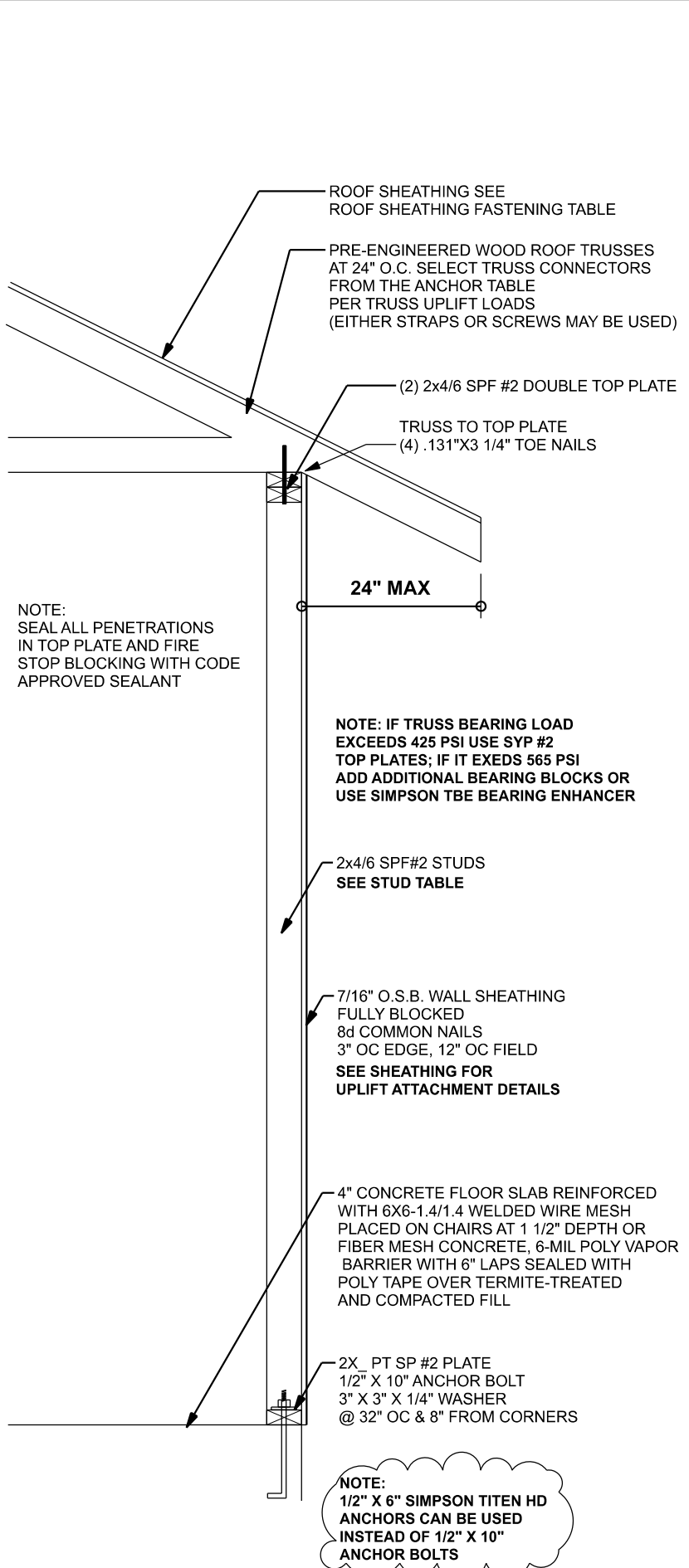
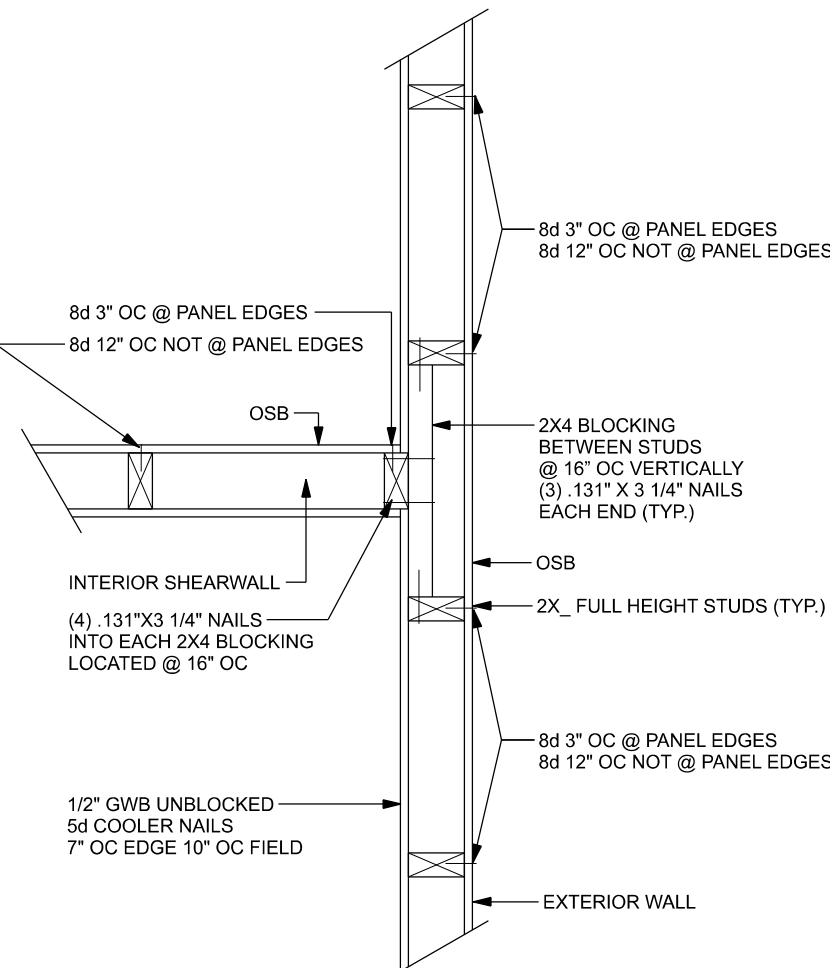
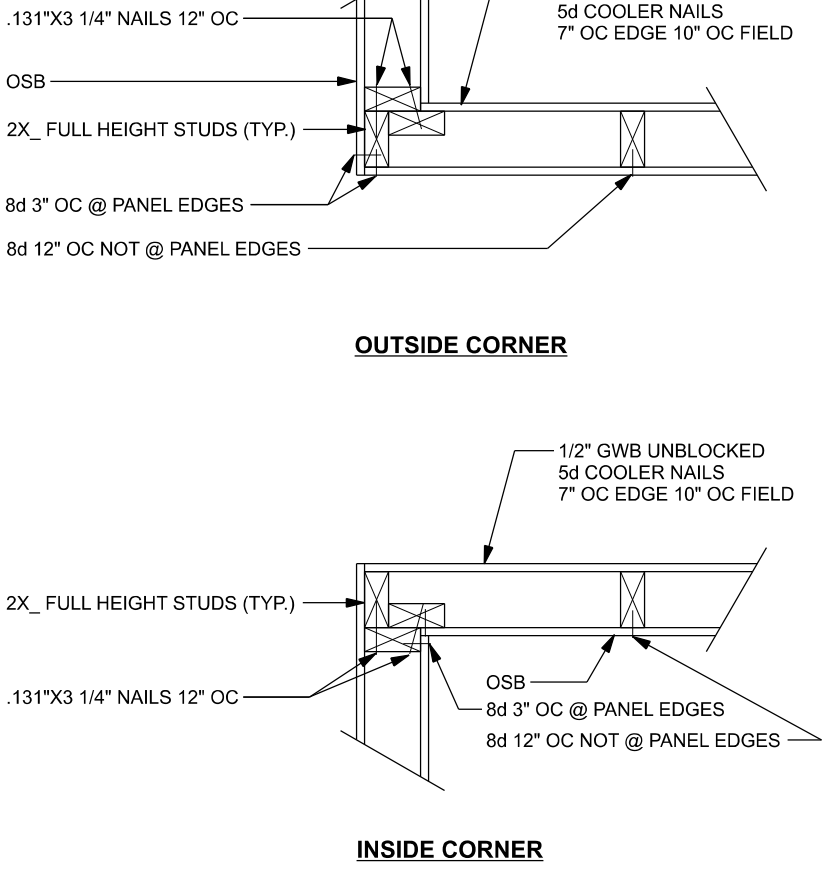




ONE STORY WALL SECTION
SCALE: 3/4\"/>



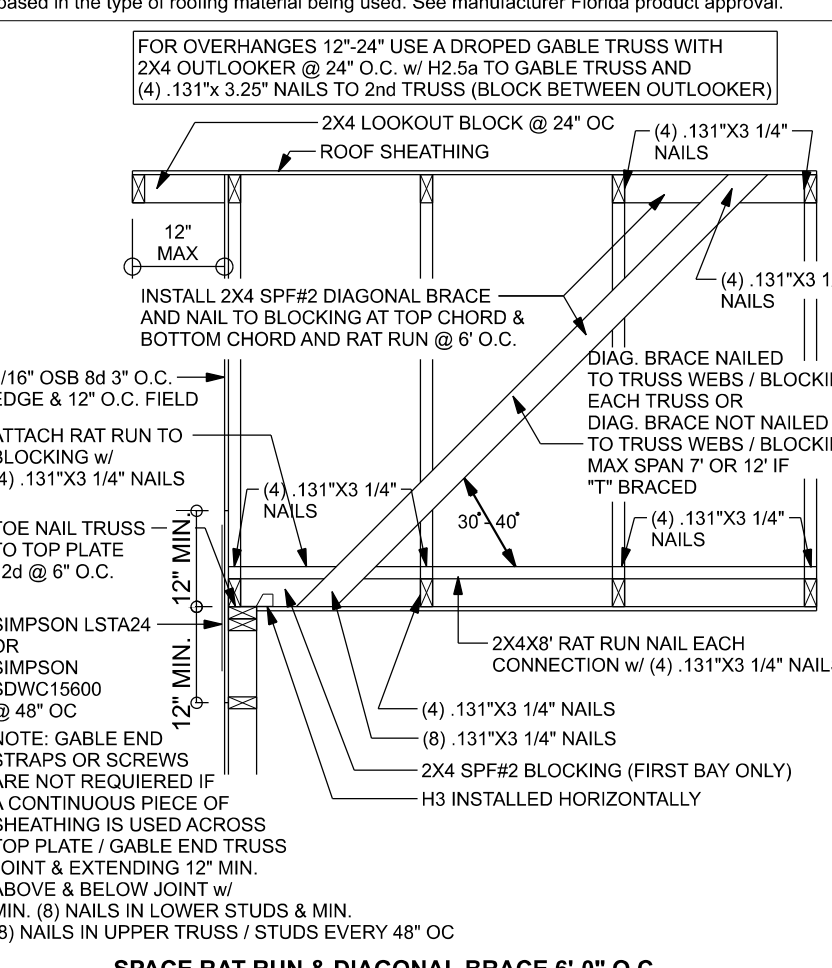
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



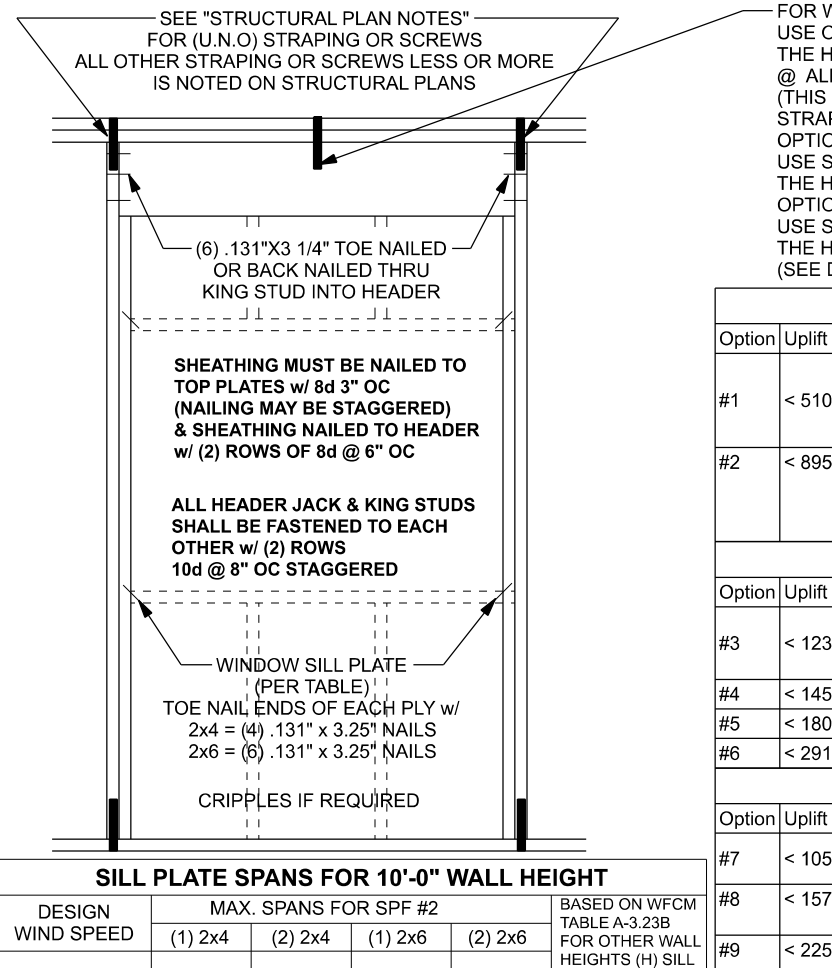
(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	8" oc
120 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	4" oc	4" oc

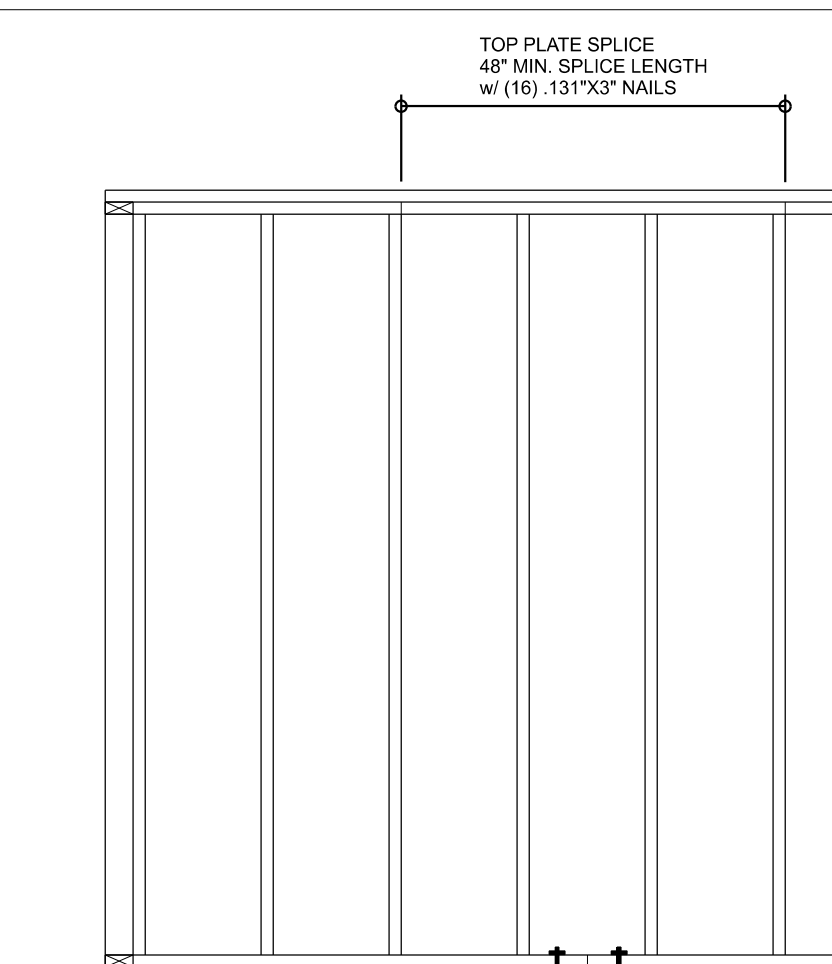
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 8 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



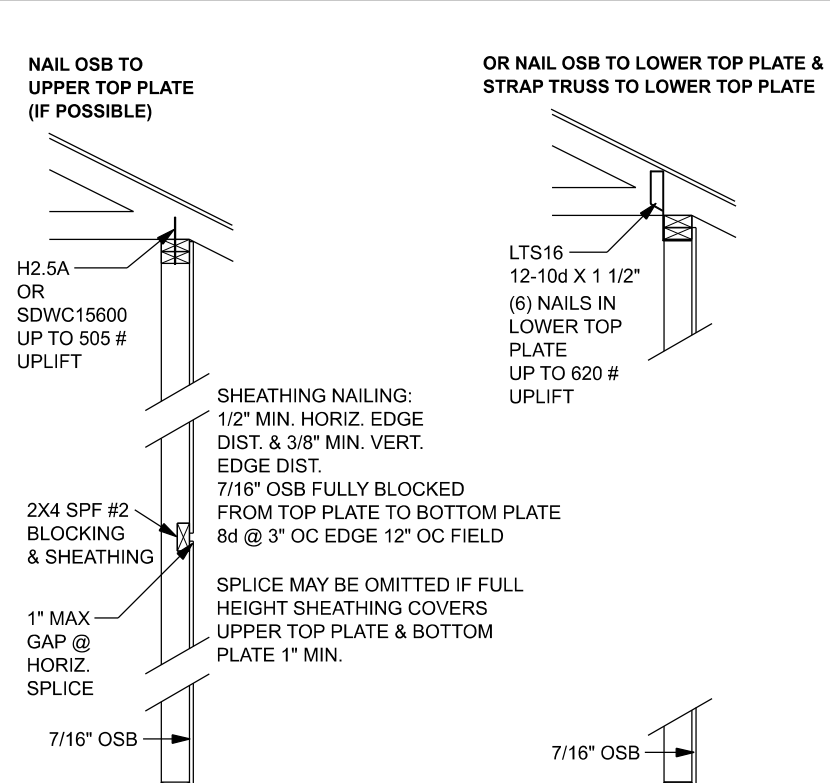
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



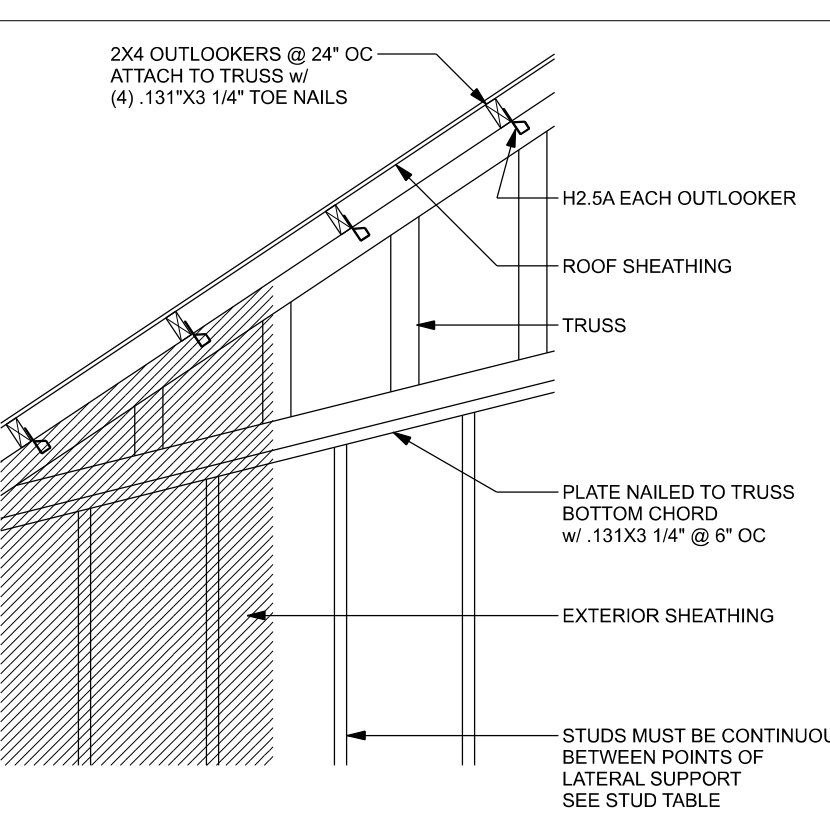
TYPICAL HEADER STRAPPING OR SCREWS DETAIL



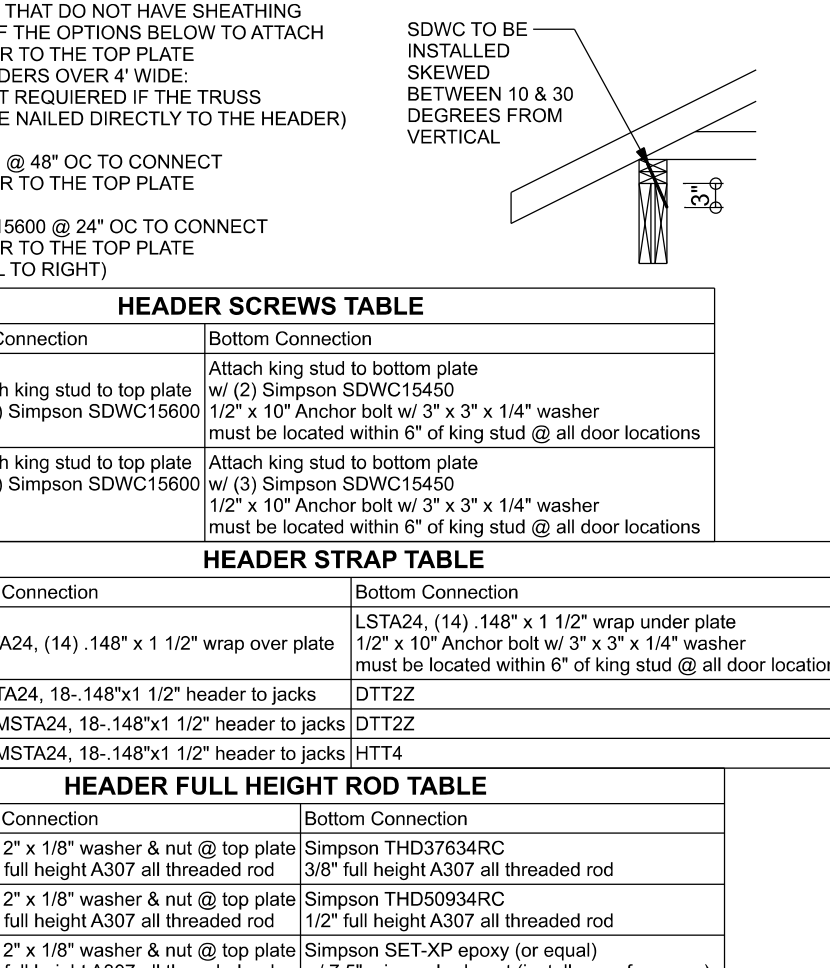
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



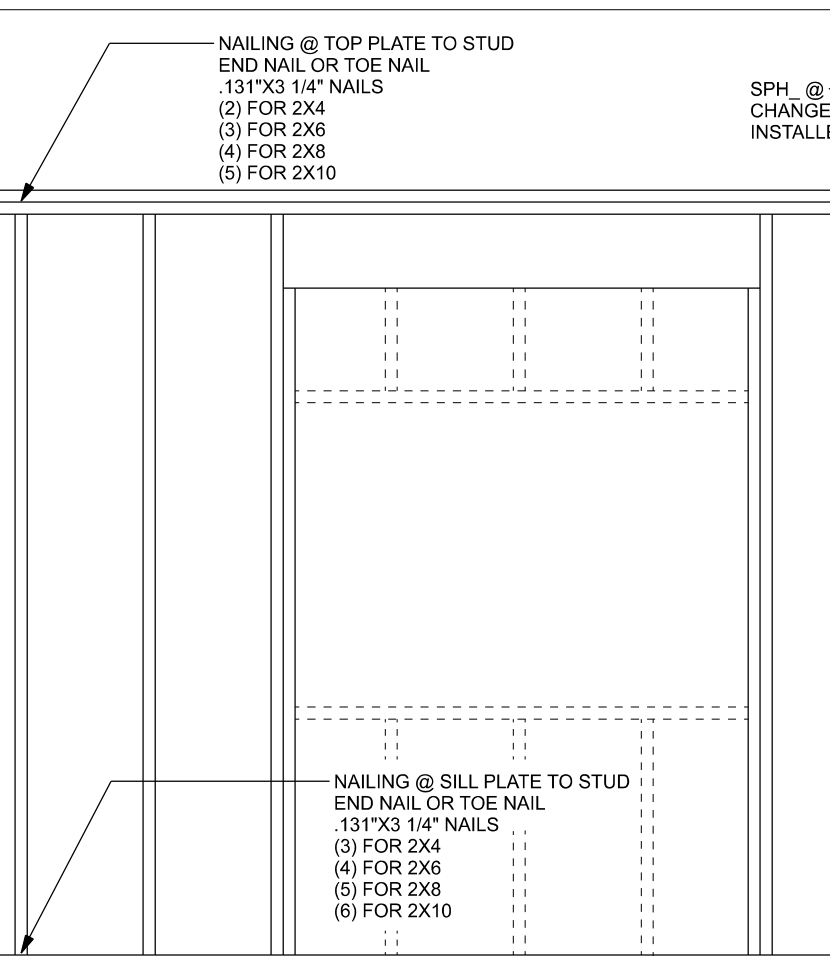
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



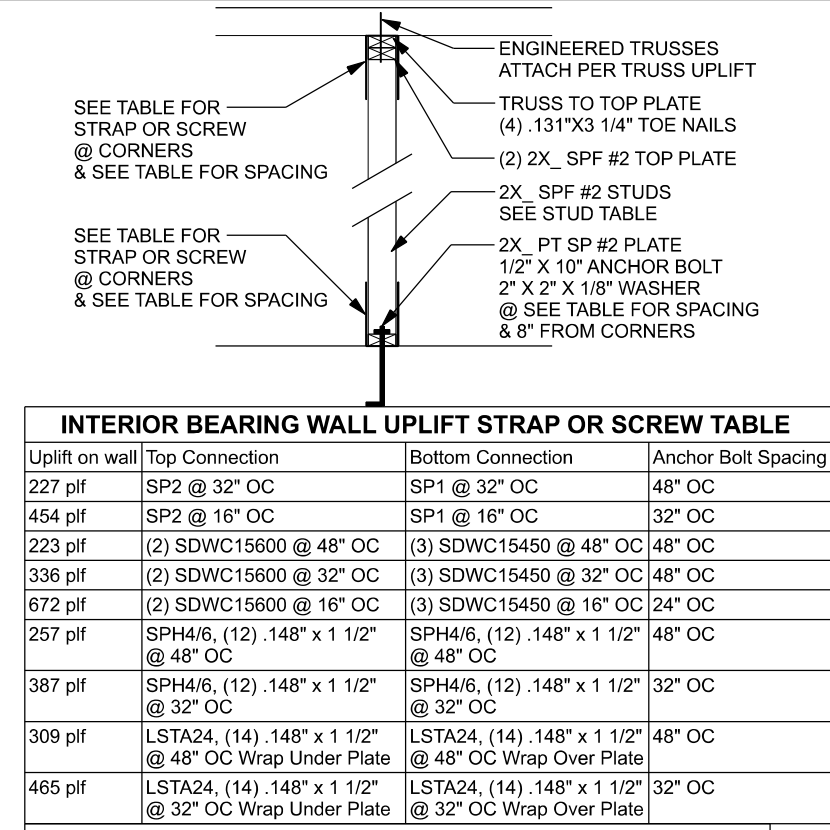
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



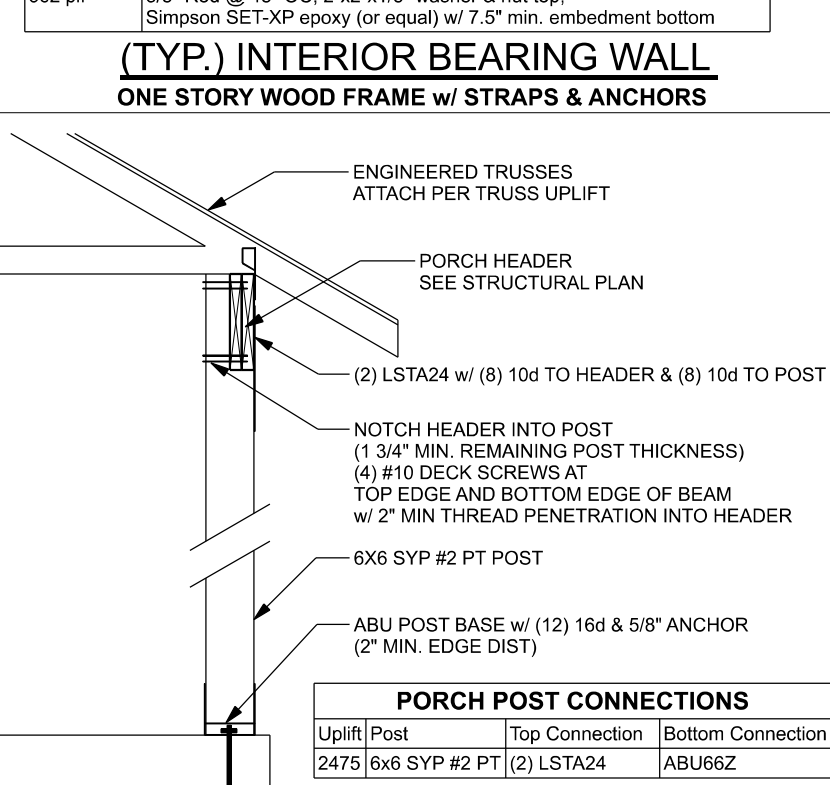
(TYP.) PORCH POST
ONE STORY WOOD



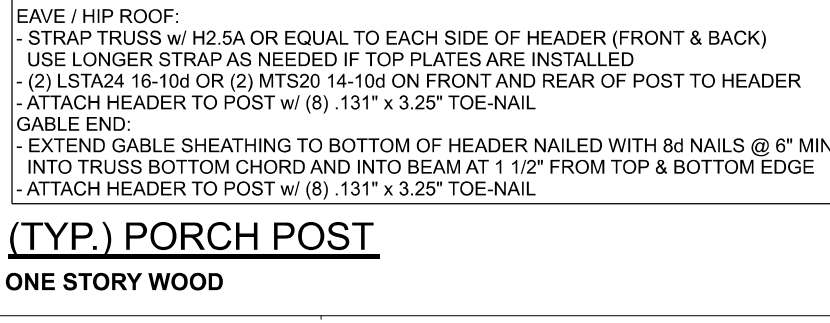
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



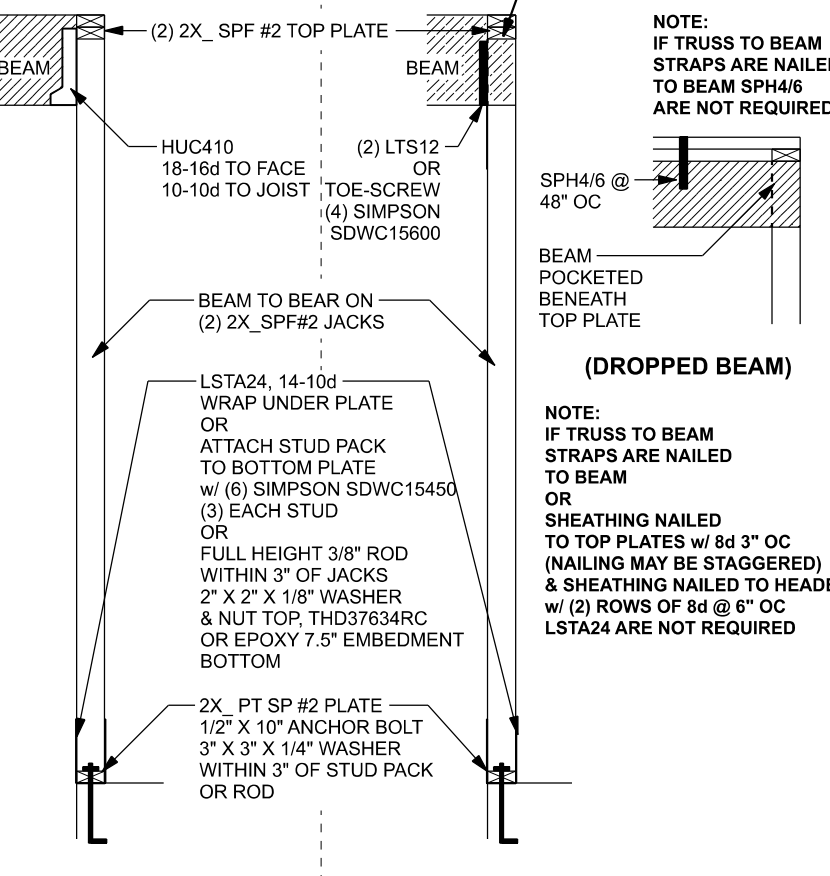
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



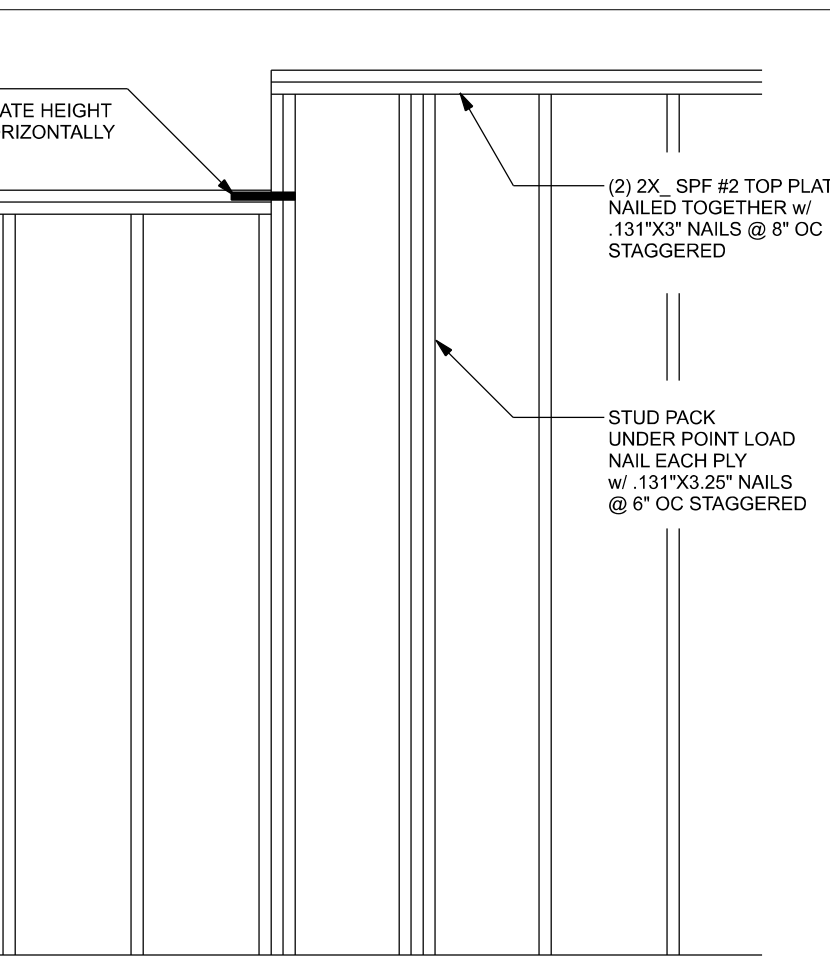
(TYP.) PORCH POST CONNECTIONS



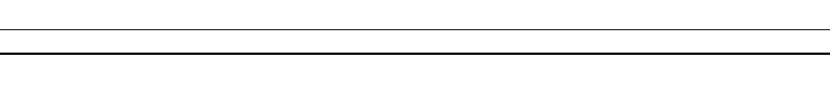
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



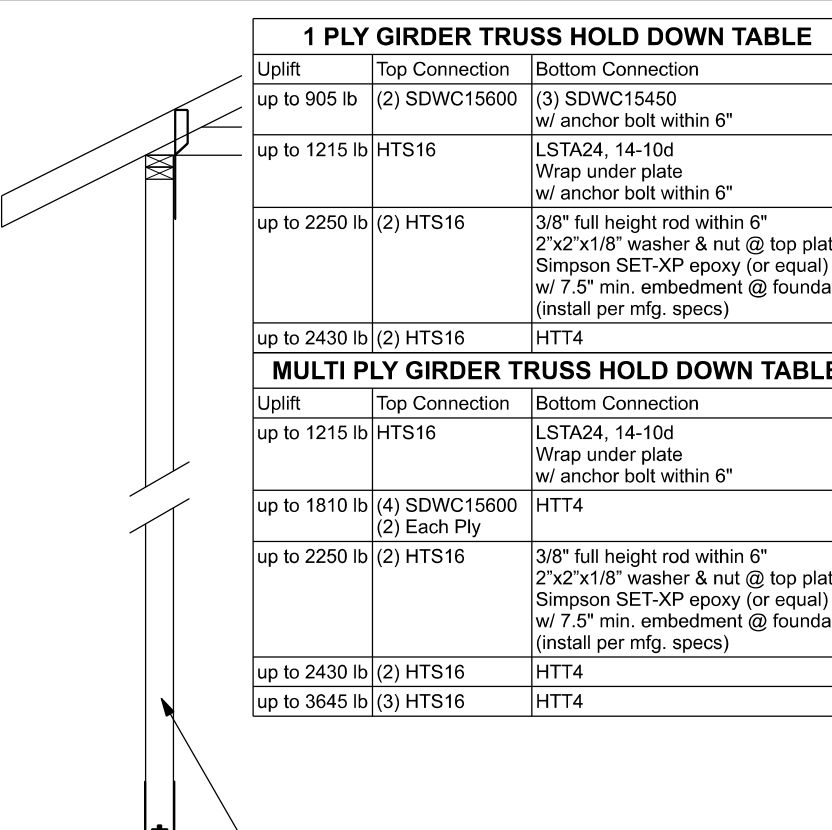
HEADER SCREWS TABLE



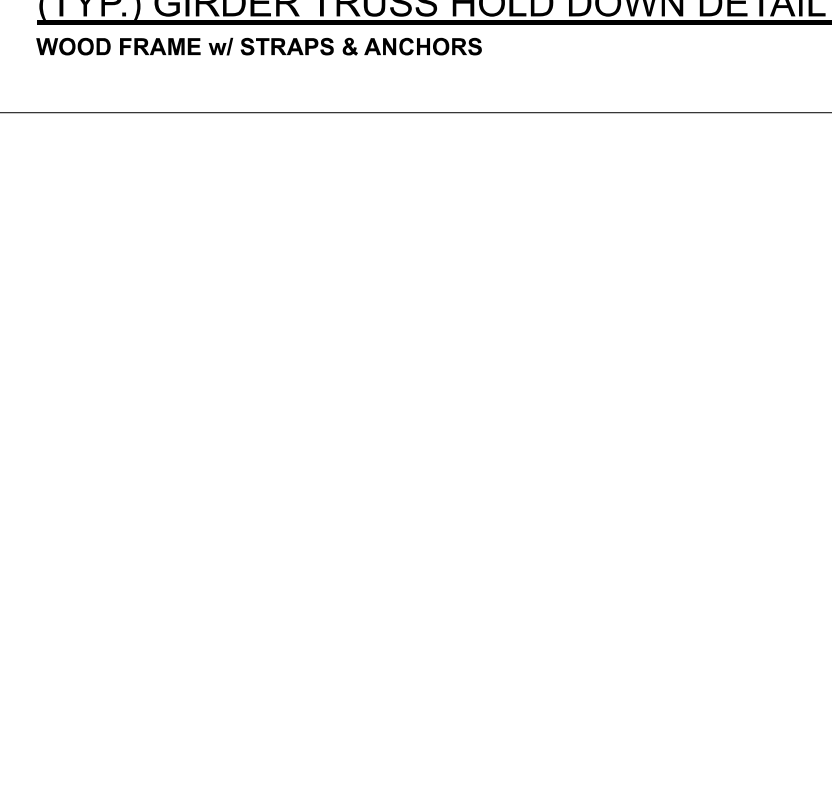
HEADER STRAP TABLE



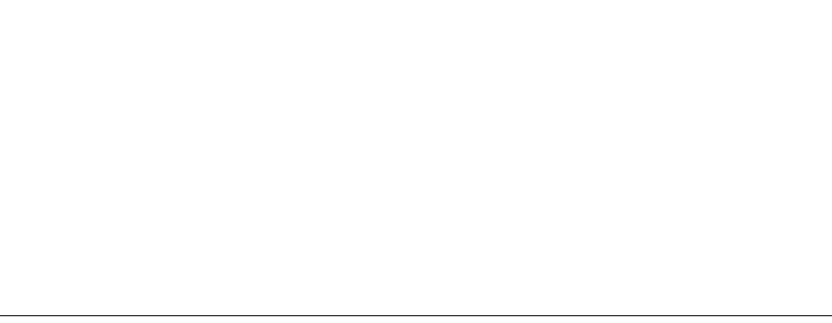
HEADER FULL HEIGHT ROD TABLE



1 PLY GIRDER TRUSS HOLD DOWN TABLE



MULTI PLY GIRDER TRUSS HOLD DOWN TABLE



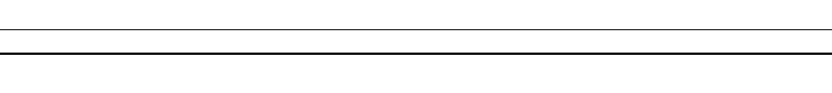
CONNECTOR TABLE



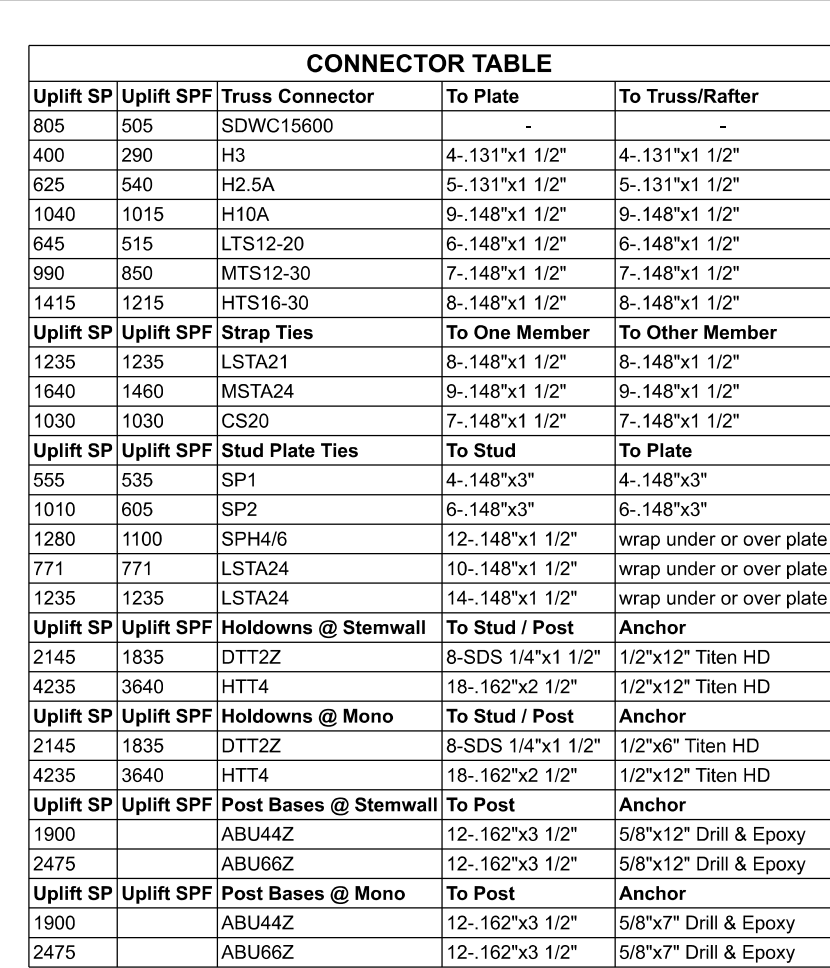
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:



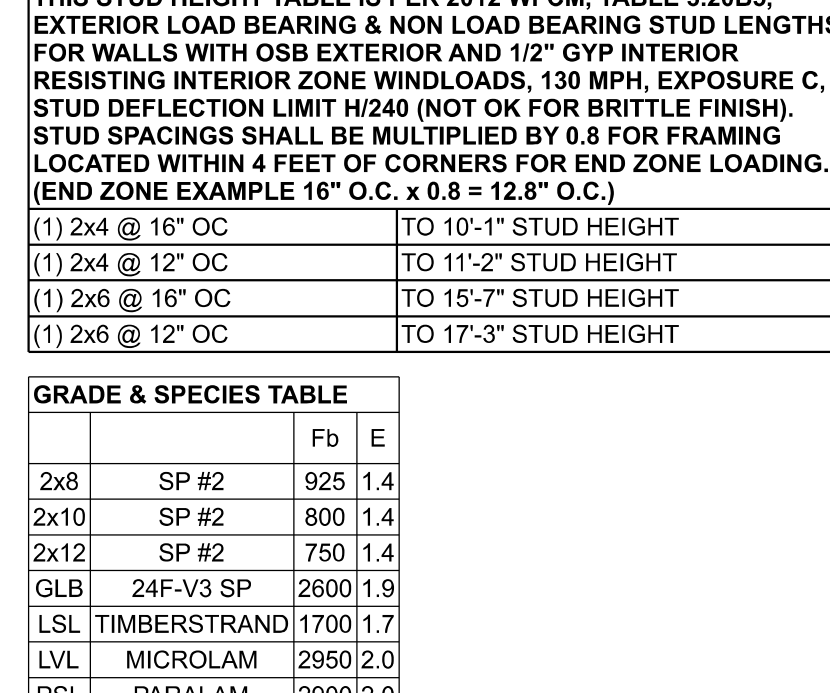
2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT



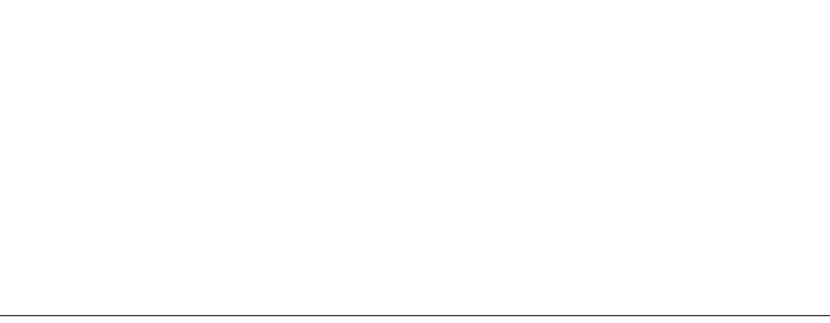
(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME



EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:



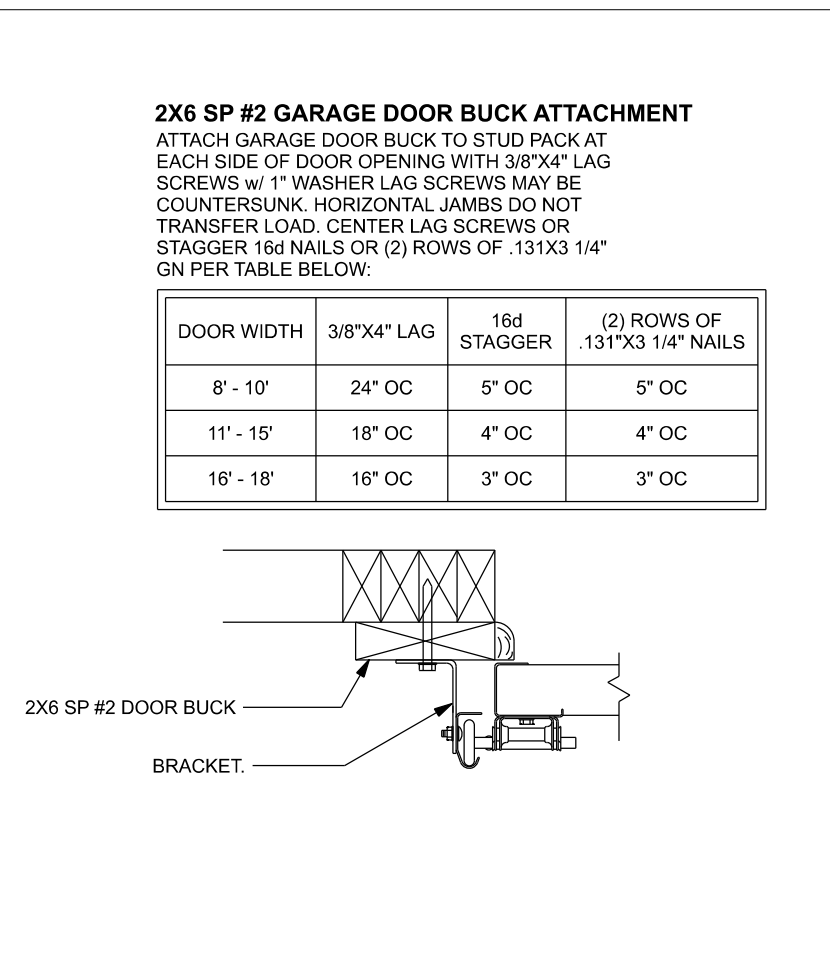
GRADE & SPECIES TABLE



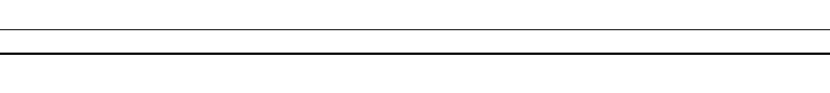
DESIGN CRITERIA & LOADS:



DIMENSIONS:



COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)



JOB NUMBER:
260565
S-1
OF 3 SHEETS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN A SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 8KSI, WELDED WIRE REINFORCEMENT (WWR) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWR OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL). IF CONTROL JOINTS ARE NOT INTERFERED WITH, PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILERED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBER REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OBTAINS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOADS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CARRIED OUT BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR DESIGN SHEETS.



FL PE 53915

This item has been digitally signed and sealed by Mark Discoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DESIGN CRITERIA & LOADS:

BUILDING CODE 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023).

CODE FOR DESIGN LOADS ASCE 7-22.

WINDLOADS

BASIC WIND SPEED (ASCE 7-22, 3S Gust) 130 MPH.

WIND EXPOSURE (BUILDER MUST FIELD VERIFY) C.

TOPOGRAPHIC FACTOR I.

(BUILDER MUST FIELD VERIFY).

RISK CATEGORY II.

ENCLOSURE CLASSIFICATION ENCLOSED.

INTERNAL PRESSURE COEFFICIENT 0.18.

ROOF ANGLE 7-45 DEGREES.

MEAN ROOF HEIGHT 30 FT.

C&C DESIGN PRESSURES SEE TABLE.

FLOOR LOADING

ROOMS OTHER THAN SLEEPING ROOMS 40 PSF LIVE LOAD.

SLEEPING ROOMS 30 PSF LIVE LOAD.

ROOF LOADING

FLAT OR < 4:12 20 PSF LIVE LOAD.

4:12 TO < 12:12 16 PSF LIVE LOAD.

12:12 & GREATER 12 PSF LIVE LOAD.

SOIL BEARING CAPACITY 1500 PSF.

FLOOD ZONE THIS BUILDING IS NOT IN THE FLOOD ZONE.

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (Ft2) ZONE 4 INTERIOR ZONE 5 END 4' FROM ALL OUTSIDE CORNER.

0 - 20 +25.6(Vsdt) -27.8(Vsdt) +25.6(Vsdt) -34.2(Vsdt).

0 - 20 +42.6(Vsdt) -46.2(Vsdt) +42.6(Vsdt) -57(Vsdt).

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

6x7 GARAGE DOOR +22.6(Vsdt) -25.5(Vsdt).

16x7 GARAGE DOOR +21.7(Vsdt) -24.1(Vsdt).

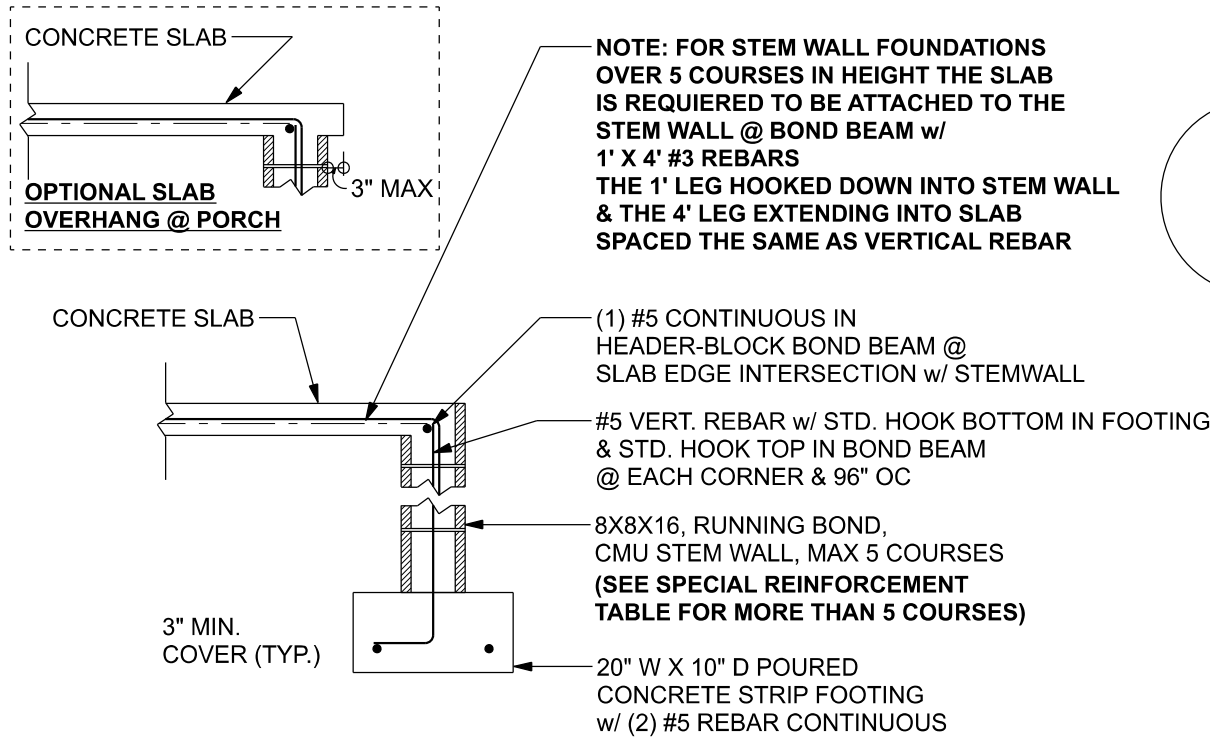
JOB NUMBER:
260565
S-1
OF 3 SHEETS

Mark Discoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

Amira Builders

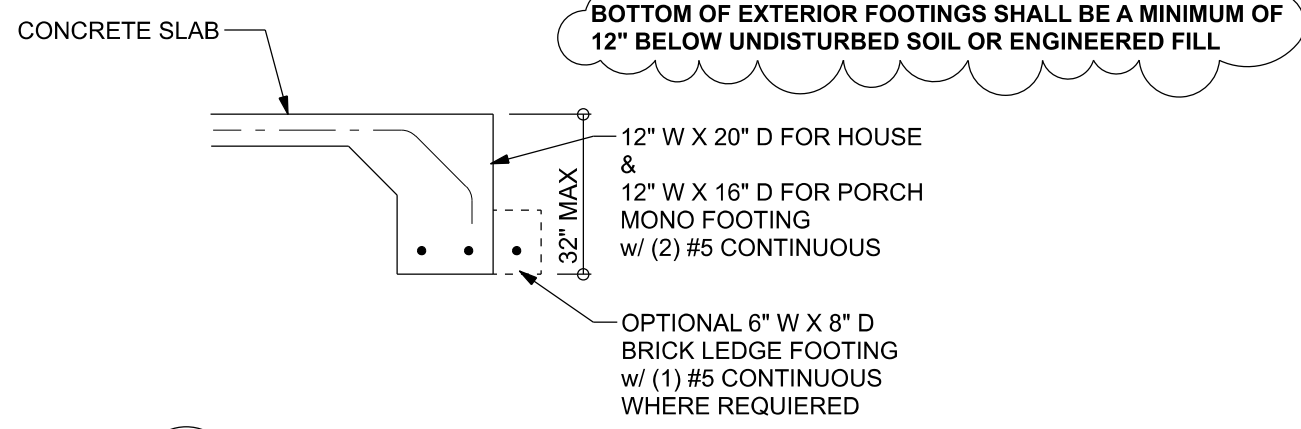
Cristelli Res.

PROJECT ADDRESS:
Parcel: 16-7S-17-1000-106,
Columbia County FL



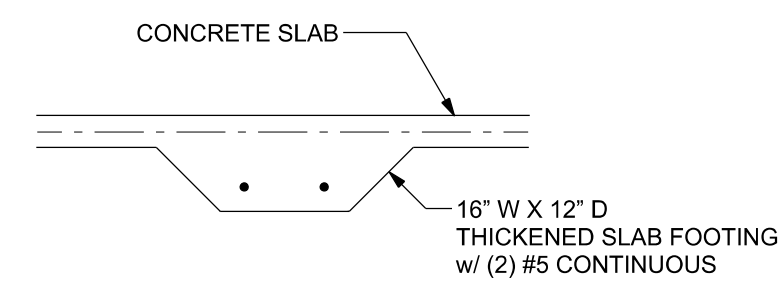
F1 S-2 STEM WALL FOOTING

SCALE: 1/2" = 1'-0"



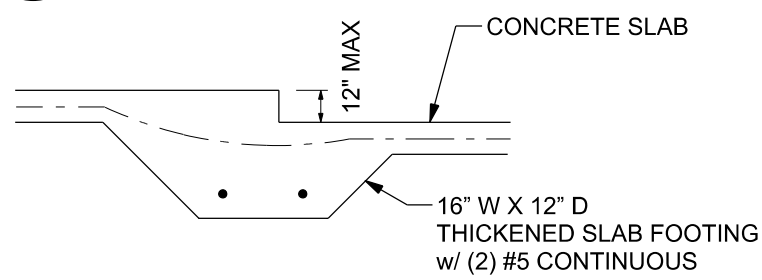
F1 S-2 OPTIONAL MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"



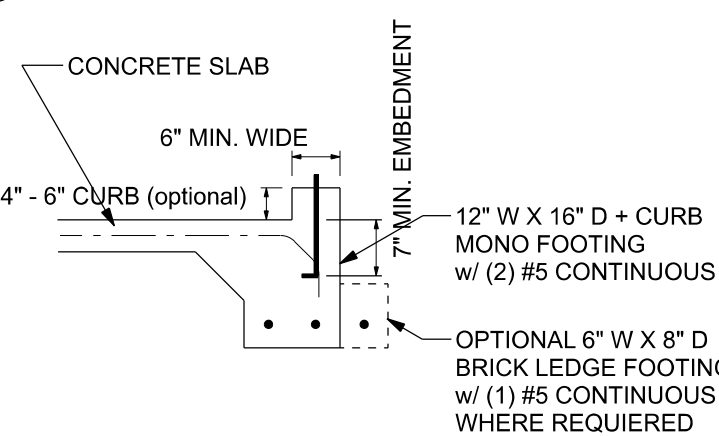
F2 S-2 INTERIOR BEARING FOOTING

SCALE: 1/2" = 1'-0"



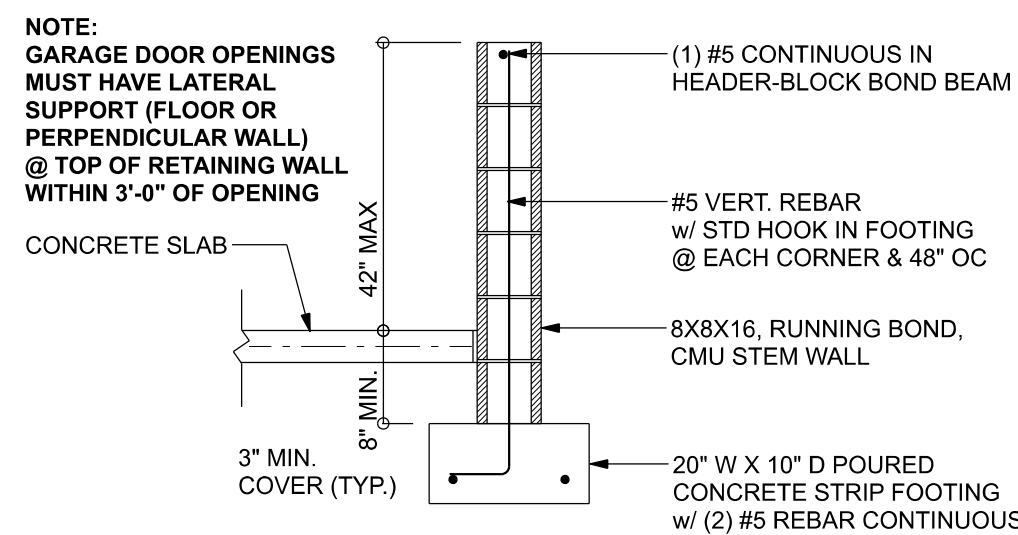
F3 S-2 INTERIOR BEARING STEP FOOTING

SCALE: 1/2" = 1'-0"



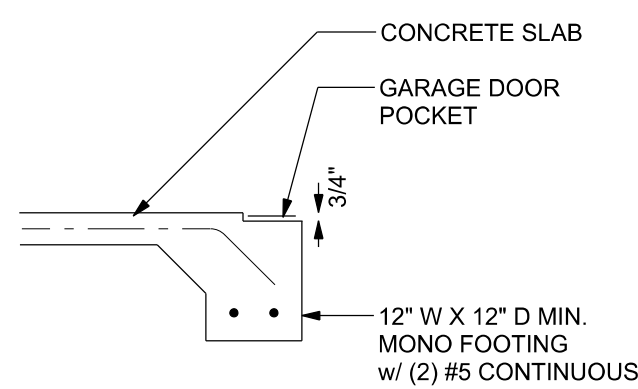
F4 S-2 OPTIONAL MONOLITHIC CURB FOOTING

SCALE: 1/2" = 1'-0"



F4 S-2 STEM WALL CURB FOOTING

SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING

SCALE: 1/2" = 1'-0"

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12'. CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING

TALL STEM WALL TABLE:

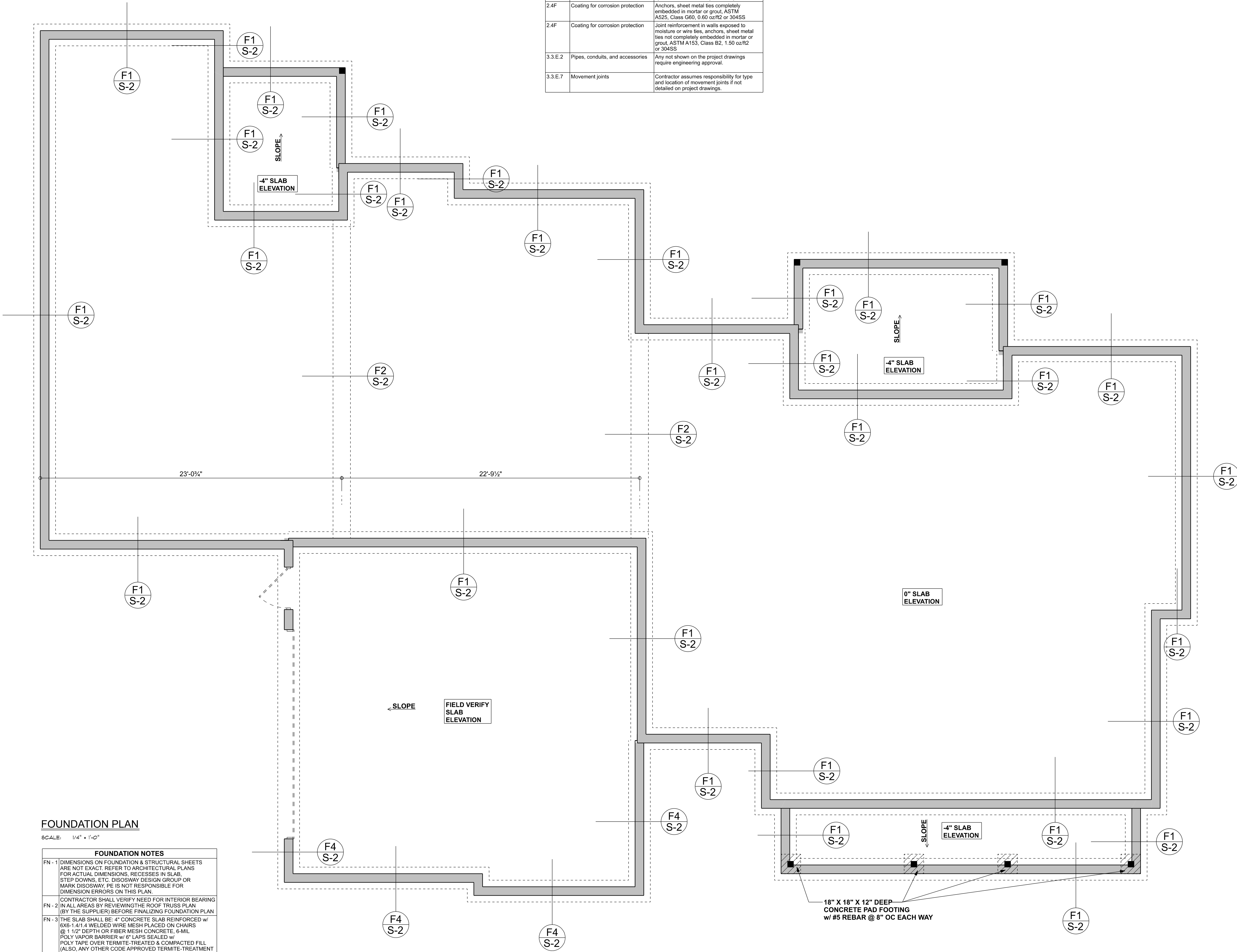
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

MASONRY NOTE:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM A615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb2 or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



Amira Builders

Cristelli Res.

PROJECT ADDRESS:
Parcel: 167S-17-1006-106
Columbia County, FL

FL PE 53915

This item has been digitally signed and sealed by Mark Disoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

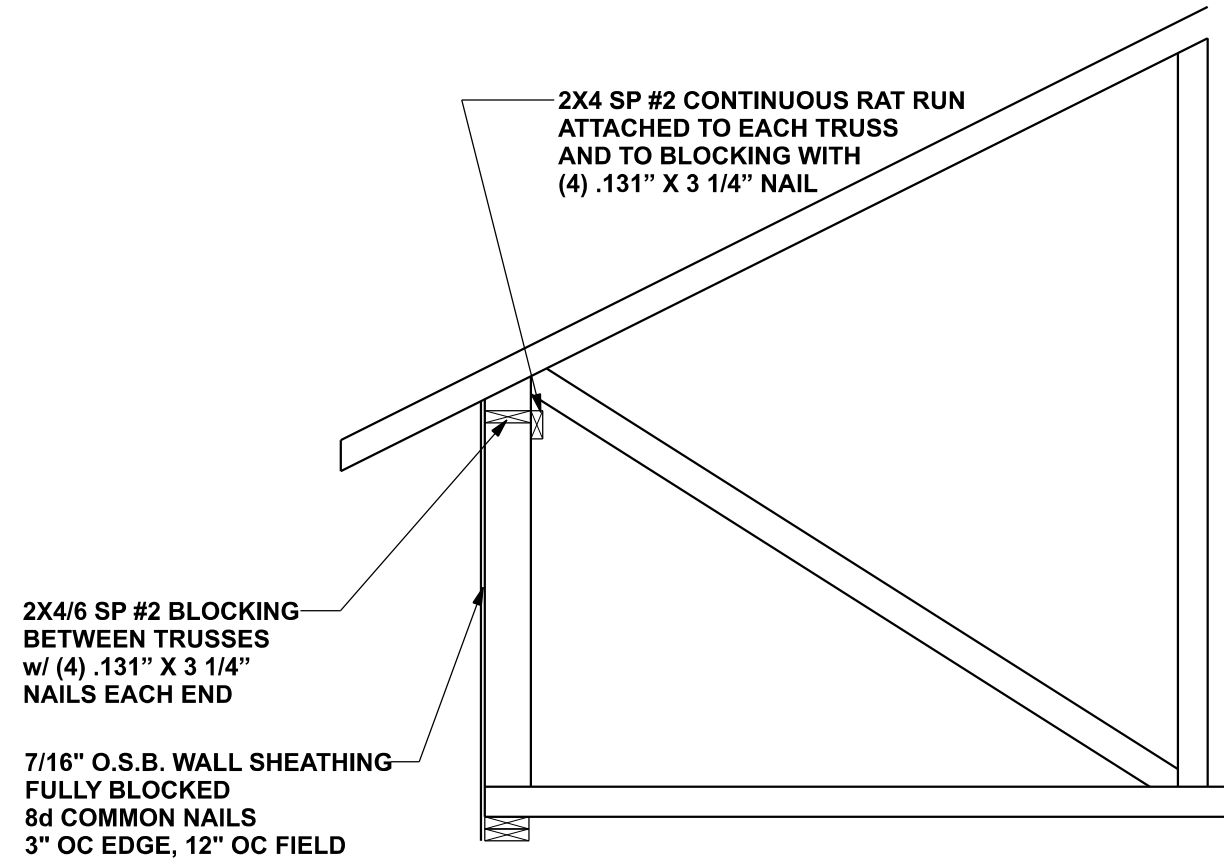
LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

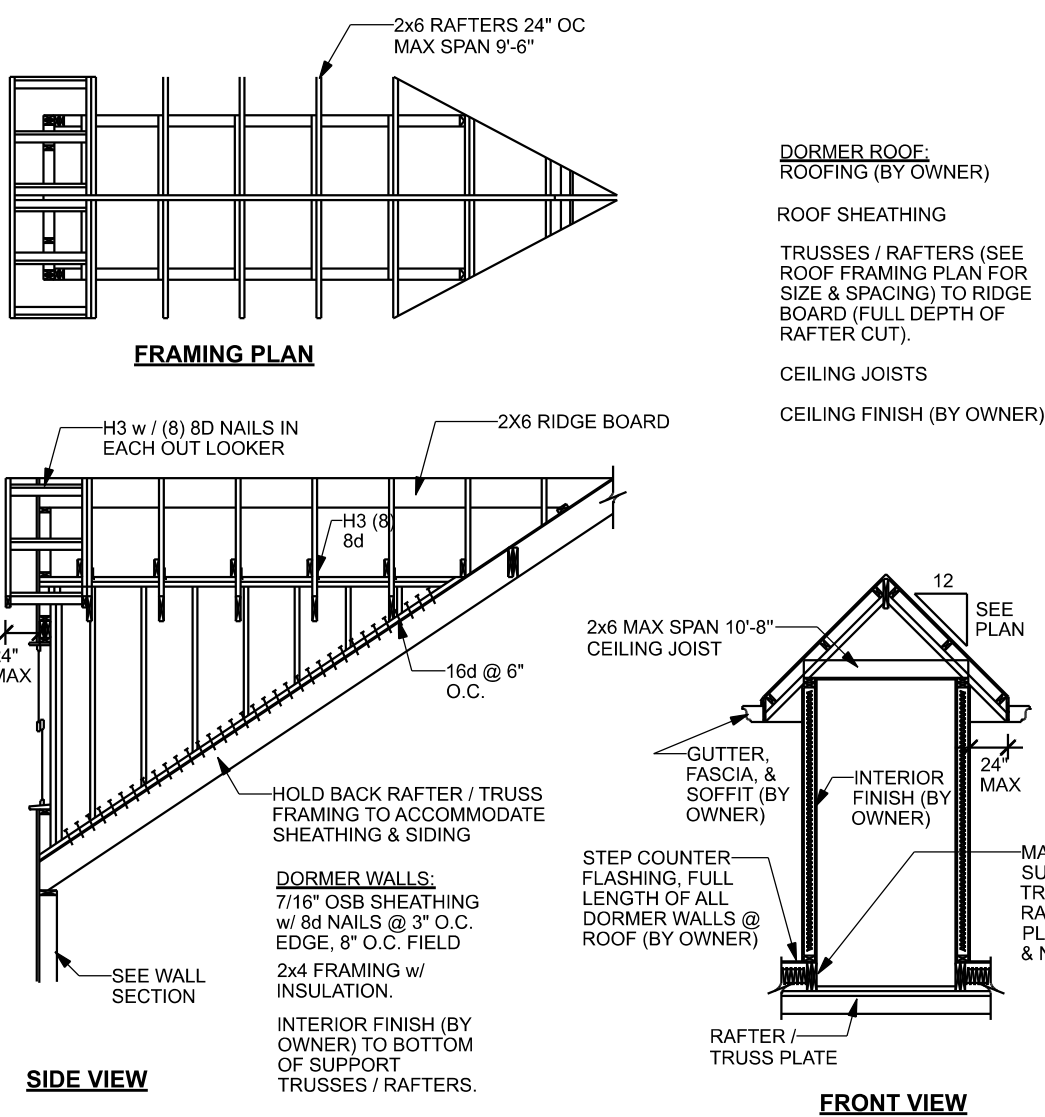
JOB NUMBER:
260565

S-2

OF 3 SHEETS



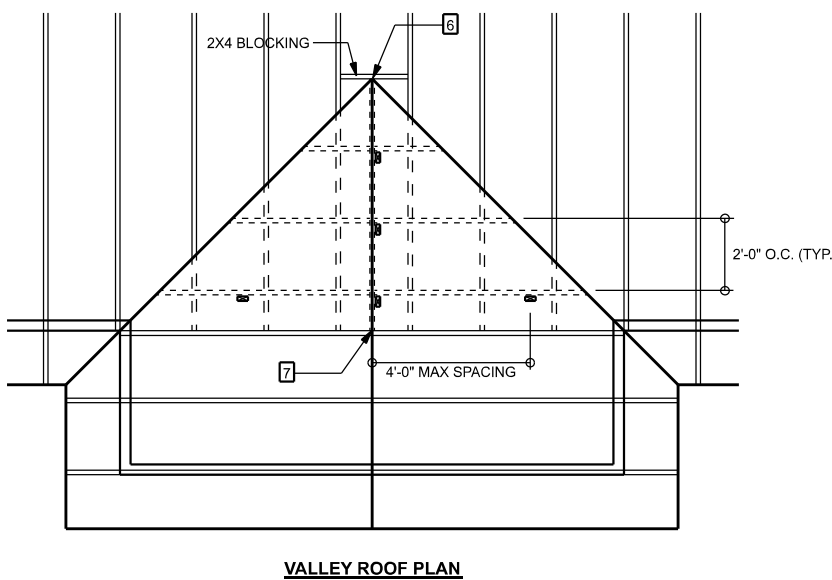
DETAIL @ TRUSSES WITH RAISED HEELS
SCALE: 1/2" = 1'-0"



DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #2 OR 2 PARALLEL 2X4 SPF #2
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN MEMBER LEGEND

—	TRUSS
- - -	TRUSS UNDER VALLEY FRAMING
- . - . -	VALLEY RAFTER OR RIDGE
----	CRIPPLE

CRIPPLES 4\"/>

CONNECTION REQUIREMENT NOTES

1. 2X4 RAFTERS TO RIDGE	4. 131 x 3\"/>
2. CRIPPLE TO RIDGE	4. 131 x 3\"/>
3. CRIPPLE TO RAFTERS	4. 131 x 3\"/>
4. RAFTER TO SLEEPER OR BLOCKING	4. 131 x 3\"/>
5. SLEEPER TO TRUSS	4. 131 x 3\"/>
6. RIDGE BOARD TO ROOF BLOCK	4. 131 x 3\"/>
7. RIDGE BOARD TO TRUSS	4. 131 x 3\"/>
8. PURLIN TO TRUSS (TYP)	4. 131 x 3\"/>
9. PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN	6. 131 x 3\"/>
10. CRIPPLE TO TRUSS	4. 131 x 3\"/>
11. CRIPPLE TO PURLIN	4. 131 x 3\"/>

GENERAL NOTES

MAXIMUM RAFTER SPACING: 6\"/>

CRIPPLE BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 2\"/>

STRUCTURAL PLAN

SCALE: 1/4\"/>

STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03.

ACTUAL vs REQUIRED SHEARWALL

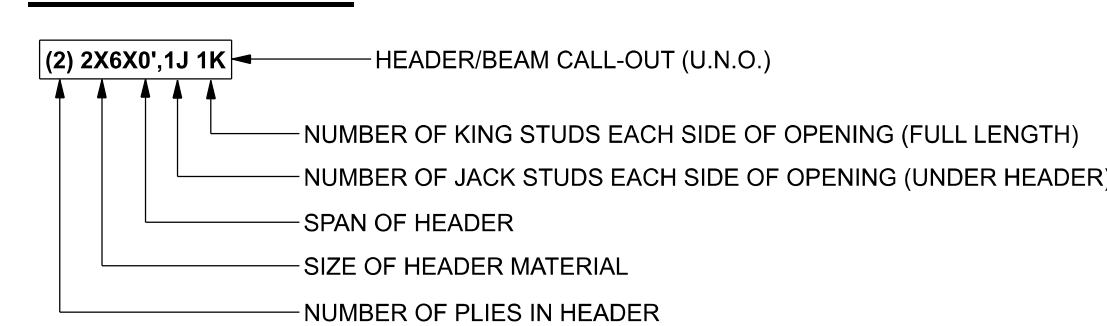
	TRANSVERSE	LONGITUDINAL
ACTUAL	25111 LBF	23556 LBF
REQUIRED	21887 LBF	16989 LBF

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS)

SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2\"/>
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #9481028

Amira Builders

Cristelli Res.

PROJECT ADDRESS:
Parcel 16-75-17-1000-106
Columbia County, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disosway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
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Lake City, Florida 32025
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JOB NUMBER:
260565

S-3
OF 3 SHEETS