

Columbia County New Building Permit Application

For Office Use Only		Application # <u>44479</u>	Date Received <u>2/7</u>	By <u>MG</u>	Permit # <u>39357/39358</u>
Zoning Official <u>LW/UT</u>	Date <u>2-10-20</u>	Flood Zone <u>X</u>	Land Use <u>EL0</u>	Zoning <u>RSF-2</u>	
FEMA Map # _____	Elevation _____	MFE _____	River _____	Plans Examiner <u>J.C.</u>	Date <u>2-13-20</u>
Comments <u>1' above road</u>					
☐ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Well letter ☐ 911 Sheet ☐ Parent Parcel # _____ ☐ Dev Permit # _____ ☐ In Floodway ☒ Letter of Auth. from Contractor ☐ F W Comp. letter _____ ☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☐ App Fee Paid ☒ Sub VF Form _____					

Septic Permit No. 20-0094 OR City Water ☐ Fax _____

Applicant (Who will sign/pickup the permit) Hayden Lunde Phone 386-867-1213

Address 618 SW Florida Gateway Dr LC, FL 32024

Owners Name Hayden & Katelyn Lunde Phone 386-867-1213

911 Address 298 NW Horizon St LC, FL 32055

Contractors Name Blake N Lunde II Phone 386-754-5810

Address 618 SW Florida Gateway DR LC, FL 32024

Contractor Email blake@blakeconstruction.com ***Include to get updates on this job.

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Disosway Engineering 163 SW Midtown PL #103 LC, FL 32025

Mortgage Lenders Name & Address First Federal 2571 W US Hwy 90 LC, FL 32055

Circle the correct power company ☒ FL Power & Light ☐ Clay Elec. ☐ Suwannee Valley Elec. ☐ Duke Energy

Property ID Number 02374-081 Estimated Construction Cost 225000

Subdivision Name _____ Lot _____ Block _____ Unit _____ Phase _____

Driving Directions from a Major Road 90 West to Brown road T-R go to Horizon Rd T-L property on left

Construction of single family residence Commercial OR ☒ Residential

Proposed Use/Occupancy RSF Number of Existing Dwellings on Property 0

Is the Building Fire Sprinkled? NO If Yes, blueprints included _____ Or Explain _____

Circle Proposed ☒ Culvert Permit or ☐ Culvert Waiver or ☐ D.O.T. Permit or ☐ Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 215' Side 75' Side 122' Rear 180'

Number of Stories 1 Heated Floor Area 1879 Total Floor Area 3094 Acreage 3

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

2/18 email sent MG

Columbia County Building Permit Application

CODE: Florida Building Code 2017 and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Hayden Lunde

Print Owners Name

Hayden Lunde

Owners Signature

Digitally signed by Hayden Lunde
Date: 2020.01.24 13:33:26 -05'00'

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Blake N Lunde II

Contractor's Signature

Digitally signed by Blake N Lunde II
Date: 2020.01.24 13:34:32 -05'00'

Contractor's License Number CBC1253408
Columbia County
Competency Card Number 498

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 24 day of January 2020.

Personally known ☒ or Produced Identification ☐

Linda L. Carter

State of Florida Notary Signature (For the Contractor)

SEAL:

Linda L. Carter
NOTARY PUBLIC
STATE OF FLORIDA
Commission # 17004932
Expires 2/16/2020



COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave, Suite B-21, Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

LETTER OF AUTHORIZATION TO SIGN FOR PERMITS

I, Blake N Lunde II (license holder name), licensed qualifier
for Blake Construction Company of North Florida Inc (company name), do certify that
the below referenced person(s) listed on this form is/are contracted/hired by me, the license
holder, or is/are employed by me directly or through an employee leasing arrangement; or, is an
officer of the corporation; or, partner as defined in Florida Statutes Chapter 468, and the said
person(s) is/are under my direct supervision and control and is/are authorized to purchase
permits, call for inspections and sign on my behalf.

Printed Name of Person Authorized	Signature of Authorized Person
1. Hayden Lunde	1.
2.	2.
3.	3.
4.	4.
5.	5.

I, the license holder, realize that I am responsible for all permits purchased, and all work done
under my license and fully responsible for compliance with all Florida Statutes, Codes, and
Local Ordinances. I understand that the State and County Licensing Boards have the power and
authority to discipline a license holder for violations committed by him/her, his/her agents,
officers, or employees and that I have full responsibility for compliance with all statutes, codes
and ordinances inherent in the privilege granted by issuance of such permits.

If at any time the person(s) you have authorized is/are no longer agents, employee(s), or
officer(s), you must notify this department in writing of the changes and submit a new letter of
authorization form, which will supersede all previous lists. Failure to do so may allow
unauthorized persons to use your name and/or license number to obtain permits.

Blake N Lunde II Digitally signed by Blake N Lunde II
Date: 2020.01.24 13:47:20 -05'00' CBC1253408 1-24-20
License Holders Signature (Notarized) License Number Date

NOTARY INFORMATION:
STATE OF: Florida COUNTY OF: Columbia

The above license holder, whose name is Blake N Lunde II
personally appeared before me and is known by me or has produced identification
(type of I.D.) personally known on this 24th day of January, 2020.

Linda L. Carter
NOTARY'S SIGNATURE



Linda L. Carter
NOTARY PUBLIC
STATE OF FLORIDA
Comm# FF954932
Expires 2/16/2020

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # _____ JOB NAME Lunde Residence

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL ☒	Print Name <u>Matt Burns</u> Signature <u>Matt Burns</u> Company Name: <u>Matt Burns Electric</u> License #: <u>EC13006531</u> Phone #: <u>386-935-0444</u>	Verified by PDFfiller 01/10/2020 Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
MECHANICAL/ A/C ☒	Print Name <u>Clint G Wilson</u> Signature <u>Clint Wilson</u> Company Name: <u>Wilson Heat & Air</u> License #: <u>CAC057886</u> Phone #: <u>386-496-9000</u>	Verified by PDFfiller 01/10/2020 Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
PLUMBING/ GAS ☒	Print Name <u>Don Bills</u> Signature <u>Don Bills</u> Company Name: <u>Hometown Plumbing Service</u> License #: <u>CFC1428890</u> Phone #: <u>386-754-6140</u>	Verified by PDFfiller 01/10/2020 Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
ROOFING ☒	Print Name <u>Blake N Lunde II</u> Signature <u>Blake N Lunde II</u> Company Name: <u>Blake Construction Company</u> License #: <u>CBC1253408 1331699</u> Phone #: <u>386-754-5810</u>	Verified by PDFfiller 01/10/2020 Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
SHEET METAL ☐	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
FIRE SYSTEM/ SPRINKLER ☐	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
SOLAR ☐	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
STATE SPECIALTY ☐	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need ☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE

Columbia County Property Appraiser

Jeff Hampton

2020 Working Values

updated: 1/6/2020

Parcel: << 28-3S-16-02374-081 >>

Owner & Property Info

Result: 10 of 13

Owner	LUNDE HAYDEN & KATELYN 244 SE PINE DR LAKE CITY, FL 32025		
Site			
Description*	COMM SW COR OF NE1/4 OF NW1/4, RUN N 242.97 FT FOR POB, CONT N 639.89 FT TO S R/W OF A CO RD, SE ALONG RD 304.57 FT, S 532.17 FT, W 276.86 FT TO POB, EX 0.60 AC DESC ORB 1210- 1876. 412-635, PB 754-167, QC 1299-1823, WD 1307-2218, WD 1374-1999, WD 1395- ...more>>>		
Area	3 AC	S/T/R	28-3S-16
Use Code**	VACANT (000000)	Tax District	2

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.

**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Property & Assessment Values

2019 Certified Values		2020 Working Values	
Mkt Land (1)	\$31,496	Mkt Land (1)	\$31,496
Ag Land (0)	\$0	Ag Land (0)	\$0
Building (0)	\$0	Building (0)	\$0
XFOB (0)	\$0	XFOB (0)	\$0
Just	\$31,496	Just	\$31,496
Class	\$0	Class	\$0
Appraised	\$31,496	Appraised	\$31,496
SOH Cap [?]	\$0	SOH Cap [?]	\$0
Assessed	\$31,496	Assessed	\$31,496
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$31,496 city:\$31,496 other:\$31,496 school:\$31,496	Total Taxable	county:\$31,496 city:\$31,496 other:\$31,496 school:\$31,496

Aerial Viewer Pictometry Google Maps

2019 2016 2013 2010 2007 2005 Sales



Sales History

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
9/27/2019	\$37,000	1395/1137	WD	I	Q	01
4/9/2018	\$100	1374/1999	WD	I	U	11
1/8/2016	\$100	1307/2218	WD	V	U	30
8/5/2015	\$100	1299/1823	QC	V	U	11

Building Characteristics

Bldg Sketch	Bldg Item	Bldg Desc*	Year Blt	Base SF	Actual SF	Bldg Value
NONE						

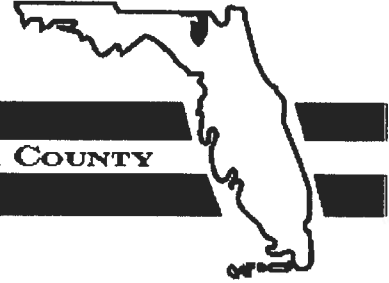
Extra Features & Out Buildings (Codes)

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Land Code	Desc	Units	Adjustments	Eff Rate	Land Value
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District No. 1 - Ronald Williams
District No. 2 - Rocky Ford
District No. 3 - Bucky Nash
District No. 4 - Toby Witt
District No. 5 - Tim Murphy



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued: **12/16/2019 8:46:16 PM**
Address: **298 NW HORIZON St**
City: **LAKE CITY**
State: **FL**
Zip Code **32055**

Parcel ID **02374-081**

REMARKS: Address for proposed structure on parcel.

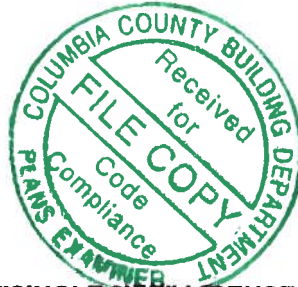
NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: **Signed:/ Matt Crews**

Columbia County GIS/911 Addressing Coordinator

**COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT**

**263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125
Email: gis@columbiacountyfla.com**



David E. Wiggins - Architect
2725 Highland Trail
Leander, TX. 78641
PHONE: (512) 260-2750
FAX: (512) 260-1649
wiggins_david@att.net

REPRODUCIBLES/PDF/CAD File "SINGLE-USE" LICENSE to use HOUSE PLAN # 5517

David E. Wiggins - Architect (Designer) grants you a non-exclusive license to use this house plan and related documentation (hereafter referred to as "design") for the construction of your or your client's home, subject to the following terms and conditions.

YOU MAY:

- Provide this design, in whole or part, to others (sub-contractors, suppliers, architects, engineers and others (herein referred to as "Agents") for the purpose of allowing them to assist in the estimating and construction of **one structure**.
- Use this design, as is or altered by you (or your agents), for the sole purpose of constructing **one structure**, at the site listed below.
- Print or make additional copies of this plan for the sole purpose of constructing **one structure** at the site listed below.

YOU MAY NOT:

- Sell this design or altered designs based on this design.
- Build more than a single structure from these plans, in their original or altered form.
- Distribute or transfer this design in its original or altered form, in any way or on any media, except as necessary to complete alterations through your agents or except as necessary to use this design to construct **one structure** at the site listed below.
- Imply that DFD or the architect/designer is responsible for any revisions to the design as altered by you or your agents.
- Remove the architect/designer copyright or the logo block that identifies the design as a derivative of the original architect/designer stock house plan.
- Extract any portion of this information from the context of this design and apply to other projects or distribute to others, any part or portion of the design. All symbols, graphic conventions and collections of drawn entities contained within these drawings, representing the design, are considered a portion of the whole and are subject to all the terms and conditions herein.

OWNERSHIP

The Designer retains title to, ownership of and all other proprietary rights in the original and all subsequent copies of the design and any part thereof, regardless of the form or media in which and which the original and other copies may exist. Title, ownership and all other proprietary rights remain with such architect/designer even if the design or any part thereof, are modified by you or your agents or are included in altered designs created or modified by your or your agents. This license is not a sale.

GENERAL PROVISIONS

In no event shall the Designer, DFD or any entity under its control be held liable for any part of this design, as modified by you or any Agent. You are fully responsible for adapting the original design to the requirements of your building site and any applicable local codes and/or regulations.

Failure to comply with any term or condition of this license agreement will automatically terminate this license and will make available to the Designer other legal remedies.

This license agreement is the entire agreement between the Designer and you, the end user, and supersedes any other communications or advertising with respect to this design and related documentation. If any provision of this license agreement is held to be invalid, the remainder of this agreement shall continue in full force and effect.

End User (You)		Site Address (Address you will build your new home)	
Name:	Hayden Lunde		
Address:	244 se pine dr	298 NW Horizon St	
	Lake city, FL 32025	Lake City, FL	32055
Phone:	3868671213		
Email:	haydenlunde@hotmail.com		

With my signature, I do hereby agree to the above terms and conditions and indicate my understanding that **ALL SALES ARE FINAL**.

Signed Hayden Lunde Date 10/25/2019

Signed: 

Print Full Name Hayden Lunde

By: **David Wiggins, David E. Wiggins - Architect**

INSTRUCTIONS: Complete both **End User** and **Site Address** boxes. **SITE ADDRESS MUST BE ENTERED.**

By electronically signing and sending this form, you are legally agreeing to the above terms. Plans can't ship until agreement is received by signing this form and acknowledging and agreeing that electronic files are not exchangeable and non-refundable.

your specific building codes, we recommend contacting your local building department.

Now that you have your house plans, make sure to view the important information we provide on our website at DFDHousePlans.com. From [great articles on home building](#) to [new home product ideas](#), we have a lot of useful advice and guidance to help your client understand the different phases of building a new home.

If there is anything that you need along the way, don't hesitate to [email](#), [livechat](#), or call us at **877-895-5299**. We are happy to assist you in any way that we can.

Best wishes with your new home building project!

Direct From The Designers™
483 Monroe Turnpike, Suite 115
Monroe, CT 06468
877-895-5299 Orders, Questions & Shipping Information
admin@dfdhouseplans.com

Order Number: 164721
Order Date: 10/26/2019
Invoice Number: SJ25183524-532
Order Number: 164721



[View PDFs NOW!™ Downloads Available](#)

Bill To:

Hayden Lunde
Blake construction company
244 se pine dr
Lake city, FL FL 32025
United States

Phone: 3868671213
Email: haydenlunde@hotmail.com

Ship To:

Deliver by Email

New Home Building Site:

244 se pine dr
Lake city, FL 32025

Ship Via: Deliver by Email

Payment Type: MC

Purchase Detail:

Line	Description	Qty	Unit Price	Total
1	Plan 5517 Durham Drive PDF PDFs NOW!™	1		

Shipping: PDFs are sent within one business day for free!

\$0.00

2 Plan 5517
Durham Drive
Slab
PDFs NOW!™

1

\$0.00

\$0.00

Sub Total:

Shipping:

Tax:

Total:

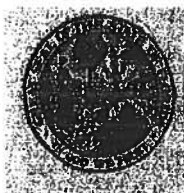
\$0.00

\$0.00

To ensure all of our customers are completely satisfied, you can exchange your first house plan order for a full credit towards another house plan purchase minus applicable restocking fees. Orders must be returned within 14 days of the original order and returned blueprints that look used, redlined or copied, are not exchangeable due to copyright rules and regulations.

Please note: Reproducibles, PDFs, CAD files, Study/Review Sets, PDF Bid Sets and Modified Plans are not exchangeable for any reason.

[Your Account.](#) [Privacy Policy.](#)



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM

APPLICATION FOR CONSTRUCTION PERMIT

APPLICATION FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Innovative
☐ Repair ☐ Abandonment ☐ Temporary ☐

APPLICANT: HAYDEN & KATELYN LUNDE

AGENT: BLAKE CONSTRUCTION

TELEPHONE: (386) 752-0675

MAILING ADDRESS: 618 SW FLORIDA GATEWAY DRIVE LAKE CITY FL 32024

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3) (m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: N/A BLOCK: N/A SUBDIVISION: METES AND BOUNDS PLATTED: _____

PROPERTY ID #: 28-3S-16-02374-081 ZONING: RES I/M OR EQUIVALENT: ☐ NO ☐

PROPERTY SIZE: 3.000 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ ≤ 2000 GPD ☐ > 2000 GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? ☐ NO ☐ DISTANCE TO SEWER: N/A FT

PROPERTY ADDRESS: 298 NW HORIZON STREET LAKE CITY, FL 32065

DIRECTIONS TO PROPERTY:

TAKE HWY 90 WEST, TURN RIGHT ON BROWN ROAD, TURN LEFT ON HORIZON DRIVE, SITE IS 4TH ON THE LEFT.

BUILDING INFORMATION ☒ RESIDENTIAL ☐ COMMERCIAL

Unit No.	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	HOUSE	3	1,879	
2				
3				
4				

☐ Floor/Equipment Drains ☒ Other (Specify) _____

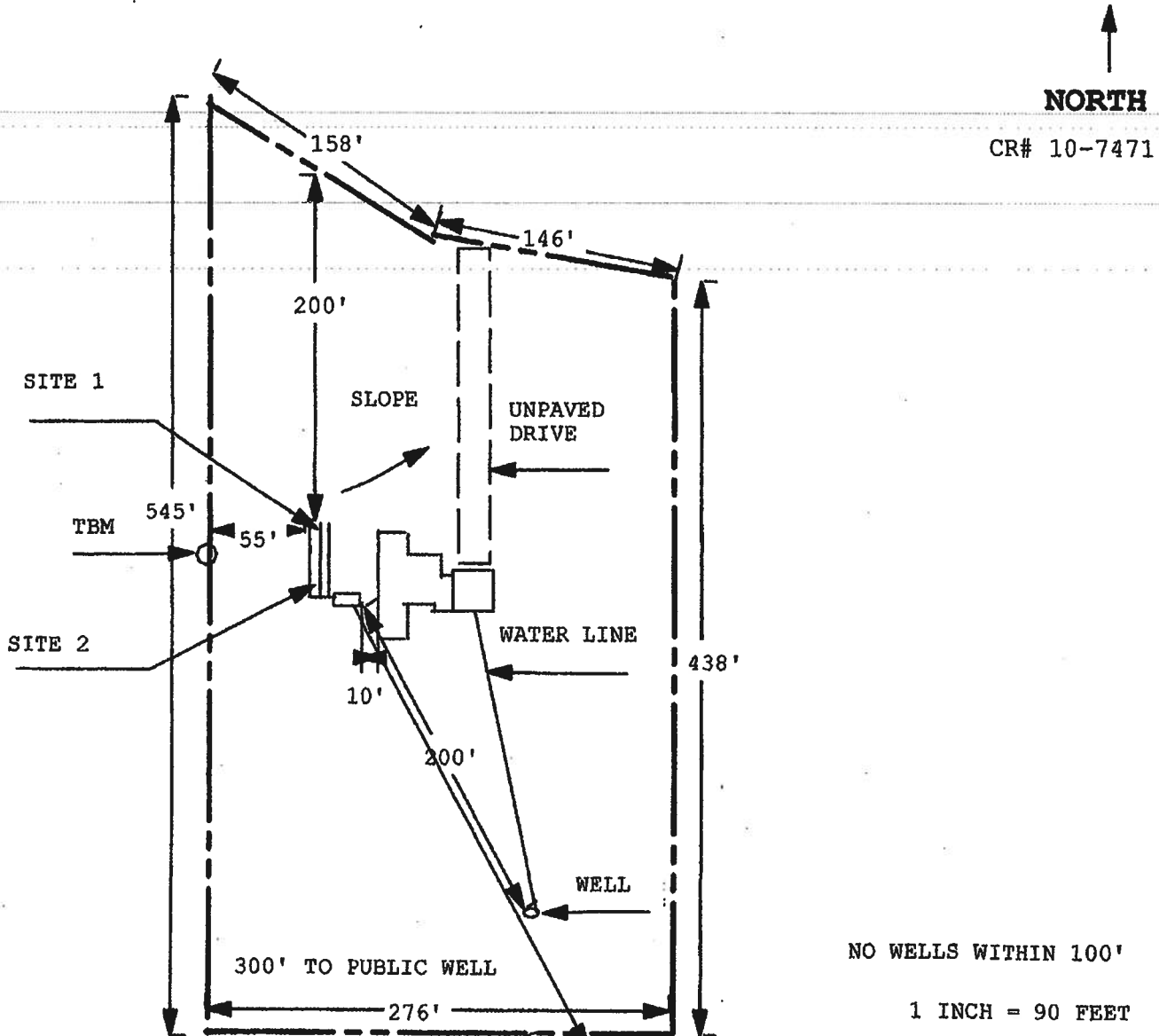
SIGNATURE: _____

DATE: 1/29/20

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 20-0094

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul R. Rapp Date 2/4/20
Plan Approved ☒ Not Approved ☐ Date 2.12.20

By Steve Kipph ESI. Columbia CPHU

Notes: _____

PREPARED BY & RETURN TO:

TRISH LANG, an employee of
Integrity Title Services, LLC
757 WEST DUVAL STREET
Lake City, FL 32055
File No. 19-12026TL

Inst: 202012003741 Date: 02/14/2020 Time: 3:56PM
Page 1 of 3 B: 1405 P: 1846, P.DeWitt Cason, Clerk of Court
Columbia, County, By: BD
Deputy Clerk

Permit No. _____

Tax Folio No. R02374-081

NOTICE OF COMMENCEMENT

State of Florida
County of Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: (legal description of the property, and street address if available)

TBD NW HORIZON STREET, LAKE CITY, FL 32055

Township 3 South, Range 16 East, Section 28: A part of the NE 1/4 of the NW 1/4 of Section 28, Township 3 South, Range 16 East, being more particularly described as follows:

Commence at the Southwest corner of the NE 1/4 of the NW 1/4 and run thence Northerly along the West boundary of said NE 1/4 of NW 1/4 a distance of 242.97 feet to the Northwest corner of Lot 11 of Country Dale Estates, a subdivision as recorded in Plat Book 4, Page 106, public records of Columbia County, Florida, and the POINT OF BEGINNING; thence continue Northerly along the West boundary of said NE 1/4 of NW 1/4 a distance of 639.89 feet to a concrete monument on the South right-of-way line of Horizon Drive, a 70 foot county road; thence run S 56°20'13" E along South right-of-way line of Horizon Drive 158.03 feet to a concrete monument; thence S 83°08'26" E along said South right-of-way line of Horizon Drive 146.54 feet to the Northwest corner of Lot 7 of Fairfield Hills, a subdivision as recorded in Plat Book 4, Pages 107 and 107-A ; thence run S 0°05'41" E along the West line of Lot 7, Fairfield Hills, a distance of 532.17 feet to the Southwest corner of said Lot 7; thence run Westerly along the Northern boundary of the aforesaid Country Dale Estates Subdivision, as recorded in Plat Book 4, Page 106, a distance of 276.86 feet to the Northwest corner of Lot 11 of Country Dale Estates Subdivision and the POINT OF BEGINNING.

LESS AND EXCEPT lands conveyed by Special Warranty Deed recorded in Official Records Book 1210, Page 1876, of the Public Records of Columbia County, Florida.

2. General description of improvement: **Construction of single family dwelling**

3. Owner information:

- a. Name and address: **HAYDEN B. LUNDE and KATELYN C. LUNDE
244 SE PINE DRIVE, LAKE CITY, FL 32025**
- b. Interest in property: **Fee Simple**
- c. Name and address of fee simple titleholder (if other than Owner):

4. Contractor:

- a. Name and address: **BLAKE CONSTRUCTION COMPANY OF NORTH FLORIDA
618 SW FLORIDA GATEWAY DRIVE, LAKE CITY, FL 32055**
- b. Phone number: **386-754-5810**



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 7/1/18

Website: <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Select From Drop down

1	Two (2) complete sets of plans containing the following:	☒			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	☒			
3	Condition space (Sq. Ft.) 1879	Total (Sq. Ft.) under roof 3094	Yes	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	Yes		
5	Dimensions of all building set backs	Yes		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	NA		
7	Provide a full legal description of property.	Yes		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
		Select From Drop down		
9	Basic wind speed (3-second gust), miles per hour	Yes		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	Yes		
11	Wind importance factor and nature of occupancy	Yes		
12	The applicable internal pressure coefficient, Components and Cladding	Yes		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifically designed by the registered design professional.	Yes		

Elevations Drawing including:

14	All side views of the structure	Yes		
15	Roof pitch	Yes		
16	Overhang dimensions and detail with attic ventilation	Yes		
17	Location, size and height above roof of chimneys	Yes		
18	Location and size of skylights with Florida Product Approval	NA		
19	Number of stories	Yes		
20	Building height from the established grade to the roofs highest peak	Yes		

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	Yes		
22	Raised floor surfaces located more than 30 inches above the floor or grade	-		
23	All exterior and interior shear walls indicated	Yes		
24	Shear wall opening shown (Windows, Doors and Garage doors)	Yes		
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	Yes		
26	Safety glazing of glass where needed	Yes		
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	Yes		
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	Yes		
29	Identify accessibility of bathroom (see FBCR SECTION 320)	Yes		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
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FBCR 403: Foundation Plans

Select From Drop down

30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Yes		
31	All posts and/or column footing including size and reinforcing	Yes		
32	Any special support required by soil analysis such as piling.	NA		
33	Assumed load-bearing value of soil Pound Per Square Foot	Yes		
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	Yes		

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints taped 6 inches and sealed)	Yes		
36	Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports	Yes		

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	Yes		
----	--	-----	--	--

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	Yes		
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	NA		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	Yes		
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	Yes		
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	Yes		
43	Attachment of joist to girder	Yes		
44	Wind load requirements where applicable	Yes		
45	Show required under-floor crawl space	NA		
46	Show required amount of ventilation opening for under-floor spaces	NA		
47	Show required covering of ventilation opening	NA		
48	Show the required access opening to access to under-floor spaces	NA		
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	Yes		
50	Show Draftstopping, Fire caulking and Fire blocking	Yes		
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	Yes		
52	Provide live and dead load rating of floor framing systems (psf).	Yes		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
---	--	--	--	--

Select from Drop down

53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	Yes		
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	Yes		
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	Yes		
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	Yes		
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	Yes		
58	Indicate where pressure treated wood will be placed	Yes		
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	Yes		
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	Yes		

FBCR :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10. 1 Wood trusses	Yes		
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	Yes		
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	Yes		
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Yes		
65	Provide dead load rating of trusses	Yes		

FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	-		
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	Yes		
68	Valley framing and support details	Yes		
69	Provide dead load rating of rafter system	-		

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	Yes		
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	Yes		

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assembles covering	Yes		
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	Yes		

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. **Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
---	--	--	--	--

Select from Drop Down

74	Show the insulation R value for the following areas of the structure	Yes		
75	Attic space	Yes		
76	Exterior wall cavity	Yes		
77	Crawl space	NA		

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	Yes		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	Yes		
80	Show clothes dryer route and total run of exhaust duct	Yes		

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	Yes		
82	Show the location of water heater	Yes		

Private Potable Water

83	Pump motor horse power	Yes		
84	Reservoir pressure tank gallon capacity	Yes		
85	Rating of cycle stop valve if used	Yes		

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	Yes		
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	Yes		
88	Show the location of smoke detectors & Carbon monoxide detectors	Yes		
89	Show service panel, sub-panel, location(s) and total ampere ratings	Yes		
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	Yes		
91	Appliances and HVAC equipment and disconnects	Yes		
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	Yes		

Notice Of Commencement:

A notice of commencement form **RECORDED** in the Columbia County Clerk Office is required to be filed with the Building Department **BEFORE ANY INSPECTIONS** can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
--	--

****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.****

Select from Drop down

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	Yes		
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	Yes		
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	Yes		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	NA		
97	Toilet facilities shall be provided for all construction sites	Yes		
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	NA		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	NA		
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	NA		
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	-		
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes		
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	Yes		

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Legend

SectionTownshipAndRange

SRWMD Wetlands



2018Aerials



2018 Flood Zones

0.2 PCT ANNUAL CHANCE



A

AE

AH

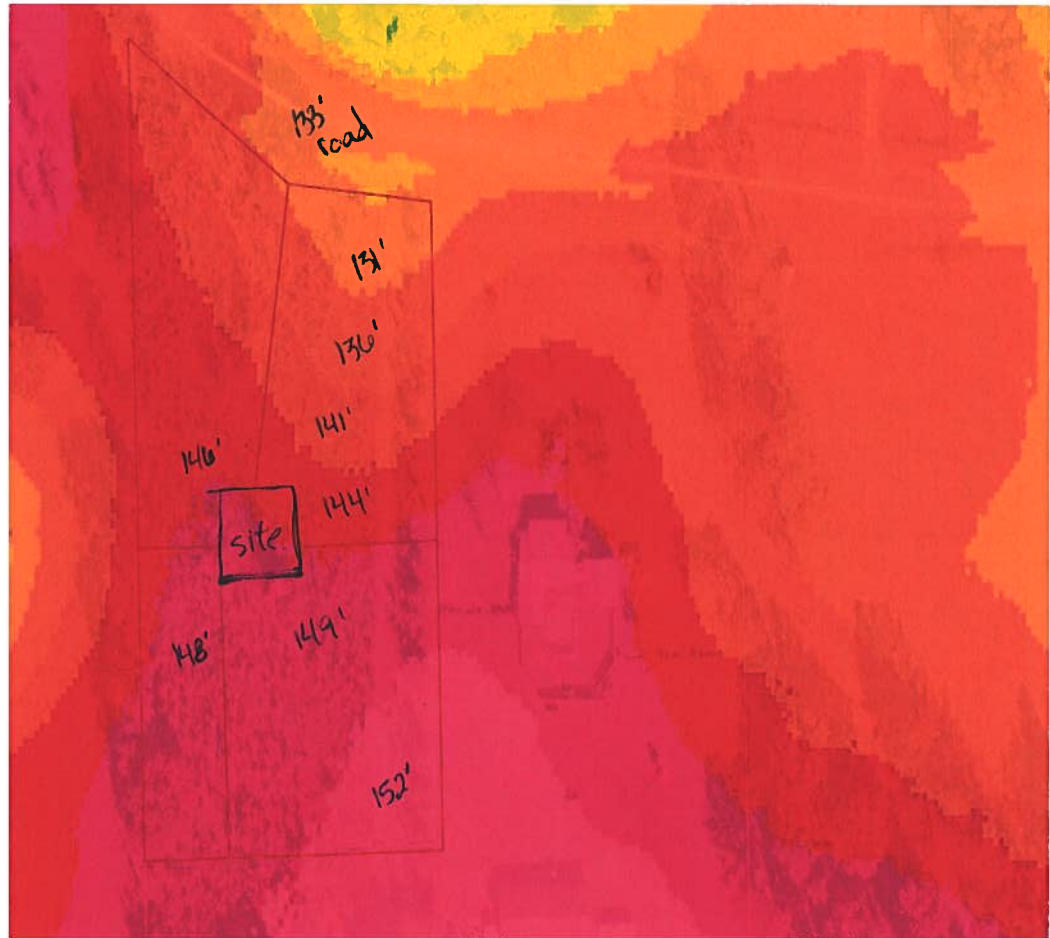
Parcels

LidarElevations



Columbia County, FLA - Building & Zoning Property Map

Printed: Mon Feb 10 2020 11:48:12 GMT-0500 (Eastern Standard Time)



Parcel Information

Parcel No: 28-3S-16-02374-081

Owner: LUNDE HAYDEN & KATELYN

Subdivision:

Lot:

Acres: 2.9862802

Deed Acres: 3 Ac

District: District 3 Bucky Nash

Future Land Uses: Residential - Low

Flood Zones:

Official Zoning Atlas: RSF-2

All data, information, and maps are provided "as is" without warranty or any representation of accuracy, timeliness of completeness. Columbia County, FL makes no warranties, express or implied, as to the use of the information obtained here. There are no implied warranties of merchantability or fitness for a particular purpose. The requester acknowledges and accepts all limitations, including the fact that the data, information, and maps are dynamic and in a constant state of maintenance, and update.

Roads
Roads
others

**PAT LYNCH
LYNCH DRILLING CORP.**

P. O. BOX 934
BRANFORD, FL 32008
(386) 935-1076 PHONE
(386) 935-1199 FAX

DATE: 12/17/2019

CUSTOMER: Hayden & Katelyn Lunde

LOCATION: 298 NW Horizon St LC, FL 32055

WE WILL CONSTRUCT A 4' WATER WELL COMPLETE WITH 4" BLACK WATER WELL STEEL CASING, IHP SUBMERSIBLE PUMP (20GPM) WITH 1 1/4 " GALVANIZED DROP PIPE, AND AN 81 GALLON CAPTIVE AIR TANK (21.9 GALLON DRAWDOWN)

WELL WILL BE COMPLETE AT THE WELL SITE. WE DO NOT INCLUDE ELECTRICAL NOR PLUMBING CONNECTIONS FROM THE WELL TO THE HOME AND/OR POWER POLE.

ANY VARIATIONS OF THE ABOVE ARE SUBJECT TO APPROVAL FROM THE CUSTOMER AND/OR CONTRACTOR PRIOR TO COMMENCEMENT OF THE INDIVIDUAL JOB.

NOT RESPONSIBLE FOR THE QUALITY OF WATER

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Therma-Tru Corporation		FL5891-R6
B. SLIDING			
C. SECTIONAL/ROLL UP	C.H.I.	2250 model	FL15012-R6
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	PGT	5400 Series SH	FL239-R26
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hardie	HardiePlank lap siding	FL13192.2
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCTURAL METAL	Ultra-Rib	26 GA	FL4595.7
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCTURAL COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.


 Verified by PDFfiller
 05/13/2019

Blake N Lunde
 12/17/2019

Contractor OR Agent Signature

Date

NOTES: _____

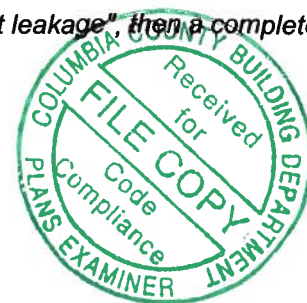
RESIDENTIAL ENERGY CONSERVATION CODE DOCUMENTATION CHECKLIST**Florida Department of Business and Professional Regulation
Simulated Performance Alternative (Performance) Method**

Applications for compliance with the 2017 Florida Building Code, Energy Conservation via the residential Simulated Performance Method shall include:

- ☐ This checklist
- ☐ A Form R405 report that documents that the Proposed Design complies with Section R405.3 of the Florida Energy Code. This form shall include a summary page indicating home address, e-ratio and the pass or fail status along with summary areas and types of components, whether the home was simulated as a worst-case orientation, name and version of the compliance software tool, name of individual completing the compliance report (one page) and an input summary checklist that can be used for field verification (usually four pages/may be greater).
- ☐ Energy Performance Level (EPL) Display Card (one page)
- ☐ HVAC system sizing and selection based on ACCA Manual S or per exceptions provided in Section R403.7
- ☐ Mandatory Requirements (five pages)

Required prior to CO for the Performance Method:

- ☐ Air Barrier and Insulation Inspection Component Criteria checklist (Table R402.4.1.1 - one page)
- ☐ A completed Envelope Leakage Test Report (usually one page)
- ☐ If Form R405 duct leakage type indicates anything other than "default leakage," then a completed Form R405 Duct Leakage Test Report (usually one page)



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: 191376 Hayden
 Street: 298 NW Horizon St.
 City, State, Zip: Lake City, FL,
 Owner: Hayden Res
 Design Location: FL, Gainesville

Builder Name: Blake Construction
 Permit Office:
 Permit Number:
 Jurisdiction:
 County: Columbia (Florida Climate Zone 2)

1. New construction or existing	New (From Plans)
2. Single family or multiple family	Single-family
3. Number of units, if multiple family	1
4. Number of Bedrooms	3
5. Is this a worst case?	No
6. Conditioned floor area above grade (ft²)	1879
Conditioned floor area below grade (ft²)	0
7. Windows (429.5 sqft.)	Description Area
a. U-Factor:	Dbl, U=0.30 429.50 ft²
SHGC:	SHGC=0.20
b. U-Factor:	N/A ft²
SHGC:	
c. U-Factor:	N/A ft²
SHGC:	
d. U-Factor:	N/A ft²
SHGC:	
Area Weighted Average Overhang Depth:	11.069 ft.
Area Weighted Average SHGC:	0.200
8. Floor Types (1879.0 sqft.)	Insulation Area
a. Slab-On-Grade Edge Insulation	R=0.0 1879.00 ft²
b. N/A	R= ft²
c. N/A	R= ft²

9. Wall Types (2294.6 sqft.)	Insulation Area
a. Frame - Wood, Exterior	R=13.0 2120.60 ft²
b. Frame - Wood, Adjacent	R=13.0 174.00 ft²
c. N/A	R= ft²
d. N/A	R= ft²
10. Ceiling Types (1967.0 sqft.)	Insulation Area
a. Roof Deck (Unvented)	R=22.0 1967.00 ft²
b. N/A	R= ft²
c. N/A	R= ft²
11. Ducts	R ft²
a. Sup: Attic, Ret: Attic, AH: Main	6 375.8
12. Cooling systems	kBtu/hr Efficiency
a. Central Unit	41.0 SEER:17.00
13. Heating systems	kBtu/hr Efficiency
a. Electric Heat Pump	31.0 HSPF:9.00
14. Hot water systems	Cap: 50 gallons
a. Electric	EF: 0.950
b. Conservation features	
None	
15. Credits	Pstat

Glass/Floor Area: 0.229

Total Proposed Modified Loads: 58.04

Total Baseline Loads: 58.48

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Evan Beamsley
 DATE: 2020-01-13

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
 DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____
 DATE: _____

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 7.00 ACH50 (R402.4.1.2).

INPUT SUMMARY CHECKLIST REPORT

PROJECT

Title:	191376 Hayden	Bedrooms:	3	Address Type:	Street Address
Building Type:	User	Conditioned Area:	1879	Lot #	
Owner Name:	Hayden Res	Total Stories:	1	Block/Subdivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:	Blake Construction	Rotate Angle:	180	Street:	298 NW Horizon St.
Permit Office:		Cross Ventilation:		County:	Columbia
Jurisdiction:		Whole House Fan:		City, State, Zip:	Lake City , FL ,
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

✓	Design Location	TMY Site	Design Temp		Int Design Temp		Heating	Design	Daily Temp
			97.5 %	2.5 %	Winter	Summer	Degree Days	Moisture	Range
_____	FL, Gainesville	FL_GAINESVILLE_REGI	32	92	70	75	1305.5	51	Medium

BLOCKS

Number	Name	Area	Volume
1	Block1	1879	18564.5

SPACES

Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Infil ID	Finished	Cooled	Heated
1	Main	1879	18564.5	Yes	6	3	1	Yes	Yes	Yes

FLOORS

✓	#	Floor Type	Space	Perimeter	R-Value	Area		Tile	Wood	Carpet
_____	1	Slab-On-Grade Edge Insulatio	Main	237 ft	0	1879 ft²	----	0.3	0.3	0.4

ROOF

✓	#	Type	Materials	Roof Area	Gable Area	Roof Color	Rad Barr	Solar Absor.	SA Tested	Emitt	Emitt Tested	Deck Insul.	Pitch (deg)
_____	1	Gable or shed	Composition shingles	2259 ft²	626 ft²	Dark	N	0.92	No	0.9	No	22	33.7

ATTIC

✓	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
_____	1	Full attic	Unvented	0	1879 ft²	N	N

CEILING

✓	#	Ceiling Type	Space	R-Value	Ins Type	Area	Framing Frac	Truss Type
_____	1	Under Attic (Unvented)	Main	0	Blown	1967 ft²	0.01	Wood

INPUT SUMMARY CHECKLIST REPORT

WALLS

✓ #	Omt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	In	Height Ft	In	Area	Sheathing R-Value	Framing Fraction	Solar Absor.	Below Grade%
___ 1	N=>S	Exterior	Frame - Wood	Main	13	8		9		72.0 ft²		0.23	0.75	0
___ 2	N=>S	Exterior	Frame - Wood	Main	13	11	8	10		116.7 ft²		0.23	0.75	0
___ 3	E=>W	Exterior	Frame - Wood	Main	13	7	0	10		70.0 ft²		0.23	0.75	0
___ 4	N=>S	Exterior	Frame - Wood	Main	13	17	8	11	0	194.3 ft²		0.23	0.75	0
___ 5	W=>E	Exterior	Frame - Wood	Main	13	16	0	10		160.0 ft²		0.23	0.75	0
___ 6	N=>S	Exterior	Frame - Wood	Main	13	16	8	10		166.7 ft²		0.23	0.75	0
___ 7	E=>W	Exterior	Frame - Wood	Main	13	30	3	10		302.5 ft²		0.23	0.75	0
___ 8	E=>W	Exterior	Frame - Wood	Main	13	23	0	9		207.0 ft²		0.23	0.75	0
___ 9	S=>N	Exterior	Frame - Wood	Main	13	5	4	9		48.0 ft²		0.23	0.75	0
___ 10	E=>W	Exterior	Frame - Wood	Main	13	2	0	9		18.0 ft²		0.23	0.75	0
___ 11	S=>N	Exterior	Frame - Wood	Main	13	11	8	9		105.0 ft²		0.23	0.75	0
___ 12	W=>E	Exterior	Frame - Wood	Main	13	9	11	9		89.3 ft²		0.23	0.75	0
___ 13	S=>N	Exterior	Frame - Wood	Main	13	6		10		60.0 ft²		0.23	0.75	0
___ 14	E=>W	Exterior	Frame - Wood	Main	13	2		10		20.0 ft²		0.23	0.75	0
___ 15	S=>N	Exterior	Frame - Wood	Main	13	11	8	10		116.7 ft²		0.23	0.75	0
___ 16	W=>E	Exterior	Frame - Wood	Main	13	10	8	10		106.7 ft²		0.23	0.75	0
___ 17	S=>N	Exterior	Frame - Wood	Main	13	11	4	10		113.3 ft²		0.23	0.75	0
___ 18	W=>E	Exterior	Frame - Wood	Main	13	8	3	10		82.5 ft²		0.23	0.75	0
___ 19	S=>N	Exterior	Frame - Wood	Main	13	8		9		72.0 ft²		0.23	0.75	0
___ 20	W=>E	Garage	Frame - Wood	Main	13	19	4	9		174.0 ft²		0.23	0.75	0

DOORS

✓ #	Omt	Door Type	Space	Storms	U-Value	Width Ft	In	Height Ft	In	Area
___ 1	N=>S	Insulated	Main	None	.4	2		8		16 ft²
___ 2	N=>S	Insulated	Main	None	.4	2		8		16 ft²
___ 3	W=>E	Insulated	Main	None	.4	2		8		16 ft²
___ 4	S=>N	Insulated	Main	None	.4	1		8		8 ft²
___ 5	W=>E	Insulated	Main	None	.4	2		8		16 ft²
___ 6	W=>E	Insulated	Main	None	.4	3		6	8	20 ft²

WINDOWS

Orientation shown is the entered orientation (=>) changed to As Built (rotated 180 degrees).

✓ #	Omt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Overhang Depth	Separation	Int Shade	Screening
___ 1	N=>S	1	Metal	Low-E Double	Yes	0.3	0.2	N	18.0 ft²	1 ft 6 in	1 ft 0 in	None	None
___ 2	N=>S	2	Metal	Low-E Double	Yes	0.3	0.2	N	36.0 ft²	1 ft 6 in	1 ft 0 in	None	None
___ 3	E=>W	3	Metal	Low-E Double	Yes	0.3	0.2	N	18.0 ft²	99 ft 0 in	1 ft 0 in	None	None
___ 4	N=>S	4	Metal	Low-E Double	Yes	0.3	0.2	N	64.0 ft²	13 ft 6 in	2 ft 0 in	None	None
___ 5	W=>E	5	Metal	Low-E Double	Yes	0.3	0.2	N	32.0 ft²	21 ft 2 in	1 ft 0 in	None	None
___ 6	N=>S	6	Metal	Low-E Double	Yes	0.3	0.2	N	54.0 ft²	1 ft 6 in	6 ft 0 in	None	None
___ 7	E=>W	7	Metal	Low-E Double	Yes	0.3	0.2	N	24.0 ft²	1 ft 6 in	10 ft 0 in	None	None
___ 8	E=>W	8	Metal	Low-E Double	Yes	0.3	0.2	N	36.0 ft²	1 ft 6 in	10 ft 0 in	None	None
___ 9	S=>N	11	Metal	Low-E Double	Yes	0.3	0.2	N	36.0 ft²	1 ft 6 in	0 ft 0 in	None	None

INPUT SUMMARY CHECKLIST REPORT

WINDOWS

Orientation shown is the entered orientation (=>) changed to As Built (rotated 180 degrees).

✓	#	Ornt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Overhang Depth	Separation	Int Shade	Screening
✓	10	S=>N	13	Metal	Low-E Double	Yes	0.3	0.2	N	24.0 ft²	11 ft 7 in	1 ft 0 in	None	None
✓	11	S=>N	15	Metal	Low-E Double	Yes	0.3	0.2	N	36.0 ft²	9 ft 0 in	1 ft 0 in	None	None
✓	12	W=>E	16	Metal	Low-E Double	Yes	0.3	0.2	N	24.0 ft²	11 ft 4 in	1 ft 0 in	None	None
✓	13	S=>N	17	Metal	Low-E Double	Yes	0.3	0.2	N	12.5 ft²	18 ft 3 in	1 ft 0 in	None	None
✓	14	S=>N	19	Metal	Low-E Double	Yes	0.3	0.2	N	15.0 ft²	1 ft 6 in	1 ft 0 in	None	None

GARAGE

✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
✓	1	573.6 ft²	573.6 ft²	32 ft	9 ft	1

INFILTRATION

#	Scope	Method	SLA	CFM 50	ELA	EqLA	ACH	ACH 50
1	Wholehouse	Proposed ACH(50)	.000439	2165.9	118.9	223.61	.1799	7

HEATING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Block	Ducts
✓	1	Electric Heat Pump/	None	HSPF:9	41 kBtu/hr	1	sys#1

COOLING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Block	Ducts
✓	1	Central Unit/	None	SEER: 17	41 kBtu/hr	1230 cfm	0.75	1	sys#1

HOT WATER SYSTEM

✓	#	System Type	SubType	Location	EF	Cap	Use	SetPnt	Conservation
✓	1	Electric	None	Main	0.95	50 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
✓	None	None			ft²		

DUCTS

✓	#	--- Supply --- Location	R-Value	Area	--- Return --- Location	Area	Leakage Type	Air Handler	CFM 25 TOT	CFM25 OUT	QN	RLF	HVAC # Heat	Cool
✓	1	Attic	6	375.8 ft	Attic	93.95 ft	Default Leakage	Main	(Default)	(Default)			1	1

INPUT SUMMARY CHECKLIST REPORT

TEMPERATURES													
Programable Thermostat: Y		Ceiling Fans:											
Cooling	☐ Jan	☐ Feb	☐ Mar	☐ Apr	☐ May	☒ Jun	☒ Jul	☒ Aug	☒ Sep	☐ Oct	☐ Nov	☐ Dec	
Heating	☒ Jan	☒ Feb	☒ Mar	☐ Apr	☐ May	☐ Jun	☐ Jul	☐ Aug	☐ Sep	☐ Oct	☒ Nov	☒ Dec	
Venting	☐ Jan	☐ Feb	☒ Mar	☒ Apr	☐ May	☐ Jun	☐ Jul	☐ Aug	☐ Sep	☒ Oct	☒ Nov	☒ Dec	
Thermostat Schedule: HERS 2006 Reference													
Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
MASS													
Mass Type		Area		Thickness		Furniture Fraction		Space					
Default(8 lbs/sq.ft.		0 ft²		0 ft		0.3		Main					

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD**ESTIMATED ENERGY PERFORMANCE INDEX* = 99****The lower the Energy Performance Index, the more efficient the home.**

1. New home or, addition	1. <u>New (From Plans)</u>	12. Ducts, location & insulation level	
2. Single-family or multiple-family	2. <u>Single-family</u>	a) Supply ducts	R <u>6.0</u>
3. No. of units (if multiple-family)	3. <u>1</u>	b) Return ducts	R <u>6.0</u>
4. Number of bedrooms	4. <u>3</u>	c) AHU location	Main
5. Is this a worst case? (yes/no)	5. <u>No</u>	13. Cooling system:	Capacity <u>41.0</u>
6. Conditioned floor area (sq. ft.)	6. <u>1879</u>	a) Split system	SEER <u> </u>
7. Windows, type and area		b) Single package	SEER <u> </u>
a) U-factor:(weighted average)	7a. <u>0.300</u>	c) Ground/water source	SEER/COP <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	7b. <u>0.200</u>	d) Room unit/PTAC	EER <u> </u>
c) Area	7c. <u>429.5</u>	e) Other	<u>17.0</u>
8. Skylights		14. Heating system:	Capacity <u>41.0</u>
a) U-factor:(weighted average)	8a. <u>NA</u>	a) Split system heat pump	HSPF <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	8b. <u>NA</u>	b) Single package heat pump	HSPF <u> </u>
9. Floor type, insulation level:		c) Electric resistance	COP <u> </u>
a) Slab-on-grade (R-value)	9a. <u>0.0</u>	d) Gas furnace, natural gas	AFUE <u> </u>
b) Wood, raised (R-value)	9b. <u> </u>	e) Gas furnace, LPG	AFUE <u> </u>
c) Concrete, raised (R-value)	9c. <u> </u>	f) Other	<u>9.00</u>
10. Wall type and insulation:		15. Water heating system	
A. Exterior:		a) Electric resistance	EF <u>0.95</u>
1. Wood frame (Insulation R-value)	10A1. <u>13.0</u>	b) Gas fired, natural gas	EF <u> </u>
2. Masonry (Insulation R-value)	10A2. <u> </u>	c) Gas fired, LPG	EF <u> </u>
B. Adjacent:		d) Solar system with tank	EF <u> </u>
1. Wood frame (Insulation R-value)	10B1. <u>13.0</u>	e) Dedicated heat pump with tank	EF <u> </u>
2. Masonry (Insulation R-value)	10B2. <u> </u>	f) Heat recovery unit	HeatRec% <u> </u>
11. Ceiling type and insulation level		g) Other	
a) Under attic	11a. <u>0.0</u>	16. HVAC credits claimed (Performance Method)	
b) Single assembly	11b. <u> </u>	a) Ceiling fans	<u> </u>
c) Knee walls/skylight walls	11c. <u> </u>	b) Cross ventilation	<u>No</u>
d) Radiant barrier installed	11d. <u>No</u>	c) Whole house fan	<u>No</u>
		d) Multizone cooling credit	<u> </u>
		e) Multizone heating credit	<u> </u>
		f) Programmable thermostat	<u>Yes</u>

*Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

I certify that this home has complied with the Florida Building Code, Energy Conservation, through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL display card will be completed based on installed code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: 298 NW Horizon St.City/FL Zip: Lake City, FL

Florida Building Code, Energy Conservation, 6th Edition (2017)

Mandatory Requirements for Residential Performance, Prescriptive and ERI Methods

ADDRESS: 298 NW Horizon St.
Lake City, FL,

Permit Number:

MANDATORY REQUIREMENTS See individual code sections for full details.

SECTION R401 GENERAL

- ☐ **R401.3 Energy Performance Level (EPL) display card (Mandatory).** The building official shall require that an energy performance level (EPL) display card be completed and certified by the builder to be accurate and correct before final approval of the building for occupancy. Florida law (Section 553.9085, Florida Statutes) requires the EPL display card to be included as an addendum to each sales contract for both presold and nonpresold residential buildings. The EPL display card contains information indicating the energy performance level and efficiencies of components installed in a dwelling unit. The building official shall verify that the EPL display card completed and signed by the builder accurately reflects the plans and specifications submitted to demonstrate code compliance for the building. A copy of the EPL display card can be found in Appendix RD.

- ☐ **R402.4 Air leakage (Mandatory).** The building thermal envelope shall be constructed to limit air leakage in accordance with the requirements of Sections R402.4.1 through R402.4.5.

Exception: Dwelling units of R-2 Occupancies and multiple attached single family dwellings shall be permitted to comply with Section C402.5.

- ☐ **R402.4.1 Building thermal envelope.** The building thermal envelope shall comply with Sections R402.4.1.1 and R402.4.1.2. The sealing methods between dissimilar materials shall allow for differential expansion and contraction.

- ☐ **R402.4.1.1 Installation.** The components of the building thermal envelope as listed in Table R402.4.1.1 shall be installed in accordance with the manufacturer's instructions and the criteria listed in Table R402.4.1.1, as applicable to the method of construction. Where required by the code official, an approved third party shall inspect all components and verify compliance.

- ☐ **R402.4.1.2 Testing.** The building or dwelling unit shall be tested and verified as having an air leakage rate not exceeding seven air changes per hour in Climate Zones 1 and 2, and three air changes per hour in Climate Zones 3 through 8. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or (7), Florida Statutes, or individuals licensed as set forth in Section 489.105(3)(f), (g) or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the code official. Testing shall be performed at any time after creation of all penetrations of the building thermal envelope.

Exception: Testing is not required for additions, alterations, renovations, or repairs, of the building thermal envelope of existing buildings in which the new construction is less than 85 percent of the building thermal envelope.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, backdraft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

- ☐ **R402.4.2 Fireplaces.** New wood-burning fireplaces shall have tight-fitting flue dampers or doors, and outdoor combustion air. Where using tight-fitting doors on factory-built fireplaces listed and labeled in accordance with UL 127, the doors shall be tested and listed for the fireplace. Where using tight-fitting doors on masonry fireplaces, the doors shall be listed and labeled in accordance with UL 907.

- ☐ **R402.4.3 Fenestration air leakage.** Windows, skylights and sliding glass doors shall have an air infiltration rate of no more than 0.3 cfm per square foot (1.5 L/s/m²), and swinging doors no more than 0.5 cfm per square foot (2.6 L/s/m²), when tested according to NFRC 400 or AAMA/WDMA/CSA 101/I.S.2/A440 by an accredited, independent laboratory and listed and labeled by the manufacturer.

Exception: Site-built windows, skylights and doors.

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R402.4.4 Rooms containing fuel-burning appliances.** In Climate Zones 3 through 8, where open combustion air ducts provide combustion air to open combustion fuel burning appliances, the appliances and combustion air opening shall be located outside the building thermal envelope or enclosed in a room, isolated from inside the thermal envelope. Such rooms shall be sealed and insulated in accordance with the envelope requirements of Table R402.1.2, where the walls, floors and ceilings shall meet not less than the basement wall R-value requirement. The door into the room shall be fully gasketed and any water lines and ducts in the room insulated in accordance with Section R403. The combustion air duct shall be insulated where it passes through conditioned space to a minimum of R-8.

Exceptions:

1. Direct vent appliances with both intake and exhaust pipes installed continuous to the outside.
2. Fireplaces and stoves complying with Section R402.4.2 and Section R1006 of the Florida Building Code, Residential.

- ☐ **R402.4.5 Recessed lighting.** Recessed luminaires installed in the building thermal envelope shall be sealed to limit air leakage between conditioned and unconditioned spaces. All recessed luminaires shall be IC-rated and labeled as having an air leakage rate not more than 2.0 cfm (0.944 L/s) when tested in accordance with ASTM E283 at a 1.57 psf (75 Pa) pressure differential. All recessed luminaires shall be sealed with a gasket or caulk between the housing and the interior wall or ceiling covering.

SECTION R403 SYSTEMS

R403.1 Controls.

- ☐ **R403.1.1 Thermostat provision (Mandatory).** At least one thermostat shall be provided for each separate heating and cooling system.

- ☐ **R403.1.3 Heat pump supplementary heat (Mandatory).** Heat pumps having supplementary electric-resistance heat shall have controls that, except during defrost, prevent supplemental heat operation when the heat pump compressor can meet the heating load.

- ☐ **R403.3.2 Sealing (Mandatory)** All ducts, air handlers, filter boxes and building cavities that form the primary air containment passageways for air distribution systems shall be considered ducts or plenum chambers, shall be constructed and sealed in accordance with Section C403.2.9.2 of the Commercial Provisions of this code and shall be shown to meet duct tightness criteria below.

Duct tightness shall be verified by testing in accordance with ANSI/RESNET/ICC 380 by either individuals as defined in Section 553.993(5) or (7), Florida Statutes, or individuals licensed as set forth in Section 489.105(3)(f), (g) or (i), Florida Statutes, to be "substantially leak free" in accordance with Section R403.3.3.

- ☐ **R403.3.2.1 Sealed air handler.** Air handlers shall have a manufacturer's designation for an air leakage of no more than 2 percent of the design airflow rate when tested in accordance with ASHRAE 193.

- ☐ **R403.3.3 Duct testing (Mandatory).** Ducts shall be pressure tested to determine air leakage by one of the following methods:

1. Rough-in test: Total leakage shall be measured with a pressure differential of 0.1 inch w.g. (25 Pa) across the system, including the main air handler enclosure if installed at the time of the test. All registers shall be taped or otherwise sealed during the test.
2. Postconstruction test: Total leakage shall be measured with a pressure differential of 0.1 inch w.g. (25 Pa) across the entire system, including the manufacturer's air handler enclosure. Registers shall be taped or otherwise sealed during the test.

Exceptions:

1. A duct air leakage test shall not be required where the ducts and air handlers are located entirely within the building thermal envelope.
2. Duct testing is not mandatory for buildings complying by Section 405 of this code.

A written report of the results of the test shall be signed by the party conducting the test and provided to the code official.

- ☐ **R403.3.5 Building cavities (Mandatory).** Building framing cavities shall not be used as ducts or plenums.

- ☐ **R403.4 Mechanical system piping insulation (Mandatory).** Mechanical system piping capable of carrying fluids above 105°F (41°C) or below 55°F (13°C) shall be insulated to a minimum of R-3.

- ☐ **R403.4.1 Protection of piping insulation.** Piping insulation exposed to weather shall be protected from damage, including that caused by sunlight, moisture, equipment maintenance and wind, and shall provide shielding from solar radiation that can cause degradation of the material. Adhesive tape shall not be permitted.

- ☐ **R403.5.1 Heated water circulation and temperature maintenance systems (Mandatory)** Heated water circulation systems shall be in accordance with Section R403.5.1.1. Heat trace temperature maintenance systems shall be in accordance with Section R403.5.1.2. Automatic controls, temperature sensors and pumps shall be accessible. Manual controls shall be readily accessible.

- ☐ **R403.5.1.1 Circulation systems.** Heated water circulation systems shall be provided with a circulation pump. The system return pipe shall be a dedicated return pipe or a cold water supply pipe. Gravity and thermosiphon circulation systems shall be prohibited. Controls for circulating hot water system pumps shall start the pump based on the identification of a demand for hot water within the occupancy. The controls shall automatically turn off the pump when the water in the circulation loop is at the desired temperature and when there is no demand for hot water.

- ☐ **R403.5.1.2 Heat trace systems.** Electric heat trace systems shall comply with IEEE 515.1 or UL 515. Controls for such systems shall automatically adjust the energy input to the heat tracing to maintain the desired water temperature in the piping in accordance with the times when heated water is used in the occupancy.

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R403.5.5 Heat traps (Mandatory).** Storage water heaters not equipped with integral heat traps and having vertical pipe risers shall have heat traps installed on both the inlets and outlets. External heat traps shall consist of either a commercially available heat trap or a downward and upward bend of at least 3 ½ inches (89 mm) in the hot water distribution line and cold water line located as close as possible to the storage tank.
- R403.5.6 Water heater efficiencies (Mandatory).**
- ☐ **R403.5.6.1.1 Automatic controls.** Service water-heating systems shall be equipped with automatic temperature controls capable of adjustment from the lowest to the highest acceptable temperature settings for the intended use. The minimum temperature setting range shall be from 100°F to 140°F (38°C to 60°C).
- ☐ **R403.5.6.1.2 Shut down.** A separate switch or a clearly marked circuit breaker shall be provided to permit the power supplied to electric service systems to be turned off. A separate valve shall be provided to permit the energy supplied to the main burner(s) of combustion types of service water-heating systems to be turned off.
- ☐ **R403.5.6.2 Water-heating equipment.** Water-heating equipment installed in residential units shall meet the minimum efficiencies of Table C404.2 in Chapter 4 of the Florida Building Code, Energy Conservation, Commercial Provisions, for the type of equipment installed. Equipment used to provide heating functions as part of a combination system shall satisfy all stated requirements for the appropriate water-heating category. Solar water heaters shall meet the criteria of Section R403.5.6.2.1.
- ☐ **R403.5.6.2.1 Solar water-heating systems.** Solar systems for domestic hot water production are rated by the annual solar energy factor of the system. The solar energy factor of a system shall be determined from the Florida Solar Energy Center Directory of Certified Solar Systems. Solar collectors shall be tested in accordance with ISO Standard 9806, Test Methods for Solar Collectors, and SRCC Standard TM-1, Solar Domestic Hot Water System and Component Test Protocol. Collectors in installed solar water-heating systems should meet the following criteria:
1. Be installed with a tilt angle between 10 degrees and 40 degrees of the horizontal; and
 2. Be installed at an orientation within 45 degrees of true south.
- ☐ **R403.6 Mechanical ventilation (Mandatory).** The building shall be provided with ventilation that meets the requirements of the Florida Building Code, Residential, or Florida Building Code, Mechanical, as applicable, or with other approved means of ventilation including: Natural, Infiltration or Mechanical means. Outdoor air intakes and exhausts shall have automatic or gravity dampers that close when the ventilation system is not operating.
- ☐ **R403.6.1 Whole-house mechanical ventilation system fan efficacy.** When installed to function as a whole-house mechanical ventilation system, fans shall meet the efficacy requirements of Table R403.6.1.
- Exception:** Where whole-house mechanical ventilation fans are integral to tested and listed HVAC equipment, they shall be powered by an electronically commutated motor.
- ☐ **R403.6.2 Ventilation air.** Residential buildings designed to be operated at a positive indoor pressure or for mechanical ventilation shall meet the following criteria:
1. The design air change per hour minimums for residential buildings in ASHRAE 62.2, Ventilation for Acceptable Indoor Air Quality, shall be the maximum rates allowed for residential applications.
 2. No ventilation or air-conditioning system make-up air shall be provided to conditioned space from attics, crawlspaces, attached enclosed garages or outdoor spaces adjacent to swimming pools or spas.
 3. If ventilation air is drawn from enclosed space(s), then the walls of the space(s) from which air is drawn shall be insulated to a minimum of R-11 and the ceiling shall be insulated to a minimum of R-19, space permitting, or R-10 otherwise.
- R403.7 Heating and cooling equipment (Mandatory).**
- ☐ **R403.7.1 Equipment sizing.** Heating and cooling equipment shall be sized in accordance with ACCA Manual S based on the equipment loads calculated in accordance with ACCA Manual J or other approved heating and cooling calculation methodologies, based on building loads for the directional orientation of the building. The manufacturer and model number of the outdoor and indoor units (if split system) shall be submitted along with the sensible and total cooling capacities at the design conditions described in Section R302.1. This Code does not allow designer safety factors, provisions for future expansion or other factors that affect equipment sizing. System sizing calculations shall not include loads created by local intermittent mechanical ventilation such as standard kitchen and bathroom exhaust systems. New or replacement heating and cooling equipment shall have an efficiency rating equal to or greater than the minimum required by federal law for the geographic location where the equipment is installed.

**TABLE R403.6.1
WHOLE-HOUSE MECHANICAL VENTILATION SYSTEM FAN EFFICACY**

FAN LOCATION	AIRFLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY ^a (CFM/WATT)	AIRFLOW RATE MAXIMUM (CFM)
Range hoods	Any	2.8 cfm/watt	Any
In-line fan	Any	2.8 cfm/watt	Any
Bathroom, utility room	10	1.4 cfm/watt	<90
Bathroom, utility room	90	2.8 cfm/watt	Any

For SI: 1 cfm = 28.3 L/min.

a. When tested in accordance with HVI Standard 916

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R403.7.1.1 Cooling equipment capacity.** Cooling only equipment shall be selected so that its total capacity is not less than the calculated total load but not more than 1.15 times greater than the total load calculated according to the procedure selected in Section 403.7, or the closest available size provided by the manufacturer's product lines. The corresponding latent capacity of the equipment shall not be less than the calculated latent load.

The published value for AHRI total capacity is a nominal, rating-test value and shall not be used for equipment sizing. Manufacturer's expanded performance data shall be used to select cooling-only equipment. This selection shall be based on the outdoor design dry-bulb temperature for the load calculation (or entering water temperature for water-source equipment), the blower CFM provided by the expanded performance data, the design value for entering wet-bulb temperature and the design value for entering dry-bulb temperature.

Design values for entering wet-bulb and dry-bulb temperatures shall be for the indoor dry bulb and relative humidity used for the load calculation and shall be adjusted for return side gains if the return duct(s) is installed in an unconditioned space.

Exceptions:

1. Attached single- and multiple-family residential equipment sizing may be selected so that its cooling capacity is less than the calculated total sensible load but not less than 80 percent of that load.
2. When signed and sealed by a Florida-registered engineer, in attached single- and multiple-family units, the capacity of equipment may be sized in accordance with good design practice.

R403.7.1.2 Heating equipment capacity.

- ☐ **R403.7.1.2.1 Heat pumps.** Heat pump sizing shall be based on the cooling requirements as calculated according to Section R403.7.1.1, and the heat pump total cooling capacity shall not be more than 1.15 times greater than the design cooling load even if the design heating load is 1.15 times greater than the design cooling load.

- ☐ **R403.7.1.2.2 Electric resistance furnaces.** Electric resistance furnaces shall be sized within 4 kW of the design requirements calculated according to the procedure selected in Section R403.7.1.

- ☐ **R403.7.1.2.3 Fossil fuel heating equipment.** The capacity of fossil fuel heating equipment with natural draft atmospheric burners shall not be less than the design load calculated in accordance with Section R403.7.1.

- ☐ **R403.7.1.3 Extra capacity required for special occasions.** Residences requiring excess cooling or heating equipment capacity on an intermittent basis, such as anticipated additional loads caused by major entertainment events, shall have equipment sized or controlled to prevent continuous space cooling or heating within that space by one or more of the following options:

1. A separate cooling or heating system is utilized to provide cooling or heating to the major entertainment areas.
2. A variable capacity system sized for optimum performance during base load periods is utilized.

- ☐ **R403.8 Systems serving multiple dwelling units (Mandatory).** Systems serving multiple dwelling units shall comply with Sections C403 and C404 of the IECC—Commercial Provisions in lieu of Section R403.

- ☐ **R403.9 Snow melt and ice system controls (Mandatory)** Snow- and ice-melting systems, supplied through energy service to the building, shall include automatic controls capable of shutting off the system when the pavement temperature is above 50°F (10°C), and no precipitation is falling and an automatic or manual control that will allow shutoff when the outdoor temperature is above 40°F (4.8°C).

- ☐ **R403.10 Pools and permanent spa energy consumption (Mandatory).** The energy consumption of pools and permanent spas shall be in accordance with Sections R403.10.1 through R403.10.5.

- ☐ **R403.10.1 Heaters.** The electric power to heaters shall be controlled by a readily accessible on-off switch that is an integral part of the heater mounted on the exterior of the heater, or external to and within 3 feet (914 mm) of the heater. Operation of such switch shall not change the setting of the heater thermostat. Such switches shall be in addition to a circuit breaker for the power to the heater. Gas-fired heaters shall not be equipped with continuously burning ignition pilots.

- ☐ **R403.10.2 Time switches.** Time switches or other control methods that can automatically turn off and on according to a preset schedule shall be installed for heaters and pump motors. Heaters and pump motors that have built-in time switches shall be in compliance with this section.

Exceptions:

1. Where public health standards require 24-hour pump operation.
2. Pumps that operate solar- and waste-heat-recovery pool heating systems.
3. Where pumps are powered exclusively from on-site renewable generation.

- ☐ **R403.10.3 Covers.** Outdoor heated swimming pools and outdoor permanent spas shall be equipped with a vapor-retardant cover on or at the water surface or a liquid cover or other means proven to reduce heat loss.

Exception: Where more than 70 percent of the energy for heating, computed over an operation season, is from site-recovered energy, such as from a heat pump or solar energy source, covers or other vapor-retardant means shall not be required.

- ☐ **R403.10.4 Gas- and oil-fired pool and spa heaters.** All gas- and oil-fired pool and spa heaters shall have a minimum thermal efficiency of 82 percent for heaters manufactured on or after April 16, 2013, when tested in accordance with ANSI Z 21.56. Pool heaters fired by natural or LP gas shall not have continuously burning pilot lights.

- ☐ **R403.10.5 Heat pump pool heaters.** Heat pump pool heaters shall have a minimum COP of 4.0 when tested in accordance with AHRI 1160, Table 2, Standard Rating Conditions-Low Air Temperature. A test report from an independent laboratory is required to verify procedure compliance. Geothermal swimming pool heat pumps are not required to meet this standard.
- ☐ **R403.11 Portable spas (Mandatory)** The energy consumption of electric-powered portable spas shall be controlled by the requirements of APSP-14.

SECTION R404

ELECTRICAL POWER AND LIGHTING SYSTEMS

- ☐ **R404.1 Lighting equipment (Mandatory).** Not less than 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps or not less than 75 percent of the permanently installed lighting fixtures shall contain only high-efficacy lamps.
Exception: Low-voltage lighting.
- R404.1.1 Lighting equipment (Mandatory)** Fuel gas lighting systems shall not have continuously burning pilot lights.

2017 - AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA

TABLE 402.4.1.1
AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA

Project Name: 191376 Hayden Street: 298 NW Horizon St. City, State, Zip: Lake City , FL , Owner: Hayden Res Design Location: FL, Gainesville			Builder Name: Blake Construction Permit Office: Permit Number: Jurisdiction:	CHECK
COMPONENT	AIR BARRIER CRITERIA	INSULATION INSTALLATION CRITERIA		
General requirements	A continuous air barrier shall be installed in the building envelope. The exterior thermal envelope contains a continuous air barrier. Breaks or joints in the air barrier shall be sealed.	Air-permeable insulation shall not be used as a sealing material.		
Ceiling/attic	The air barrier in any dropped ceiling/soffit shall be aligned with the insulation and any gaps in the air barrier shall be sealed. Access openings, drop down stairs or knee wall doors to unconditioned attic spaces shall be sealed.	The insulation in any dropped ceiling/soffit shall be aligned with the air barrier.		
Walls	The junction of the foundation and sill plate shall be sealed. The junction of the top plate and the top of exterior walls shall be sealed. Knee walls shall be sealed.	Cavities within corners and headers of frame walls shall be insulated by completely filling the cavity with a material having a thermal resistance of R-3 per inch minimum. Exterior thermal envelope insulation for framed walls shall be installed in substantial contact and continuous alignment with the air barrier.		
Windows, skylights and doors	The space between window/door jambs and framing, and skylights and framing shall be sealed.			
Rim joists	Rim joists shall include the air barrier.	Rim joists shall be insulated.		
Floors (including above-garage and cantilevered floors)	The air barrier shall be installed at any exposed edge of insulation.	Floor framing cavity insulation shall be installed to maintain permanent contact with the underside of subfloor decking, or floor framing cavity insulation shall be permitted to be in contact with the top side of sheathing, or continuous insulation installed on the underside of floor framing and extends from the bottom to the top of all perimeter floor framing members.		
Crawl space walls	Exposed earth in unvented crawl spaces shall be covered with a Class I vapor retarder with overlapping joints taped.	Where provided instead of floor insulation, insulation shall be permanently attached to the crawlspace		
Shafts, penetrations	Duct shafts, utility penetrations, and flue shafts opening to exterior or unconditioned space shall be sealed.			
Narrow cavities		Batts in narrow cavities shall be cut to fit, or narrow cavities shall be filled by insulation that on installation readily conforms to the available cavity spaces.		
Garage separation	Air sealing shall be provided between the garage and conditioned spaces.			
Recessed lighting	Recessed light fixtures installed in the building thermal envelope shall be sealed to the drywall.	Recessed light fixtures installed in the building thermal envelope shall be air tight and IC rated.		
Plumbing and wiring		Batt insulation shall be cut neatly to fit around wiring and plumbing in exterior walls, or insulation that on installation readily conforms to available space shall <u>extend behind piping and wiring.</u>		
Shower/tub on exterior wall	The air barrier installed at exterior walls adjacent to showers and tubs shall separate them from the showers and tubs.	Exterior walls adjacent to showers and tubs shall be insulated.		
Electrical/phone box on exterior walls	The air barrier shall be installed behind electrical or communication boxes or air-sealed boxes shall be installed.			
HVAC register boots	HVAC register boots that penetrate building thermal envelope shall be sealed to the sub-floor or drywall.			
Concealed sprinklers	When required to be sealed, concealed fire sprinklers shall only be sealed in a manner that is recommended by the manufacturer. Caulking or other adhesive sealants shall not be used to fill voids <u>between fire sprinkler cover plates and walls or ceilings.</u>			

a. In addition, inspection of log walls shall be in accordance with the provisions of ICC-400.

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance

2017 Florida Building Code, Energy Conservation, 6th Edition

Jurisdiction:

Permit #:

Job Information

Builder: Blake Construction

Community:

Lot: NA

Address: 298 NW Horizon St.

City: Lake City

State: FL

Zip:

Air Leakage Test Results *Passing results must meet either the Performance, Prescriptive, or ERI Method*

☐ **PRESCRIPTIVE METHOD**-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding 7 air changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Climate Zones 1 and 2.

☐ **PERFORMANCE or ERI METHOD**-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding the selected ACH(50) value, as shown on Form R405-2017 (Performance) or R406-2017 (ERI), section labeled as infiltration, sub-section ACH50.
ACH(50) specified on Form R405-2017-Energy Calc (Performance) or R406-2017 (ERI): 7.000

$$\frac{\text{CFM}(50)}{\text{Building Volume}} \times 60 \div \frac{18565}{\text{ACH}(50)} = \text{ACH}(50)$$

☐ **PASS**

☐ When ACH(50) is less than 3, Mechanical Ventilation installation must be verified by building department.

Method for calculating building volume:

- ☐ Retrieved from architectural plans
- ☒ Code software calculated
- ☐ Field measured and calculated

R402.4.1.2 Testing. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 Pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or (7), *Florida Statutes*, or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the *code official*. Testing shall be performed at any time after creation of all penetrations of the *building thermal envelope*.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, back draft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

Testing Company

Company Name: _____ Phone: _____

I hereby verify that the above Air Leakage results are in accordance with the 2017 6th Edition Florida Building Code Energy Conservation requirements according to the compliance method selected above.

Signature of Tester: _____ Date of Test: _____

Printed Name of Tester: _____

License/Certification #: _____ Issuing Authority: _____

Residential System Sizing Calculation

Summary

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden

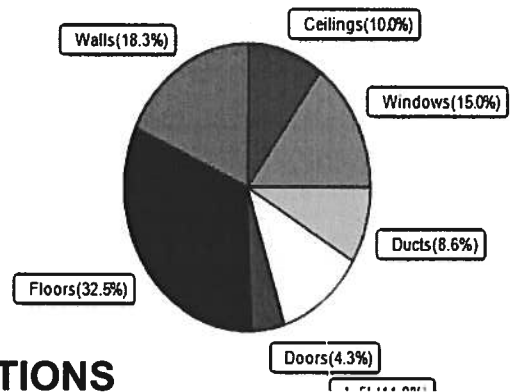
2020-01-13

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature(TMY3 99%)	30	F	Summer design temperature(TMY3 99%)	94	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	40	F	Summer temperature difference	19	F
Total heating load calculation	34392	Btuh	Total cooling load calculation	24494	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.2	41000	Sensible (SHR = 0.75)	154.5	30750
Heat Pump + Auxiliary(0.0kW)	119.2	41000	Latent	223.2	10250
			Total (Electric Heat Pump)	167.4	41000

WINTER CALCULATIONS

Winter Heating Load (for 1879 sqft)

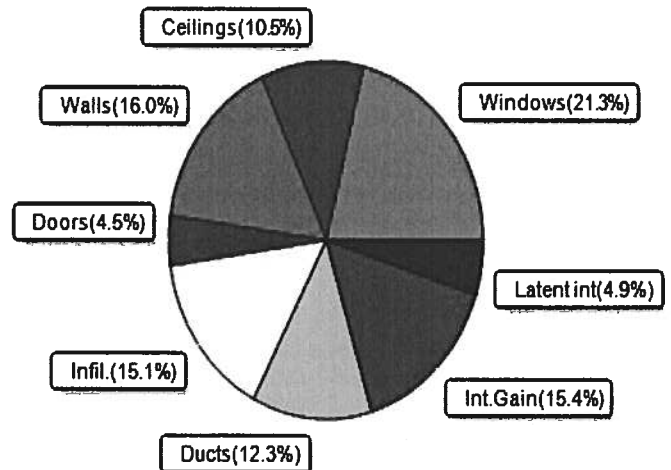
Load component		Load
Window total	430 sqft	5154 Btuh
Wall total	1773 sqft	6295 Btuh
Door total	92 sqft	1472 Btuh
Ceiling total	1967 sqft	3436 Btuh
Floor total	1879 sqft	11186 Btuh
Infiltration	89 cfm	3900 Btuh
Duct loss		2949 Btuh
Subtotal		34392 Btuh
Ventilation	0 cfm	0 Btuh
TOTAL HEAT LOSS		34392 Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1879 sqft)

Load component		Load
Window total	430 sqft	5208 Btuh
Wall total	1773 sqft	3924 Btuh
Door total	92 sqft	1104 Btuh
Ceiling total	1967 sqft	2577 Btuh
Floor total		0 Btuh
Infiltration	67 cfm	1389 Btuh
Internal gain		3780 Btuh
Duct gain		1920 Btuh
Sens. Ventilation	0 cfm	0 Btuh
Blower Load		0 Btuh
Total sensible gain		19903 Btuh
Latent gain(ducts)		1087 Btuh
Latent gain(infiltration)		2305 Btuh
Latent gain(ventilation)		0 Btuh
Latent gain(internal/occupants/other)		1200 Btuh
Total latent gain		4592 Btuh
TOTAL HEAT GAIN		24494 Btuh



8th Edition

EnergyGauge® System Sizing
PREPARED BY: Evan Beamsley
DATE:2020-01-13

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden
Building Type: User

2020-01-13

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)

Component Loads for Whole House								
Window	Panes/Type	Frame	U	Orientation	Area(sqft)	X	HTM=	Load
1	2, NFRC 0.20	Metal	0.30	S	18.0		12.0	216 Btuh
2	2, NFRC 0.20	Metal	0.30	S	36.0		12.0	432 Btuh
3	2, NFRC 0.20	Metal	0.30	W	18.0		12.0	216 Btuh
4	2, NFRC 0.20	Metal	0.30	S	64.0		12.0	768 Btuh
5	2, NFRC 0.20	Metal	0.30	E	32.0		12.0	384 Btuh
6	2, NFRC 0.20	Metal	0.30	S	54.0		12.0	648 Btuh
7	2, NFRC 0.20	Metal	0.30	W	24.0		12.0	288 Btuh
8	2, NFRC 0.20	Metal	0.30	W	36.0		12.0	432 Btuh
9	2, NFRC 0.20	Metal	0.30	N	36.0		12.0	432 Btuh
10	2, NFRC 0.20	Metal	0.30	N	24.0		12.0	288 Btuh
11	2, NFRC 0.20	Metal	0.30	N	36.0		12.0	432 Btuh
12	2, NFRC 0.20	Metal	0.30	E	24.0		12.0	288 Btuh
13	2, NFRC 0.20	Metal	0.30	N	12.5		12.0	150 Btuh
14	2, NFRC 0.20	Metal	0.30	N	15.0		12.0	180 Btuh
Window Total					429.5(sqft)			5154 Btuh
Walls	Type	Ornt.	Ueff.	R-Value (Cav/Sh)	Area	X	HTM=	Load
1	Frame - Wood	- Ext	(0.089)	13.0/0.0	54		3.55	192 Btuh
2	Frame - Wood	- Ext	(0.089)	13.0/0.0	81		3.55	286 Btuh
3	Frame - Wood	- Ext	(0.089)	13.0/0.0	52		3.55	185 Btuh
4	Frame - Wood	- Ext	(0.089)	13.0/0.0	98		3.55	349 Btuh
5	Frame - Wood	- Ext	(0.089)	13.0/0.0	112		3.55	398 Btuh
6	Frame - Wood	- Ext	(0.089)	13.0/0.0	113		3.55	400 Btuh
7	Frame - Wood	- Ext	(0.089)	13.0/0.0	279		3.55	989 Btuh
8	Frame - Wood	- Ext	(0.089)	13.0/0.0	171		3.55	607 Btuh
9	Frame - Wood	- Ext	(0.089)	13.0/0.0	48		3.55	170 Btuh
10	Frame - Wood	- Ext	(0.089)	13.0/0.0	18		3.55	64 Btuh
11	Frame - Wood	- Ext	(0.089)	13.0/0.0	69		3.55	245 Btuh
12	Frame - Wood	- Ext	(0.089)	13.0/0.0	89		3.55	317 Btuh
13	Frame - Wood	- Ext	(0.089)	13.0/0.0	28		3.55	99 Btuh
14	Frame - Wood	- Ext	(0.089)	13.0/0.0	20		3.55	71 Btuh
15	Frame - Wood	- Ext	(0.089)	13.0/0.0	81		3.55	286 Btuh
16	Frame - Wood	- Ext	(0.089)	13.0/0.0	67		3.55	237 Btuh
17	Frame - Wood	- Ext	(0.089)	13.0/0.0	101		3.55	358 Btuh
18	Frame - Wood	- Ext	(0.089)	13.0/0.0	83		3.55	293 Btuh
19	Frame - Wood	- Ext	(0.089)	13.0/0.0	57		3.55	202 Btuh
20	Frame - Wood	- Adj	(0.089)	13.0/0.0	154		3.55	547 Btuh
Wall Total					1773(sqft)			6295 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden
Building Type: User

2020-01-13

Doors	Type	Storm Ueff.		Area X	HTM=	Load
1	Insulated - Exterior, n	(0.400)		16	16.0	256 Btuh
2	Insulated - Exterior, n	(0.400)		16	16.0	256 Btuh
3	Insulated - Exterior, n	(0.400)		16	16.0	256 Btuh
4	Insulated - Exterior, n	(0.400)		8	16.0	128 Btuh
5	Insulated - Exterior, n	(0.400)		16	16.0	256 Btuh
6	Insulated - Garage, n	(0.400)		20	16.0	320 Btuh
	Door Total			92(sqft)		1472Btuh
Ceilings	Type/Color/Surface	Ueff.	R-Value	Area X	HTM=	Load
1	Unvent Attic/D/Shing	(0.044)	0.0/22.0	1967	1.7	3436 Btuh
	Ceiling Total			1967(sqft)		3436Btuh
Floors	Type	Ueff.	R-Value	Size X	HTM=	Load
1	Slab On Grade	(1.180)	0.0	237.0 ft(perim.)	47.2	11186 Btuh
	Floor Total			1879 sqft		11186 Btuh
	Envelope Subtotal:					27543 Btuh
Infiltration	Type	Wholehouse ACH	Volume(cuft)	Wall Ratio	CFM=	
	Natural	0.29	18565	1.00	89.1	3900 Btuh
Duct load	Average sealed, R6.0, Supply(Att), Return(Att) (DLM of 0.094)					2949 Btuh
All Zones	Sensible Subtotal All Zones					34392 Btuh

WHOLE HOUSE TOTALS

Totals for Heating	Subtotal Sensible Heat Loss Ventilation Sensible Heat Loss Total Heat Loss	34392 Btuh 0 Btuh 34392 Btuh
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EQUIPMENT

1. Electric Heat Pump	#	41000 Btuh
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Manual J Winter Calculations

Residential Load - Component Details (continued)

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden
Building Type: User

2020-01-13

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values)
or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)

U - (Window U-Factor)

HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden

2020-01-13

Reference City: Gainesville, FL

Temperature Difference: 19.0F(TMY3 99%) Humidity difference: 51gr.

Component Loads for Whole House

Window	Type*						Overhang		Window Area(sqft)			HTM		Load	
	Panes	SHGC	U	InSh	IS	Ormt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2 NFRC	0.20, 0.30	No	No	S		1.5ft.	1.0ft.	18.0	18.0	0.0	10	11	178	Btuh
2	2 NFRC	0.20, 0.30	No	No	S		1.5ft.	1.0ft.	36.0	36.0	0.0	10	11	357	Btuh
3	2 NFRC	0.20, 0.30	No	No	W		99.0f	1.0ft.	18.0	18.0	0.0	10	25	178	Btuh
4	2 NFRC	0.20, 0.30	No	No	S		13.5f	2.0ft.	64.0	64.0	0.0	10	11	634	Btuh
5	2 NFRC	0.20, 0.30	No	No	E		21.2f	1.0ft.	32.0	32.0	0.0	10	25	317	Btuh
6	2 NFRC	0.20, 0.30	No	No	S		1.5ft.	6.0ft.	54.0	22.7	31.3	10	11	584	Btuh
7	2 NFRC	0.20, 0.30	No	No	W		1.5ft.	10.0f	24.0	0.0	24.0	10	25	600	Btuh
8	2 NFRC	0.20, 0.30	No	No	W		1.5ft.	10.0f	36.0	0.0	36.0	10	25	899	Btuh
9	2 NFRC	0.20, 0.30	No	No	N		1.5ft.	0.0ft.	36.0	0.0	36.0	10	10	357	Btuh
10	2 NFRC	0.20, 0.30	No	No	N		11.6f	1.0ft.	24.0	0.0	24.0	10	10	238	Btuh
11	2 NFRC	0.20, 0.30	No	No	N		9.0ft.	1.0ft.	36.0	0.0	36.0	10	10	357	Btuh
12	2 NFRC	0.20, 0.30	No	No	E		11.3f	1.0ft.	24.0	24.0	0.0	10	25	238	Btuh
13	2 NFRC	0.20, 0.30	No	No	N		18.3f	1.0ft.	12.5	0.0	12.5	10	10	124	Btuh
14	2 NFRC	0.20, 0.30	No	No	N		1.5ft.	1.0ft.	15.0	0.0	15.0	10	10	149	Btuh
Window Total									430 (sqft)					5208 Btuh	
Walls	Type					U-Value	R-Value	Area(sqft)			HTM		Load		
							Cav/Sheath								
1	Frame - Wood - Ext						0.09	13.0/0.0	54.0			2.3		122	Btuh
2	Frame - Wood - Ext						0.09	13.0/0.0	80.7			2.3		183	Btuh
3	Frame - Wood - Ext						0.09	13.0/0.0	52.0			2.3		118	Btuh
4	Frame - Wood - Ext						0.09	13.0/0.0	98.3			2.3		223	Btuh
5	Frame - Wood - Ext						0.09	13.0/0.0	112.0			2.3		253	Btuh
6	Frame - Wood - Ext						0.09	13.0/0.0	112.7			2.3		255	Btuh
7	Frame - Wood - Ext						0.09	13.0/0.0	278.5			2.3		630	Btuh
8	Frame - Wood - Ext						0.09	13.0/0.0	171.0			2.3		387	Btuh
9	Frame - Wood - Ext						0.09	13.0/0.0	48.0			2.3		109	Btuh
10	Frame - Wood - Ext						0.09	13.0/0.0	18.0			2.3		41	Btuh
11	Frame - Wood - Ext						0.09	13.0/0.0	69.0			2.3		156	Btuh
12	Frame - Wood - Ext						0.09	13.0/0.0	89.3			2.3		202	Btuh
13	Frame - Wood - Ext						0.09	13.0/0.0	28.0			2.3		63	Btuh
14	Frame - Wood - Ext						0.09	13.0/0.0	20.0			2.3		45	Btuh
15	Frame - Wood - Ext						0.09	13.0/0.0	80.7			2.3		183	Btuh
16	Frame - Wood - Ext						0.09	13.0/0.0	66.7			2.3		151	Btuh
17	Frame - Wood - Ext						0.09	13.0/0.0	100.8			2.3		228	Btuh
18	Frame - Wood - Ext						0.09	13.0/0.0	82.5			2.3		187	Btuh
19	Frame - Wood - Ext						0.09	13.0/0.0	57.0			2.3		129	Btuh
20	Frame - Wood - Adj						0.09	13.0/0.0	154.0			1.7		260	Btuh
Wall Total									1773 (sqft)					3924 Btuh	
Doors	Type							Area (sqft)			HTM		Load		
1	Insulated - Exterior						16.0			12.0		192		Btuh	
2	Insulated - Exterior						16.0			12.0		192		Btuh	
3	Insulated - Exterior						16.0			12.0		192		Btuh	
4	Insulated - Exterior						8.0			12.0		96		Btuh	
5	Insulated - Exterior						16.0			12.0		192		Btuh	
6	Insulated - Garage						20.0			12.0		240		Btuh	
Door Total									92 (sqft)					1104 Btuh	
Ceilings	Type/Color/Surface					U-Value	R-Value	Area(sqft)			HTM		Load		
1	Unvented Attic/DarkShingle						0.044	0.0/22.0	1967.0			1.31		2577	Btuh
Ceiling Total									1967 (sqft)					2577 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden

Climate:FL_GAINESVILLE_REGIONAL_A

2020-01-13

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	1879 (ft-perimeter)	0.0	0 Btuh
	Floor Total		1879.0 (sqft)		0 Btuh
	Envelope Subtotal:				12814 Btuh
Infiltration	Type	Average ACH	Volume(cuft)	Wall Ratio	CFM=
	Natural	0.22	18565	1	66.8
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
	Sensible Envelope Load:				17983 Btuh
Duct load	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DGM of 0.107)				1920 Btuh
	Sensible Load All Zones				19903 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Hayden Res
298 NW Horizon St.
Lake City, FL

Project Title:
191376 Hayden

Climate:FL_GAINESVILLE_REGIONAL_A

2020-01-13

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	17983 Btuh
	Sensible Duct Load	1920 Btuh
	Total Sensible Zone Loads	19903 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	19903 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	2305 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1087 Btuh
	Latent occupant gain (6.0 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	4592 Btuh
	TOTAL GAIN	24494 Btuh

EQUIPMENT

1. Central Unit	#	41000 Btuh
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*Key: Window types (Panels - Number and type of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value)
 (U - Window U-Factor)
 (InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))
 - For Blinds: Assume medium color, half closed
 For Draperies: Assume medium weave, half closed
 For Roller shades: Assume translucent, half closed
 (IS - Insect screen: none(N), Full(F) or Half(½))
 (Omt - compass orientation)



Version 8



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 2150579 - BLAKE CONST. - LUNDE RES.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Blake Const. Project Name: Lund Res. Model: Custom
Lot/Block: N/A Subdivision: N/A
Address: 298 NW Horizon Street, N/A
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

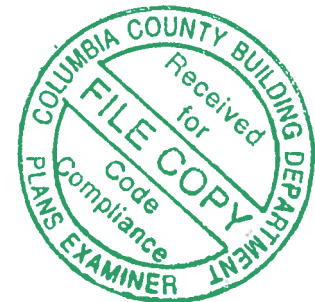
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: N/A psf

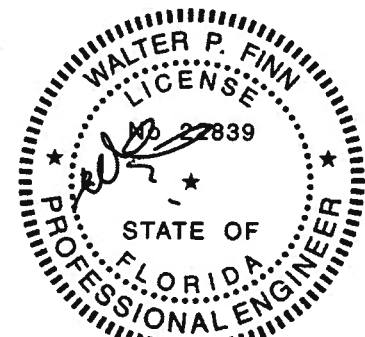
This package includes 44 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T18976587	PB01	12/23/19	23	T18976609	T10	12/23/19
2	T18976588	PB01G	12/23/19	24	T18976610	T10G	12/23/19
3	T18976589	PB02	12/23/19	25	T18976611	T11	12/23/19
4	T18976590	PB02G	12/23/19	26	T18976612	T12G	12/23/19
5	T18976591	PB03	12/23/19	27	T18976613	T13	12/23/19
6	T18976592	PB03G	12/23/19	28	T18976614	T13G	12/23/19
7	T18976593	PB04	12/23/19	29	T18976615	T14	12/23/19
8	T18976594	PB04G	12/23/19	30	T18976616	V01	12/23/19
9	T18976595	T01	12/23/19	31	T18976617	V02	12/23/19
10	T18976596	T01G	12/23/19	32	T18976618	V03	12/23/19
11	T18976597	T02	12/23/19	33	T18976619	V04	12/23/19
12	T18976598	T02G	12/23/19	34	T18976620	V05	12/23/19
13	T18976599	T03	12/23/19	35	T18976621	V06	12/23/19
14	T18976600	T04	12/23/19	36	T18976622	V07	12/23/19
15	T18976601	T04G	12/23/19	37	T18976623	V08	12/23/19
16	T18976602	T05	12/23/19	38	T18976624	V09	12/23/19
17	T18976603	T06	12/23/19	39	T18976625	V10	12/23/19
18	T18976604	T07	12/23/19	40	T18976626	V11	12/23/19
19	T18976605	T07G	12/23/19	41	T18976627	V12	12/23/19
20	T18976606	T08	12/23/19	42	T18976628	V13	12/23/19
21	T18976607	T09	12/23/19	43	T18976629	V14	12/23/19
22	T18976608	T09G	12/23/19	44	T18976630	V15	12/23/19



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Finn, Walter
My license renewal date for the state of Florida is February 28, 2021.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

December 23, 2019

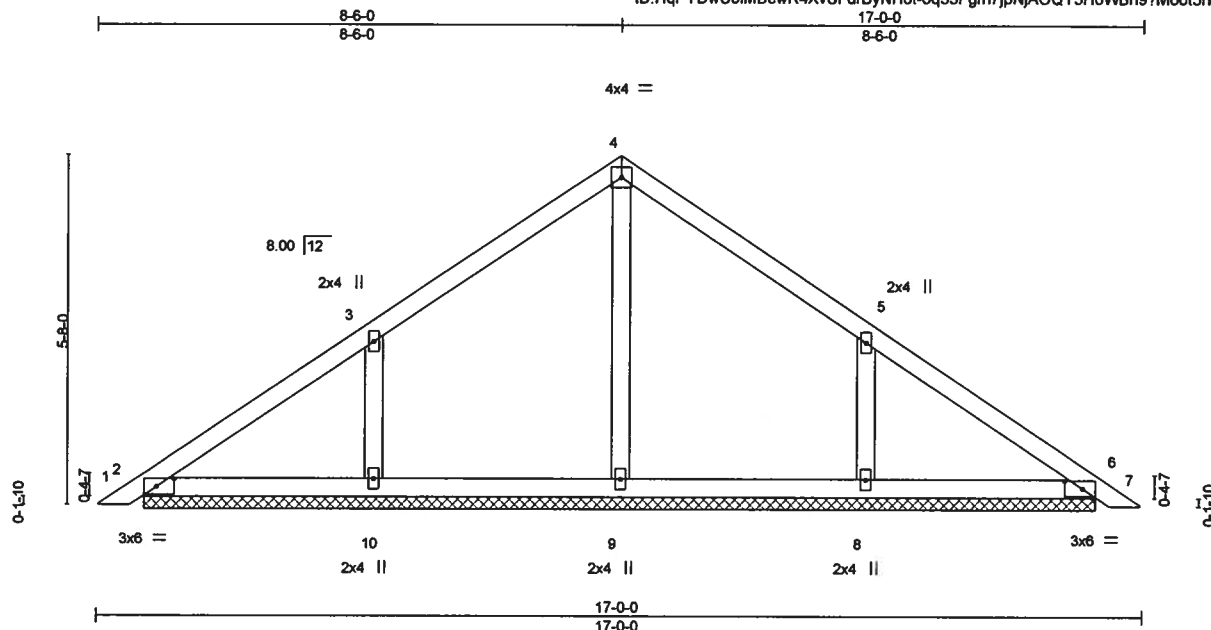
Finn, Walter

1 of 1

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976587
2150579	PB01	GABLE	22	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:14 2019 Page 1
ID: HqPYDwS0IMBewR4XvSFurByNH0t-oq33Fgm7jpNjAOQY5H0WBh97Moot5rPlhwbJ4y64nh



Scale = 1:35.9

Plate Offsets (X,Y) -		[2:0-3-9,0-1-8], [6:0-3-9,0-1-8]		17-0-0		17-0-0			
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.17	Vert(LL)	0.00	7	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.12	Vert(CT)	0.00	7	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.08	Horz(CT)	0.00	6	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 67 lb	FT = 20%

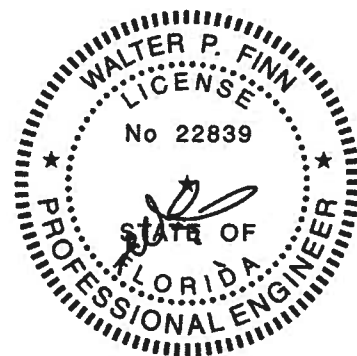
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-5-12.
(lb) - Max Horz 2=171(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=261(LC 12), 8=261(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=379(LC 19), 8=378(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-10=311/276, 5-8=311/276

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 4-0-0 oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=261, 8=261.
 - 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

December 23,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss PB01G	Truss Type GABLE	Qty 2	Ply 1	BLAKE CONST. - LUNDE RES.	T18976588
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Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:15 2019 Page 1
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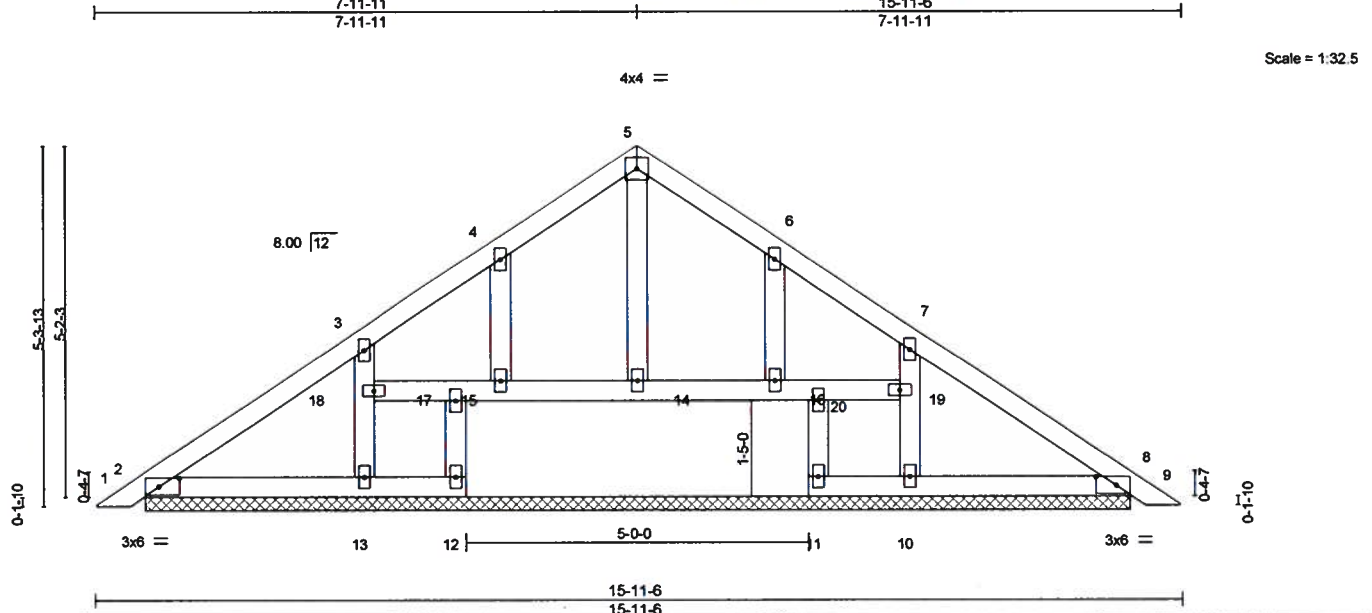


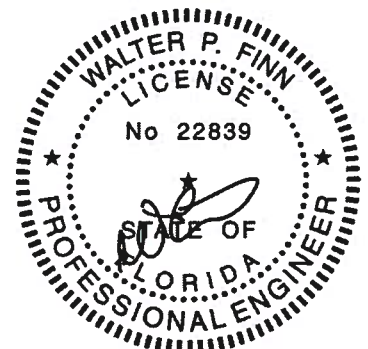
Plate Offsets (X,Y) - [2-0-3-9,0-1-8], [8-0-3-9,0-1-8]									
LOADING (psf)	SPACING-	2-0-0	CSI	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.11	Vert(LL)	0.00 9	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.07	Vert(CT)	0.00 9	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.12	Horz(CT)	0.01 8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 73 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 14-5-2.
(lb) - Max Horz 2=160(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 14, 11, 8 except 2=106(LC 13), 15=105(LC 12), 13=172(LC 12), 16=104(LC 13), 19=165(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 12, 14, 15, 16, 10, 11, 8, 19 except 13=275(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 11, 8 except (jt=lb) 2=106, 15=105, 13=172, 16=104, 19=165.
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Date:

December 23, 2019

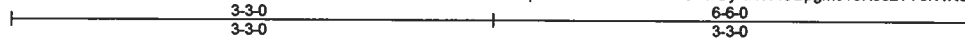
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



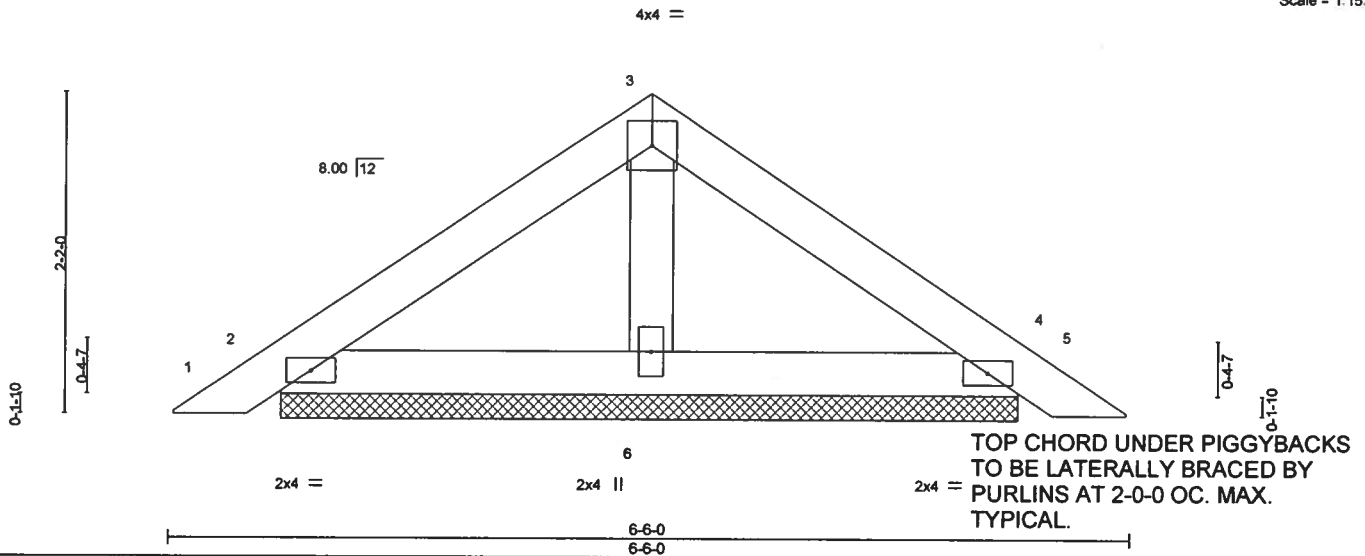
Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976589
2150579	PB02	Piggyback	3	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:16 2019 Page 1
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Scale = 1:15.0



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.10	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.06	Vert(CT)	0.00	5	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 21 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=127/4-11-12, 4=127/4-11-12, 6=167/4-11-12
Max Horz 2=62(LC 10)
Max Uplift 2=67(LC 12), 4=76(LC 13), 6=31(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

December 23,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-88 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

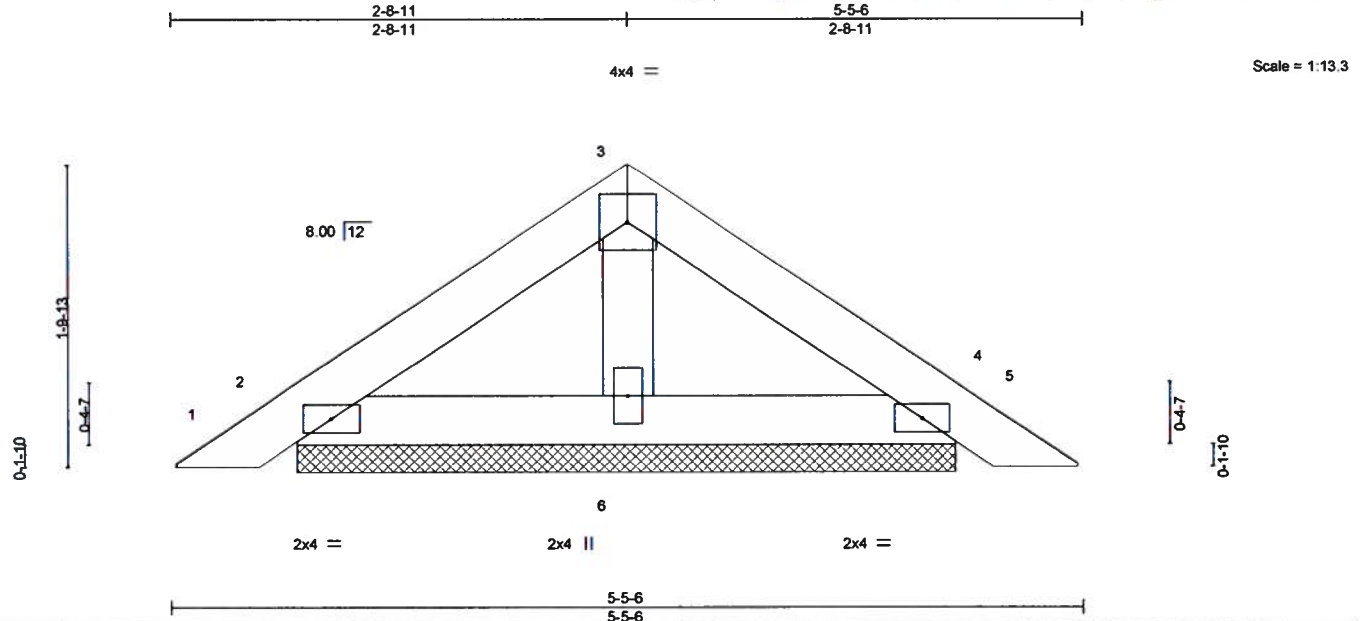
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss PB02G	Truss Type GABLE	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976590
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:17 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHot-DPIBtiffPeCyad7?DDq8qJi_arm4TfrR9FwPy64ne



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.06	Vert(LL)	0.00	4	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.04	Vert(CT)	0.00	5	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.01	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 17 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

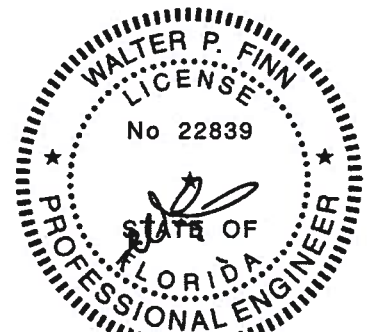
TOP CHORD Structural wood sheathing directly applied or 5-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=107/3-11-2, 4=107/3-11-2, 6=130/3-11-2
Max Horz 2=51(LC 11)
Max Uplift 2=58(LC 12), 4=64(LC 13), 6=23(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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December 23, 2019

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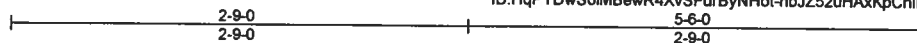


6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss PB03	Truss Type Piggyback	Qty 3	Ply 1	BLAKE CONST. - LUNDE RES.	T18976591
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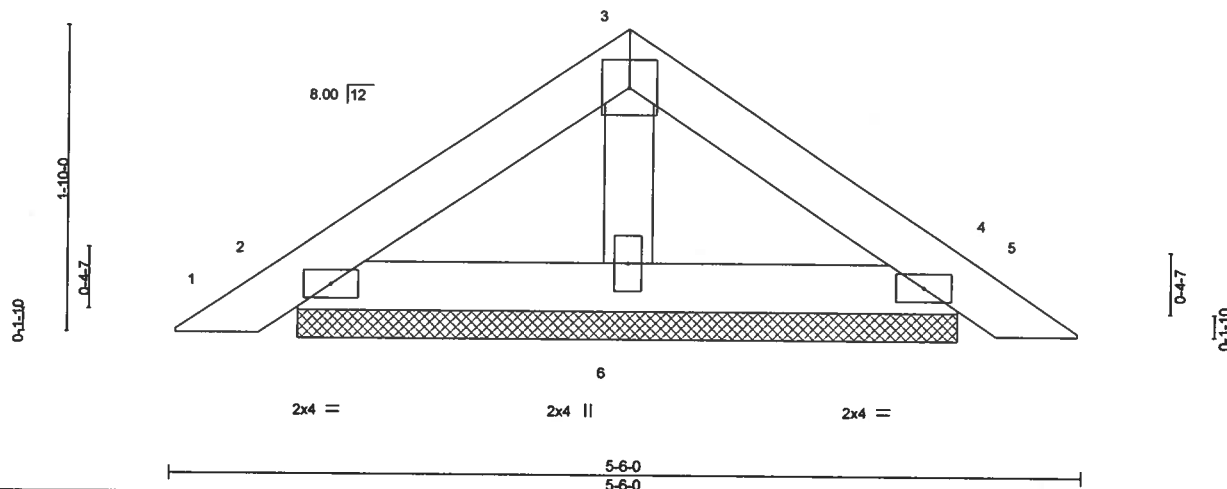
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:18 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHol-hbJZ52uHAXKpCniBnxMyg1ssi_B_pwt?gJupSry64nd



4x4 =

Scale = 1:13.3



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.07	Vert(LL)	0.00	4	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.04	Vert(CT)	0.00	5	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2017/TP12014		Matrix-P							
									Weight: 17 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

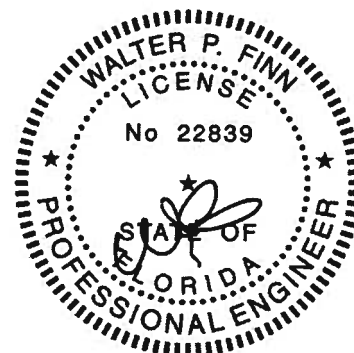
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=108/3-11-12, 4=108/3-11-12, 6=131/3-11-12
Max Horz 2=51(LC 11)
Max Uplift 2=58(LC 12), 4=65(LC 13), 6=23(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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December 23, 2019

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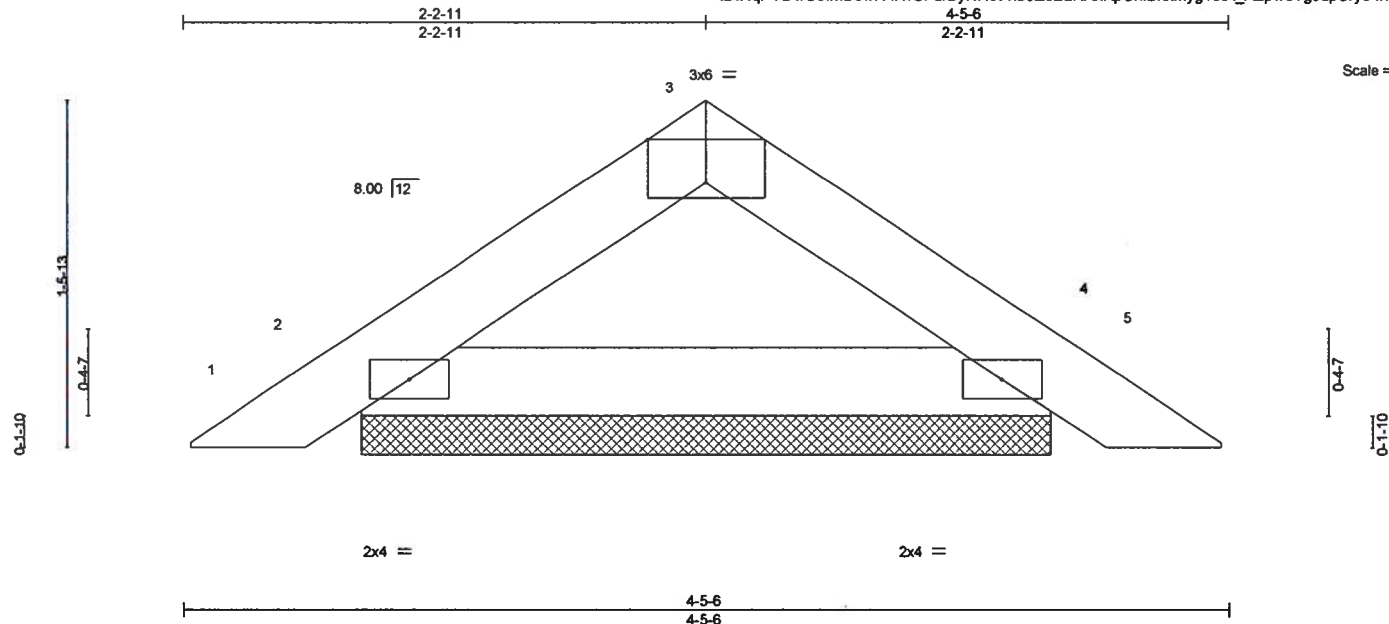
6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss PB03G	Truss Type GABLE	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976592
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:18 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNH0t-hbJZ52uHAXKpCniBnxMyg1stA_A2pw6?gJupSny64nd



Scale = 1:9.4

Plate Offsets (X,Y) - [3:0-3:0,Edge]									
LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.04	Vert(LL)	0.00	4	n/r	120	MT20 244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.10	Vert(CT)	0.00	4	n/r	120	
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	4	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 12 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=134/2-11-2, 4=134/2-11-2
Max Horz 2=-40(LC 10)
Max Uplift 2=-56(LC 12), 4=-56(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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December 23, 2019

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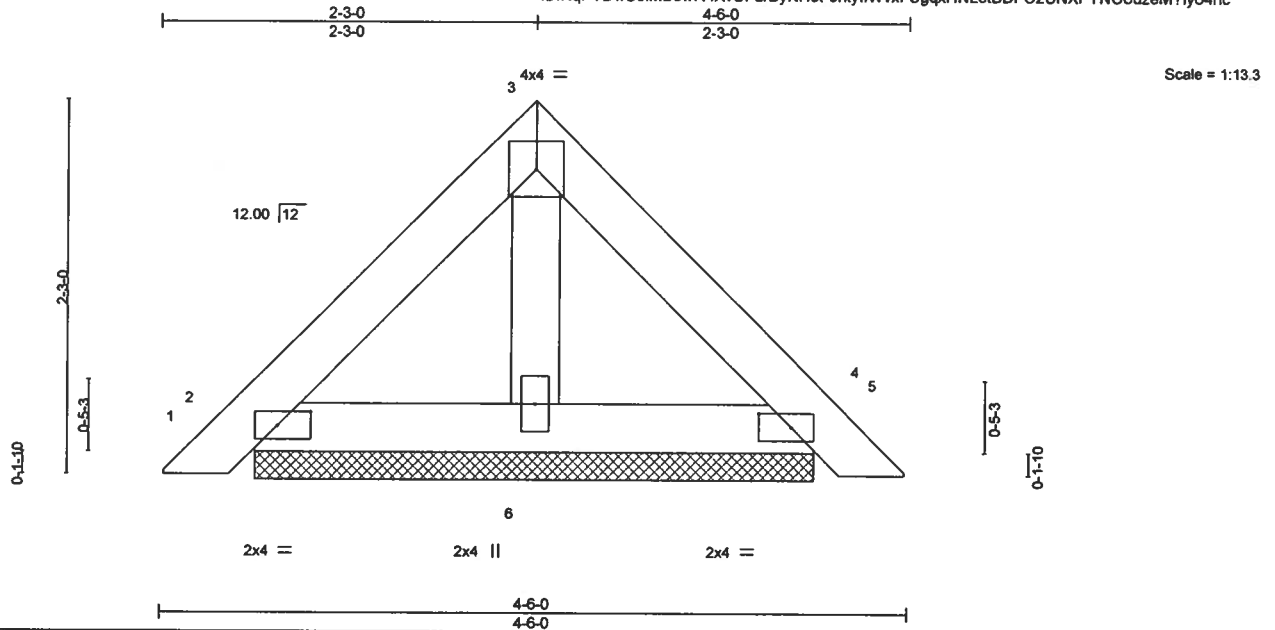
Job 2150579	Truss PB04	Truss Type Piggyback	Qty 12	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976593
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Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:19 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNHot-9ntyINvxxFSgqxHNLetBDF02UNXPYNC8uzeM7ly64nc



Scale = 1:13.3

Plate Offsets (X,Y)- [2:0-2-6,0-1-0], [4:0-2-6,0-1-0]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.06	Vert(LL)	0.00	4	n/r	120	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.03	Vert(CT)	0.00	4	n/r	120	244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.01	Horz(CT)	0.00	4	n/a	n/a	
BCDL 10.0	Code	FBC2017/TP12014	Matrix-P						
								Weight: 17 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=95/3-4-6, 4=95/3-4-6, 6=97/3-4-6
Max Horz 2=63(LC 11)
Max Uplift 2=46(LC 12), 4=53(LC 13), 6=11(LC 12)
Max Grav 2=95(LC 1), 4=95(LC 1), 6=98(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCp=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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December 23,2019

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Job 2150579	Truss PB04G	Truss Type GABLE	Qty 2	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976594
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:20 2019 Page 1
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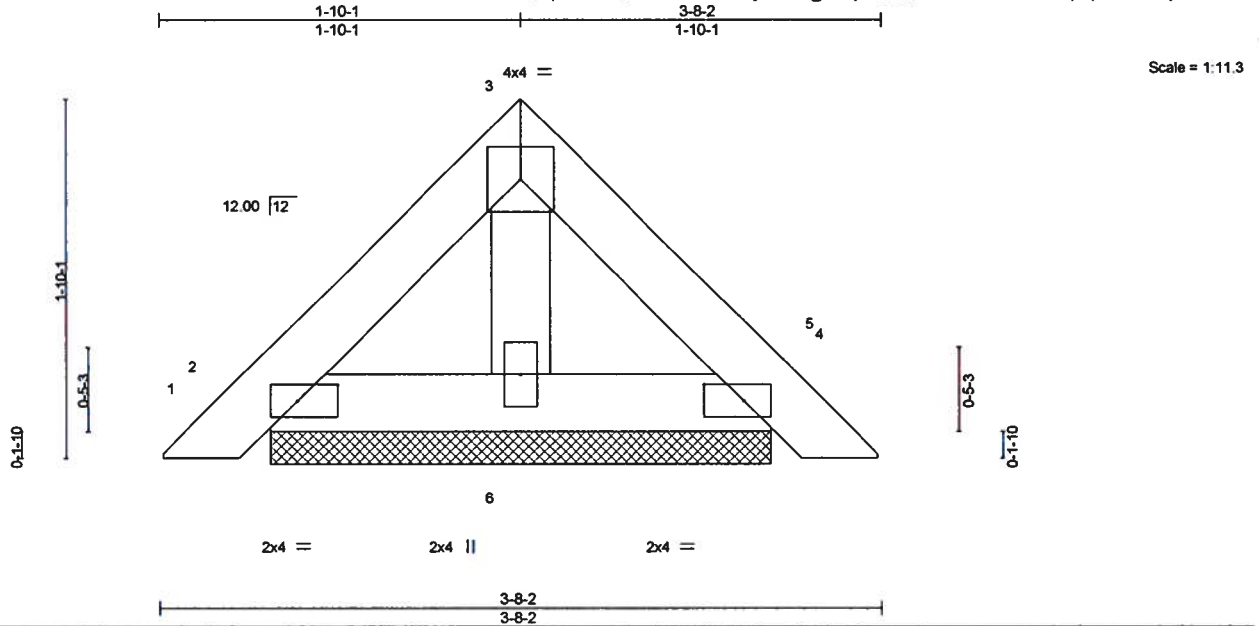


Plate Offsets (X,Y)=		[2:0-2-6,0-1-0], [4:0-2-6,0-1-0]								PLATES	GRIP
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d		MT20	244/190
TCCL 20.0	Plate Grip DOL	1.25	TC 0.04	Vert(LL)	0.00	4	n/r	120			
TCDL 7.0	Lumber DOL	1.25	BC 0.02	Vert(CT)	0.00	4	n/r	120			
BCLL 0.0	Rep Stress Incr	YES	WB 0.01	Horz(CT)	0.00	4	n/a	n/a			
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P							Weight: 13 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

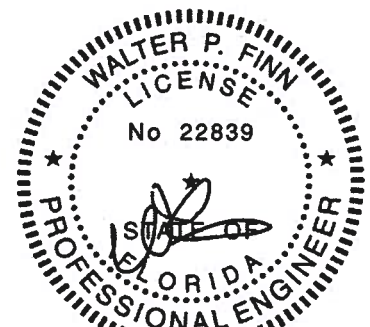
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=77/2-6-8, 4=77/2-6-8, 6=72/2-6-8
Max Horz 2=-51(LC 10)
Max Uplift 2=-38(LC 12), 4=-44(LC 13), 6=-7(LC 12)
Max Grav 2=77(LC 1), 4=77(LC 1), 6=74(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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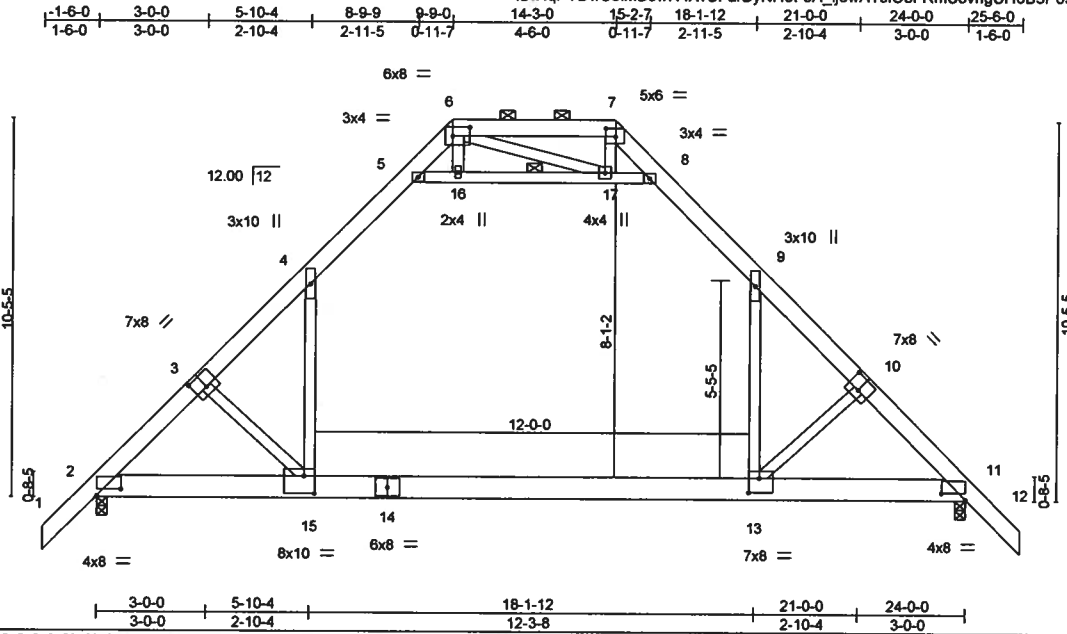
6904 Parke East Blvd.
Tampa, FL 36610

Job 2150579	Truss T01	Truss Type ATTIC	Qty 3	Ply 1	BLAKE CONST. - LUNDE RES.	T18976595
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ID:HqPYDwS0IMBewR4XvSFurByNH0t-5A_jj3wATsiO3FRmS3vflgUH0B5P05YRMH7T3Ay64na



Scale = 1:61.1

Plate Offsets (X,Y)-	[2:0-8-0,0-2-2], [3:0-4-0,0-4-8], [6:0-5-8,0-3-0], [7:0-3-4,0-2-12], [10:0-4-0,0-4-8], [11:0-8-0,0-2-2], [13:0-3-8,0-4-12], [15:0-3-8,0-5-12]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.51	Vert(LL)	-0.34	13-15	>851	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.51	Vert(CT)	-0.56	13-15	>516	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.79	Horz(CT)	0.01	11	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS	Attic	-0.21	13-15	687	360	
								Weight: 207 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 "Except"	TOP CHORD Structural wood sheathing directly applied or 5-0-9 oc purlins, except
3-6,7-10: 2x6 SP M 26	2-0-0 oc purlins (10-0-0 max.): 6-7.
BOT CHORD 2x8 SP 2400F 2.0E	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-8

REACTIONS. (lb/size) 2=1213/0-3-8, 11=1213/0-3-8
 Max Horz 2=353(LC 10)
 Max Uplift 2=161(LC 12), 11=161(LC 13)
 Max Grav 2=1446(LC 2), 11=1446(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=1989/211, 3-4=1911/223, 4-5=1017/306, 5-6=38/420, 7-8=14/461,
 8-9=1019/306, 9-10=1908/222, 10-11=1986/210, 6-7=52/801
 BOT CHORD 2-15=133/1559, 13-15=0/1192, 11-13=30/1400
 WEBS 5-16=1833/334, 16-17=1823/336, 8-17=1882/347, 4-15=0/1251, 9-13=0/1246,
 3-15=515/273, 10-13=515/274

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-16, 16-17, 8-17; Wall dead load (5.0psf) on member(s). 4-15, 9-13
 - 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
 - 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=161, 11=161.
 - 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - 11) Attic room checked for L/360 deflection.



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 Tampa, FL 33610



Job 2150579	Truss T02	Truss Type ATTIC	Qty 9	Ply 1	BLAKE CONST. - LUNDE RES.	T18976597
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8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:23 2019 Page 1
ID: HqPYDwS0IMBawR4XvSFurByNH0t-2Y6S8lyQ7Uy6fYa8aUy7N5ZdW?nvU??kpbca83y64nY

1-6-0	3-0-0	5-10-4	8-9-9	9-9-0	14-3-0	15-2-7	18-1-12	21-0-0	24-0-0
1-6-0	3-0-0	2-10-4	2-11-5	0-11-7	4-6-0	0-11-7	2-11-5	2-10-4	3-0-0

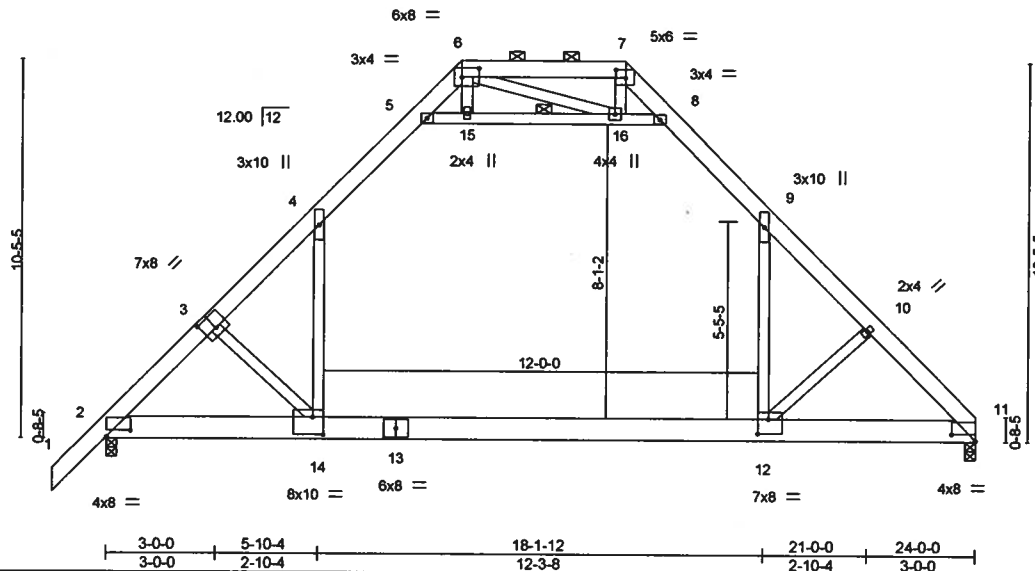


Plate Offsets (X,Y)-	[2:0-8,0,0-2,2], [3:0-4,0,0-4,8], [6:0-5,8,0-3,0], [7:0-3,4,0-2,12], [11:0-8,0,0-2,2], [12:0-3,8,0-4,12], [14:0-3,8,0-5,12]
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LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.51	Vert(LL)	-0.34 12-14	>856	240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.50	Vert(CT)	-0.56 12-14	>518	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.79	Horz(CT)	0.01 11	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS	Attic	-0.21 12-14	690	360		
							Weight: 202 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP M 26 *Except*	TOP CHORD Structural wood sheathing directly applied or 5-0-8 oc purlins, except
6-7,1-3: 2x6 SP No.2	2-0-0 oc purlins (10-0-0 max.): 6-7.
BOT CHORD 2x8 SP 2400F 2.0E	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-8

REACTIONS. (lb/size) 2=1215/0-3-8, 11=1129/0-3-8
Max Horz 2=338(LC 11)
Max Uplift 2=163(LC 12), 11=108(LC 13)
Max Grav 2=1448(LC 2), 11=1378(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=1994/224, 3-4=1916/236, 4-5=1021/313, 5-6=37/421, 7-8=12/461,
8-9=1020/312, 9-10=1928/242, 10-11=2007/232, 6-7=53/800
BOT CHORD 2-14=165/1537, 12-14=13/1170, 11-12=114/1407
WEBS 5-15=1843/352, 15-16=1833/354, 8-16=1882/362, 4-14=0/1251, 9-12=0/1264,
3-14=514/272, 10-12=549/302

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-15, 15-16, 8-16; Wall dead load (5.0psf) on member(s). 4-14, 9-12
 - Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=163, 11=108.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Attic room checked for L/360 deflection.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

December 23,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:24 2019 Page 1
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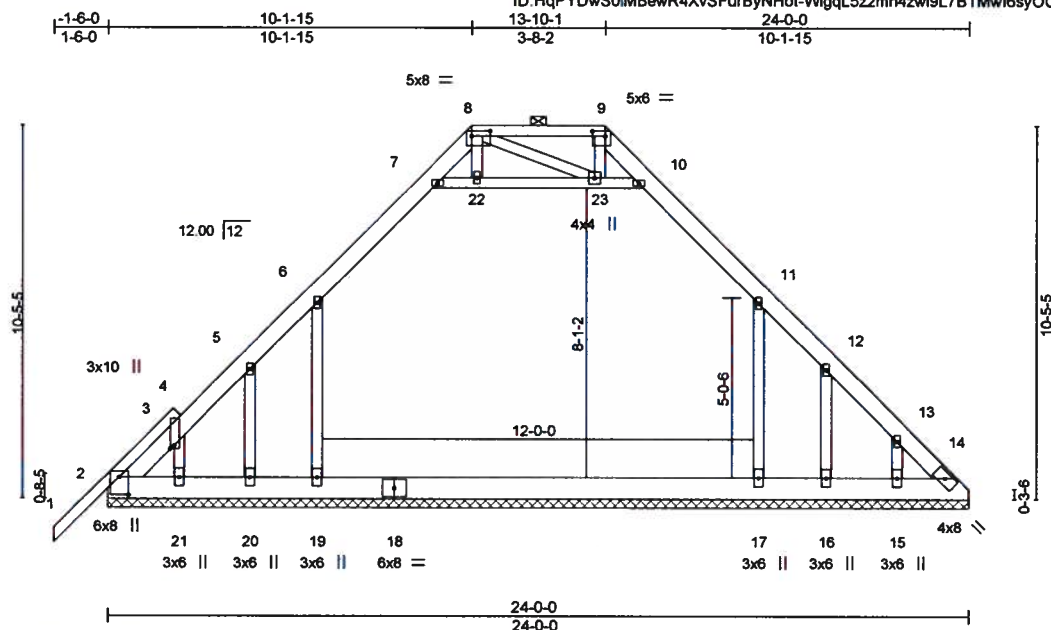


Plate Offsets (X,Y)— [2:0-6-1,0-3-0], [3:0-1-0,0-1-4], [8:0-6-4,0-1-12], [9:0-4-4,0-1-12]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.21	Vert(LL) -0.00 1 n/r 120	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.15	Vert(CT) -0.00 1 n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.14	Horz(CT) 0.00 14 n/a n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-S		Weight: 197 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.2 "Except" 1-4,8-9: 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 8-9.
BOT CHORD	2x8 SP 2400F 2.0E	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 24-0-0.
(lb) - Max Horz 2=336(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 21 except 2=139(LC 8), 14=139(LC 9), 20=512(LC 18),
16=523(LC 18), 15=157(LC 13), 19=204(LC 12), 17=199(LC 13)
Max Grav All reactions 250 lb or less at joint(s) except 2=384(LC 1), 14=309(LC 22), 21=264(LC 19), 15=308(LC
20), 19=983(LC 19), 17=981(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

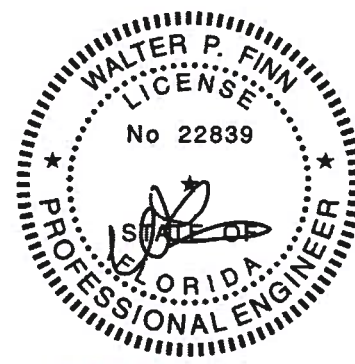
TOP CHORD 2-3=-432/203, 3-5=-376/199, 5-6=-358/230, 6-7=-402/266, 10-11=-402/266,
11-12=-355/220, 12-13=-395/200, 13-14=-456/213

BOT CHORD 2-21=-160/333, 20-21=-159/327, 19-20=-159/327, 17-19=-159/327, 16-17=-158/326,
15-16=-158/326, 14-15=-157/326

WEBS 6-19=-269/293, 11-17=-266/287

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., Gcpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21 except (jt=1b) 2=139, 14=139, 20=512, 16=523, 15=157, 19=204, 17=199.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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December 23, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 rev. 10/03/2015 BEFORE USE.

WARNING: Verify design parameters and READ NOTES on THIS and INCLUDED MITER ERECTION PAGE MH-1413 rev. 10/03/2016 BEFORE USE.

Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI 1 Quality Criteria, DSB-89 and BCS Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



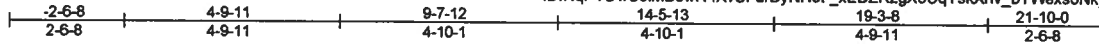
6904 Parke East Blvd.
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Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976599
2150579	T03	Common	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

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ID:HqPYDwS0IMBewR4XvSFurByNHol_xEDZRzgX5CqYskXhv_bTWexsoNkyzf1Hv5gCxy64nW



4x4 =

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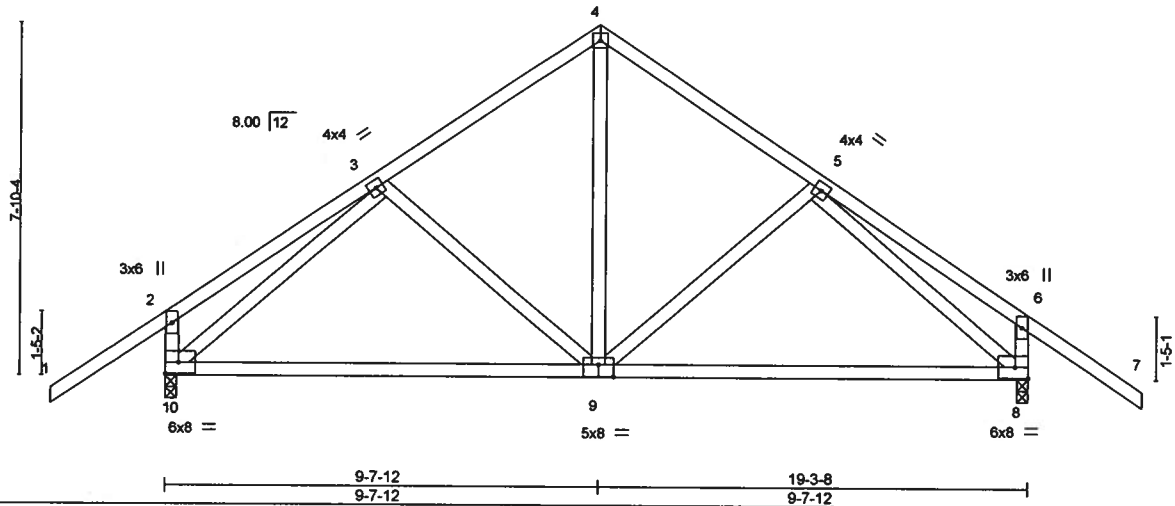


Plate Offsets (X,Y)-	[5:0-0-0,0-0-0], [6:0-0-0,0-0-0], [9:0-4-0,0-3-4]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.58	Vert(LL)	-0.16 9-10	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.86	Vert(CT)	-0.33 9-10	>701	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.52	Horz(CT)	0.02 8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 122 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

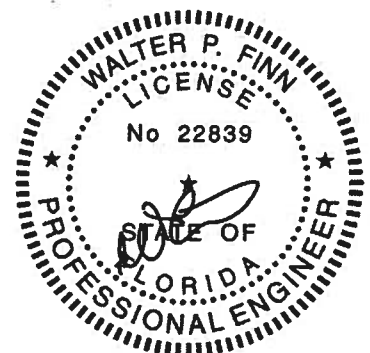
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 10=848/0-3-0, 8=848/0-3-0
Max Horz 10=308(LC 10)
Max Uplift 10=339(LC 12), 8=339(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=675/336, 4-5=674/336, 2-10=342/282, 6-8=342/281
BOT CHORD 9-10=211/643, 8-9=105/620
WEBS 4-9=183/484, 3-10=703/317, 5-8=704/317

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=339, 8=339.



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Date:

December 23, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

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6904 Parke East Blvd.
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Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976600
2150579	T04	Piggyback Base	5	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:26 2019 Page 1
ID:HqPYDwSOIMBewR4XvSFurByNHol-S7obmn_IIPK90JjFcVq?jB70CnThPkAVZqEkOy64nV

TOP CHORD UNDER PIGGYBACKS
TO BE Laterally BRACED BY
PURLINS AT 2-0-0 OC. MAX.
TYPICAL.

Scale = 1:66.3

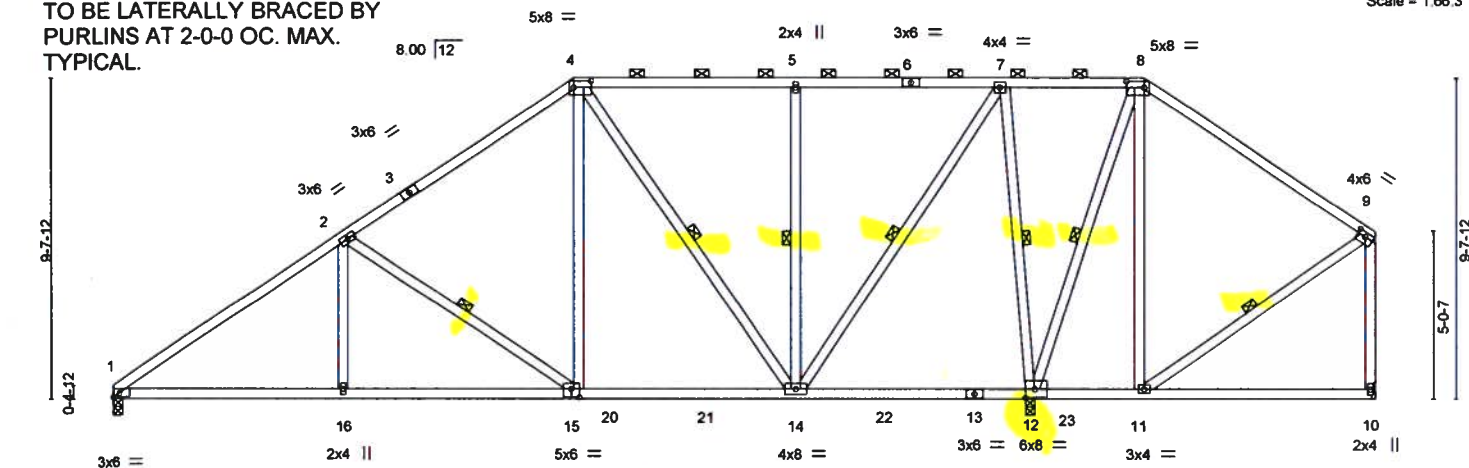


Plate Offsets (X,Y)	[4:0-6-4,0-2-4], [8:0-6-4,0-2-4], [9:0-3-0,0-1-8], [12:0-3-8,0-3-0], [15:0-3-0,0-3-0]								
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL 1.25	TC 0.56	Vert(LL)	0.10 16-19	>999	240	MT20	244/190	
TCDL 7.0	Lumber DOL 1.25	BC 0.51	Vert(CT)	-0.14 10-11	>844	180			
BCLL 0.0	Rep Stress Incr YES	WB 0.54	Horz(CT)	0.03 12	n/a	n/a			
BCDL 10.0	Code FBC2017/TP12014	Matrix-MS							
							Weight: 257 lb	FT = 20%	

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-15, 4-14, 5-14, 7-14, 7-12, 8-12, 9-11

REACTIONS. (lb/size) 1=920/0-3-8, 12=1753/0-3-8, 10=113/Mechanical
Max Horz 1=326(LC 12)
Max Uplift 1=312(LC 12), 12=707(LC 9), 10=185(LC 8)
Max Grav 1=922(LC 23), 12=1753(LC 1), 10=199(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=1353/561, 2-4=872/462, 4-5=397/348, 5-7=397/348, 7-8=80/296, 8-9=76/271
BOT CHORD 1-16=571/1094, 15-16=571/1094, 14-15=283/624
WEBS 2-16=0/294, 2-15=645/419, 4-15=193/535, 4-14=494/257, 5-14=389/296, 7-14=490/1020, 7-12=1141/608, 8-12=564/487, 8-11=291/267

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=312, 12=707, 10=185.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Date:

December 23, 2019

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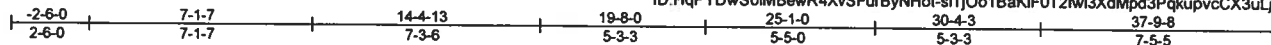
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Tampa, FL 33610

Job 2150579	Truss T04G	Truss Type GABLE Gable Gable COMMON Gable	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES.	T18976601
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:29 2019 Page 1
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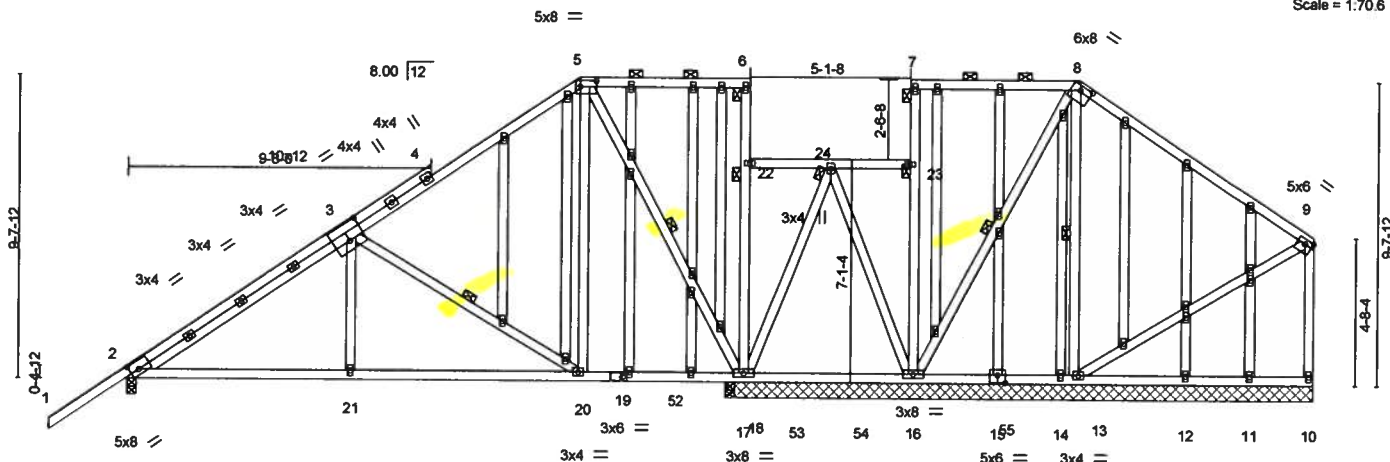


Plate Offsets (X,Y)-	[2-0-3-5,0-3-0]	[3-0-6-0,0-6-8]	[5-0-6-4,0-2-4]	[8-0-4-8,0-1-12]	[9-Edge,0-1-12]	[15-0-3-0,0-3-0]	[19-0-1-8,0-1-8]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.63	Vert(LL)	-0.06 20-21	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.48	Vert(CT)	-0.14 20-21	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.38	Horz(CT)	0.02 10	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 410 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-11-14 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 7-8, 5-6.
BOT CHORD 2x4 SP No.2	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 2x4 SP No.3	9-3-0 oc bracing: 2-21,20-21
OTHERS 2x4 SP No.3	6-0-0 oc bracing: 16-17.
	WEBS 1 Row at midpt 3-20, 5-17, 8-16, 8-13
	JOINTS 1 Brace at Jt(s): 7, 6, 22, 23, 24

REACTIONS. All bearings 18-8-8 except (jt=length) 2=0-3-8, 18=0-3-8.
(lb) - Max Horz 2=385(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 10, 18 except 2=306(LC 12), 16=177(LC 8), 13=165(LC 13), 17=288(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 13, 11, 12, 18 except 2=870(LC 23), 16=329(LC 26), 10=283(LC 24), 17=760(LC 23), 14=262(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=968/245, 3-5=474/131, 9-10=251/37
BOT CHORD 2-21=406/883, 20-21=406/883, 18-20=129/361, 17-18=129/361
WEBS 3-21=0/292, 3-20=734/435, 5-20=189/518, 5-17=671/391

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 18 except (jt=lb) 2=306, 16=177, 13=165, 17=288.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

December 23,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

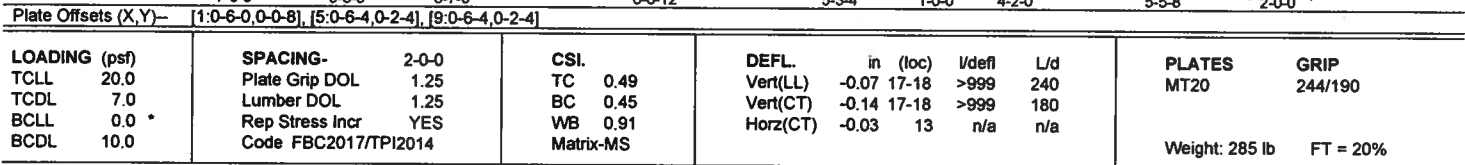
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:32 2019 Page 1
ID: HqPYDwSOIMBewR4XvSFurByNHot-HH9s1q33iF4qtxmtbticEF_R9Ydsq51P3uVHYy1y64nP



LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
 4-22: 2x6 SP No.2, 8-15: 2x4 SP No.3
WEBS 2x4 SP No.3

REACTIONS. (lb/size) 22=1667/0-3-8, 13=1181/0-3-8
 Max Horz 22=316(LC 12)
 Max Uplift 22=547(LC 12), 13=336(LC 8)
 Max Grav 22=1667(LC 1), 13=1199(LC 26)

BRACING-TOP CHORD	Structural wood sheathing directly applied or 5-11-9 oc purlins, except end verticals, and 2-0-0 oc purlins (5-7-12 max.): 5-9.
BOT CHORD	Rigid ceiling directly applied or 5-5-14 oc bracing. Except: 1 Row at midpt 8-17
WEBS	10-0-0 oc bracing: 15-17 1 Row at midpt 5-20, 6-18, 9-14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=300/321, 2-4=359/483, 4-5=783/344, 5-6=897/511, 6-8=897/511, 8-9=819/483,
 9-10=652/377
BOT CHORD 1-22=211/825, 21-22=1487/798, 4-21=1416/811, 20-21=473/301, 18-20=346/570,
 17-18=351/872, 8-17=411/344
WEBS 4-20=484/1071, 5-20=380/281, 5-18=316/532, 6-18=390/297, 14-17=158/548,
 9-17=394/777, 9-14=508/280, 10-14=246/576, 10-13=1053/497

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCp1=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 22=547, 13=336.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Date:

December 23, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 10/03/2016 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-88 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

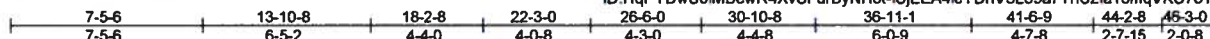
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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976604
2150579	T07	Piggyback Base	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:33 2019 Page 1
ID: HqPYDwS0IMBewR4XvSFurByNH0t-IUjEEA4ieYDhV5L39a7TnCzla13mqVKC7816UUy64nO



Scale = 1:85.2

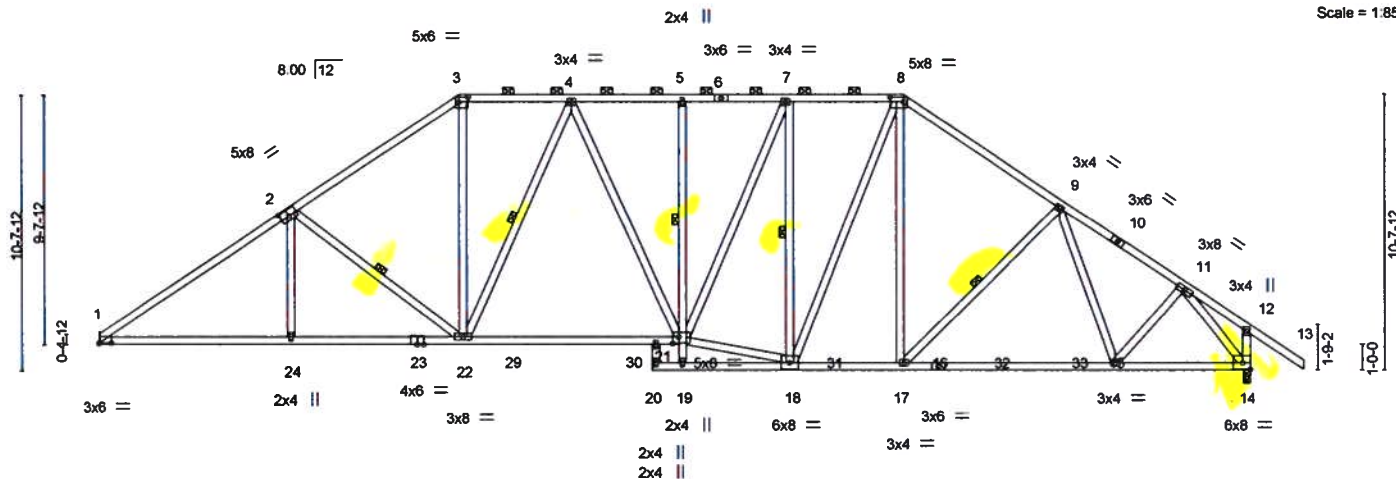


Plate Offsets (X,Y)~		[1:0-6-0,0-0-4], [2:0-4-0,0-3-0], [3:0-4-4,0-2-4], [8:0-6-4,0-2-4], [21:0-2-12,0-3-0]									
LOADING (psf)	SPACING-	2-0-0	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL 20.0	Plate Grip DOL	1.25	TC 0.60	Vert(LL)	-0.31 21-22	>999	240	MT20	244/190		
TCDL 7.0	Lumber DOL	1.25	BC 0.98	Vert(CT)	-0.55 21-22	>954	180				
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.86	Horz(CT)	0.14 14	n/a	n/a				
BCDL 10.0	Code FBC2017/TP12014		Matrix-MS								
										Weight: 325 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
5-19: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins, except end verticals, and 2-0-0 oc purlins (4-1-10 max.): 3-8.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
1 Row at midpt 5-21
10-0-0 oc bracing: 19-21
WEBS 1 Row at midpt 2-22, 4-22, 7-18, 9-17

REACTIONS. (lb/size) 1=1639/Mechanical, 14=1763/0-3-0
Max Horz 1=348(LC 11)
Max Uplift 1=512(LC 12), 14=578(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=2599/1157, 2-3=2142/1068, 3-4=1698/968, 4-5=1952/1085, 5-7=1947/1084,
7-8=1679/998, 8-9=1876/994, 9-11=1837/894, 12-14=256/265
BOT CHORD 1-24=789/2086, 22-24=788/2090, 21-22=649/1876, 17-18=416/1478, 15-17=562/1566,
14-15=422/1157
WEBS 2-24=0/270, 2-22=627/414, 3-22=388/888, 4-22=547/368, 4-21=152/293,
18-21=491/1753, 7-21=299/680, 7-18=872/469, 8-18=362/621, 8-17=163/440,
9-17=295/288, 9-15=259/162, 11-15=138/531, 11-14=1897/765

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCp=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=512, 14=578.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 8634
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Date:

December 23, 2019

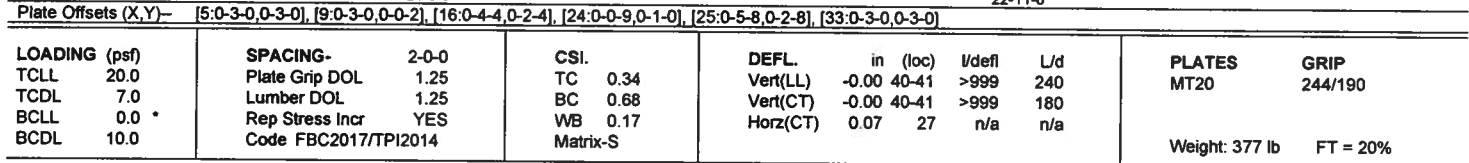
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:36 2019 Page 1
 ID: HqPYDwS0IMBewR4XvSFurByNH0t-93ONsB6axTbGMY4eqjHAPqbtvEAE11yep6Fm5py64nL
 14-4-13 19-9-12 24-11-4 30-4-3 44-2-8 46-3-0
 14-4-13 5-4-15 5-1-8 5-4-15 13-10-5 2-0-8



REACTIONS. All bearings 22-11-8 except (jt=length) 1=21-3-0, 1=21-3-0, 51=21-3-0, 50=21-3-0, 49=21-3-0, 48=21-3-0, 47=21-3-0, 45=21-3-0, 44=21-3-0, 43=21-3-0, 42=21-3-0, 41=21-3-0.

(lb) - Max Horz 1=344(LC 11)

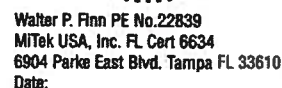
Max Uplift All uplift 100 lb or less at joint(s) 1, 27, 34, 35, 44, 43, 42, 41, 57 except 39=163(LC 11), 38=356(LC 13), 28=372(LC 13), 29=102(LC 13), 30=114(LC 13), 31=111(LC 13), 32=113(LC 13), 33=113(LC 13), 51=111(LC 12), 50=115(LC 12), 49=101(LC 12), 48=112(LC 12), 47=117(LC 12), 45=115(LC 12), 40=151(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 27, 38, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 51, 50, 49, 48, 47, 45, 44, 43, 42, 41, 40, 57 except 1=264(LC 12), 39=349(LC 13), 27=420(LC 13)

NOTES:-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP1 1.
- 4) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Vertical gable studs spaced at 2-0-0 oc and horizontal gable studs spaced at 2-0-0 oc.

Continued on page 2. This truss is designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976605
2150579	T07G	GABLE II	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:36 2019 Page 2
ID:HqPYDwS0IMBewR4XvSFurByNH0t-93ONsB6axTbGMY4eqhAPqbtvEAE11yep6Fm5py64nL

NOTES-

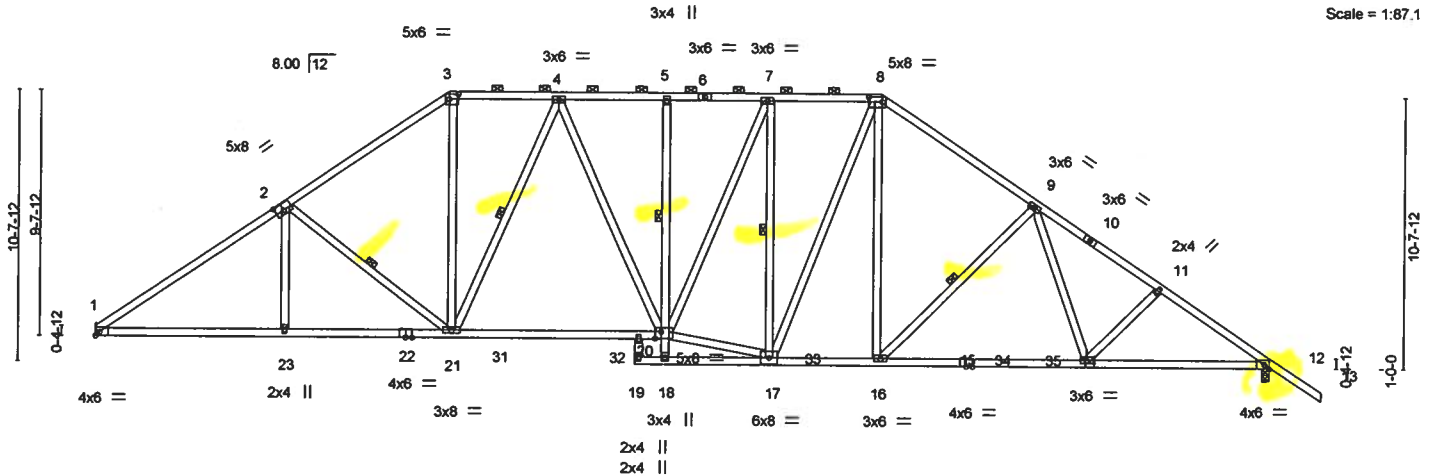
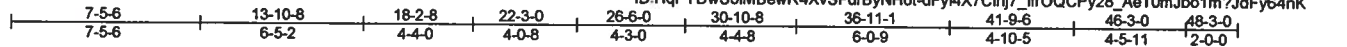
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Bearing at joint(s) 40, 57 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 27, 34, 35, 44, 43, 42, 41, 57 except (jt=lb) 39=163, 38=356, 28=372, 29=102, 30=114, 31=111, 32=113, 33=113, 51=111, 50=115, 49=101, 48=112, 47=117, 45=115, 40=151.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see *ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information* available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Tampa, FL 36610



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LUMBER.
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
 5-18: 2x4 SP No.3, 20-22: 2x4 SP M 31
WEBS 2x4 SP No.3

REACTIONS. (lb/size) 1=1721/Mechanical, 12=1832/O-3-8
 Max Horz 1=-344(LC 8)
 Max Uplift 1=-529(LC 12), 12=-623(LC 13)

BRACING-TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (3-11-4 max.): 3-8.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 1 Row at midpt 5-20 10-0-0 oc bracing: 18-20
WEBS	1 Row at midpt 2-21, 4-21, 7-17, 9-16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

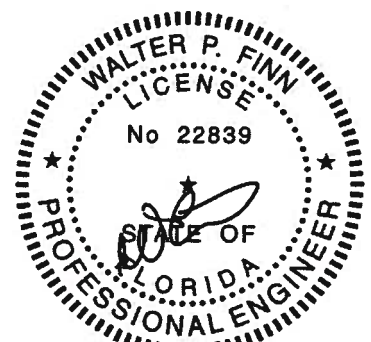
TOP CHORD 1-2=274/1218, 2-3=2288/1130, 3-4=1821/1020, 4-5=2147/1168, 5-7=2142/1167, 7-8=1890/1087, 8-9=2166/1118, 9-11=2627/1223, 11-12=2776/1233

BOT CHORD 1-23=794/2206, 21-23=794/2211, 20-21=636/2035, 16-17=481/1723, 14-16=732/2046, 12-14=888/2252

WEBS 2-23=0/269, 2-21=625/414, 3-21=422/967, 4-21=630/386, 4-20=171/375, 17-20=539/1947, 7-20=291/652, 7-17=847/462, 8-17=355/559, 8-16=241/661, 9-16=601/394, 9-14=100/383, 11-14=259/206

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=529, 2=623.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

December 23, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MJ-7473 rev. 10/03/2015 BEFORE USE.

WARNING – verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI 1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22301. 22314.



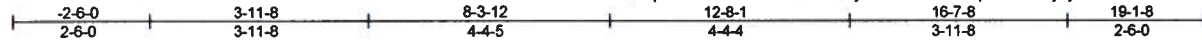
6904 Parke East Blvd.
Tampa, FL 36610

Job 2150579	Truss T09	Truss Type Common	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976607
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:38 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNHot-5RW7Ht8qT5rzsE1y8jeUFhBI2t7VxMxGQkIAhy64nJ



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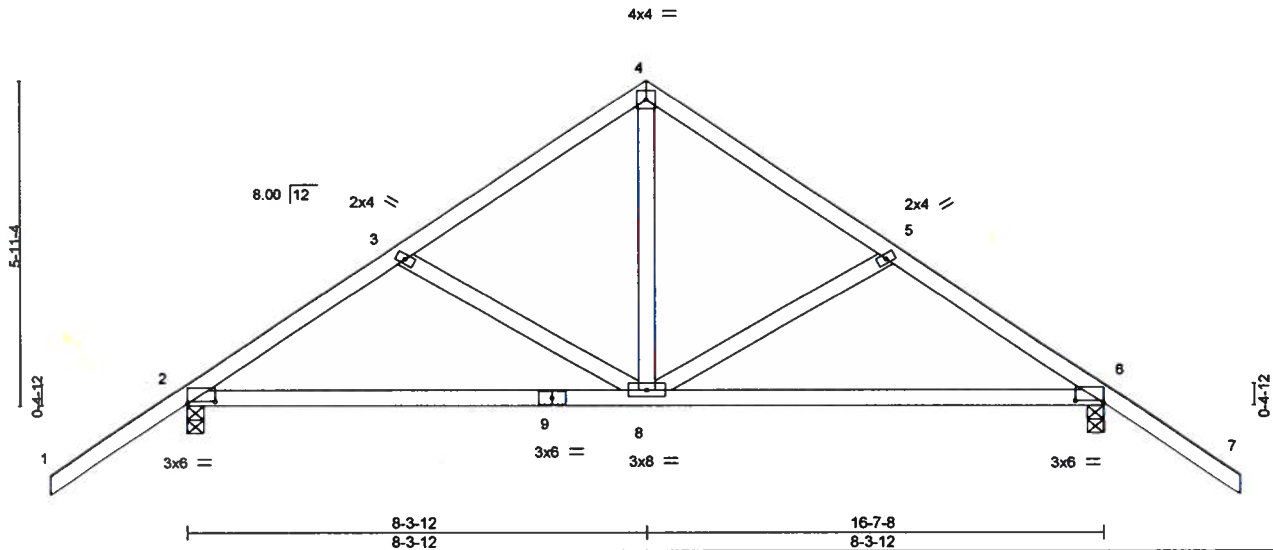


Plate Offsets (X,Y)=[2:0-6-0,0-0-8], [6:0-6-0,0-0-8]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.50	Vert(LL)	-0.08	8-12	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.59	Vert(CT)	-0.15	8-12	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.17	Horz(CT)	0.02	6	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						Weight: 86 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

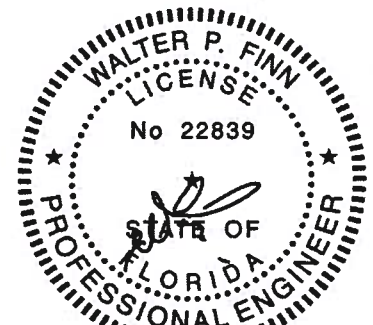
(lb/size) 2=750/0-3-8, 6=750/0-3-8
Max Horz 2=224(LC 11)
Max Uplift 2=310(LC 12), 6=310(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-827/343, 3-4=-653/273, 4-5=-653/273, 5-6=-827/342
BOT CHORD 2-8=-239/716, 6-8=-144/712
WEBS 4-8=-117/485, 5-8=-340/225, 3-8=-340/224

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=310, 6=310.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
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Date:

December 23, 2019

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



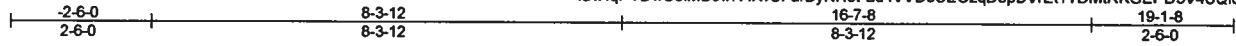
6904 Parke East Blvd.
Tampa, FL 36610

Job 2150579	Truss T09G	Truss Type Common Supported Gable	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES.	T18976608
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:40 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNHot-ad4VVD8SE0zqD0pDvREt1TDMIRKGEPSV4UQi6y64nl



Scale = 1:39.2

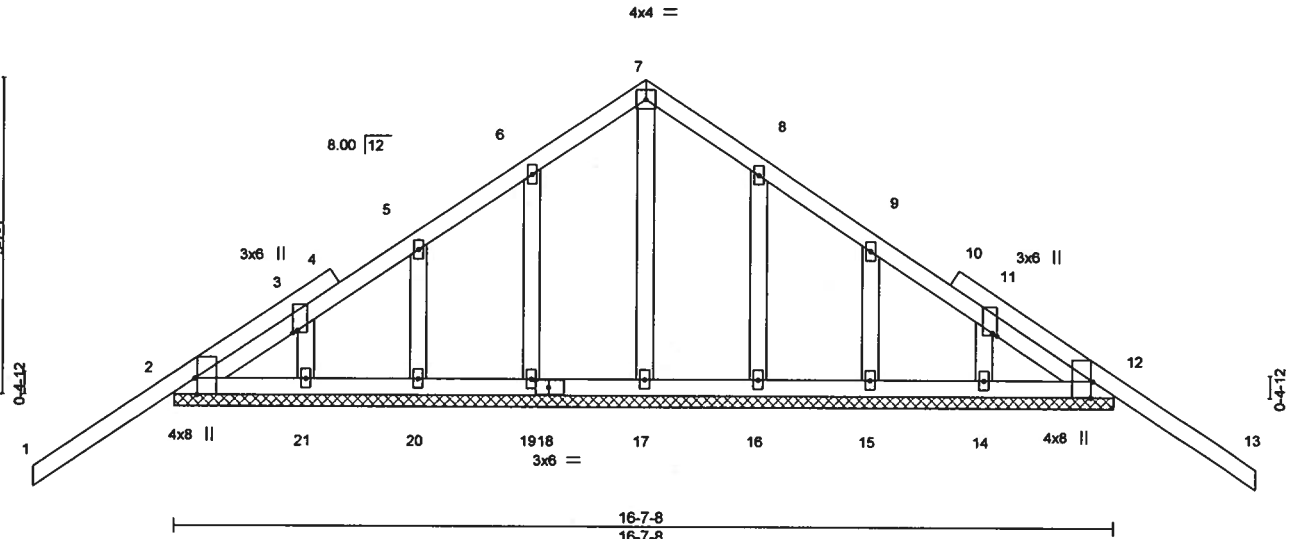


Plate Offsets (X,Y)- [2:0-3-8,Edge], [3:0-0-9,0-1-0], [11:0-0-9,0-1-0], [12:0-3-8,Edge], [18:0-2-12,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.48	Vert(LL)	-0.05	13	n/r	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.08	Vert(CT)	-0.07	13	n/r		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	12	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 101 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

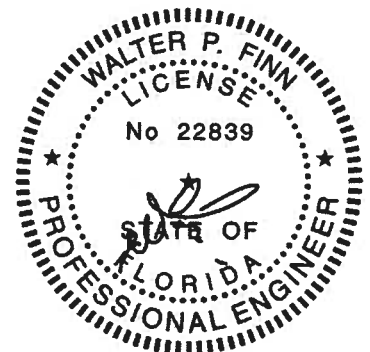
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-7-8.
(lb) - Max Horz 2=214(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 21, 14 except 2=123(LC 12), 12=151(LC 13), 19=114(LC 12), 20=120(LC 12), 16=112(LC 13), 15=124(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 17, 19, 20, 21, 16, 15, 14 except 2=294(LC 1), 12=294(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 14 except (jt=lb) 2=123, 12=151, 19=114, 20=120, 16=112, 15=124.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 12.



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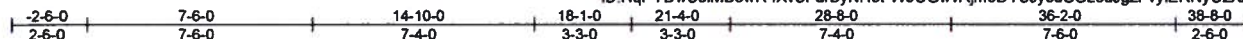
6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss T10	Truss Type Piggyback Base	Qty 3	Ply 1	BLAKE CONST. - LUNDE RES.	T18976609
Job Reference (optional)						

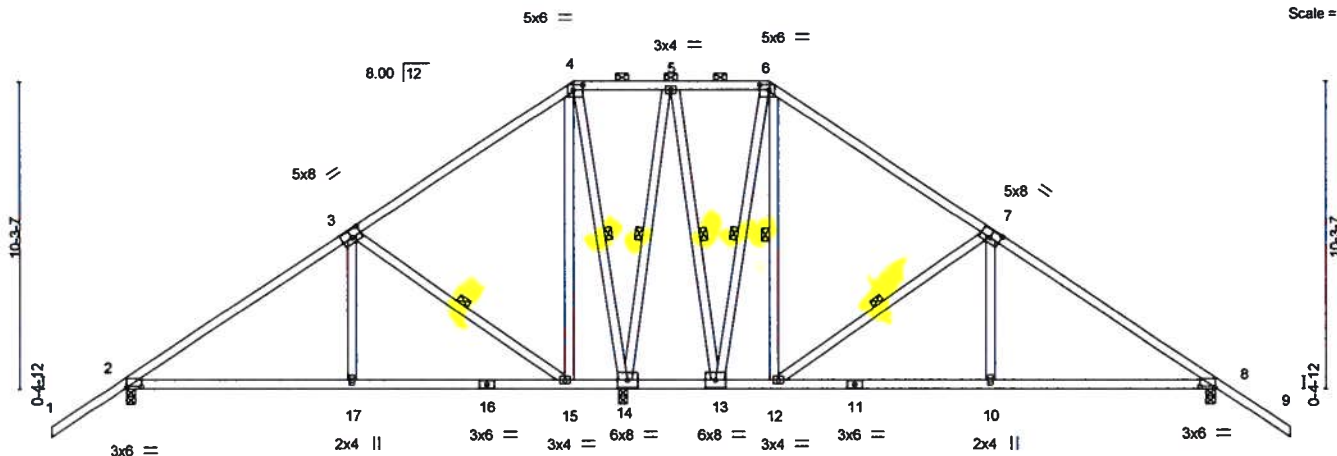
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:41 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNH0t-W0CgwvAjm0DYSJycdGGL6uJgzFvyIERNyOzXm0y64nG



Scale = 1:73.8



	7-6-0	14-10-0	16-5-12	19-8-4	21-4-0	28-8-0	36-2-0
	7-6-0	7-4-0	1-7-12	3-2-8	1-7-12	7-4-0	7-6-0
Plate Offsets (X,Y)-	[2:0-6-0,0-0-8], [3:0-4-0,0-3-0], [4:0-3-12,0-2-0], [6:0-3-12,0-2-0], [7:0-4-0,0-3-0], [8:0-6-0,0-0-8]						

LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.64	Vert(LL)	0.14	10-23	>999	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.51	Vert(CT)	-0.17	17-20	>999	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.41	Horz(CT)	0.02	8	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
									Weight: 253 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-8-12 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-15, 4-14, 5-14, 5-13, 6-13, 6-12, 7-12

REACTIONS. (lb/size) 2=624/0-3-8, 14=1562/0-3-8, 8=761/0-3-8
Max Horz 2=361(LC 10)
Max Uplift 2=280(LC 12), 14=505(LC 9), 8=391(LC 8)
Max Grav 2=658(LC 23), 14=1562(LC 1), 8=796(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=589/203, 3-4=123/254, 4-5=38/266, 5-6=204/334, 6-7=331/429, 7-8=834/823
BOT CHORD 2-17=196/558, 15-17=195/560, 14-15=282/398, 13-14=302/329, 10-12=508/619, 8-10=503/617
WEBS 3-17=0/325, 3-15=726/421, 4-15=179/461, 4-14=760/350, 5-14=767/725, 5-13=647/627, 6-13=627/618, 6-12=619/456, 7-12=663/729, 7-10=323/322

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=280, 14=505, 8=391.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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December 23, 2019

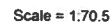
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



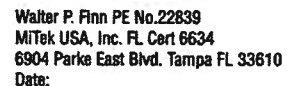
6904 Parke East Blvd.
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8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:43 2019 Page 1
MBewR4XvSFurByNHot-SPJOKbBzHdTGid6 kh.lpB.IQOY3V7A9iqQiSdrvv64nE



NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2'-0" oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 15 except (R=lb) 2=373, 18=264, 14=313, 9=117, 11=102, 19=318, 9=117.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



December 23, 2019

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Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSYS/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

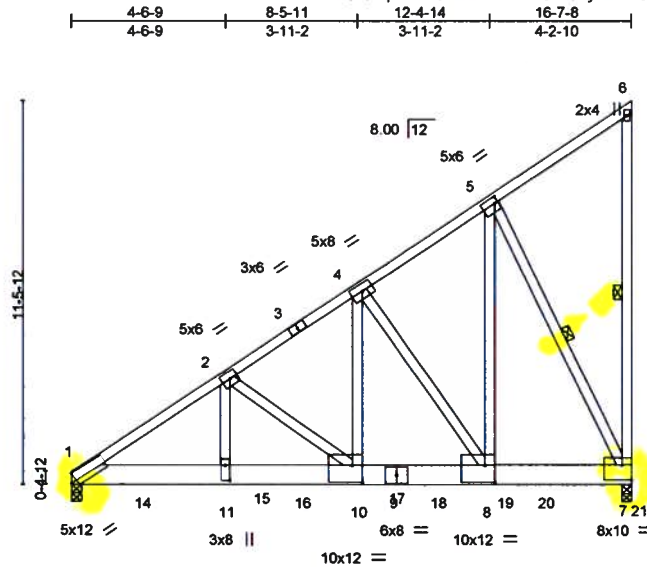


Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976611
2150579	T11	Monopitch Girder	1	2	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:45 2019 Page 1

ID: HqPYDwS0IMBewR4XvSFurByNH0t-OnRnIGDDpEj_xxGNS6LHHkTNFsHqev4zt0xkyny64nC



Scale = 1:66.0

Plate Offsets (X, Y) —		[1: Edge, 0-3-0], [5: 0-1-4, 0-2-4], [7: Edge, 0-5-8], [8: 0-3-8, 0-6-4], [10: 0-3-8, 0-6-4], [11: 0-5-12, 0-1-8]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCCL 20.0	Plate Grip DOL	1.25	TC 0.55
TCDL 7.0	Lumber DOL	1.25	BC 0.45
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.94
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MS
			DEFL. in (loc) l/defl L/d
			Vert(LL) -0.11 10-11 >999 240
			Vert(CT) -0.21 10-11 >941 180
			Horz(CT) 0.03 7 n/a n/a
			PLATES GRIP
			MT20 244/190
			Weight: 298 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x8 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except*
 4-10, 5-8, 5-7: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 6-7, 5-7

REACTIONS. (lb/size) 7=8069/0-3-8 (req. 0-4-12), 1=6600/0-3-8 (req. 0-3-14)
 Max Horz 1=535(LC 8)
 Max Uplift 7=2805(LC 8), 1=2002(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=10181/3058, 2-4=6866/2012, 4-5=3572/1004
 BOT CHORD 1-11=2993/8450, 10-11=2993/8450, 8-10=1988/5662, 7-8=1029/2933
 WEBS 2-11=1166/3708, 2-10=3486/1256, 4-10=1772/5502, 4-8=4735/1664, 5-8=2463/7573,
 5-7=6565/2302

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-7-0 oc.
 Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- WARNING: Required bearing size at joint(s) 7, 1 greater than input bearing size.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=2805, 1=2002.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1619 lb down and 532 lb up at 2-0-12, 1619 lb down and 532 lb up at 4-0-12, 1701 lb down and 549 lb up at 6-0-12, 1701 lb down and 549 lb up at 8-0-12, 1701 lb down and 549 lb up at 10-0-12, 1701 lb down and 549 lb up at 12-0-12, and 1701 lb down and 549 lb up at 14-0-12, and 1707 lb down and 543 lb up at 16-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25



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Continued on page 2

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 Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976611
2150579	T11	Monopitch Girder	1	2	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:45 2019 Page 2
ID:HqPYDwS0IMBewR4XvSFurByNHol-OnRnIGDDpEJ_xGNS6LHHkTNFsHqev4zt0xkvny64nC

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-6=-54, 1-7=-20

Concentrated Loads (lb)

Vert: 14=-1619(B) 15=-1619(B) 16=-1701(B) 17=-1701(B) 18=-1701(B) 19=-1701(B) 20=-1701(B) 21=-1707(B)

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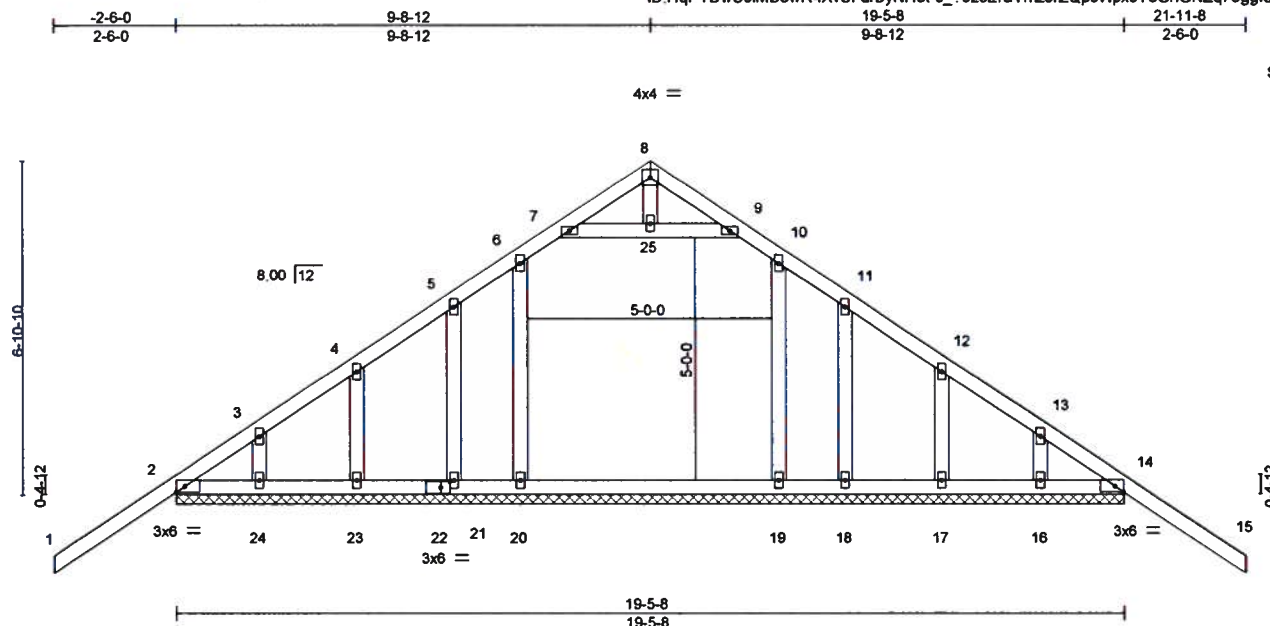


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Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976612
2150579	T12G	GABLE	1	1	Job Reference (optional)	

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8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:46 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHol-s_?9zcEraYrrZ5rZQpsWpx0Y8GhGNZq76ggISDy64nB



Scale = 1/4\"

Plate Offsets (X,Y)–		[14:0-2-3,Edge], [22:0-2-4,0-1-8]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.48	Vert(LL)	-0.06	15	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.25	Vert(CT)	-0.09	15	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	WB 0.08	Horz(CT)	0.01	14	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
								Weight: 115 lb	FT = 20%	

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-5-8.
(lb) - Max Horz 2=254(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 20, 19, 21, 24, 16 except 2=131(LC 12), 23=143(LC 12), 18=103(LC 13), 17=142(LC 13), 14=159(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 21, 23, 24, 18, 17, 16 except 2=369(LC 1), 20=412(LC 19), 19=374(LC 20), 14=369(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; End., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 19, 21, 24, 16 except (jt=lb) 2=131, 23=143, 18=103, 17=142, 14=159.



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Date:

December 23,2019

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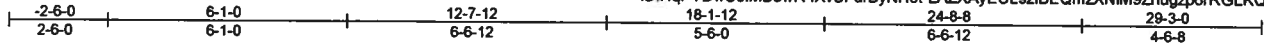
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss T13	Truss Type Piggyback Base	Qty 3	Ply 1	BLAKE CONST. - LUNDE RES.	T18976613
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:47 2019 Page 1
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4x8 =

4x8 =

Scale = 1:56.1

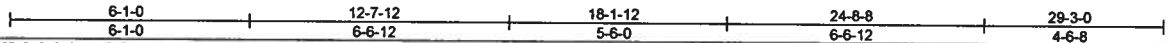
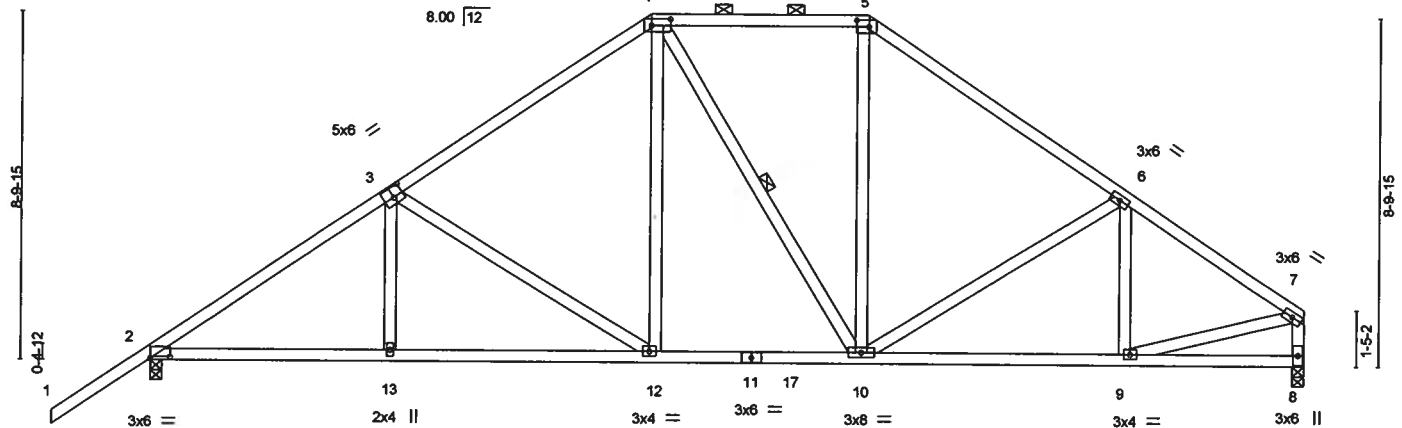


Plate Offsets (X,Y)- [2:0-6-0,0-0-8], [3:0-3-0,0-3-0], [4:0-5-12,0-2-0], [5:0-3-12,0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.61	Vert(LL)	0.16 12-13	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.49	Vert(CT)	-0.15 12-13	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.82	Horz(CT)	0.05 8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 180 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purlins, except end verticals, and 2-0-0 oc purlins (5-9-6 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 4-8-10 oc bracing.
WEBS 1 Row at midpt 4-10

REACTIONS.

(lb/size) 2=1218/0-3-8, 8=1071/0-3-8
Max Horz 2=294(LC 9)
Max Uplift 2=514(LC 9), 8=466(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1619/1754, 3-4=-1196/1358, 4-5=-876/1178, 5-6=-1155/1311, 6-7=-1276/1400, 7-8=-1035/1116
BOT CHORD 2-13=-1412/1281, 12-13=-1413/1282, 10-12=-880/911, 9-10=-1107/1023
WEBS 3-13=-291/267, 3-12=-567/637, 4-12=-587/409, 5-10=-494/348, 6-10=-291/324, 7-9=-1099/1021

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=514, 8=466.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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December 23, 2019

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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976614
2150579	T13G	GABLE	1	1		

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:49 2019 Page 1
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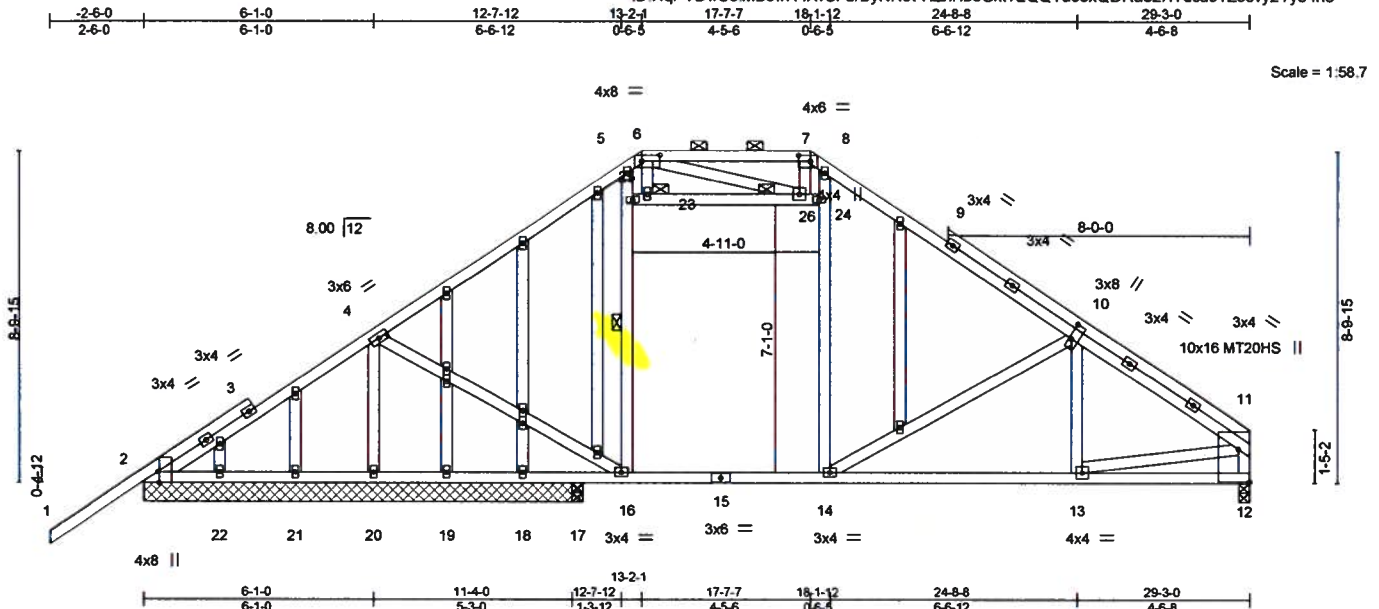


Plate Offsets (X,Y) - [2:0-3-8,Edge], [6:0-5-12,0-2-0], [7:0-3-12,0-2-0], [10:0-5-0,0-0-12], [11:Edge,0-3-8], [12:0-0-0,0-1-12]

LOADING (psf)	SPACING-	CSL	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	2-0-0	TC 0.55	Vert(LL)	0.23 13-14	>904	240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.59	Vert(CT)	-0.22 13-14	>948	180	MT20HS	187/143
BCCL 0.0 *	Rep Stress Incr YES	WB 0.60	Horz(CT)	0.03 12	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS					Weight: 237 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-4-3 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 5-4-0 oc bracing.
WEBS 1 Row at midpt 5-16
JOINTS 1 Brace at Jt(s): 23, 24

REACTIONS. All bearings 11-7-8 except (it=length) 12=0-3-8, 17=0-3-8.
(lb) - Max Horz 2=295(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 22 except 2=296(LC 8), 20=492(LC 12), 12=497(LC 8), 18=113(LC 8), 17=335(LC 24)
Max Grav All reactions 250 lb or less at joint(s) 22, 21, 19, 18 except 2=518(LC 1), 20=861(LC 19), 12=932(LC 24), 17=311(LC 8), 2=518(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=574/581, 4-5=913/1013, 5-6=690/904, 6-7=750/1006, 7-8=722/897, 8-10=950/1057, 10-11=1182/1275, 11-12=904/954
BOT CHORD 2-22=345/477, 21-22=345/477, 20-21=345/477, 19-20=345/477, 18-19=345/477, 17-18=345/477, 16-17=345/477, 14-16=610/696, 13-14=1028/964
WEBS 4-20=822/712, 4-16=291/428, 6-23=305/285, 10-14=417/480, 11-13=1024/964, 16-25=275/273, 5-25=294/315, 14-26=458/321, 8-26=318/261

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 22 except (it=lb) 2=296, 20=492, 12=497, 18=113, 17=335, 2=296.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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December 23, 2019

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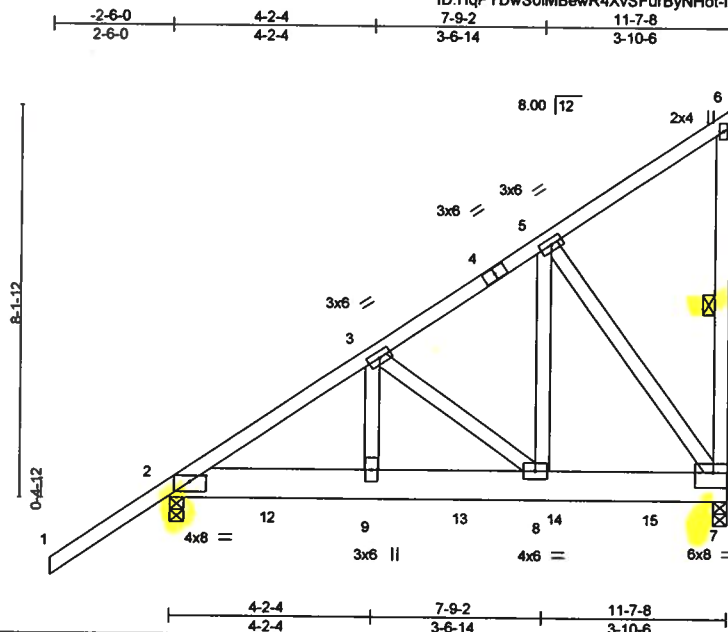
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss T14	Truss Type Monopitch Girder	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES.	T18976615
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:50 2019 Page 1
ID: HqPYDwS0IMBewR4XvSFurByNHol-IEgo_HMenMG2i8KfxSznAFil5cJFqi1eVb7y64n7



Scale = 1.46.1

Plate Offsets (X, Y) - [2:0-4-0,0-1-9], [7:Edge,0-3-12]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.44	Vert(LL)	0.03	8-9	>999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.10	Vert(CT)	-0.03	8-9	>999		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.59	Horz(CT)	-0.01	7	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 97 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-5-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7

REACTIONS.

(lb/size) 2=995/0-3-8, 7=884/0-3-8
Max Horz 2=442(LC 8)
Max Uplift 2=619(LC 8), 7=814(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

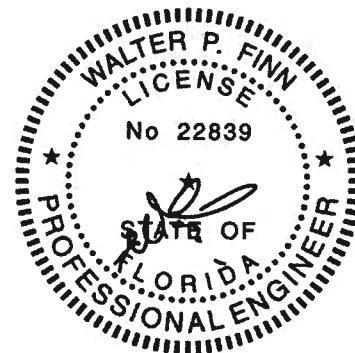
TOP CHORD 2-3=1123/710, 3-5=625/356
BOT CHORD 2-9=837/894, 8-9=837/894, 7-8=445/479
WEBS 3-9=403/409, 3-8=522/493, 5-8=668/748, 5-7=807/749

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=619, 7=814.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 179 lb down and 205 lb up at 2-0-12, 179 lb down and 205 lb up at 4-0-12, 179 lb down and 205 lb up at 6-0-12, and 179 lb down and 205 lb up at 8-0-12, and 179 lb down and 205 lb up at 10-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=54, 2-7=20
Concentrated Loads (lb)
Vert: 9=179(B) 12=179(B) 13=179(B) 14=179(B) 15=179(B)



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Date:

December 23, 2019

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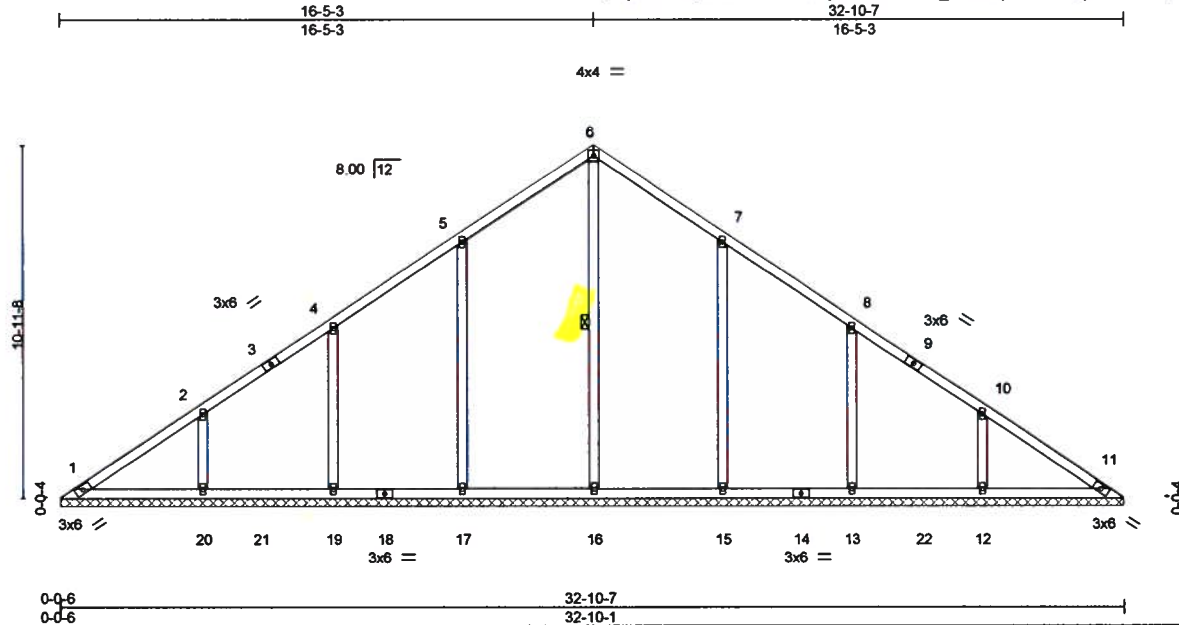


6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss V01	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976616
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:51 2019 Page 1
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Scale = 1:68.6

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.19	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.16	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.35	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 11 n/a n/a		
	Code FBC2017/TPI2014			Weight: 166 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-16

REACTIONS.

All bearings 32-9-11.
(lb) - Max Horz 1=331(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 11 except 17=242(LC 12), 19=212(LC 12), 20=252(LC 12), 15=242(LC 13), 13=213(LC 13), 12=252(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 16=425(LC 22), 17=476(LC 19), 19=431(LC 19), 20=387(LC 19), 15=475(LC 20), 13=431(LC 20), 12=387(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=304/254, 5-6=275/305, 6-7=275/303
WEBS 5-17=292/266, 4-19=267/238, 2-20=303/269, 7-15=292/265, 8-13=267/239, 10-12=303/269

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except (jt=lb) 17=242, 19=212, 20=252, 15=242, 13=213, 12=252.



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Date:

December 23, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss V02	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976617
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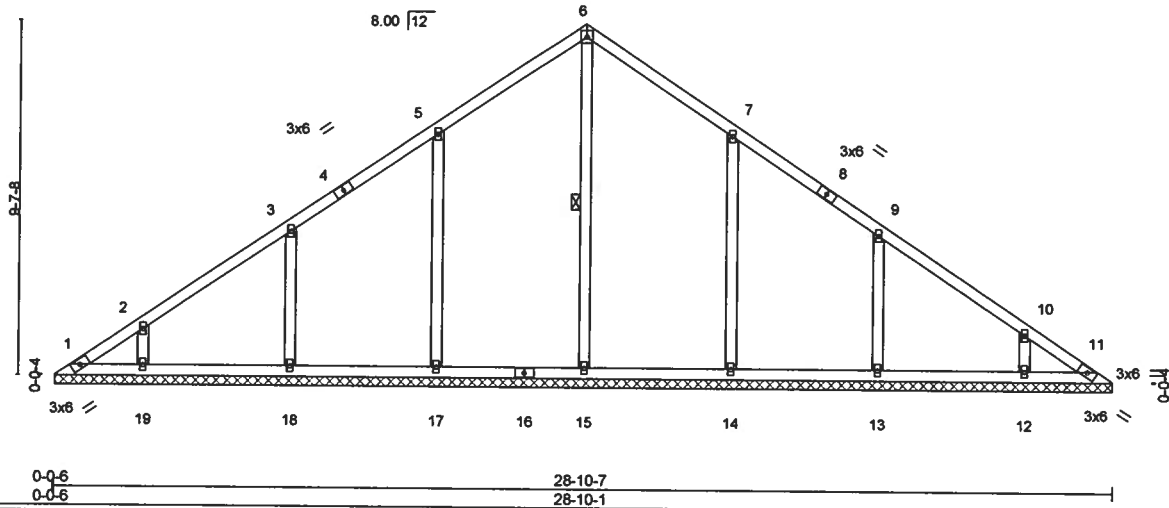
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:52 2019 Page 1
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14-5-3
14-5-3
28-10-7
14-5-3

4x4 =

Scale = 1:60.2



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.17	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.17	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.24	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 11 n/a n/a		
	Code FBC2017/TPI2014			Weight: 140 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-15

REACTIONS.

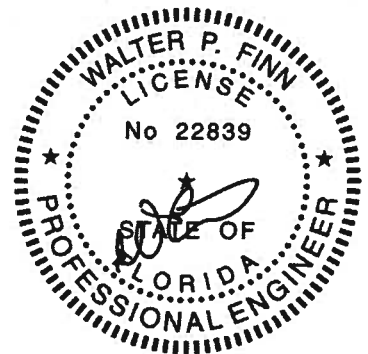
All bearings 28-9-11.
(lb) - Max Horz 1=290(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 11 except 17=239(LC 12), 18=226(LC 12), 19=192(LC 12), 14=239(LC 13), 13=226(LC 13), 12=192(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 15=400(LC 22), 17=478(LC 19), 18=397(LC 19), 19=274(LC 19), 14=477(LC 20), 13=398(LC 20), 12=274(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=296/230, 5-6=241/267, 6-7=241/262
WEBS 5-17=289/263, 3-18=281/251, 7-14=289/262, 9-13=281/251

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except (jt=lb) 17=239, 18=226, 19=192, 14=239, 13=226, 12=192.



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MiTek USA, Inc. FL Cert 6634
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Date:

December 23, 2019

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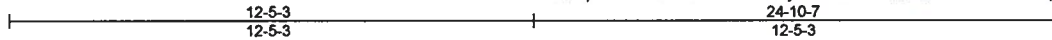
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2150579	Truss V03	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976618
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:53 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNH0t-9KwoR?JExikrv9tvKnU9bQoqB55CWk8jGt9BKy64n4



Scale = 1:52.6

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.17	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.17	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.22	Horz(CT)	0.01	7	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 113 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 24-9-11.
(lb) - Max Horz 1=248(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=234(LC 12), 13=247(LC 12), 9=234(LC 13), 8=248(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=387(LC 22), 12=456(LC 19), 13=381(LC 19), 9=455(LC 20), 8=381(LC 20)

FORCES.

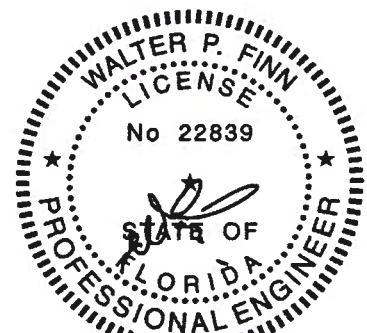
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS

3-12=285/260, 2-13=299/264, 5-9=285/260, 6-8=299/264

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpj=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=234, 13=247, 9=234, 8=248.



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December 23, 2019

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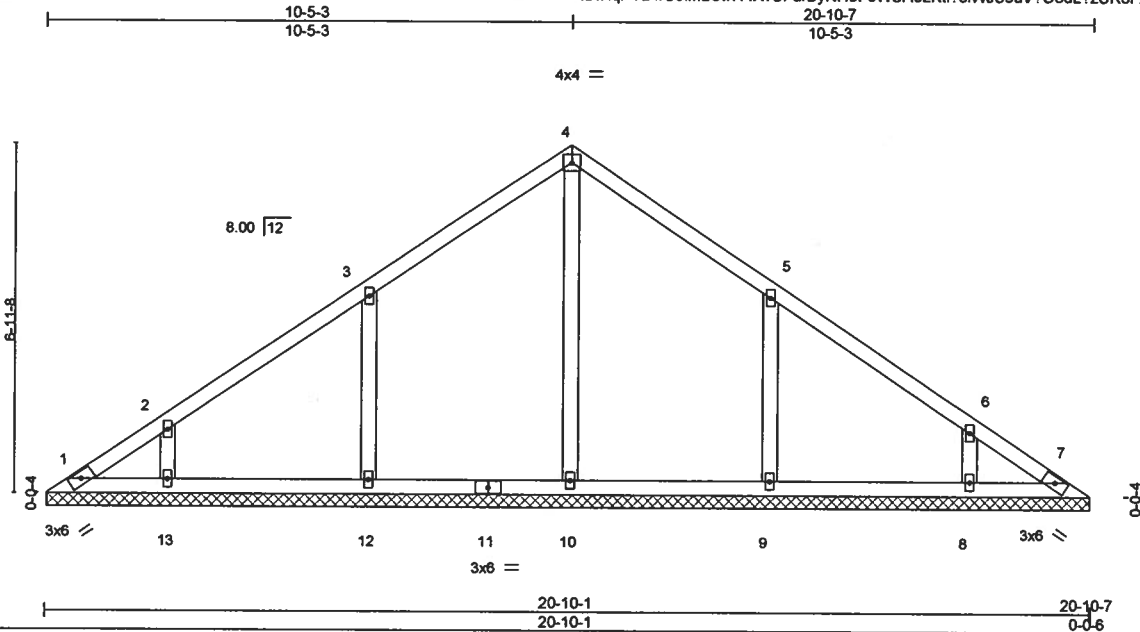


6904 Parke East Blvd.
Tampa, FL 36610

Job 2150579	Truss V04	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES.	T18976619
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:54 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNH0t-eWUAeLKti?siWJS6uV?O8dL?zUR6FAylywgkmy64n3



Scale = 1:44.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.17	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.19	Vert(LL) n/a - n/a 999		
BCLL 0.0	Lumber DOL 1.25	WB 0.13	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 7 n/a n/a		
	Code FBC2017/TPI2014			Weight: 90 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 20-9-11.
(lb) - Max Horz 1=207(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=249(LC 12), 13=184(LC 12), 9=249(LC 13), 8=185(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=374(LC 22), 12=421(LC 19), 13=266(LC 19), 9=421(LC 20), 8=266(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-12=301/274, 5-9=301/274

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) All plates are 2x4 MT20 unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=249, 13=184, 9=249, 8=185.



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December 23,2019

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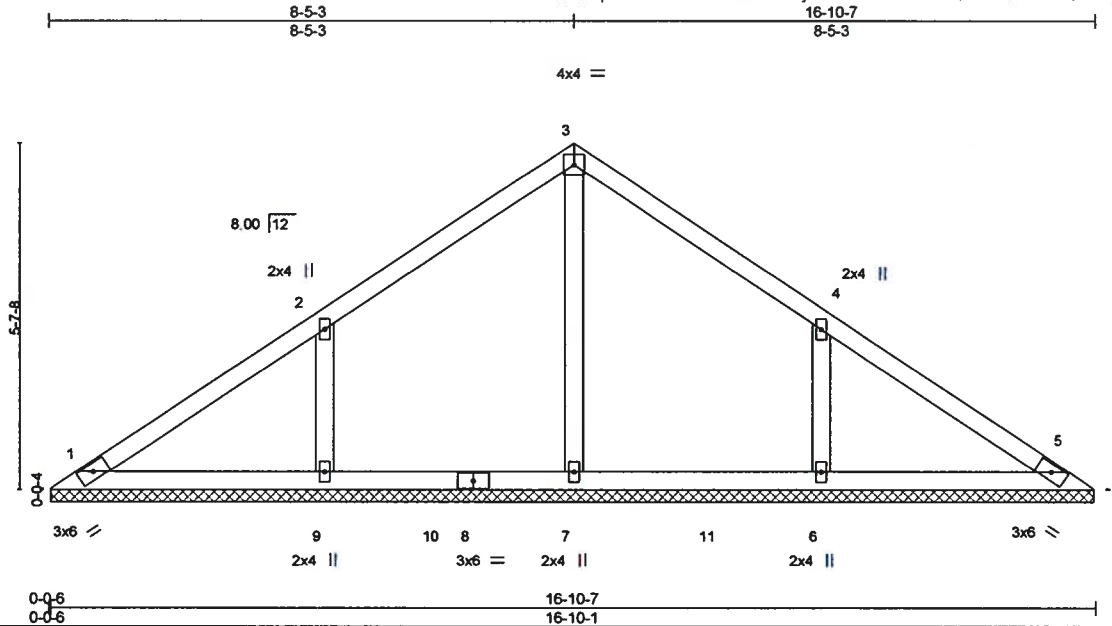
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976620
2150579	V05	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:56 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNH0t-avcx31L7Dd6QmdcU7w2sD2QLDI7Jj58bPD5poey64n1



Scale = 1/32

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.19	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.14	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr YES	WB 0.09	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-S					Weight: 68 lb	FT = 20%

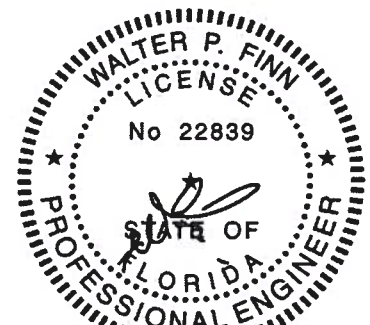
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS. All bearings 16-9-11.
(lb) - Max Horz 1=165(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=269(LC 12), 6=269(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=330(LC 19), 9=406(LC 19), 6=406(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=319/286, 4-6=319/286

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=269, 6=269.



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Date:

December 23, 2019

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Tampa, FL 33610

Job 2150579	Truss V06	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES.	T18976621
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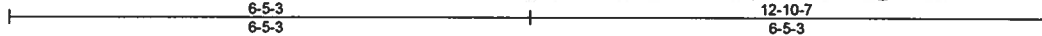
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:57 2019 Page 1

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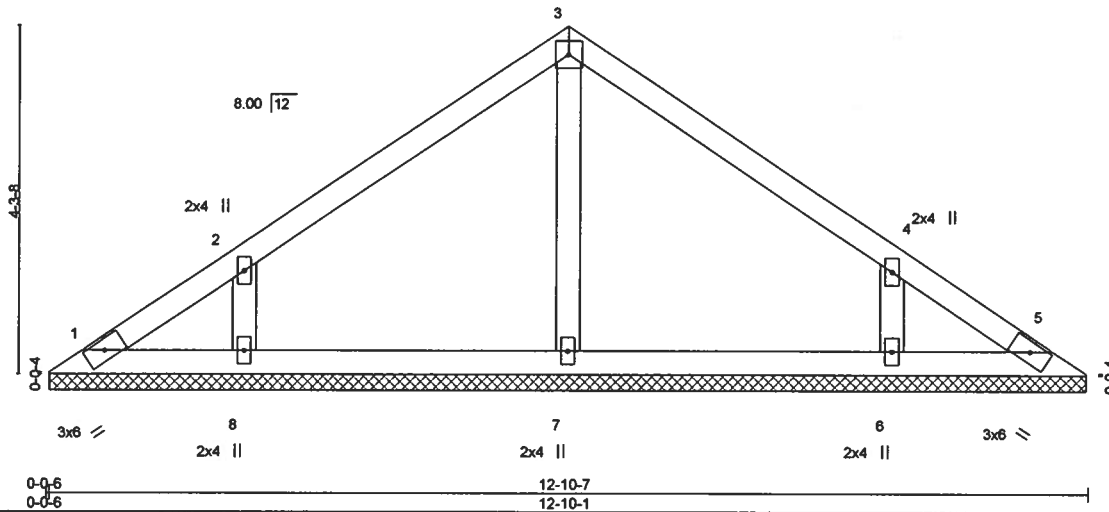
12-10-7

6-5-3



4x4 =

Scale = 1:27.3



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.15	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.12	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr YES	WB 0.07	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-S					Weight: 49 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 12-9-11.

(lb) - Max Horz 1=124(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=215(LC 12), 6=215(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=308(LC 19), 6=308(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-263/237, 4-6=-263/237

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Endl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=215, 6=215.



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December 23, 2019

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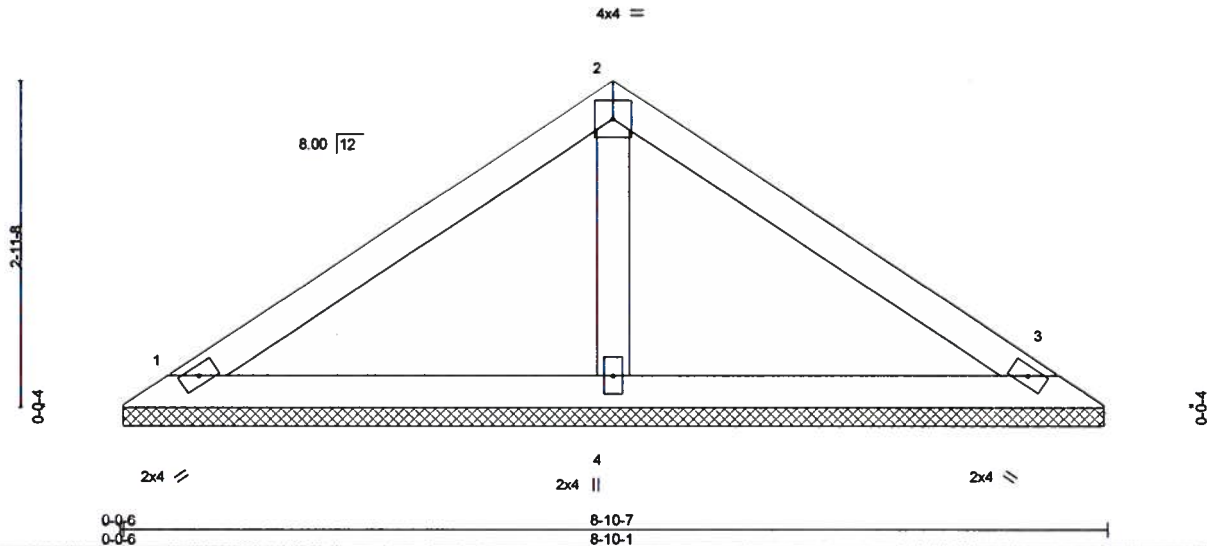
Job 2150579	Truss V07	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976622
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:58 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNH0t-WHjhUjNNIEM8?xm17K4KITWwho6oZB0PusXawtXy64n?



Scale = 1.20.0



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.18	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.15	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight: 31 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

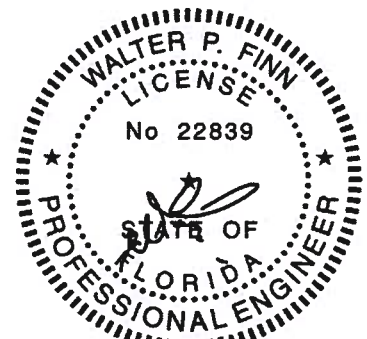
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=143/8-9-11, 3=143/8-9-11, 4=299/8-9-11
Max Horz 1=82(LC 8)
Max Uplift 1=64(LC 12), 3=75(LC 13), 4=82(LC 12)
Max Grav 1=143(LC 1), 3=144(LC 20), 4=299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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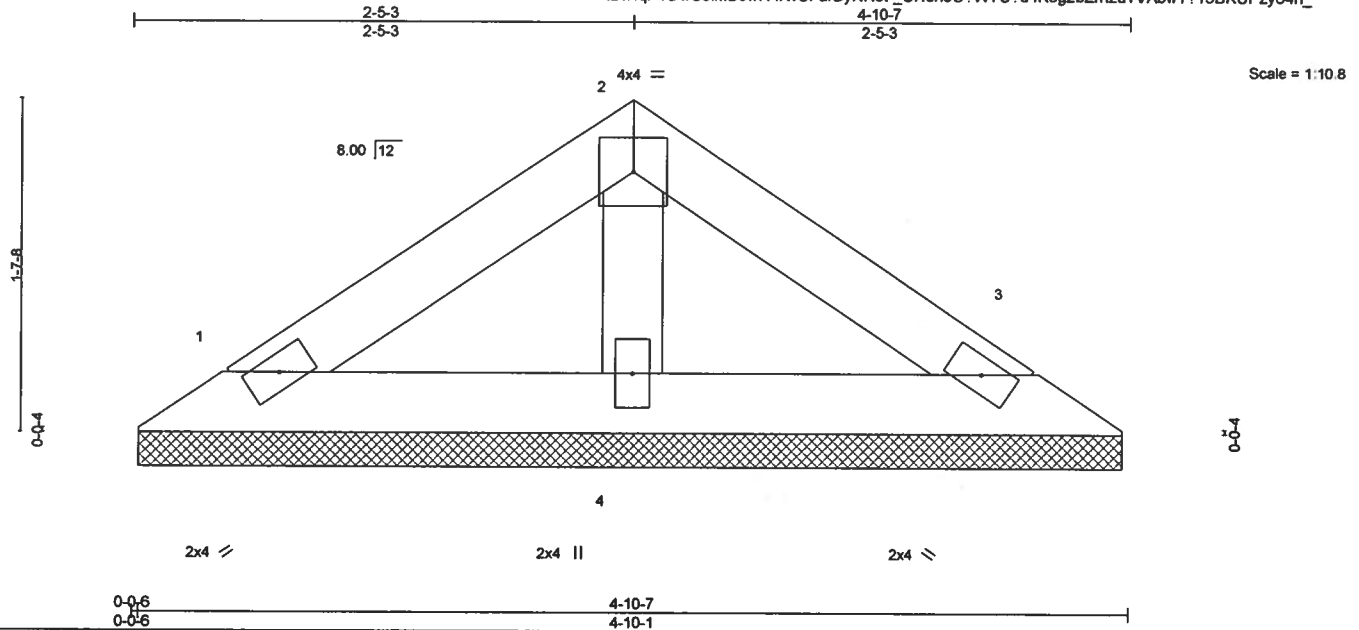


6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976623
2150579	V08	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:59 2019 Page 1
ID:HqPYDwS0lMBewR4XvSFurByNHol- UH3h3O?WYU?d4K3g2bZrh2uTVAbwT?15BKUPzy64n_



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.04	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 16 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

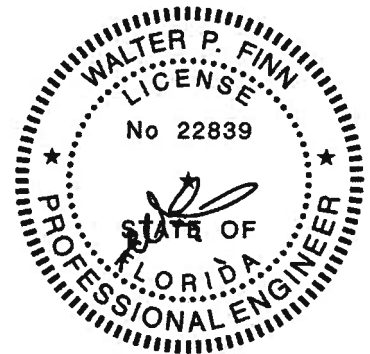
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-10-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=77/4-9-11, 3=77/4-9-11, 4=134/4-9-11
Max Horz 1=41(LC 10)
Max Uplift 1=38(LC 12), 3=43(LC 13), 4=28(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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December 23, 2019

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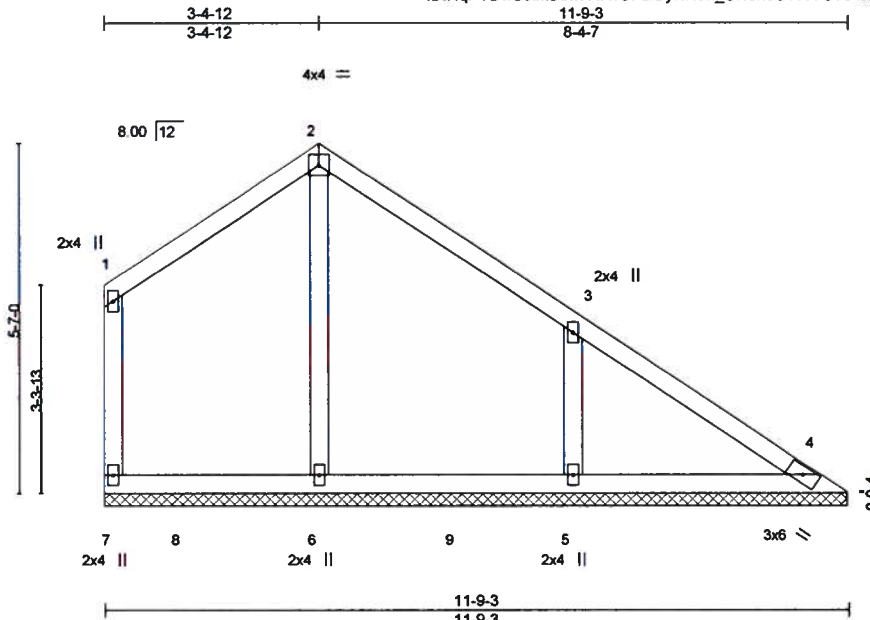
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Tampa, FL 33610

Job 2150579	Truss V09	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976624
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:23:59 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHot_UH3h3O7WYU?d4K3g2bZrh2rJV87wSo15BKUPzy64n_



Scale = 1:35.2

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.20	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.13	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.09	Horz(CT)	0.00	4	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 53 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

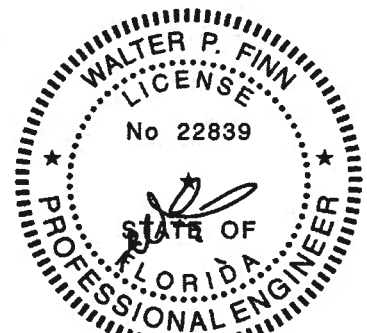
All bearings 11-8-13.
(lb) - Max Horz 7=196(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 7, 4, 6 except 5=267(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 7, 4 except 6=358(LC 20), 5=402(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-5=325/284

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4, 6 except (jt=lb) 5=267.



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December 23, 2019

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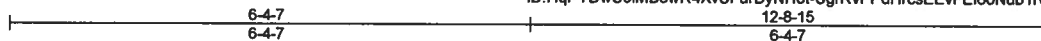
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976625
2150579	V10	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:00 2019 Page 1

ID:HqPYDwS0IMBewR4XvSFurByNH0t-SgrRvPPdHrcsEEvFEI6oNub1rvVXfvQAKr31wQy64mz



Scale = 1:27.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCCL 7.0	Plate Grip DOL 1.25	BC 0.12	Vert(LL) n/a - n/a 999		
BCCL 0.0	Lumber DOL 1.25	WB 0.07	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
	Code FBC2017/TPI2014			Weight: 48 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 12-8-3.
(lb) - Max Horz 1=122(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=215(LC 12), 6=215(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=307(LC 19), 6=307(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=263/237, 4-6=263/237

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=215, 6=215.



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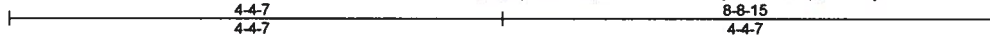
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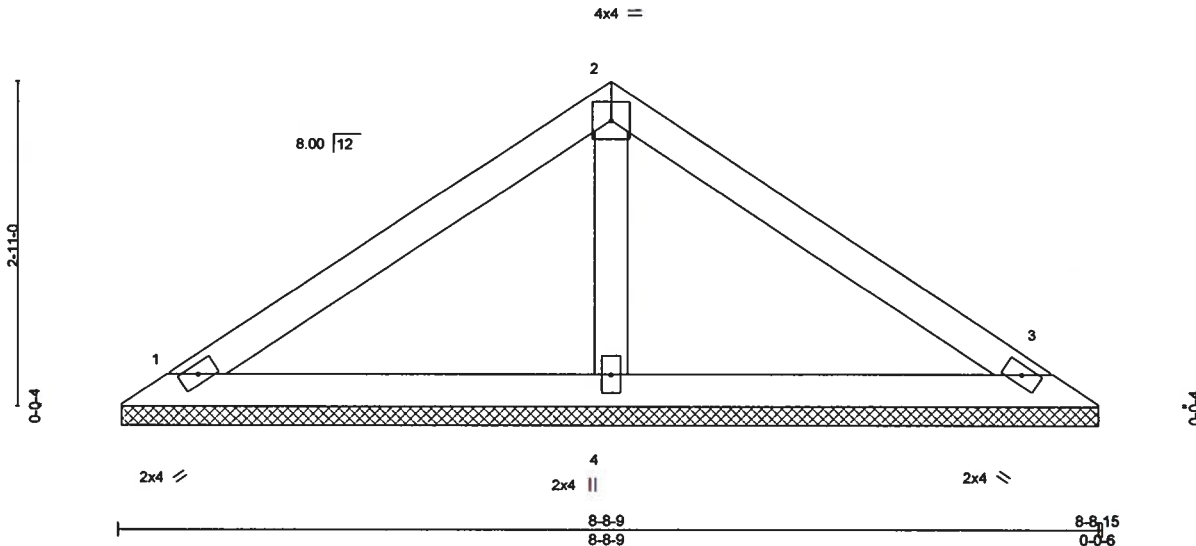
Job 2150579	Truss V11	Truss Type Valley	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976626
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:01 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHol-wsPg6iPG29kjsOUsoTd1w68C8JqLONAKZVpaTsy64my



Scale = 1:19.8



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.17	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.15	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight: 30 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

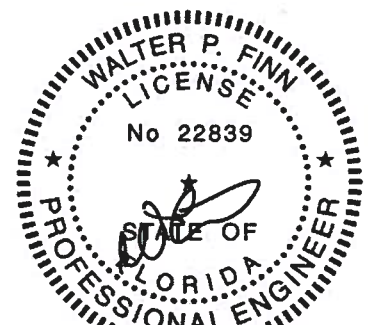
REACTIONS.

(lb/size) 1=141/8-8-3, 3=141/8-8-3, 4=294/8-8-3
Max Horz 1=81(LC 9)
Max Uplift 1=63(LC 12), 3=74(LC 13), 4=81(LC 12)
Max Grav 1=141(LC 1), 3=142(LC 20), 4=294(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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December 23, 2019

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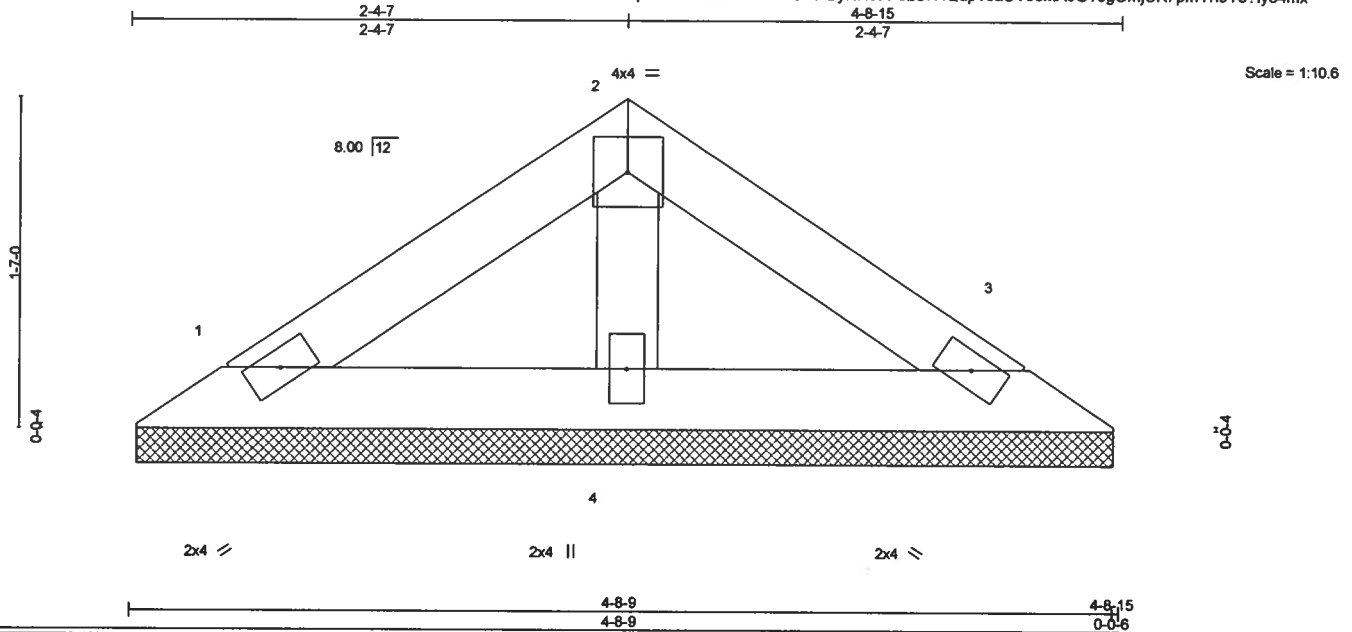
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Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976627
2150579	V12	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:02 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHol-P3zCK4QupTsaUY3eMA9GTJgOmjCK7pmTn9Y8?ly64mx



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.05	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.04	Vert(LL) n/a - n/a 999		
BCLL 0.0	Lumber DOL 1.25	WB 0.02	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code FBC2017/TPI2014			Weight: 15 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

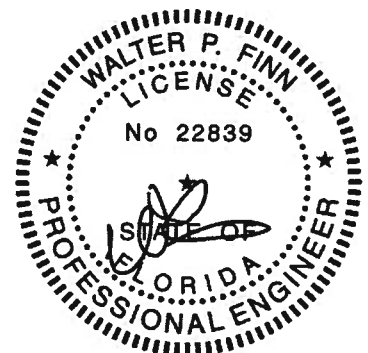
BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=75/4-8-3, 3=75/4-8-3, 4=130/4-8-3
Max Horz 1=39(LC 8)
Max Uplift 1=37(LC 12), 3=42(LC 13), 4=27(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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December 23, 2019

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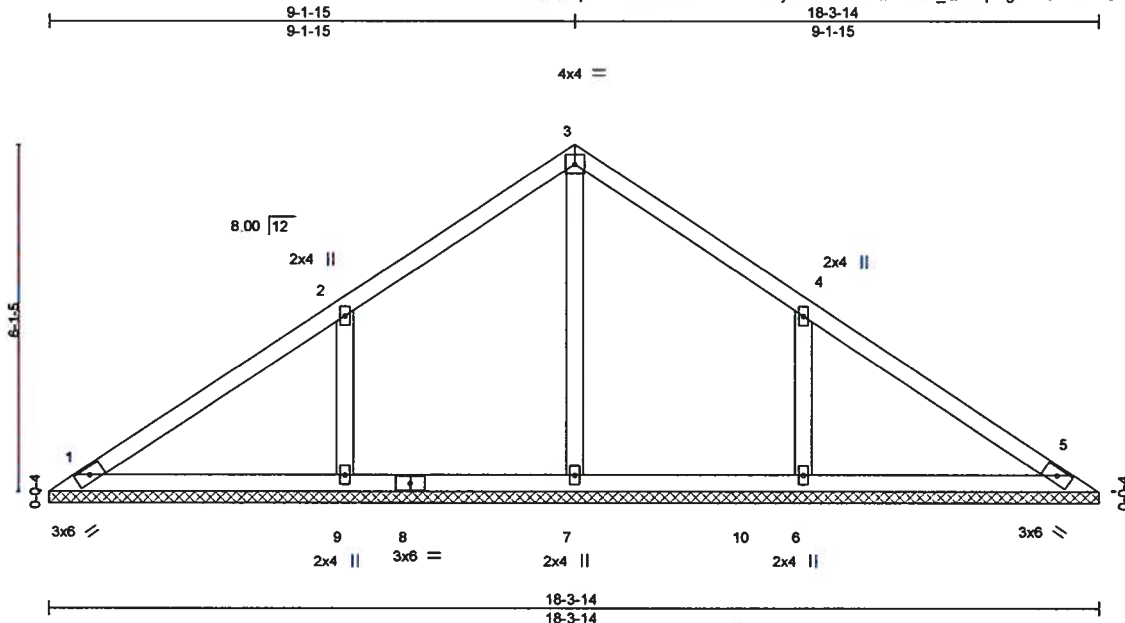
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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	BLAKE CONST. - LUNDE RES.	T18976628
2150579	V13	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:03 2019 Page 1
ID:HqPYDwSOIMBewR4XvSFurByNHol-tFXaXQRWam_Q5ieqvugV?XDWh7WSsFod0phXky64mw



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.23	Vert(LL)	n/a	-	n/a	999	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.17	Vert(CT)	n/a	-	n/a	999	
BCLL 0.0	Rep Stress Incr	YES	WB 0.09	Horz(CT)	0.00	5	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 75 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

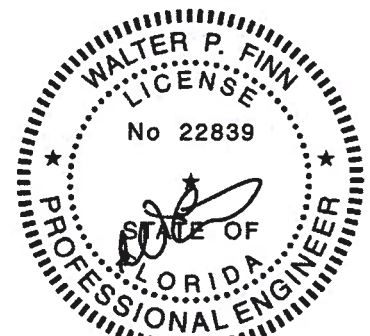
All bearings 18-3-14.
(lb) - Max Horz 1=180(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=298(LC 12), 6=298(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=335(LC 22), 9=462(LC 19), 6=464(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=353/315, 4-6=353/315

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (it=lb) 9=298, 6=298.



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December 23, 2019

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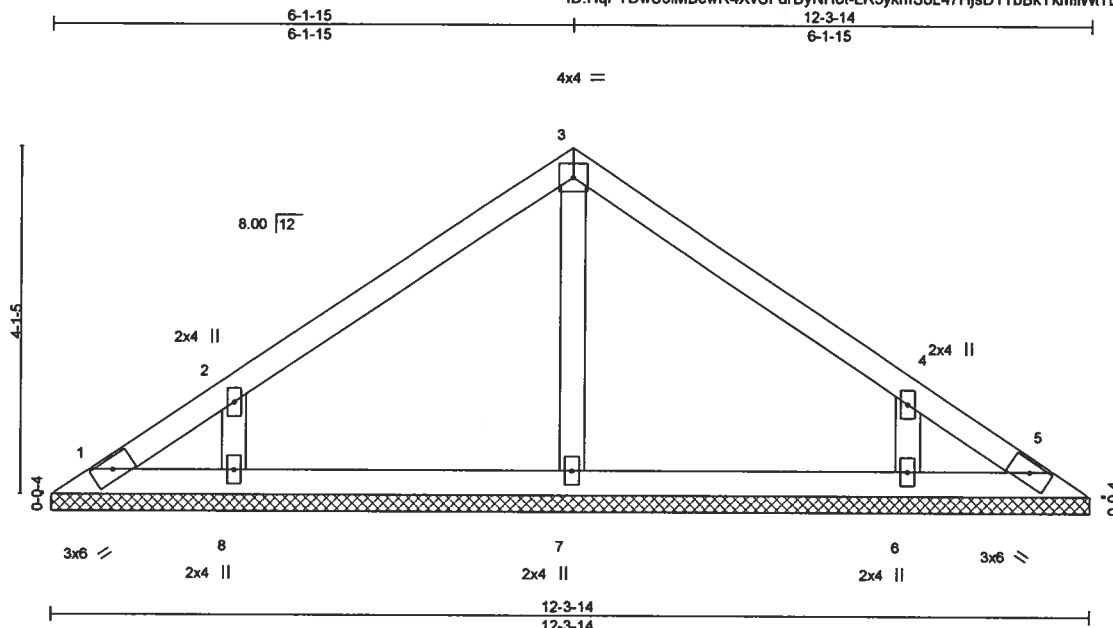


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Job 2150579	Truss V14	Truss Type GABLE	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976629
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:04 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHot-LR5ykmS8L47HjsD1TbBkYkmiWTbiPmFT1F3By64mv
12-3-14
6-1-15



Scale = 1:26.3

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.12	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight: 46 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

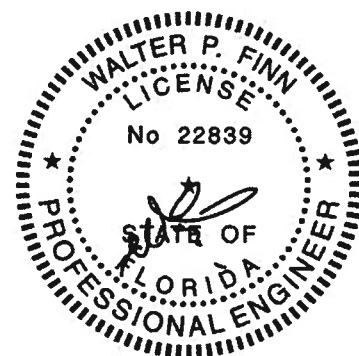
- All bearings 12-3-14.
(lb) - Max Horz 1=118(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=214(LC 12), 6=214(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=305(LC 19), 6=305(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=263/238, 4-6=263/237

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=214, 6=214.



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Date:

December 23,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

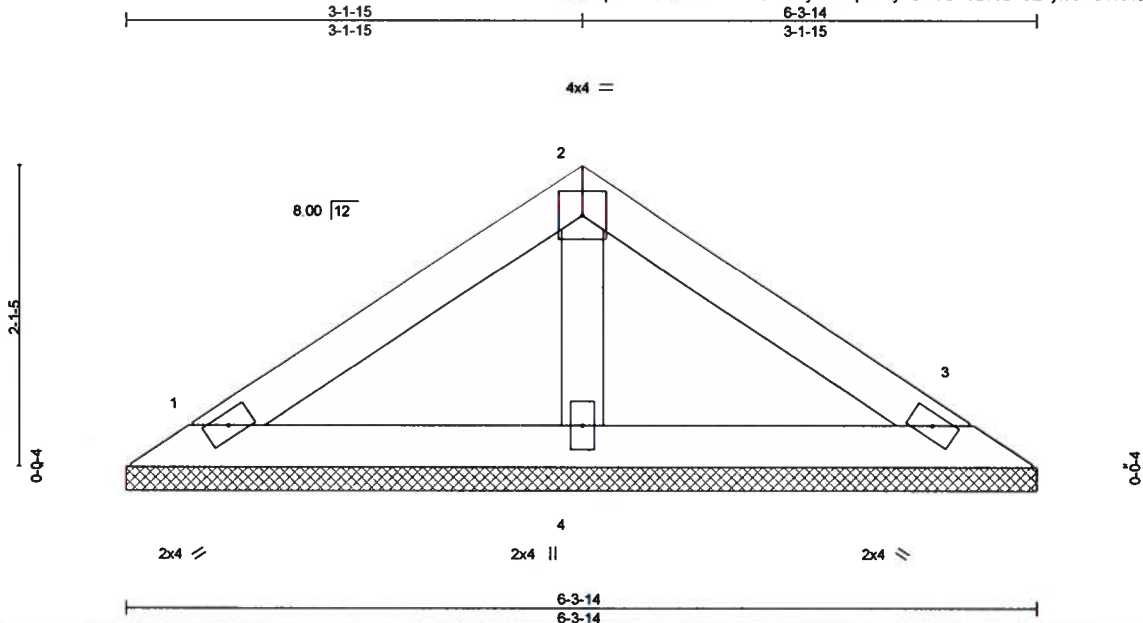
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Tampa, FL 33610

Job 2150579	Truss V15	Truss Type GABLE	Qty 1	Ply 1	BLAKE CONST. - LUNDE RES. Job Reference (optional)	T18976630
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Dec 6 2019 MiTek Industries, Inc. Mon Dec 23 12:24:05 2019 Page 1
ID:HqPYDwS0IMBewR4XvSFurByNHol-peeKy6Sm6OF8L7oD1Jiz4ylu3wCTKAOWT7nocdy64mu



Scale = 1:15.4

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.07	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 21 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=106/6-3-14, 3=106/6-3-14, 4=184/6-3-14
Max Horz 1=56(LC 11)
Max Uplift 1=52(LC 12), 3=60(LC 13), 4=38(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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December 23, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

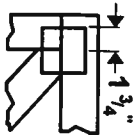
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

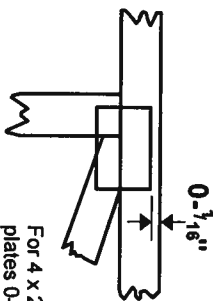
6904 Parke East Blvd.
Tampa, FL 33610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

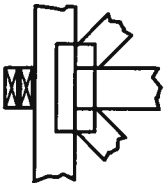
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



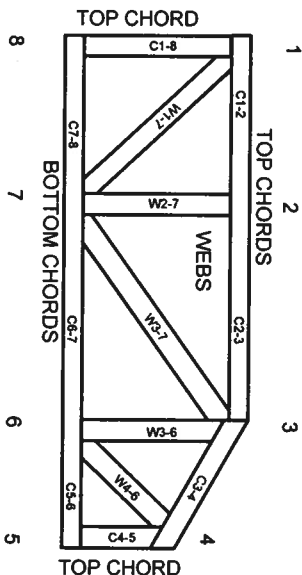
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

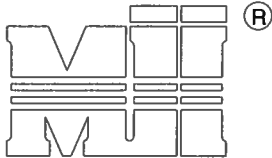
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft, spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.

AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2



MiTek USA, Inc. Page 1 of 1

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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern

T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss

Specified Continuous Rows of Lateral Bracing

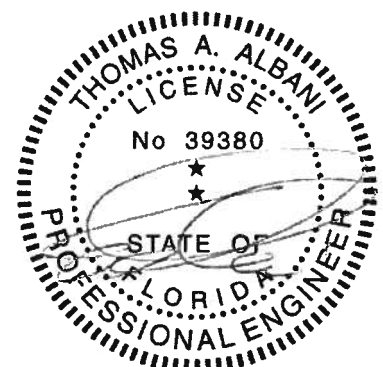
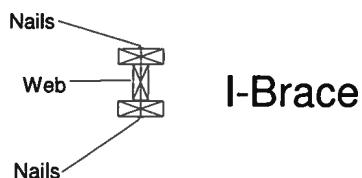
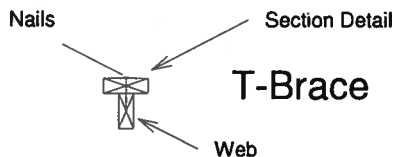
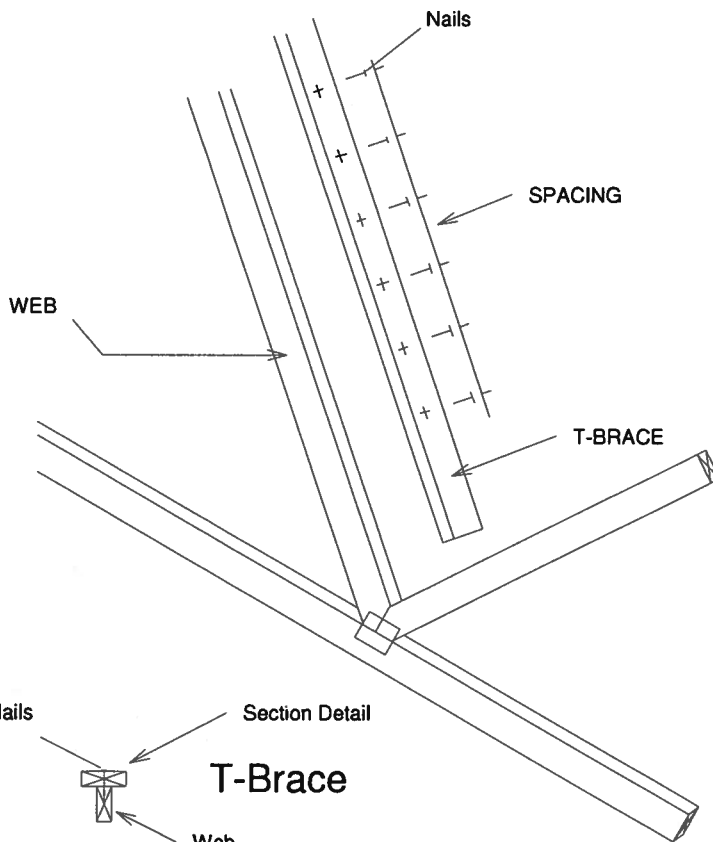
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss

Specified Continuous Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



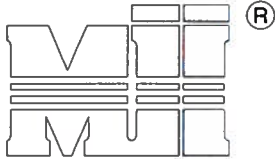
Thomas A. Albani PE No.39380
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Date:

February 12, 2018

AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE



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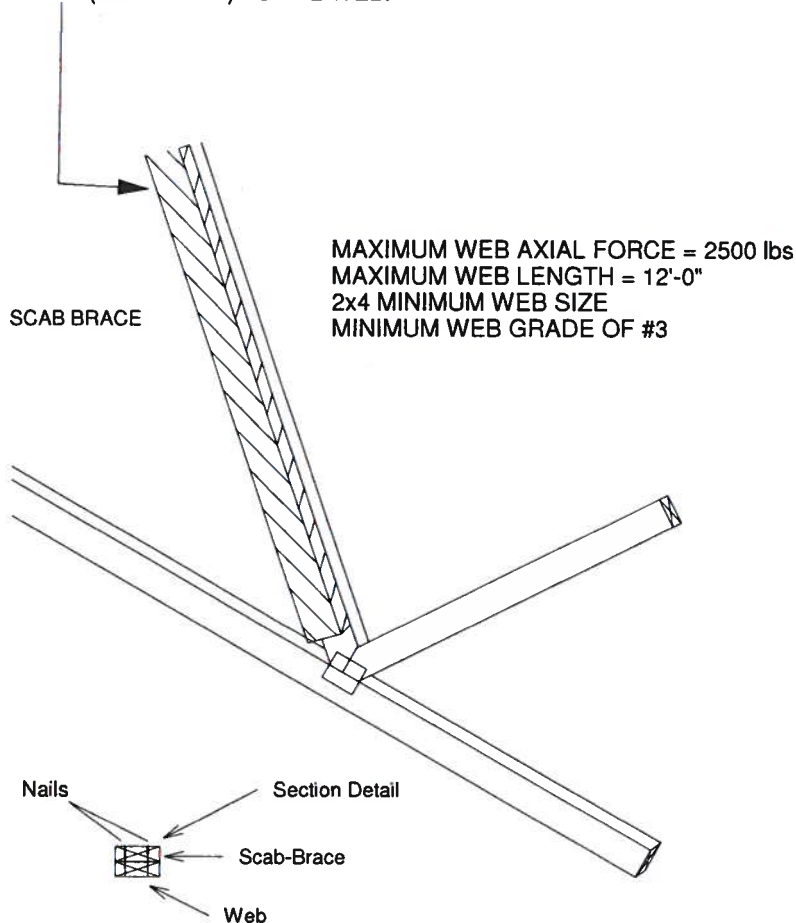
MiTek USA, Inc.

Page 1 of 1

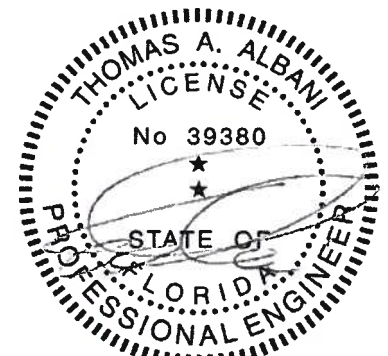
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APPLICABLE WHEN BRACING IS ***
REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



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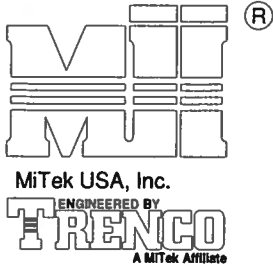
February 12, 2018

AUGUST 1, 2016

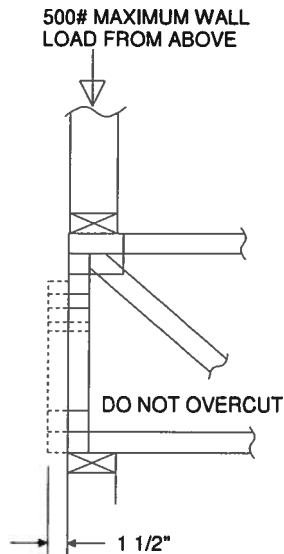
STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

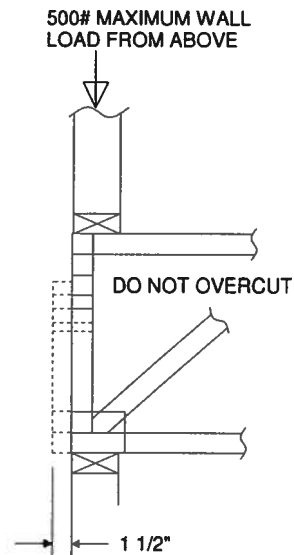
MiTek USA, Inc. Page 1 of 1



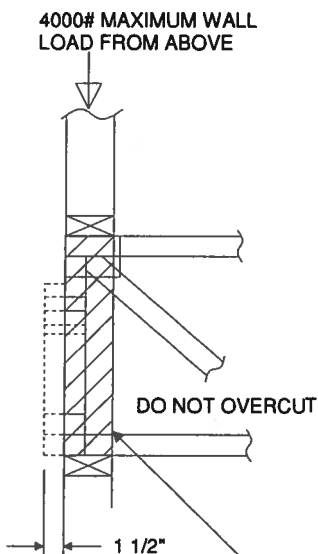
1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



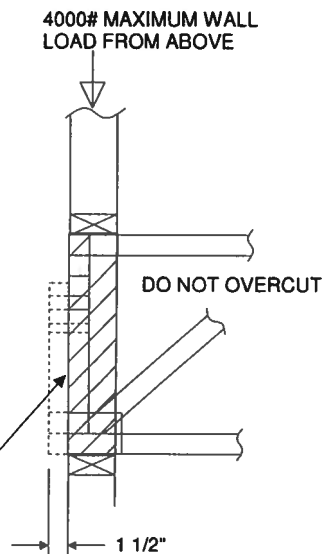
REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

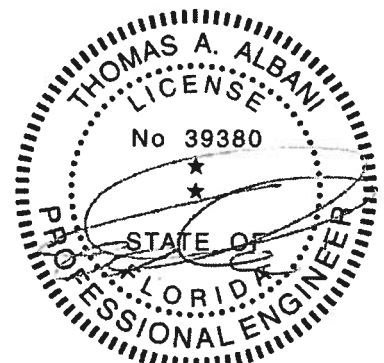


REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

ATTACH 2x4 SQUASH BLOCK (CUT TO FIT TIGHTLY) TO BOTH SIDES OF THE TRUSS AS SHOWN WITH 10d (0.131" X 3") NAILS SPACED 3" O.C.



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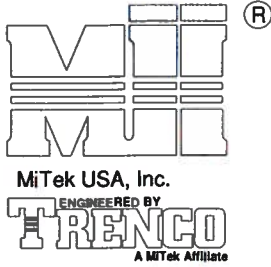
February 12, 2018

AUGUST 1, 2016

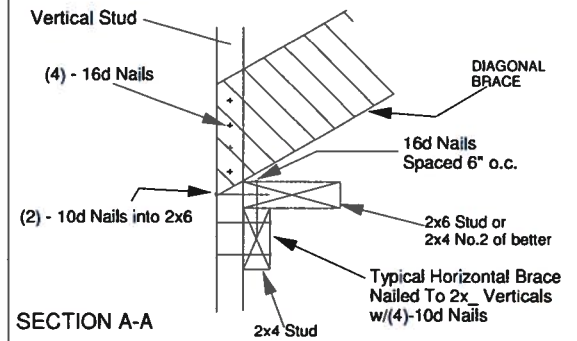
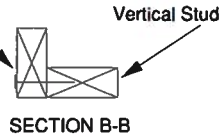
Standard Gable End Detail

MII-GE130-D-SP

MiTek USA, Inc. Page 1 of 2



Typical $\frac{1}{4}$ L-Brace Nailed To
2x Verticals W/10d Nails spaced 6" o.c.

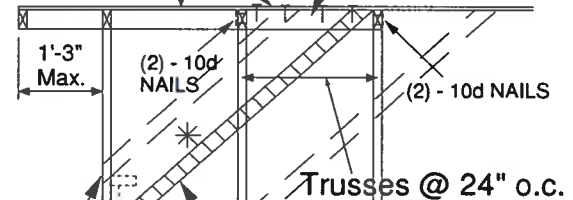


TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST
TWO TRUSSES AS NOTED. TOENAIL BLOCKING
TO TRUSSES WITH (2) - 10d NAILS AT EACH END.
ATTACH DIAGONAL BRACE TO BLOCKING WITH
(5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD
SHEATHING TO 2x4 STD SPF BLOCK

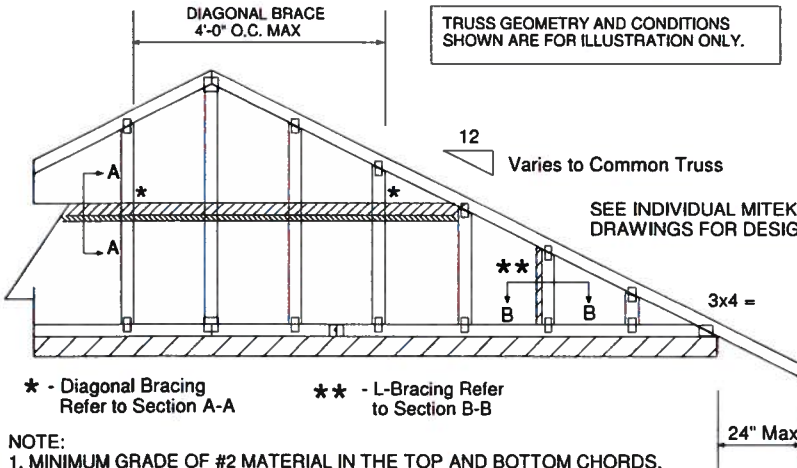
Roof Sheathing



Diag. Brace
at 1/3 points
if needed

End Wall

HORIZONTAL BRACE
(SEE SECTION A-A)



* - Diagonal Bracing
Refer to Section A-A

** - L-Bracing Refer
to Section B-B

NOTE:

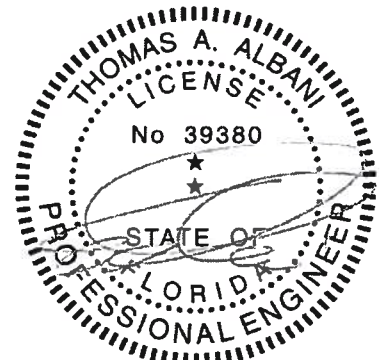
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS $L/240$.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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February 12, 2018

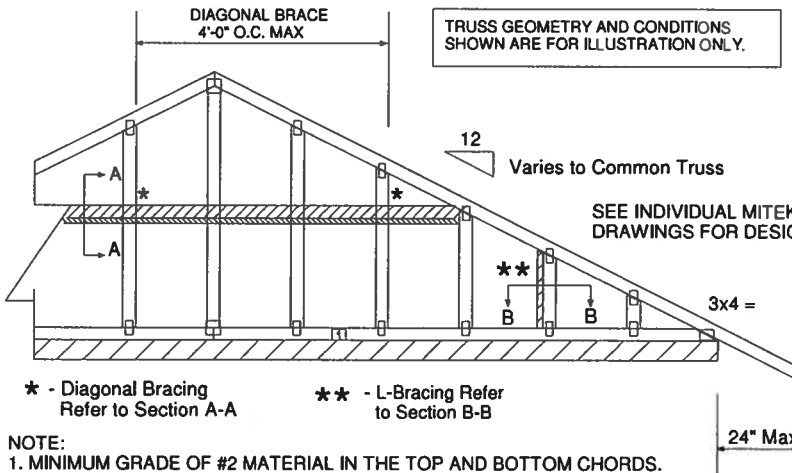
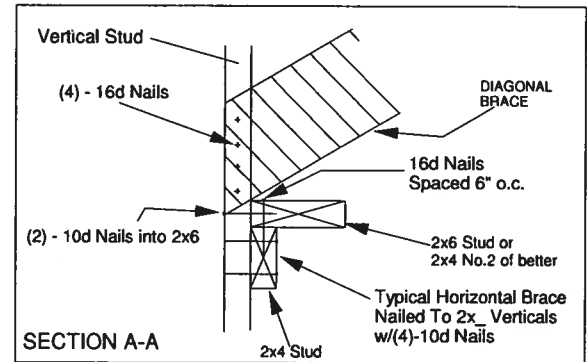
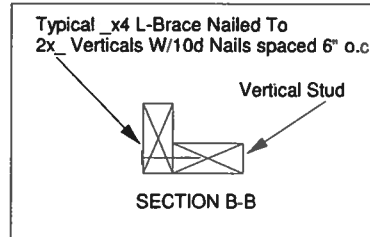
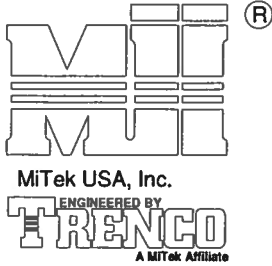
AUGUST 1, 2016

Standard Gable End Detail

MII-GE130-SP

MITek USA, Inc.

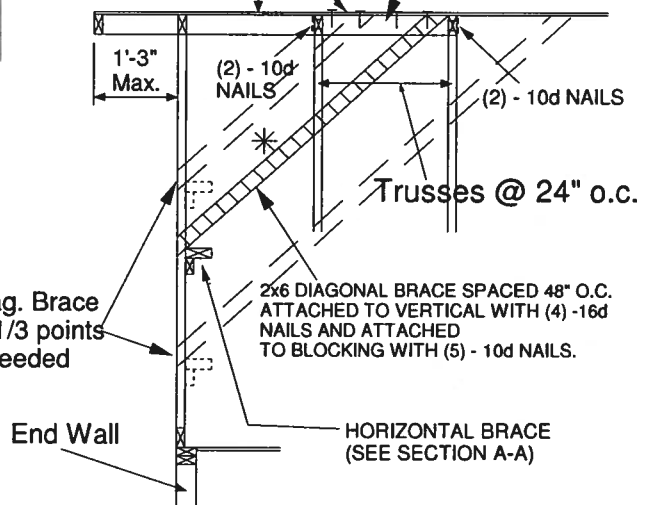
Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing



NOTE:

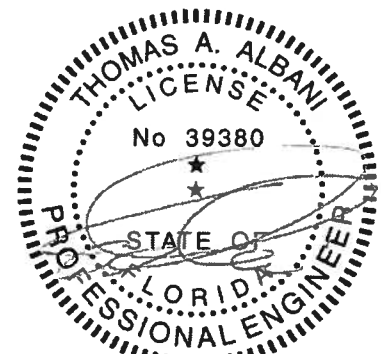
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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MITek USA, Inc. FL Cert 6634
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Date:

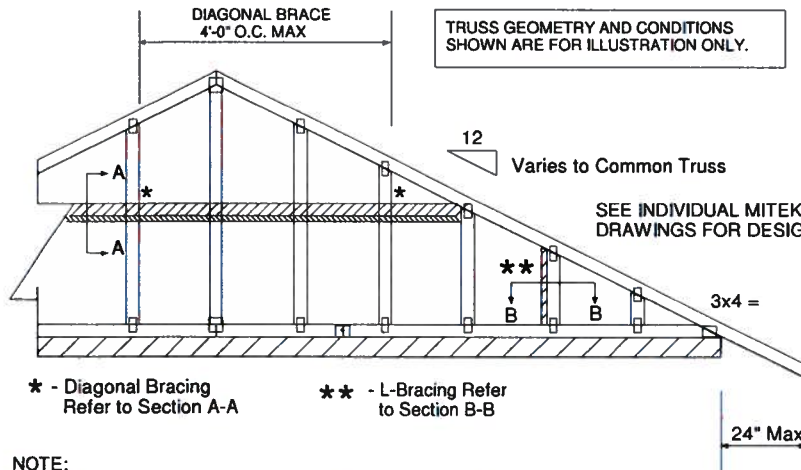
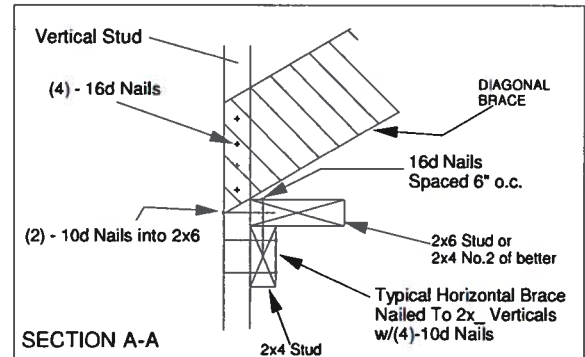
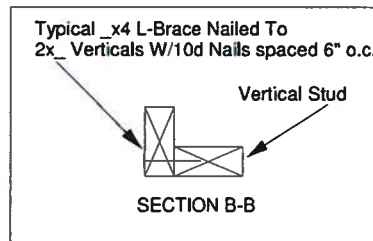
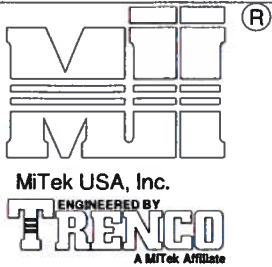
February 12, 2018

JANUARY 6, 2017

Standard Gable End Detail

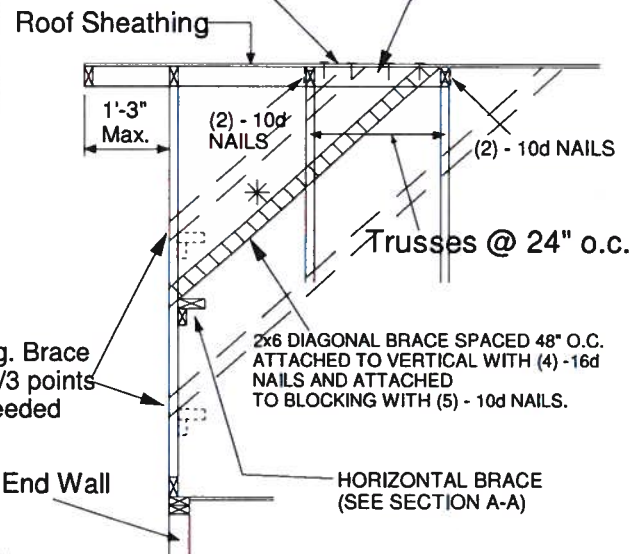
MII-GE140-001

MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK



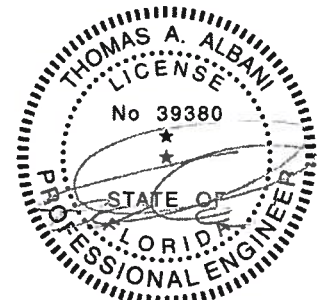
- NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

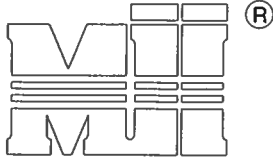
MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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Date:

January 19, 2018



MiTek USA, Inc.

ENGINEERED BY
TRENCO
A MiTek AffiliateTypical 2x4 L-Brace Nailed To
2x4 Verticals w/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

SEE INDIVIDUAL MITTEK ENGINEERING
DRAWINGS FOR DESIGN CRITERIA

3x4 =

24" Max

* - Diagonal Bracing
Refer to Section A-A** - L-Bracing Refer
to Section B-B

NOTE:

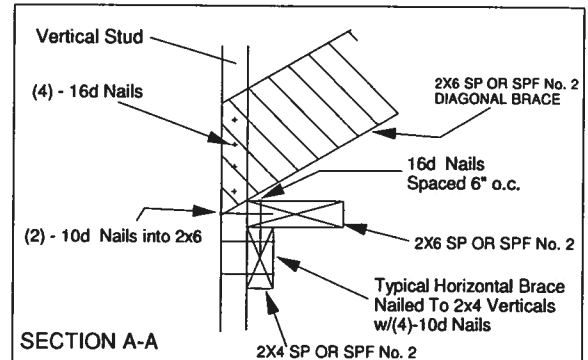
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 170 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



SECTION A-A

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-0" Max.

(2) - 10d NAILS

(2) - 10d NAILS

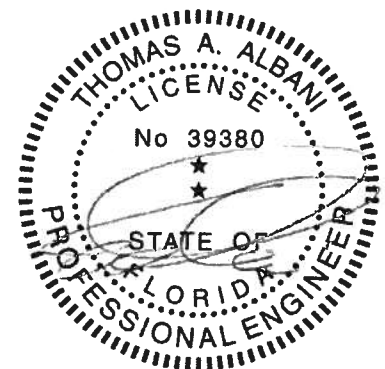
Trusses @ 24" o.c.

Diag. Brace at 1/3 points if needed

End Wall

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) - 16d NAILS, AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE (SEE SECTION A-A)



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Date:

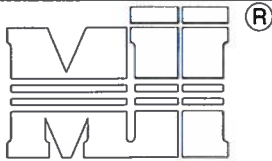
February 12, 2018

AUGUST 1, 2016

Standard Gable End Detail

MII-GE180-D-SP

MiTek USA, Inc. Page 1 of 2



MiTek USA, Inc.

ENGINEERED BY
TRENCOA MiTek Affiliate
DIAGONAL BRACE
4'-0" O.C. MAXTypical 2x4 L-Brace Nailed To
2x4 Verticals W/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

SEE INDIVIDUAL MITTEK ENGINEERING
DRAWINGS FOR DESIGN CRITERIA

3x4 =

24" Max

* - Diagonal Bracing
Refer to Section A-A** - L-Bracing Refer
to Section B-B

NOTE:

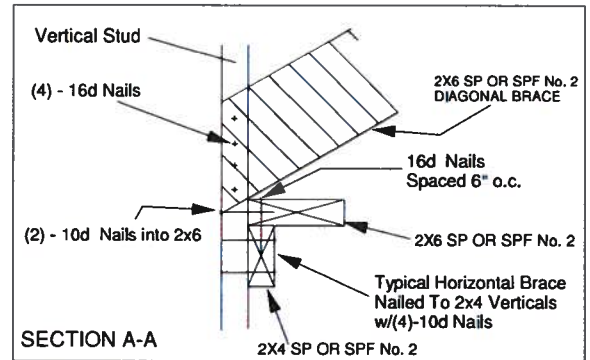
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 180 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



SECTION A-A

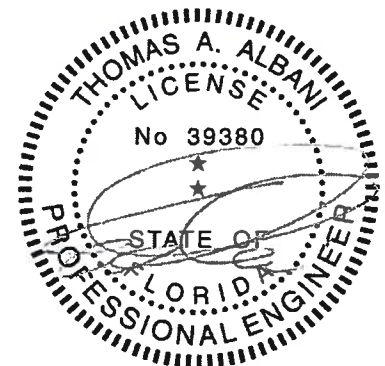
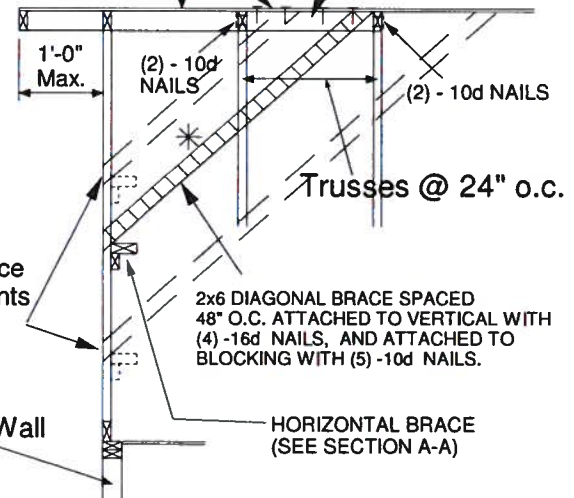
PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

Diag. Brace at 1/3 points if needed

End Wall



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Date:

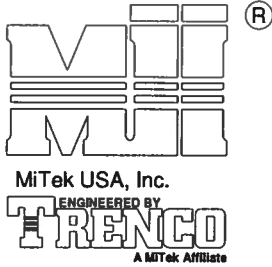
February 12, 2018

AUGUST 1, 2016

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-10

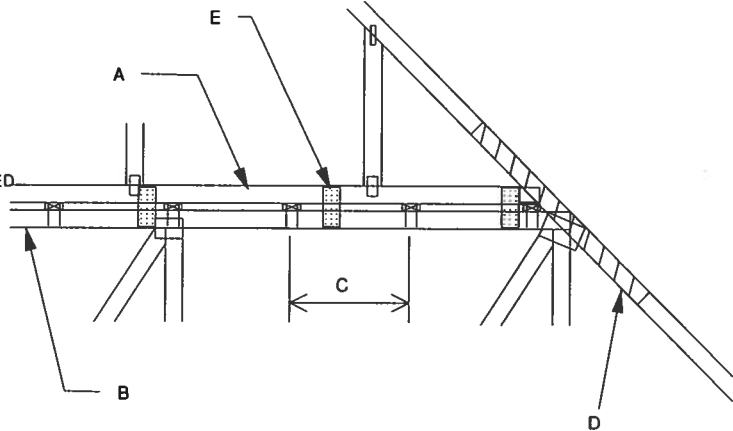
MiTek USA, Inc. Page 1 of 1



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

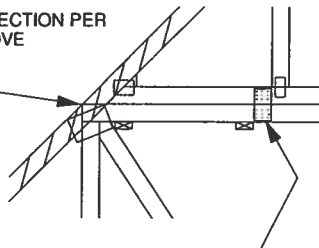
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X $\frac{1}{2}$ " X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



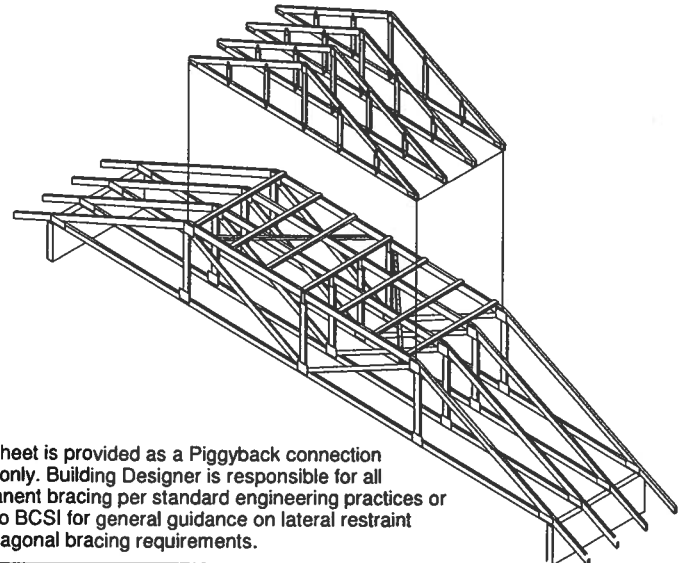
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

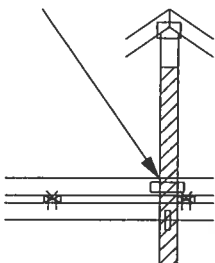


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



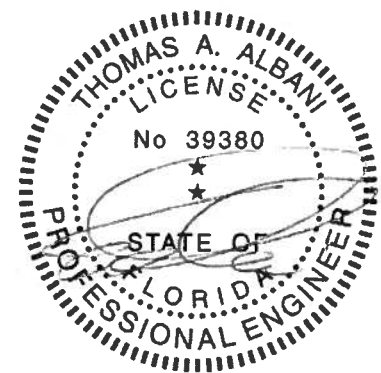
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x $\frac{1}{2}$ " X 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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6904 Parke East Blvd. Tampa FL 33610
Date:

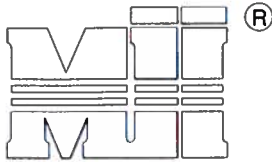
February 12, 2018

AUGUST 1, 2016

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT
7-10

MITek USA, Inc. Page 1 of 1



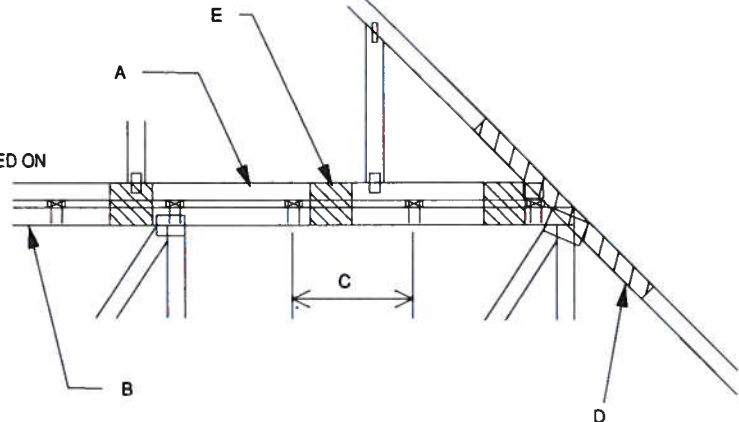
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MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

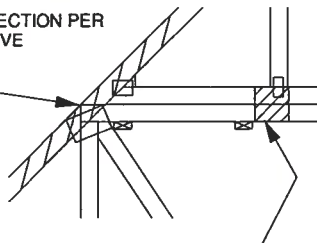
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0(0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



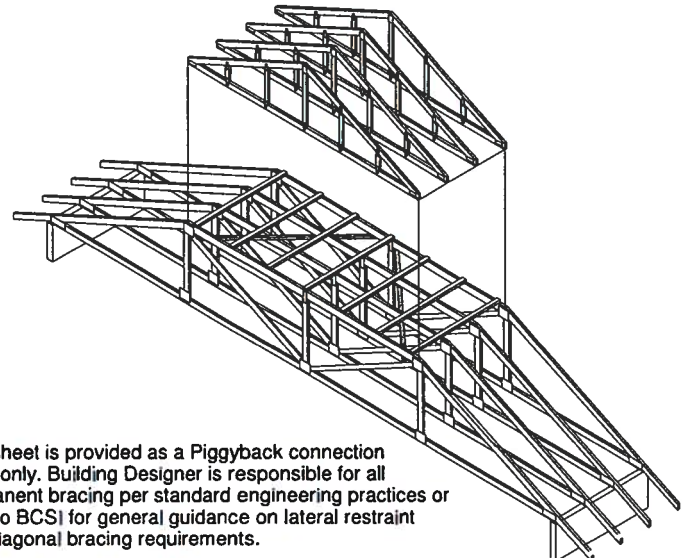
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

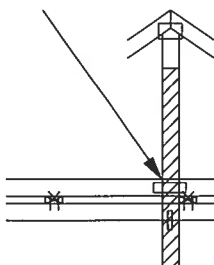


7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



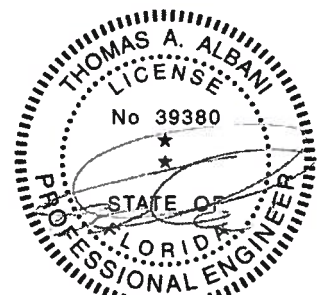
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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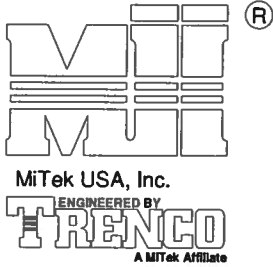
January 19, 2018

AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS
AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc. Page 1 of 1



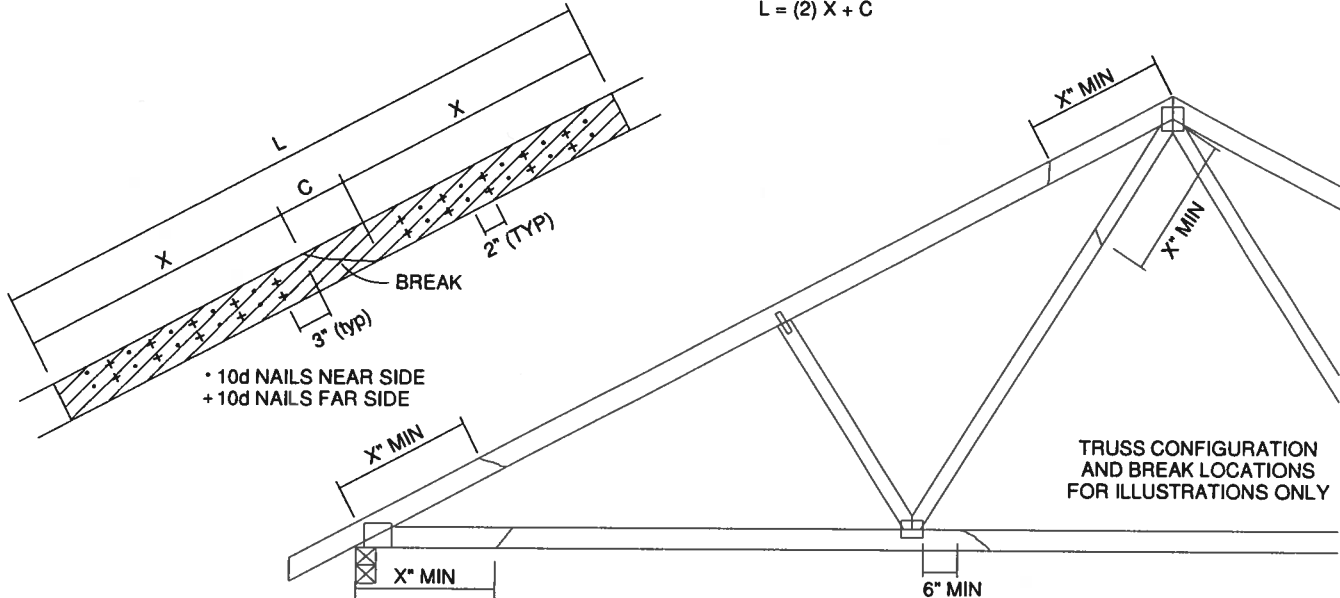
TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH
FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS
(TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.
STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C.
SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:

$$L = (2) X + C$$

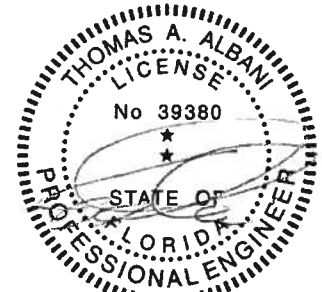


THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY
PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

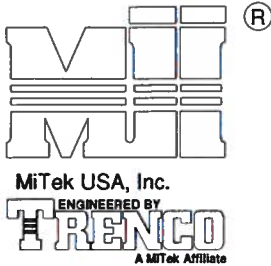
NOTES:

1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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January 19, 2018



NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN).
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE
THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
ILLUSTRATION PURPOSES ONLY

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)						
	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

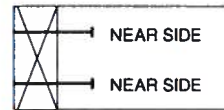
EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

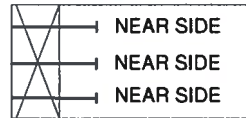
For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

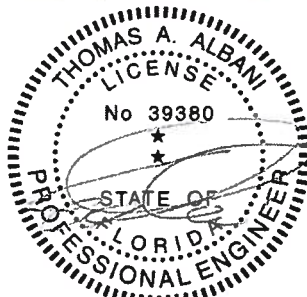
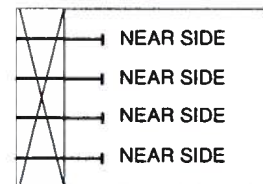
SIDE VIEW
(2x3)
2 NAILS



SIDE VIEW
(2x4)
3 NAILS

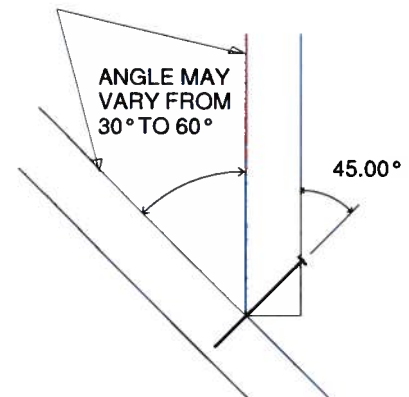
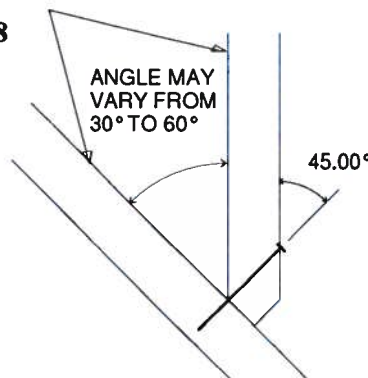
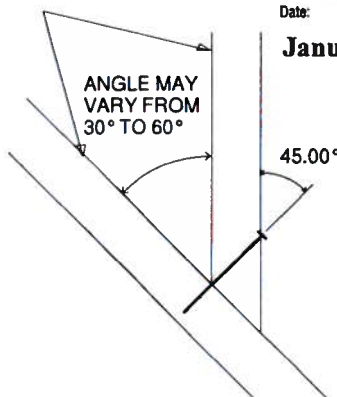


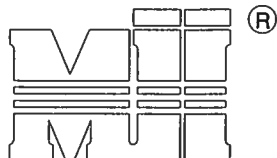
SIDE VIEW
(2x6)
4 NAILS



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January 19, 2018

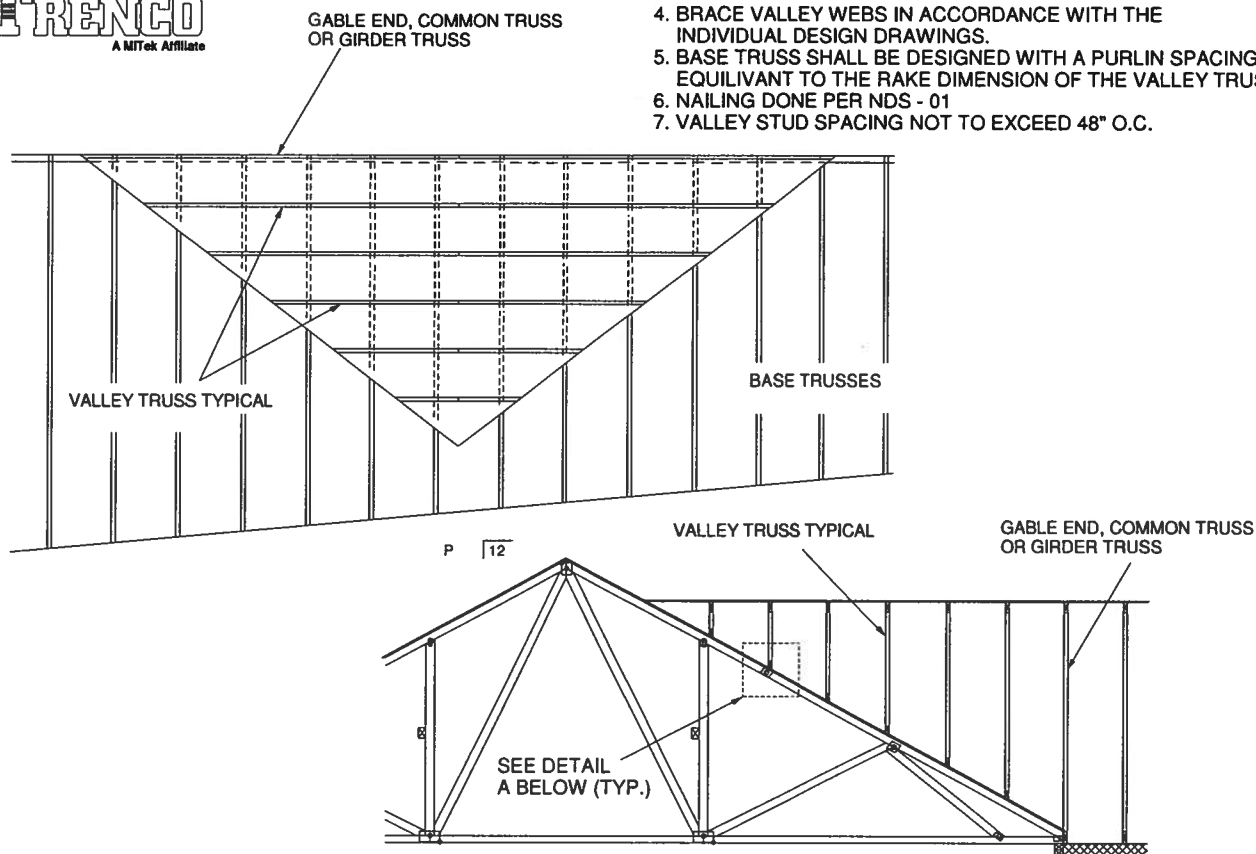




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ENGINEERED BY
TRENCO
A MiTek Affiliate

GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

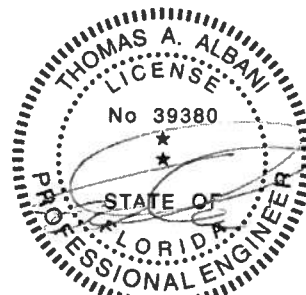


SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.

ATTACH 2x4 CONTINUOUS NO.2 SP
TO THE ROOF W/ TWO USP WS3 (1/4" X 3")
WOOD SCREWS INTO EACH BASE TRUSS.

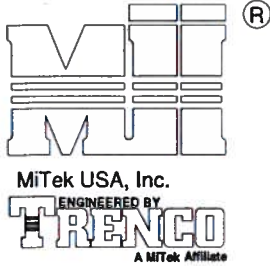
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES

DETAIL A
(NO SHEATHING)
N.T.S.



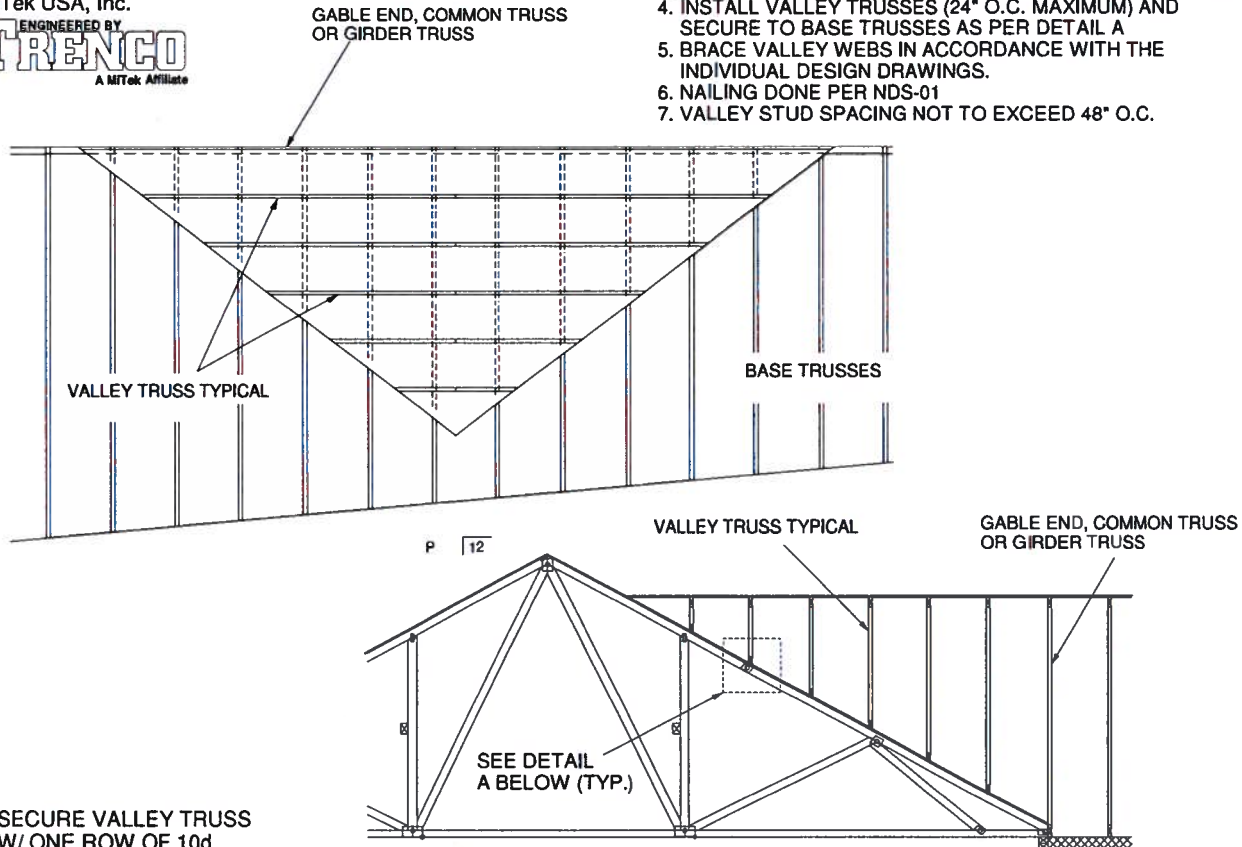
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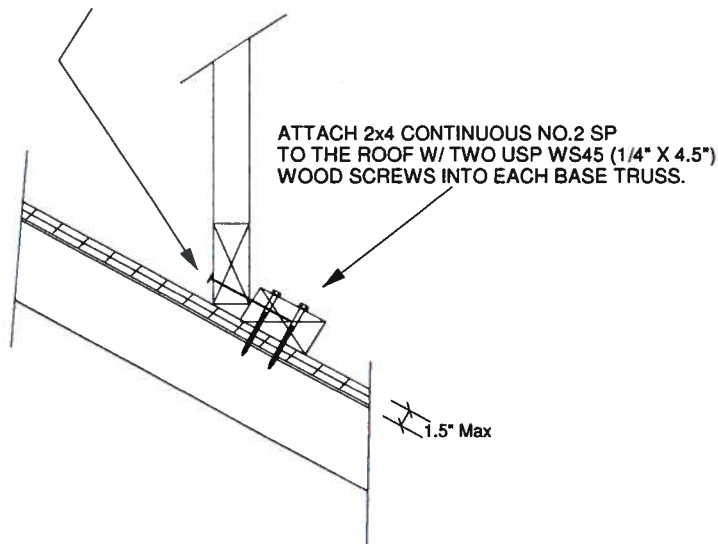


GENERAL SPECIFICATIONS

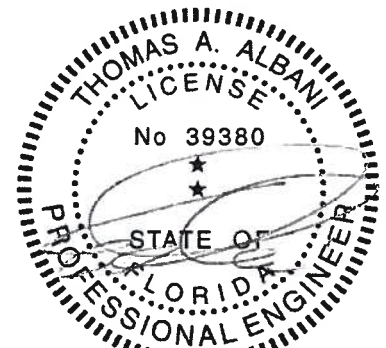
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
6. NAILING DONE PER NDS-01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



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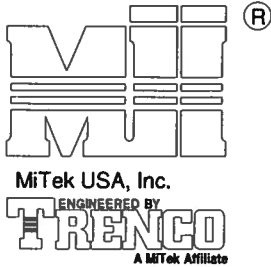
AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL

MII-VALLEY SP

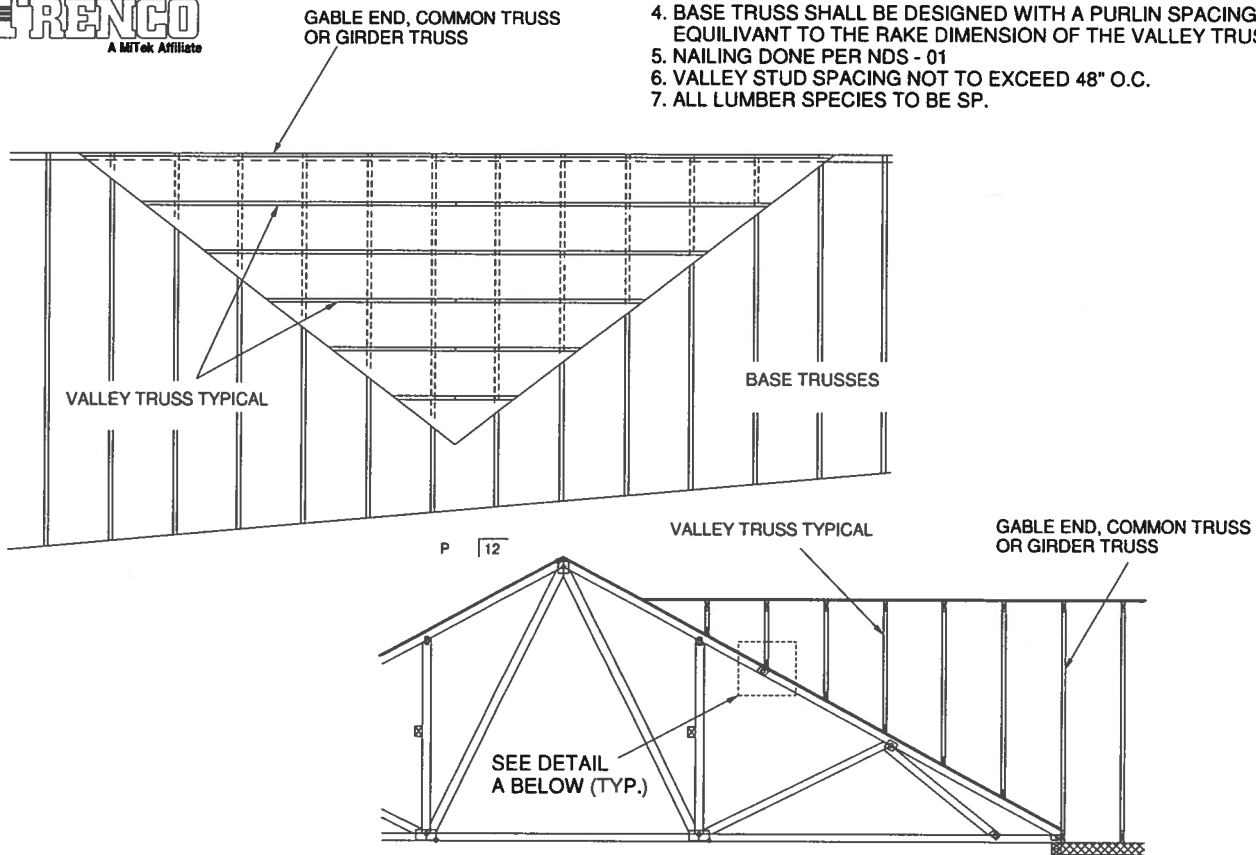
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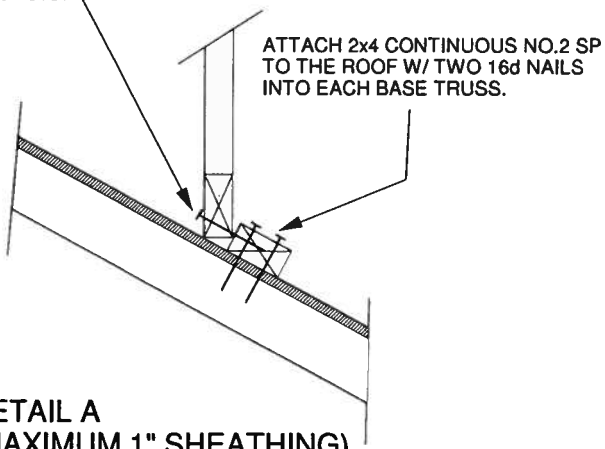


GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVARIANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP.

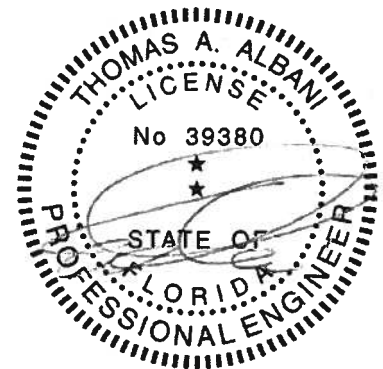


SECURE VALLEY TRUSS
W/ ONE ROW OF 16d
NAILS 6" O.C.



DETAIL A
(MAXIMUM 1" SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH
WIND DESIGN PER ASCE 7-10 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF
ON THE TRUSSES



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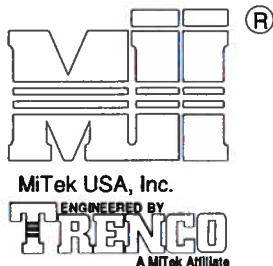
February 12, 2018

AUGUST 1, 2016

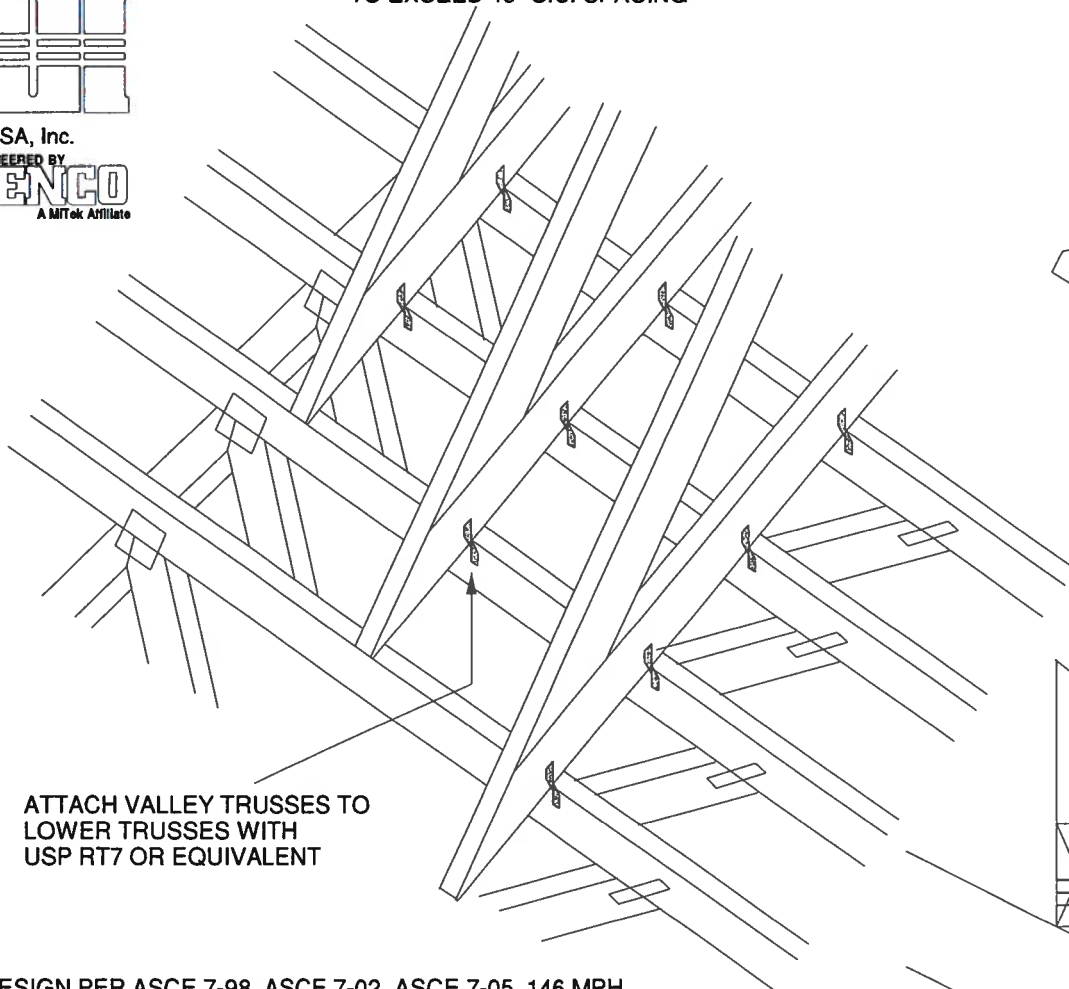
TRUSSED VALLEY SET DETAIL
(HIGH WIND VELOCITY)

MII-VALLEY

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NOTE: VALLEY STUD SPACING NOT
TO EXCEED 48" O.C. SPACING



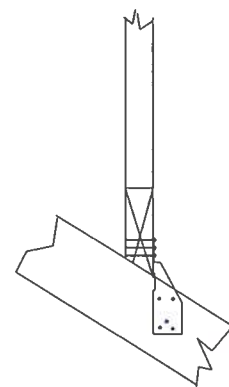
ATTACH VALLEY TRUSSES TO
LOWER TRUSSES WITH
USP RT7 OR EQUIVALENT

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
WIND DURATION OF LOAD INCREASE : 1.6
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)

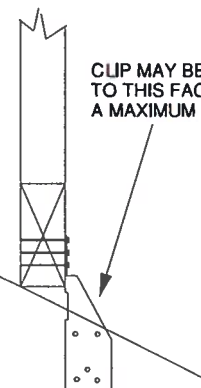
SUPPORTING TRUSSES DIRECTLY UNDER
VALLEY TRUSSES MUST BE DESIGNED
WITH A MAXIMUM UNBRACED LENGTH OF
2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER
INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR
SPF-S SPECIES LUMBER.

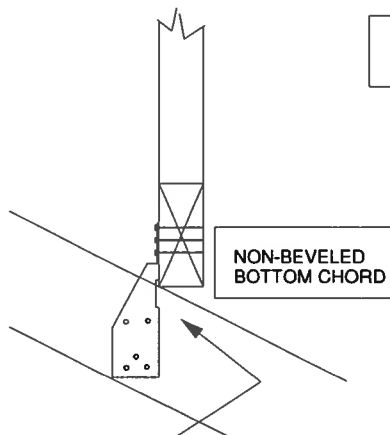


FOR BEVELED BOTTOM
CHORD, CLIP MAY BE
APPLIED TO EITHER FACE



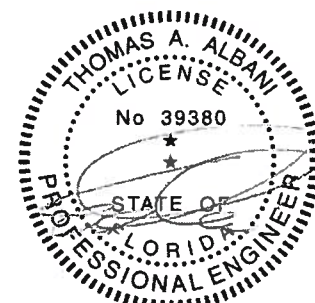
CLIP MAY BE APPLIED
TO THIS FACE UP TO
A MAXIMUM 6/12 PITCH

NON-BEVELED
BOTTOM CHORD



NON-BEVELED
BOTTOM CHORD

CLIP MUST BE APPLIED
TO THIS FACE WHEN
PITCH EXCEEDS 6/12.
(MAXIMUM 12/12 PITCH)



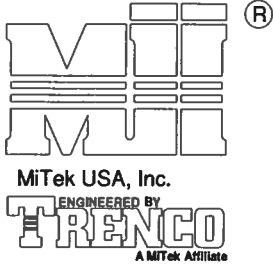
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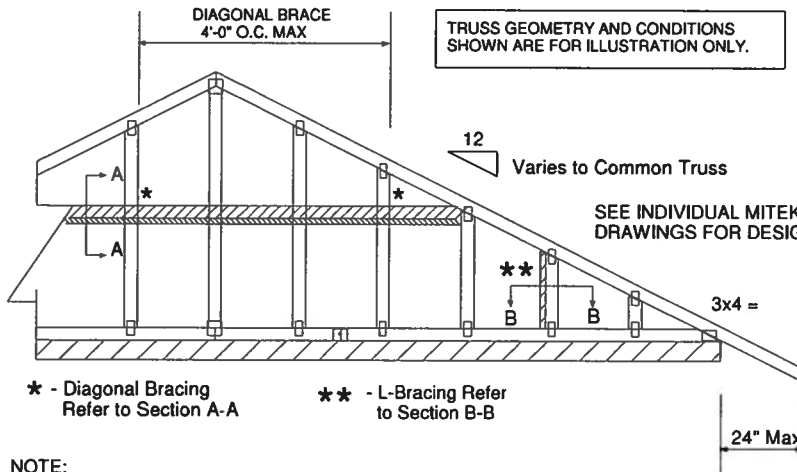
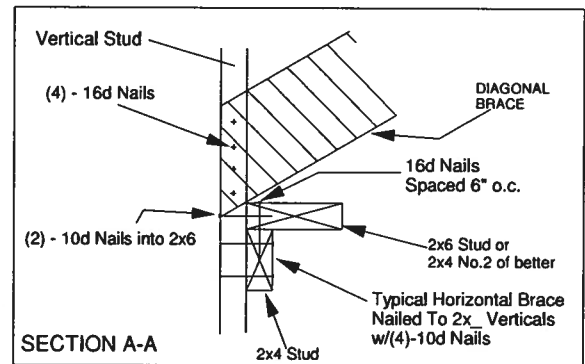
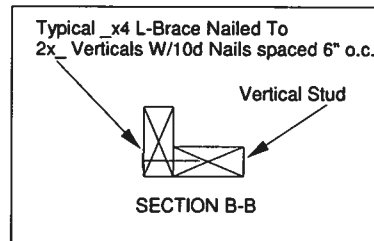
AUGUST 1, 2016

Standard Gable End Detail

MII-GE146-001



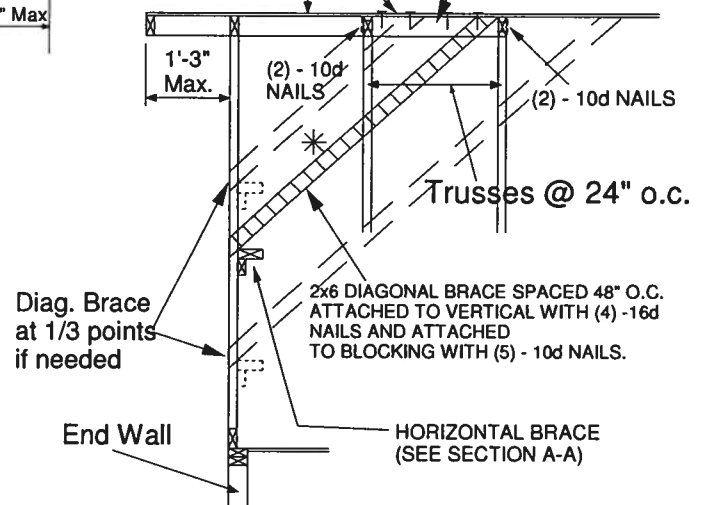
MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

Roof Sheathing



NOTE:

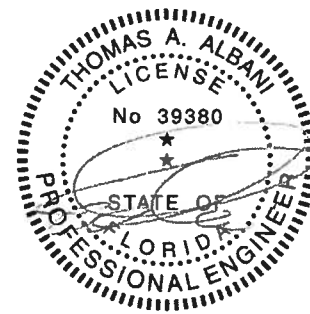
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

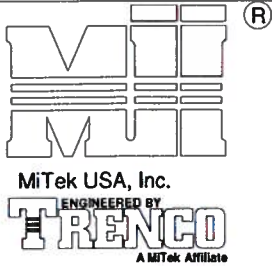
MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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January 19, 2018



TRUSS CRITERIA:

LOADING: 40-10-0-10

DURATION FACTOR: 1.15

SPACING: 24" O.C.

TOP CHORD: 2x4 OR 2x6

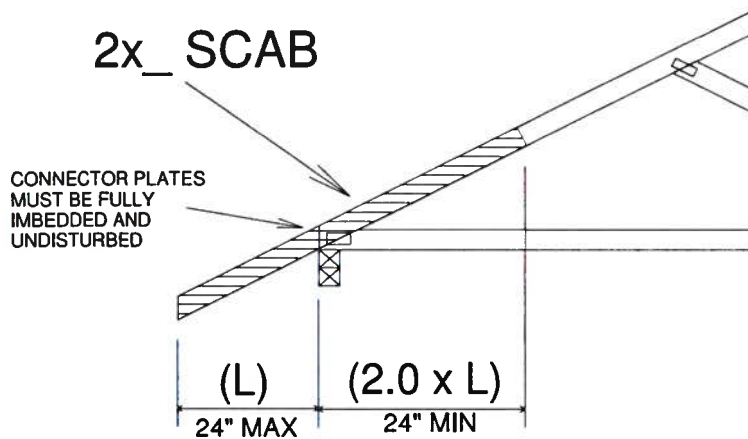
PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

NOTES:

1. ATTACH 2x SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

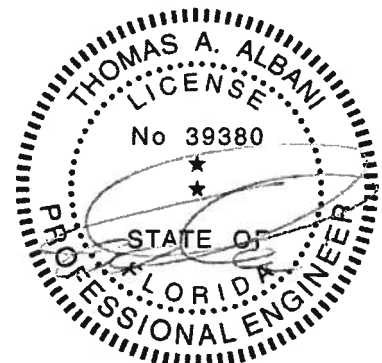


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES



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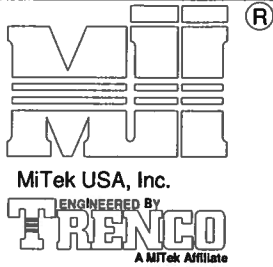
AUGUST 1, 2016

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

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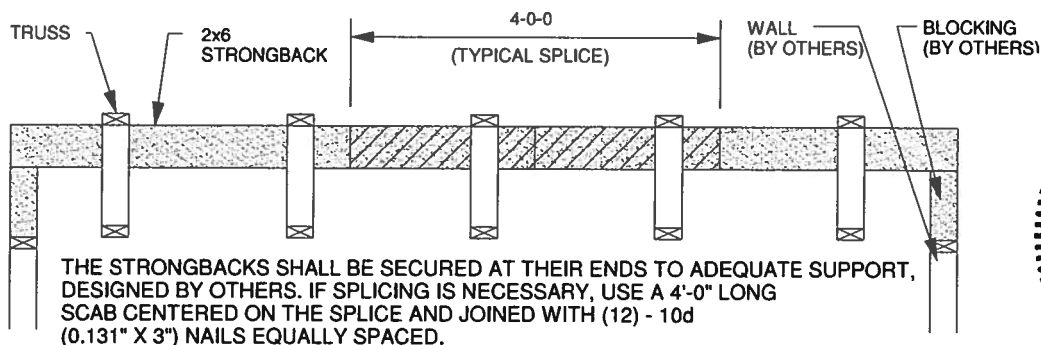
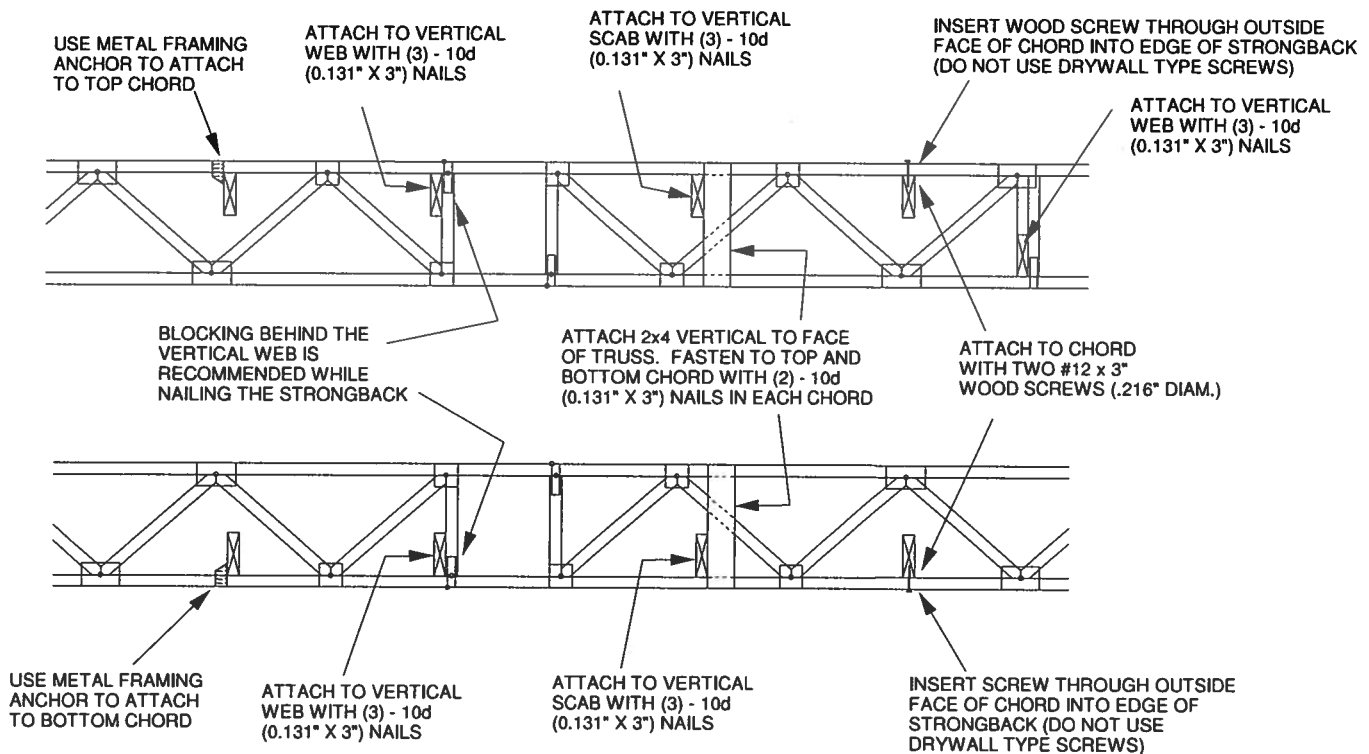
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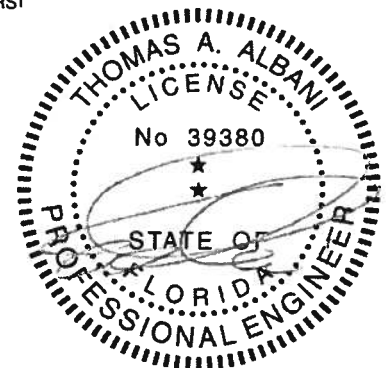
TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)



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