

73

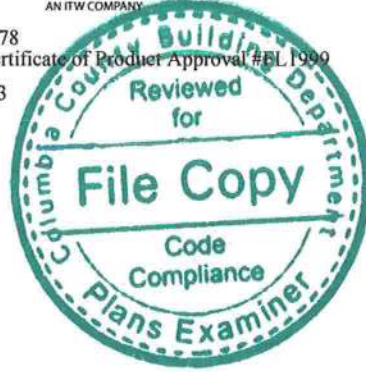


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COA #0 278
 Florida Certificate of Product Approval #FL1999
 04/27/2023

Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 23-9363
Job Description: J. Perkins	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 22.02.00 JRef #: 1XP72150002
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 11 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	117.23.1111.49653	A01
3	117.23.1111.52397	A03
5	117.23.1111.55750	B02
7	117.23.1111.58280	V01
9	117.23.1112.03503	V03
11	117.23.1112.07850	V05
13	GBLLETIN0118	
15	VALTN160118	

Item	Drawing Number	Truss
2	117.23.1111.51077	A02
4	117.23.1111.53577	B01
6	117.23.1111.57063	B03
8	117.23.1112.02247	V02
10	117.23.1112.04857	V04
12	A14015ENC160118	
14	VAL180160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

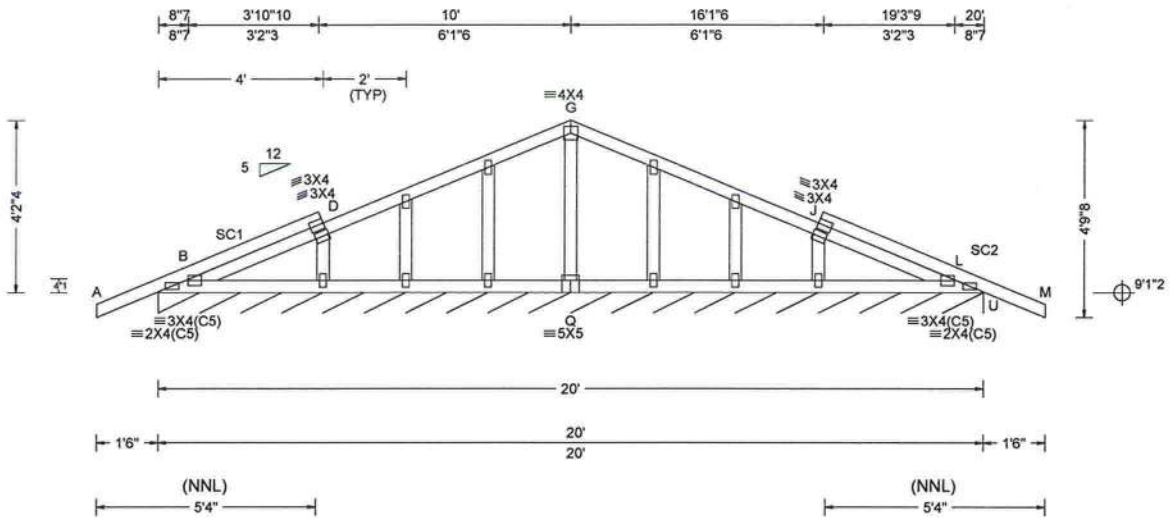
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 703287 FROM: CDM	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 23-9363 J. Perkins Truss Label: A01	Cust: R 215 JRef: 1XP72150002 T2 DrwNo: 117.23.1111.49653 SSB / DF 04/27/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 L 999 240 VERT(CL): 0.004 L 999 180 HORZ(LL): -0.001 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.266 Max BC CSI: 0.063 Max Web CSI: 0.036 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>U*</td> <td>92</td> <td>-</td> <td>-</td> <td>/45</td> <td>/17</td> <td>/6</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	U*	92	-	-	/45	/17	/6
				Loc		Gravity			Non-Gravity															
R+	/R-	/Rh	/Rw		/U	/RL																		
U*	92	-	-	/45	/17	/6																		
Wind reactions based on MWFRS U Brg Wid = 240 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#																								

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 4-2-4.

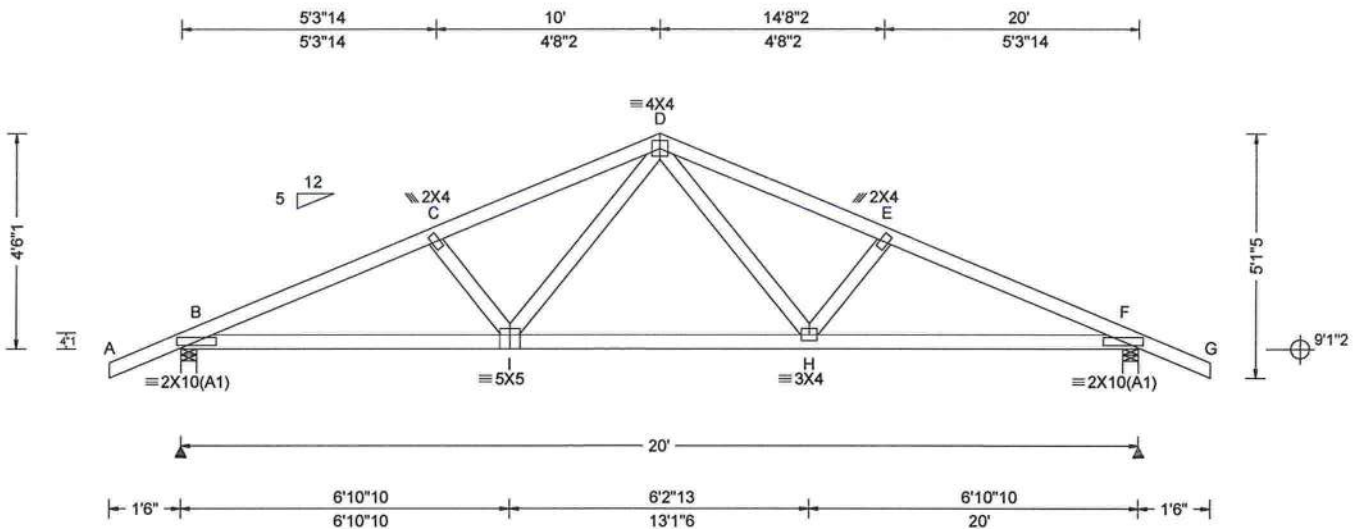


COA #0 278
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 04/27/2023

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 703290 FROM: CDM	COMN Ply: 1 Qty: 2	Job Number: 23-9363 J. Perkins Truss Label: A02	Cust R 215 JRef: 1XP72150002 T1 DrwNo: 117.23.1111.51077 SSB / DF 04/27/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.054 H 999 240 VERT(CL): 0.108 H 999 180 HORZ(LL): 0.019 F - - HORZ(TL): 0.038 F - - Creep Factor: 2.0 Max TC CSI: 0.228 Max BC CSI: 0.489 Max Web CSI: 0.169 VIEW Ver: 22.02.00.0914.12	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 916 /- /- /531 /170 /123 F 916 /- /- /531 /170 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 711 -1532 D - E 670 -1358 C - D 671 -1357 E - F 710 -1533
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-6-1.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	1365 -567	H - F	1366 -550
I - H	937 -309		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
I - D	442 -178	D - H	443 -177



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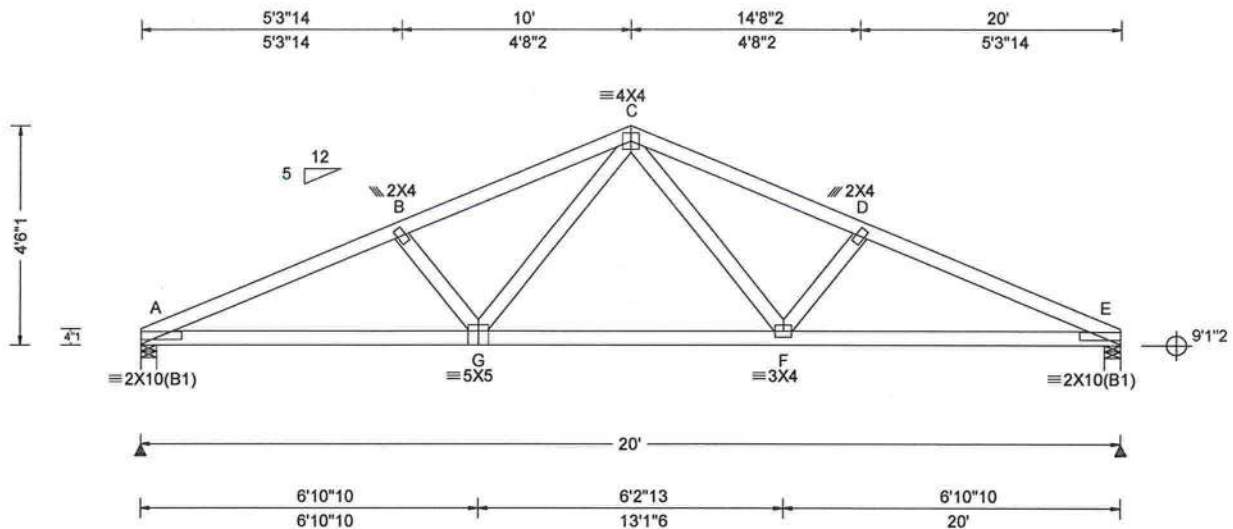
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 703293 FROM: CDM	COMN Ply: 1 Qty: 1	Job Number: 23-9363 J. Perkins Truss Label: A03	Cust R 215 JRef: 1XP72150002 T3 DrwNo: 117.23.1111.52397 SSB / DF 04/27/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.053 F 999 240 VERT(CL): 0.107 F 999 180 HORZ(LL): 0.019 E - - HORZ(TL): 0.039 E - - Creep Factor: 2.0 Max TC CSI: 0.254 Max BC CSI: 0.502 Max Web CSI: 0.178 VIEW Ver: 22.02.00.0914.12	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 817 /- /- /452 /144 /93 E 817 /- /- /452 /144 /- Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) E Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 521 -1576 C - D 493 -1398 B - C 494 -1397 D - E 521 -1577
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 4-6-1.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - G	1410 -433	F - E	1411 -422
G - F	958 -235		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
G - C	465 -130	C - F	467 -129

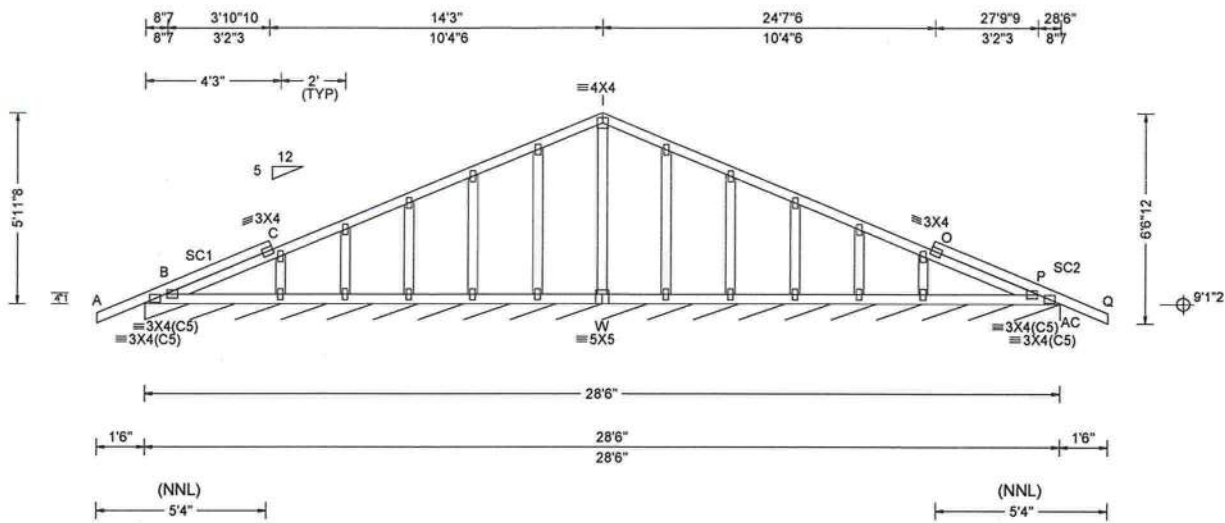


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SEQN: 703278 FROM: CDM	GABL Ply: 1 Qty: 2	Job Number: 23-9363 J. Perkins Truss Label: B01	Cust: R 215 JRef: 1XP72150002 T7 DrwNo: 117.23.1111.53577 SSB / DF 04/27/2023
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 P 999 240 VERT(CL): 0.005 P 999 180 HORZ(LL): -0.001 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.265 Max BC CSI: 0.073 Max Web CSI: 0.069 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL AC*89 - / - / - /44 /16 /6 Wind reactions based on MWFRS AC Brg Wid = 342 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 5-11-8.

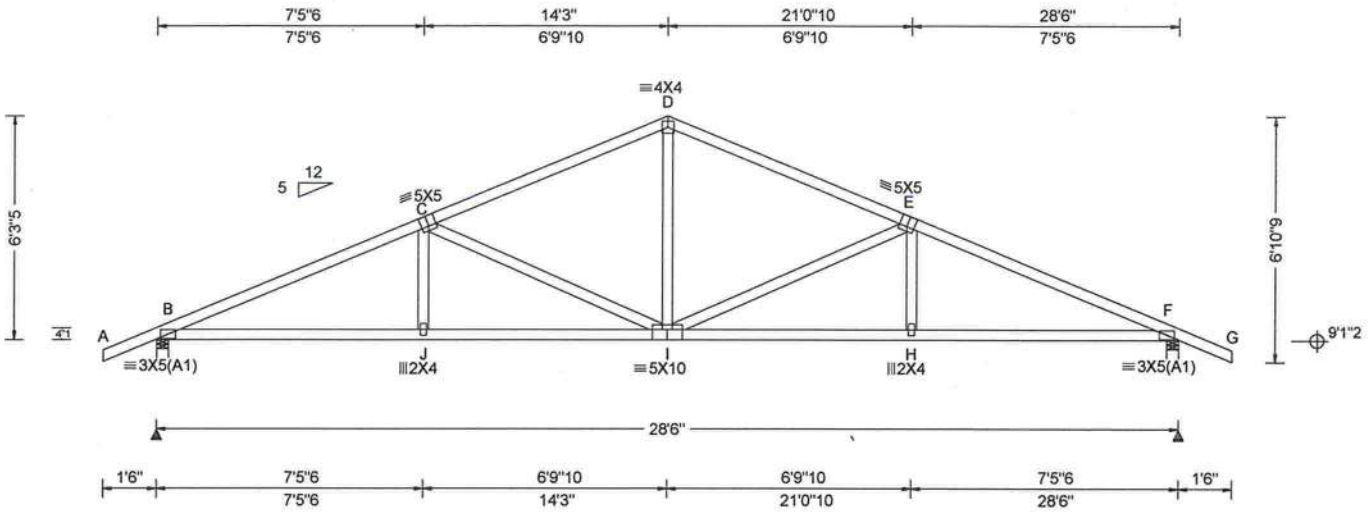


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SEQN: 703281 FROM: CDM	COMN Ply: 1 Qty: 11	Job Number: 23-9363 J. Perkins Truss Label: B02	Cust R 215 JRef: 1XP72150002 T4 DrwNo: 117.23.1111.55750 SSB / DF 04/27/2023
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L# VERT(LL): 0.115 I 999 240 VERT(CL): 0.230 I 999 180 HORZ(LL): 0.046 F - - HORZ(TL): 0.091 F - - Creep Factor: 2.0 Max TC CSI: 0.579 Max BC CSI: 0.664 Max Web CSI: 0.776 VIEW Ver: 22.02.00.0914.12	Maximum Reactions (lbs) Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity B 1263 /- /- /722 /232 /163 F 1263 /- /- /722 /232 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 945 -2302 D - E 764 -1589 C - D 764 -1589 E - F 945 -2302
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 6-3-5.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	2053 -765	I - H	2049 -747
J - I	2049 -767	H - F	2053 -745

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	385 -728	I - E	385 -728
D - I	758 -248		

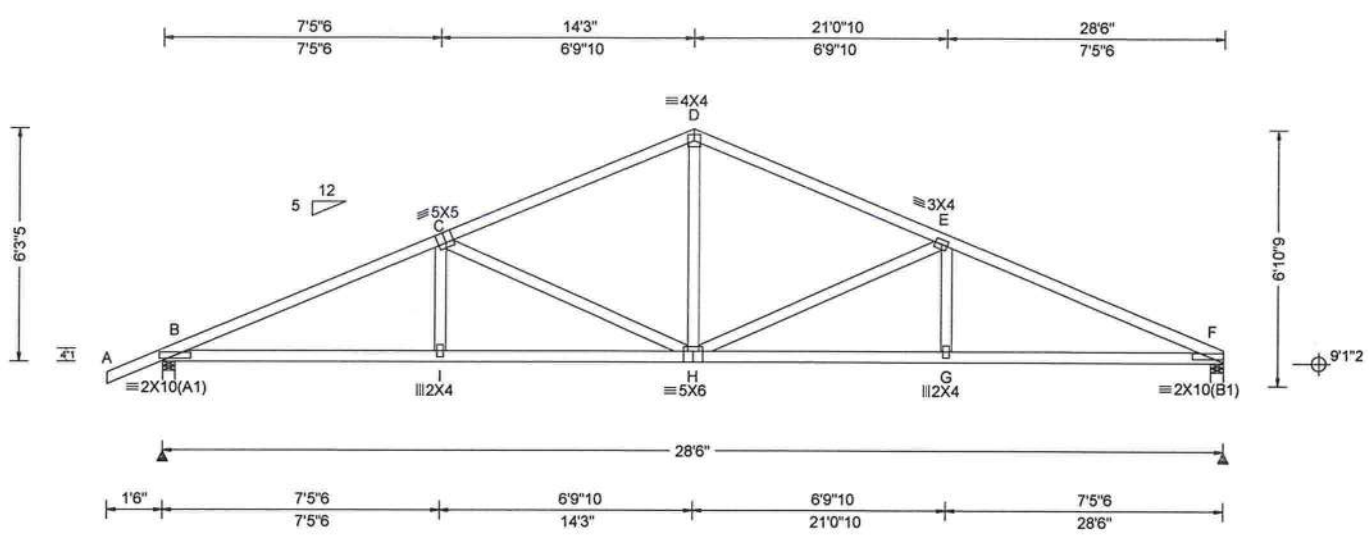


COA #0218
Florida Certificate of Product Approval #FL1999
04/27/2023

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SEQN: 703284	COMN	Ply: 1	Job Number: 23-9363	Cust: R 215	JRef: 1XP72150002	T6
FROM: CDM		Qty: 12	J. Perkins	DrwNo: 117.23.1111.57063		
			Truss Label: B03	SSB / DF		04/27/2023



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.114 H 999 240 VERT(CL): 0.230 H 999 180 HORZ(LL): 0.046 F - - HORZ(TL): 0.092 F - - Creep Factor: 2.0 Max TC CSI: 0.620 Max BC CSI: 0.686 Max Web CSI: 0.804	Maximum Reactions (lbs)																																																																																														
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1266</td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F</td> <td>1161</td> <td>-</td> <td>-</td> <td>722</td> <td>1233</td> <td>1151</td> </tr> <tr> <td colspan="7">Wind reactions based on MWFRS</td> </tr> <tr> <td>B</td> <td colspan="3">Brg Wid = 4.0</td> <td colspan="3">Min Req = 1.5 (Truss)</td> </tr> <tr> <td>F</td> <td colspan="3">Brg Wid = 4.0</td> <td colspan="3">Min Req = 1.5 (Truss)</td> </tr> <tr> <td colspan="7">Bearings B & F are a rigid surface.</td> </tr> <tr> <td colspan="7">Members not listed have forces less than 375#</td> </tr> <tr> <td colspan="7">Maximum Top Chord Forces Per Ply (lbs)</td> </tr> <tr> <th colspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th colspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <td>B - C</td> <td>535</td> <td>-2310</td> <td>D - E</td> <td>451</td> <td>-1597</td> <td></td> <td></td> </tr> <tr> <td>C - D</td> <td>447</td> <td>-1596</td> <td>E - F</td> <td>546</td> <td>-2331</td> <td></td> <td></td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1266	-	-				F	1161	-	-	722	1233	1151	Wind reactions based on MWFRS							B	Brg Wid = 4.0			Min Req = 1.5 (Truss)			F	Brg Wid = 4.0			Min Req = 1.5 (Truss)			Bearings B & F are a rigid surface.							Members not listed have forces less than 375#							Maximum Top Chord Forces Per Ply (lbs)							Chords		Tens.Comp.		Chords		Tens. Comp.		B - C	535	-2310	D - E	451	-1597			C - D	447	-1596	E - F
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 6-3-5.

Maximum Bot Chord Forces Per Ply (lbs)							
Chords		Tens.Comp.		Chords		Tens. Comp.	
B - I	2061	-437	H - G	2079	-433		
I - H	2057	-439	G - F	2084	-431		

Maximum Web Forces Per Ply (lbs)							
Webs		Tens.Comp.		Webs		Tens. Comp.	
C - H	229	-730	H - E	240	-755		
D - H	768	-125					

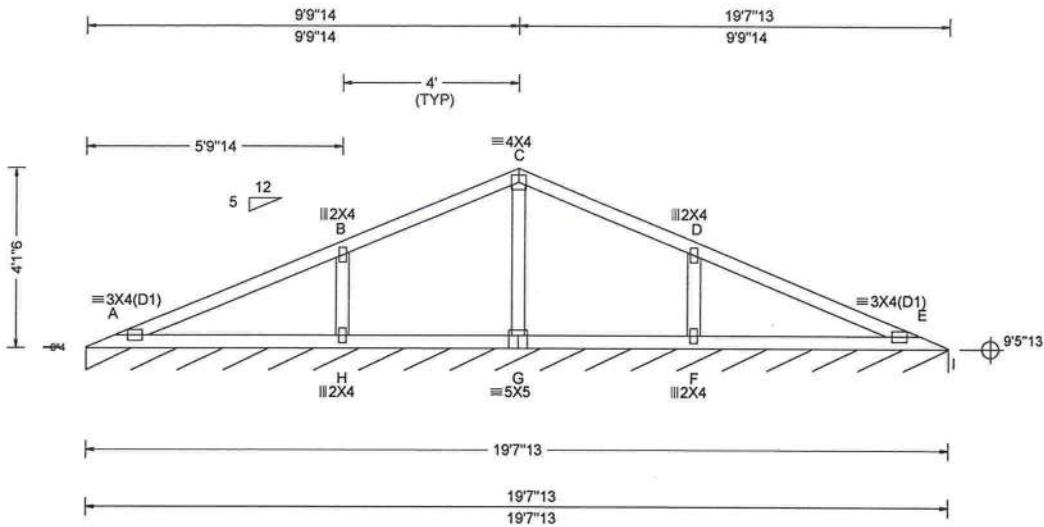


COA #0 218
 Florida Certificate of Product Approval #FL1999
 04/27/2023

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SEQN: 703296	VAL	Ply: 1	Job Number: 23-9363	Cust R 215	JRef: 1XP72150002	T5
FROM: CDM		Qty: 1	J. Perkins	DrwNo: 117.23.1111.58280		
			Truss Label: V01	SSB / DF		04/27/2023



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.023 A 999 240 VERT(CL): 0.047 A 999 180 HORZ(LL): 0.007 A - - HORZ(TL): 0.014 A - - Creep Factor: 2.0 Max TC CSI: 0.444 Max BC CSI: 0.264 Max Web CSI: 0.108 VIEW Ver: 22.02.00.0914.12	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL I* 82 /- /- /41 /12 /4 Wind reactions based on MWFRS I Brg Wid = 235 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

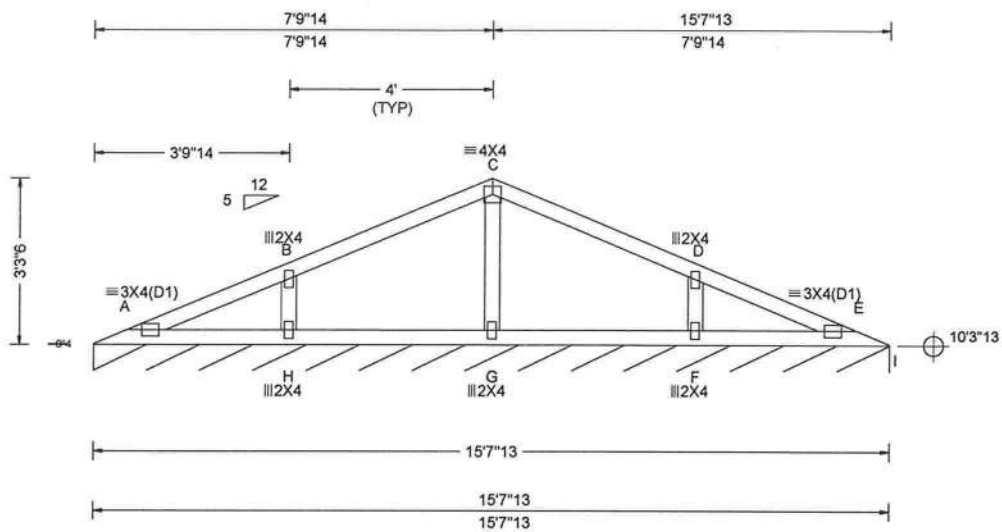
See DWGS VALTN160118 and VAL180160118 for valley details.
The overall height of this truss excluding overhang is 4-1-6.



COA #0 278
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04/27/2023

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF						
				Gravity			Non-Gravity			
Loc	R+	/R-	/Rh	/Rw	/U	/RL				
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	I*	82	-	-	/41	/12	/4
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.005 A 999 240	Wind reactions based on MWFRS						
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.010 A 999 180	I Brg Wid = 187 Min Req = -						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 E - -	Bearing A is a rigid surface.						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.003 E - -	Members not listed have forces less than 375#						
NBCCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0							
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.277							
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.131							
Spacing: 24.0"	MWFRS Parallel Dist: h/2 to h	FT/RT: 20(0)/10(0)	Max Web CSI: 0.067							
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12							
	Loc. from endwall: not in 9.00 ft	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.
 The overall height of this truss excluding overhang is 3-3-6.



COA #0 218
 Florida Certificate of Product Approval #FL1999
 04/27/2023

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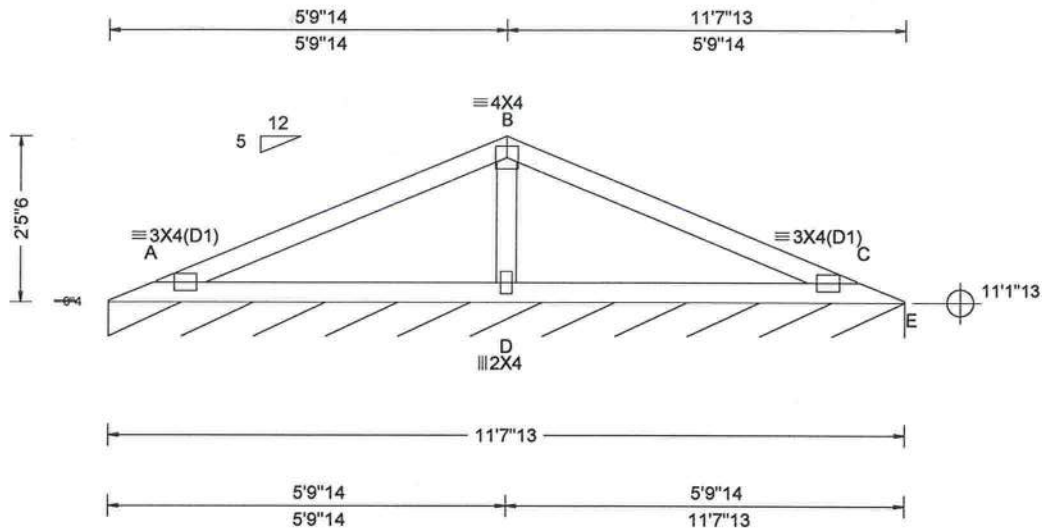
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=-PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.026 A 999 240 VERT(CL): 0.053 A 999 180 HORZ(LL): -0.009 C - - HORZ(TL): 0.018 C - - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.387 Max Web CSI: 0.120 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL E* 81 /- /- /40 /11 /4 Wind reactions based on MWFRS E Brg Wid = 139 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 485 -237 B - C 485 -248 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - D 285 -399 D - C 285 -399

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

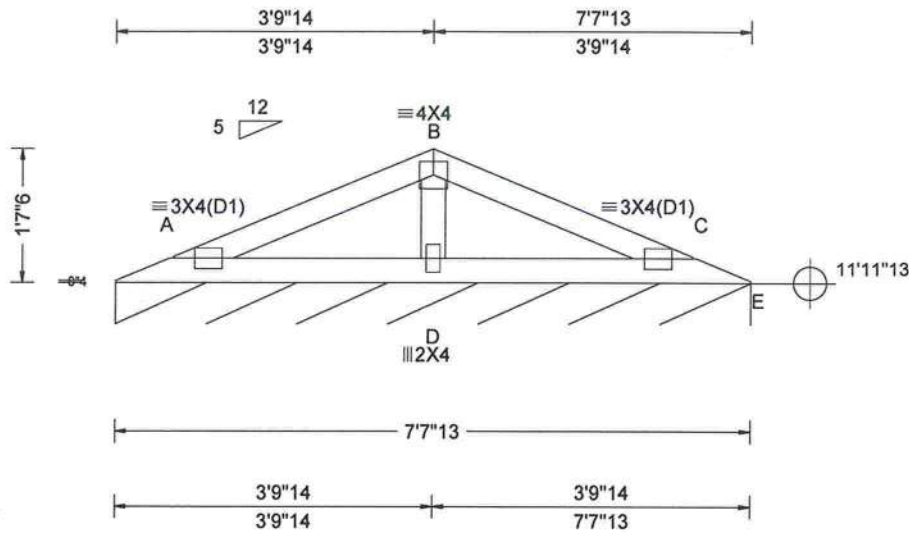
See DWGS VALTN160118 and VAL180160118 for valley details.
The overall height of this truss excluding overhang is 2-5-6.



COA #0 278
Florida Certificate of Product Approval #FL1999
04/27/2023

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 A 999 240 VERT(CL): 0.014 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.155 Max BC CSI: 0.150 Max Web CSI: 0.072 VIEW Ver: 22.02.00.0914.12	Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity E* 81 /- /- /39 /9 /4 Wind reactions based on MWFRS E Brg Wid = 91.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

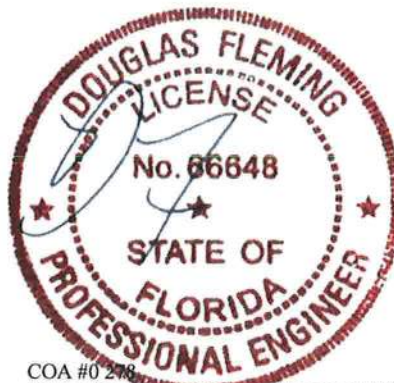
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.
The overall height of this truss excluding overhang is 1-7-6.



COA #0 248
Florida Certificate of Product Approval #FL1999
04/27/2023

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

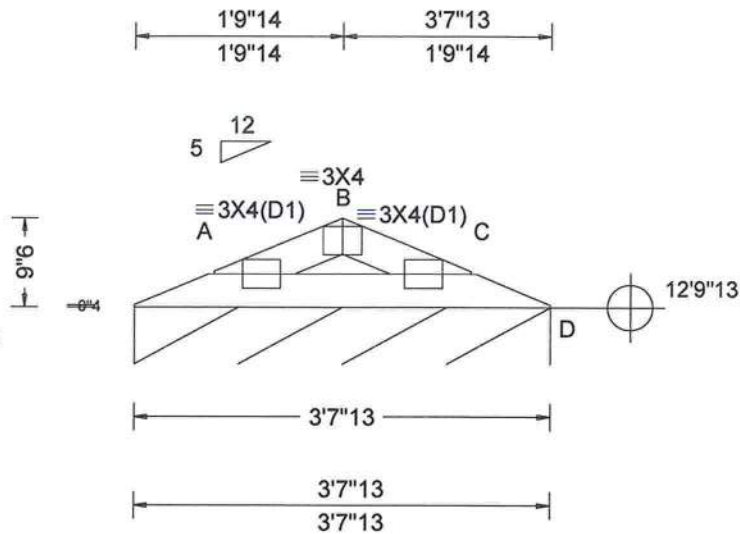
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 A 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.007 A 999 180	D* 81 /- /- /34 /- /3
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 A - -	Wind reactions based on MWFRS
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.002 A - -	D Brg Wid = 43.8 Min Req = -
NBCCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Bearing A is a rigid surface.
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.054	Members not listed have forces less than 375#
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.090	
Spacing: 24.0"	MWFRS Parallel Dist: h to 2h	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12	
	Loc. from endwall: not in 9.00 ft	WAVE		
	GCpi: 0.18			
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

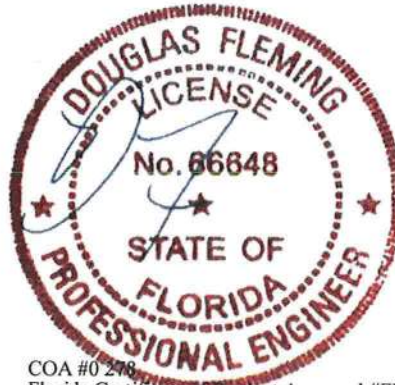
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.

The overall height of this truss excluding overhang is 0-9-6.



COA #0 218
Florida Certificate of Product Approval #FL1999
04/27/2023

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

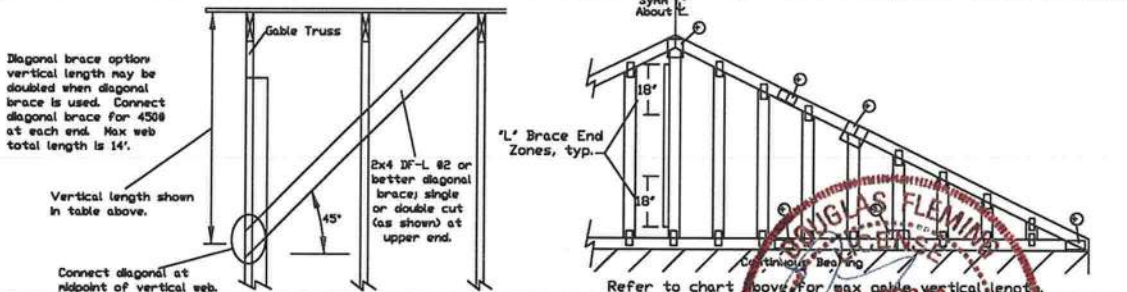
Max Gable Vertical Length	2x4 Gable Vertical Spacing	Brace Species	Brace Grade	No Braces	(1) 1x4 'L' Brace #		(1) 2x4 'L' Brace #		(2) 2x4 'L' Brace #		(1) 2x6 'L' Brace #		(2) 2x6 'L' Brace #		
					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
					Group A		Group B		Group A		Group B		Group A		Group B
24" o.c.	SPF	#1 / #2	#1	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	
					7' 1"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	
					4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		HF	Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	14' 0"
				4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	
				4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	
	SP	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	
				4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
				4' 0"	5' 3"	5' 7"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	14' 0"	
		DFL	Standard	4' 0"	5' 3"	5' 7"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	14' 0"	
				4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
				4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	SPF	#1 / #2	#1	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
					4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	
					4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	
		HF	Standard	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
				4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"		
				4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"		
	SP	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
				4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"		
				4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"	
		DFL	Standard	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
				5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		
				5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		
12" o.c.	SPF	#1 / #2	#1	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"		
					5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	
					5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	
		HF	Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		
				5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"		
				5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"		
	SP	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"			
				5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"			
				5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"			
		DFL	Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"		

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Standard
#3	Stud	#3	Stud
Douglas Fir-Larch		Southern Pine	
#1	Standard	#1	Standard
#2	Stud	#2	Stud
Group B:			
Hem-Fir		Douglas Fir-Larch	
#1 & #2	Standard	#1	Standard
#2	Stud	#2	Stud

1x4 Braces shall be SRB (Stress-Rated Board).
 For 1x4 So. Pine use only Industrial S5 or Industrial 4S Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for SS plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' braces: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 * For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes		
Vertical Length	No Splice	
Less than 4' 0"	1X4 or 2X3	
Greater than 4' 0"	3X4	

+ Refer to common truss design for peak, splice, and heel plates.
 Refer to the Building Designer for conditions not addressed by this detail.

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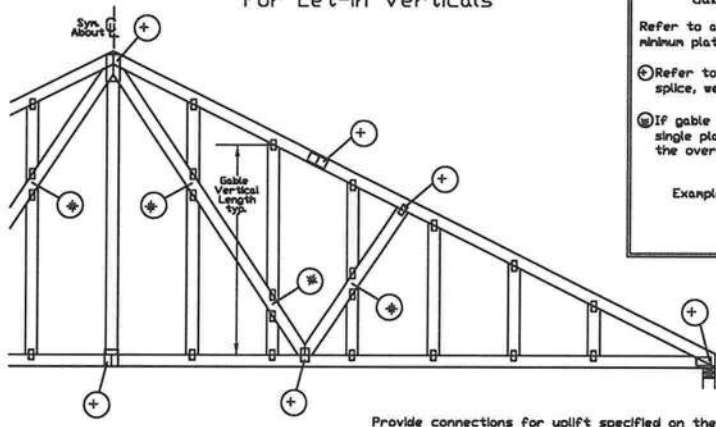
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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineinc.com TPI: www.tpi.org SICA: www.sicacomponents.com ICES: www.ices.org

REF ASCE7-16-GAB14015
 DATE 01/26/2018
 DRWG A14015ENC160118

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0'

Gable Detail For Let-In Verticals

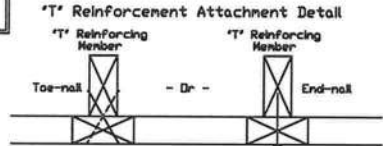


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- Refer to Engineered truss design for peak, splice, web, and heel plates.
- If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

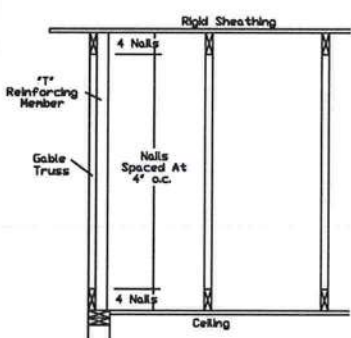
Example:



Provide connections for uplift specified on the engineered truss design. Attach each 'T' reinforcing member with

End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus (4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus (4) toenails in the top and bottom chords.



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mem. Size	'T' Increase
2x4	30 %
2x6	20 %

Examples:
 ASCE 7-10 Wind Speed = 120 mph
 Mean Roof Height = 30 ft, Kzt = 1.00
 Gable Vertical = 24' o.c. SP #3
 'T' Reinforcing Member Size = 2x4
 'T' Brace Increase (From Above) = 30% x 1.30
 (1) 2x4 'L' Brace Length = 8' 7"
 Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030PE100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015PE100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030PE100118

See appropriate Alpine gable detail for maximum 'T' reinforced gable vertical length.



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For more information see this job's general notes page and these web sites:
 ALPINE www.alpine.com TPI www.tpi.com ICCI www.icci.com SICA www.sica.com

MAX. TOT. LD. 60 PSF
 DUR. FAC. ANY
 MAX. SPACING 24.0'

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

*** Attach each valley to every supporting truss with 535# connection or with (1) Simpson H2.5A or equivalent connector for ASCE 7-16 180 mph, 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph, 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.
 Or
 Purlins at 24' o.c. or as otherwise specified on engineer's sealed design
 Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

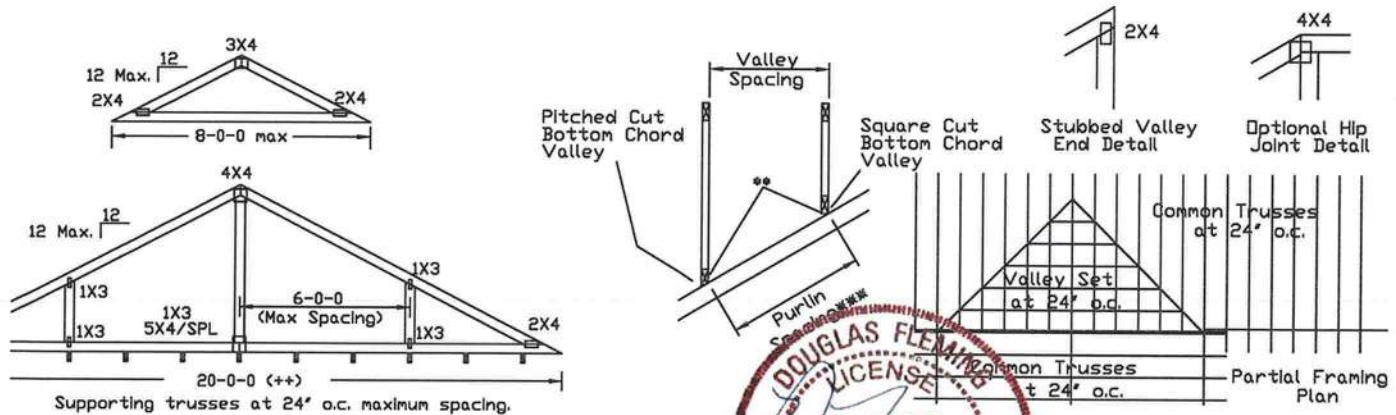
Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".

All plates shown are Alpine Wave Plates.



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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpine.com TPI: www.tpi.org SBCA: www.sbcacomponents.com BCSP: www.bcsp.org

TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0 PSF		
TOT. L.D.	60	55	57PSF		
DURFAC.	1.25/1.33	1.15	1.15		
SPACING	24.0'				

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

*** Attach each valley to every supporting truss with (2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 170 mph for SP (G = 0.55, min.), 155 mph for DF-L (G = 0.50, min.), or 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

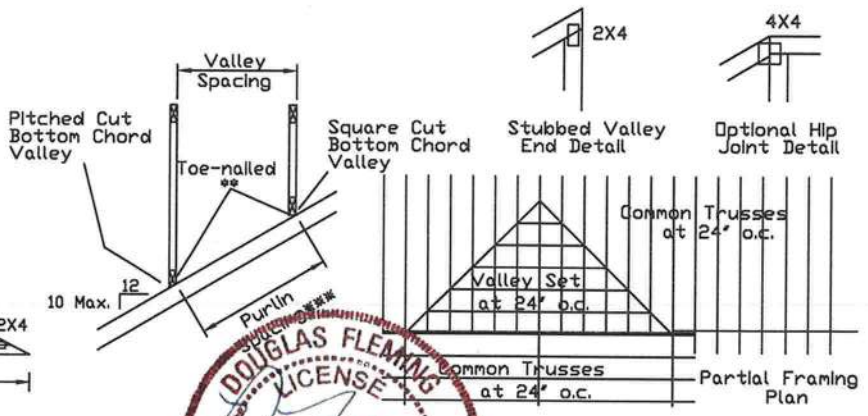
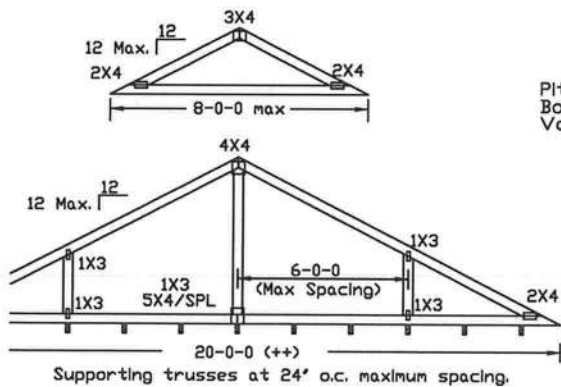
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

- Or
- Purlins at 24' o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING
 FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSC Building Component Safety Information, by TPI and SBCA, for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSC. Unless noted otherwise, top chord shall have properly attached structural sheathing and shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSC sections 23, 27 or 31b, as applicable. Apply plates to top chord of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviations from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
 For more information see this job's general notes page and these web sites:
 ALPINE: www.alphnet.com TPI: www.tpinet.org SBCA: www.sbcacomponents.com ICD: www.icdusa.org

TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VALTNI60118
BC LL	0	0	0 PSF		
TOT. L.D.	60	55	57PSF		
DURFAC.125/133	1.15	1.15			
SPACING	24.0'				



JOB #: 23-9363

Job Name: J. Perkins
 Customer: MO PERKINS
 Designer: Kelly Caudill
 ADDRESS:
 SALESMAN: BW
 : <Not Found>

JOB NO:
 23-9363

PAGE NO:
 1 OF 4

W.B. Howland Truss Co.
 610 11th St. SW
 Live Oak, FL 32064
 (386) 362-1235
 (386) 362-7124 (Fax)
howlandtruss@gmail.com

ROOF PITCH: 5/12
 OVERHANG: 18"
 CEILING: 9"
 EXT. WALLS: 4"
 LOADING: 40psf
 WIND LOAD: 130mph
 EXPOSURE: C
 DATE: 4/27/23

