

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
 Florida Engineering Certificate of Authorization Number: 0 278
 Florida Certificate of Product Approval # FL1999
 Page 1 of 1 Document ID: ITY88228Z0506163228

Truss Fabricator: Anderson Truss Company
 Job Identification: 10-003--Fill in later STANLEY CRAWFORD -- , **
 Truss Count: 38
 Model Code: Florida Building Code 2007 and 2009 Supplement
 Truss Criteria: FBC2007Res/TPI-2002(STD)
 Engineering Software: Alpine Software, Version 9.02.
 Structural Engineer of Record: The identity of the structural EOR did not exist as of
 Address: the seal date per section 61G15-31.003(5a) of the FAC
 Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
 Floor - N/A
 Wind - 110 MPH ASCE 7-05 -Closed



Seal Date: 01/06/2010

-Truss Design Engineer-
 Doug Fleming
 Florida License Number: 66648
 1950 Marley Drive
 Haines City, FL 33844

Notes:

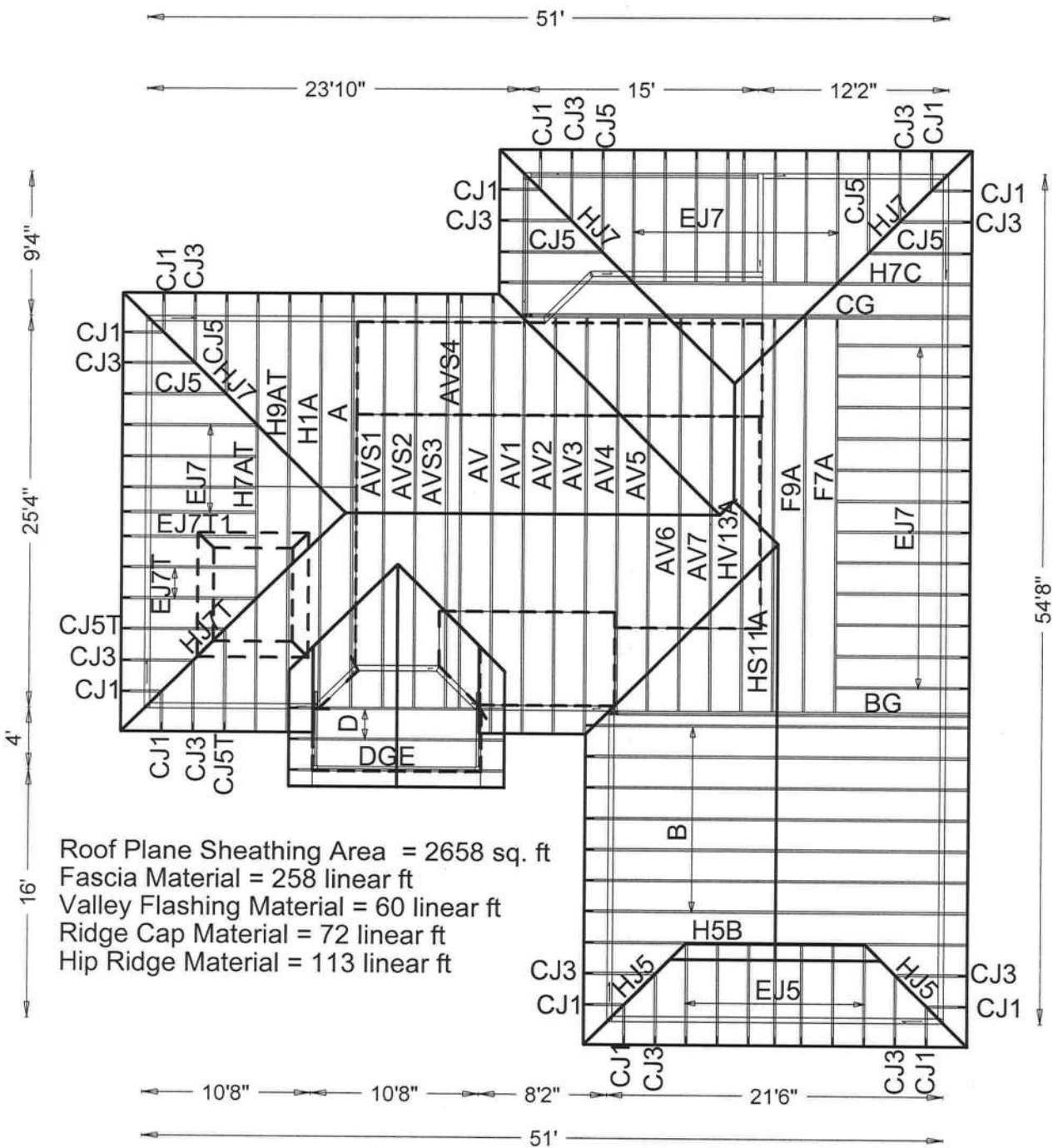
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A1101505-GBLLETIN-

#	Ref	Description	Drawing#	Date
1	95300--	F7A	10006032	01/06/10
2	95301--	F9A	10006011	01/06/10
3	95302--	HS11A	10006012	01/06/10
4	95303--	H1A	10006005	01/06/10
5	95304--	A	10006006	01/06/10
6	95305--	H7AT	10006033	01/06/10
7	95306--	H9AT	10006007	01/06/10
8	95307--	AV	10006013	01/06/10
9	95308--	AV1	10006014	01/06/10
10	95309--	AV2	10006015	01/06/10
11	95310--	AV3	10006016	01/06/10
12	95311--	AV4	10006017	01/06/10
13	95312--	AV5	10006018	01/06/10
14	95313--	AV6	10006019	01/06/10
15	95314--	AV7	10006020	01/06/10
16	95315--	HV13A	10006021	01/06/10
17	95316--	AVS1	10006022	01/06/10
18	95317--	AVS2	10006023	01/06/10
19	95318--	AVS3	10006024	01/06/10
20	95319--	AVS4	10006025	01/06/10
21	95320--	H5B	10006034	01/06/10
22	95321--	B	10006008	01/06/10
23	95322--	BG	10006035	01/06/10
24	95323--	H7C	10006036	01/06/10
25	95324--	CG	10006037	01/06/10
26	95325--	DGE	10006010	01/06/10
27	95326--	D	10006009	01/06/10
28	95327--	EJ7	10006064	01/06/10
29	95328--	EJ5	10006065	01/06/10
30	95329--	CJ5	10006026	01/06/10
31	95330--	HJ7	10006038	01/06/10
32	95331--	CJ3	10006027	01/06/10
33	95332--	HJ7T	10006039	01/06/10
34	95333--	HJ5	10006040	01/06/10
35	95334--	CJ1	10006028	01/06/10
36	95335--	EJ7T	10006029	01/06/10

#	Ref	Description	Drawing#	Date
37	95336--	EJ7T1	10006030	01/06/10
38	95337--	CJ5T	10006031	01/06/10





Roof Plane Sheathing Area = 2658 sq. ft
 Fascia Material = 258 linear ft
 Valley Flashing Material = 60 linear ft
 Ridge Cap Material = 72 linear ft
 Hip Ridge Material = 113 linear ft

STANLEY CRAWFORD/ LOT 26 CANNON CREEK

JOB DESCRIPTION: Fill in later
 /: STANLEY CRAWFORD

JOB NO:
 10-003

PAGE NO:
 1 OF 1

Top chord 2x6 SP #2
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3 :W2, W8 2x4 SP #2 Dense:

End verticals not exposed to wind pressure.

Max JT VERT DEF: LL: 0.14" DL: 0.15" recommended camber 1/4"

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load.

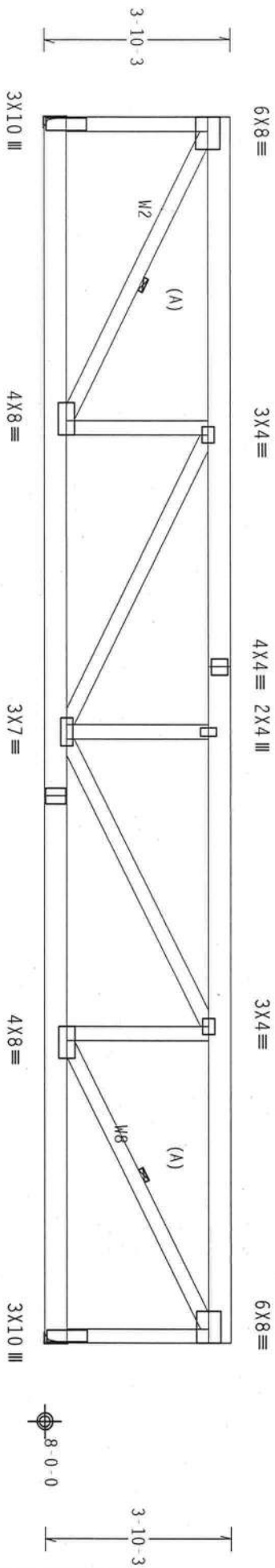
Truss must be installed as shown with top chord up.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 Gcpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Girder supports 7-0-0 span to TC/BC split one face and 2-0-0 span to TC/BC split opposite face.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



R-2370 U-863
 25-4-0 Over 2 Supports
 R-2370 U-869

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY: 1

FL/-/4/-/R/-

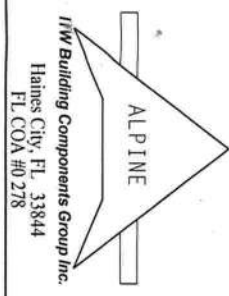
Scale = .3125"/ft.

****WARNING**** TRUSS'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (ERECTING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICK (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSS IN COMPLIANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY AERAA) AND TPI. DESIGN CONTRACTS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AERAA) AND TPI. CONNECTION PLATES ARE MADE OF 2018/1604 (AL/SS/7) ASH ALUMINUM GRADE 40/60 OR 47H/55 GALV. STEEL. APPLY 2.0" MIN. SPACING BETWEEN TRUSS MEMBERS. ALL TRUSS MEMBERS SHALL BE PROTECTED PER DA SEAL ON THIS DRAWING. INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 95300
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006032
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	75403
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TY88228Z05



Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Truss must be installed as shown with top chord up.

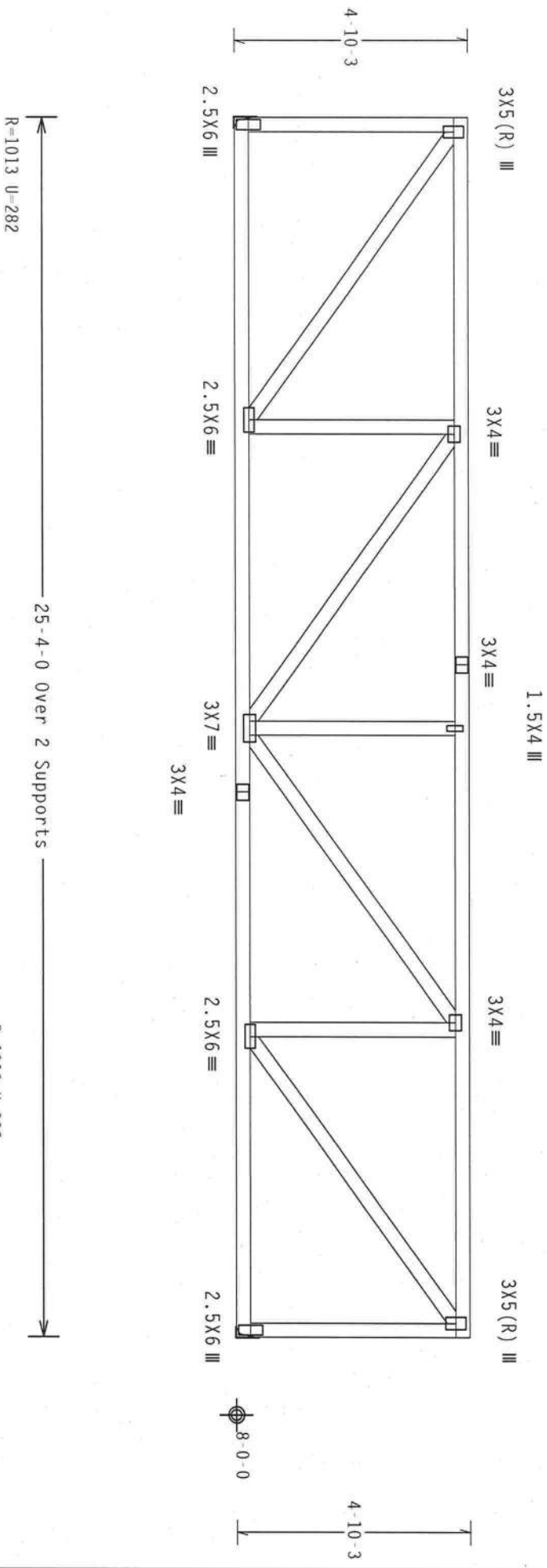
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



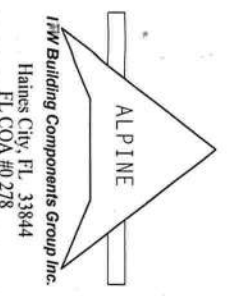
PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
 FT/RT=10%(0%) / 0(0)

9.02.00

QTY: 1 FL/-/4/-/R/-

Scale = .3125" / Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO FOLLOW THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS.



TC LL	20.0 PSF	REF	R8228- 95301
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCSUR8228 100006011
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	75471
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TV882228Z05

Top Chord 2x4 SP #2 Dense
 Bol Chord 2x6 SP #1 Dense :B1, B5 2x6 SP #2:
 Webs 2x4 SP #3 :W7, W13 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

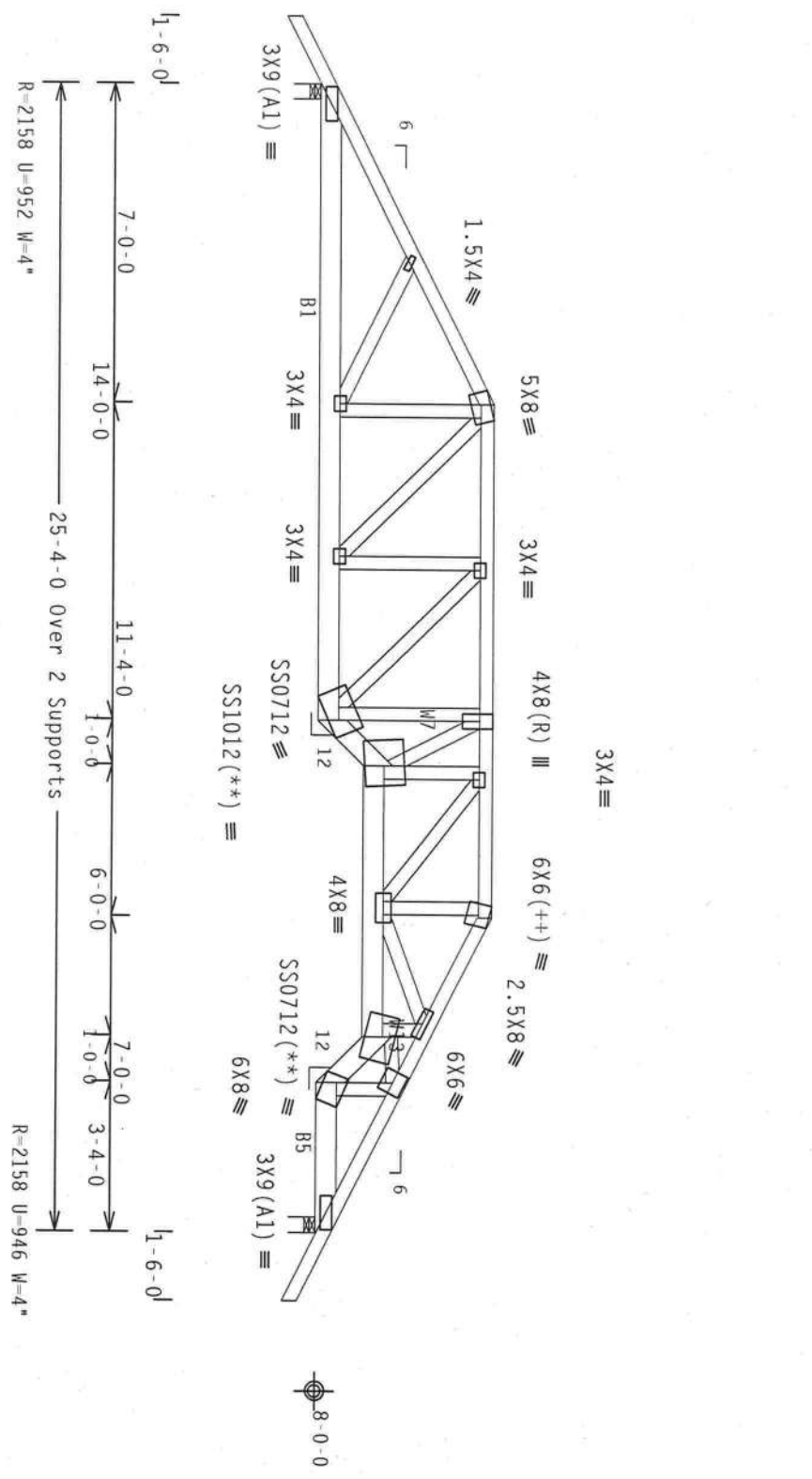
(++) - This plate works for both joints covered.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

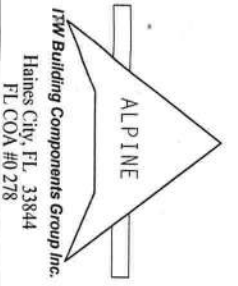


PLT TYP. 18 Gauge HS.Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
 FT/RT=10%(0%)/0(0)

OTY:1 FL/-/4/-/1-/-/1-

Scale = .25" /ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOBILE, AL 36688) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITR REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT WITH APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC. (AS APPL) AND TPI. THE REG. CONNECTOR PLATES ARE MADE OF 20/18/16GA. (6.4/5.5/4.3) ASH 4653 GRADE 40/60 (H, K/II/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER 43 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMST/TP1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 95305
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006033
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	75522
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TY88228Z05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

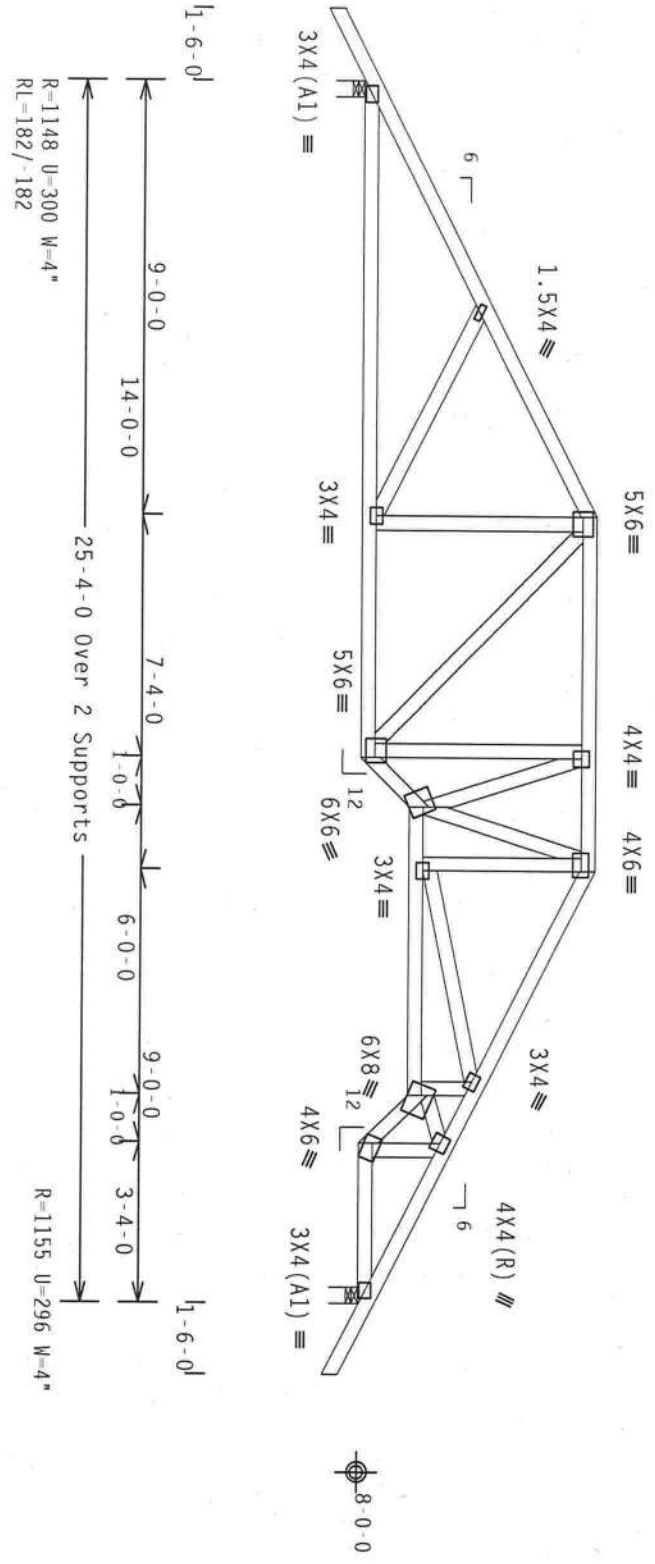
MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

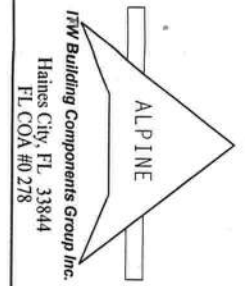


PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
 FT/RT=10%(0%)/0(0)

9.02.00

Scale = .25" / Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6800 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DESIGN CONDITIONS WITH APPLICATION, SHIPPING, INSTALLING & BRACING OF TRUSSES.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DESIGN CONDITIONS WITH APPLICATION, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/16GA OR 4/5/3/4" GALVALUM. ALL STEEL SHALL BE A36. ALL BOLTS SHALL BE A325. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANSI A3 OR TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 95306
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006007
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	75503
DUR.FAC.	1.25	JREF	1TY88228205
SPACING	24.0"		

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.29" due to live load and 0.30" due to dead load.

Deflection meets L/240 live and L/180 total load.

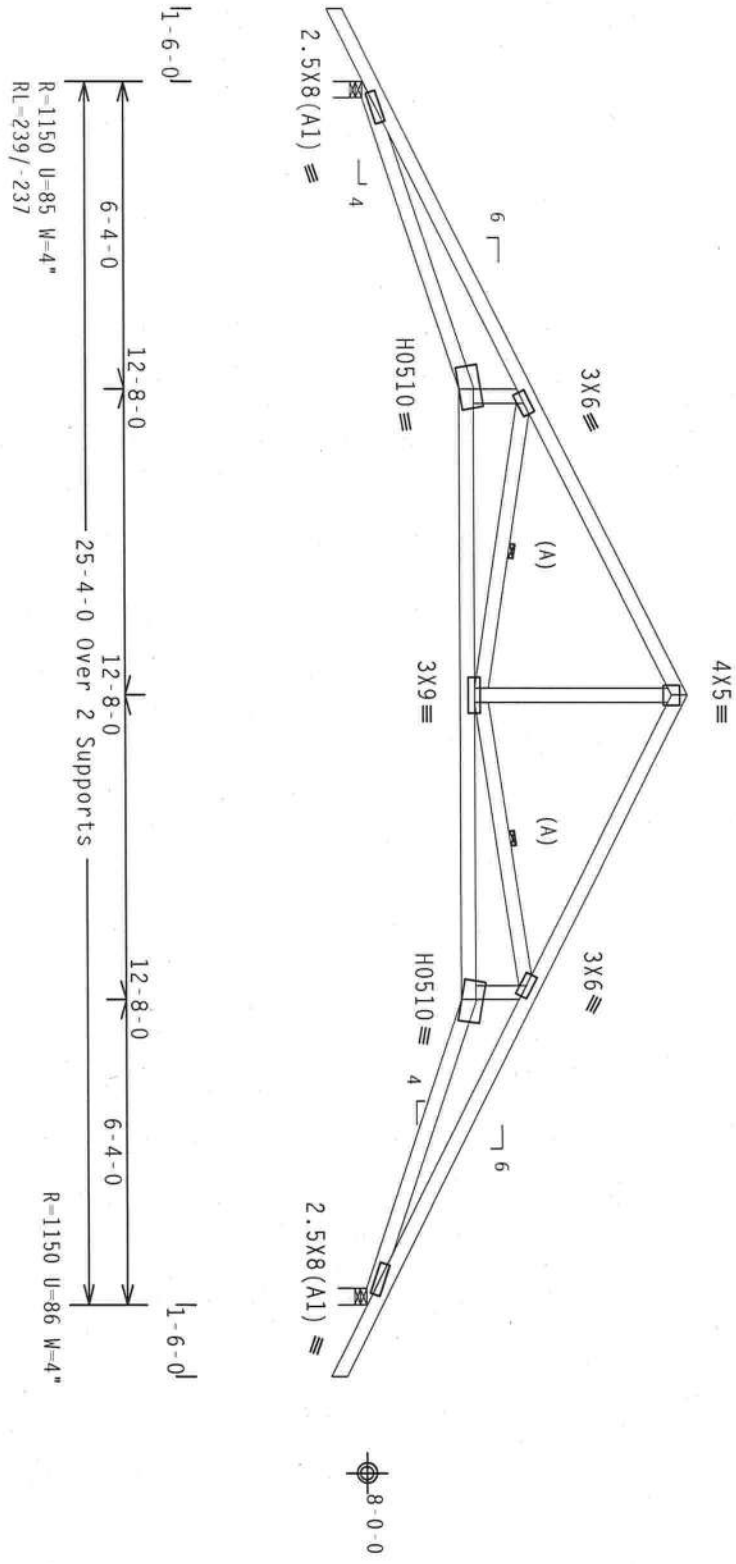
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. 20 Gauge HS Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY: 1

Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPC, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. TPI, OR FABRICATOR, SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN. DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF THE IBC OR IRC SHALL APPLY. THE CONTRACTOR SHALL APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 95307
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCSR8228 10006013
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	75847
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TY88228205

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
Fl C/OA #0 978

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.29" due to live load and 0.31" due to dead load.

Deflection meets L/240 live and L/180 total load.

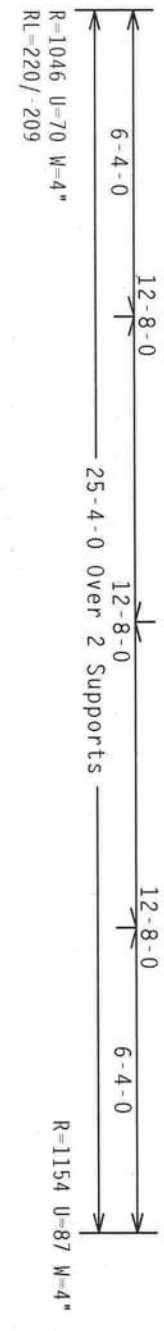
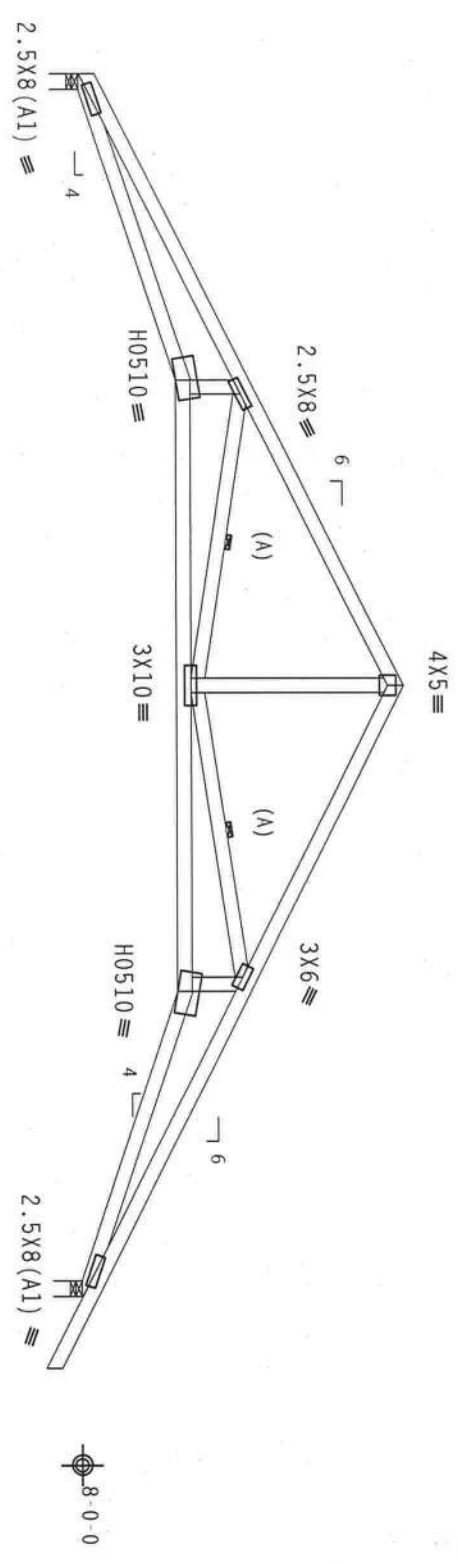
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 GCpf(+/-)=0.18

Wind reactions based on MMFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

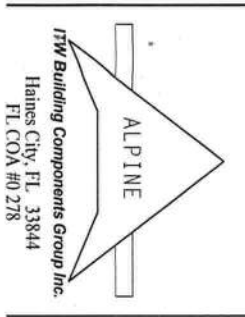
Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. 20 Gauge HS, Wave
 Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00 QTY: 1 FL/-/4/-/10/10 Scale = .25"/ft.



RTW Building Components Group Inc.
 Gaines City, FL 33844
 FL COA #0 278



TC LL	20.0 PSF	REF	R8228- 95308
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006014
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	75542
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TY88228205

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3 : W2 2x4 SP #2 Dense:

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Max JT VERT DEFL: LL: 0.34" DL: 0.37" recommended camber 5/8"

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

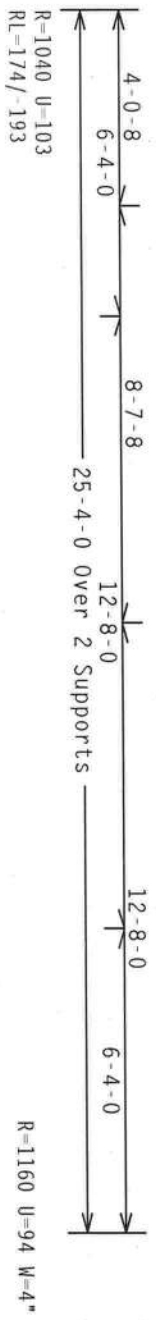
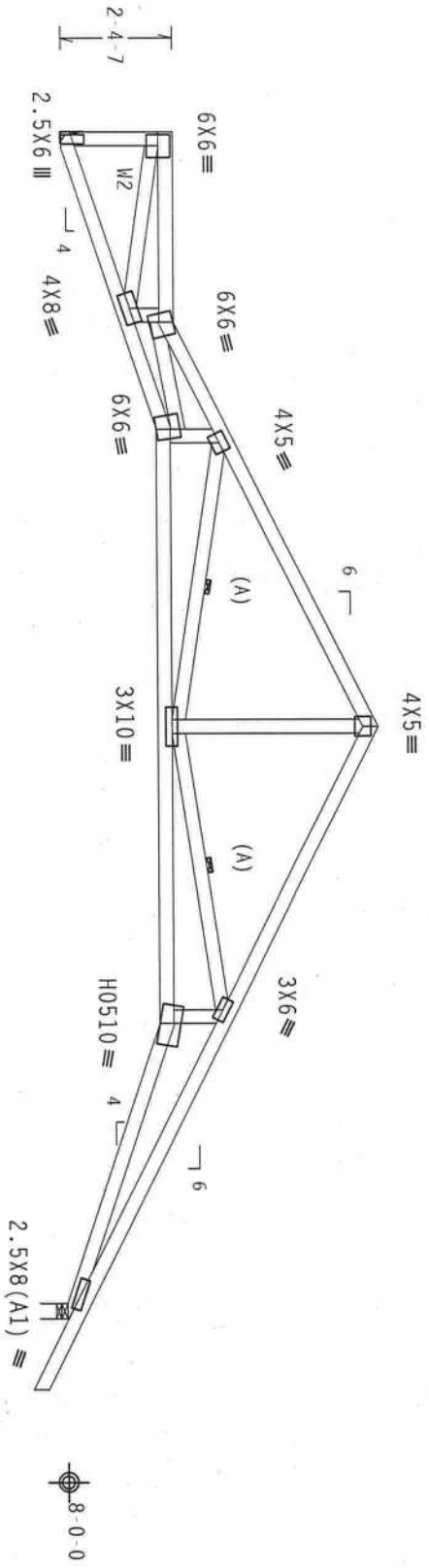
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.27" due to live load and 0.29" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



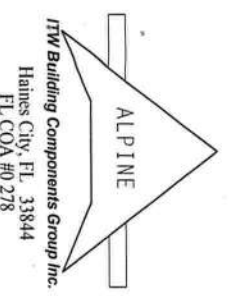
PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

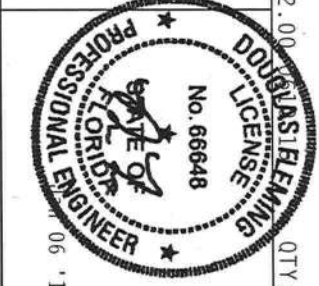
9.02.00

QTY: 1

Scale = .25"/ft.



ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278



TC LL	20.0 PSF	REF	R8228- 95310
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006016
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEON-	75574
DUR. FAC.	1.25		
SPACING	24.0"	QREF-	1TY88228Z05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense :83 2x6 SP SS:
 Webs 2x4 SP #3 :W10, W11 2x4 SP #2 Dense:

Left end vertical not exposed to wind pressure.
 Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

(J) hanger connection not found in inventory file for this condition. Provide connection.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

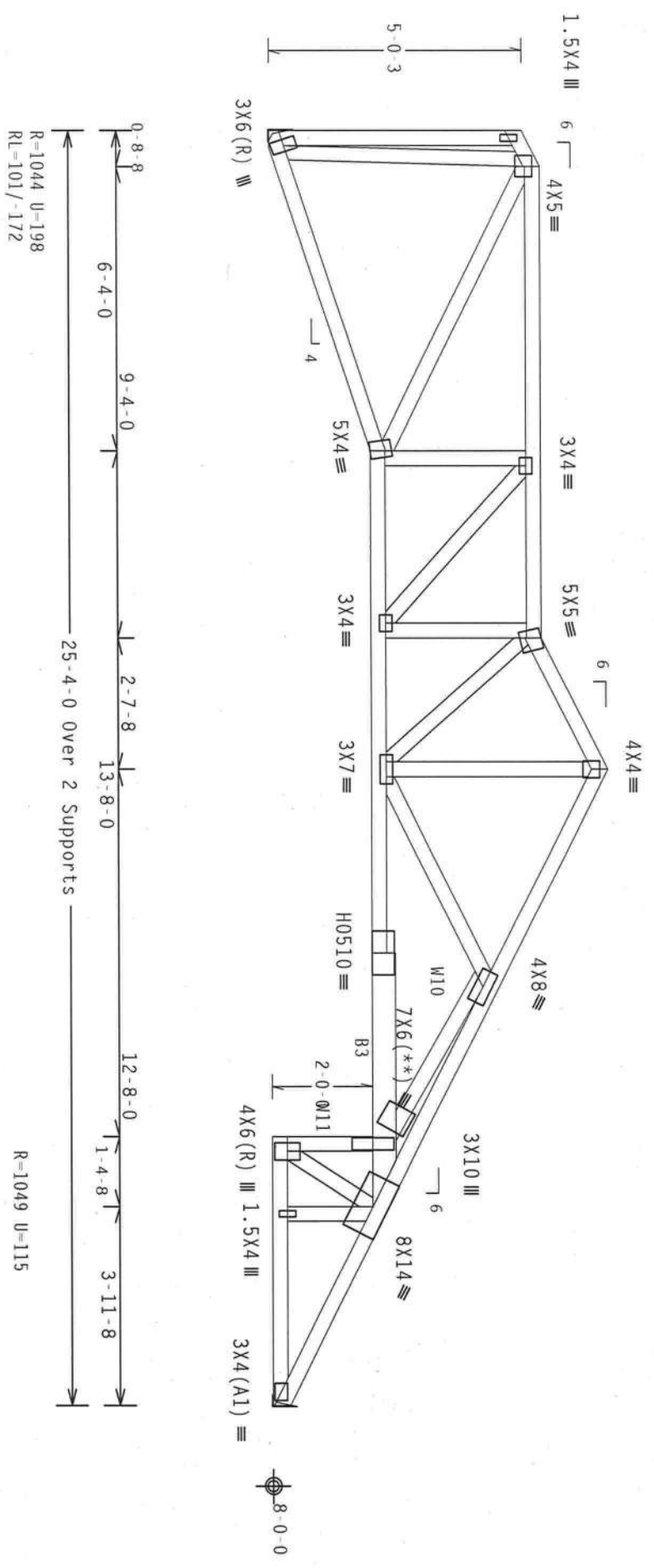
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf(+/-)=0.18

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



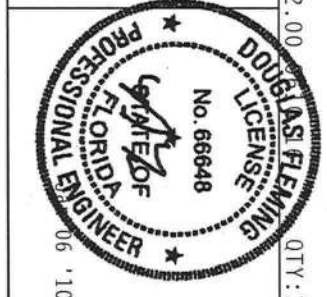
PLT TYP. 20 Gauge HS, Wave
 Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00 QTY: 1 FL/-/4/-/1-/R/- Scale = .3125"/ft.

ALPINE
 ITW Building Components Group Inc.
 Gaines City, FL 33844
 FL COA #0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). MANUFACTURED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WCA GOOD TRUSS COMPANY OF AMERICA, 4030 ENTERPRISE LANE, HANSON, MI 49429 FOR SAFETY PRACTICES PRIOR TO PREPARING THESE TRUSSES. UNLESS OTHERWISE INDICATED, CONNECTIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PLATES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ALDID CELLING.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. ENG. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 95313
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006019
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SECON-	75/08
DUR. FAC.	1.25	JREF-	1TY88228Z05
SPACING	24.0"		

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3: W10, W12 2x4 SP #2 Dense;

End verticals not exposed to wind pressure.
 Calculated horizontal deflection is 0.14" due to live load and 0.15" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

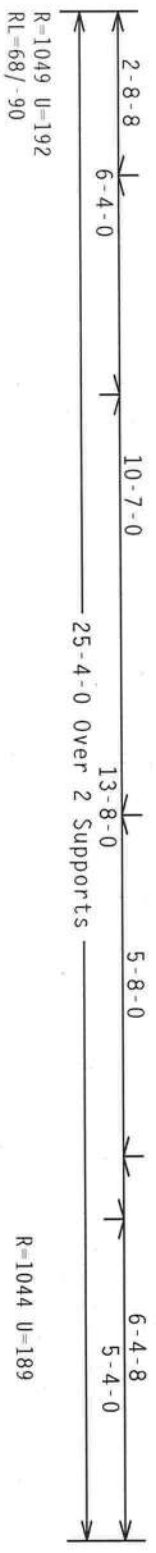
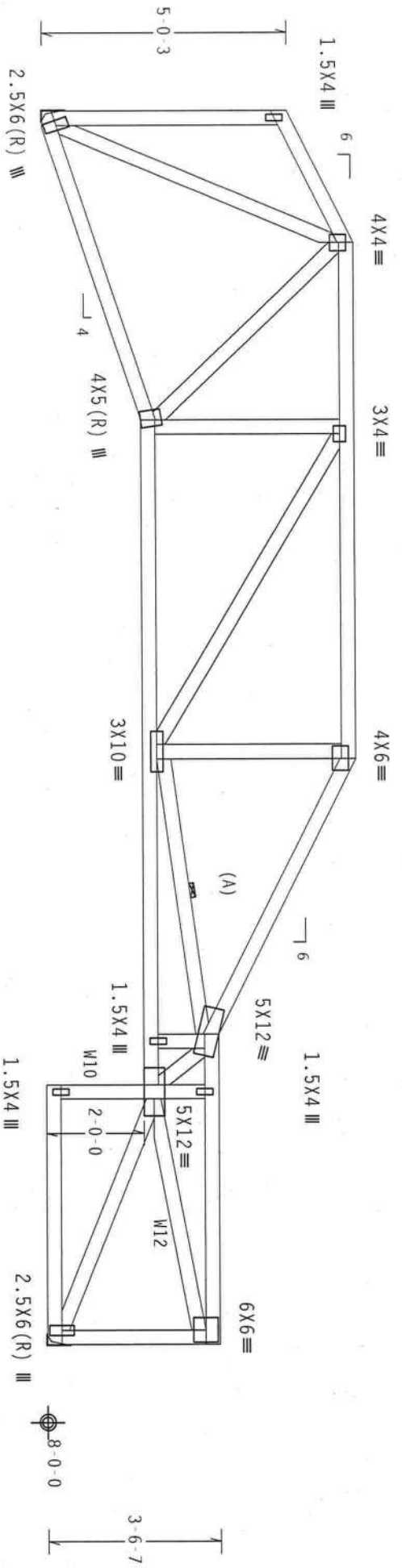
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP1(+/-)=0.18$

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

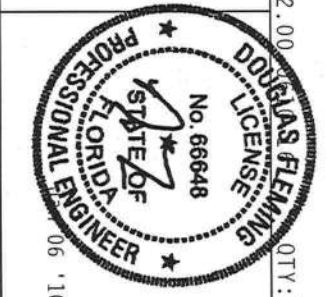
9.02.00

QTY: 1 FL/-/4/-/1/-/

Scale = .3125"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD BRASS CONDUCT OF AMERICA, 10500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW REG. INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR THE FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE MANUFACTURING, HANDLING, SHIPPING, INSTALLING & BRACING DESIGN SPECIFICATIONS OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF AIA (ARTICLE 16.1) AND AIA (ARTICLE 16.2) SHALL APPLY TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER PART/PI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228-95314
TC DL	10.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCURS8228 10006020
BC LL	0.0 PSF	HC-ENG JB/DF
TOT. LD.	40.0 PSF	SEQN- 75730
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1TY88228Z05

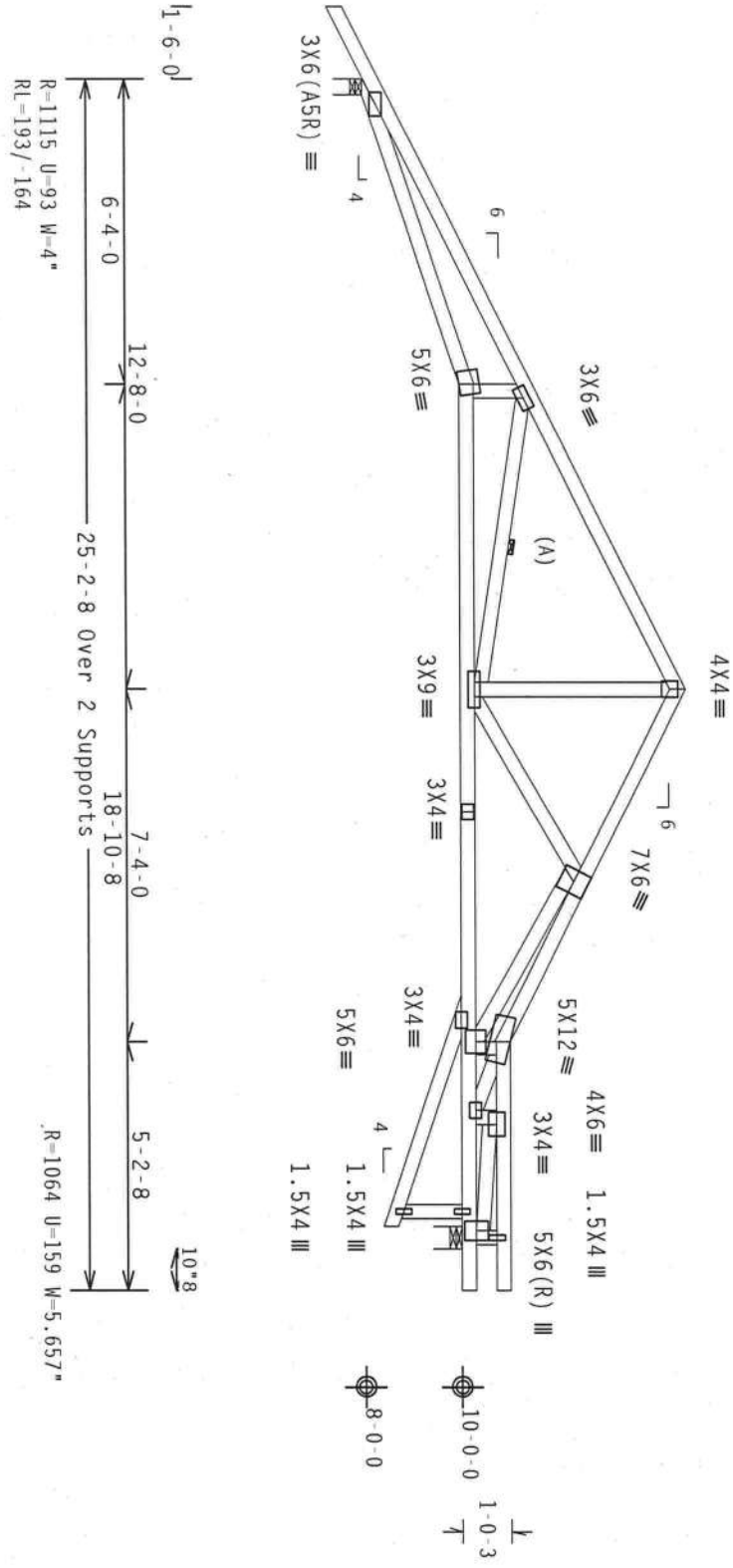
ITW Building Components Group Inc.
 Gaines City, FL 33844
 FL COA #0278

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.
 Calculated horizontal deflection is 0.23" due to live load and 0.24" due to dead load.
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Deflection meets L/240 live and L/180 total load.

Laterally brace chord above/below filler @ 24" O.C. (or as designed) including a brace on chord directly above/below both ends of filler (if no rigid diaphragm exists at that point)

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 Gcpl(+/-)=0.18
 Wind reactions based on MWFRS pressures.
 (A) Continuous lateral bracing equally spaced on member.
 In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
 MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



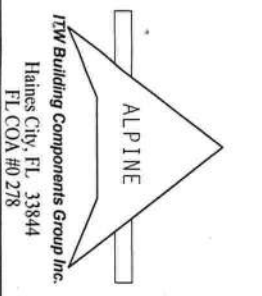
PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY: 1

Scale = .25"/ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESIGN (BUILDING COMPONENTS) SHEET FOR BRACING AND CONNECTIONS. ALL TRUSSES MUST BE INSTALLED TO THE DESIGNER'S SPECIFICATIONS. THE DESIGNER IS RESPONSIBLE FOR THE DESIGN AND THE CONTRACTOR IS RESPONSIBLE FOR THE CONSTRUCTION. THE CONTRACTOR SHALL VERIFY THE TRUSS DESIGNER'S IDENTIFICATION AND THE TRUSS DESIGNER'S IDENTIFICATION SHALL BE PROPERLY ATTACHED TO THE TRUSS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE DESIGNER IS RESPONSIBLE FOR THE DESIGN AND THE CONTRACTOR IS RESPONSIBLE FOR THE CONSTRUCTION. THE CONTRACTOR SHALL VERIFY THE TRUSS DESIGNER'S IDENTIFICATION AND THE TRUSS DESIGNER'S IDENTIFICATION SHALL BE PROPERLY ATTACHED TO THE TRUSS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



FL	/	4	/	/	/	RT
TC	LL	20.0	PSF	REF	R8228	95319
TC	DL	10.0	PSF	DATE	01/06/10	
BC	DL	10.0	PSF	DRW	HCUSR828	10006025
BC	LL	0.0	PSF	HC-ENG	JB/DF	
TOT.	LD.	40.0	PSF	SEQN-	75841	
DUR.	FAC.	1.25		JREF	1TY88228205	
SPACING		24.0"				

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

See DWGS A11015050109 & GBLLETTINO109 for more requirements.

+ MEMBER TO BE Laterally Braced For Out Of Plane Wind Loads.
+ BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

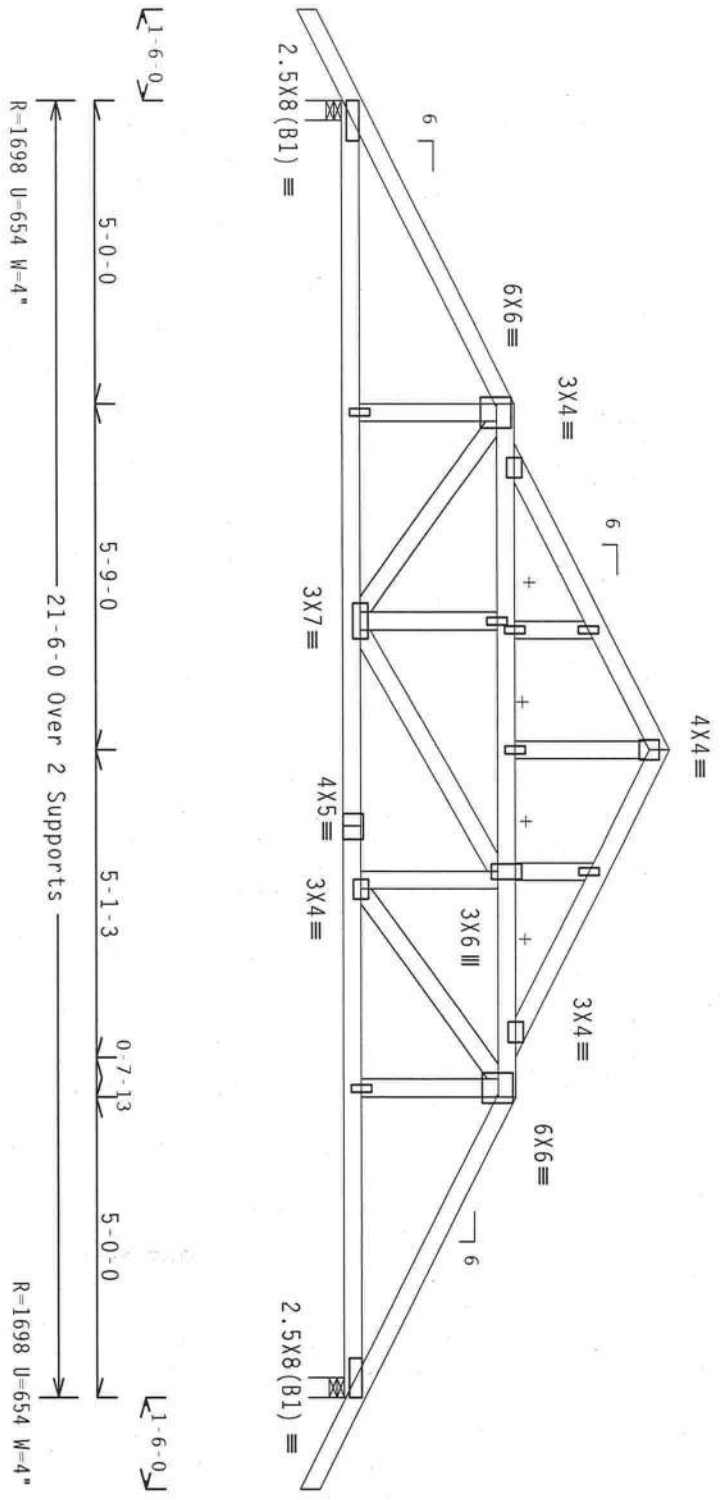
Gable end supports 8" max rake overhang.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.18

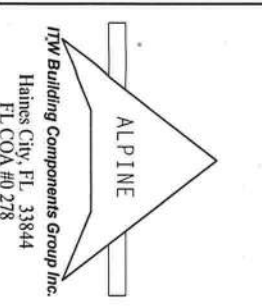
Wind reactions based on MWFRS pressures.

Boston hip supports 5-0-0 jacks to BC. TC supports 1-0-0 overhang.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSD (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO REPERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF MEMBER OR JOINT OF THE TRUSS IN COMPLIANCE WITH THE DESIGN CONCEPTS WITH APPLICABLE PROVISIONS OF BOS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/19/16GA (R-H/SS/Z) ASTM A653 GRADE 40/50 (W, K/H, SSI) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS E604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228-95320
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCSUR8228 10006034
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SECN	75464
DUR. FAC.	1.25		
SPACING	24.0"	JREF	1TY88228Z05

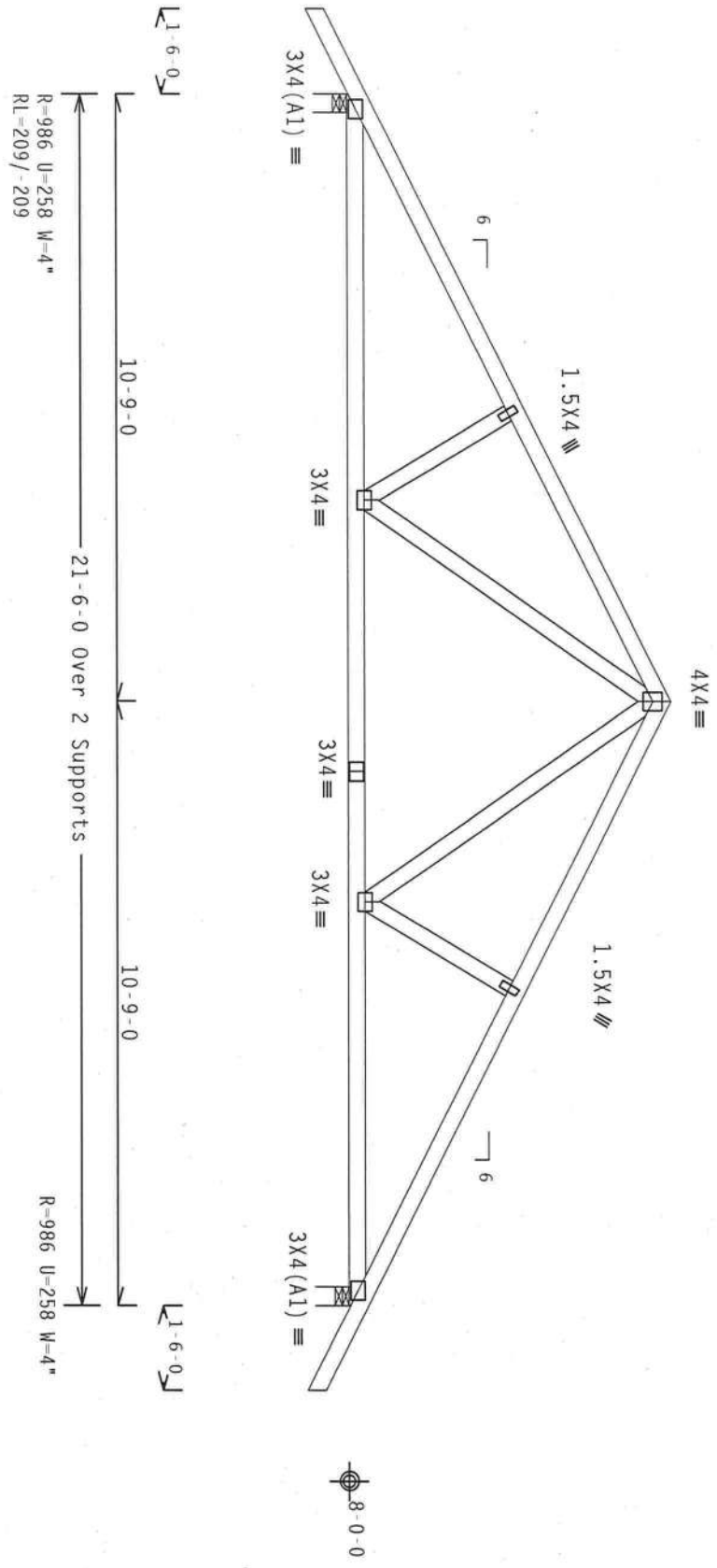
QTY: 1 FL-/4/-/-/R/- Scale = .3125"/ft.
06'10"

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.
 Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)-0.18

Wind reactions based on MWFRS pressures.
 Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

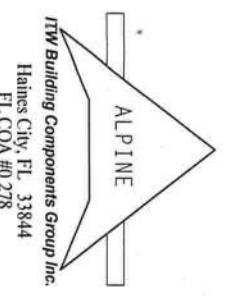
Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY: 7

FL/-/4/-/R/-

Scale = .3125"/ft.



****WARNING**** BRUSSES, ROUTINE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.



TC LL	20.0 PSF	REF	R8228-95321
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006008
BC LL	0.0 PSF	HG-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN	75417
DUR. FAC.	1.25		
SPACING	24.0"	JREF	1TY88228205

Top chord 2x4 SP #2 Dense
 Bot chord 2x6 SP #1 Dense : B2 2x6 SP #2:
 Webs 2x4 SP #3 : W5 2x4 SP #2 Dense:

Special Loads

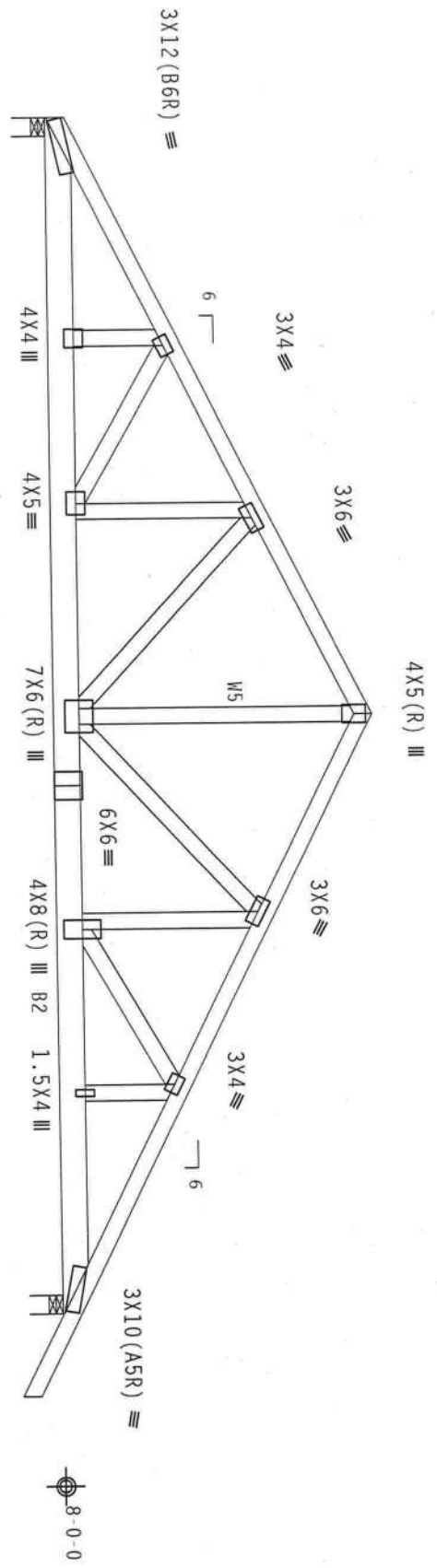
TC	From	Dur.	Fac.	-1.25 /	Plate	Dur.	Fac.	-1.25)
TC	From	62 pif	at	0.00	to	62 pif	at	10.75
TC	From	62 pif	at	10.75	to	62 pif	at	23.00
BC	From	20 pif	at	0.00	to	20 pif	at	21.50
BC	From	4 pif	at	21.50	to	4 pif	at	23.00
BC	From	1049 lb	Conc.	Load	at	0.44		
BC	From	1044 lb	Conc.	Load	at	6.44		
BC	From	1057 lb	Conc.	Load	at	10.44		
BC	From	1013 lb	Conc.	Load	at	12.44		
BC	From	2370 lb	Conc.	Load	at	14.44		

Deflection meets L/240 live and L/180 total load.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 1 Row @ 3.50" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi(+/-)=0.18
 Wind reactions based on MMFRS pressures.
 Roof overhang supports 2.00 psf soffit load.



10-9-0
 21-6-0 Over 2 Supports
 10-9-0
 R=6802 U=1335 W=4"
 R=4745 U=1360 W=4"

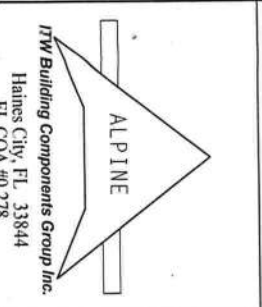
PLT TYP. Wave

Design Cmt: FBC2007Res/TP1-2002 (STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

Scale = .3125"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY IMPROVEMENT) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 100 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FABRICATING OR BRACING OF TRUSSES SHOULD BE DONE IN ACCORDANCE WITH THE NATIONAL DESIGN SPEC. BY AEPRA) AND TPI. THE BCG DESIGN ENGINEERS HAVE MADE OF 2017/8/16/66 (04/4/55X) ASIN 6653 GRADE 40/60 (4, K/IN/55) GALV. STEEL. APPLY CHECKS TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



FL/-/4/-/-/R/-	OTV:1	REF	DATE	DATE	DATE	DATE
TC LL	20.0 PSF	R8228	01/06/10	95322		
TC DL	10.0 PSF					
BC DL	10.0 PSF	DRW	HCUSR8228	10006035		
BC LL	0.0 PSF	HC-ENG	JB/DF			
TOT.LD.	40.0 PSF	SEQN-	75771			
DUR.FAC.	1.25					
SPACING	24.0"	JREF-	1TY88228Z05			

Top chord 2x4 SP #2 Dense : 12 2x6 SP #2:
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.

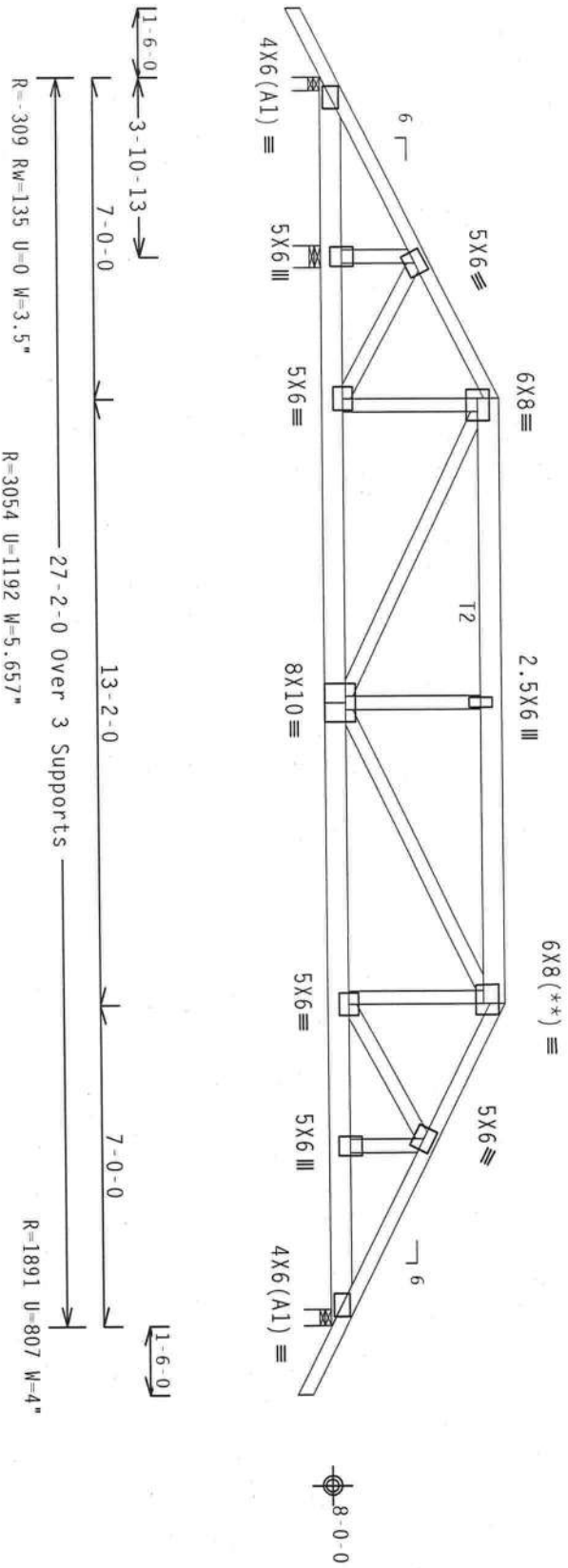
Negative reaction(s) of -308# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang.



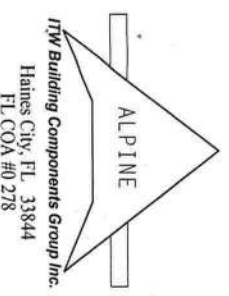
PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

Scale = .25"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO RCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE NATIONAL INSTITUTE OF STANDARDS AND TECHNOLOGY, 400 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 FOR SAFETY GUIDELINES. CONTACT THE NATIONAL INSTITUTE OF STANDARDS AND TECHNOLOGY AT (800) 551-7273 FOR MORE INFORMATION. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES, TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (AIA) AND TPI. THE RCG CONNECTOR PLATES ARE MADE OF 2018/7664 (A/H/S/S) ASTM A653 GRADE 40/60 (A, K/H, S5) GALV. STEEL. THE RCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. THE RCG COMPONENTS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE ASSOCIATION OF BUILDING DESIGNERS PER ANSI/TPI 1 SEC. 2.



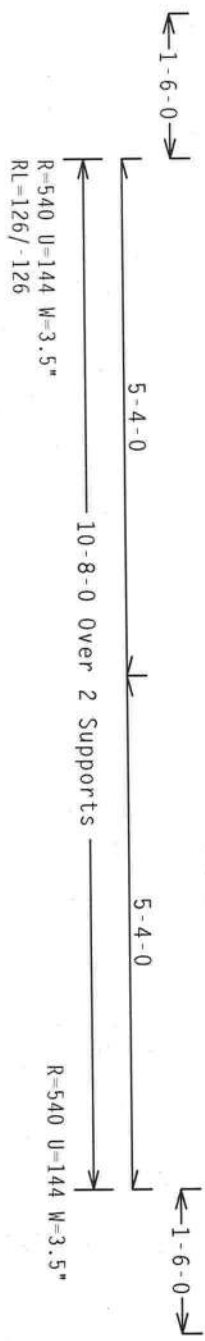
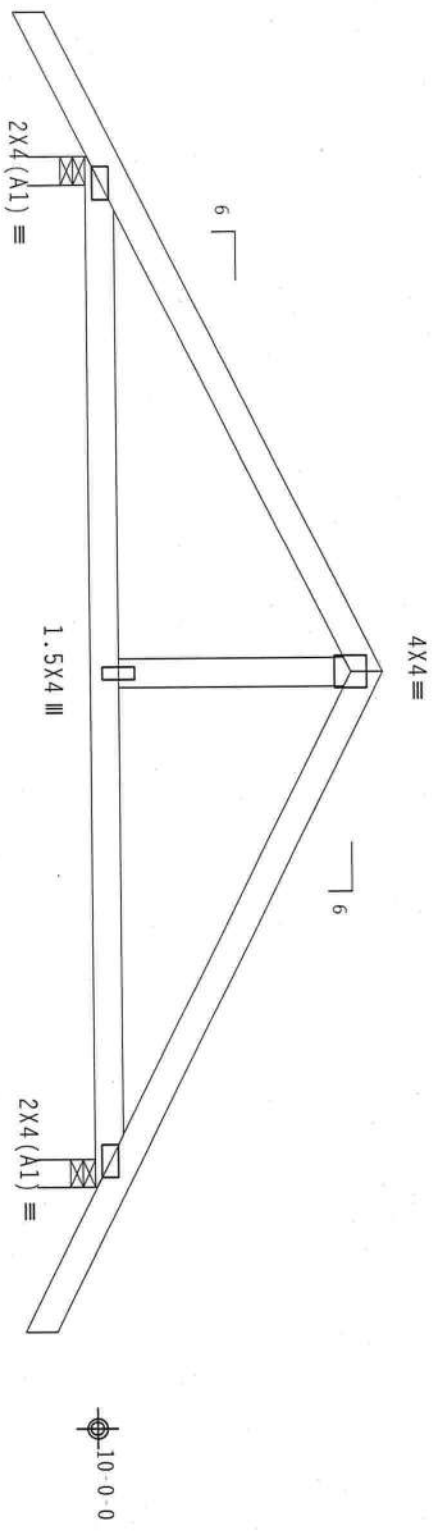
FL /- /4 /- /- /R /-	TC LL	20.0 PSF	REF	R8228- 95323
	TC DL	10.0 PSF	DATE	01/06/10
	BC DL	10.0 PSF	DRW	HCUSR8228 10006036
	BC LL	0.0 PSF	HC-ENG	JB/DF
	TOT. LD.	40.0 PSF	SEQN-	75396
	DUR. FAC.	1.25		
	SPACING	24.0"	JREF-	1TY88228Z05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.
 Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.
 Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crts: FBC2007Res/TP1-2002(STD)
 FT/RT=10% (0%)/0 (0)

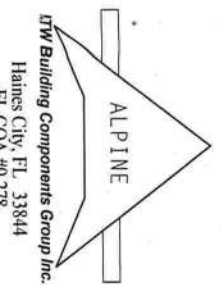
9.02.00

QTY: 2 FL/-/4/-/ -/R/-

Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSS (QUALITY CONTROL COMPONENT SAFETY AND PROTECTION) FOR TRUSS SAFETY PRACTICES. 218 NORTH TEE STREET, SUITE 312, ALEXANDRIA, VA 22304. CONTACT THE QUALITY CONTROL BOARD OF AMERICA, 6300 ENTERPRISE LANE, SUITE 100, WASHINGTON, DC 20004. SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE NOTED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** PURCHASER A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AREA AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS) ASH 6653 GRADE 40/60 (W, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. UNLESS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASST/TP1 1 SEC.-2.



DW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0278

TC LL	20.0 PSF	REF	R8228-95326
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCUSR8228 10006009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	75375
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TY88228205

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

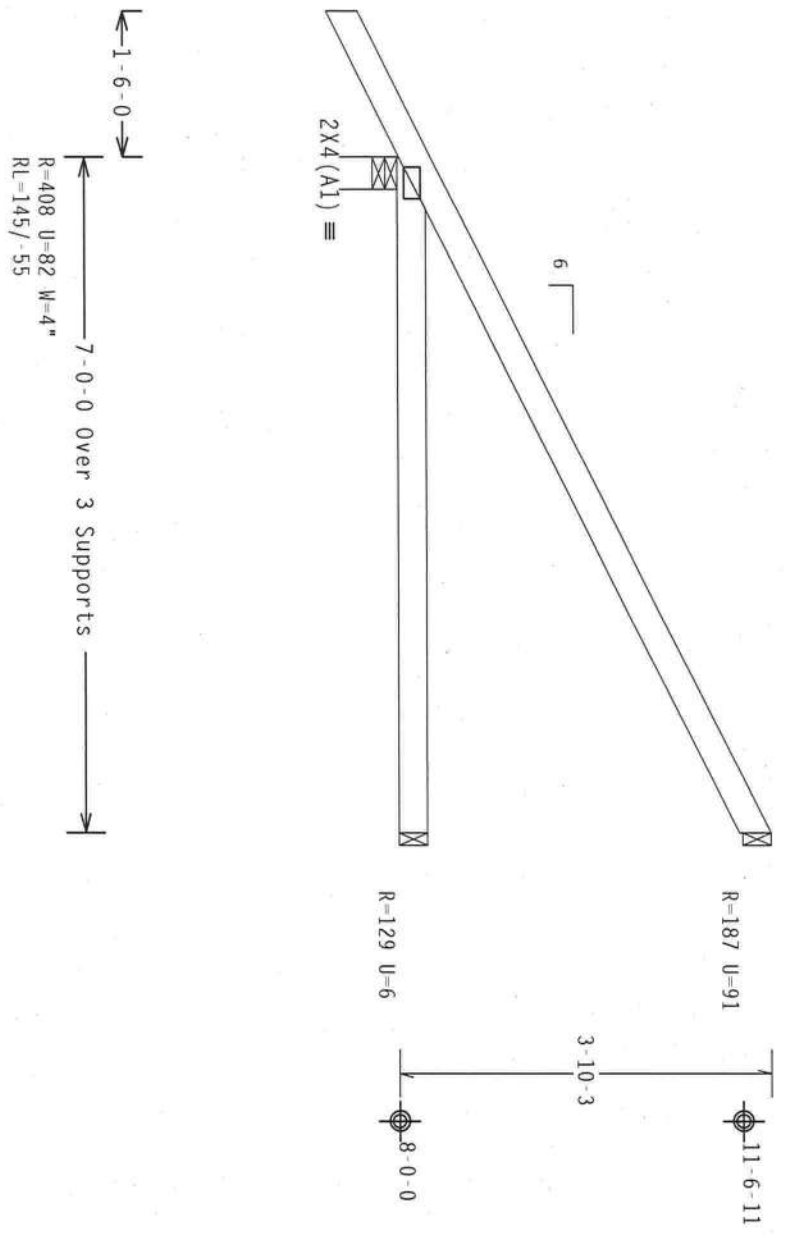
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ gcpl (+/-) = 0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

8.07.00

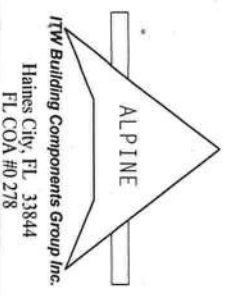
QTY: 24 FL/-/4/-/1/R/-

Scale = .5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WETA (WOOD TRUSS CONSTRUCTION STANDARDS, 2180 EXETER STREET, HOUSTON, TX 77059) FOR SAFETY PRACTICES. ALL TRUSS FABRICATORS SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF TRUSS PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. THE REG. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/VS) ASTM A653 GRADE 40/60 (W, W/H, SS) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 95327
TC DL	10.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUR8228 10006064
BC LL	0.0 PSF	HC-ENG TCE/DLJ
TOT. LD.	40.0 PSF	SEON- 59357
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1TY88228205

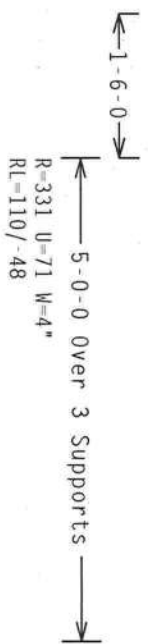
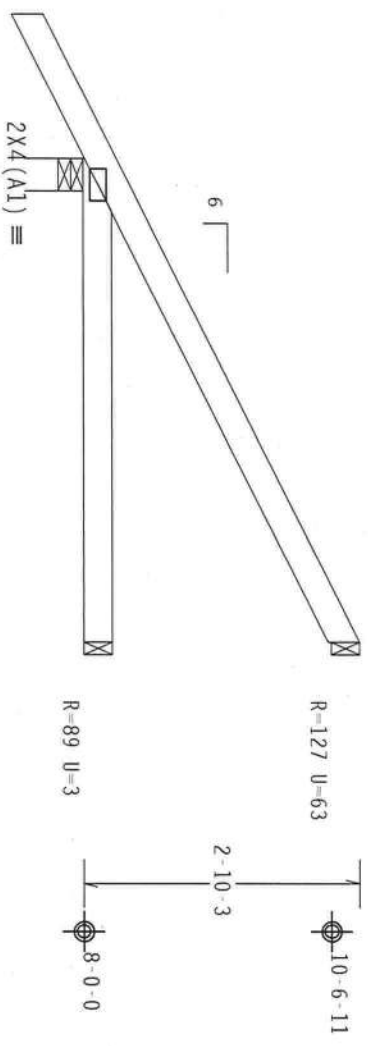
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCP(+/-)=0.18
Wind reactions based on MMFRS pressures.
Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

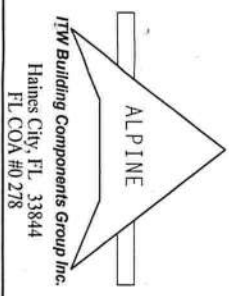
9.02.00 QTY:6

FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, WELDING, SHIPPING, INSTALLING AND BRACING. PROTECT FROM DAMAGE AND CORROSION. SAFETY INFORMATION PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WEA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HANSON, WI 53129) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN FROM THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN. SIGNATURE OF THE DESIGNER SHALL BE PROPERLY ATTACHED TO THE TRUSS COMPONENT DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

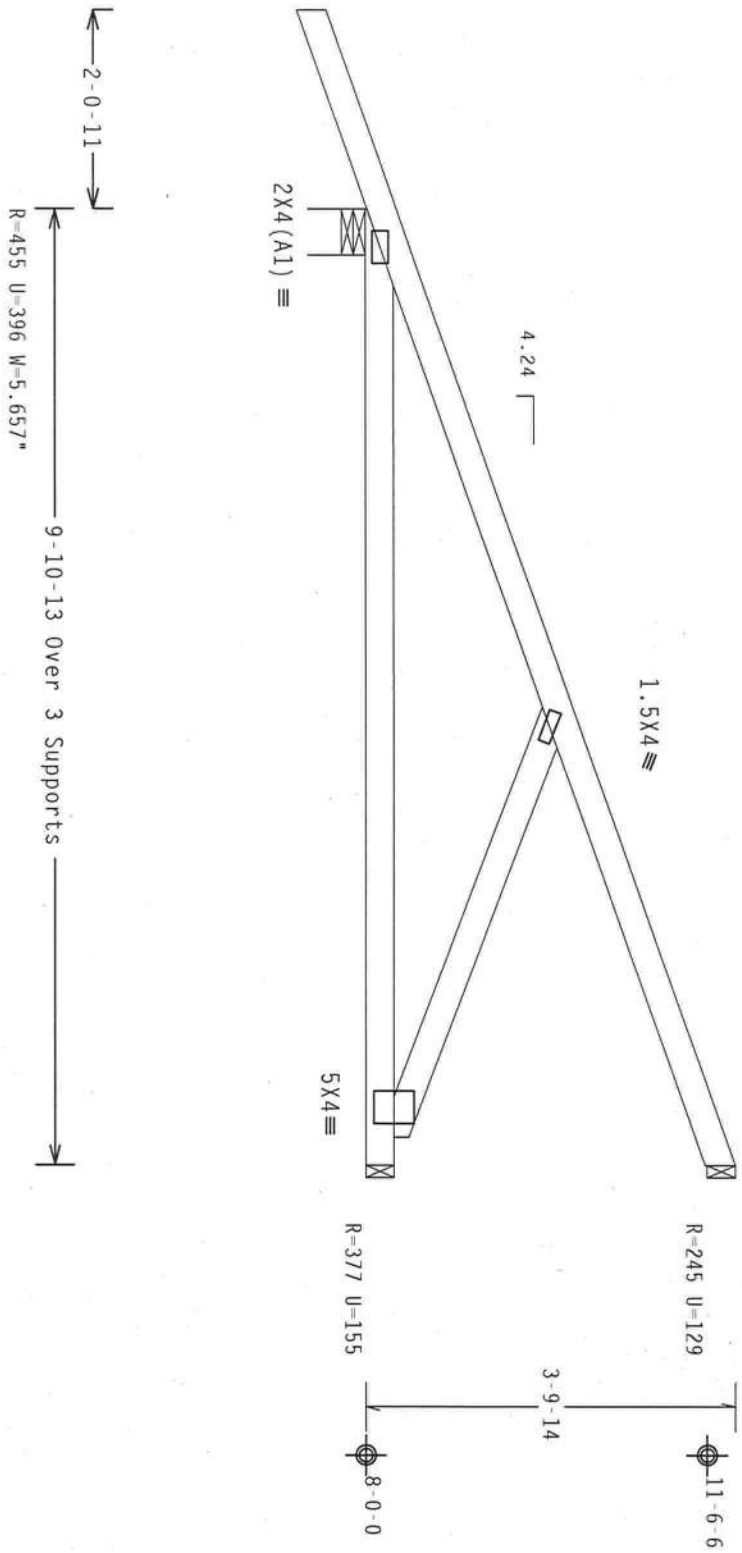


TC LL	20.0 PSF	REF R8228- 95329
TC DL	10.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUSR8228 10006026
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEON- 75308
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TY88228Z05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Hipjack supports 7-0-0 setback jacks with no webs.
 Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $Gcp1(+/-)=0.18$
 Wind reactions based on MWFRS pressures.
 Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
 Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 FT/RT=10%(0%)/0(0)

9.02.00

QTY: 3

FL/-/4/-/1-/R/-

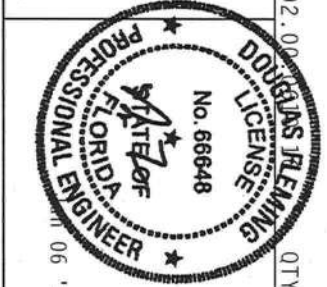
Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. BEFORE PRESS FORMING CONCRETE, REINFORCING STEEL (MECHANICALLY PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WEA GOOD TRUSS COUNCIL OF AMERICA, 6100 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY ARKPA) AND TPI. THE BCG DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF B05 (NATIONAL DESIGN SPEC. BY ARKPA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/S) ASTM A653 GRADE 40/50 (W. K/H. SSI GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY THE DESIGNER SHALL BE THE DESIGNER'S RESPONSIBILITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 95330
TC DL	10.0 PSF	DATE 01/06/10
BC DL	10.0 PSF	DRW HCUR8228 10006038
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEON- 75326
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TY88228Z05

Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

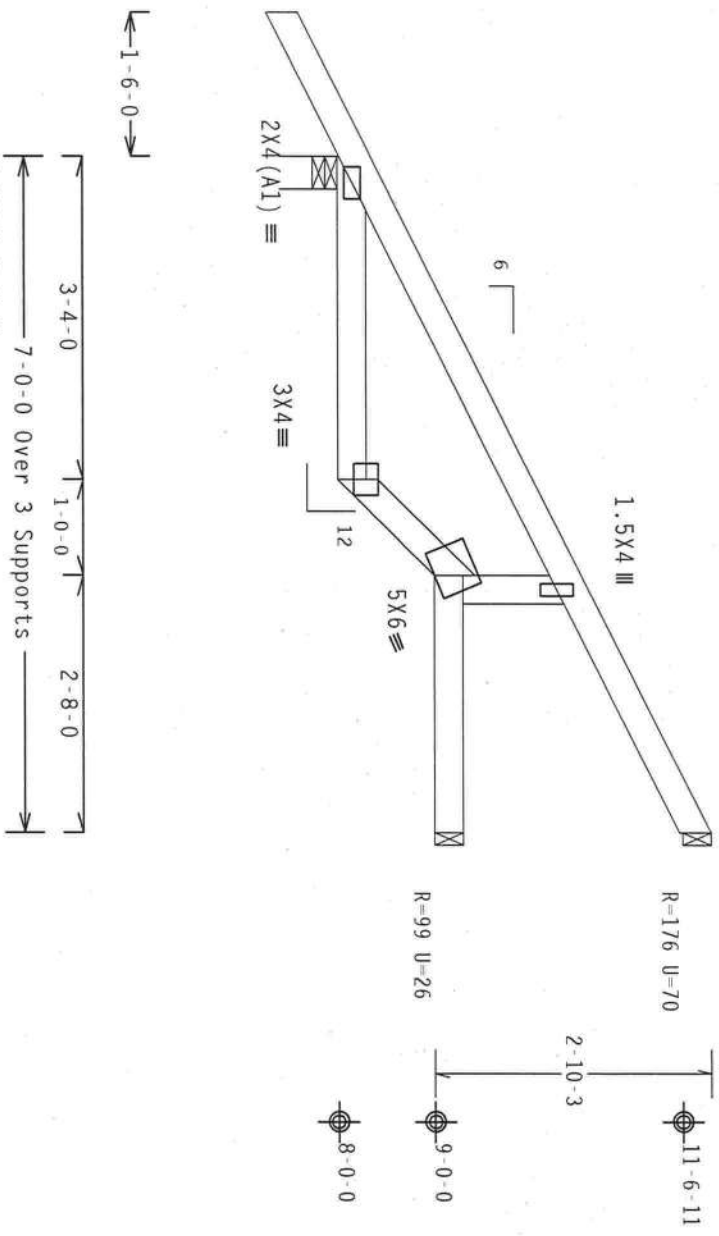
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

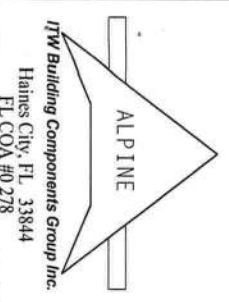


PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
 FT/RT=10%(0%)/0(0)

QTY: 2 FL/-/4/-/1-/R/-

Scale = .5"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE OF TRUSSES, OR CONNECTIONS, OR BRACING, OR DESIGN COMPONENTS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI. THE BCG CONNECTION PLATES ARE MADE OF 20/10/16GA (U, W, S, X) ASH A663 GRADE 40/60 (U, W, S, X) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC. 3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 95335
TC DL	10.0 PSF	DATE	01/06/10
BC DL	10.0 PSF	DRW	HCSR8228 10006029
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SE0N-	75336
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TY88228Z05

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE, FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

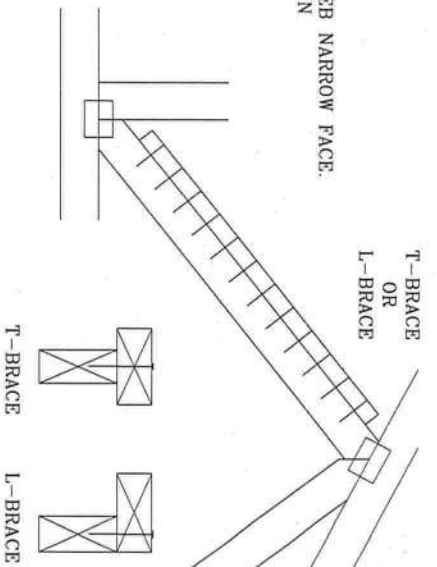
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW 2 ROWS	2X4 2X6	1-2X4 2-2X4
2X6	1 ROW 2 ROWS	2X4 2X6	1-2X6 2-2X4(*)
2X8	1 ROW 2 ROWS	2X6 2X6	1-2X8 2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

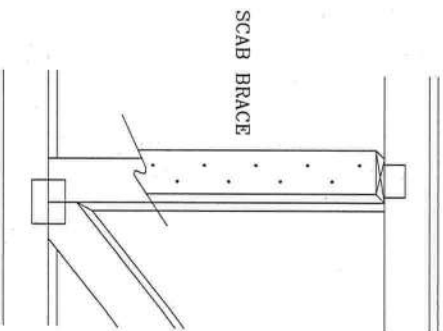
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3." MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3." MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



Building Components Group Inc.

Earth City, MO 63045

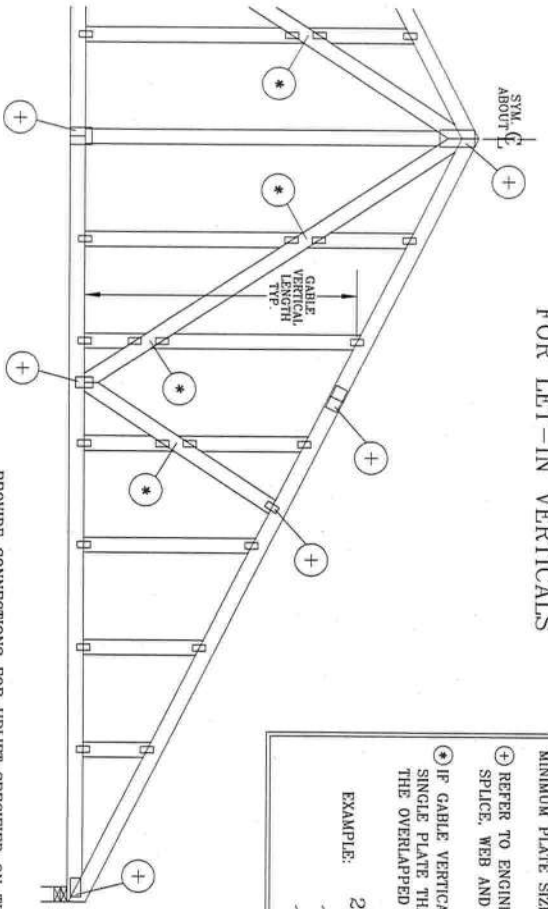
****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached right end panel. All panels shall have the bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/16/18GA (41H/5/8) ASTM A653 grade 57/40/60 (0.0156" thick) galvalume coated steel. All panels shall be attached to the truss in conformance with the responsibility of the Building Designer. Per ANSI/TPI 1 Sec. 2.
ITW-BCSI: www.bcsi.com; TPI: www.tpi.net; WCA: www.wcaindustry.com; ICC: www.iccsafe.org.



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

GABLE DETAIL FOR LET-IN VERTICALS



GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK SPLICE, WEB AND HEEL PLATES.

⊙ IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH END DRIVEN NAILS:

10d COMMON (0.148" X 3.3" MIN) NAILS AT 4" O.C. PLUS

(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILS:

10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015980109, A12015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109

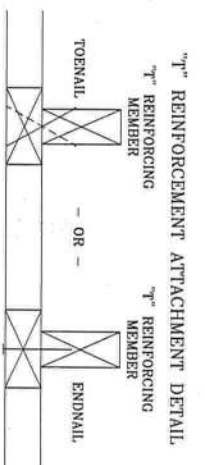
ASCE 7-02 GABLE DETAIL DRAWINGS

A13015020109, A12015020109, A10015020109, A14015020109, A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015050109, A12015050109, A10015050109, A14015050109, A13030050109, A12030050109, A11030050109, A10030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "L" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"L" REINFORCING MEMBER SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	50 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT, $K_{zt} = 1.00$

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"T" BRACE INCREASE (FROM ABOVE) = 10% = 110

(1) 2X4 "L" BRACE LENGTH = 6'-7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"



Building Components Group Inc.

Earth City, MO 63045

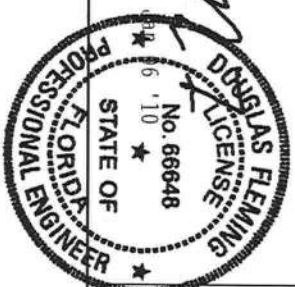
****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BRS Building Component Safety Information, by TR and WTA) for safety practices prior to performing any work on trusses. Truss bracing shall be installed in accordance with the design drawings. Truss bracing shall have properly attached end plates. Truss bracing shall be installed in accordance with the design drawings. Truss bracing shall be installed in accordance with the design drawings. Truss bracing shall be installed in accordance with the design drawings.

****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, or for any damage to the building or its contents, or for any injury to persons or property, arising from the use of any truss or bracing of trusses. ITWBCG connector plates are made of 2016/1606 (NHS/76) ASTM A36 (K/W/HS) galv. steel. Apply plates to each face of truss, positioned as shown above and on final details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/PPI 1 Sec. 2.

ITW-BDC: www.itwbcg.com; TFI: www.tfi.com; WTA: www.abindustry.com; ICC: www.iccsafe.org



REF	LET-IN VERT
DATE	1/1/09
DRWG	GBLETTN0109
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"