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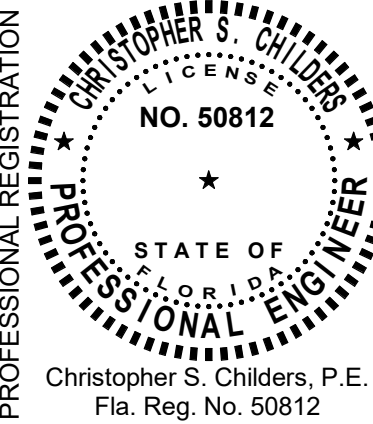
O'LENO STATE PARK

SHEET TITLE

COVER SHEET

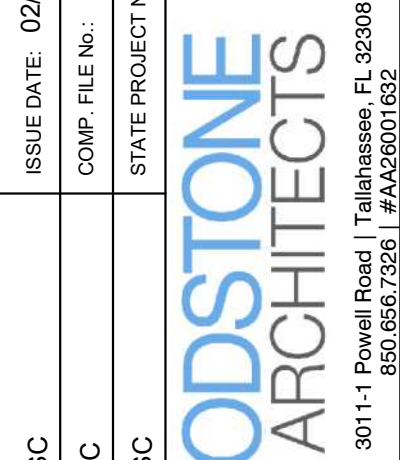
PROJECT TITLE
PAVILION RESTORATION

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Christopher S Childers, P.E.
Date: 2026.02.12
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DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:

ISSUE DATE: 02/13/2026
COMP. FILE NO.:
STATE PROJECT No. 6D201



*Department of Environmental Protection
Division of Recreation and Parks
Bureau of Design and Construction
3800 Commonwealth Blvd., Tallahassee, FL 32309 (850) 245-2300*

SHEET LIST						
SHEET NUMBER	SHEET NAME	AS-BUILTS 6/11/25	60% CD	100% CD	100% CD w/Owner Comments	CURRENT REV.
S000	COVER SHEET		•	•	•	
S101	STRUCTURAL NOTES		•	•	•	
S102	STRUCTURAL PLAN SPECIFICATION		•	•	•	
S111	ROOF WIND DIAGRAM		•	•	•	
S112	NORTH / EAST WALL WIND DIAGRAMS		•	•	•	
S113	SOUTH / WEST WALL WIND DIAGRAMS		•	•	•	
S201	GROUND PLAN	•	•	•	•	
S202	ROOF FRAMING PLAN	•	•	•	•	
S301	BUILDING SECTIONS	•	•	•	•	
S302	BUILDING ELEVATIONS	•	•	•	•	
S303	BUILDING ELEVATIONS	•	•	•	•	
S304	BUILDING SECTIONS		•	•	•	
S511	TYPICAL CONCRETE DETAILS		•	•	•	
S701	WOOD DETAILS		•	•	•	
S702	WOOD DETAILS		•	•	•	
						Current Rev. Date
						Current Revision Description

STRUCTURAL NOTES

GENERAL NOTES

- GN-1 The governing Code for this Project is the Florida Building Code, 8th Edition (2023). This Code prescribes which edition of each referenced standard applies to this Project.
- GN-2 To the best of our knowledge, the Structural Drawings and Specifications comply with the applicable requirements of the governing Building Code.
- GN-3 Construction is to comply with the requirements of the governing Building Code and all other applicable Federal, State, and Local Codes, Standards, Regulations and Laws.
- GN-4 Use these Notes in conjunction with the Plan Specifications or Project Specifications. If a conflict exists, the more stringent governs.
- GN-5 See Plan Specifications or Project Specifications for testing. See the Structural Inspection Plan for inspection requirements.

GENERAL NOTES - CONTRACTOR REQUIREMENTS

- GN-6 Contractor refers to the General Contractor, Construction Manager, or the organization that is assigned to have overall responsibility and supervision of the Project.
- GN-7 The Contractor shall be solely responsible for, and have control over, the means, methods, supervision, techniques, sequences, procedures of construction, quality, and correctness of the work. The Contractor is solely responsible for jobsite safety including all OSHA requirements.
- GN-8 The Contractor shall coordinate all Contract Documents with field conditions and dimensions and Project Shop Drawings prior to construction. Do not scale drawings; use only printed dimensions. Report any discrepancies in writing to the Architect prior to proceeding with work. Do not change size or location of structural members without written instructions from the Structural Engineer of Record.
- GN-9 Contractors who discover discrepancies, omissions or variations in the Contract Documents during bidding shall immediately notify the Architect. The Architect will resolve the condition and issue a written clarification.
- GN-10 The Contractor shall protect adjacent property, his own work and the public from harm. The Contractor is advised to document the condition of adjacent property with a photographic survey and other documentation, including crack monitoring, prior to and during construction.
- GN-11 The Structure is designed to be structurally sound when completed. The Contractor shall not overload the structure during construction. Prior to completion, the Contractor is responsible for stability and temporary bracing, including, but not limited to, masonry walls. Wherever the Contractor is unsure of these requirements, the Contractor shall retain a Florida Licensed Engineer to design and inspect the temporary bracing and stability of the structure.

GENERAL NOTES - DRAWINGS

- GN-12 The Structural drawings shall be used in conjunction with the architectural drawings and all other drawings and documents, including shop drawings prepared by equipment suppliers and delegated engineers.
- GN-13 Openings shown on Structural Drawings are only pictorial. See the Architectural for complete information such as slab depressions, slopes, curbs, finishes, and opening locations in structural members as required by MEP systems and architectural elements.

- GN-14 Details labeled "typical" apply to all situations that are the same or similar to those specifically referenced, whether or not they are keyed in at each location. Questions regard-ing the applicability of typical details shall be resolved by the Architect.
- GN-15 When joists or beams are not specifically dimensioned, they are located equally between gridlines or equally between dimensioned members.

- GN-16 See Architectural drawings for fireproofing and waterproofing details and requirements.

GENERAL NOTES - DESIGN LOADS

- GN-17 Design Loads:

OCCUPANCY	LIVE LOAD	DEAD LOAD	CONCENTRATED LOAD
Roof Sloped	10 psf	20 psf	
Public Areas	100 psf	20 psf	
Live Load reduction for beams, columns and foundations has been taken in accordance to the governing building code. Live Load reduction for slabs and joist is not permitted.			
GN-18 Design Rain Loads:			
Rain Load	3.0 psf		
Rain Intensity	i = 8.24 in/hr (15 — min)		
Static Head	ds = N/A, free end		
Hydraulic Head	dh = 0.52 in, free end		

- GN-19 Design Wind Loads:

Governing Code	FBC 8 th Edition (2023) / ASCE 7-22
Building Risk Category	II
Ultimate Wind Speed	Vult = 121 mph
Allowable Stress Design Wind Speed	Vasd = 94 mph
Mean Roof Height	13.5 feet
Directionality Factor	Kd = 0.85
Gust-Effect Factor	G = 0.85
Exposure	C, Open Building
Internal Pressure Coefficient	Gcpi = +/- 0

SHOP DRAWINGS AND OTHER SUBMITTALS

- SD-1 Refer to the applicable Plan Specifications or Project Specifications for technical content requirements. Incomplete submittals will be returned without review.
- SD-2 Submit specific components, such as columns, footings, etc., in a single package. Submit similar floors together.
- SD-3 On first submittal, clearly flag and cloud all differences from the Contract Documents. On resubmittals, flag and cloud all changes and additions to previous submittal; only clouded items will be reviewed.
- SD-4 Submittals for special structural, load-bearing items that are required by Codes or Standards to resist forces must be prepared by, or under the direct supervision of, a Delegated Engineer. Examples include Prefabricated Wood Components, and Shoring and Reshoring.

- SD-5 A Delegated Engineer is defined as a Florida Licensed Engineer who specializes in and undertakes the design of Structural Components or Structural Systems included in a specific submittal prepared for this Project and is an employee or officer of, or consultant to, the Contractor, Subcontractor, Fabricator, or Erector responsible for the submittal. The Delegated Engineer shall sign, seal and date the submittal, including calculations and drawings. See Plan Specifications or Project Specifications for more specific criteria.
- SD-6 The Trade Contractor is responsible for confirming and correlating dimensions at the job sites, for tolerances, clearances, quantities, fabrication processes and techniques of construction, coordination of the work with other trades and full compliance with the Contract Documents.
- SD-7 The Contractor shall review and approve submittals, including substitution requests and shop drawings, and shall sign and date each drawing prior to submitting to the Architect. This approval is to confirm that the submittal is complete, complies with the submittal requirements and is coordinated with field dimensions, other trades, erection sequencing and constructability. Submittals not reviewed by the Contractor will be returned without review.
- SD-8 Bliss & Nyitray (BNI) reviews submittals to confirm that the submittal is in general conformance with the design concept presented in the Contract Documents. Quantities and dimensions are not checked. Notations on submittals do not authorize changes to the contract sum. Checking of the submittal by BNI shall not relieve the Contractor of responsibility for deviations from the Contract Documents and from errors or omissions in the submittal.
- SD-9 BNI's review of Delegated Engineer submittals is limited to verifying that the specified structural submittal has been furnished, signed and sealed by the Delegated Engineer and that the Delegated Engineer has understood the design intent and used the specified structural criteria. No detailed check of calculations is made. The Delegated Engineer is solely responsible for his/her design, including but not limited to the accuracy of his/her calculations and compliance with the applicable codes and standards.
- SD-10 BNI may transfer to the Contractor BIM files, CAD files or other electronic data for use in preparing Shop Drawings. The contractor shall email a request for BIM or CAD files along with an acknowledgement that he/she has read and agreed to the following terms and conditions:

- A. This electronic data remains the property of BNI, and in no case shall the transfer of these files be considered a sale. The files shall not be used for other projects, additions to this project, or for completion of this project by others.
- B. The Contractor is responsible for assuring that the electronic data accurately represents the Contract Documents. In the event of a conflict, the Contract Documents shall govern.
- C. The electronic data is current as of the date of transfer but may subsequently be revised or supplemented. If so, then the Contractor may request updated electronic data.
- D. The use of these electronic data shall not modify Contractor's responsibility for coordination with other trades, or for the proper checking and coordination of dimensions, details, member sizes and gage, and quantities of materials to facilitate complete and accurate fabrication and erection.
- E. Do not scale dimensions since the electronic data may not be precise and, in some cases, have been intentionally altered for presentation purposes.
- F. Contractor shall indemnify, defend and hold harmless BNI from all claims, damages, losses, expenses, penalties, and liabilities, including attorneys' fees, arising out of or resulting from the use of the electronic data by Contractor or others.

SLABS ON GRADE

- SG-1 Refer to Geotechnical Report for subgrade preparation more than 12" below bottom of slab.
- SG-2 Above subgrade, use fill containing not more than 10% passing #200 sieve and maximum 1 inch diameter. Compact to 95% of maximum dry density as determined by modified proctor ASTM D-1557. Each layer of fill shall not exceed 6" loose thickness. Compact prior to placement of the next layer.
- SG-3 Fill placement and compaction shall be monitored and accepted by the testing agency. Take a min. of one field density test (ASTM D-1556 or D-2922) for each 2,500 square feet of each layer. The testing agency shall randomly select test locations.
- SG-4 For interior slabs, use 10 mil vapor retarder complying with ASTM E1745 between soil and bottom of slab and install in conformance with ASTM E1643. Lap joints 6" and seal with manufacturer approved tape. Repair all punctures and tears and seal around all penetrations. Do not use any sheeting below exterior concrete slabs.
- SG-5 Reinforce slabs on grade with welded wire reinforcement supplied in flat sheets only. Use chairs to support wire reinforcement in the center of slab. Fiber reinforced concrete shall not be used at slabs to receive a broom finish.
- SG-6 Provide crack control joints at 5 feet maximum to limit areas between joints to match existing sq. ft. in all floating slabs on grade 2 hours after final finish but not more than 8 hours after completion of the pour. Aspect ratio shall not exceed 1.25. Avoid L-shaped panels. Locate to conform to bay spacing whenever possible, add crack control joints at re-entrant corners which tend to invite cracks.
- SG-7 In sidewalks and walkways, locate isolation joints at 20 ft. o.c. maximum score and tool between isolation joints in equal bays of 5 ft. or less.
- SG-8 See the Architectural Drawings for slab on grade depressions and other requirements.
- SG-9 Termite protection shall be provided by a Registered Termiticides, and a certificate of compliance shall be issued to the building department per the Florida Building Code, section 1816. Refer to the Architectural Drawings for additional information.

REINFORCED CONCRETE

- RC-1 Comply with ACI SPEC-301 and CODE-318 and Plan Specifications or Specification Sections 031000, 032000 and 033000.
- RC-2 Provide structural concrete with a minimum ultimate compressive design strength in 28 days as follows:
- | Element | Strength |
|----------------|----------|
| Slabs on Grade | 3000 psi |
- RC-3 Use normal weight concrete for all structural members. u.o.n.
- RC-4 Provide ASTM A615 Grade 60 reinforcing steel. Reinforcing shall be accurately placed, rigidly supported and firmly tied in place, with appropriate bar supports and spacers. Lap continuous reinforc-ing 48 bar dia. Provide cover over reinforcing as follows:
- | Element | Bottom | Top | Sides |
|--------------------------|--------|--------|--------|
| Slabs on Grade | 2" | 1" | 2" |
| Slabs Exposed to Weather | 1 1/2" | 1 1/2" | 1 1/2" |

- RC-5 Provide A706 reinforcing steel when the rebar is to be welded. Do not weld A615 bars.
- RC-6 Deformed Bar Anchor (DBA) shall conform to ASTM A496 with a minimum yield strength of 70,000 PSI. Reinforcing bars, A615 or A706, is not an acceptable substitution for DBA's.
- RC-7 Provide galvanized reinforcement in areas exposed to the environment, such as balconies, or as indicated on the drawings.
- RC-8 Where specified, provide plain, cold-drawn electrically-welded wire reinforcement conforming to ASTM A185. Supply in flat sheets only. Lap splice two cross wire spacings.
- RC-9 Utilities shall not penetrate beams or columns but may pass through slabs and walls individually, uon. For openings 24" long or less, cut reinforcing and replace alongside opening with splice bars of equivalent area with 48 bar dia. lap. Prepare and submit shop drawings for openings longer than 24". For rectangular openings 12" long or longer, add 1#5 x 6" mid depth diagonal at all 4 corners.
- RC-10 Where reinforcing steel congestion permits, conduit and pipes up to 1" diameter may be embedded in concrete per ACI CODE-318, Section 20.6. Space at 3 diameters o.c. Place in the middle third of the slab depth. If conduits are significantly congested, additional reinforcing perpendicular to piping may be required. Requests to embed larger pipes should be accompanied by a detailed description and be submitted to the architect for evaluation. Aluminum conduits shall not be placed in concrete.
- RC-11 Provide construction joints in accordance with ACI CODE-318, Section 26.5.6. Provide keyways and adequate dowels. Submit drawings showing location of construction joints and direction of pour for review.
- RC-12 Provide 3/4" chamfer for all exposed corners.
- RC-13 Provide reinforcing steel installer with a set of Structural Drawings for field reference. Inspect reinforcing steel placing from structural drawings.

POST-INSTALLED ANCHORS - GENERAL

- AN-1 Substitution requests will be considered for products having an ICC-ES report recognizing the product for the appropriate application. Substitute concrete anchors must be approved for use in cracked concrete. Substitution requests shall include signed and sealed calculations prepared by a Florida Licensed Engineer who demonstrates that substituted product is capable of achieving the equivalent performance values of the design basis product.
- AN-2 Confirm the absence of reinforcing steel by drilling a 1/4" diameter pilot hole for each anchor in non-post-tension applications. For post-tensioned slabs, confirm the absence of reinforcing steel by nondestructive testing prior to drilling holes. Do not cut reinforcing steel without approval of the Structural Engineer.
- AN-3 Install in accordance with manufacturer's printed installation instructions (MPII) (ACI PRC-314, 26.7.2). Refer to MPII for appropriate drill size. Clean hole and remove dust.
- AN-4 Anchors listed below may not be used to substitute the specified anchors in a product's Notice of Acceptance (NOA) or Florida Product Approval.
- AN-5 Anchors shall be installed in concrete having a minimum age of 21-days at time of anchor installation (ACI CODE-318-19, 26.7.2(f))

POST-INSTALLED ANCHORS - ADHESIVE ANCHORS

- AN-6 For upwardly inclined or horizontal anchors, installer shall be certified by the ACI/CRSI Adhesive Anchor Installation Certification Program.
- AN-7 Install adhesive anchors in accordance with manufacturer's requirements for concrete age, temperature, moisture content, acceptable drilling methods, and hole preparation in conformance with ACI CODE-318-19, 26.7.1(i).
- AN-8 For anchoring into concrete: Adhesive anchors shall have been tested and qualified for use in accordance with ACI CODE-355.4 and ICC-ES AC308. Pre-approved standard cure time adhesives include Hilti RE500v3, DeWalt Pure 110+, and Simpson Set-XP.
- AN-9 For anchoring into grouted masonry: Adhesive anchors shall have been tested and qualified for use in accordance with ICC-ES AC58. Pre-approved anchors include Hilti HIT-HY 200-R, DeWalt PURE110+, and Simpson Set-XP.
- AN-10 Threaded rods for use with adhesive are galvanized ASTM F1554 Grade 36 U.O.N.

WOOD

- WD-1 All wood construction and connections shall conform to AITC "American Institute of Timber Construction" manual, and to NDS "National Design Specifications" for wood construction, and to the Florida Building Code, 8th Edition (2023), Chapter 23, and Plan Specifications or Specification Section 061100.
- WD-2 Wood species specified in section WD-3 are Bald Cypress (B.C.#2). Bald Cypress assumed Specific Gravity is 0.49 from typical range of 0.45 — 0.51. Round wood 5" — 6" diameter, bald cypress assumed to be equivalent to Douglas Fir Larch (North) No.2 Beams & Stringers. Round wood 10" — 12" diameter, Bald Cypress assumed to be equivalent to Douglas Fir Larch (North) No.2 Posts and Timbers. Assumptions are based on NDS 2018 Supplemental Table 4D for wood properties.
- WD-3 All member sizes are to be as shown on drawings and provide the following minimum properties:
- | Member | Species | Fb (psi) | Fv (psi) | Fc _{perp} (psi) | Fc _{Parallel} | E (psi) | E _{min} (psi) |
|-----------------|---------|----------|----------|--------------------------|------------------------|-----------|------------------------|
| A. 1"-4" Thick | B.C.#2 | 825 | 160 | 615 | 900 | 1,300,000 | 470,000 |
| B. 5"-6" Round | B.C.#2 | 875 | 170 | 625 | 600 | 1,300,000 | 470,000 |
| C. 10-12" Round | B.C.#2 | 725 | 170 | 625 | 700 | 1,300,000 | 470,000 |
| D. 1"-4" Thick | M.O.#2 | 800 | 170 | 800 | 625 | 900,000 | 330,000 |
| E. 5"-6" Round | M.O.#2 | 725 | 155 | 800 | 450 | 800,000 | 290,000 |
| F. 10-12" Round | M.O.#2 | 575 | 155 | 800 | 350 | 800,000 | 290,000 |
- WD-4 All wood in contact with concrete or masonry shall be pressure treated.
- WD-5 All bolts for bolted connections shall conform to ASTM A307, U.O.N. Use washers between wood and all bolt heads and nuts
- WD-6 All metal wood connectors shall be galvanized or stainless steel type 316
- WD-7 Do not splice structural members between supports unless otherwise indicated.
- WD-8 Where beams or columns are formed of two or more members, they shall be full length and fastened together per table on these drawings.

DEMOLITION

- DE-1 Contractor shall provide all temporary scaffolding, platforms, barricades, railings, screening, etc. necessary to protect existing facilities, structures and the public during demolition and erection of the new construction, as well as, for job safety. Job safety, construction and demolition procedures are the sole responsibility of the contractor, including removal of hazardous material prior to start of the work. Contractor is required to take all precautions to minimize vibration, noise, dust, and debris in all areas adjacent to areas of demolition.
- DE-2 Contractor shall coordinate with Owner the temporary suspension of use of any facility or portion thereof and the associated barricading requirements a minimum of 7 days prior to commencing work.
- DE-3 Contractor shall perform work in a manner which will not conflict with any operation which is to remain functional during the course of the project, until such operation is scheduled to be shut down.
- DE-4 Contractor shall coordinate with Owner the temporary suspension of use of any utility system, a minimum of 3 days prior to commencing work.
- DE-5 Install all temporary and/or permanent bracing and supports before demolishing any structural element. Wherever the Contractor is unsure of these requirements, the Contractor shall retain a Florida licensed engineer to design and inspect the bracing, support, and stability of the structure. Demolition sequence shall not be performed in a manner that will overload existing structural framing.
- DE-6 Provide temporary closure of all roof fascia, and other openings to protect building from exposure to undesirable elements until new construction is weatherproofed, at which time such temporary construction shall be removed. All temporary enclosures that are subject to wind loads are to be designed by a Florida licensed Delegated Engineer. Submit drawings and calculations for review.
- DE-7 Contractor shall engage a Florida licensed Delegated Engineer to design any temporary protected egress paths shown on the Architectural drawings.
- DE-8 At all locations where new construction will interface with existing elements, cut through existing structure in straight and true lines to insure a neat interface.
- DE-9 Drilling and saw cutting into the existing structure shall be performed in a manner which avoids damage to all existing reinforcing unless approved by the EOR. Prior to drilling or saw cutting, locate existing reinforcing by use of GPR or X-ray to avoid damage. Contractor shall submit penetration layout drawings for openings not shown on the structural drawings. Drawings shall indicate opening size, location, and approximate number of reinforcing steel bars that will be cut for review by the EOR.
- DE-10 All penetrations with widths less than existing reinforcing spacing shall be located to avoid damage to existing reinforcing. Penetrations with widths greater than existing reinforcing spacing shall be located to minimize the number of cut reinforcing.
- DE-11 Refer to details for trenching of existing slab on grade. The Contractor is solely responsible for all excavation procedures including lagging, shoring, and protection of adjacent property, structures, streets, and utilities in accordance with the requirements of the local building department and OSHA regulations. Do not excavate within one foot of the angle of repose of any soil bearing foundation or slab unless the foundation or slab is properly protected against settlement.
- DE-12 At all locations where the demolition of a concrete member leaves the ends of reinforcing steel exposed, provide the following:

- A. Chip concrete from around the steel to a depth of 1" (interior) or 1 3/4" (exterior).
- B. Cut off reinforcing steel not less than 3/4" (interior) or 1 1/2" (exterior) below the concrete surface.
- C. Fill the cavity flush with Sikadur 31 Hi-Mod Gel epoxy paste.
- DE-13 Upon completion of new construction under each phase, all demolished areas shall be restored to acceptable usage according to the contract documents as determined by the Architect/Engineer.
- DE-14 Remove completely from the site and legally dispose of all debris generated by the demolition work as the work progresses. Stockpiling of debris and burning of debris on premises is strictly prohibited.

EXISTING CONDITIONS

- EC-1 Limited Record Drawings of the existing building were available, or the available existing drawings do not match observed field conditions. Contractor is to field verify existing conditions before proceeding with fabrication and installation of new work.
- EC-2 Contractor shall verify existing conditions which may not be consistent with the existing structure depicted in the plans and details, or which may interfere with the installation of the new structure. Contractor shall assume that some details may have to be adjusted to fit existing conditions once uncovered.
- EC-3 Contractor is to notify the Architect of any discrepancies and shall include the necessary written descriptions, sketches, field measured dimensions, sizes and locations of existing members and photographs to describe the discrepancy.
- EC-4 If the existing conditions once uncovered represent questionable materials, some testing may be required by a testing laboratory to be hired by the owner. BNI will select areas and/or materials to be tested, and the strengthening shall be completed prior to connecting new members to the questionable existing members. The strengthening work will be paid by the Owner with a change order.



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PROFESSIONAL REGISTRATION



Christopher S. Childers, P.E.
Fla. Reg. No. 50812

Digitally signed by
Christopher S Childers
Date: 2026.02.12
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O'LENO STATE PARK

SHEET TITLE

STRUCTURAL NOTES

PROJECT TITLE

PAVILION RESTORATION

S101

Department of Environmental Protection
Division of Recreation and Parks
Bureau of Design and Construction

3800 Commonwealth Blvd., Tallahassee, FL 32309 (850) 245-2300

BLISS & NYITRAY, INC. STRUCTURAL ENGINEERS Certificate of Authorization No. 674 227 N Bronough St., Suite 7300 Tallahassee, Florida, 32301 T: 850-222-4454 www.bneengineers.com										PROFESSIONAL REGISTRATION CHARTERED PROFESSIONAL ENGINEER NO. 50812 STATE OF FLORIDA CHRISTOPHER S. CHILDERS P.E. Fla. Reg. No. 50812 Digitally signed by Christopher S Childers Date: 2026.02.12 11:50:41-05'00'										O'LENO STATE PARK SHEET TITLE STRUCTURAL PLAN SPECIFICATION PROJECT TITLE PAVILION RESTORATION										SHEET NO. S102																																																																																																																									
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SGN-1 These Plan Specifications are intended to be used for projects without Project Specification Books. If they are accidentally issued along with Project Specification Books, the Project Specification Books shall supersede the Plan Specifications.										RISK CATEGORY Risk Category II										1.0 MULTIPLIER 700 YEAR MRI										0.60 MULTIPLIER 50-YEAR MRI										0.42 MULTIPLIER 25-YEAR MRI										SGN-2 The structure is designed for lateral movement of H/400 or better. This drift is based on wind loads with a 700-year mean recurrence interval.										SGN-3 The floor and roof members are designed for a vertical deflection of L/240 for total load, and L/360 for live load at occupied floors and L/240 at roofs. It is advised that all interior partitions and exterior precast or curtain wall system be attached to the structure by the Delegated engineer, with a connection that would allow for vertical movement.																																																																																											
SHOP DRAWINGS AND SUBMITTALS										SSD-1 Material substitution shall not be submitted in the shop drawings without a substitution request being made to the Architect in advance and in writing, along with detailed substitution cost savings to be credited to the Owner. Upon approval by the Architect, the material substitution can be included in the submitted shop drawings.										SSD-2 All signed and sealed Shop Drawings prepared by a Delegated Engineer shall be accompanied by signed and sealed calculations. Shop Drawing submittals without calculations will be returned without review.										SSD-3 All structural Shop Drawings shall be submitted in PDF format to BNI for review and approval. Submittals shall be reviewed and electronically stamped by the Contractor as having "No Exception Taken".										SSD-4 Manufacturer Literature and Product Data shall be submitted in PDF format. The submittals will be stamped as "Received, for record only" by BNI and returned accordingly.										SSD-5 All structural Shop Drawings and calculations prepared by a Delegated Engineer shall be submitted in PDF format and electronically signed and sealed by the Delegated Engineer. Once the submittal is approved by BNI, then a signed and sealed hard copy shall be submitted to BNI to receive an "Approved" stamp so the submittal can be submitted to the building department.																																																																																																					
REINFORCED CONCRETE										SRC-1 Prepare and submit formwork shop drawings in compliance with ACI CODE-301 and ACI PRC-347. Formwork design for safety, structural adequacy, and efficiency is the Contractor's responsibility.										SRC-2 Prepare and submit shoring and reshoring shop drawings prepared by a Florida Licensed Engineer. Drawings shall contain the location, size, and type of all shoring, reshoring, mud sills, blocking, temporary lateral bracing and other accessories necessary to safely support and brace the structure during construction. Use a factor of safety of 2.0 for metal shores and 3.0 for wood shores. Shoring and reshoring design and construction is the sole responsibility of the Contractor and his Engineer.										SRC-3 Provide form-facing panels that will provide continuous, true, and smooth concrete surfaces.										SRC-4 Formwork for the sides of beams, walls, columns and similar elements, that does not support the weight of concrete may be removed after curing at not less than 50 degrees for 24 hours after placing concrete if concrete is hard enough to not be damaged by form removal.										SRC-5 Prepare and submit reinforcing steel shop drawings prepared according to ACI PRC-315 and ACI MNL-66. Include bar sizes, length, material, grade, bar schedules, stirrup spacing, bent bar diagrams, arrangement location of splices, length of splices, tie spacing, hoop spacing and supports of reinforcement.										SRC-6 Fabricate and install steel reinforcement according to CRSI's "Manual of Standard Practice."										SRC-7 Clean reinforcement of loose rust and mill scale, earth, ice, and other foreign materials that reduce bond to concrete.										SRC-8 Repair cut and damaged zinc coatings with zinc repair material according to ASTM A780.										SRC-9 Submit design mixes for each concrete mix for the following concrete grades: <table><tr><td>Element</td><td>Strength</td><td>Air Yes/No</td><td>Max. Aggregate Size</td><td>W/C or W/(C&P)*</td><td>Exposure Class*</td></tr><tr><td>Slab on Grade</td><td>3000</td><td>N</td><td>1"</td><td>0.64</td><td>S0</td></tr></table> * Letter in Exposure Category denotes Exposure Class: F: Freezing and thawing S: Sulfate W: Concrete in contact with water C: Corrosion protection of reinforcement										Element	Strength	Air Yes/No	Max. Aggregate Size	W/C or W/(C&P)*	Exposure Class*	Slab on Grade	3000	N	1"	0.64	S0	SRC-10 The minimum portland cement content of any concrete mix with slag cement is 280 lbs/CY, for all other concrete mixes, the minimum portland cement content is 423 lbs/CY. Limit percentage, by weight, of cementitious materials other than portland cement in concrete as follows: A. Concrete mixes containing fly ash: 15%-20%. B. Concrete mixes containing slag cement: 40%-50%. C. Concrete mixes containing fly ash and slag cement: 50% with fly ash or slag not exceeding 25%.										SRC-11 Provide concrete having entrained air content of 3%-5% except 1%-3% for concrete to receive a hard trowel finish (floor slabs).										SRC-12 Place concrete within 90 minutes of adding water to the mix. The Contractor may request additional time from the special inspector who can authorize an additional 30 minutes.										SRC-13 The amount of water added to the mix at the site is limited to the amount identified on the batch ticket as that being withheld at the batch plant. Water shall be added prior to initial discharge of concrete. No water can be added at the site if the batch ticket does not clearly identify the amount withheld at the plant. No water may be added once concrete placement has started.									
Element	Strength	Air Yes/No	Max. Aggregate Size	W/C or W/(C&P)*	Exposure Class*																																																																																																																																																		
Slab on Grade	3000	N	1"	0.64	S0																																																																																																																																																		
STRUCTURAL PLAN SPECIFICATIONS										SRC-14 Provide batch ticket for each ready-mixed batch discharged and used in the Work, indicating Project identification name and number, date, mix type and number, batch time, mix time, quantity, and amount of water added, and amount withheld at the plant. Record approximate location of final deposit in structure.										SRC-15 Concrete columns shall be cast at least 24 hours before horizontal members they support are cast. Exception: Tie columns and grout in masonry cells shall be cast at least 4 hours before beams are slabs are cast on top of masonry.										SRC-16 Deposit concrete continuously in one layer or in horizontal layers so that no new concrete will be placed on concrete that has hardened. Avoid inclined construction joints. Consolidate concrete with mechanical vibrating equipment. Do not use vibrators to transport concrete inside forms.										SRC-17 Cure concrete according to ACI 308.1 and as follows: A. Curing Compound: Apply to all concrete surfaces that are not permanently exposed. Provide a second coat applied at 90 degrees to initial application within three hours of initial application. B. Curing and Sealing Compound: Apply to permanently exposed concrete surfaces. Repeat process after 24 hours. C. Contractor shall confirm that curing compounds are compatible with flooring finishes and will not adversely affect the performance or warranty of the flooring.										SRC-18 Sample all concrete after water and admixtures have been added. Obtain at least one composite sample for each 100 CY or fraction thereof of each concrete mix placed daily. For slabs 6 inches or thinner, increase frequency to each 50 CY or fraction thereof of each concrete mix placed daily.										SRC-19 Cast and laboratory cure one set of four standard cylinder specimens and cast and field cure one set of four standard cylinder specimens for each composite sample. Take sample at point of placement for pumped concrete.										SRC-20 Test one specimen at 7-days and three at 28-days. If one of the first two 28-day test falls below specified strength, test the remaining specimen at 56-days.										SRC-21 Strength of each concrete mix will be satisfactory if the average of two cylinders at 28-days equals or exceeds the specified concrete strength, if not, then the average of any three consecutive strength tests (two at 28-days and one at 56-day) equals or exceeds specified compressive strength and no compressive strength test falls below specified compressive strength by 10% or 500 psi, whichever is less.										SRC-22 Provide test results to Architect, Engineer, and Concrete Company.										SRC-23 Contractor shall notify Architect and BNI of any concrete that fails to meet the design strength. Additional testing including destructive testing may be required to validate the in-place concrete strength. Testing with a Swiss Hammer is not an acceptable method of establishing in-place concrete strength.										SRC-24 CONCRETE CURING MATERIALS FOR TREATMENTS A. Liquid Membrane-Forming Cure and Seal Compound: VOC Complaint, conforming to ASTM C309, TYPE 1, Class B and ASTM C1315, Type 1, Class A or B. The compound shall be a clear styrene acrylate type, 25% solids content minimum, and have test data from an independent testing laboratory indicating to a maximum moisture loss of .040 grams per square cm. When applied at a coverage rate of 200 sq. ft. per gallon. B. Products: Subject to compliance with requirements, provide one of the following 1. ChemMasters, Inc. Polyseal WB. 2. Euclid Chemical Company (The); an RPM company; Super Diamond Clear VOX 3. Kaufman Products, Inc; Krystal 25 Emulsion. 4. Lambert Corporation; Crystal Clear Seal 1315 WB. 5. Laticrete International, Inc.; L&M Dress & Seal WB 25. 6. Metalcrete Industries; Metcure 30. 7. Nox-Crete Products Group; Cure & Seal 250E. 8. Right Pointe; Right Sheen WB30. 9. SpecChem, LLC; Cure & Seal WB 25. 10. TK Products; TK-Bright Kure & Seal 1315 VOC. 11. Vexcon Chemicals Inc.; StarSeal 1315. 12. W.R. Meadows, Inc.; Vocomp-30.										SRC-25 TOLERANCES FOR CONCRETE TREATMENTS A. Conform to ACI SPEC-117.										SCR-26 APPLICATION OF LIQUID CONCRETE FLOOR TREATMENTS A. Penetrating liquid floor treatment: prepare apply, and finish penetrating liquid floor penetrating liquid floor treatment according to manufacturer's written instructions. B. Remove curing compounds, sealers, oil, dirt, laitance, and other contaminants and complete surface repairs. C. Sealing coat: Uniformly apply a continuous sealing coat of curing and sealing compound to hardened concrete by power spray or roller according to manufacturer's written instructions.																					



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SHEET NO.

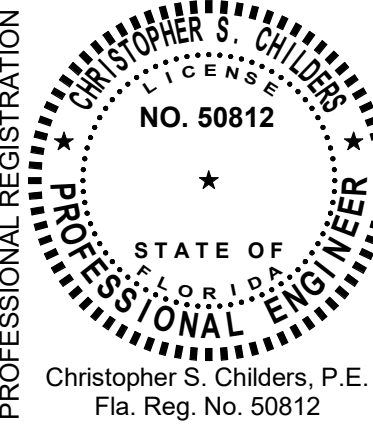
S111

O'LENO STATE PARK

ROOF WIND DIAGRAM

PAVILION RESTORATION

Digitally signed by
Christopher S Childers, P.E.
Date: 2026.02.12
11:51:03-05'00'



DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:



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Department of Environmental Protection
Division of Recreation and Parks
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02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS

This item has been digitally signed and sealed by Christopher S. Childers, P.E.
on 02/12/2026. Printed copies of this document are not considered signed and
sealed and the signature must be verified on any electronic copies.

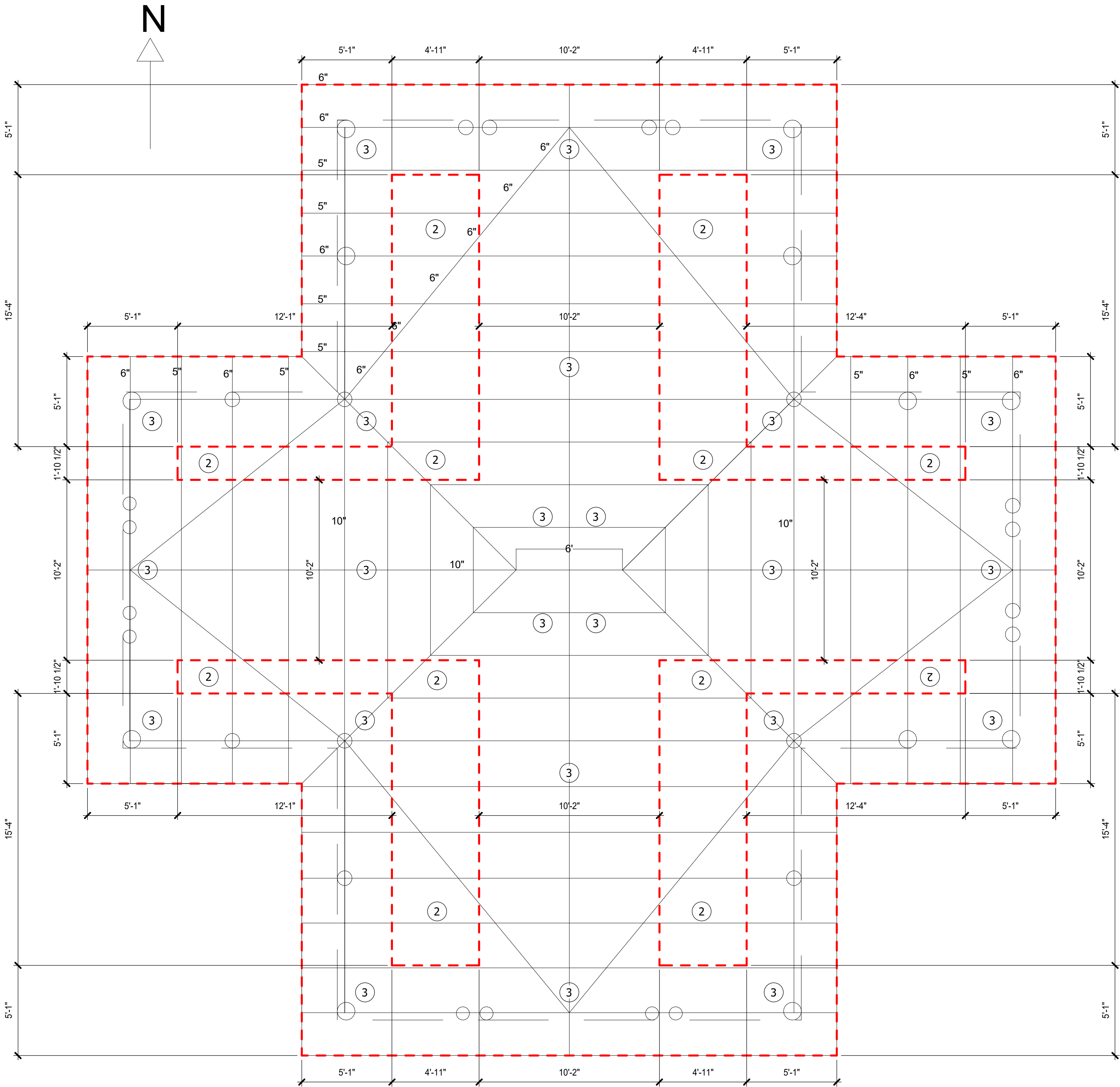
ZONES	ROOF WIND PRESSURES (PSF)					
	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
1	-12.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0
2	-18.0 / +10.0	-18.0 / +10.0	-18.0 / +10.0	-18.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0
3	-23.0 / +10.0	-23.0 / +10.0	-18.0 / +10.0	-18.0 / +10.0	-12.0 / +10.0	-12.0 / +10.0

WIND PRESSURE NOTES

- Numbers on this sheet are the components and cladding gross allowable pressures perpendicular to the surface (in P.S.F.) based on tributary area. Multiply service pressures by 1.67 to obtain W pressures for factored loads using strength design (ASCE 7-22 2.3).
- Pressures are derived from ASCE 7-22.
- Directionality factor Kd = .85
- Negative pressures act away from surface, positive pressures act toward surface.
- Dimensions shown on the skewed or radial elevations are measured parallel to surface.

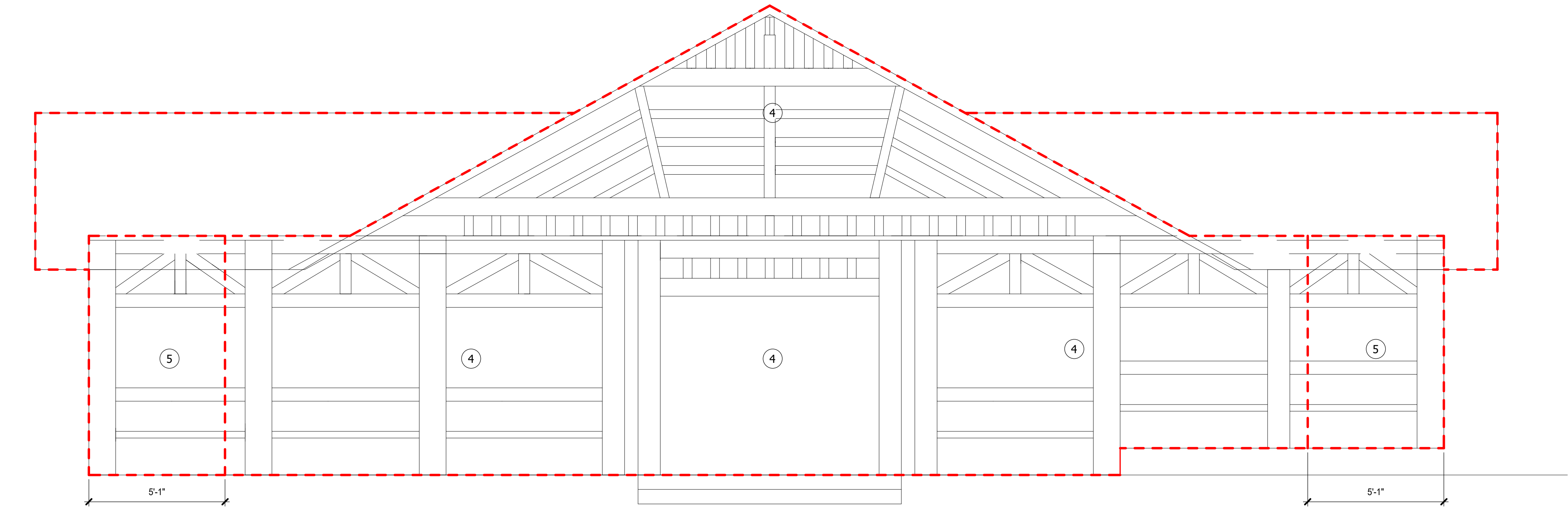
WIND PRESSURE LEGEND

- # Denotes wind pressure zone
- - - Denotes wind load separation

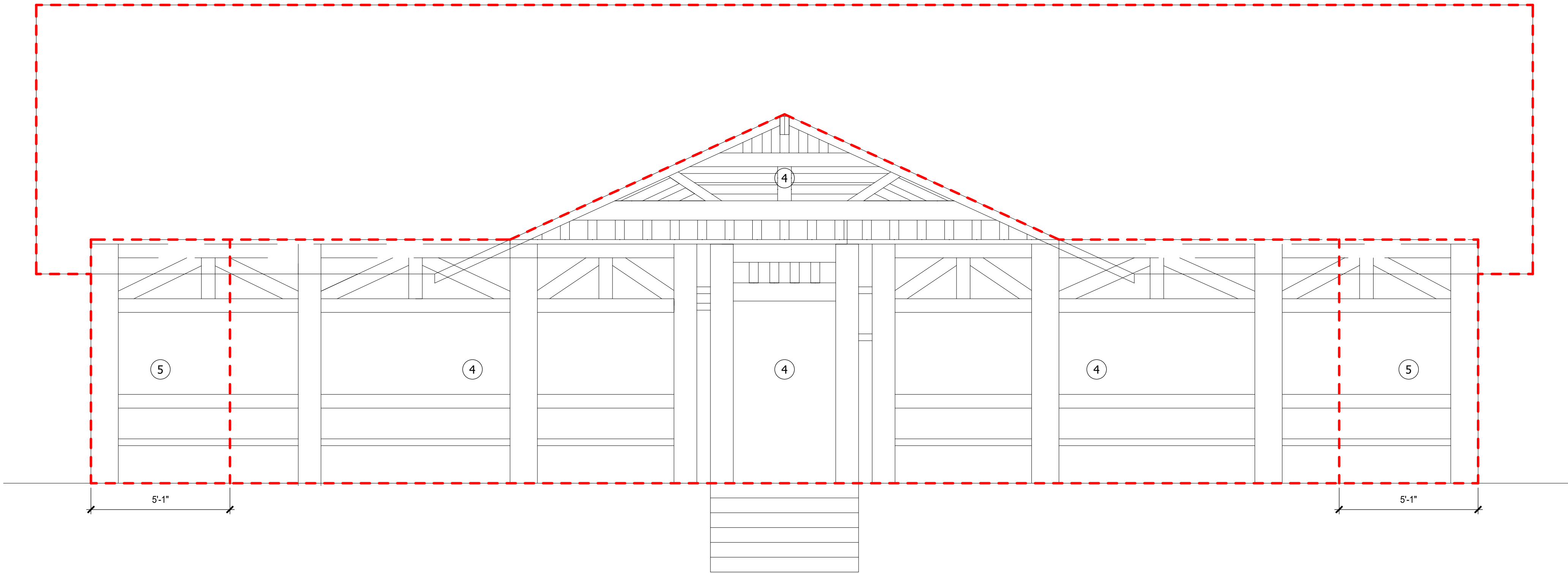


WL ROOF - WIND DIAGRAM

1/4" = 1'-0"



A WL NORTH ELEVATION - WIND
DIAGRAM
3/8" = 1'-0"



B WL EAST ELEVATION - WIND DIAGRAM
3/8" = 1'-0"

ZONE	WALL WIND PRESSURES (PSF)					
	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
4	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0
5	-29.3 / +16.0	-29.3 / +16.0	-25.6 / +16.0	-22.6 / +16.0	-20.0 / +16.0	-16.3 / +16.0

- WIND PRESSURE NOTES**
- Numbers on this sheet are the components and cladding gross allowable pressures perpendicular to the surface (in P.S.F.) based on tributary area. Multiply service pressures by 1.67 to obtain W pressures for factored loads using strength design (ASCE 7-22 2.3).
 - Pressures are derived from ASCE 7-22.
 - Directionality factor $K_d = .85$
 - Negative pressures act away from surface, positive pressures act toward surface.
 - Dimensions shown on the skewed or radial elevations are measured parallel to surface.

- WIND PRESSURE LEGEND**
- # Denotes wind pressure zone
 - - - Denotes wind load separation



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ISSUE DATE: 02/13/2026
COMP. FILE NO.:
STATE PROJECT NO. 6D201

DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:

SYMBOL
A
B

SYMBOL
C
D

REVISION
DATE

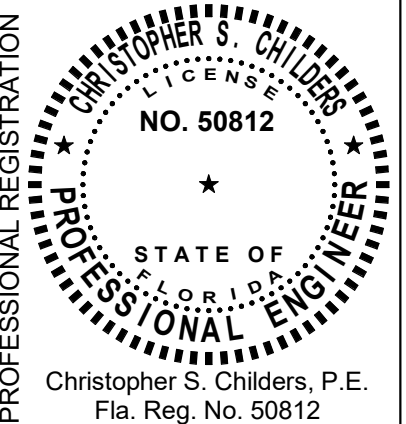
REVISION
DATE

SHEET NO.
S112

PROJECT TITLE
PAVILION RESTORATION

SHEET TITLE
NORTH / EAST WALL WIND
DIAGRAMS

O'LENO STATE PARK



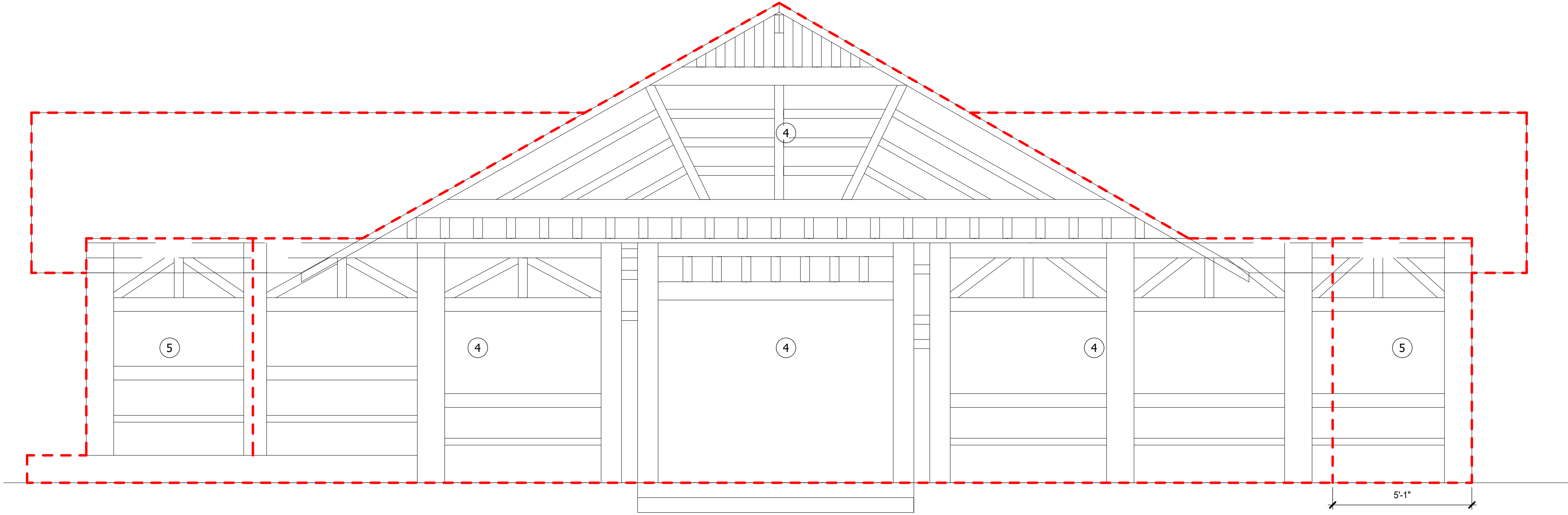
PROFESSIONAL REGISTRATION
Christopher S. Childers, P.E.
Fla. Reg. No. 50812

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Christopher S Childers
Date: 2026.02.12
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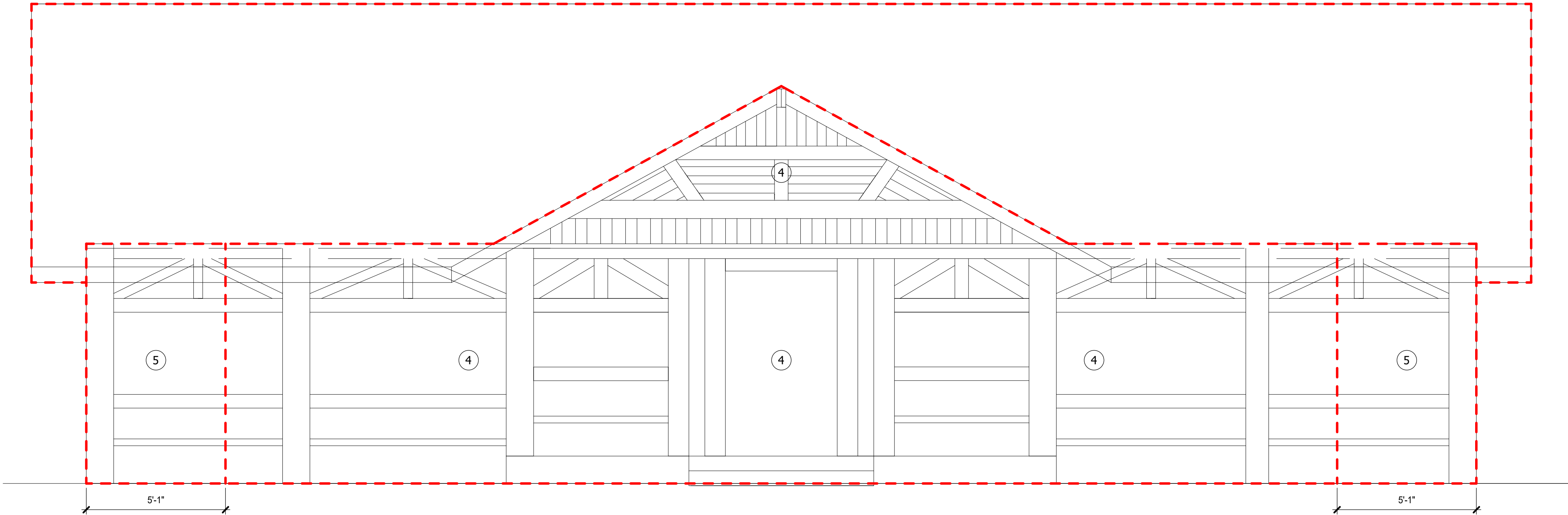


A WL SOUTH ELEVATION - WIND
DIAGRAM
3/8" = 1'-0"

ZONE	WALL WIND PRESSURES (PSF)					
	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
4	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0	-16.0 / +16.0
5	-29.3 / +16.0	-29.3 / +16.0	-25.6 / +16.0	-22.6 / +16.0	-20.0 / +16.0	-16.3 / +16.0

- WIND PRESSURE NOTES**
- Numbers on this sheet are the components and cladding gross allowable pressures perpendicular to the surface (in P.S.F.) based on tributary area. Multiply service pressures by 1.67 to obtain W pressures for factored loads using strength design (ASCE 7-22 2.3).
 - Pressures are derived from ASCE 7-22.
 - Directionality factor $K_d = .85$
 - Negative pressures act away from surface, positive pressures act toward surface.
 - Dimensions shown on the skewed or radial elevations are measured parallel to surface.

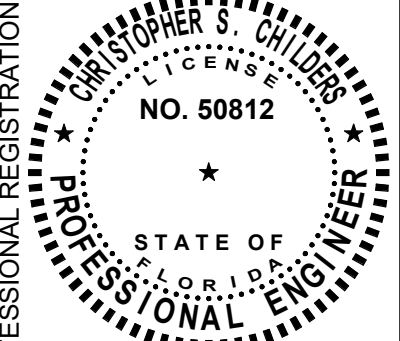
- WIND PRESSURE LEGEND**
- # Denotes wind pressure zone
 - - - Denotes wind load separation

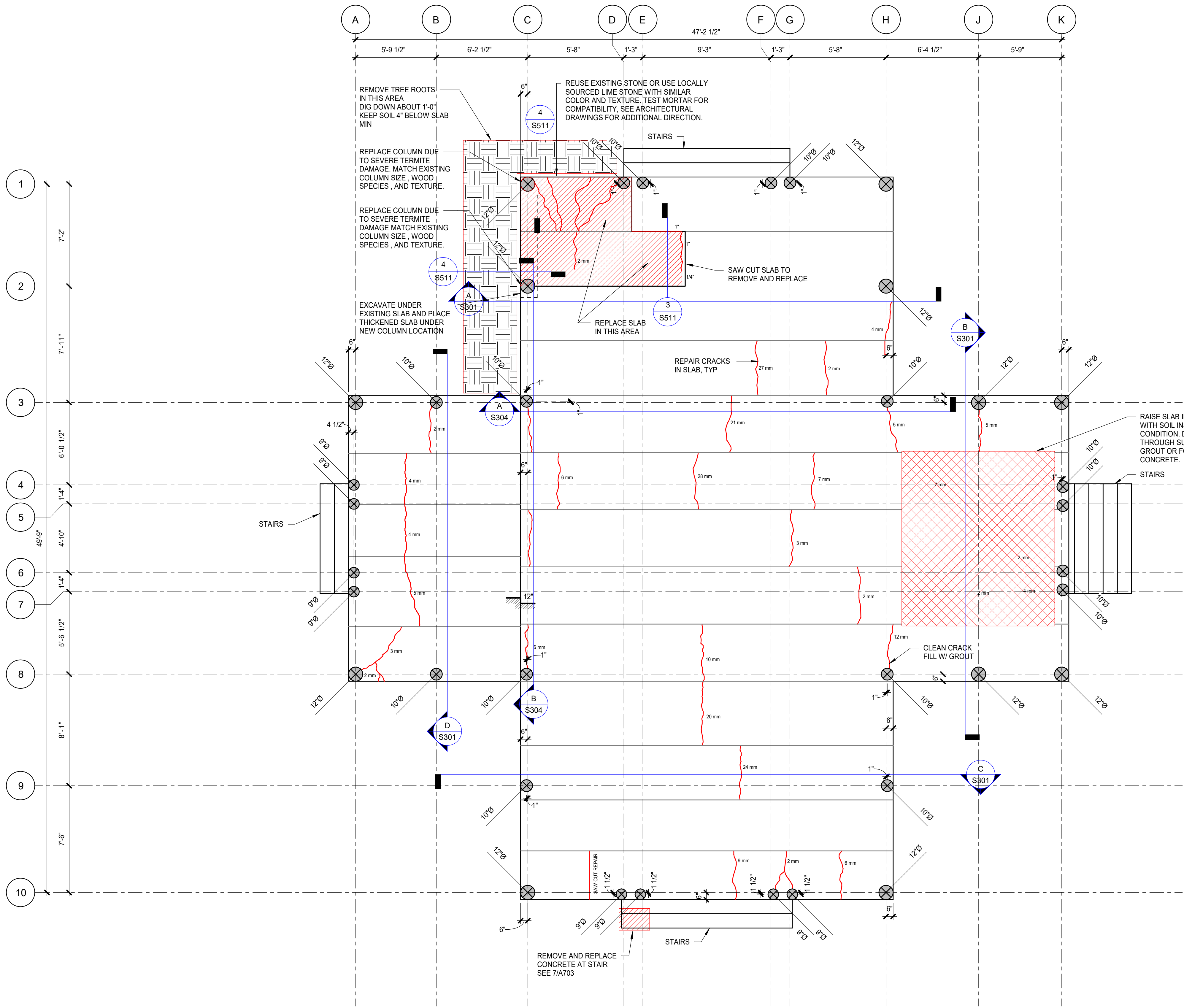


B WL WEST ELEVATION - WIND
DIAGRAM
3/8" = 1'-0"



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O'LENO STATE PARK		ISSUE DATE: 02/13/2026	SYMBOL	REVISION	DATE
SOUTH / WEST WALL WIND DIAGRAMS		COMP. FILE NO.:	A		
PROJECT TITLE PAVILION RESTORATION		STATE PROJECT NO. 6D201	B		
SHEET NO. S113		PROFESSIONAL REGISTRATION  Christopher S. Childers, P.E. Fla. Reg. No. 50812 Digitally signed by Christopher S Childers Date: 2026.02.12 11:51:42-05'00'			
		DODSTONE ARCHITECTS 3011-1 Powell Road Tallahassee, FL 32308 850.656.7266 #AA26001632 Department of Environmental Protection Division of Recreation and Parks Bureau of Design and Construction 3800 Commonwealth Blvd., Tallahassee, FL 32399 (850) 245-2300			



- NOTES:
1. ENTIRE STRUCTURE TO BE TREATED FOR TERMITES.
 2. SLAB CRACKS WIDER THAN 1.5 mm TO BE REPAIRED. SEE DETAIL 2/S511
 3. SLAB DISPLACEMENT OF MORE THAN 1/4" MUST BE REPAIRED.
 4. REPAIR SLAB VERTICAL DISPLACEMENTS BY LIFTING LOW AREAS WITH FOAM INJECTION.
 5. WHERE DRAWINGS CALL FOR COLUMNS TO BE REPLACED, FIELD MEASURE PRIOR TO ORDERING. CONTRACTOR IS REQUIRED TO CONNECT EXISTING STRUCTURAL MEMBERS TO THE COLUMNS MATCHING THE EXISTING CONSTRUCTION. DIAMETERS SHOWN ARE MINIMUMS. LARGER DIAMETERS CAN BE USED TO FACILITATE CONNECTIONS.
 6. ALL DIMENSIONS SHOWN ON DRAWINGS ARE FOR REFERENCE ONLY AND CONTRACTOR MUST FIELD VERIFY ALL DIMENSIONS PRIOR TO FABRICATING MEMBERS.

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O'LENO STATE PARK

SHEET TITLE

GROUND PLAN

PROJECT TITLE
PAVILION RESTORATION

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Christopher S Childers, P.E.
Date: 2026.02.12
11:52:02-05'00'

PROFESSIONAL REGISTRATION
CHRISTOPHER S. CHILDERS
NO. 50812
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Christopher S. Childers, P.E.
Fla. Reg. No. 50812

DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:

ISSUE DATE: 02/13/2026
COMP. FILE NO.:
STATE PROJECT No. 6D201

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DATE

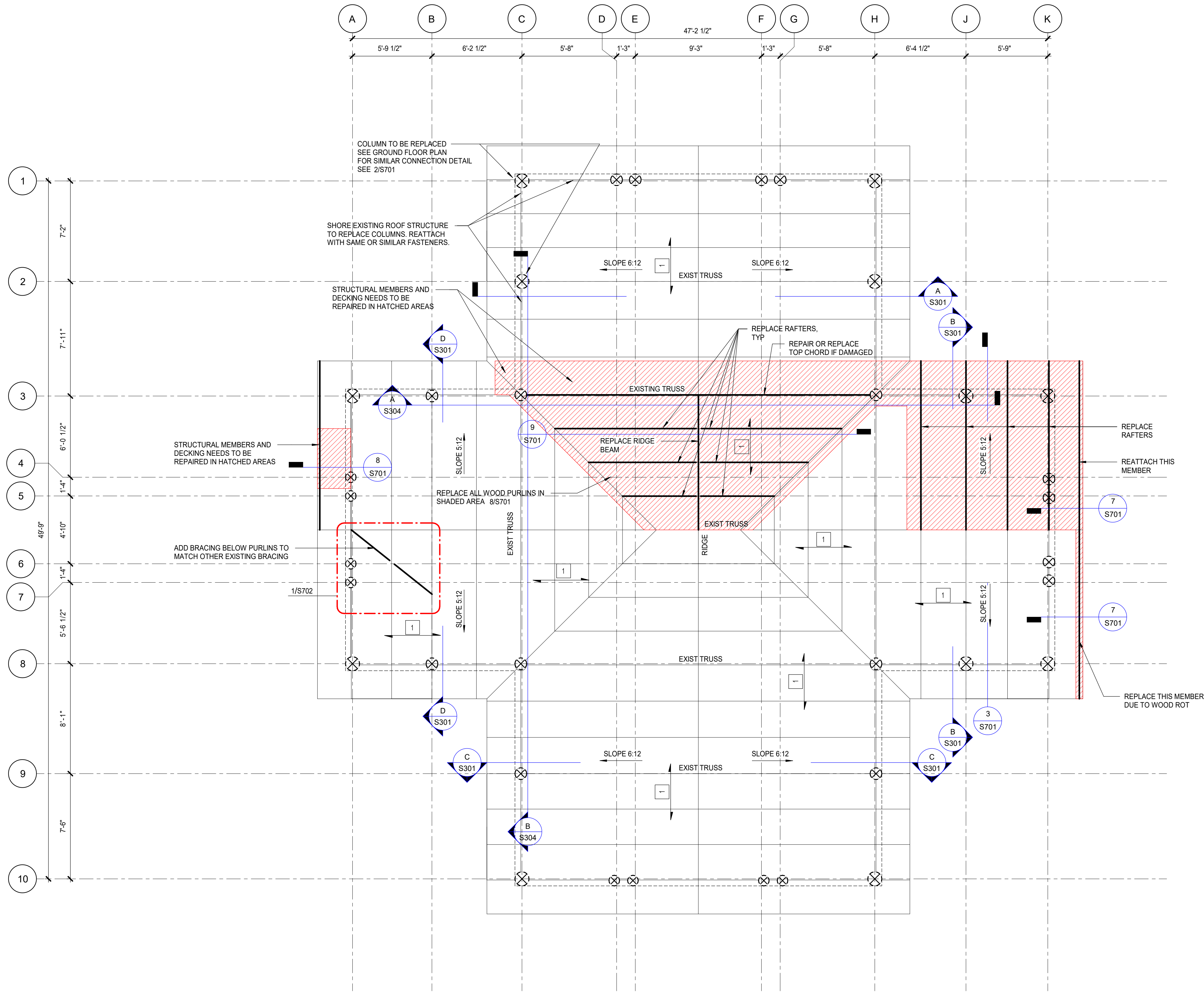
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REVISION
DATE

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Division of Recreation and Parks
Bureau of Design and Construction
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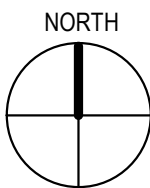
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02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS



EXISTING ROOF FRAMING PLAN

1/4" = 1'-0"



- NOTES:**
- ENTIRE STRUCTURE TO BE TREATED FOR TERMITES.
 - THE INTENT IS TO REUSE EXISTING WOOD TO THE GREATEST EXTENT POSSIBLE. IF CONTRACTOR HAS QUESTIONS TO THE SUITABILITY FOR REUSE OF EXISTING WOOD PURLINS THEN THEY SHOULD CONTACT THE A/E FOR RESOLUTION. IN GENERAL REPLACEMENT OF MEMBERS WOULD BE DUE TO EXISTING SPLITTING, ROT, AND TERMITE DAMAGE.
 - IN AREAS WHERE RAFTERS ARE BEING REPLACED, ADD NEW PURLINS OR SHIFT EXISTING PURLINS AS NEEDED FOR MAX 1'-0" O.C. SEPARATION.
 - ENTIRE EXISTING METAL ROOF DECK TO BE REPLACED.
 - WHERE DRAWINGS CALL FOR COLUMNS TO BE REPLACED, FIELD MEASURE PRIOR TO ORDERING. CONTRACTOR IS REQUIRED TO CONNECT EXISTING STRUCTURAL MEMBERS TO THE COLUMNS MATCHING THE EXISTING CONSTRUCTION. DIAMETERS SHOWN ARE MINIMUMS. LARGER DIAMETERS CAN BE USED TO FACILITATE CONNECTIONS.
 - ALL DIMENSIONS SHOWN ON DRAWINGS ARE FOR REFERENCE ONLY AND CONTRACTOR MUST FIELD VERIFY ALL DIMENSIONS PRIOR TO FABRICATING MEMBERS.
- PLAN KEYNOTES:**
- 1 GULF COAST 5V-1 CRIMP 26 GAUGE EXPOSED FASTENER GALVANIZED PANEL DECK SUPPORTED ON 1" WOOD PURLINS. FASTEN DECK W/ #9 1.5" FASTENER SCREWS IN 12"-12" PATTERN W/ FASTENER ROWS AT 12" O.C. SEE ARCH COLOR TO MATCH EXISTING.

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O'LENO STATE PARK
SHEET TITLE
ROOF FRAMING PLAN
PROJECT TITLE
PAVILION RESTORATION

Digitally signed by
Christopher S Childers, P.E.
Date: 2026.02.12
11:52:19-05'00'

PROFESSIONAL REGISTRATION
Christopher S. Childers, P.E.
Fla. Reg. No. 50812

DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:

ISSUE DATE: 02/13/2026
COMP. FILE NO.:
STATE PROJECT No. 6D201

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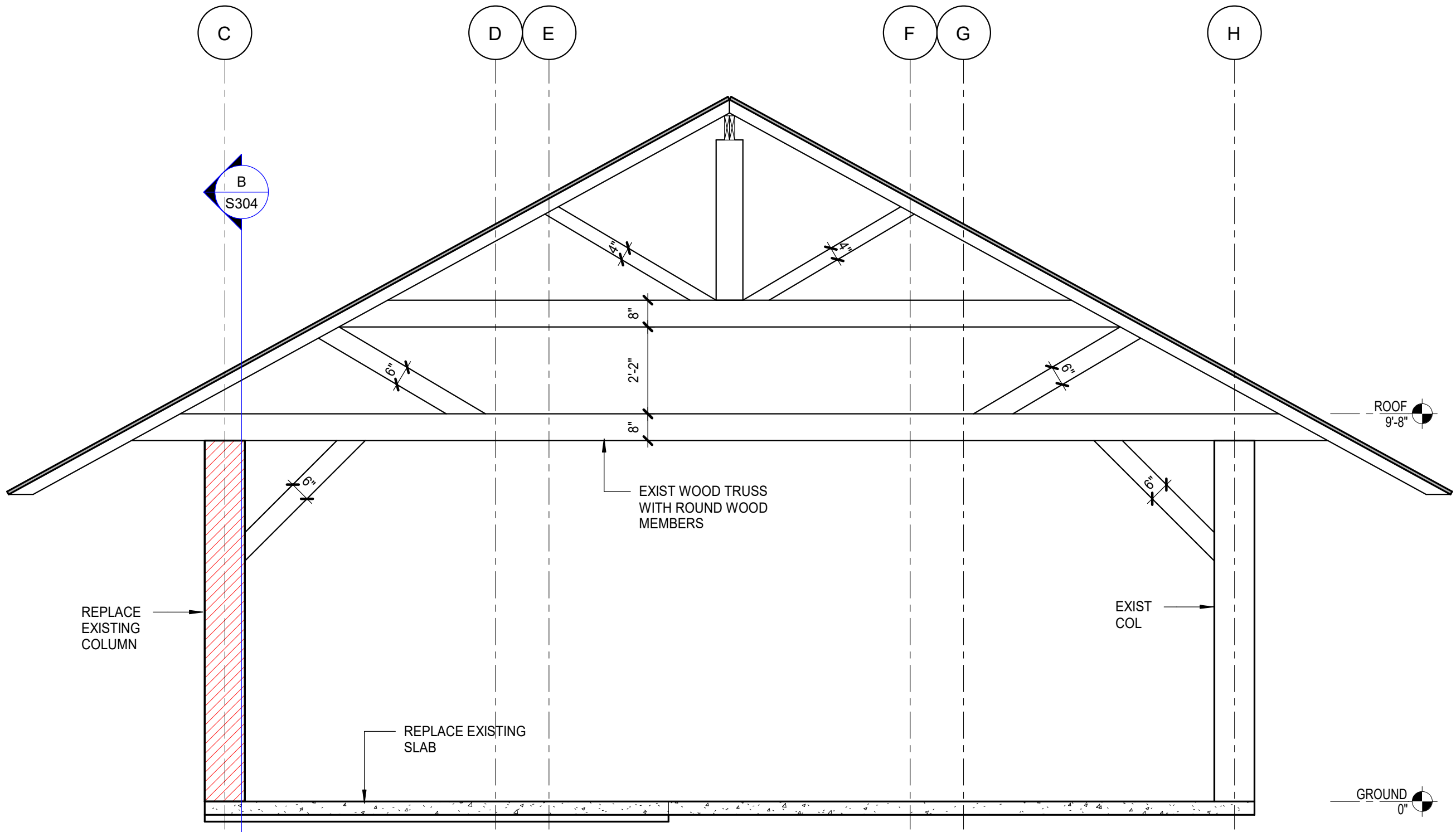
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DATE

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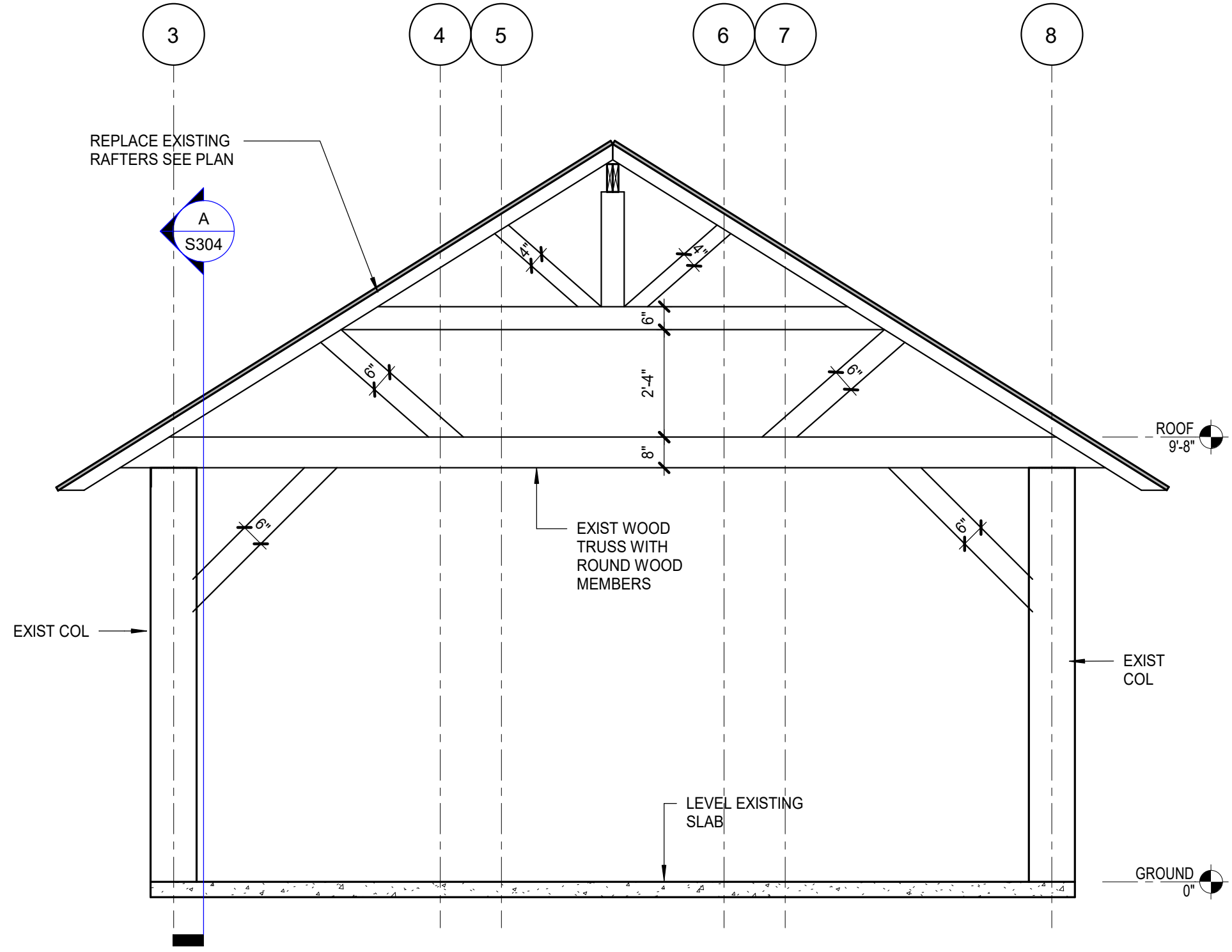
REVISION
DATE

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Division of Recreation and Parks
Bureau of Design and Construction
3800 Commonwealth Blvd., Tallahassee, FL 32399 (850) 245-2300

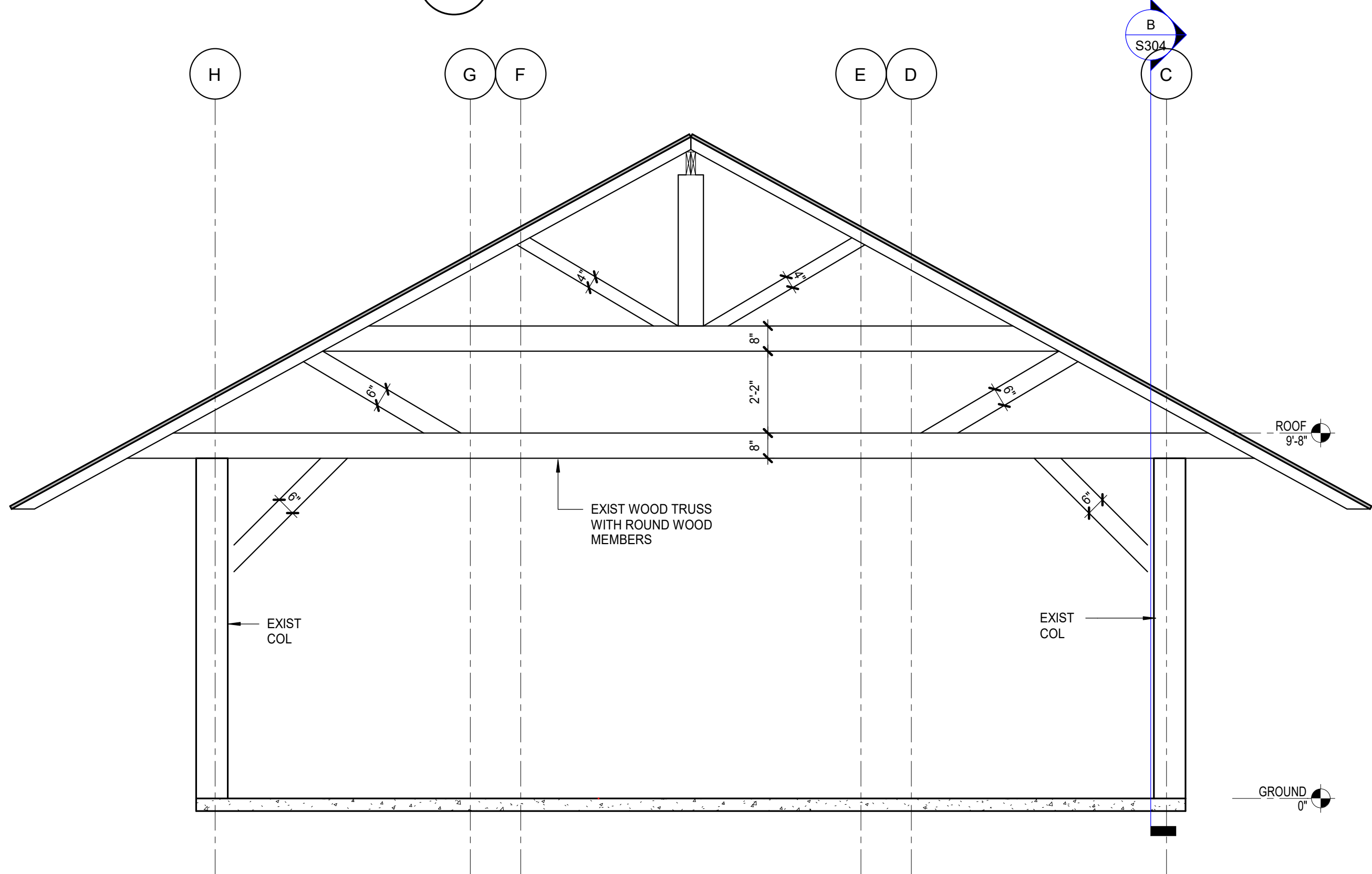
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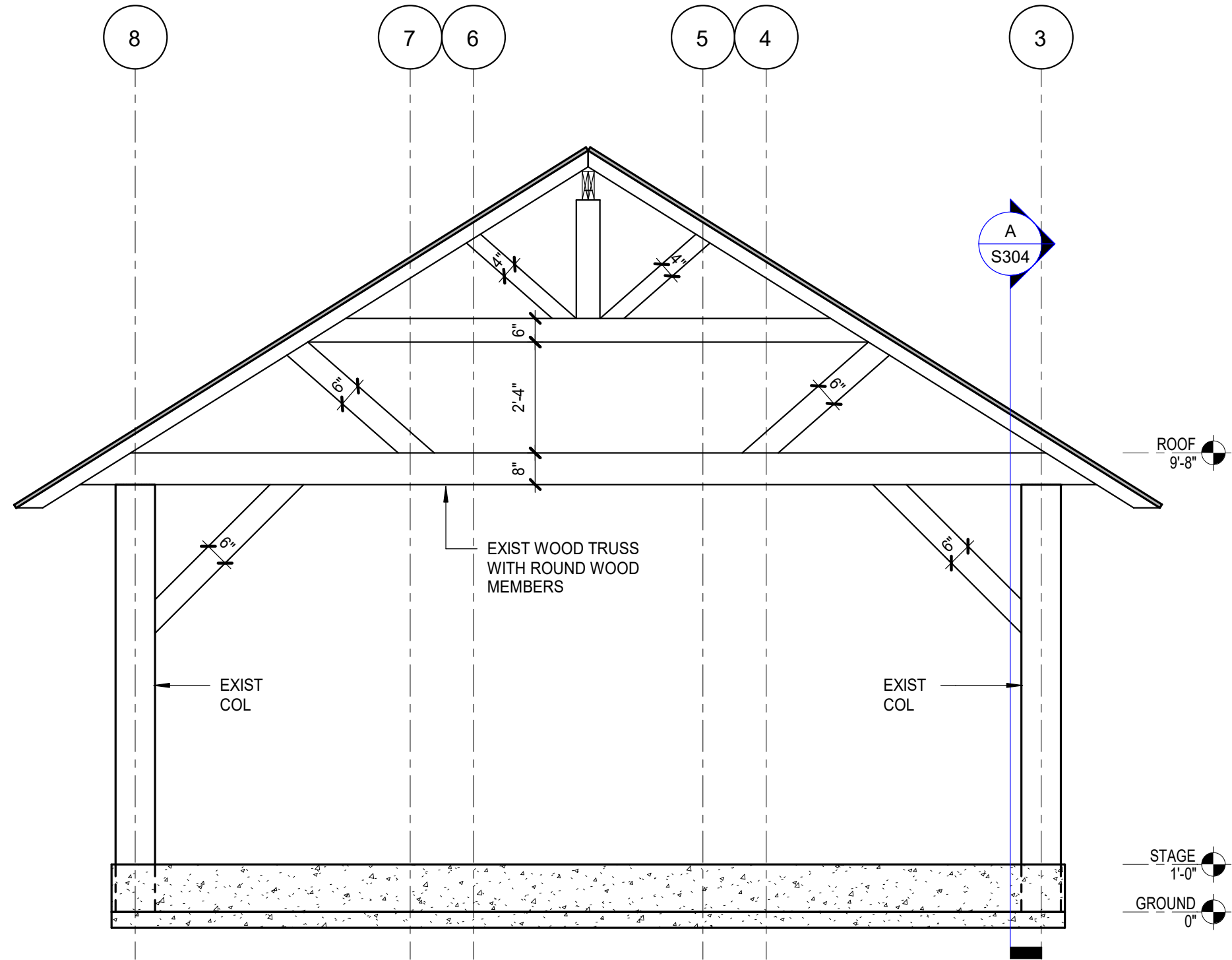
A BUILDING SECTION
3/8" = 1'-0"



B BUILDING SECTION
3/8" = 1'-0"



C BUILDING SECTION
3/8" = 1'-0"



D BUILDING SECTION
3/8" = 1'-0"



SHEET NO.	O'LENO STATE PARK		ISSUE DATE: 02/13/2026		DESIGNER: CSC	PROFESSIONAL REGISTRATION  Digitally signed by Christopher S Childers Date: 2026.02.12 11:54:27-05'00'	PROJECT TITLE		PAVILION RESTORATION
	BUILDING SECTIONS		COMP. FILE NO.:	DRAWN BY: TLC	SHEET TITLE				
			STATE PROJECT No. 6D201	REVIEWED BY: CSC					
				Consultant:					
SYMBOL		A	SYMBOL		C	REVISION		DATE	
		B			D				



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SHEET NO.

S302

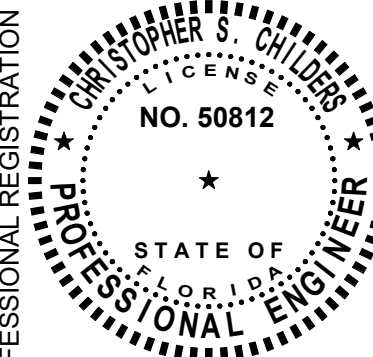
O'LENO STATE PARK

BUILDING ELEVATIONS

PAVILION RESTORATION

Digitally signed by
Christopher S Childers
Date: 2026.02.12
11:54:47-05'00'

Christopher S. Childers, P.E.
Fla. Reg. No. 50812



DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:

DODSTONE
ARCHITECTS

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Department of Environmental Protection
Division of Recreation and Parks
Bureau of Design and Construction
3800 Commonwealth Blvd., Tallahassee, FL 32399 (850) 245-2300

SYMBOL
A
B

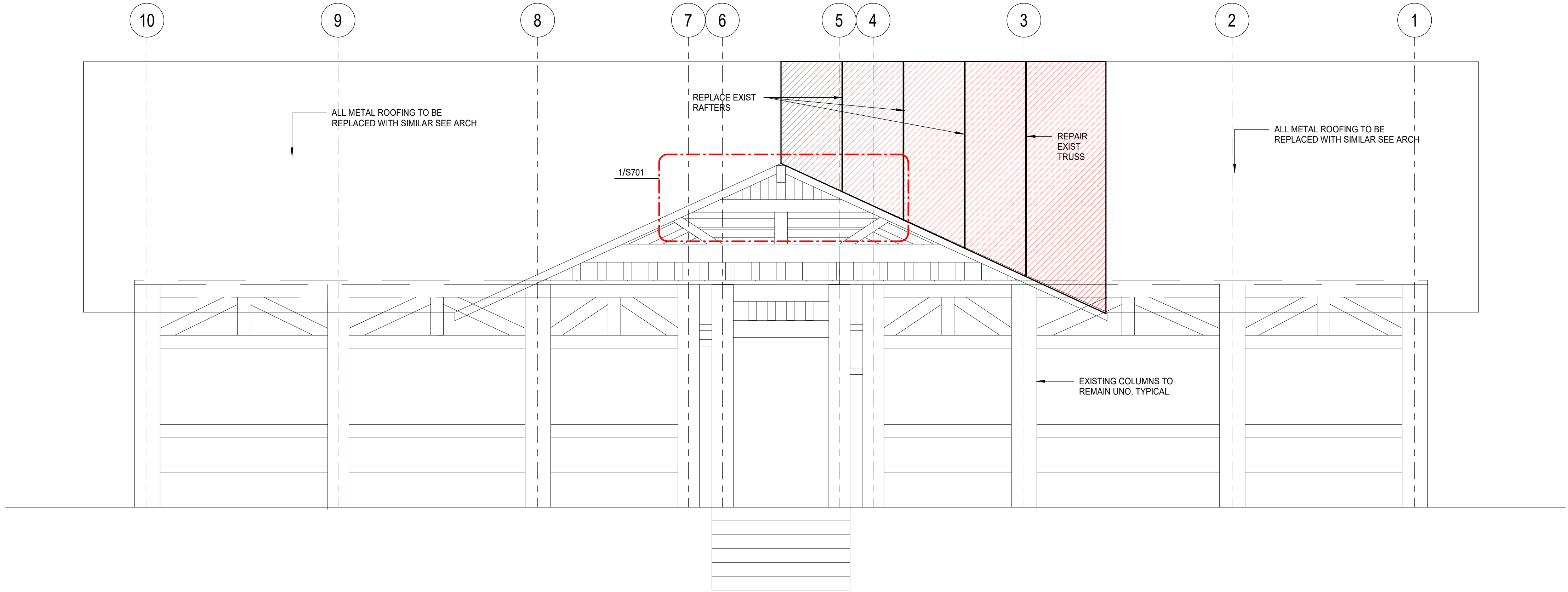
REVISION

DATE

SYMBOL
C
D

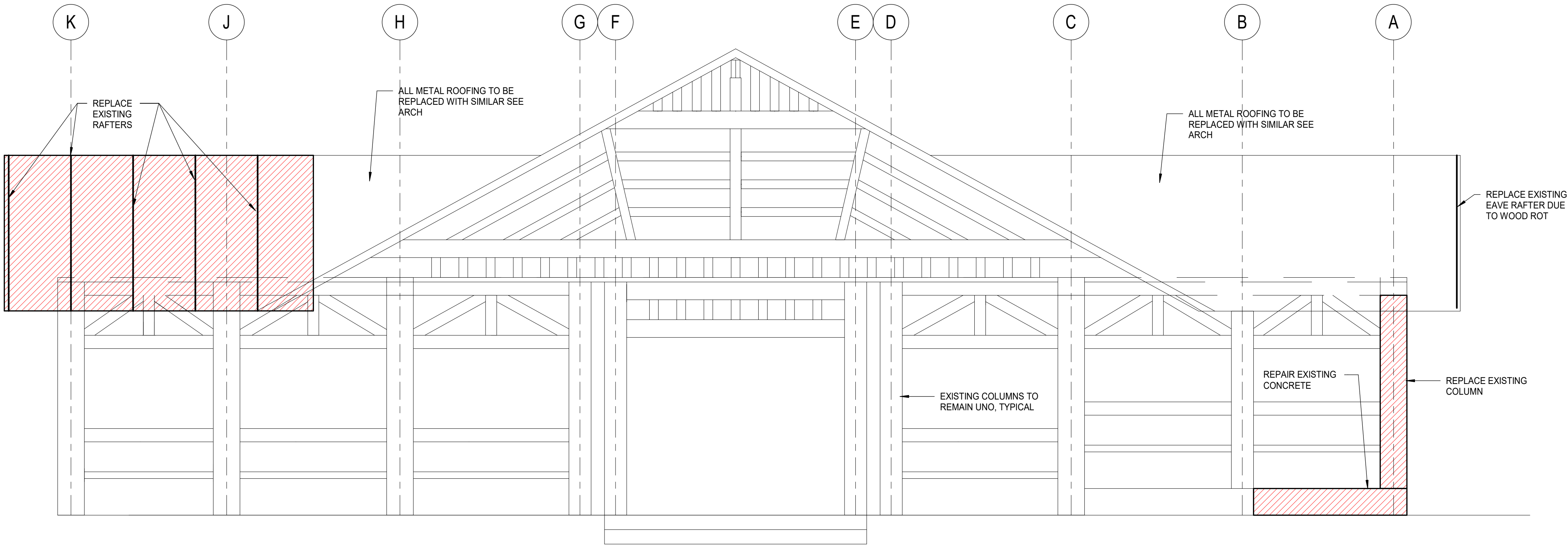
REVISION

DATE



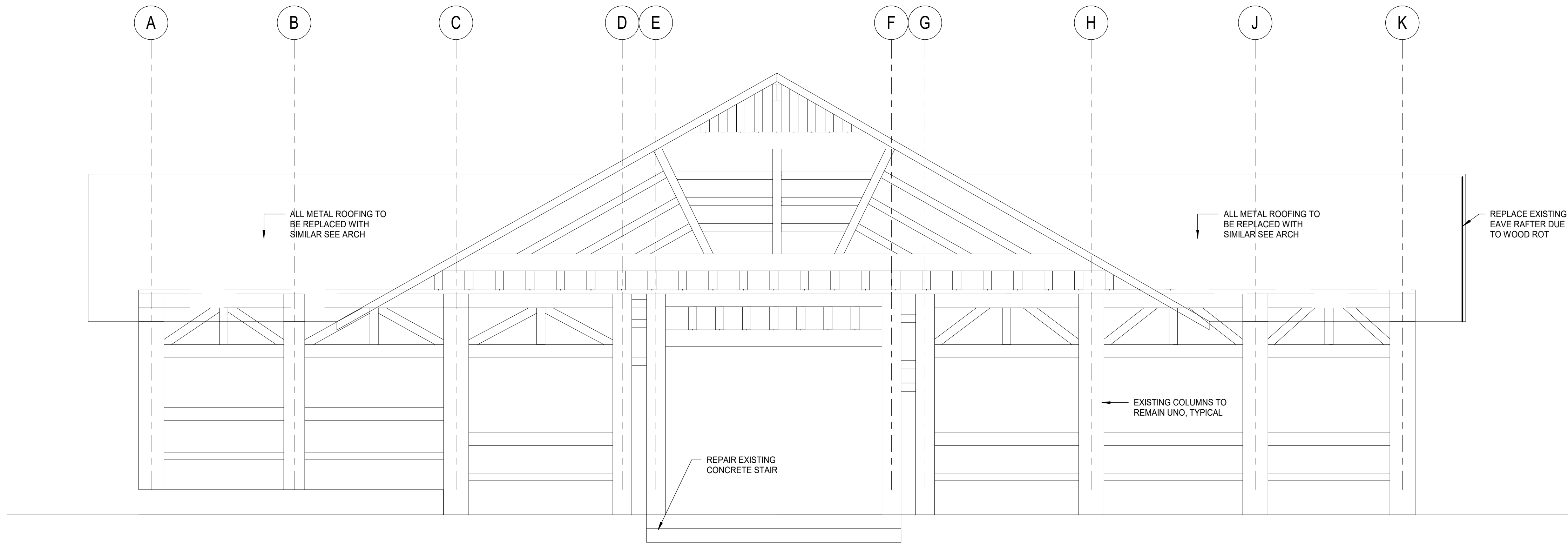
B EAST ELEVATION

3/8" = 1'-0"

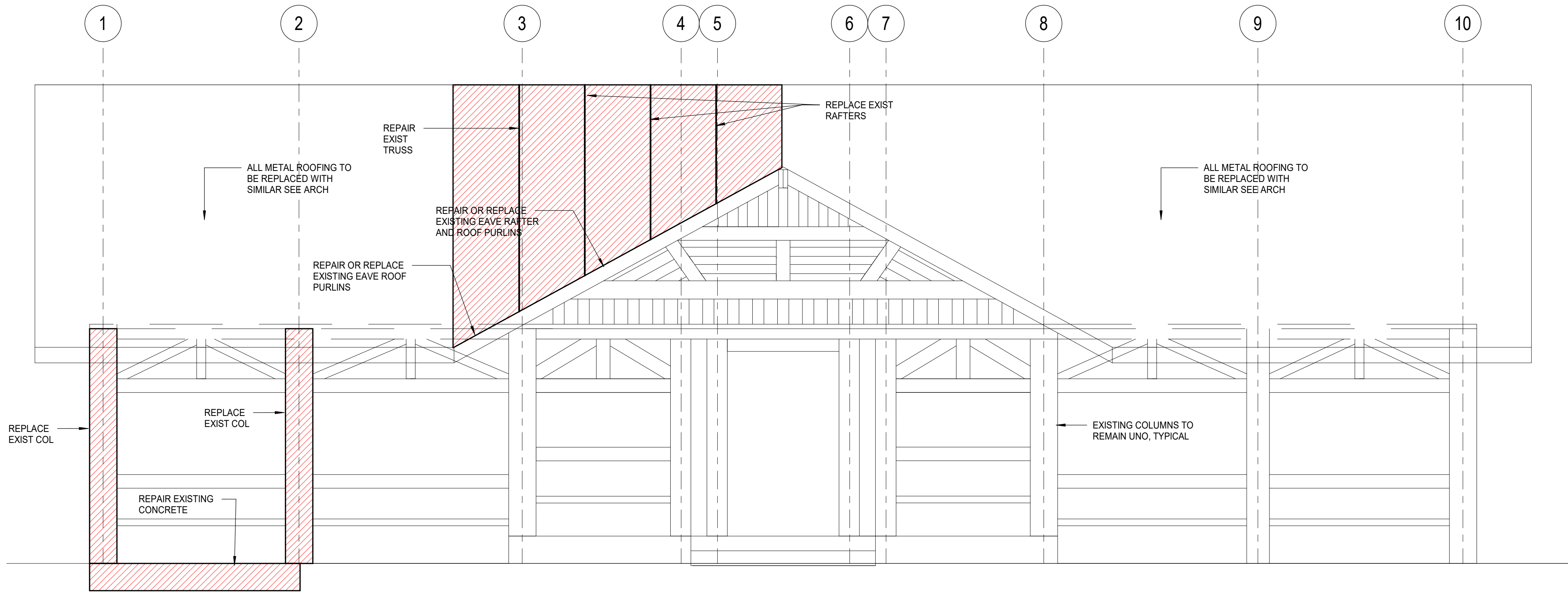


A NORTH ELEVATION

3/8" = 1'-0"



A SOUTH ELEVATION
3/8" = 1'-0"

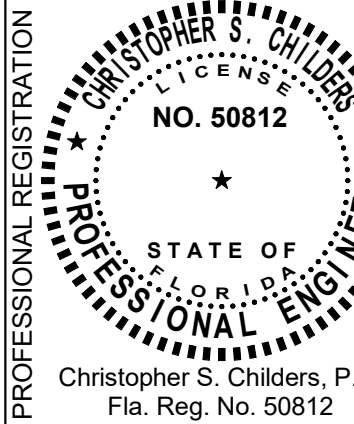


B WEST ELEVATION
3/8" = 1'-0"



O'LENO STATE PARK
BUILDING ELEVATIONS
PAVILION RESTORATION

SHEET NO.
S303



Digitally signed by
Christopher S Childers
Date: 2026.02.12
11:55:13-05'00'

DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:
DODSTONE ARCHITECTS
3011-1 Powell Road | Tallahassee, FL 32308
850.656.7326 | #A226001632

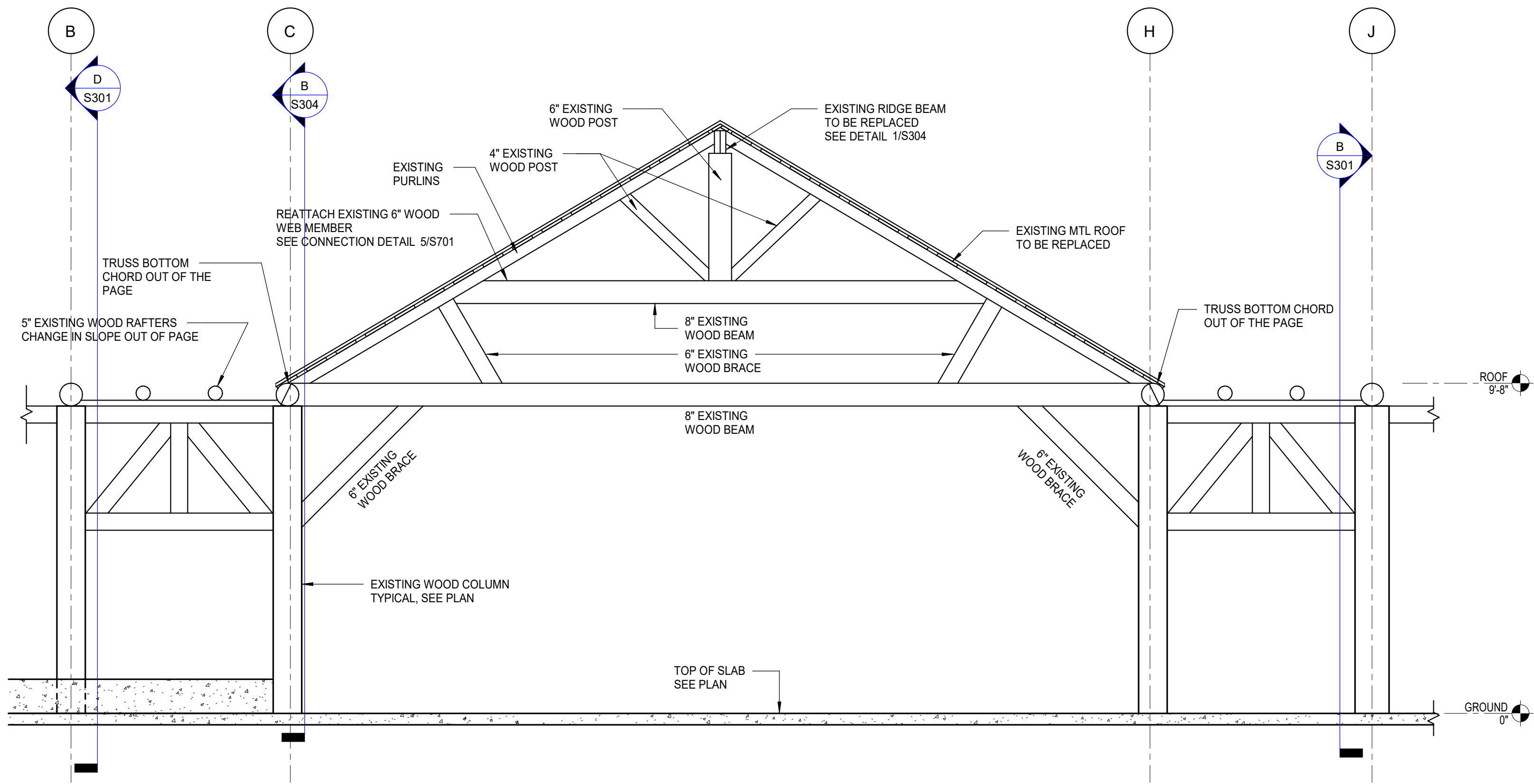
ISSUE DATE: 02/13/2026
COMP. FILE NO.:
STATE PROJECT No. 6D201

SYMBOL
A
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SYMBOL
C
D
REVISION
DATE
DATE

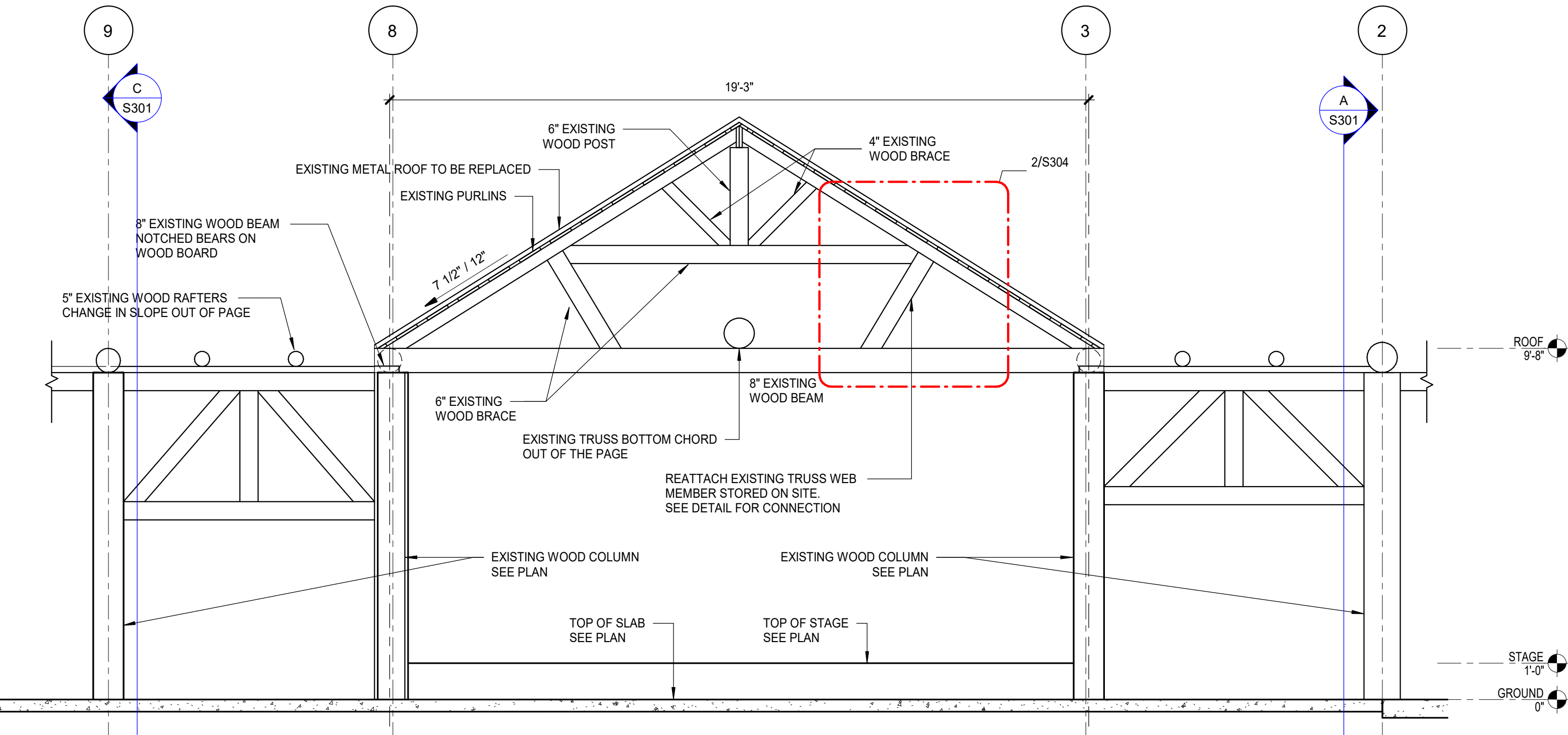
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Division of Recreation and Parks
Bureau of Design and Construction
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02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS

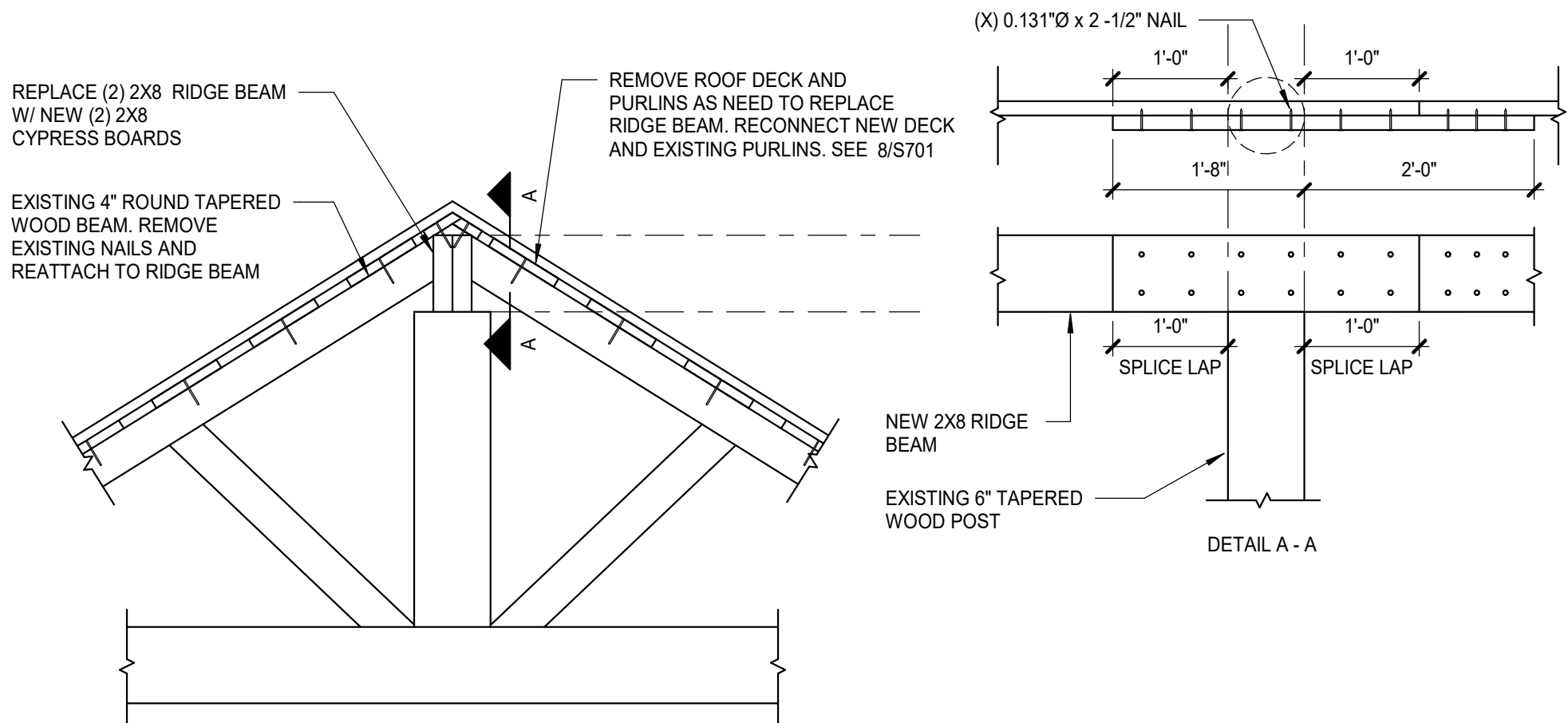
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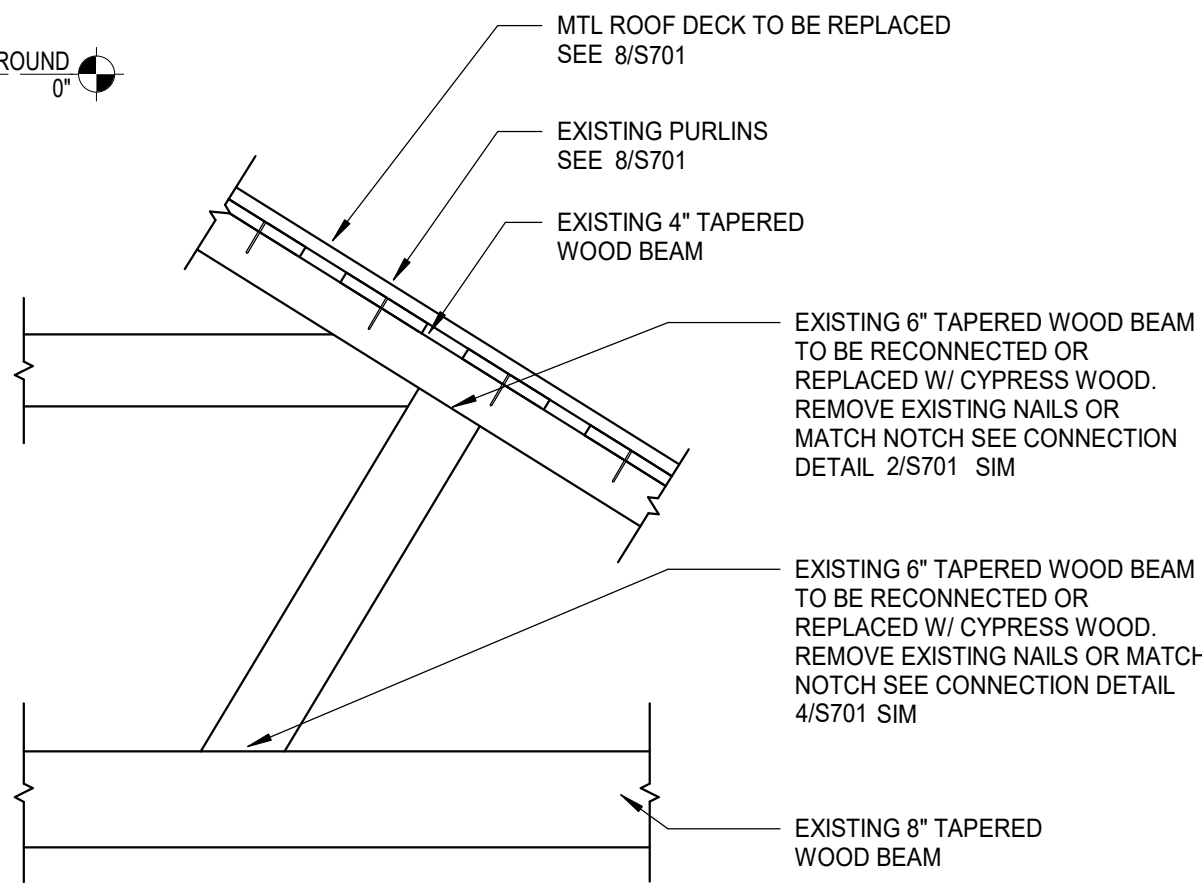
A SECTION AT TRUSS W/ ROOF DAMAGE
3/8" = 1'-0"



B SECTION AT TRUSS W/ MISSING KNEE BRACE
3/8" = 1'-0"



1 DETAIL CONNECTION - DAMAGED RIDGE BEAM AT TRUSS
3/4" = 1'-0"



2 DETAIL CONNECTION - MISSING WEB MEMBER
3/4" = 1'-0"



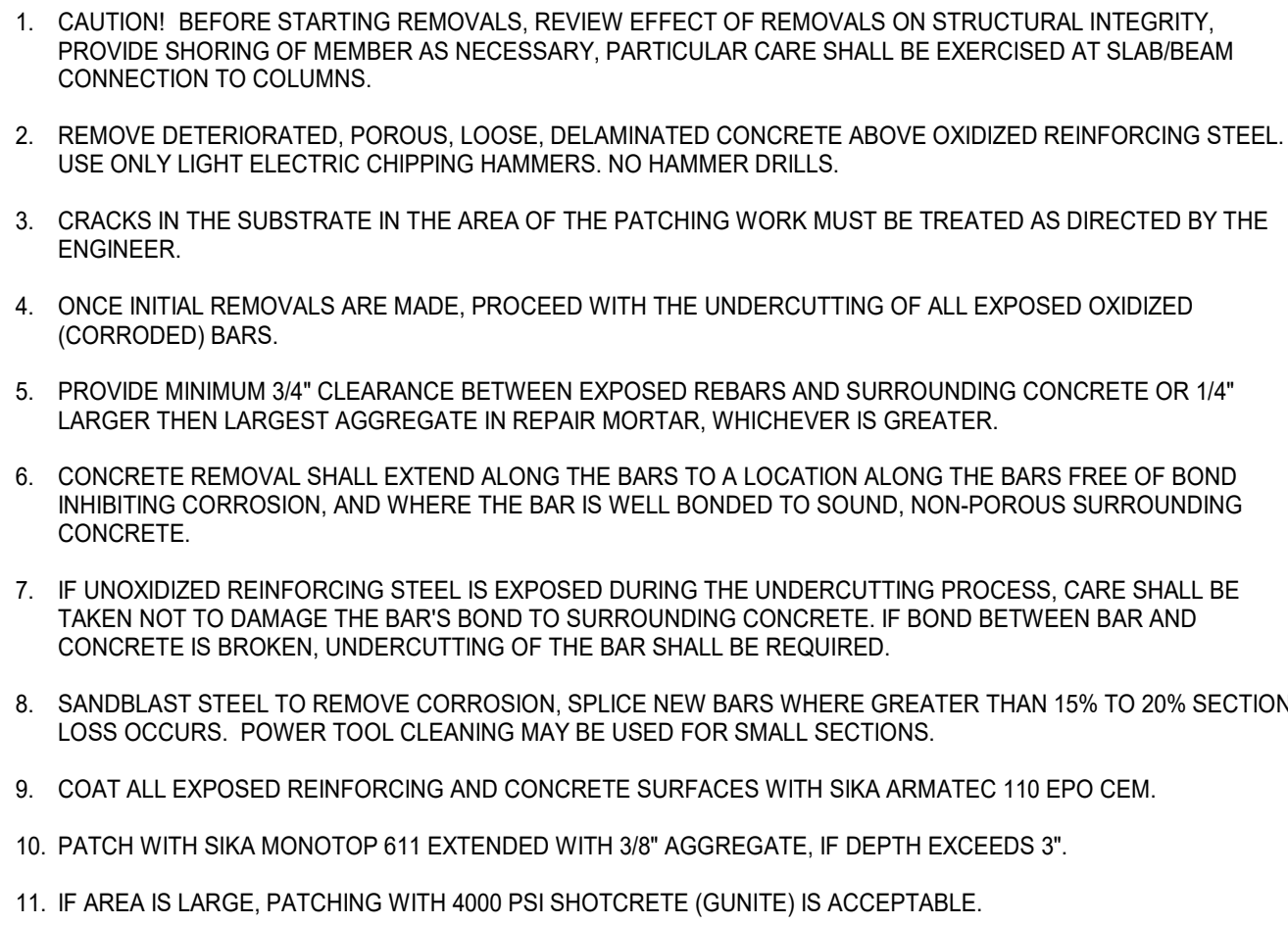
REVISION	DATE	SYMBOL	SYMBOL	REVISION	DATE	SYMBOL	SYMBOL	ISSUE DATE: 02/13/2026	COMP. FILE NO.:	STATE PROJECT NO. 6D201	DESIGNER: CSC	TLC	REVIEWED BY: CSC	Consultant:	PROFESSIONAL REGISTRATION	Digitally signed by Christopher S Childers, P.E. Date: 2026.02.12 11:57:31-05'00'	PROJECT TITLE	SHEET NO.
																	O'LENO STATE PARK	S304
																	BUILDING SECTIONS	
																	PAVILION RESTORATION	

Department of Environmental Protection
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Bureau of Design and Construction
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DODSTONE
ARCHITECTS
3011-1 Powell Road | Tallahassee, FL 32308
850.656.7326 | #AA29001632

PROFESSIONAL REGISTRATION
Christopher S. Childers, P.E.
Fla. Reg. No. 50812
NO. 50812
STATE OF FLORIDA
PROFESSIONAL ENGINEER
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02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS



LARGER SPALLS DEEPER THAN 1/2"
OR REINFORCING IS EXPOSED

1. ROUT OPEN CRACKS, APPROXIMATELY 3/8"x3/8".
2. CLEAN ALL ROUTED CRACK SURFACES TO RECEIVE SEALANT. THE ROUTED CRACK & ADJACENT SUBSTRATE MUST BE CLEAN, DRY, SOUND, & FREE OF ALL CONTAMINANTS. BLOW OUT ROUTED CRACK USING COMPRESSED AIR LINE EQUIPPED WITH AN OIL TRAP.
3. INSTALL SIKAFLEX-1A, A POLYURETHANE BASED, NON-SAG ELASTOMERIC SEALANT.
4. THE SEALANT MUST BE GUNNED INTO THE ROUTED CRACK TO A MINIMUM DEPTH OF 1/4". ENSURE STEADY FLOW OF SEALANT TO AVOID AIR ENTRAPMENT. TOOL AS REQUIRED TO PROPERLY FILL THE ROUTED CRACK.
5. ADHERE TO ALL LIMITATIONS & CAUTIONS FOR THE POLYURETHANE SEALANT AS STATED IN THE MANUFACTURER'S LITERATURE.
6. LEAVE WORK AREA IN A NEAT, CLEAR CONDITION WITHOUT EVIDENCE OF SPILLOVERS INTO ADJACENT AREAS.


$$\frac{3}{4}'' = 1'-0''$$

$$\frac{3}{4}'' = 1'-0''$$


PROFESSIONAL REGISTRATION										CONSULTANT									
																			
Christopher S. Childers, P.E. Fla. Reg. No. 50812										30111 Powell Road Tallahassee, FL 32308 850.695.7326 #AA26001632									
Digitally signed by Christopher S Childers										Department of Environmental Protection									
Date: 2026.02.12										Division of Recreation and Parks									
11:57:56-05'00'										Bureau of Design and Construction									
PROJECT TITLE										3800 Commonwealth Blvd., Tallahassee, FL 32309 (850) 245-2300									
O'LENO STATE PARK																			
SHEET TITLE																			
TYPICAL CONCRETE DETAILS																			
PROJECT TITLE																			
PAVILION RESTORATION																			
SHEET NO.																			
S511																			

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02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS



SHEET NO.		O'LENO STATE PARK		PROFESSIONAL REGISTRATION	
SHEET TITLE		WOOD DETAILS			
PROJECT TITLE		PAVILION RESTORATION		Christopher S. Childers, P.E. Fla. Reg. No. 50812	
Digitally signed by Christopher S Childers Date: 2026.02.12 12:08:20-05'00'		DESIGNER : CSC		PROFESSIONAL ENGINEER	
DRAWN BY: TLC		ISSUE DATE: 02/13/2026		SYMBOL	
REVIEWED BY: CSC		COMP. FILE No.:		SYMBOL	
STATE PROJECT No.: 6D201		REVISION		DATE	
Consultant :		REVISION		DATE	
DODSTONE ARCHITECTS		SYMBOL		DATE	
30111 Powell Road Tallahassee, FL 32308 850.656.7398 #AAS28001632		SYMBOL		DATE	
Department of Environmental Protection		SYMBOL		DATE	
Division of Recreation and Parks		SYMBOL		DATE	
Bureau of Design and Construction		SYMBOL		DATE	
3800 Commonwealth Blvd., Tallahassee, FL 32399 (850) 245-2300		SYMBOL		DATE	

02-13-2026 CONSTRUCTION DOCUMENTS w/ OWNER COMMENTS

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BLISS & NYTRAY, INC.
STRUCTURAL ENGINEERS
Certificate of Authorization No. 674
7227 N Bronough St., Suite 7300
Tallahassee Florida, 32301
T: 850-222-4454
www.bnengineers.com

SHEET NO.
S702

O'LENO STATE PARK

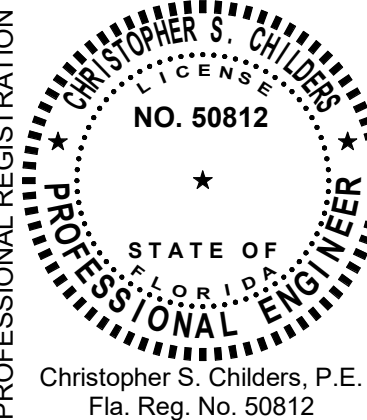
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WOOD DETAILS

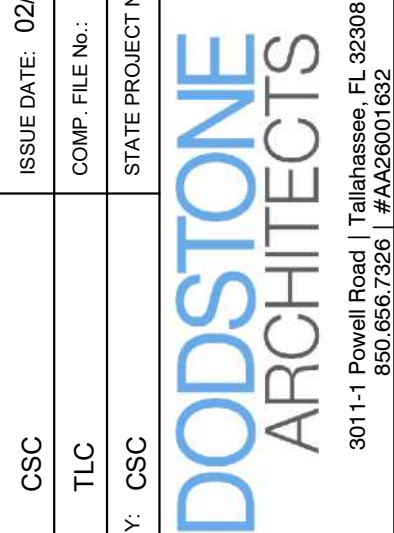
PROJECT TITLE

PAVILION RESTORATION

Digitally signed by
Christopher S Childers
Date: 2026.02.12
12:08:47-05'00'



DESIGNER: CSC
DRAWN BY: TLC
REVIEWED BY: CSC
Consultant:



ISSUE DATE: 02/13/2026
COMP. FILE No.:
STATE PROJECT No. 6D201

SYMBOL
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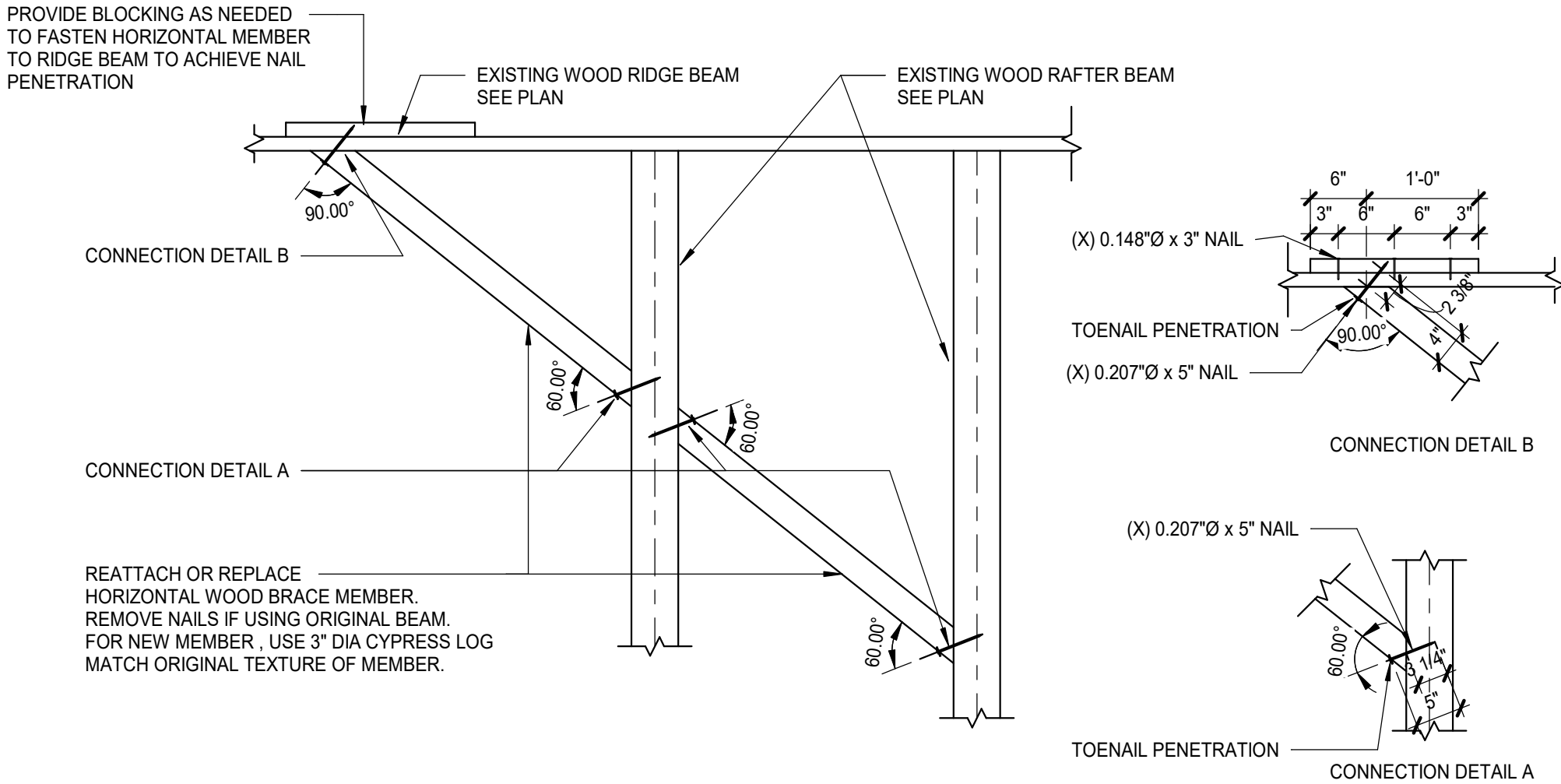
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1
HORIZONTAL BM TO RAFTER BRACING
CONNECTION
PLAN
3/4" = 1'-0"