOTECHNICAL INVESTIGATION REPORT.
- C. SUBGRADE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT. PROVIDE TERMITE TREATMENT BELOW SLAB-ON-GROUND.
- D. FOR INTERIOR SLABS, PLACE A 10-MIL (MINIMUM) VAPOR RETARDER BETWEEN THE SOIL AND BOTTOM OF SLAB. SEE CAST-IN-PLACE CONCRETE SPECIFICATIONS FOR APPROVED VAPOR RETARDER PRODUCTS/MANUFACTURERS. DO NOT USE VAPOR RETARDERS AT EXTERIOR SLABS. SEE ARCHITECTURAL CONTRACT DOCUMENTS FOR PROJECT SPECIFIC REQUIREMENTS.
- E. IF THE SLAB-ON-GROUND HAS BEEN DESIGNATED AS A STRUCTURAL SLAB-ON-GROUND IN THE CONTRACT DOCUMENTS, NO SAW CUTTING OF THE SLAB IS PERMITTED.
- F. CONTROL JOINTS SHALL BE CUT INTO THE SURFACE OF THE SLAB, IN EACH DIRECTION. SEE THE TYPICAL SAW CUT JOINT DETAIL FOR THE DEPTH, AND SPACING OF JOINT REQUIREMENTS UNLESS NOTED OTHERWISE. CONTROL JOINTS SHALL BE CONSTRUCTED SUCH THAT THE AREA CONTAINED BY THE CONTROL JOINTS HAS A MAXIMUM RATIO OF LONG SIDE TO SHORT SIDE OF 1.5 TO 1 UNLESS NOTED OTHERWISE. DO NOT CONSTRUCT CONTROL JOINTS SUCH THAT L-SHAPED SLAB PANELS ARE CREATED.
- G. COLUMN ISOLATION JOINTS SHALL BE CONSTRUCTED PER THE TYPICAL COLUMN ISOLATION JOINT DETAIL IN ORDER TO PROVIDE ADEQUATE SPACE FOR COLUMN INSTALLATION.
- H. CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. SLAB CONSTRUCTION JOINTS SHALL BE DOWELED.
- I. WHERE SPECIFIED ON PLAN, WELDED WIRE REINFORCEMENT SHALL BE INSTALLED. WELDED WIRE REINFORCEMENT SHALL BE PROPERLY CHAIRED SUCH THAT IT IS LOCATED AT A DEPTH OF 1 1/2" FROM THE TOP OF SLAB.
- J. REFERENCE ARCHITECTURAL AND MEP DOCUMENTS FOR SLAB FINISHES AND SLOPES NOT REFERENCED ON THE STRUCTURAL DOCUMENTS. THE MINIMUM SLAB THICKNESS SPECIFIED IN THE CONTRACT DOCUMENTS MUST BE MAINTAINED.
- K. REFERENCE ARCHITECTURAL DOCUMENTS FOR VAPOR RETARDER AND SLAB AND CONTROL JOINT SEALANT REQUIREMENTS.
- L. CONDUITS SHALL NOT BE PLACED WITHIN THE SLAB. CONDUITS SHALL BE PLACED BENEATH THE SLAB.
- CONCRETE REINFORCEMENT**
- A. ALL CONCRETE REINFORCEMENT SHALL BE INSTALLED IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
- B. ALL REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- C. WHERE WELDS ARE INDICATED FOR REINFORCING STEEL ON THE DRAWINGS, REINFORCING STEEL SHALL BE A706, GRADE 60 UNLESS OTHERWISE NOTED.
- D. WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE MATERIAL REQUIREMENTS OF ASTM A1064.
- E. ALL 90°, 135°, AND 180° HOOKED REINFORCEMENT SPECIFIED AND GRAPHICALLY DEPICTED IN THE CONTRACT DOCUMENTS SHALL BE DETAILED IN ACCORDANCE WITH ACI 318 STANDARD HOOK GEOMETRY FOR DEFORMED BARS IN TENSION AND FOR STIRRUPS, TIES, AND HOOPS.
- F. FOR EVERY VERTICAL OR HORIZONTAL BAR CONTINUED BY AN OPENING, ONE BAR (MINIMUM OF 2 BARS) SHALL BE ADDED AT SIDE OF OPENING (HALF TO EACH SIDE, TYPICAL).
- G. FOR CONCRETE CLEAR COVER TO REINFORCEMENT SEE SCHEDULE ON SHEET S-003 UNLESS NOTED OTHERWISE. CLEAR COVER IN PARENTHESES () DENOTES CLEAR COVER WHEN THE AS-BUILT APPLICATION IS EXPOSED TO WEATHER.
- H. ALL LAP SPLICES SHALL BE CLASS B TENSION LAP SPLICES IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE. SEE LAP SPLICE SCHEDULE ON SHEET S-003 FOR LAP SPLICE LENGTHS, UNLESS NOTED AS CONTINUOUS. REINFORCEMENT SHALL ONLY BE SPLICED AT LOCATIONS SHOWN ON THE CONTRACT DOCUMENTS. SPLICES AT NON-SPECIFIED LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- I. A MINIMUM LAP SPLICE OF 8" SHALL BE PROVIDED AT ALL END AND SIDE LAP CONDITIONS FOR WELDED WIRE REINFORCEMENT UNLESS NOTED OTHERWISE.
- J. MECHANICAL SPLICES ARE REQUIRED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. MECHANICAL SPLICES ARE ALSO REQUIRED TO SPlice #14 AND #18 BARS. MECHANICAL SPLICES MAY ALSO BE USED AT THE CONTRACTOR'S OPTION, PROVIDED THE MECHANICAL SPLICES HAVE A CURRENT ICC-ES REPORT DEMONSTRATING THEY CAN DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION OR COMPRESSION. MECHANICAL SPLICES SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- K. THE USE OF WELDED SPLICES IS PROHIBITED UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL SUBMIT THE LOCATIONS OF WELDED SPLICES TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. IF APPROVED, WELDED SPLICES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.
- L. DOWELS SHALL MATCH SIZE AND SPACING OF PRIMARY REINFORCEMENT UNLESS NOTED OTHERWISE.
- M. SEE TYPICAL DETAILS FOR REINFORCEMENT BARS, ELEVATIONS AND PENETRATIONS.
- N. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCEMENT BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
- CONCRETE**
- A. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
- B. FOR CONCRETE MIXTURE REQUIREMENTS SEE SCHEDULE ON SHEET S-003.
- C. THE USE OF RECYCLED CONCRETE IS PROHIBITED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
- D. NORMAL WEIGHT CONCRETE SHALL BE USED FOR ALL CONCRETE MEMBERS UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE SHALL HAVE A CURED DENSITY OF 145 PCF ±5 PCF. WHERE LIGHT WEIGHT CONCRETE IS SPECIFIED THE CURED DENSITY SHALL BE 112 PCF ±3 PCF.
- E. EACH MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER AND THE INTENDED LOCATION OF PLACEMENT ON THE SPECIFIC PROJECT SHALL BE CLEARLY STATED.
- F. ALL PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN BEAMS, WALLS, AND SLABS UNLESS SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS OR BY WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD. FOR MILD REINFORCED MEMBERS, CONSTRUCTION JOINTS SHALL OCCUR WITHIN THE MIDDLE THIRD OF A MEMBER'S SPAN. ALL APPROVED CONSTRUCTION JOINTS SHALL BE INDICATED, DIMENSIONED, AND DETAILED ON THE CONCRETE REINFORCEMENT SHOP DRAWINGS.
- G. GIRDERS, BEAMS, HAUNCHES, DROP PANELS, DROP CAPS, AND CAPITALS SHALL BE POURED MONOLITHICALLY AS PART OF THE SLAB ON GROUND UNLESS NOTED OTHERWISE.
- H. PROVIDE A ¼ INCH CHAMFER AT ALL EXPOSED CORNERS OF BEAMS, WALLS, ETC UNLESS NOTED OTHERWISE.
- I. CONCRETE CORING AND INSTALLATION OF DRILLED ANCHORS IS NOT PERMITTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
- J. REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL COORDINATE BETWEEN THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS TO FURNISH DIMENSIONED DRAWINGS TO LOCATE AND SIZE ALL SLAB EDGES, OPENINGS, AND PENETRATIONS. THESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- K. EMBEDDED CONDUITS, PIPES, AND SLEEVES
- THE OUTSIDE DIAMETER OF CONDUITS, PIPES, AND SLEEVES SHALL NOT EXCEED ONE-THIRD THE THICKNESS OF THE SLAB, WALL, OR BEAM IN WHICH THEY ARE EMBEDDED. EMBEDMENTS SHALL NOT SIGNIFICANTLY REDUCE THE CAPACITY OF THE MEMBERS THEY PENETRATE.
 - THE MINIMUM CLEAR COVER FOR CONDUITS, PIPES, AND SLEEVES SHALL BE 1 ½" FOR CONCRETE EXPOSED TO EARTH OR WEATHER AND ¾" FOR CONCRETE NOT EXPOSED TO EARTH OR WEATHER.
 - ALUMINUM EMBEDMENTS AND EMBEDMENTS MADE OF ANY OTHER MATERIAL HARMFUL TO THE CONCRETE OR REINFORCEMENT ARE PROHIBITED.
 - EMBEDMENTS NOT SHOWN ON THE CONTRACT DOCUMENTS SHALL BE DESIGNED TO RESIST THE EFFECTS OF MATERIAL, PRESSURE, AND TEMPERATURE THAT THEY WILL BE SUBJECTED TO. THE WORK SHALL BE COORDINATED AMONGST ALL CONSTRUCTION TRADES.
 - THE CONTENTS OF EMBEDDED PIPES SHALL NOT FLOW UNTIL THE CONCRETE HAS REACHED ITS SPECIFIED DESIGN STRENGTH.
 - CONDUITS, PIPES, AND SLEEVES SHALL BE PLACED BETWEEN TOP AND BOTTOM LAYERS OF REINFORCEMENT IN SLABS AND BETWEEN INNER AND OUTER LAYERS OF REINFORCEMENT IN WALLS.
 - EMBEDDED ITEMS SHALL BE FABRICATED AND INSTALLED SUCH THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS SPECIFIED LOCATION IS NOT REQUIRED.

STRUCTURAL DRAWING LIST	
SHEET NUMBER	SHEET NAME
e-S001	GENERAL NOTES
e-S002	GENERAL NOTES
e-S003	GENERAL SCHEDULES
e-S004	SPECIAL INSPECTION PLAN
e-S101	FOUNDATION PLAN
e-S201	MEZZANINE FRAMING PLAN
e-S202	ROOF FRAMING PLAN
e-S301	FOUNDATION SECTIONS & DETAILS
e-S302	FOUNDATION SECTIONS & DETAILS
e-S303	FOUNDATION SECTIONS & DETAILS
e-S401	FRAMING SECTIONS & DETAILS
e-S402	FRAMING SECTIONS & DETAILS
e-S403	FRAMING SECTIONS & DETAILS
e-S404	FRAMING SECTIONS & DETAILS



TO THE BEST OF THE ENGINEERS' KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

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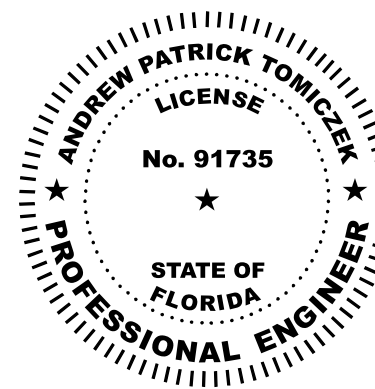
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ISC STORES / PGD WAREHOUSE
BUILDING E
COLUMBIA COUNTY, FLORIDA**



03/18/2026

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ANDREW P TOMICZEK, P.E.
ON THE DATE ADJACENT TO THE SEAL.

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FL PE No. 91735
DESIGNED BY
JEZERINAC GROUP
DRAWN BY
CN
PROJECT NUMBER
C09414.003
DATE
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THE APPLICABLE BUILDING CODE
AND MATERIAL SPECIFICATION

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GENERAL NOTES

DRAWING NO.

e-S002

- A. DESIGN, CONSTRUCTION, AND ERECTION OF TILT-UP CONCRETE PANELS AND THEIR CONNECTIONS TO ONE ANOTHER SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS, ACI 318 AND ACI 551.
- B. CONTRACTOR SHALL RETAIN A DELEGATED ENGINEER FOR THE DESIGN OF ALL TILT-UP CONCRETE PANELS, COLUMNS, AND ROOFS AS DETAILED IN THE CONTRACT DOCUMENTS.
- C. DELEGATED ENGINEER SHALL REFER TO ARCHITECTURAL DRAWINGS FOR PANEL SIZES AND LAYOUT. PROVIDE WALL JOINTS TO ALLOW FOR THE PROPER EXPANSION AND CONTRACTION OF WALLS. ALL PANEL EDGES SHALL BE CHAMFERED, UNLESS NOTED OTHERWISE.
- D. TILT-UP CONCRETE PANELS SHALL BE DESIGNED TO RESIST APPLICABLE GRAVITY AND WIND LOADS. TILT-UP CONCRETE PANELS SHALL BE DESIGNED WITH ADEQUATE CAPACITY TO SPAN BETWEEN BRACING LOCATIONS.
- E. BRACING FOR TILT-UP CONCRETE PANELS SHALL NOT BE REMOVED UNTIL ALL PERMANENT CONNECTIONS ARE COMPLETED AND HAVE BEEN INSPECTED BY THE TESTING AGENCY.
- F. DELEGATED ENGINEER SHALL PROVIDE ALL ENGINEERING AND DETAILING OF THE TILT-UP CONCRETE PANELS, PANEL-TO-PANEL CONNECTIONS, PANEL-TO-STRUCTURAL STEEL COLUMN CONNECTIONS, AND PANEL-TO-FOUNDATION CONNECTIONS TO RESIST THE LOADS SPECIFIED IN THE GENERAL NOTES, WIND LOADING DIAGRAMS, BRACING PLAN, AND ERECTION SEQUENCE.
- G. DELEGATED ENGINEER TO DESIGN ALL HARDWARE FOR LIFTING AND BRACING AS REQUIRED. ADD ADDITIONAL REINFORCING AS REQUIRED BY DESIGN. DELEGATED ENGINEER SHALL PROPERLY CONSIDER STRESSES INDUCED DURING THE LIFTING PROCESS AND SHALL PROVIDE ANY ADDITIONAL REINFORCING REQUIRED FOR LIFTING AND ERECTION STRESSES.
- H. TILT-UP CONCRETE PANELS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT THE TIME OF ERECTION.
- I. DELEGATED ENGINEER SHALL PROVIDE COMPLETE FABRICATION AND ERECTION DRAWINGS, DRAWINGS AND CALCULATIONS SHALL BE PROVIDED TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD FOR REVIEW. DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY DELEGATED ENGINEER PRIOR TO REVIEW BY ARCHITECT AND ENGINEER OF RECORD.
- J. CONTRACTOR SHALL REMOVE AND REPAIR ALL INSERTS AFTER BRACING HAS BEEN REMOVED. REPAIR SHALL MEET ALL ARCHITECTURAL REQUIREMENTS FOR FINISHES.

A. POST-INSTALLED ANCHORS SHALL INCLUDE MECHANICAL, SCREW, AND ADHESIVE ANCHORS OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE STRUCTURAL DRAWINGS.

B. MECHANICAL ANCHORS (EXPANSION ANCHORS/EXPANSION BOLTS) INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:

1. KWIK BOLT T2 ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
2. STRONG-BOLT 2 ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
3. POWER-STEEL- SD2 ANCHORS MANUFACTURED BY DEWALT

C. MECHANICAL ANCHORS (EXPANSION ANCHORS/EXPANSION BOLTS) INTO EXISTING CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:

1. KWIK BOLT 3 ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
2. EDGE-ALL ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
3. POWER-STEEL- SD1 ANCHORS MANUFACTURED BY DEWALT

D. SCREW ANCHORS INTO EXISTING CONCRETE AND CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING PRODUCTS:

1. KWIK HUS EZ ANCHORS MANUFACTURED BY HILTI FASTENING SYSTEMS
2. TITENUD ANCHORS MANUFACTURED BY SIMPSON STRONGTIE COMPANY
3. SCREW-BOLT- ANCHORS MANUFACTURED BY DEWALT

E. ADHESIVE ANCHORS (EPOXY ANCHORS/DRILL & EPOXY) INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:

1. HIT-HY200 EPOXY ADHESIVE WITH HAS ROD MANUFACTURED BY HILTI FASTENING SYSTEMS
2. PURE-H110- EPOXY ADHESIVE MANUFACTURED BY DEWALT WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD

F. ADHESIVE ANCHORS (EPOXY ANCHORS/DRILL & EPOXY) INTO EXISTING CONCRETE MASONRY AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:

1. HIT-HY70 INJECTION ADHESIVE WITH HAS ROD MANUFACTURED BY HILTI FASTENING SYSTEMS
2. AT-XP ADHESIVE MANUFACTURED BY SIMPSON STRONGTIE COMPANY WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD
3. AT100- GOLD MANUFACTURED BY DEWALT WITH AN ALL-THREAD F1554 GRADE 36 STEEL ROD

G. ADHESIVE FOR ANCHORING REINFORCING BARS INTO EXISTING CONCRETE AS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE ONE OF THE FOLLOWING ADHESIVE PRODUCTS:

1. HIT-HY200 EPOXY ADHESIVE MANUFACTURED BY HILTI FASTENING SYSTEMS
2. PURE-H110- EPOXY ADHESIVE MANUFACTURED BY DEWALT

H. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

I. SUBSTITUTION REQUESTS FOR ALTERNATIVE PRODUCTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD WITH CALCULATIONS THAT ARE PREPARED AND SIGNED BY A LICENSED STRUCTURAL ENGINEER. CALCULATIONS SHALL SHOW THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE REFERENCED BUILDING CODE.

(E.G. WIND, SEISMIC, SUSTAINED TENSION, ETC). ALTERNATIVE PRODUCTS SUBMITTED SHALL INDICATE THAT ANCHORS ARE PERMITTED FOR RESISTING LOADS IN CRACKED CONCRETE. RESEARCH REPORTS SHALL BE ISSUED BY A SOURCE APPROVED BY THE AUTHORITY HAVING JURISDICTION.

K. LOCATE, BY NON-DESTRUCTIVE MEANS, ALL EXISTING REINFORCEMENT, AND AVOID DURING INSTALLATION OF ANCHORS, IF EXISTING REINFORCEMENT LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD IMMEDIATELY.

L. HOLES SHALL BE DRILLED AND CLEANED, AND ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN ANCHOR ADHESIVE. ADHESIVE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH.

M. HOT DIPPED GALVANIZED STEEL ANCHORS SHALL BE USED AT ALL EXTERIOR LOCATIONS AND WHERE SPECIFICALLY INDICATED ON THE DRAWINGS.

N. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HOLLOW CORE MASONRY. IF INSTALLATION INTO HOLLOW CORE MASONRY IS DESIRED, SUBMIT ALTERNATIVE PRODUCT FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD.

O. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HEAD JOINTS.

P. IN ADDITION TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, THE FOLLOWING GUIDELINES SHALL BE FOLLOWED FOR INSTALLATION OF ADHESIVE ANCHORS:

1. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT THE TIME OF INSTALLATION UNLESS HIGHER STRENGTH IS REQUIRED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
2. ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, AND DURING DRY CONDITIONS.
3. ADHESIVE ANCHORS SHALL BE INSTALLED IN HOLES PREDRILLED WITH A CARBIDE TIPPED DRILL BIT.
4. ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE RANGE SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, BUT NOT OUTSIDE OF THE DESIGN TEMPERATURE RANGE. LOADS SHALL NOT BE APPLIED TO ADHESIVE ANCHORS UNTIL THE FULL CURING TIME ASSOCIATED WITH THE INSTALLATION TEMPERATURE HAS ELAPSED.

Q. INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT.

R. SPECIAL INSTRUCTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND EVALUATION REPORTS, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

S. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCULATED TO RESIST SUSPENDED TENSILE LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.

T. CONTINUOUS INSPECTION SHALL BE PROVIDED FOR ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCULCATED ORIENTATIONS TO RESIST SUSPENDED TENSILE LOADS.

U. ADHESIVE ANCHORS SHALL BE PROOF TESTED AS REQUIRED IN THE SPECIFICATIONS. EACH TYPE AND SIZE OF ANCHOR SHALL BE PROOF TESTED IN TENSION BY AN INDEPENDENT TESTING LABORATORY. TENSION TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM A488 AND ACI 355.4. THE INDEPENDENT TESTING LABORATORY SHALL SUBMIT AN ANCHORAGE TESTING PLAN TO THE STRUCTURAL ENGINEER OF RECORD TO ENSURE THE TESTING REQUIREMENTS ARE FULFILLED.

V. TENSILE PROOF LOADING PERCENTAGE OF ANCHORS TO BE TESTED ARE INDICATED ON 0.03 PROOF LOADING SHALL BE PERFORMED AFTER THE 28-DAY CONCRETE CURING PERIOD AND AFTER THE MINIMUM EPOXY CURING PERIOD SPECIFIED BY THE MANUFACTURER. PROOF LOADING SHALL BE PERFORMED ON PRODUCTION ANCHORS; SACRIFICIAL ANCHORS SHALL NOT BE CONSIDERED ACCEPTABLE. MAINTAIN THE PROOF LOAD AT THE REQUIRED LOAD LEVEL FOR A MINIMUM OF 10 SECONDS.

W. ANCHORS SHALL HAVE NO VISIBLE INDICATIONS OF DISPLACEMENT OR DAMAGE DURING OR AFTER PROOF LOAD APPLICATION. CONCRETE CRACKING IN THE VICINITY OF THE ANCHOR AFTER LOADING SHALL BE CONSIDERED A FAILURE.

X. IF MORE THAN 10 PERCENT OF THE TESTED ANCHORS FAIL TO ACHIEVE THE SPECIFIED PROOF LOAD WITHIN THE LIMITS DEFINED IN THESE NOTES, 100% OF THE ANCHORS OF THE SAME DIAMETER AND TYPE AS THE FAILED ANCHOR SHALL BE PROOF TESTED.

Y. IN THE EVENT OF FAILURE TO ACHIEVE PROOF LOAD OR EXCESSIVE DISPLACEMENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRS TO THE CONCRETE.

Z. HOLE DRILLING AND INSTALLATION OF ADHESIVE ANCHORS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN CONCRETE DRY CONDITION.

7. SPECIAL INSPECTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE ANCHOR MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND/OR EVALUATION REPORTS, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

8. EVALUATION OF ANCHORS MUST BE CONDUCTED BY AN UPWARDLY INCLINED TO RESIST STASSTENSILE LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI/CESI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.

9. CONTINUOUS OBSERVATION SHALL BE PROVIDED FOR ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST STASSTENSILE LOADS.

10. ADHESIVE ANCHORS SHALL BE PROOF TESTED AS REQUIRED IN THE SPECIFICATIONS. EACH TYPE AND SIZE OF ANCHOR SHALL BE PROOF TESTED IN TENSION BY AN INDEPENDENT TESTING LABORATORY. TENSION TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM E488 AND ACI 308.4. THE INDEPENDENT TESTING LABORATORY SHALL SUBMIT AN ANCHORAGE TESTING PLAN TO THE STRUCTURAL ENGINEER OF RECORD TO ENSURE THE TESTING METHOD IS APPROPRIATE AND ADEQUATE.

11. TENSION PROOF LOADS AND PERCENTAGE OF ANCHORS TO BE TESTED ARE INDICATED ON 0.03. PROOF LOADING SHALL BE PERFORMED AFTER THE 28-DAY CONCRETE CURING PERIOD AND AFTER THE MINIMUM EPOXY CURING PERIOD SPECIFIED BY THE MANUFACTURER. PROOF LOADING SHALL BE PERFORMED ON PRODUCTION ANCHORS; REMOVAL OF ANCHORS IS NOT BE CONSIDERED ACCEPTABLE. MAINTAIN THE PROOF LOAD AT THE REQUIRED LOAD LEVEL FOR A MINIMUM OF 10 SECONDS.

12. ANCHORS SHALL HAVE NO VISIBLE INDICATIONS OF DISPLACEMENT OR DAMAGE DURING OR AFTER PROOF LOAD APPLICATION. CONCRETE CRACKING IN THE VICINITY OF THE ANCHOR AFTER LOADING SHALL BE CONSIDERED A FAILURE.

13. MORE THAN 10 PERCENT OF THE TESTED ANCHORS FAIL TO ACHIEVE THE SPECIFIED PROOF LOAD WITHIN THE LIMITS DEFINED IN THESE NOTES. 100% OF THE ANCHORS OF THE SAME DIAMETER AND TYPE AS THE FAILED ANCHOR SHALL BE PROOF TESTED.

14. IN THE EVENT OF FAILURE TO ACHIEVE PROOF LOAD OR EXCESSIVE DISPLACEMENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRS TO THE CONCRETE.

15. DRILLING AND INSTALLATION OF ADHESIVE ANCHORS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN CONCRETE DRY CONDITION.

STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS:

1.	ROLLED SHAPES AND CHANNELS:	ASTM A572 OR A992, MIN. YIELD STRENGTH 50 KSI
2.	ANGLE FOR TRUSSES AND BRACES:	ASTM A36 MIN YIELD STRENGTH 36 KSI
3.	MISCELLANEOUS ANGLES:	ASTM A36
4.	HOLLOW STRUCTURAL SECTIONS:	ASTM A500 GRADE C, MIN YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS

B. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:

1.	ANGLES:	ASTM A36
2.	WTS:	ASTM A992
3.	PLATES:	ASTM A572, MIN YIELD STRENGTH 50 KSI
4.	BOLTS:	ASTM A325
5.	NUTS:	ASTM A563
6.	WASHERS:	ASTM F436
7.	ANCHOR RODS:	ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENT S1
8.	WELD ELECTRODES:	MATCH FILLER METAL TO BASE METAL PER AWS D.1.1

C. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.

D. CANTILEVERED BEAMS WITH NATURAL MILL CAMBER SHALL BE ERECTED SUCH THAT THE CAMBER IS ORIENTED DOWNWARD (OR CONCEALED UP).

E. SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE S.E.R. IN WRITING.

F. FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH.

G. PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1/16" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.

H. SHOW ALL COPIES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR TO COMPLY WITH OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.

I. FIELD MODIFICATIONS OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.

J. WHERE BEAM SHEAR IS NOT NOTED, DESIGN FOR 10K.

K. ALL CONNECTIONS SHALL BE DESIGNED FOR THE SPECIFIED SHEAR, MOMENT, AND AXIAL LOADS ON THE DRAWINGS.

L. THE CONNECTIONS SHALL BE DESIGNED FOR LOAD REVERSAL. ALL CONNECTIONS FORCES SPECIFIED ON PLAN ARE ULTIMATE LEVEL FORCES UNLESS NOTED OTHERWISE.

M. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, AND FABRICATION OF ALL STEEL FRAMING.

CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATIONS AND THE BUILDING CODE. CONNECTIONS SHALL BE CAPABLE OF RESISTING VERTICAL AND HORIZONTAL LOADS LISTED ON THE DRAWINGS. CONNECTION DESIGN SHALL PROVIDE AN ADEQUATE LOAD PATH TO TRANSFER THE LOADS FROM EACH MEMBER, THROUGH THE CONNECTION, INTO THE SUPPORTING MEMBER, AND SHALL CONSIDER THE EFFECTS OF THE FORCES ON EACH MEMBER. PROVIDE STIFFENER PLATES, WEB DOUBLER PLATES, FLANGE CONTINUITY PLATES, ETC, AS REQUIRED. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS.

STEEL CONNECTION DETAILS SHOW GENERAL CRITERIA FOR DESIGN AND DETAILING, AND ARE NOT INTENDED TO SHOW COMPLETE CONNECTION CONFIGURATIONS OR OTHER SPECIFIC INFORMATION THAT ARE THE RESPONSIBILITY OF THE CONNECTION DESIGN ENGINEER. ALTERNATIVE CONNECTION CONFIGURATION MAY BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL. CONNECTIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS ARE TO BE FABRICATED AS SHOWN.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

A. ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION PER AMERICAN WELDING SOCIETY D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WELDING PROCEDURE SPECIFICATION SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.

B. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM TO K5).

C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, TAPERS, AND TRANSITIONS OF UNEQUAL PARTS.

D. WELDS SHALL BE DONE BY WELDERS WITH CURRENT AMERICAN WELDING SOCIETY CERTIFICATION.

E. FIELD WELDING SYMBOLS SHALL BE NECESSARILY BE INCLUDED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMERICAN WELDING SOCIETY D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.

F. ALL WELD SIZES SHALL BE THE LARGER OF: THE SIZE REQUIRED BY THE CONNECTION FORCES, MINIMUM SIZE PER ANSII/AWS D1.1 OR 3/16 INCH MINIMUM FILLET WELD, UNLESS NOTED OTHERWISE.

G. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE FORCE OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.

H. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED BY THE TESTING FIRM.

I. GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.

J. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATES LESS THAN OR EQUAL TO 1/4-INCH THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.

K. A RUN-OFF TAB SHALL BE USED AT ALL BEVEL AND FULL PENETRATION WELDS. RUN-OFF TABS SHALL BE REMOVED BY MEAT CUT AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.

L. WHERE REQUIRED BY DETAIL, REMOVE WELD BACKING BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.

M. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.

N. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARPY V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:

1. ASTM A588M HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2 INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING 2 INCHES IN THICKNESS: 20 FT-LB AT 70° FAHREHIT.
2. REHARDENED STEEL SHAPES, LATERAL BRACING SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDBS, BRACES), 20 FT-LB AT 70° FAHREHIT.
3. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50° FAHREHIT: 20 FT-LB AT CUMULATIVE TEMPERATURE + 20° FAHREHIT; 40° FAHREHIT MAXIMUM.
4. WELD METAL: 20 FT-LB AT -20° FAHREHIT AND 40 FT-LB AT 70° FAHREHIT.

5. TESTING IS TO BE IN ACCORDANCE WITH ASTM A588M, SUPPLEMENTARY REQUIREMENT S30, CHARPY V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES. ALTERNATE THE LOCATION, AT ROLLED SHAPES AND ASTM A573 FOR PLATES, FOR ANY OF THE LISTED LOCATIONS. WELD METAL SHALL BE TESTED IN ACCORDANCE WITH ASTM E23. STANDARD METHODS FOR NOTCHED BAR IMPACT TESTING OF METALLIC MATERIALS FOR WELD METAL.

A. MANUFACTURE AND ERECT ALL STRUCTURAL STEEL JOISTS, JOIST GIRDERS, AND BRIDGING IN ACCORDANCE WITH SPECIFICATION SECTION 052100. SPECIFICATIONS OF THE STEEL JOIST INSTITUTE AND ALL OSHA REQUIREMENTS.

B. JOIST MANUFACTURER SHALL DESIGN JOISTS PER LISTED DESIGN CRITERIA AND ANY ADDITIONAL LOADING SHOWN ON PLAN OR IN DETAILS. AT A MINIMUM, JOIST SHALL BE DESIGNED PER STEEL JOIST INSTITUTE (SJI) LOAD TABLES.

C. JOISTS SHALL BE EQUALLY SPACED BETWEEN COLUMN LINES OR OTHER SPECIFICALLY LOCATED FRAMING MEMBERS UNLESS NOTED OTHERWISE.

D. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR NET UPLIFT (NEGATIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO THE APPLICABLE BUILDING CODE LISTED IN DESIGN CRITERIA FOR LOAD COMBINATIONS.

1. DEAD LOAD (MIN) = 10 PSF

E. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR ADDITIONAL DOWN FORCE RESULTING FROM WIND (POSITIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS.

1. DEAD LOAD (MAXIMUM) = 30 PSF

F. JOIST MANUFACTURER SHALL DESIGN AND DETAIL ALL BRIDGING PER SJI REQUIREMENTS AND CLEARLY INDICATE LOCATION ON STEEL JOIST ERECTION DRAWINGS.

1. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR JOISTS NOT BRACED BY STEEL ROOF DECK.

2. BOTTOM CHORD OF ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT (COMPONENTS & CLADDING) SHOWN ON DIAGRAM ON S-003, UNLESS NOTED OTHERWISE.

G. AN ALLOWABLE STRESS INCREASE FOR LOAD COMBINATIONS INCLUDING WIND IS NOT PERMITTED.

H. JOIST MANUFACTURER SHALL CAMBER JOISTS PER SJI CRITERIA.

I. PROVIDE MINIMUM BEARING PER SJI REQUIREMENTS AND CONNECT TO STEEL SUPPORTS.

1. K-SERIES: 3/16" X 1" FILLET WELD EACH SIDE

2. LH AND DLH SERIES: 1/4" X 2" FILLET WELD EACH SIDE

J. PROVIDE STANDARD DEPTH OF BEARING FOR ALL JOISTS AS SHOWN BELOW UNLESS NOTED IN DRAWINGS:

1. K-SERIES = 2 1/2"

2. LH = 5"

3. JOIST GIRDER = 7 1/2"

K. WHERE STEEL JOIST OR JOIST GIRDER SLOPE EXCEEDS 1/4" INCH PER FOOT, PROVIDE SLOPED BEARING AS NOTED IN SLOPED SEAT REQUIREMENTS OF SJI.

L. JOIST MANUFACTURER SHALL DESIGN AND DETAIL FIELD BOLTING FOR ERECTION PER SJI REQUIREMENTS.

A. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE DIVISION 05 SPECIFICATIONS.

B. STEEL DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION), UNLESS NOTED OTHERWISE.

C. THE CONTRACTOR SHALL COORDINATE SLAB/DECK OPENING SIZES AND LOCATIONS PER ARCHITECTURAL AND MEP CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE OPENING SUPPORT FRAMING AND/OR REINFORCEMENT AS REQUIRED PER TYPICAL DETAILS AND SUBMIT PROPOSED SLAB/DECK OPENINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD.

D. SHOP DRAWINGS SHALL BE SUBMITTED INDICATING:

1. MATERIAL STRENGTH
2. SECTION PROPERTIES
3. DECK GAUGE, LAYOUT
4. FASTENER TYPE
5. CONNECTION PATTERN
6. CLOSURE ANGLES

E. THE CAPACITY OF THE DECK SHALL BE BASED ON CURRENT ICC-ES EVALUATION REPORTS.

COMPOSITE STEEL FLOOR DECK SHALL BE A MINIMUM YIELD STRENGTH OF 50 KSI AND SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE.

WHERE COMPOSITE BEAM DESIGNATIONS OCCUR ON FRAMING PLANS DURING THE CALLING OUT OF A NUMBER OF REQUIRED STEEL HEADED STUD ANCHORS, FLOOR AND BEAM ARE DESIGNED TO ACT COMPOSITELY BY USE OF SHEAR STUDS ON THE BEAM TOP FLANGE.

1. SEE DISTRIBUTED STEEL HEADED STUD ANCHOR PLACEMENT DETAIL FOR PLACEMENT REQUIREMENTS.

2. IF LONGITUDINAL STEEL HEADED STUD ANCHOR SPACING EXCEEDS 12 INCHES, PLUG WELD DECK TO BEAM BETWEEN STUDS.

COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:

1. AT BUTTED ENDS: AT 12 INCHES ON CENTER
2. AT PERIMETER EDGES OF BUILDING: AT 12 INCHES ON CENTER
3. INTERMEDIATE SUPPORTS: AT 12 INCHES ON CENTER
4. SIDE LAPS: AT MID-SPAN OR AT 3'-0" ON CENTER, WHICHEVER IS SMALLER. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUT NOT PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.

CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL.

DESIGN AND DETAIL FLASHINGS AND CLOSURE PLATES AT WALL ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. COORDINATE ALL CLOSURES WITH ELEVATOR, STAIR, ESCALATOR, AND OTHER ARCHITECTURAL DETAILS.

SHEAR CONNECTORS / SHEAR STUDS SHALL BE TYPE B AND CONFORM TO AWS D1.1, ASTM A108, AND ASTM A29 SPECIFICATIONS FOR 1010 THROUGH 1020 MILD STEELS. MINIMUM YIELD STRENGTH = 51 KSI AND MINIMUM ULTIMATE TENSILE STRENGTH = 65 KSI.

ALL STEEL HEADED STUD ANCHORS SHALL BE 3/4 INCH DIAMETER x (FLUTE HEIGHT + 1 1/4 INCH) AFTER WELDING. DESIGN AND DETAIL FLASHING AND CLOSURE PLATES WITH CLEARANCE FROM TOP OF STUD TO TOP OF SLAB IS 1 INCH.

DISTRIBUTED STEEL HEADED STUD ANCHORS UNIFORMLY OVER BEAM SPAN UNLESS NOTED OTHERWISE ON PLAN. MAXIMUM SPACING OF 3/4 INCH HEADED STUD ANCHORS SHALL NOT EXCEED 12 INCHES ON CENTER (ONE STUD PER FOOT) UNLESS NOTED OTHERWISE ON PLAN.

THE COMPOSITE STEEL FLOOR DECK INSTALLATION, WHEN COMPLETE, SHOULD BE READY TO RECEIVE CONCRETE. THE COMPOSITE STEEL FLOOR DECK HAS BEEN DESIGNED TO SUPPORT CONSTRUCTION LOADS UNSHORED, UNLESS NOTED OTHERWISE. IF ANY SHORING IS USED, IT SHALL BE DESIGNED AND SUPPLIED BY THE CONTRACTOR AND SHALL BE SHOWN ON THE SHOP DRAWINGS. SHORING SHALL BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO PLACING CONCRETE.

COMPOSITE STEEL FLOOR DECKS ARE TO BE LOADED LEVEL TO COLUMNS OR AS INDICATED ON THE DRAWINGS. MAINTAIN INDICATED MINIMUM SLAB THICKNESS OVER BEAMS THROUGHOUT SPAN. ELEVATED SLAB SHALL BE PLACED WITH MINIMUM THICKNESS THROUGHOUT WITH NO MORE THAN 1/2" OF ADDITIONAL THICKNESS IN ANY LOCATION TO HELP WITH THE LEVELING OF THE SLAB.

COMPOSITE STEEL FLOOR DECK SHALL BE DESIGNED TO SUPPORT DEAD LOADS SHALL NOT EXCEED 60 POUNDS PER HANGER TAB. IN ADDITION, LOADS ON HANGER TABS SHALL BE DISTRIBUTED IN SUCH A MANNER THAT THE TRIBUTARY LOADS FOR EACH HANGER TAB SHALL NOT EXCEED 5 POUNDS PER SQUARE FOOT. DO NOT HANG MEQ EQUIPMENT AND PIPING DIRECTLY FROM STEEL DECK.

STEEL ROOF DECK SHALL BE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE. ALL INTERIOR STEEL ROOF DECK SHALL CONFORM TO ASTM A1008. FACTORY PRIMED FOR PAINT. ALL EXPOSED STEEL ROOF DECK SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE. SEE STRUCTURAL DRAWINGS FOR DETAILS.

STEEL ROOF DECK SHALL BE ATTACHED TO STEEL SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS AND TO COLD-FORMED METAL FRAMING WITH #12 SELF-DRILLING SCREWS UNLESS NOTED OTHERWISE. WHEN DECK THICKNESS IS LESS THAN 0.28 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.

WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD LIGHT TO STEEL, WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.

ERECT STEEL DECK CLOSURES AND OTHER LIGHT GAGE MATERIAL REQUIRED TO PRODUCE A COMPLETED INSTALLATION.

FLAT, RIDGE, AND VALLEY PLATES:

UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS 1/8 INCH PER FOOT.

DO NOT HANG CEILINGS, DUCTS, LIGHT FIXTURES, EQUIPMENT, OR OTHER ITEMS FROM THE ROOF DECK WITHOUT PRIOR APPROVAL FROM THE DECK SUPPLIER AND REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

SEE ARCHITECTURAL DRAWINGS FOR FINAL ROOF SLOPES. WHERE STRUCTURAL FRAMING DOES NOT CREATE THE SPECIFIED ROOF SLOPE, IT SHALL BE CREATED WITH RIGID INSULATION ABOVE THE DECK.

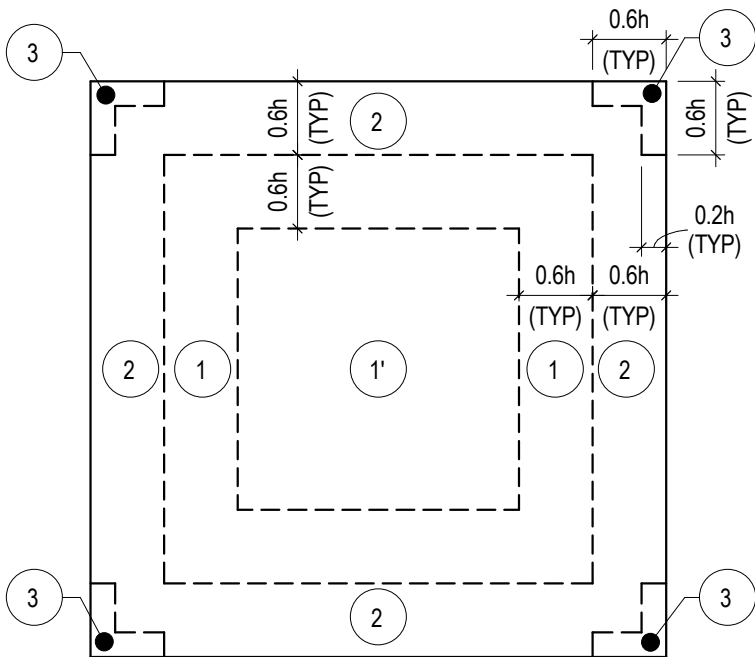
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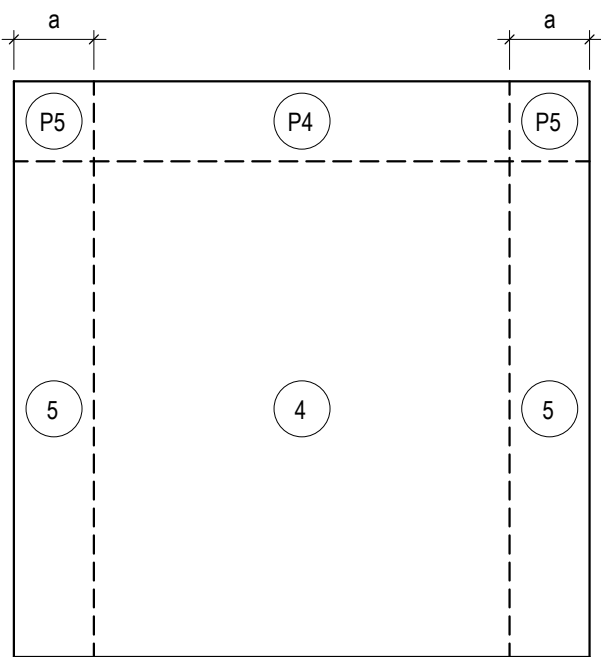
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E



ROOF PLAN



WALL ELEVATION

CONCRETE MIXTURE REQUIREMENTS								
APPLICATION	EXPOSURE CLASS	f _c	TEST AGE	MODULUS OF ELASTICITY	MAXIMUM W/CM	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE	MAXIMUM CONCRETE WEIGHT
FOOTINGS	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
EXTERIOR SLAB-ON-GROUND	F1	4500 PSI	28 DAYS	3824 KSI	0.55	4.5% ± 1.5%	1"	150 PCF
SLAB-ON-GROUND	F0	3000 PSI	28 DAYS	3122 KSI	SEE NOTE 2	SEE NOTE 3	1"	150 PCF
ELEVATED SLABS ON METAL DECK	F0	4000 PSI	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
TILT-WALLS	F0	4000 PSI (MIN)	28 DAYS	3605 KSI	SEE NOTE 2	SEE NOTE 3	3/4"	150 PCF
NOTES:								
1. EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE.								
2. WATER/CEMENT RATIO SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX DESIGN. THERE IS NO MAXIMUM WATER/CEMENT RATIO REQUIREMENT FOR THE EXPOSURE CLASSIFICATION ASSOCIATED WITH THIS APPLICATION. MAXIMUM WATER/CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE.								
3. THERE IS NO MANDATORY TARGET AIR CONTENT FOR THIS APPLICATION. THE CONTRACTOR MAY CHOOSE TO ADD AIR ENTRAINMENT TO IMPROVE THE WORKABILITY AND FINISHING PROPERTIES OF THE MIX. AIR CONTENT SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX.								

WALL REINFORCEMENT - HORIZONTAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4,000 PSI	f _c = 5,000 PSI	f _c = 6,000 PSI	
#4	1,500	33	30	27	15
#5	1,875	40	36	34	19
#6	2,250	48	44	40	23
#7	2,625	105	96	87	27
#8	3,000	121	108	99	30
#9	3,500	137	122	112	34
#10	3,875	153	138	125	39
#11	4,250	170	152	139	43

WALL REINFORCEMENT - VERTICAL BAR LAP SPlice LENGTH SCHEDULE (INCHES)					
BAR SIZE	MIN BAR SPACING (INCHES)	TENSION (LTS)			COMPRESSION (LCS)
		f _c = 4000 PSI	f _c = 5000 PSI	f _c = 6000 PSI	
#4	2,000	25	23	21	15
#5	2,125	31	28	26	19
#6	2,250	37	34	31	23
#7	2,375	54	49	45	27
#8	2,500	62	56	51	30
#9	2,875	70	63	57	34
#10	3,250	118	106	96	39
#11	3,625	131	117	107	43

NOTES:

- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
- 0.6h = 15'-0". SEE WALL ELEVATION MAP BELOW FOR LOCATION OF ROOF ZONES.
- a = 9'-7". SEE WALL ELEVATION MAP BELOW FOR LOCATION OF WALL ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - PA/S SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF 193 PSF AND VERTICAL VALUE OF 153 PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUS, AHUS, AND SCREEN WALLS.
- ROH# : DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
- ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

CAST-IN-PLACE CONCRETE (NON-PRESTRESSED) CLEAR COVER SCHEDULE			
APPLICATION	BOTTOM	TOP	SIDES
FOUNDATIONS	3"	2"	3"
SLAB-ON-GROUND	SEE DETAILS	SEE DETAILS	3"
RETAINING WALLS	N/A	N/A	2"
WALLS	N/A	N/A	1 1/2" (2")
COLUMNS	N/A	N/A	1 1/2" (2")
ELEVATED SLABS	3/4" (2")	1 1/2" (2")	1 1/2" (2")
BEAMS	1 1/2" (2")	1 1/2" (2")	1 1/2" (2")

LAP SPlice AND DEVELOPMENT LENGTH SCHEDULE																											
BAR SIZE	MINIMUM BAR SPACING (NOTE 3)		LAP SPlice LENGTHS										DEVELOPMENT LENGTHS														
			f _c 28-DAYS (PSI)		TOP		OTHER		TOP		OTHER		TOP		OTHER		TOP		OTHER		HOOKED (L _{dh})						
			BAR LOCATION		3,000		4,000		5,000		6,000		3,000		4,000		5,000		6,000		3,000		4,000		5,000		6,000
	STRAIGHT	HOKED																									
#3	1,375	2,250			26	21	24	19	21	16	19	15	20	16	18	14	16	12	14	11	6	6	6	6			
#4	1,500	1,375			36	28	30	24	28	21	25	20	27	21	23	18	21	16	19	15	8	7	7	7			
#5	1,625	3,750			43	34	38	24	34	26	32	24	33	26	29	22	26	20	24	18	10	10	9	9			
#6	1,750	4,500			52	41	46	36	41	32	37	29	40	21	36	27	31	24	28	22	13	13	12	12			
#7	1,875	5,250			65	50	62	43	73	39	67	36	50	38	63	33	56	30	51	27	17	16	15	15			
#8	2,000	6,000			94	73	93	63	84	56	76	51	72	56	71	48	64	43	58	39	20	19	18	18			
#9	2,375	6,768			107	82	104	72	94	64	86	59	82	63	80	55	72	49	66	45	24	23	22	22			
#10	2,625	7,625			121	94	119	81	106	73	97	67	93	72	91	62	81	56	74	51	29	27	26	26			
#11	2,875	8,500			136	104	130	91	117	81	107	75	104	80	100	70	90	62	82	57	34	32	31	30			
NOTES:																											
1. VALUES LISTED CORRELATE TO AN F _c VALUE OF 60 KSI. LENGTHS ARE IN INCHES. REQUEST ALTERNATE SCHEDULE FOR DIFFERENT REBAR STRENGTHS.																											
2. WHERE BARS OF DIFFERENT SIZES ARE LAP SPICED, THE LAP SPlice LENGTH SHALL BE THE LARGER OF THE DEVELOPMENT LENGTH OF THE LARGER BAR AND THE LAP SPlice OF THE SMALLER BAR.																											
3. VALUES LISTED ASSUME MINIMUM SPACING AS SHOWN. REQUEST ALTERNATE VALUES FOR BARS WITH TIGHTER SPACING.																											
4. IF APPLICABLE, MODIFY VALUES LISTED AS FOLLOWS:																											
A. LIGHT WEIGHT CONCRETE = MULTIPLY ALL VALUES BY 0.75																											
B. EPOXY COATED REBAR = MULTIPLY L _d & LAP SPlice LENGTH VALUES BY 1.5 & L _{dh} VALUES BY 1.2																											

ABBREVIATIONS		ABBREVIATIONS	
ADDL	ADDITIONAL	KSI	KIPS PER SQUARE INCH
ADJ	ADJACENT	L	LENGTH
AFF	ABOVE FINISHED FLOOR	LB(S)	POUND(S)
ALT	ALTERNATE	LL	LIVE LOAD
APPROX	APPROXIMATE	LLH	LONG LEG HORIZONTAL
ARCH	ARCHITECT OR ARCHITECTURAL	LLV	LONG LEG VERTICAL
ASD	ALLOWABLE STRESS DESIGN	LONG	LONGITUDINAL
B/	BOTTOM OF	LRFD	LOAD RESISTANCE FACTORED DESIGN
B/B	BACK-TO-BACK	LSH	LONG SIDE HORIZONTAL
BLDG	BUILDING	LSV	LONG SIDE VERTICAL
BLKG	BLOCKING	LTS	LAP TENSION SPlice
BP	BASE PLATE	LW	LIGHT WEIGHT
BRG	BEARING	LWC	LIGHT WEIGHT CONCRETE
BOT	BOTTOM	M	MOMENT
BTWN	BETWEEN	MAX	MAXIMUM
C	COMPRESSION	MC	MOMENT CONNECTION(S)
CFS	COLD-FORMED STEEL	MECH	MECHANICAL
CIP	CAST-IN-PLACE	MEP	MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION
CJ	CONTROL JOINT	MFR	MANUFACTURER
CJP	COMPLETE JOINT PENETRATION	MID	MIDDLE
CL	CENTER LINE	MIN	MINIMUM
CLR	CLEAR OR CLEARANCE	MISC	MISCELLANEOUS
CMU	CONCRETE MASONRY UNIT	NIC	NOT IN CONTRACT
COL	COLUMN	NS	NEAR SIDE
CONC	CONCRETE	NTS	NOT TO SCALE
CONN(S)	CONNECTION(S)	NWC	NORMAL WEIGHT CONCRETE
CONST	CONSTRUCTION	OC	ON CENTER
CONT	CONTINUOUS	OD	OUTSIDE DIAMETER
COORD	COORDINATE	OF	OUTSIDE FACE
D&E	DRILL & EPOXY	OH	OPPOSITE HAND
Ø	REINFORCING BAR DIAMETER	OPNG(S)	OPENING(S)
DBA	DEFORMED BAR ANCHOR	OPP	OPPOSITE
DOW	DEMAND CRITICAL WELD	OSL	OUTSTANDING LEG
*	DEGREE(S)	PAF	POWDER ACTUATED FASTENER
Ø	DIAMETER	PERP	PERPENDICULAR
DIAG	DIAGONAL	PJF	PREFORMED JOINT FILLER
DIM(S)	DIMENSIONS(S)	PJP	PARTIAL JOINT PENETRATION
DL	DEAD LOAD	PL	PLATE
DWG(S)	DRAWING(S)	PLF	POUNDS PER LINEAL FOOT
EA	EACH	PCST	PRECAST
EF	EACH FACE	PREFAB	PRE-FABRICATED
EJ	EXPANSION JOINT	PSF	POUNDS PER SQUARE FOOT
EL	ELEVATION	PSI	POUNDS PER SQUARE INCH
ELEV	ELEVATOR	PT	POST-TENSIONED
EOS	EDGE-OF-SLAB	REF	REFERENCE
EQ	EQUAL	REINF	REINFORCE(D) (ING) OR (MENT)
EQUIP	EQUIPMENT	REQ(D)	REQUIRE(D)
EW	EACH WAY	REV	REVISION
EXIST	EXISTING	RTU	ROOF TOP UNIT
EXP	EXPANSION	SCHED	SCHEDULE(D)
EXT	EXTERIOR	SDL	SUPERIMPOSED DEAD LOAD
F/F	FACE-TO-FACE	SE	STRUCTURAL ENGINEER OF RECORD
FD	FLOOR DRAIN	SF	SQUARE FOOT (FEET)
FF	FINISH FLOOR	SIM	SIMILAR
FND	FOUNDATION	SLRS	SEISMIC LOAD RESISTING SYSTEM
FS	FAR SIDE	SOG	SLAB-ON-GROUND
FT	FEET	SP	SPACE
FTG	FOOTING	SPEC(S)	SPECIFICATIONS(S)
GA	GAGE, GAUGE	SS	STAINLESS STEEL
GALV	GALVANIZED	STD	STANDARD
GB	GRADE BEAM	STIFF	STIFFENER
GC	GENERAL CONTRACTOR	STR	STRUCTURE OR STRUCTURAL
GDR	GIRDER	SYM	SYMMETRICAL
GEN	GENERAL	T	TENSION
GYP	GYPSUM	T&B	TOP AND BOTTOM
HCA	HEADED CONCRETE ANCHORS	T&G	TONGUE & GROOVE
HORIZ	HORIZONTAL	T/	TOP OF
HSS	HOLLOW STRUCTURAL SECTION	TEMP	TEMPERATURE OR TEMPORARY
ID	INSIDE DIAMETER	TYP	TYPICAL
IF	INSIDE FACE	UNO	UNLESS NOTED OTHERWISE
IN	INCH	V	SHEAR
INFO	INFORMATION	VERT	VERTICAL
INT	INTERIOR	VIF	VERIFY IN FIELD
ICC-ES	INTERNATION CODE COUNCIL - EVALUATION SERVICE	W/	WITH
JST(S)	JOIST(S)	WO	WITHOUT
K	KIPS (1,000 POUNDS)	WP	WORK POINT
KLF	KIP PER LINEAL FOOT	WWR	WELDED WIRE REINFORCEMENT
KSF	KIP PER SQUARE FOOT		

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COLUMBIA STORM PROCESSING FACILITY
ISC STORES / PGD WAREHOUSE
BUILDING E
COLUMBIA COUNTY, FLORIDA

ANDREW PATRICK TOMCZEK
LICENSE
No. 91735
STATE OF FLORIDA
PROFESSIONAL ENGINEER

03/18/2026

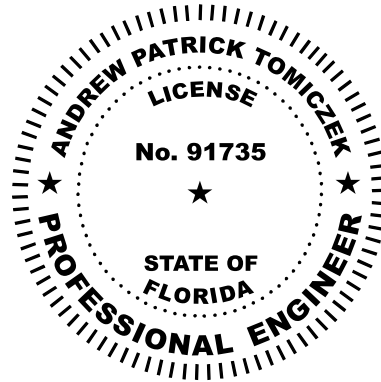
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ANDREW P. TOMCZEK
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PROJECT NUMBER
C09414.003
DATE
03/13/2026
TITLE
GENERAL SCHEDULES
DRAWING NO.
e-S003

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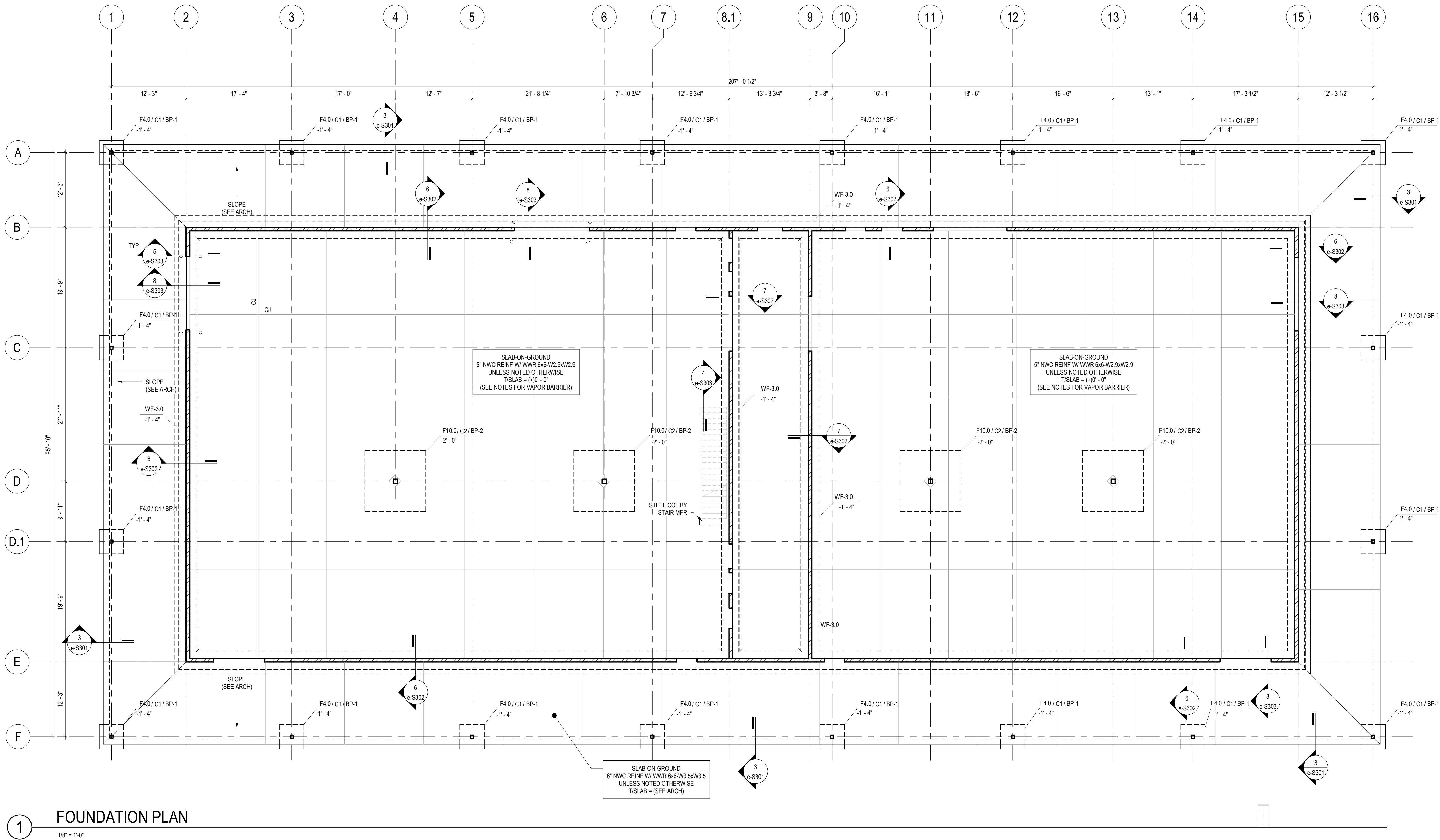
03/13/2026

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FOUNDATION PLAN

DRAWING NO.

e-S101



NOTES:

1. FF#1: DENOTES ISOLATED FOOTING MARK, SEE SCHEDULE ON S-303
2. WFF#: DENOTES CONTINUOUS FOOTING MARK, SEE SCHEDULE ON S-302
3. CH#: DENOTES HSS COLUMN WITH BASE PLATE, SEE SCHEDULE ON S-303
4. BP#: DENOTES BASE PLATE, SEE SCHEDULE ON S-303
5. CL#: DENOTES SLAB-ON-GRADE AND CONTROL JOINT, SPACING SHALL NOT EXCEED 15' - 0"
6. UNLESS NOTED OTHERWISE.
7. FOR RE-ENTRANT CORNER BARS, SEE 6/e s-301
8. PLACE MIN 10 MIL VAPOR BARRIER OVER TERMITES TREATED, WELL COMPACTED FILL, SEE ARCH.
9. FOR CONCRETE FINISH, SEE ARCH.
10. PIPING MUST PASS OVER COLUMN FOOTINGS, SEE DETAILS
11. FOR STANDARD DETAILS OF PIPING PASSING UNDER WALL, FOOTINGS AND TRENCHES ADJACENT TO FOOTINGS. PIPING PASSING UNDER FOOTINGS MUST BE IN PLACE AND INSPECTED BEFORE FOOTINGS ARE PREPARED OR FOOTING MUST STEP BELOW PIPE.
12. GC SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATION AS NEEDED. ADDITIONALLY, GC SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NEEDED. ANY CHANGES REQUIRED TO FOUNDATION LOCATIONS TO STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
13. GC SHALL PROVIDE HOUSEKEEPING PADS PER DETAIL 7/e s-303AND COORDINATE LOCATION WITH MECHANICAL/ELECTRICAL ENGINEER DRAWINGS.



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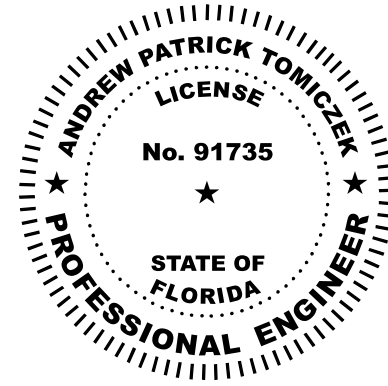
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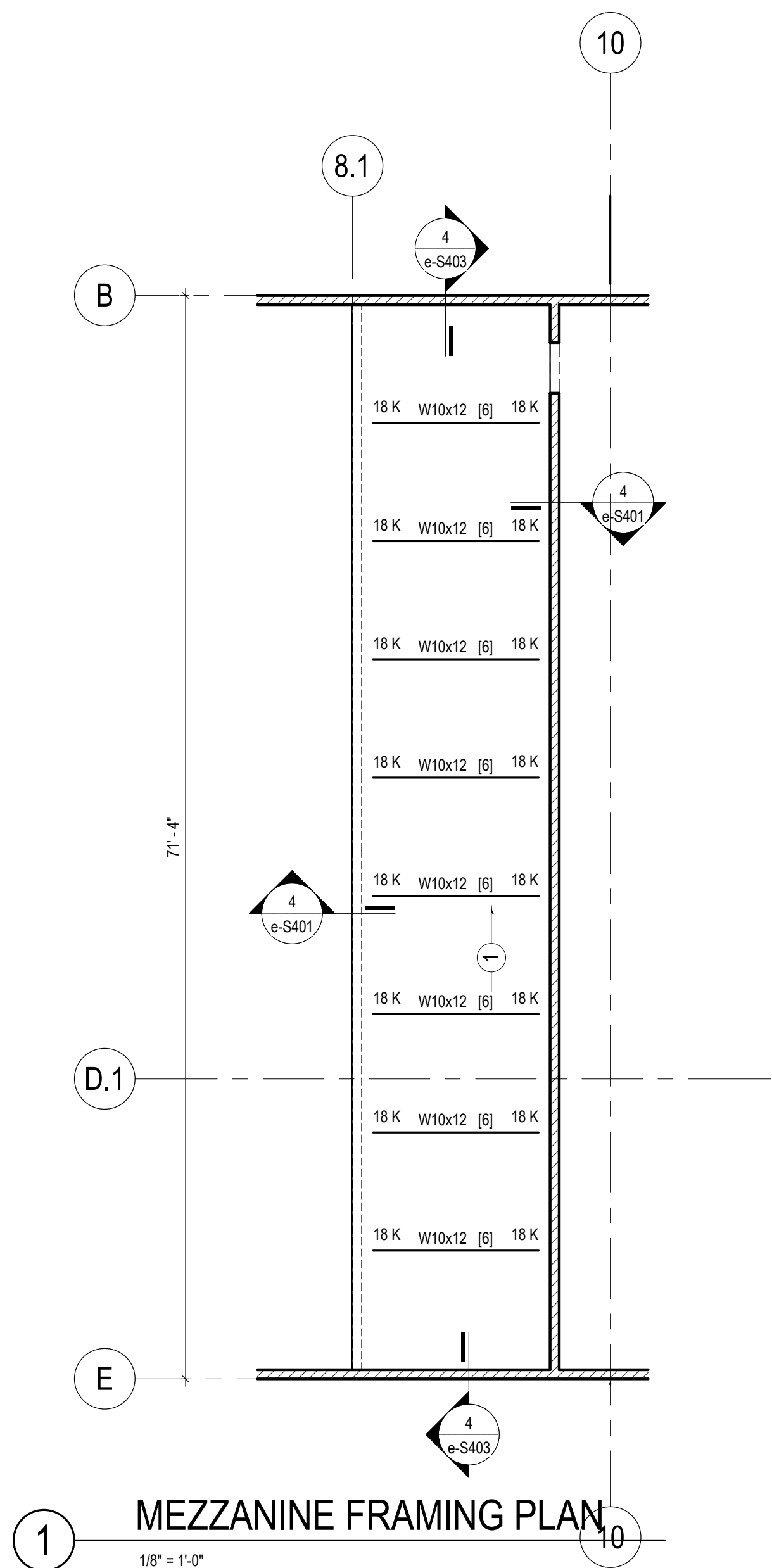
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MEZZANINE FRAMING PLAN

DRAWING NO.

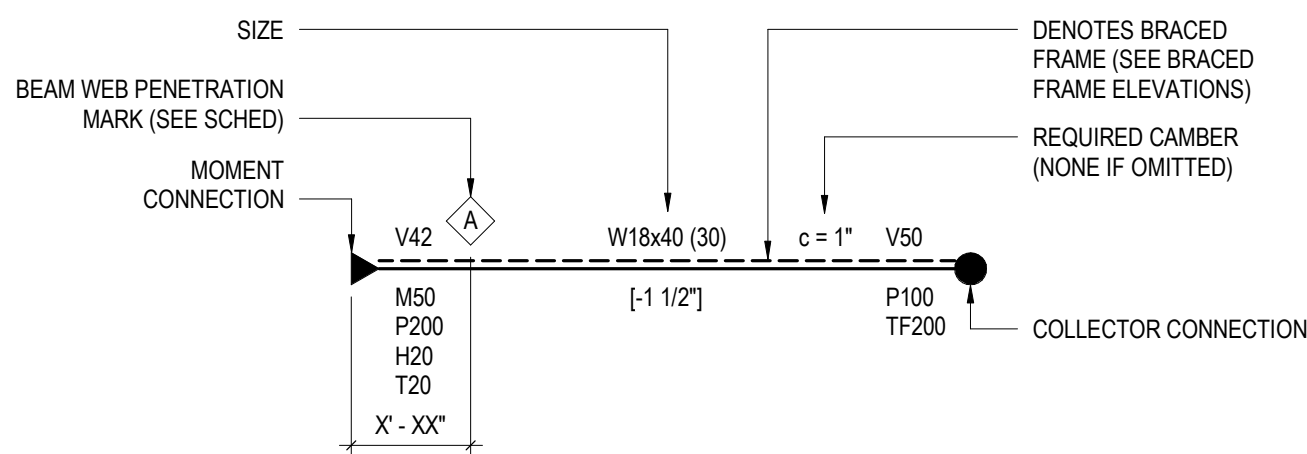
e-S201



NOTES:

1.  DENOTES 4 1/2" OF NORMALWEIGHT CONCRETE ON 1 1/2" 20 GAGE CORRUGATED COMPOSITE DECK REINFORCED W/ WWR 6x6 W2. 1xW2. 1x
2. T/STEEL = (+)11' - 6"
3. PROVIDE AFTERSET CONCRETE HOUSEKEEPING PAD FOR MECHANICAL EQUIPMENT AS REQUIRED (SEE MECHANICAL DRAWINGS). SEE 3/ e-S303

STEEL BEAM LEGEND



- V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS



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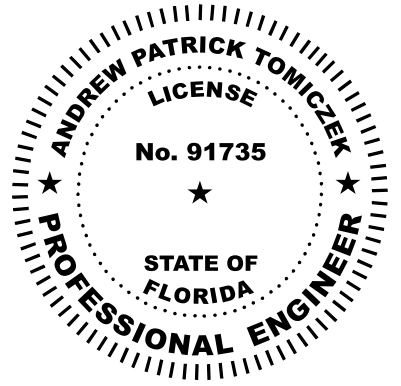
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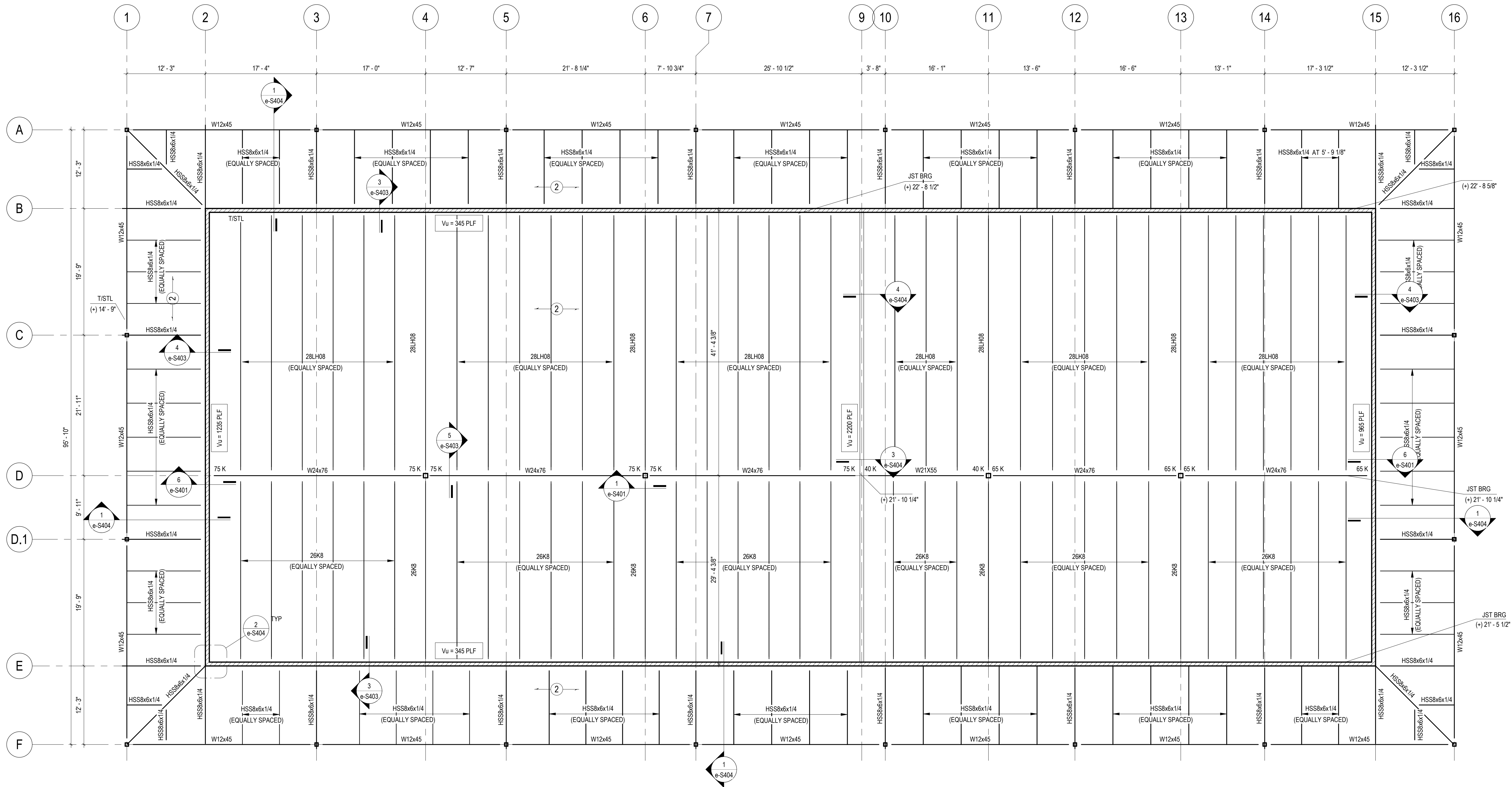
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D

ROOF FRAMING PLAN

DRAWING NO.

e-S202



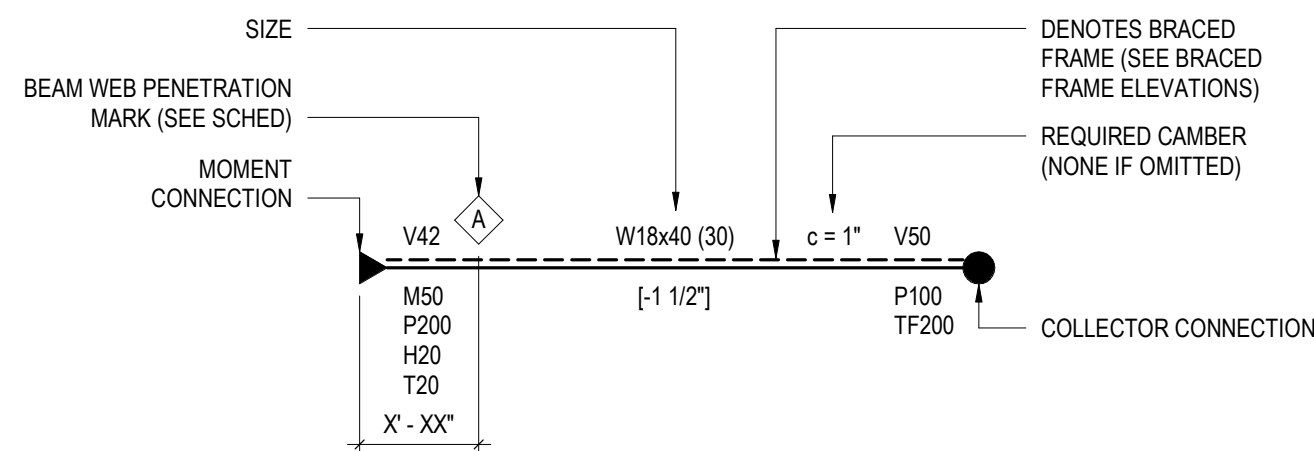
② ROOF FRAMING PLAN

$$1/8'' = 1'-0''$$

NOTES:

- | | | |
|----|---|---|
| 1. |  | DENOTES 4 1/2" OF NORMAL WEIGHT CONCRETE ON 1 1/2" 20 GAGE CORRUGATED COMPOSITE DECK REINFORCED W/ WWR #6-W1.2xW2.1 |
| 2. |  | DENOTES 3" NORMAL WEIGHT CONCRETE ON 1 1/2", 22 GAGE CORRUGATED COMPOSITE DECK REINFORCED WITH WWR #6-W1.4xW1.4 |
| 3. | | GC TO COORDINATE ON OPEN WEB STEEL JOIST BRIDGING AND BRACING REQUIREMENTS PER OPEN WEB STEEL JOIST SUPPLIER AND SJI. |
| 4. | | T/SJTEL = VARIES (SEE PLAN) |
| 5. | | HSS SPACING SHALL NOT EXCEED 6' - 0" UNLESS NOTED OTHERWISE. |
| 6. | | OPEN WEB STEEL JOIST SPACING SHALL NOT EXCEED 5' - 0" UNLESS NOTED OTHERWISE. |
| 7. | | PROVIDE 5' JOIST SEAT FOR 20K6 AT GRIDLINE D. |
| 8. | $V_u = X \text{ PLF}$ | DENOTES DIAPHRAGM SHEAR FORCE. |

STEEL BEAM LEGEND



V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS



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COLUMBIA COUNTY, FLORIDA

ANDREW PATRICK TOMCZEK

LICENSE

No. 91735

STATE OF FLORIDA

PROFESSIONAL ENGINEER

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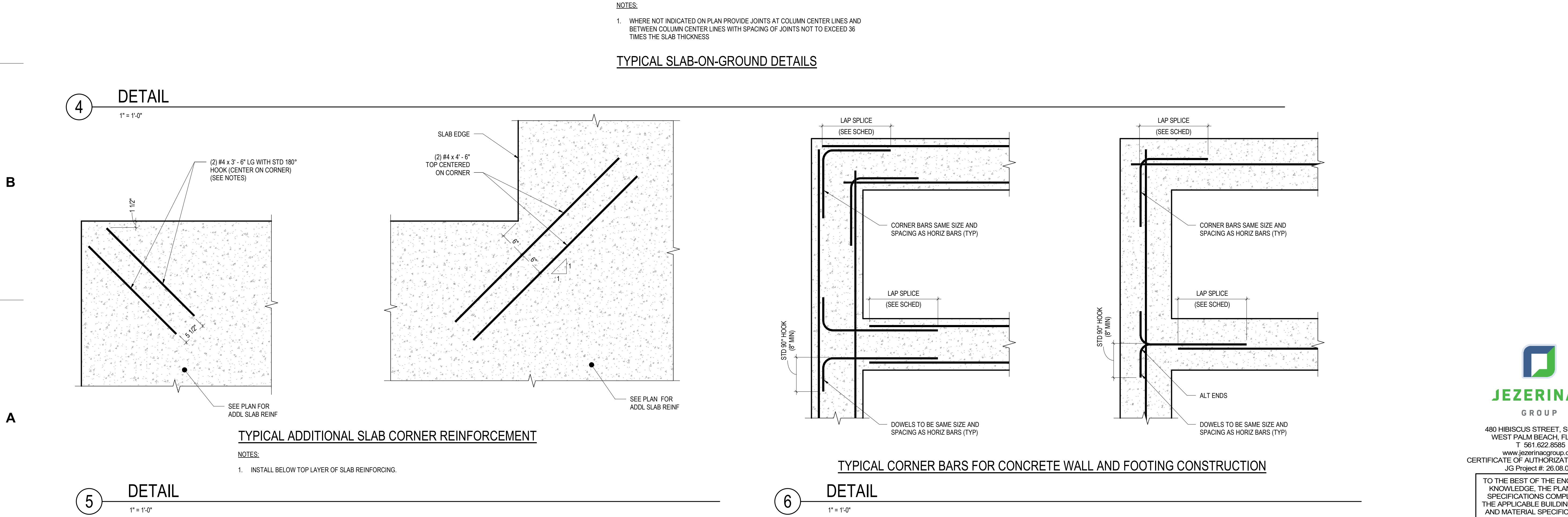
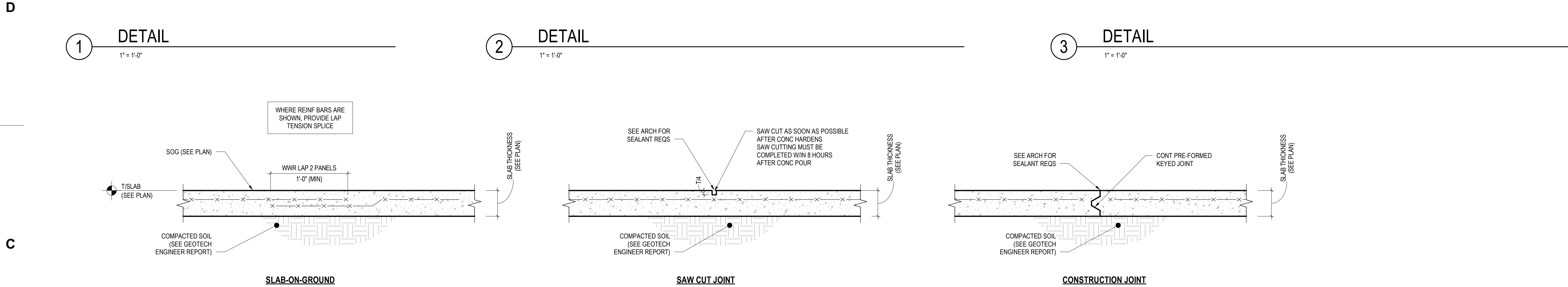
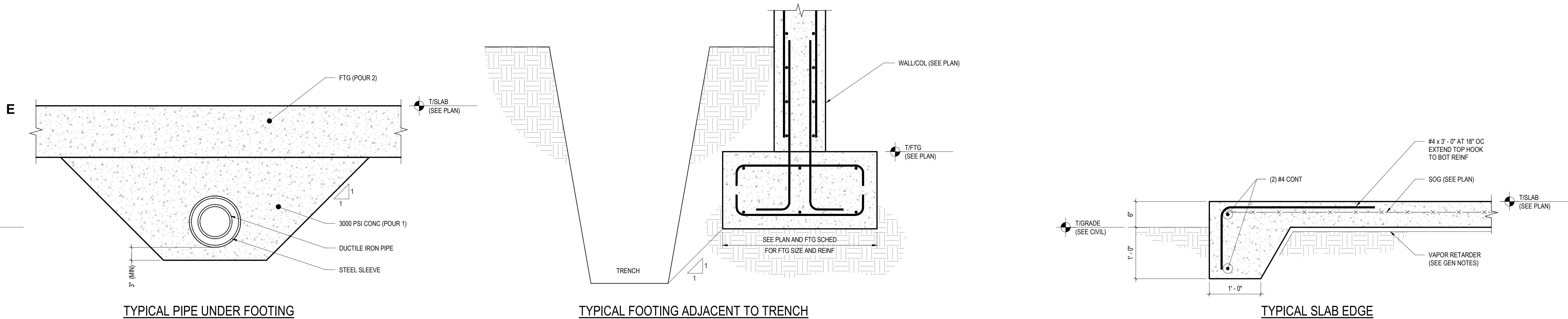
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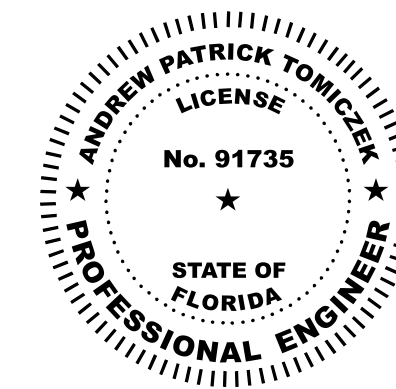
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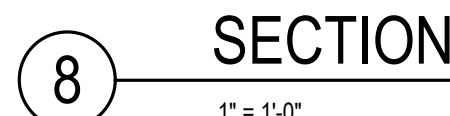
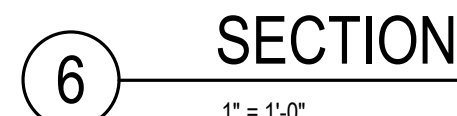
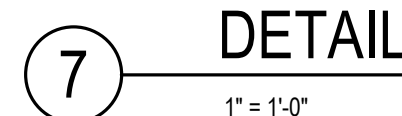
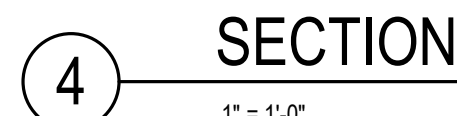
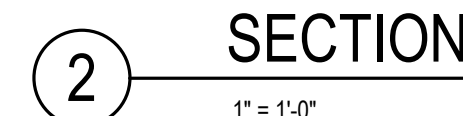
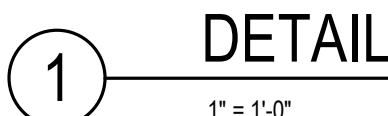
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FOUNDATION SECTIONS & DETAILS

DRAWING NO.

|e-S303

Autodesk Docs://C09414.003 - FPL
Columbia Warehouse
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NOTES:

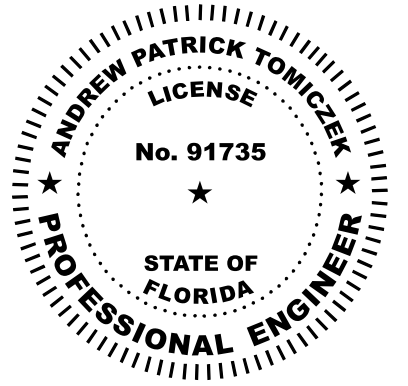
1. SEE ARCHITECTURAL AND MEP DRAWINGS FOR SIZE AND LOCATION OF HOUSEKEEPING PADS.

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AND MATERIAL SPECIFICATIONS.

[illegible]

**FLORIDA POWER & LIGHT COMPANY
COLUMBIA STORM PROCESSING FACILITY
ISC STORES / PGD WAREHOUSE
BUILDING E
COLUMBIA COUNTY, FLORIDA**



03/18/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY
ANDREW P TOMICZEK, P.E.
ON THE DATE ADJACENT TO THE SEAL.

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ARCHENGR OF RECORD
ANDREW P. TOMICZEK
FL PE No. 91735
DESIGNED BY
JEZERINAC GROUP
DRAWN BY
CN

PROJECT NUMBER
C09414.003

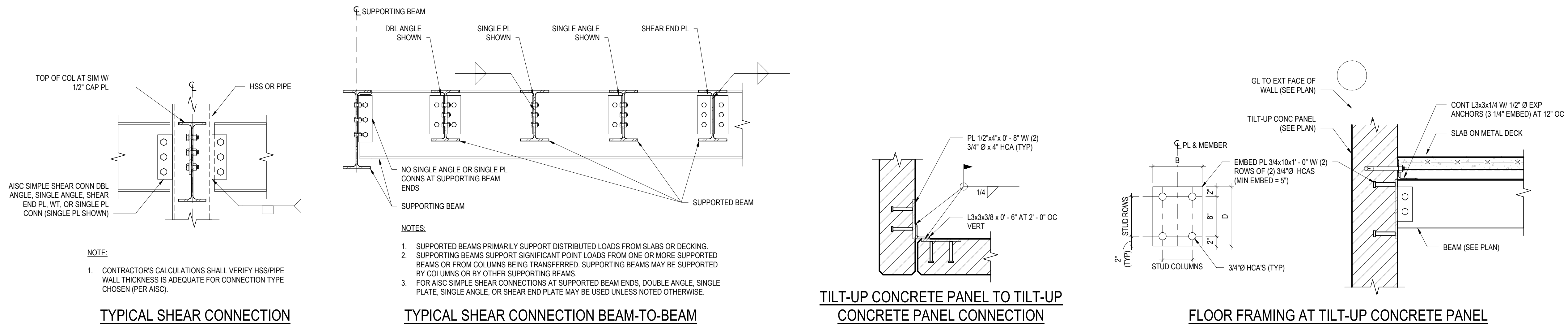
DATE
03/13/2026

TITLE

FRAMING SECTIONS & DETAILS

DRAWING NO.

e-S401

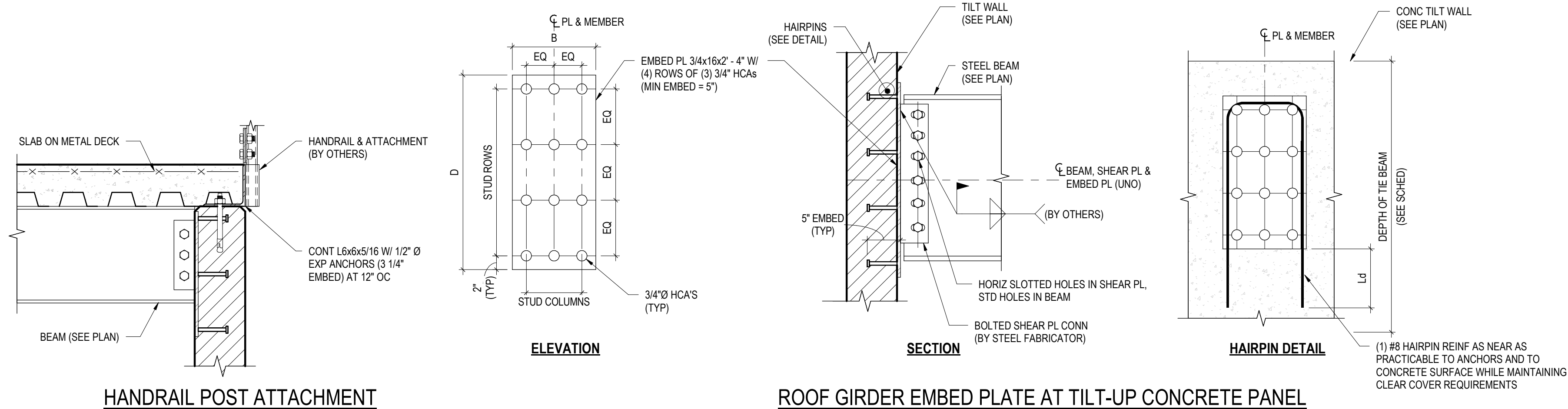


1 SECTION
1" = 1'-0"

2 SECTION
1" = 1'-0"

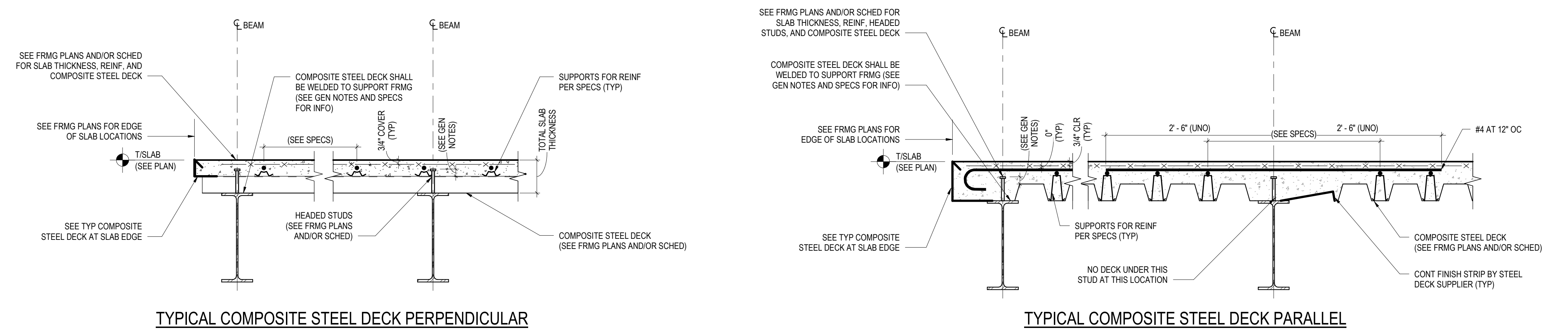
3 SECTION
1" = 1'-0"

4 SECTION
1" = 1'-0"



5 SECTION
1" = 1'-0"

6 DETAIL
1" = 1'-0"

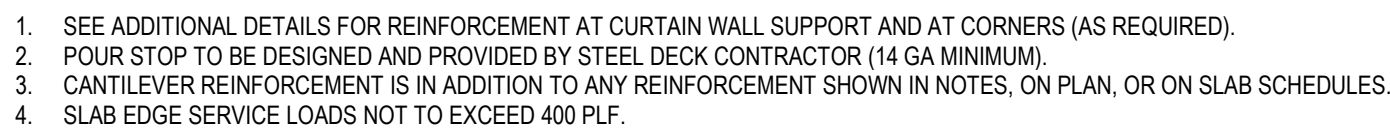


7 SECTION
1" = 1'-0"

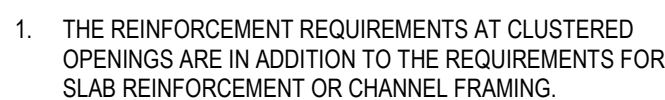
8 SECTION
1" = 1'-0"

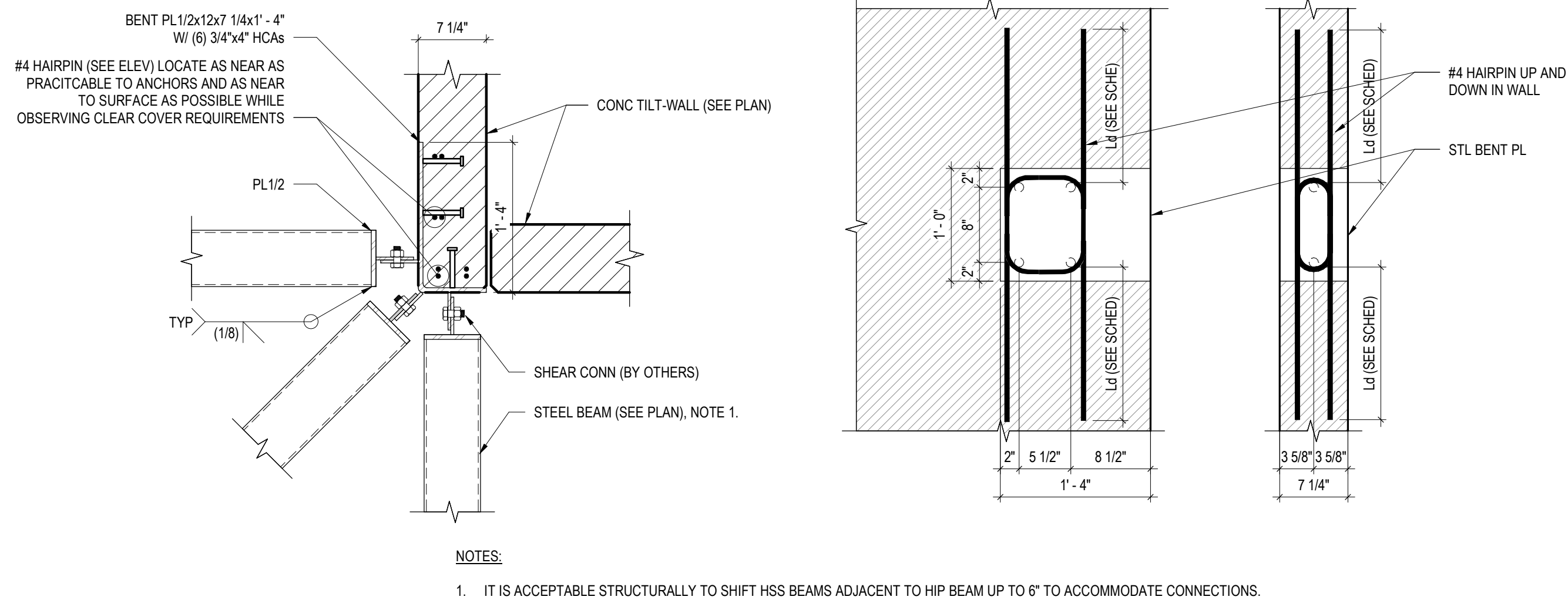
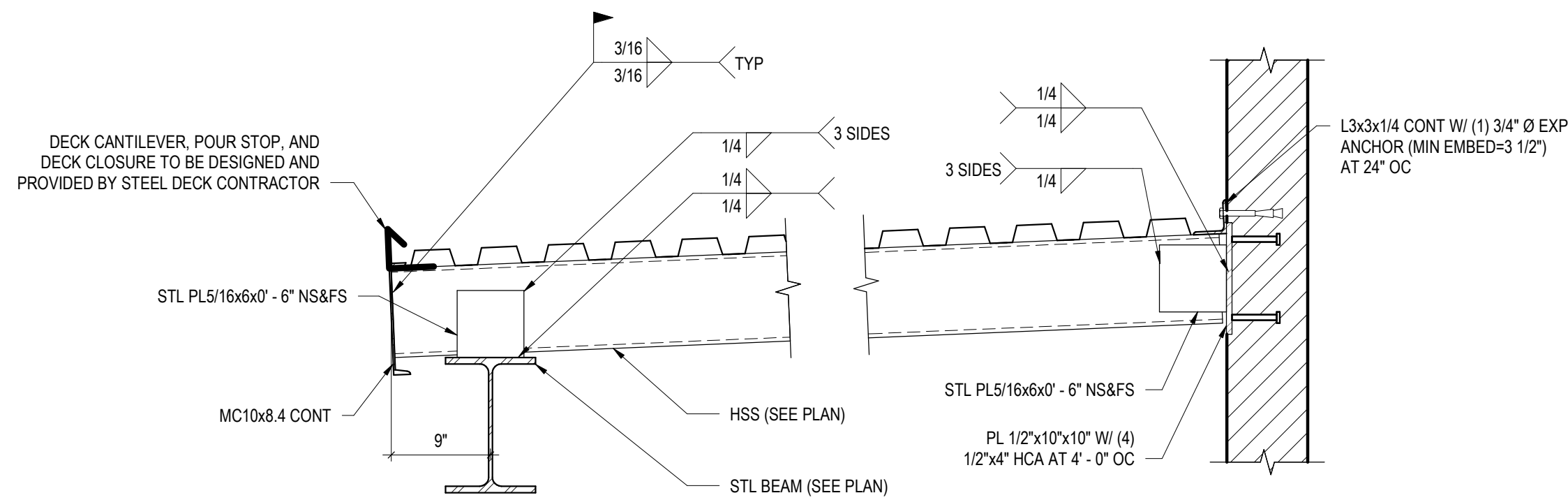
[illegible]

e-S402


$$1'' = 1'-0''$$


1. SEE GENERAL NOTES FOR HEADED STUD SIZE, LENGTH AND MAXIMUM SPACING. NUMBER OF STUDS IS INDICATED ON FRAMING PLAN.
2. HEADED STUDS IN ONE ROW SHALL BE PLACED DIRECTLY OVER THE BEAM WEB.
3. PLACE STUDS IN A SINGLE ROW WHERE SPACING REQUIRES PERMIT. STUDS SHALL BE PLACED IN TWO ROWS OR THREE ROWS ONLY WHERE REQUIRED IN ORDER TO PLACE THE TOTAL REQUIRED STUDS.
4. MAXIMUM NUMBER OF STUDS PERMITTED PER INDIVIDUAL METAL DECK RIB IS 3.
5. STUD PLACEMENT PROCEDURE FOR DECK PERPENDICULAR TO SUPPORT: PLACE A STUD IN ALTERNATE FLUTES FOR ENTIRE LENGTH OF BEAM.
6. STUD PLACEMENT STUDS IN FLUTES WITHOUT STUDS, STARTING AT THE ENDS OF THE BEAM AND CONTINUING TO BEAM CENTER. DISTRIBUTE STUDS EQUALLY FROM EACH END TO BEAM CENTER. PLACE A THIRD STUD IN EACH FLUTE, EQUALLY SPACED FROM EACH END AND TOWARD CENTER OF BEAM. IF STUDS REMAIN, PLACE A THIRD STUD IN EACH FLUTE, EQUALLY SPACED FROM EACH END TOWARD CENTER OF BEAM.
7. SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF HEADED STUDS PER SPECIFICATIONS.

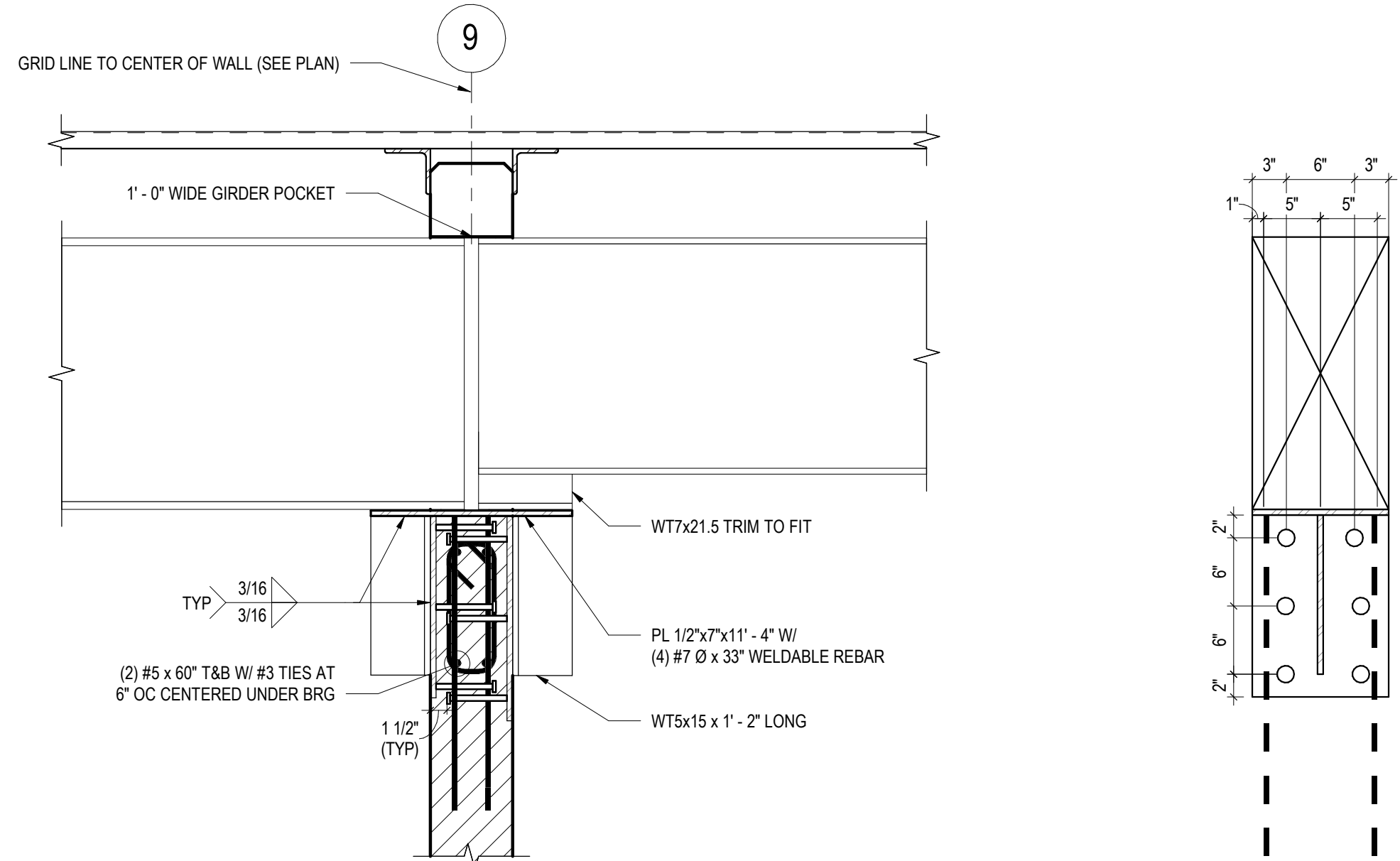
$$1'' = 1'-0''$$

$$1'' = 1'-0''$$



CANOPY FRAMING

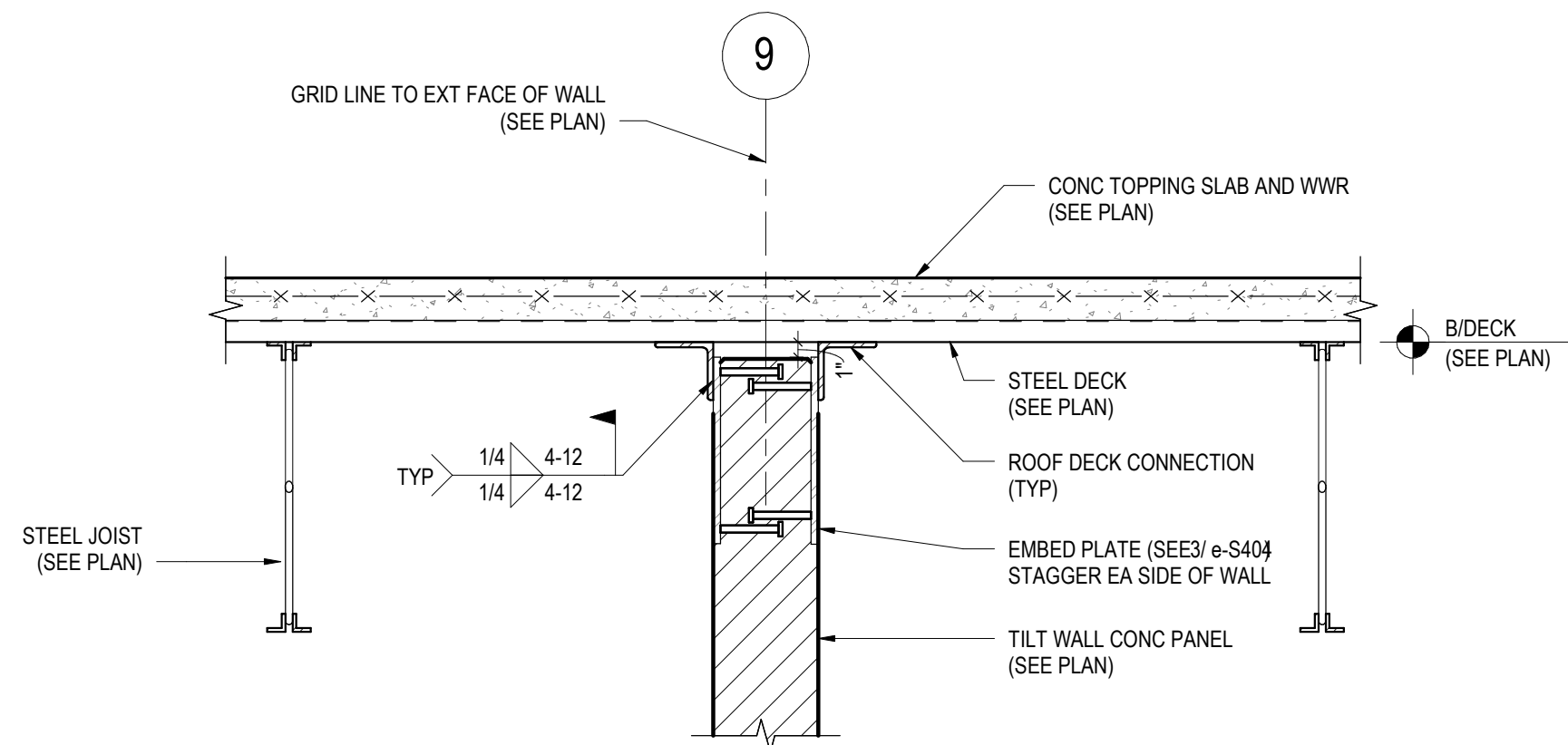
CANOPY CORNER CONNECTION

SECTION

$$1'' = 1'-0$$


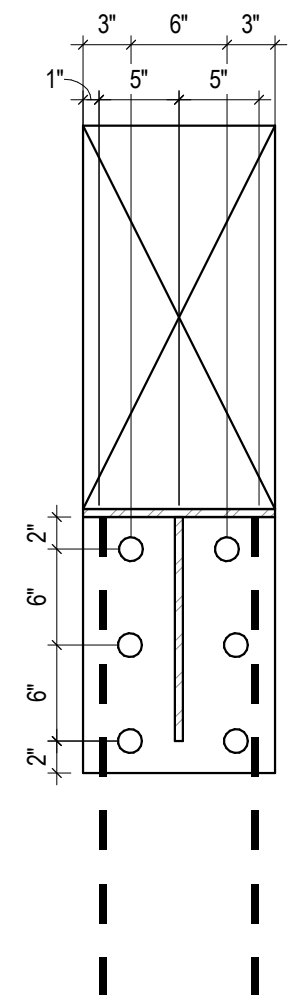
ROOF GIRDER CONNECTION TO INTERIOR WALL

DETAIL

$$1'' = 1'-0''$$


DECK SUPPORT ANGLE AT PARALLEL JOISTS

SECTION

$$1'' = 1'-0$$


SECTION

$$1'' = 1'-0''$$