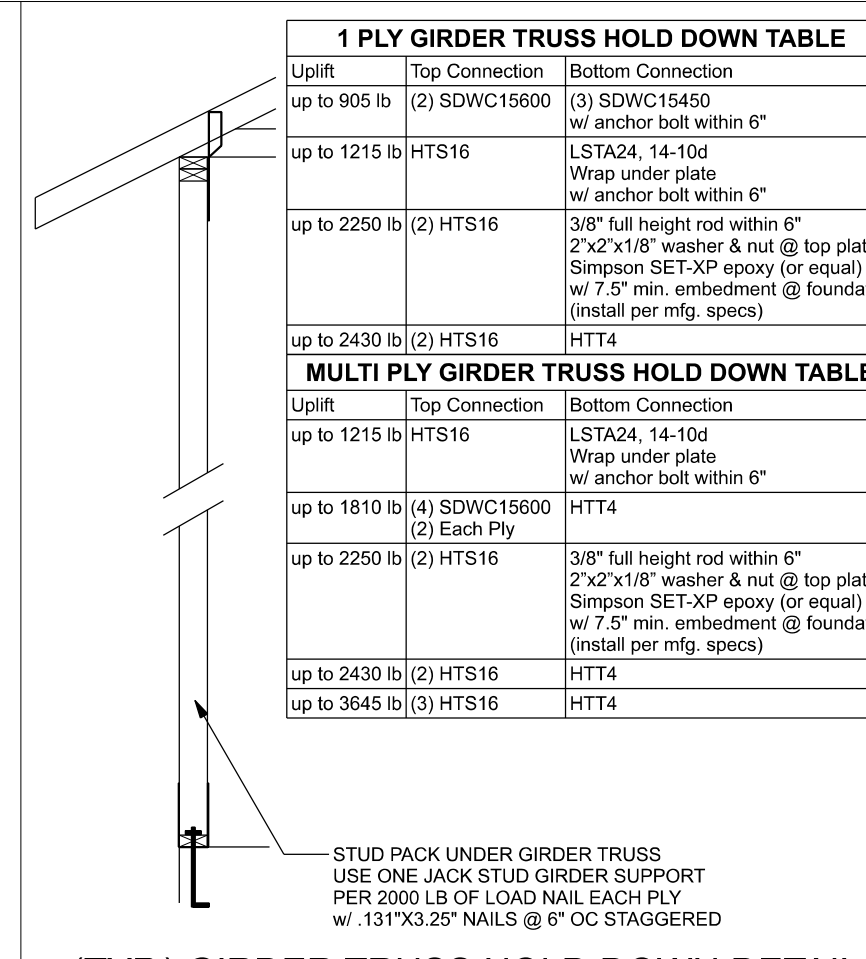
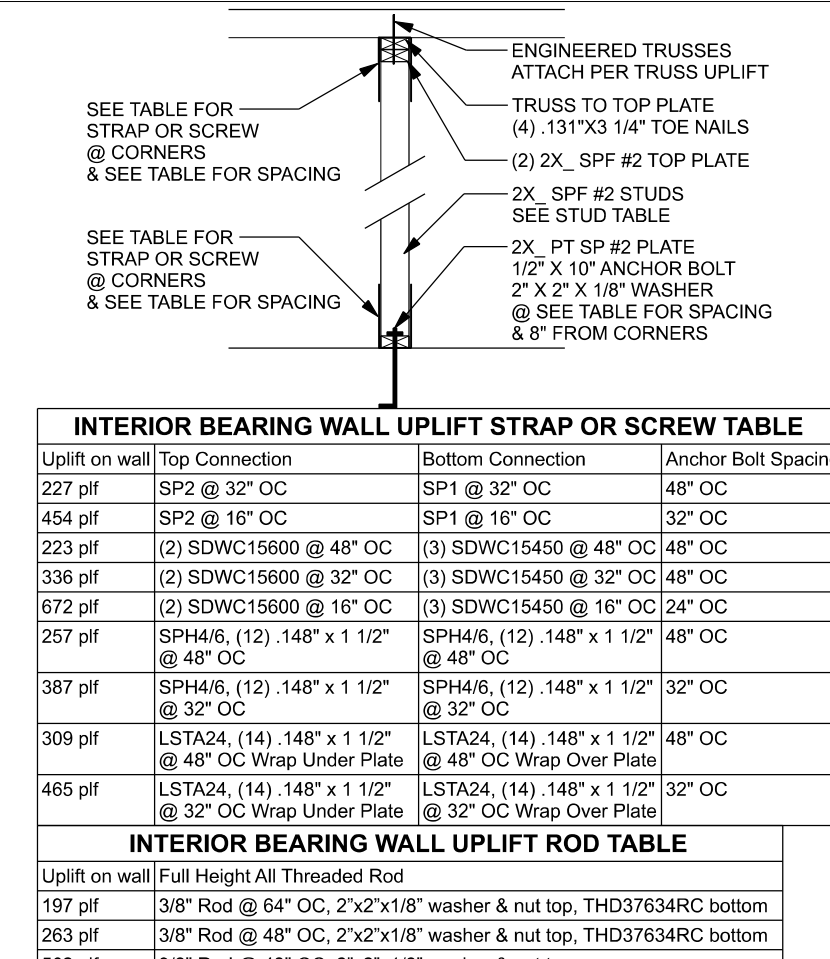
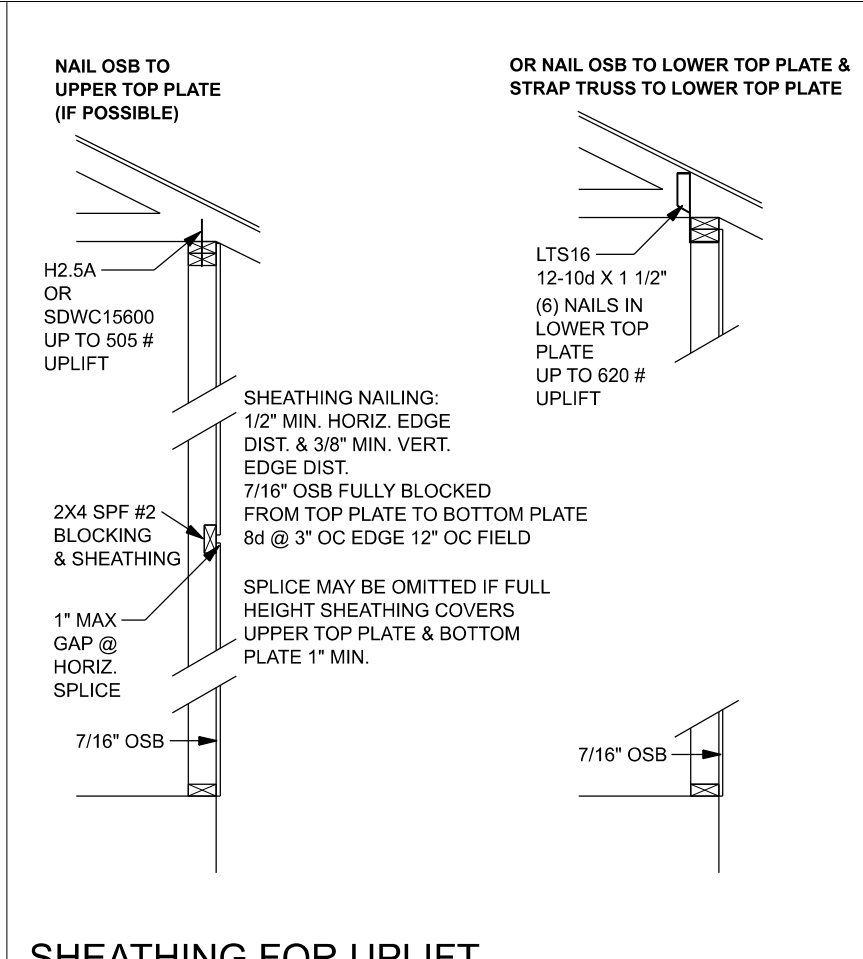


Wind Speed	Sheathing Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" OC	12" OC
120 mph Exp. C	7/16"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" OC	6" OC
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" OC	6" OC
130 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" OC	6" OC
140 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" OC	6" OC
140 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" OC	6" OC
150 mph Exp. C	19/32"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" OC	6" OC
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" OC	6" OC
150 mph Exp. C	19/32"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" OC	6" OC
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" OC	6" OC



Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
605	505	SDWC15600	4- 131'x1 1/2"	4- 131'x1 1/2"
400	200	H3	5- 131'x1 1/2"	5- 131'x1 1/2"
625	540	H2.5A	5- 131'x1 1/2"	5- 131'x1 1/2"
1040	1015	H10A	8- 148'x1 1/2"	8- 148'x1 1/2"
645	515	LTS12-20	8- 148'x1 1/2"	8- 148'x1 1/2"
990	850	MST24	7- 148'x1 1/2"	7- 148'x1 1/2"
1415	1215	HTS16-30	8- 148'x1 1/2"	8- 148'x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8- 148'x1 1/2"	8- 148'x1 1/2"
1560	1460	MSTA24	9- 148'x1 1/2"	9- 148'x1 1/2"
1030	1030	CS20	7- 148'x1 1/2"	7- 148'x1 1/2"
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4- 148'x3"	4- 148'x3"
1010	605	SP2	6- 148'x3"	6- 148'x3"
1290	1100	SPH48	12- 148'x1 1/2"	wrap under or over plate
771	771	LSTA24	10- 148'x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14- 148'x1 1/2"	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
2325	3640	HTT4	18- 162'x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
2325	3640	HTT4	18- 162'x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Stud	Anchor
1900	1400	ABU422	12- 162'x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU422	12- 162'x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	1400	ABU422	12- 162'x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU422	12- 162'x3 1/2"	5/8"x12" Drill & Epoxy

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. TRUSSES SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X4 RAFTERS WITH MIN. UPLIFT CONNECTION 415L EACH END, 2X4 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDE OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W14 x 4 W14. FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC: W1.4 CONFORMING TO ASTM A185. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. INSTALLING AND PROVIDING ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT JOINTS WITH OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS. FY = 40 KSI. ALL LAP SPACINGS 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

ROOF SHEATHING: ALL ROOF ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, UNDER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT OR THE SAME OR OTHER MANUFACTURER'S SHALL BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

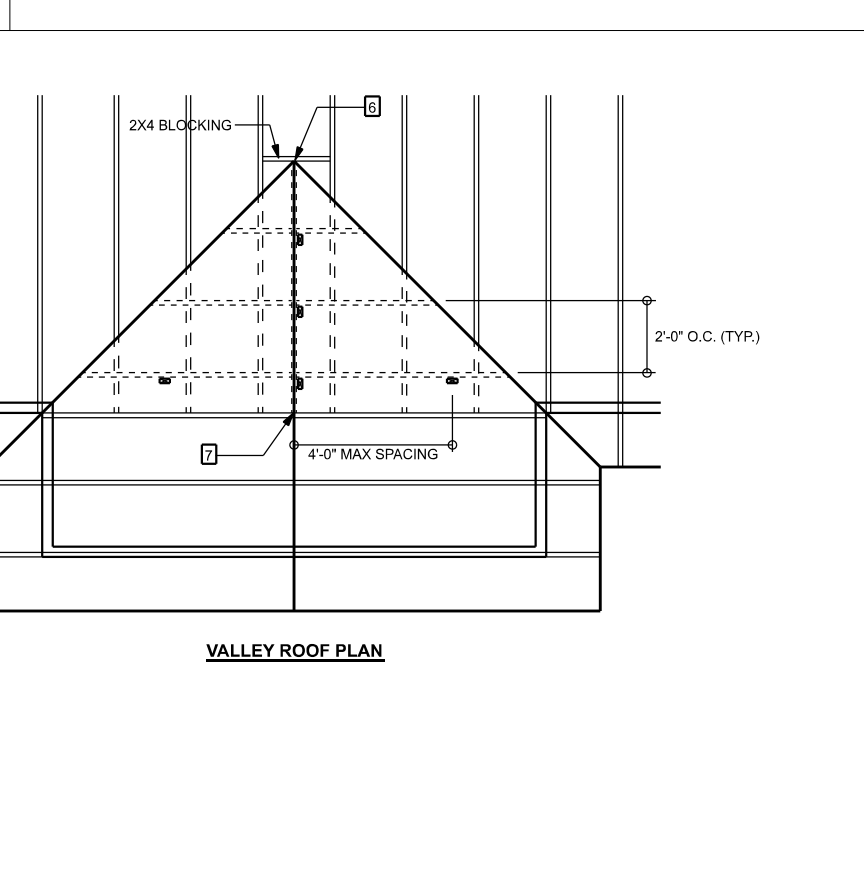
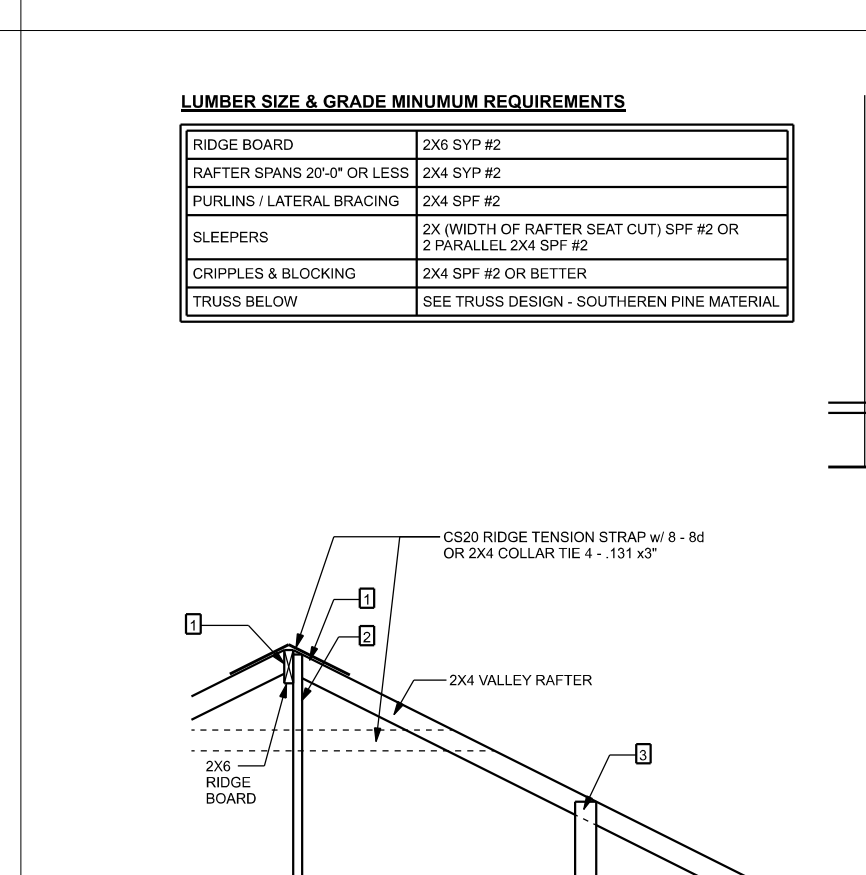
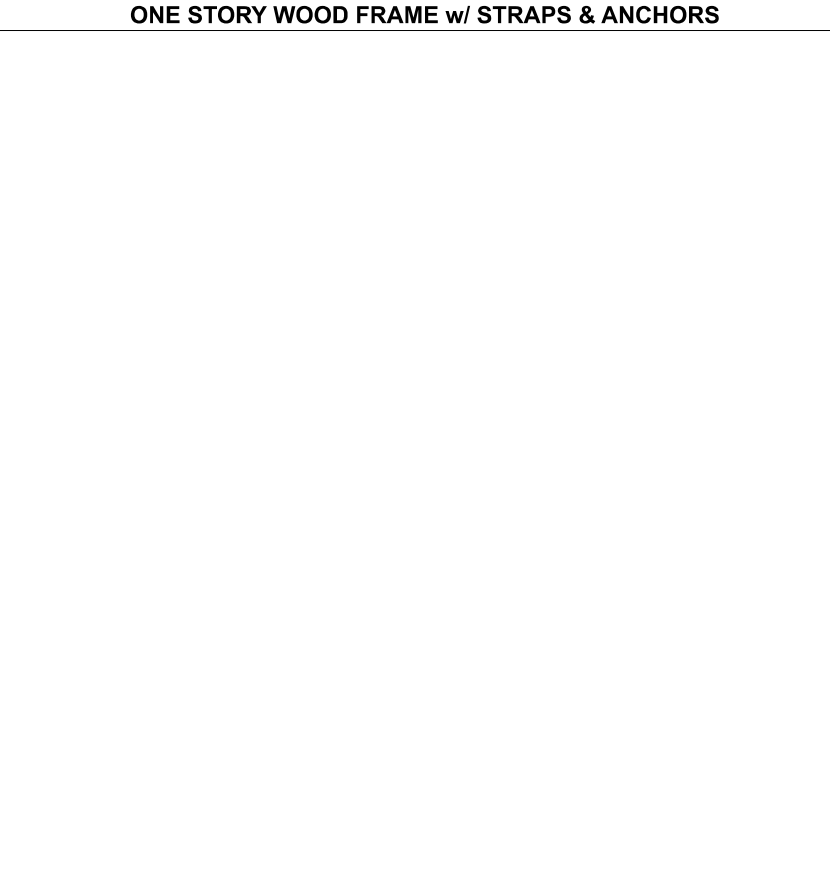
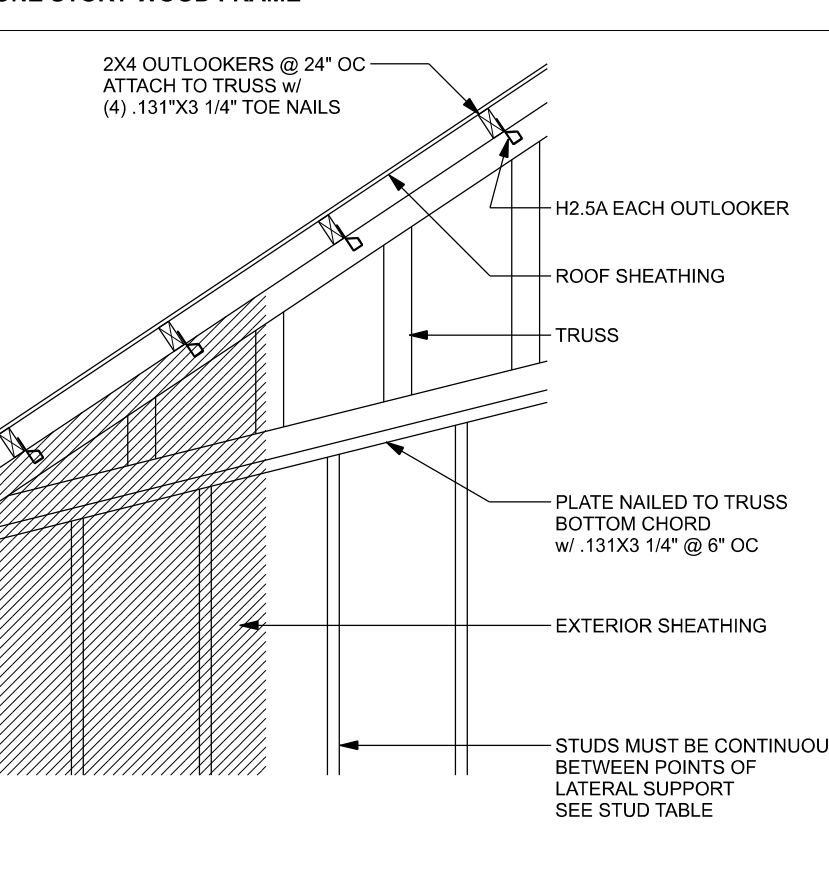
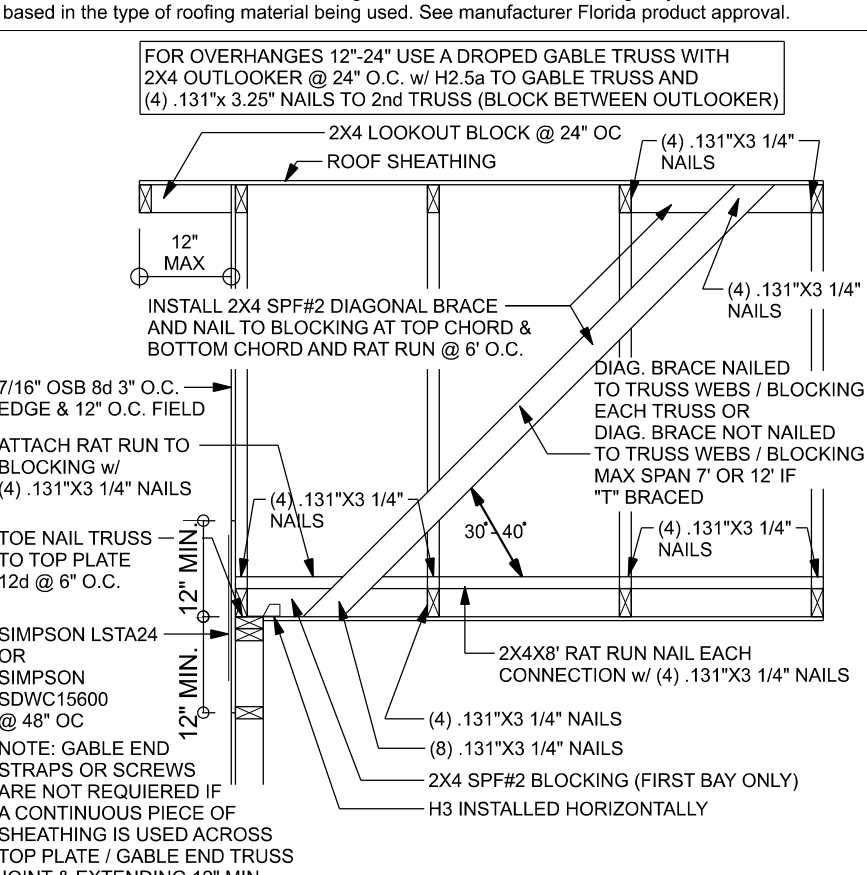
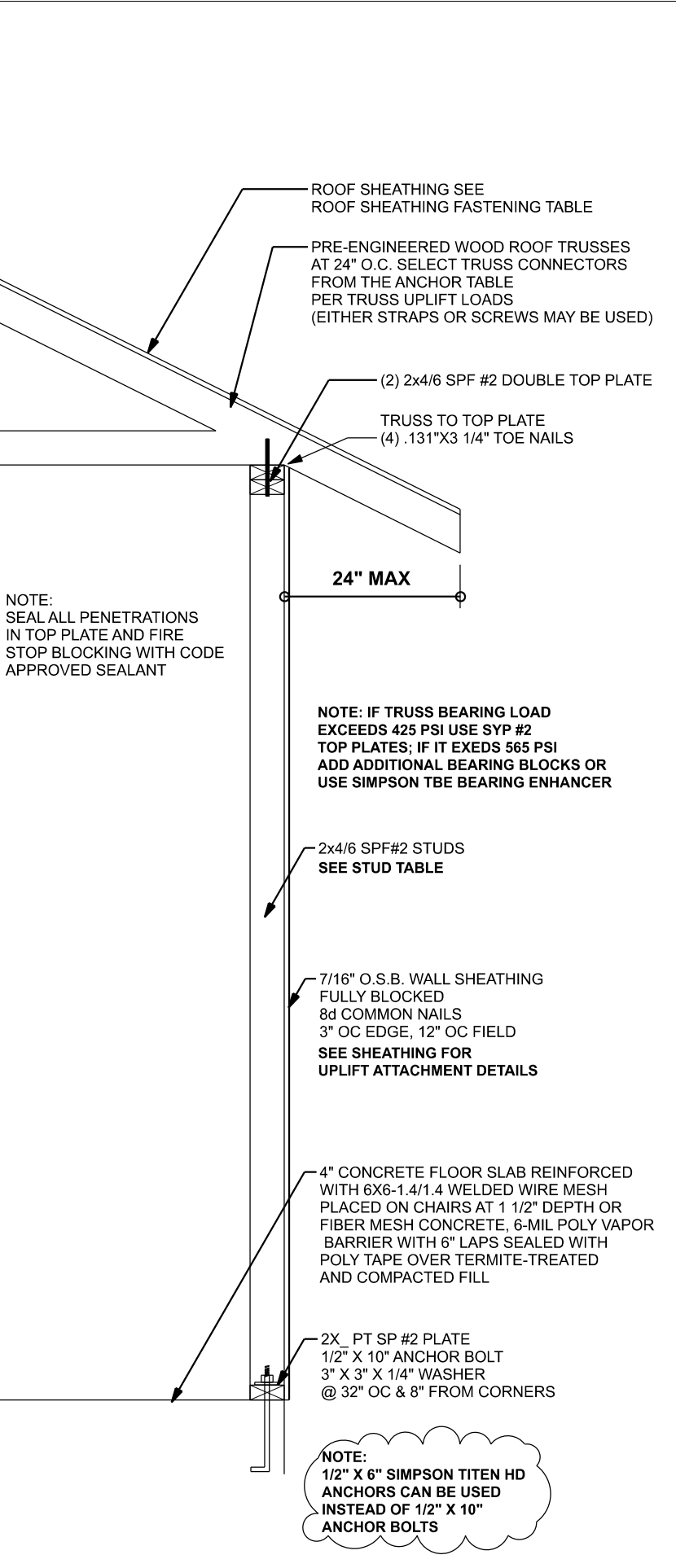
BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBER REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH IS SPECIALLY BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



VALLEY ROOF PLAN MEMBER LEGEND

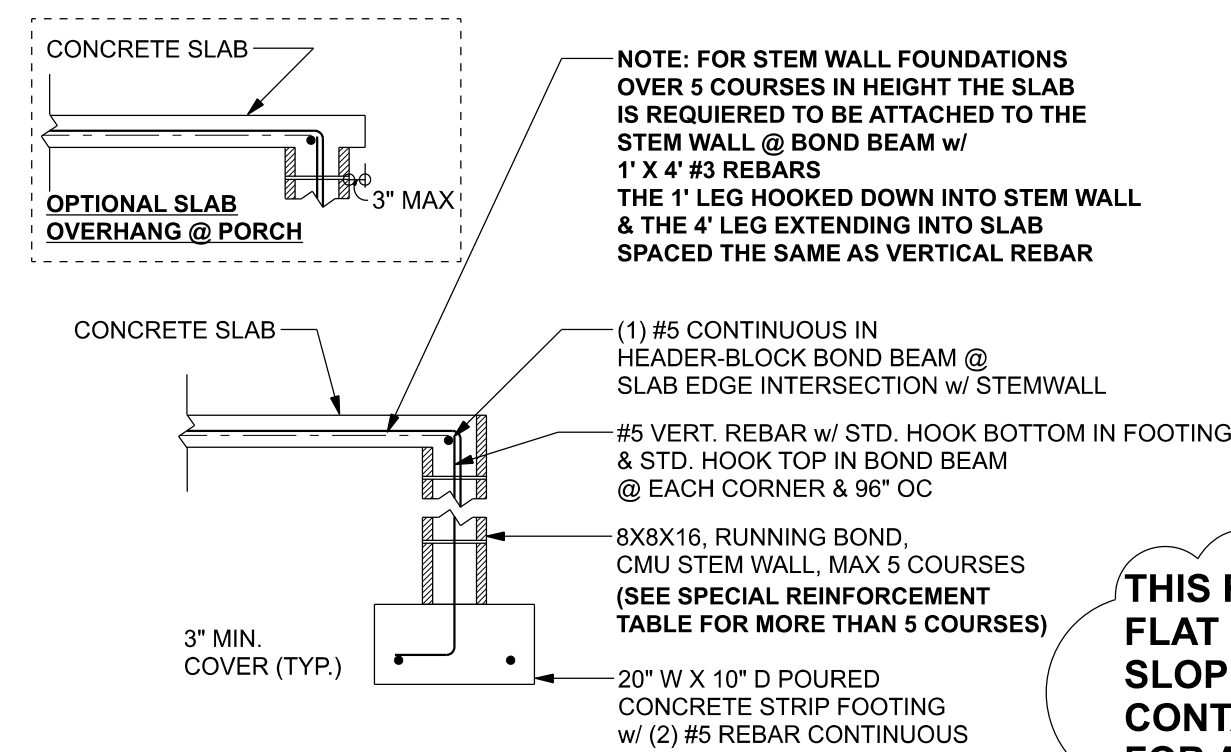
--- TRUSS
--- TRUSS UNDER VALLEY FRAMING
--- VALLEY RAFTER OR RIDGE
--- CRIPPLE

CONNECTION REQUIREMENT NOTES

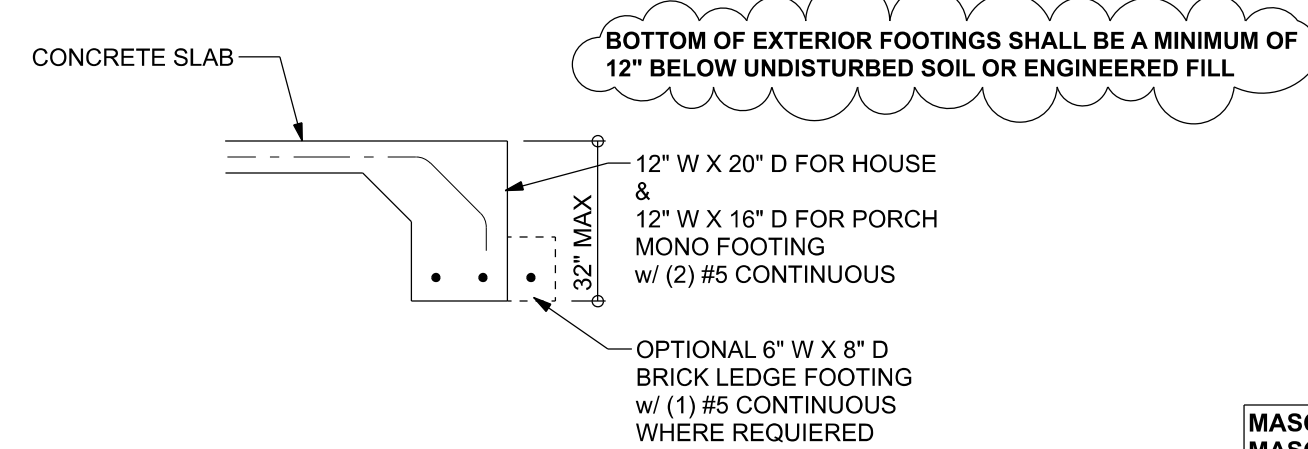
1 2x4 RAFTER TO RIDGE 4- 131' x 3" TOE NAILS
2 CRIPPLE TO RIDGE 4- 131' x 3" FACE NAILS
3 CRIPPLE TO RAFTERS 4- 131' x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING 4- 131' x 3" TOE NAILS
5 SLEEPER TO TRUSS 4- 131' x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK 4- 131' x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS 4- 131' x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP) 4- 131' x 3" NAILS
9 PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN 4- 131' x 3" END NAILS
10 TRUSS TO BLOCKING 4- 131' x 3" END NAILS
11 CRIPPLE TO TRUSS 4- 131' x 3" FACE NAILS
12 CRIPPLE TO PURLIN 4- 131' x 3" FACE NAILS

GENERAL NOTES

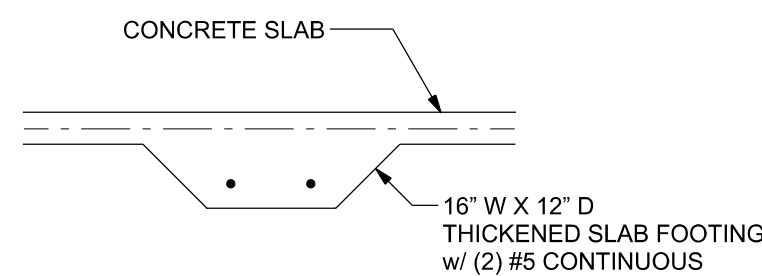
MAXIMUM RAFTER SPANS: 6'0" FOR 2x4 SPF OR 2x6 SPF #2 OR 2x8 SPF #2; 8'0" FOR 2x4 SPF OR 2x6 SPF #2; 10'0" FOR 2x4 SPF OR 2x6 SPF #2; 12'0" FOR 2x4 SPF OR 2x6 SPF #2; 14'0" FOR 2x4 SPF OR 2x6 SPF #2; 16'0" FOR 2x4 SPF OR 2x6 SPF #2; 18'0" FOR 2x4 SPF OR 2x6 SPF #2; 20'0" FOR 2x4 SPF OR 2x6 SPF #2; 22'0" FOR 2x4 SPF OR 2x6 SPF #2; 24'0" FOR 2x4 SPF OR 2x6 SPF #2; 26'0" FOR 2x4 SPF OR 2x6 SPF #2; 28'0" FOR 2x4 SPF OR 2x6 SPF #2; 30'0" FOR 2x4 SPF OR 2x6 SPF #2; 32'0" FOR 2x4 SPF OR 2x6 SPF #2; 34'0" FOR 2x4 SPF OR 2x6 SPF #2; 36'0" FOR 2x4 SPF OR 2x6 SPF #2; 38'0" FOR 2x4 SPF OR 2x6 SPF #2; 40'0" FOR 2x4 SPF OR 2x6 SPF #2; 42'0" FOR 2x4 SPF OR 2x6 SPF #2; 44'0" FOR 2x4 SPF OR 2x6 SPF #2; 46'0" FOR 2x4 SPF OR 2x6 SPF #2; 48'0" FOR 2x4 SPF OR 2x6 SPF #2; 50'0" FOR 2x4 SPF OR 2x6 SPF #2; 52'0" FOR 2x4 SPF OR 2x6 SPF #2; 54'0" FOR 2x4 SPF OR 2x6 SPF #2; 56'0" FOR 2x4 SPF OR 2x6 SPF #2; 58'0" FOR 2x4 SPF OR 2x6 SPF #2; 60'0" FOR 2x4 SPF OR 2x6 SPF #2; 62'0" FOR 2x4 SPF OR 2x6 SPF #2; 64'0" FOR 2x4 SPF OR 2x6 SPF #2; 66'0" FOR 2x4 SPF OR 2x6 SPF #2; 68'0" FOR 2x4 SPF OR 2x6 SPF #2; 70'0" FOR 2x4 SPF OR 2x6 SPF #2; 72'0" FOR 2x4 SPF OR 2x6 SPF #2; 74'0" FOR 2x4 SPF OR 2x6 SPF #2; 76'0" FOR 2x4 SPF OR 2x6 SPF #2; 78'0" FOR 2x4 SPF OR 2x6 SPF #2; 80'0" FOR 2x4 SPF OR 2x6 SPF #2; 82'0" FOR 2x4 SPF OR 2x6 SPF #2; 84'0" FOR 2x4 SPF OR 2x6 SPF #2; 86'0" FOR 2x4 SPF OR 2x6 SPF #2; 88'0" FOR 2x4 SPF OR 2x6 SPF #2; 90'0" FOR 2x4 SPF OR 2x6 SPF #2; 92'0" FOR 2x4 SPF OR 2x6 SPF #2; 94'0" FOR 2x4 SPF OR 2x6 SPF #2; 96'0" FOR 2x4 SPF OR 2x6 SPF #2; 98'0" FOR 2x4 SPF OR 2x6 SPF #2; 100'0" FOR 2x4 SPF OR 2x6 SPF #2; 102'0" FOR 2x4 SPF OR 2x6 SPF #2; 104'0" FOR 2x4 SPF OR 2x6 SPF #2; 106'0" FOR 2x4 SPF OR 2x6 SPF #2; 108'0" FOR 2x4 SPF OR 2x6 SPF #2; 110'0" FOR 2x4 SPF OR 2x6 SPF #2; 112'0" FOR 2x4 SPF OR 2x6 SPF #2; 114'0" FOR 2x4 SPF OR 2x6 SPF #2; 116'0" FOR 2x4 SPF OR 2x6 SPF #2; 118'0" FOR 2x4 SPF OR 2x6 SPF #2; 120'0" FOR 2x4 SPF OR 2x6 SPF #2; 122'0" FOR 2x4 SPF OR 2x6 SPF #2; 124'0" FOR 2x4 SPF OR 2x6 SPF #2; 126'0" FOR 2x4 SPF OR 2x6 SPF #2; 128'0" FOR 2x4 SPF OR 2x6 SPF #2; 130'0" FOR 2x4 SPF OR 2x6 SPF #2; 132'0" FOR 2x4 SPF OR 2x6 SPF #2; 134'0" FOR 2x4 SPF OR 2x6 SPF #2; 136'0" FOR 2x4 SPF OR 2x6 SPF #2; 138'0" FOR 2x4 SPF OR 2x6 SPF #2; 140'0" FOR 2x4 SPF OR 2x6 SPF #2; 142'0" FOR 2x4 SPF OR 2x6 SPF #2; 144'0" FOR 2x4 SPF OR 2x6 SPF #2; 146'0" FOR 2x4 SPF OR 2x6 SPF #2; 148'0" FOR 2x4 SPF OR 2x6 SPF #2; 150'0" FOR 2x4 SPF OR 2x6 SPF #2; 152'0" FOR 2x4 SPF OR 2x6 SPF #2; 154'0" FOR 2x4 SPF OR 2x6 SPF #2; 156'0" FOR 2x4 SPF OR 2x6 SPF #2; 158'0" FOR 2x4 SPF OR 2x6 SPF #2; 160'0" FOR 2x4 SPF OR 2x6 SPF #2; 162'0" FOR 2x4 SPF OR 2x6 SPF #2; 164'0" FOR 2x4 SPF OR 2x6 SPF #2; 166'0" FOR 2x4 SPF OR 2x6 SPF #2; 168'0" FOR 2x4 SPF OR 2x6 SPF #2; 170'0" FOR 2x4 SPF OR 2x6 SPF #2; 172'0" FOR 2x4 SPF OR 2x6 SPF #2; 174'0" FOR 2x4 SPF OR 2x6 SPF #2; 176'0" FOR 2x4 SPF OR 2x6 SPF #2; 178'0" FOR 2x4 SPF OR 2x6 SPF #2; 180'0" FOR 2x4 SPF OR 2x6 SPF #2; 182'0" FOR 2x4 SPF OR 2x6 SPF #2; 184'0" FOR 2x4 SPF OR 2x6 SPF #2; 186'0" FOR 2x4 SPF OR 2x6 SPF #2; 188'0" FOR 2x4 SPF OR 2x6 SPF #2; 190'0" FOR 2x4 SPF OR 2x6 SPF #2; 192'0" FOR 2x4 SPF OR 2x6 SPF #2; 194'0" FOR 2x4 SPF OR 2x6 SPF #2; 196'0" FOR 2x4 SPF OR 2x6 SPF #2; 198'0" FOR 2x4 SPF OR 2x6 SPF #2; 200'0" FOR 2x4 SPF OR 2x6 SPF #2; 202'0" FOR 2x4 SPF OR 2x6 SPF #2; 204'0" FOR 2x4 SPF OR 2x6 SPF #2; 206'0" FOR 2x4 SPF OR 2x6 SPF #2; 208'0" FOR 2x4 SPF OR 2x6 SPF #2; 210'0" FOR 2x4 SPF OR 2x6 SPF #2; 212'0" FOR 2x4 SPF OR 2x6 SPF #2; 214'0" FOR 2x4 SPF OR 2x6 SPF #2; 216'0" FOR 2x4 SPF OR 2x6 SPF #2; 218'0" FOR 2x4 SPF OR 2x6 SPF #2; 220'0" FOR 2x4 SPF OR 2x6 SPF #2; 222'0" FOR 2x4 SPF OR 2x6 SPF #2; 224'0" FOR 2x4 SPF OR 2x6 SPF #2; 226'0" FOR 2x4 SPF OR 2x6 SPF #2; 228'0" FOR 2x4 SPF OR 2x6 SPF #2; 230'0" FOR 2x4 SPF OR 2x6 SPF #2; 232'0" FOR 2x4 SPF OR 2x6 SPF #2; 234'0" FOR 2x4 SPF OR 2x6 SPF #2; 236'0" FOR 2x4 SPF OR 2x6 SPF #2; 238'0" FOR 2x4 SPF OR 2x6 SPF #2; 240'0" FOR 2x4 SPF OR 2x6 SPF #2; 242'0" FOR 2x4 SPF OR 2x6 SPF #2; 244'0" FOR 2x4 SPF OR 2x6 SPF #2; 246'0" FOR 2x4 SPF OR 2x6 SPF #2; 248'0" FOR 2x4 SPF OR 2x6 SPF #2; 250'0" FOR 2x4 SPF OR 2x6 SPF #2; 252'0" FOR 2x4 SPF OR 2x6 SPF #2; 254'0" FOR 2x4 SPF OR 2x6 SPF #2; 256'0" FOR 2x4 SPF OR 2x6 SPF #2; 258'0" FOR 2x4 SPF OR 2x6 SPF #2; 260'0" FOR 2x4 SPF OR 2x6 SPF #2; 262'0" FOR 2x4 SPF OR 2x6 SPF #2; 264'0" FOR 2x4 SPF OR 2x6 SPF #2; 266'0" FOR 2x4 SPF OR 2x6 SPF #2; 268'0" FOR 2x4 SPF OR 2x6 SPF #2; 270'0" FOR 2x4 SPF OR 2x6 SPF #2; 272'0" FOR 2x4 SPF OR 2x6 SPF #2; 274'0" FOR 2x4 SPF OR 2x6 SPF #2; 276'0" FOR 2x4 SPF OR 2x6 SPF #2; 278'0" FOR 2x4 SPF OR 2x6 SPF #2; 280'0" FOR 2x4 SPF OR 2x6 SPF #2; 282'0" FOR 2x4 SPF OR 2x6 SPF #2; 284'0" FOR 2x4 SPF OR 2x6 SPF #2; 286'0" FOR 2x4 SPF OR 2x6 SPF #2; 288'0" FOR 2x4 SPF OR 2x6 SPF #2; 290'0" FOR 2x4 SPF OR 2x6 SPF #2; 292'0" FOR 2x4 SPF OR 2x6 SPF #2; 294'0" FOR 2x4 SPF OR 2x6 SPF #2; 296'0" FOR 2x4 SPF OR 2x6 SPF #2; 298'0" FOR 2x4 SPF OR 2x6 SPF #2; 300'0" FOR 2x4 SPF OR 2x6 SPF #2; 302'0" FOR 2x4 SPF OR 2x6 SPF #2; 304'0" FOR 2x4 SPF OR 2x6 SPF #2; 306'0" FOR 2x4 SPF OR 2x6 SPF #2; 308'0" FOR 2x4 SPF OR 2x6 SPF #2; 310'0" FOR 2x4 SPF OR 2x6 SPF #2; 312'0" FOR 2x4 SPF OR 2x6 SPF #2; 314'0" FOR 2x4 SPF OR 2x6 SPF #2; 316'0" FOR 2x4 SPF OR 2x6 SPF #2; 318'0" FOR 2x



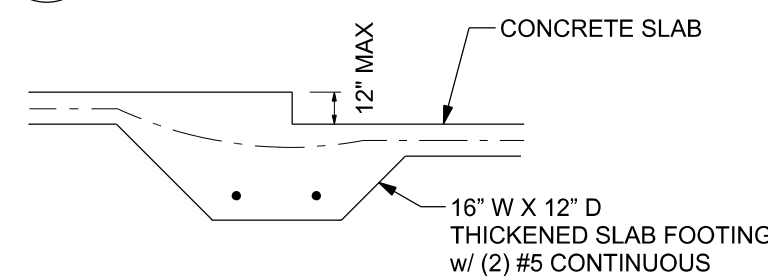
F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

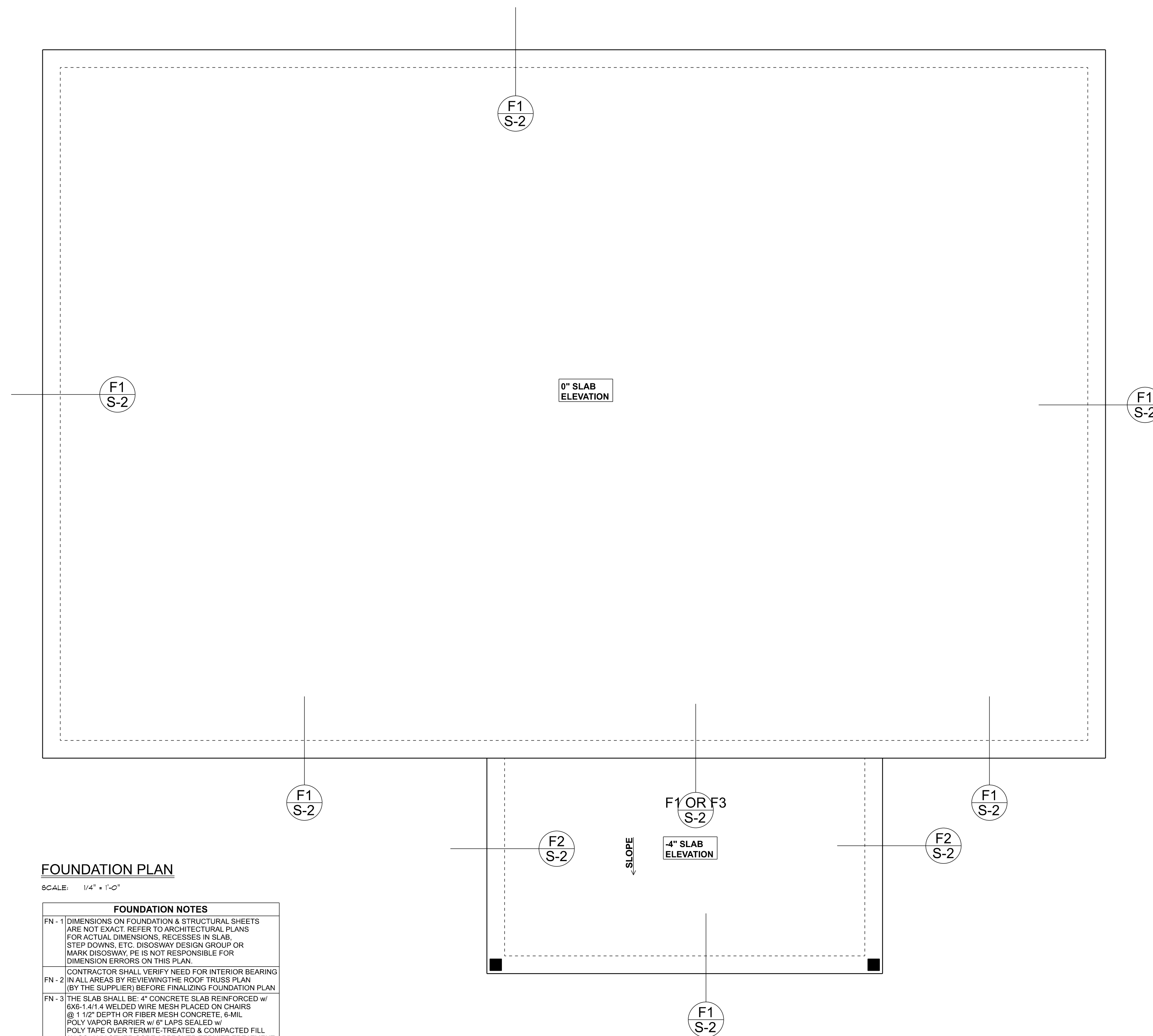
TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls F _m = 1500 psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 40, F _y = 40 ksi, Lap splices min 40 bar dia. (20" for #5)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 020, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN - 2 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY W/PPR BARRIER w/ 7 LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES-TREATMENT METHOD CAN BE USED INSTEAD).

Hunter & Ashley Faulkner Res.
PROJECT ADDRESS:
FL White, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

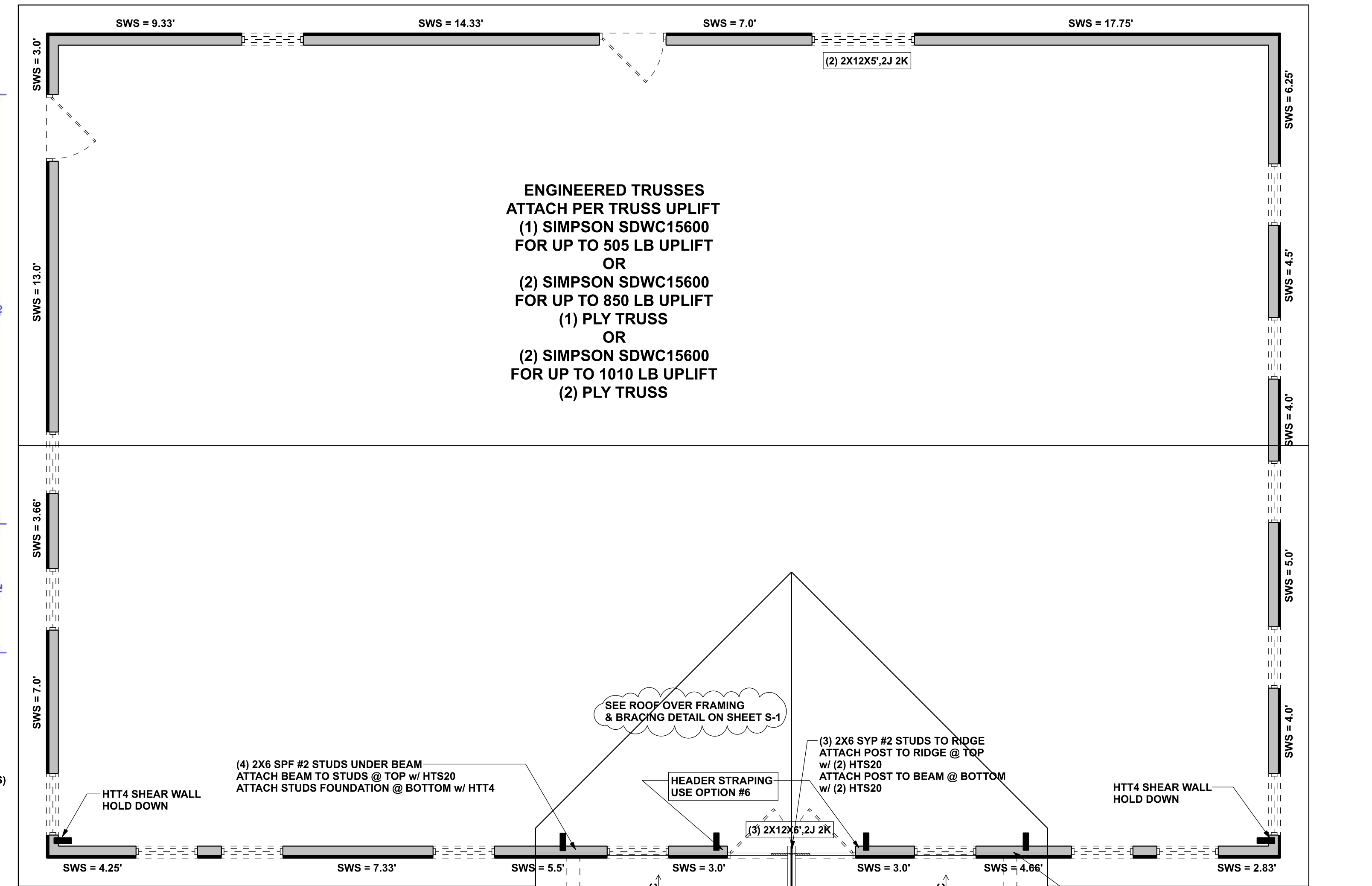
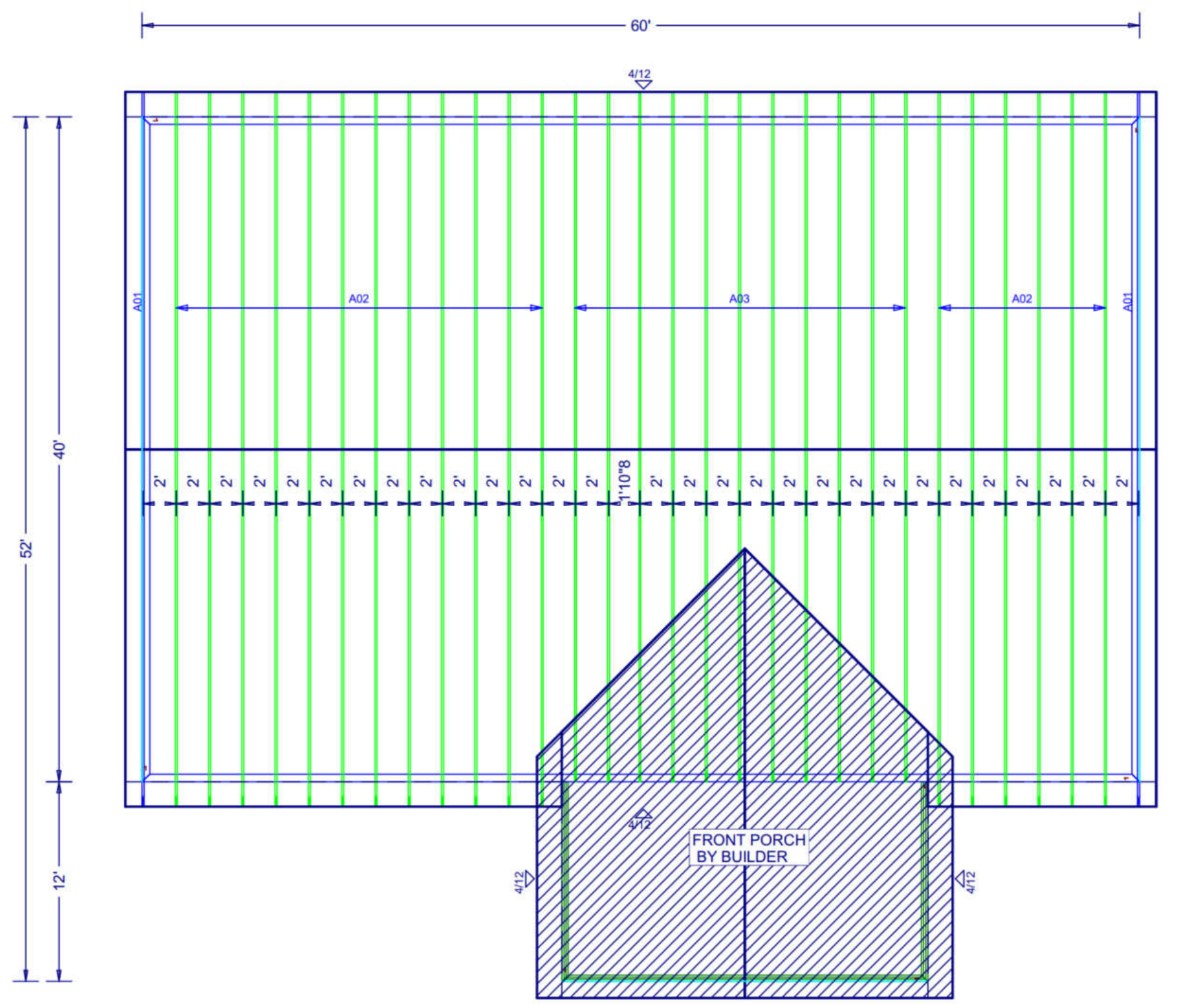
COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disoway, P.E. hereby expressly reserves all common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

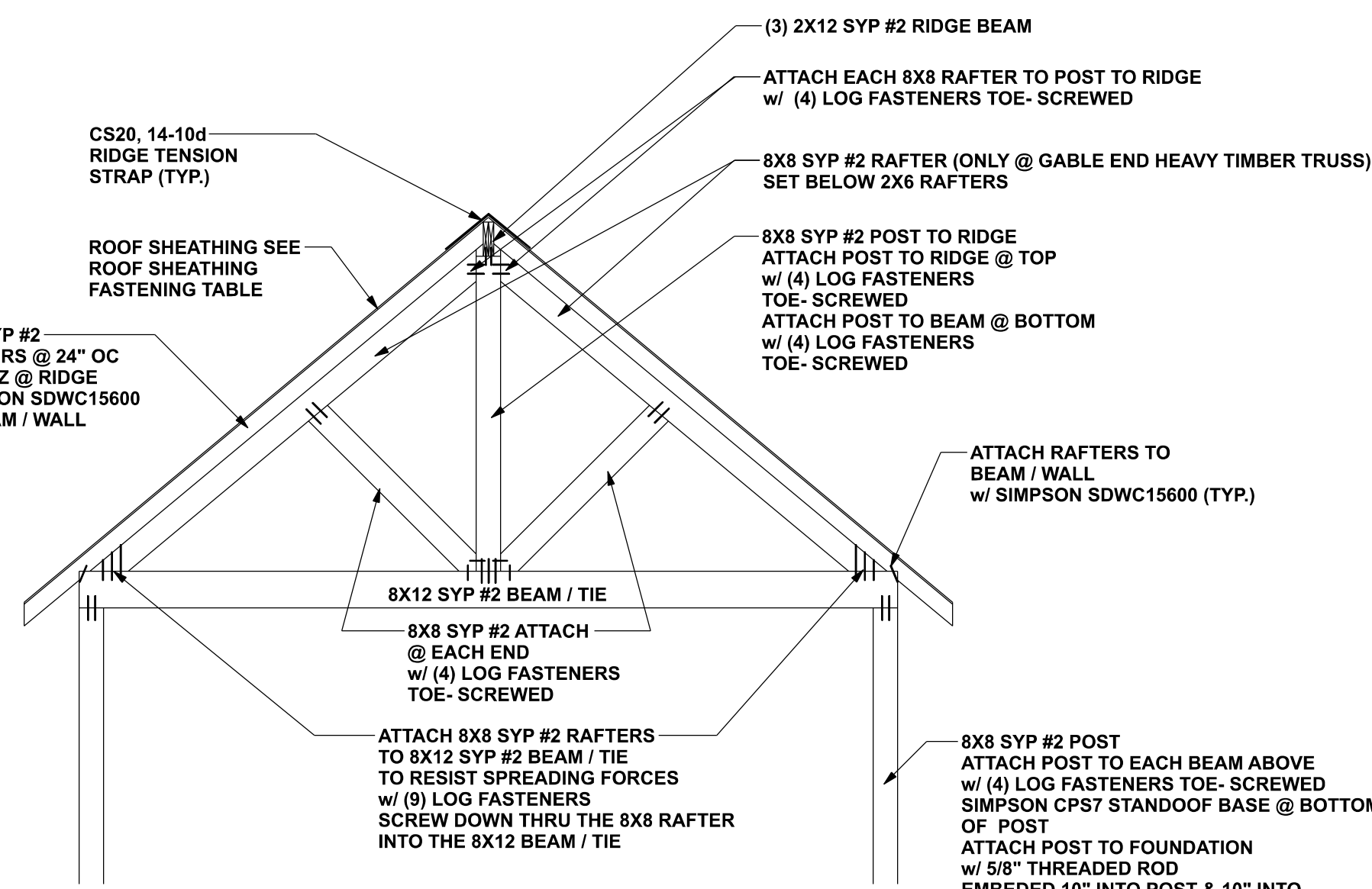
Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
250613
S-2
OF 3 SHEETS



**ENGINEERED TRUSSES
ATTACH PER TRUSS UPLIFT**
 (1) SIMPSON SDWC15600
 FOR UP TO 505 LB UPLIFT
 OR
 (2) SIMPSON SDWC15600
 FOR UP TO 850 LB UPLIFT
 (1) PLY TRUSS
 OR
 (2) SIMPSON SDWC15600
 FOR UP TO 1010 LB UPLIFT
 (2) PLY TRUSS

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"



HEAVY TIMBER ROOF SECTION @ PORCH
SCALE: 1/4" = 1'-0"

LOG HOG FASTENERS ARE SELF DRILLING, HIGH STRENGTH, F1 = 111ksi, STEEL WOOD SCREWS WITH 0.228 SHANK DIAMETER. THE FOLLOWING DESIGN VALUES ARE BASED ON CODE TEST REPORT BY ICC EVALUATION SERVICE, INC. AND NDS 2001:

LOG HOG FASTENERS ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT FASTENER OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. LISTED LOADS HAVE BEEN INCREASED FOR WIND DURATION, AND MUST BE ADJUSTED FOR OTHER DURATIONS OR SPECIES.

LOG HOG FASTENERS DESIGN VALUE FOR 1.6 WIND DURATION (LB) BASED ON 3" MIN. PENETRATION INTO MAIN MEMBER & 2.5" MIN. SIDE MEMBER

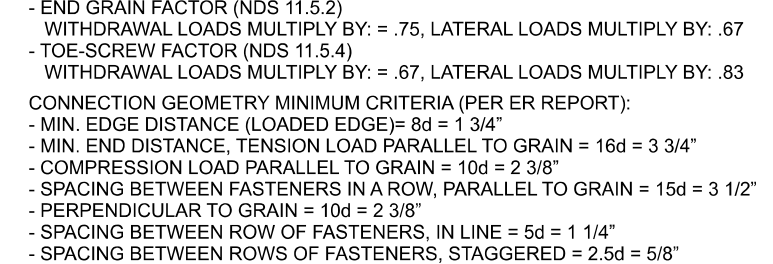
WOOD SPECIES (SPECIFIC GRAVITY)	SHINGLE SHEAR DESIGN VALUE PARALLEL TO GRAIN	SHINGLE SHEAR DESIGN VALUE PERPENDICULAR TO GRAIN	WITHDRAWAL DESIGN VALUE
SYP (.55 SG)	502	477	1060
CYP (.43 SG) SPF (.42 SG)	438	387	710

ALLOWABLE WITHDRAWAL STRENGTH AND SINGLE SHEAR RESISTANCE BASED ON VALUES LISTED IN ICC ES REPORT (ESR-1078 REISSUED FEB. 1, 2007)

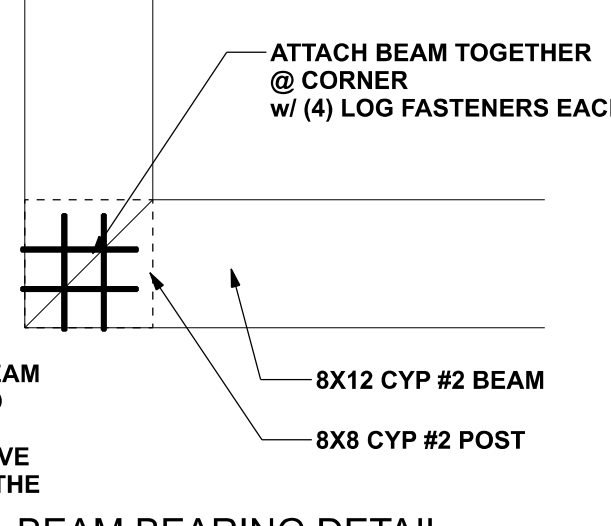
ADJUSTMENT FACTORS:
 WITHDRAWAL VALUES ARE BASED ON 3" PENETRATION INTO MAIN MEMBER.
 WHEN PENETRATION (D) IS LESS THAN 3" VALUE SHALL BE MULTIPLIED BY: p3"
 SHEAR VALUES ARE BASED ON 100 MIN. PENETRATION INTO MAIN MEMBER.
 WHEN PENETRATION (D) IS 60-90-100 SHEAR VALUE SHALL BE MULTIPLIED BY: p100
 TABLE VALUES ARE FOR 1.6 WIND DURATION. ADJUST FOR OTHER DURATIONS.
 END GRAIN FACTOR (NDS 11.5.2)
 WITHDRAWAL LOADS MULTIPLY BY: .75, LATERAL LOADS MULTIPLY BY: .67
 TOE SCREW FACTOR (NDS 11.5.4)
 WITHDRAWAL LOADS MULTIPLY BY: .67, LATERAL LOADS MULTIPLY BY: .83

CONNECTION GEOMETRY MINIMUM CRITERIA (PER ESR REPORT):
 MIN. EDGE DISTANCE (LOADED EDGE) = 8d + 1.5d
 MIN. END DISTANCE, TENSION LOAD PARALLEL TO GRAIN = 16d + 3.3d
 COMPRESSION LOAD PARALLEL TO GRAIN = 10d + 2.3d
 SPACING BETWEEN FASTENERS IN A ROW, PARALLEL TO GRAIN = 15d + 3.12"
 PERPENDICULAR TO GRAIN = 10d + 2.3d
 SPACING BETWEEN ROWS OF FASTENERS, IN LINE = 5d + 1.1d
 SPACING BETWEEN ROWS OF FASTENERS, STAGGERED = 2.5d + 5/8"

N21 LOG HOG FASTENER DATA
TYPICAL DESIGN VALUES



BEAM BEARING DETAIL
SCALE: 1" = 1'-0"



BEAM BEARING DETAIL
SCALE: 1" = 1'-0"

NOTE:
 ALL MUST HAVE 3" MIN. PENETRATION INTO MAIN MEMBER & 2.5" MIN. SIDE MEMBER
 SEE N21 LOG HOG FASTENER DATA THIS SHEET FOR MORE DETAILS

Simpson Strong-Tie® Wood Construction Connectors

CPS/PBV

Standoff Bases

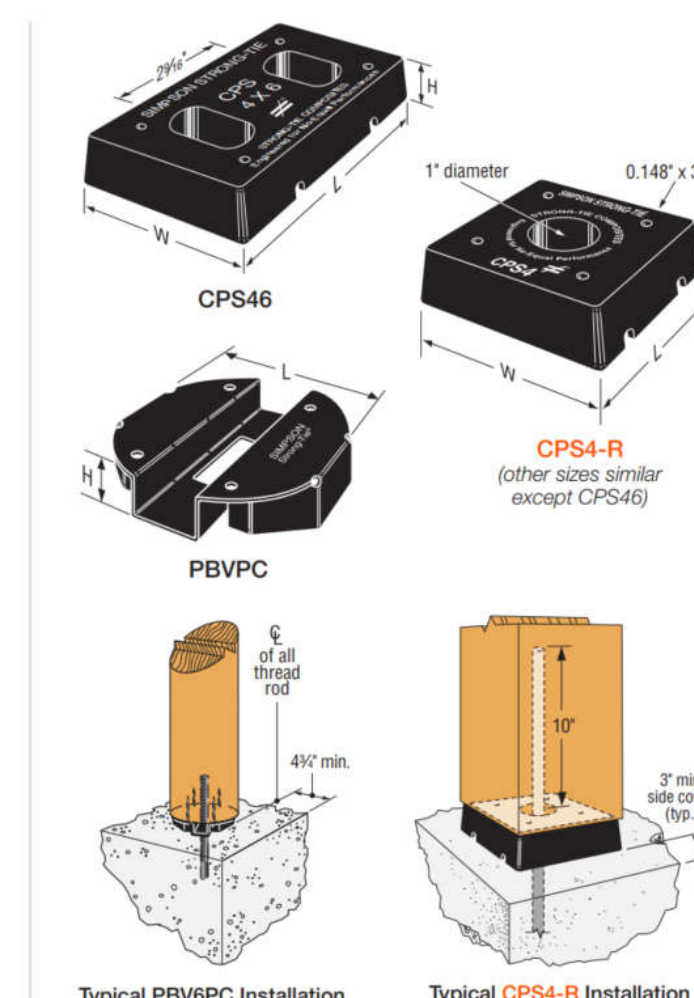
The PBV is a hidden standoff post base. Two different shapes fit a variety of post sizes. The CPS is a composite plastic standoff designed for increased concrete surface area.

Material: PBV - 14 gauge galvanized steel; CPS - engineered composite polymer
Finish: PBV - Galvanized with black powder coat
To Order: For black powder coat, order PBVPC or PBV10PC. For galvanized coating, order PBV6 or PBV10.

Installation:
PBV and CPS:
Post:
 • Drill a 1/2" diameter hole, 10" into the center of the post.
 • Clean out dust. Fill hole halfway with Simpson Strong-Tie SET-3G™ anchoring adhesive.
 • Insert all-thread rod and allow epoxy to set and cure.
 • Secure standoff to post using four 0.148" x 3" nails except PBV which uses four Strong-Drive® SDS Heavy-Duty Connector screws (sold separately).

Concrete:
 • Drill a 1/2" diameter hole per anchor design (see footnote 2 below).
 • Clean out dust. Fill hole halfway with Simpson Strong-Tie SET-3G, ET-3G™ or AT-3G™ anchoring adhesive. Insert post subassembly into hole and allow epoxy to set and cure.
 • Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non-top-supported installations (such as fences or unbraced carports).

Codes: See p. 13 for Code Reference Key Chart



ACTUAL vs REQUIRED SHEAR WALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	15123 LBF	18955 LBF
REQUIRED	12249 LBF	7832 LBF

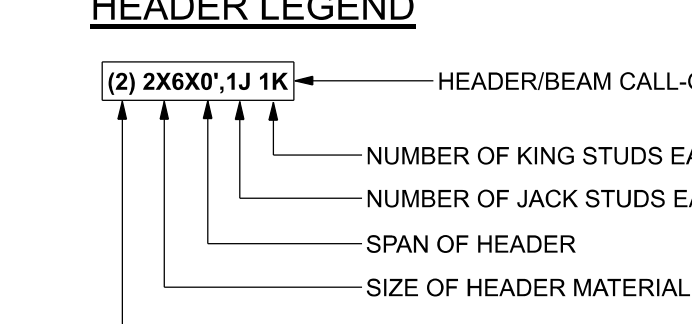
STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSD-03, BCSD-01, BCSD-02, & BCSD-03. BCSD-01, BCSD-02, & BCSD-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)
*****SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS*****

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCHEDULED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #25-2799

Hunter & Ashley Faulkner Res.
 PROJECT ADDRESS:
 Ft. White, FL

FL PE 53915
 This item has been digitally signed and sealed by Mark Disosway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
 Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
 Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
 163 SW Midtown Place
 Suite 103
 Lake City, Florida 32025
 386.754.5419
 disoswaydesign@gmail.com

JOB NUMBER:
 250613

S-3
 OF 3 SHEETS