

DATE 12/05/2005

Columbia County Building Permit

PERMIT 000023925

This Permit Expires One Year From the Date of Issue

APPLICANT JOHN NORRIS

PHONE 755-4038

ADDRESS 351 NW CORWIN GLEN

LAKE CITY

FL

32055

OWNER JOHN JOHNSON

PHONE 755-4038

LAKE CITY

FL

32024

LOCATION OF PROPERTY

90W, TL ON 252B, TR ON TROY ROAD, TR ON RUSSWOOD TERR,

TL ON BETHANY PLACE, AT END ON LEFT

TYPE DEVELOPMENT

SFD, UTILITY

ESTIMATED COST OF CONSTRUCTION

153000.00

HEATED FLOOR AREA

3060.00

TOTAL AREA

4260.00

HEIGHT

.00

FLOOR SLAB

FOUNDATION CONC

WALLS FRAMED

ROOF PITCH

7/12

MAX. HEIGHT

17

LAND USE & ZONING

RSF-2

Minimum Set Back Requirements:

STREET-FRONT

25.00

REAR

15.00

SIDE

10.00

NO. EX.D.U. 0

FLOOD ZONE

X PP

DEVELOPMENT PERMIT NO.

PARCEL ID

10-4S-16-02853-306

SUBDIVISION

RUSSWOOD ESTATES

LOT 6

BLOCK

PHASE

UNIT

TOTAL ACRES

000000908

RG0066597

Culvert Permit No.

Culvert Waiver

Contractor's License Number

BK

LU & Zoning checked by

JH

Approved for Issuance

Applicant/Owner/Contractor

New Resident

Driveway Connection

CULVERT

05-1109-N

Septic Tank Number

COMMENTS: FLOOR HEIGHT LETTER RECEIVED, NOC ON FILE

ELEVATION LETTER REQUIRED BEFORE SLAB

ALTERNATE TERMIT TREATMENT RECEIVED

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power

Foundation

Monolithic

(Footer/Slab)

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

Framing

date/app. by

Rough-in plumbing above slab and below wood floor

date/app. by

Electrical rough-in

date/app. by

Heat & Air Duct

date/app. by

Perf. beam (Lintel)

date/app. by

Permanent power

date/app. by

C.O. Final

date/app. by

Culvert

date/app. by

M/H tie downs, blocking, electricity and plumbing

date/app. by

Utility Pole

date/app. by

Re-roof

date/app. by

M/H Pole

date/app. by

Pump pole

date/app. by

date/app. by

INSPECTORS OFFICE

CLERKS OFFICE

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$

25.00

CULVERT FEE \$

25.00

TOTAL FEE

907.60

MISC. FEES \$

.00

ZONING CERT. FEE \$

50.00

FIRE FEE \$

.00

WASTE FEE \$

.00

BUILDING PERMIT FEE \$

765.00

CERTIFICATION FEE \$

21.30

SURCHARGE FEE \$

21.30

21.30

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES. "WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Department
Culvert Permit
Culvert Permit No. 000000908

DATE 12/05/2005 PARCEL ID # 10-4S-16-02853-306

APPLICANT JOHN NORRIS PHONE 758-3663

ADDRESS 351 NW CORWIN GLEN LAKE CITY FL 32055

OWNER JOHN JOHNSON PHONE 755-4038

ADDRESS 208 SW BETHANY PLACE LAKE CITY FL 32024

CONTRACTOR JOHN NORRIS PHONE 755-4038

LOCATION OF PROPERTY 90W, TL ON 252B, TR ON TROY STREET, TR ON RUSSWOOD TERR, TL ON BETHANY
AT THE END ON LEFT

SUBDIVISION/LOT/BLOCK/PASE/UNIT RUSSWOOD ESTATES 6

SIGNATURE

INSTALLATION REQUIREMENTS

☒ Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface: Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concrete turnouts.

☐ Culvert installation shall conform to the approved site plan standards.

☐ Department of Transportation Permit installation approved standards.

☐ Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



23912

Permit No. _____

Tax Folio No. _____

NOTICE OF TERMINATION OF NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF ALACHUA

The undersigned hereby gives notice that the Notice of Commencement dated September 1, 2005 heretofore recorded on or about September 9, 2005, in Official Records Book 1057, Page 2076, Public Records of Columbia County, Florida, involving construction of improvements upon the property described herein, shall, in accordance with § 713.132, Fla. Stats., cease to be effective on or about December 2, 2005, and the following information is provided in this NOTICE OF TERMINATION OF NOTICE OF COMMENCEMENT.

1. Description of property:
Lot 12, Thornwood, according to the map or plat thereof as recorded in Plat Book 7, Page 202, 203 and 204, of the Public Records of Columbia County, Florida

Address: TBD SW Greenwood Terrace
Ft. White, FL 32038

2. General description of improvement:
single family residence

3. Owner information:
(1) Name and address:
Carl F. Holmberg
8547 S.W. US Hwy 27
Ft. White, FL 32038

(2) Interest in property: Fee Simple
(3) Name and address of fee simple title holder (if other than owner): N/A

4. Contractor: Owner/Builder
Image Development Group
20074 N.W. 258th Drive
High Springs, FL 32643

5. Surety: N/A
6. Lender: Suntrust Mortgage, Inc.
211 E. Silver Springs Blvd., Ocala, FL 34470

7. Persons within the State of Florida designated by owner upon whom notices or other documents may be served as provided by § 713.13(1)(a)7, Fla. Stats.: (name and address)

8. In addition to him/herself, Owner designates

to receive a copy of the lienor's notice as provided in § 713.13(1)(b), Fla. Stats.

9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): December 2, 2005

10. All lienors providing labor, services, or materials for the construction of the above-described improvements have been paid in full, and there are no outstanding amounts due an payable for any labor, services or materials provided for the above-described improvements.

OWNER(S):

Carl F. Holmberg
Judge A. Holmberg

STATE OF FLORIDA
COUNTY OF ALACHUA

Sworn to and subscribed before me in the state and county last aforesaid, this 3 day of December, 2005, by Carl F. Holmberg and Judge A. Holmberg, the owner(s), who are personally known to me or who provided as identification and who took an oath/ did not take an oath.

NOTARY PUBLIC, STATE OF FLORIDA



Joy Glanzer
Commission #DD335922
Expires: Jul 07, 2008
Bonded Thru
Atlantic Bonding Co., Inc.



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DEWITT CASON, CLERK OF COURTS
By _____
Deputy Clerk
Date 12-6-05

EXHIBIT "A"
LEGAL DESCRIPTION

**Lot 12, Thornwood, according to the map or plat thereof as recorded in Plat
Book 7, Page 202, 203 and 204, of the Public Records of Columbia County,
Florida**

Inst:2005030104 Date:12/06/2005 Time:09:35
DC,P.Dewitt Cason,Columbia County B:1067 P:389

FINAL AFFIDAVIT

STATE OF FLORIDA
COUNTY OF ALACHUA

BEFORE ME, the undersigned authority, personally appeared RICHARD PARKER, Managing Member, IMAGE DEVELOPMENT, GROUP, LLC, who, after being by me first duly sworn, deposes and says of his personal knowledge as follows:

(1) He is the Managing Member of Image Development Group, LLC which does business in the State of Florida, hereinafter called the "Contractor".

(2) Contractor has furnished or caused to be furnished labor, material and services for the construction of certain improvements upon the following property:

Lot 12, Thornwood, according to the map or plat thereof as recorded in Plat Book 7, Page 202, 203 and 204, of the Public Records of Columbia County, Florida

Address: TBD SW Greenwood Terrace
Ft. White, FL 32038

(3) All work to be performed has been fully completed, and all liens have been paid in full, except the following listed liens:

AMOUNT DUE

NAME OF LIENOR

N/A

Signed, sealed and delivered this _____ day of December, 2005.

Contractor: IMAGE DEVELOPMENT GROUP, LLC
By: [Signature]
RICHARD PARKER (Signature)

Sworn to and subscribed before me in the
state and county last aforesaid, this _____
day of DECEMBER, 2005,
by RICHARD PARKER

Joy Glanzer
Commission #DD335922
Expires: Jul 07, 2008
Bonded Thru
Atlantic Bonding Co., Inc.



Notary Public, State of Florida
(Print, Type or Stamp commissioned
Name of Notary Public)

Personally Known OR Produced Identification
Type of Identification Produced:

Inst: 2005030103 Date: 12/06/2005 Time: 09:35
DC, P. Dewitt Cason, Columbia County B: 1067 P: 387
STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DEWITT CASON, CLERK OF COURTS
By: [Signature]
Deputy Clerk
Date: 12-6-05



Sworn to (or affirmed) and subscribed before me
this 17th day of November 2005
Personally known ☒ or Produced Identification ☐

Owner/Builder or Agent (Including Contractor)

CONTRACTORS LICENSE
 EXPIRES: JUNE 28, 2000
 BY COMMISSIONER: DR. 333886
 COMPLETION CARD NO.

BOARD OF BUILDING INDUSTRY CONTRACTORS
 1111 PINE STREET
 PHOENIX, ARIZONA 85001



Contractor Signature
Contractor License Number
AX CONN015817 DD 336586
EXPIRES: June 28, 2008
Competency Card Number

RL-0066597

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Property ID Number 10-45-16-02853-306
Subdivision Name Russwood Estates
Driving Directions Hwy 90 west, Turn left on CR 252-B, Turn right on Troy Street, Turn right on Russwood Terrace, Turn left on Bathany Place, AT end on Left
Type of Construction Single Family
Total Acreage .75 Lot size
Do you need a Culvert Permit or Culvert Waiver? Yes 35 Side 63 Side 23 Rear 66
Actual Distance of Structure from Property Lines - Front
Number of Existing Dwellings on Property 0
Number of Existing Dwellings on Property 0
Total Building Height 334 Garage 866
Heated Floor Area 3060
Roof Pitch 7/12
Estimated Cost of Construction 90,000
Unit 3 Phase

Mortgage Lenders Name & Address _____

Architect/Engineer Name & Address.

Bonding Co. Name & Address

Free Simple Owner Name & Address

Address 351 NW Corwin Glen Lake Ct. FL 32055

Contractors Name John Morris

911 Address 208 SW Livingston St Lake City, FL 32204

Owners Name John + Patricia Johnson
Phone 386-755-4038

~~Address 209 SW Justice Glen Lake City FL 32024~~

Applicants Name John & Patricia Johnson Phone 386-755-4038

Elevation letter required

Comments

Flood Zone

Development Permit N/A

Application Approved by - Zoning Official

Application # 001100

Columbia County Building Permit Application

CK# 3539

907.40

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Cal-Tech Testing, Inc.

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

November 1, 2005

John Johnson
Rt. 21, Box 970
Lake City, Florida 32024

Reference: Proposed Residence
Russwood Estates, Unit 3, Lot 6
Columbia County, Florida
Cal-Tech Project No. 05-575

Dear Mr. Johnson,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 6 of unit 3 of Russwood Estates in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed at the site and to provide recommendations for selecting a finished floor elevation.

Based upon the U.S.G.S. "Lake Cite West" quadrangle map, the center of the cul-de-sac adjacent the lot has an elevation of approximately 99.5 feet. Using this elevation as a reference, the ground surface at the center of the building site has an elevation of approximately 97.0 feet, and the lowest point within the building limits has an elevation of approximately 96.0 feet. You propose to place the finished floor approximately 40 inches above this lowest point or at an elevation of approximately 99.3 feet. This finished floor elevation will place the floor about 0.2 feet below the centerline of the adjacent roadway.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown that such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon topography provided by the "Lake City West" quadrangle map, flooding to an elevation of approximately 99.3 feet would simultaneously require the flooding of roughly 600 acres generally to the south of the building site. Additionally, the depth of floodwater would be 20 or more feet in some areas. Flooding to this extent is highly unlikely; therefore, it is our opinion elevating the floor of the residence to 12 inches above the adjacent roadway will not be necessary to substantially reduce the likelihood of flooding.

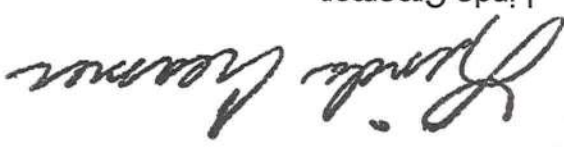
Based upon the FEMA flood map, a small flood area is located approximately 1,000 feet to the south and slightly west of the proposed building site. The flood elevation for this area has not been determined by FEMA; however, based upon the

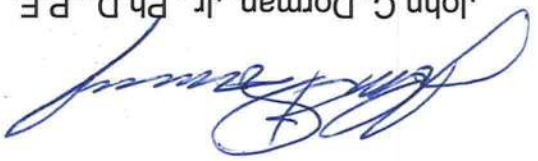
delineated flood area, this elevation is estimated to be about 87.0 to 88.0 feet. Thus the finished floor of the residence will be at least 10 feet above the local flood elevation, and again, elevating the floor of the residence to 12 inches above the adjacent roadway should not be required.

We recommend you maintain the proposed finished floor elevation of approximately 99.3 feet.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.


Linda Creamer
President / CEO


John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer
11/1/05
52612

PREPARED BY AND RETURN TO:

TERRY McDAVID

POST OFFICE BOX 1328

LAKE CITY, FL 32056-1328

Property Appraiser's

Identification Number -R-02853-306

TM File No: 05-829

WARRANTY DEED

This Warranty Deed, made this 21st day of October, 2005, BETWEEN LARRY APPELBY AND SHEILA APPELBY, Husband and Wife whose post office address is 59 Zion Church Rd., Prosperity, SC 29127, grantor*, and JOHN S. JOHNSON and PATRICIA M. JOHNSON, Husband and Wife whose post office address is 204 SW Dusty Glen, Lake City, Florida 32024, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the

sum of Ten Dollars (\$10.00), and other good and valuable

considerations to said grantor in hand paid by said grantee, the

receipt whereof is hereby acknowledged, has granted, bargained

and sold to the said grantee, and grantee's heirs and assigns

forever, the following described land, situate, lying and being

in Columbia County, Florida, to-wit:

Lot 6, Russwood Estates, Unit 3, a subdivision according to the plat thereof recorded in Plat Book 7, Pages 93-94, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

Queen Taylor
(Signature of First Witness)
Queen Taylor
(Typed Name of First Witness)

Shirley Taylor
(Signature of Second Witness)
Shirley Taylor
(Typed Name of Second Witness)

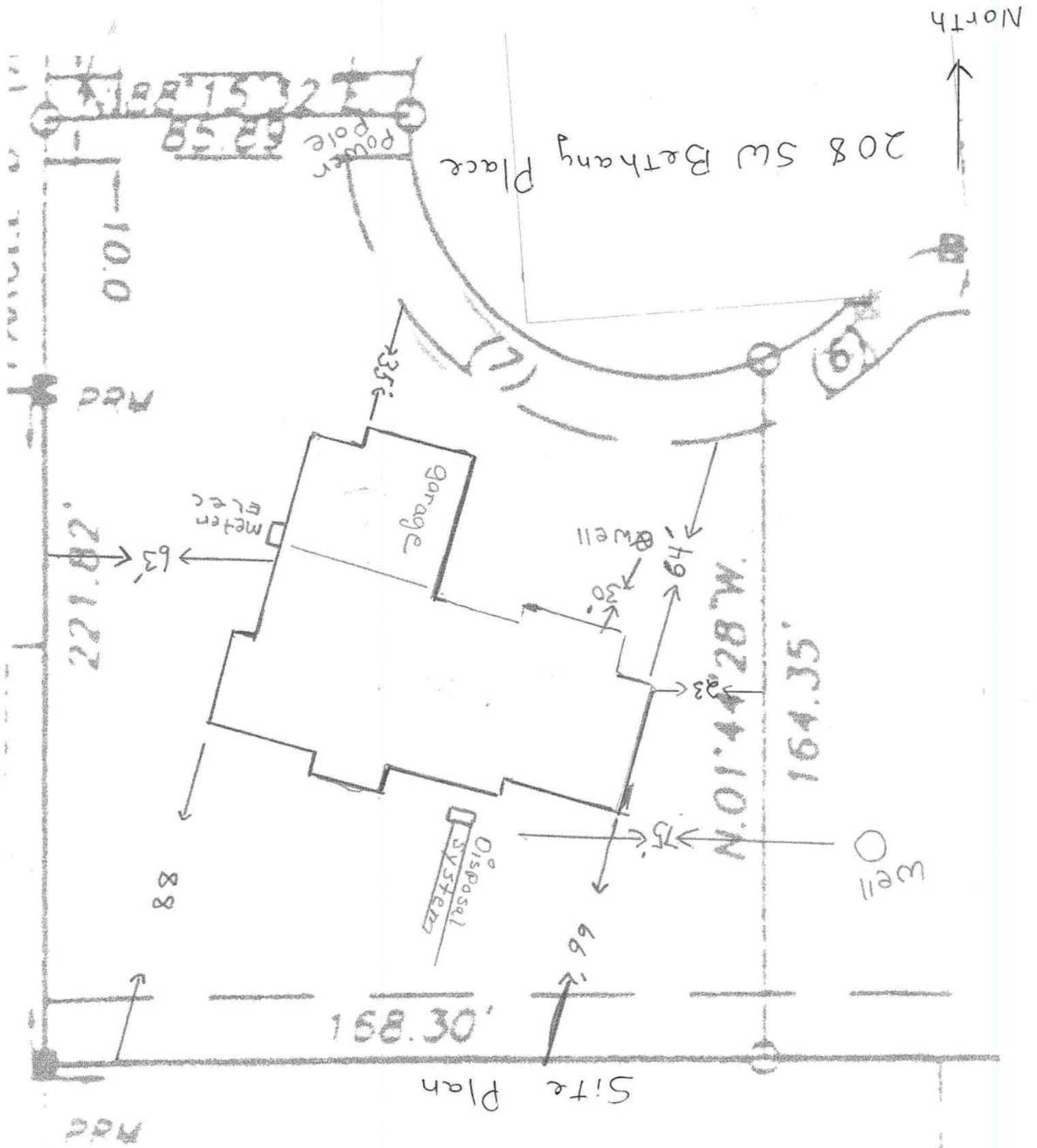
STATE OF South Carolina
COUNTY OF Marion

At day of October, 2005, by Larry Appleby and Sheila Appleby, Husband and wife who is/are personally known to me or who has/have produced Driver's License as identification and who did not take an oath.

My Commission Expires:

Maude H. Moore
Notary Public My Commission Expires January 28, 2006
Printed, typed, or stamped name:
(SEAL)

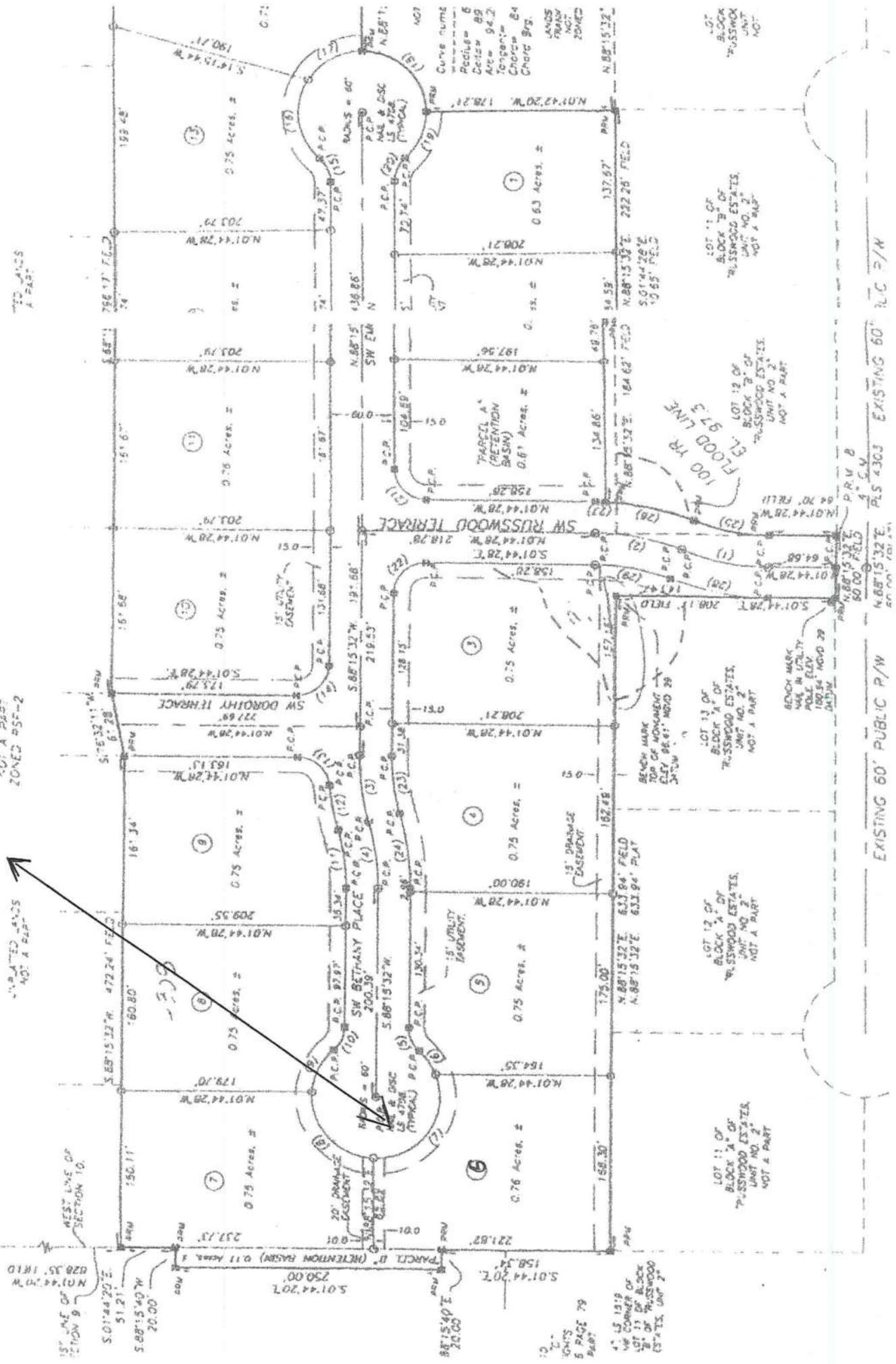
Russwood Estates
Unit 3, Lot 6
John Johnson



Russwood Estates, Unit 3

WA COVER
OF SECTION 10
TOWNSHIP 4 S. 10 E.
RANGE 15 E.
COLUMBIA COUNTY
FLORIDA

OPMER ADVIS OF
L. RUSSELL BAILLY
NOT A PLOT
ZONED PSF-2



THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

COLUMBIA COUNTY, FLORIDA

Tax Parcel ID Number 10-45-16-02853-306

1. Description of property: (legal description of the property and street address or 911 address)

208 SW Bethany Place
Lake City, FL 32024

2. General description of improvement: New Residential Construction

3. Owner Name & Address John & Patricia Johnson
204 SW Dusty Glen, Lake City, FL 32024

4. Name & Address of Fee Simple Owner (if other than owner): Interest in Property Construction of new home

5. Contractor Name John Morris
Address 351 NW Corwin Glen Lake City FL 32055
Phone Number 386-758-3663

6. Surety Holders Name
Address
Amount of Bond
Lender Name
Inst: 2005026835 Date: 10/27/2005 Time: 12:10
DC, P. Dewitt Cason, Columbia County B: 1063 P: 696

7. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name
Address
Phone Number

9. In addition to himself/herself the owner designates _____ to receive a copy of the Lessor's Notice as provided in Section 713.13 (1) - (a) 7. Phone Number of the designee
10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified))

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner
John & Johnson

NOTARY STAMP/SEAL

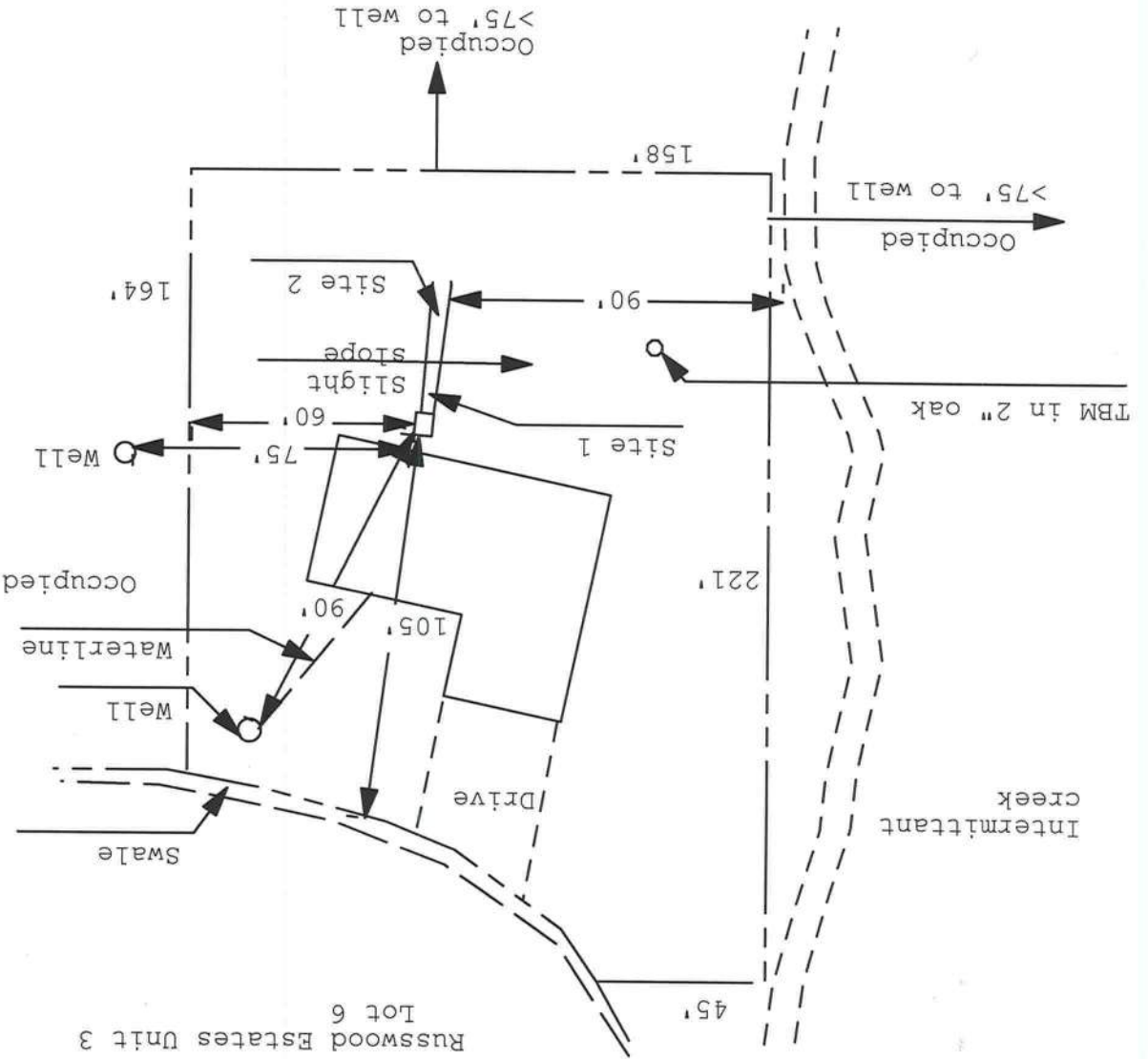
Sworn to (or affirmed) and subscribed before day of 27th of October, 2005

Signature of Notary
Lisa T. Richardson
MY COMMISSION # DD233523 EXPIRES July 29, 2007
BONDED TO THE PUBLIC BY THE STATE OF FLORIDA

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 05-1169N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

JOHNSON/CR 05-3146



1 inch = 50 feet

CPHU

Columbia

By Mr. J. M. J.

Date 10/13/05

Date 12/27/05

Not Approved

Site Plan Submitted By

Plan Approved

Notes:

RON E. BIAS WELL DRILLING

Route 2, Box 5340
Ft. White, Florida 32038
(904) 497-1045
Mobile: 364-9233

No.

Date

Name

Address

Phone

DESCRIPTION

Dis 4" deep well
1 1/2" dia. pipe 25' GP
We dug pipe
300' total on 200' Miller
E-guise
35' of all down down
with each 7' of pressure
+ 1/2" valve

Total

Deposit

Balance

Date Wanted

Authorized By

Received By

Ron E. Bias

FLORIDA ENERGY EFFICIENCY CODE
FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	509063JohnsonJonnyUnit3Lot#6
Address:	Lot: 6, Sub: Bethany Place, Plat:
City, State:	Lake City, FL 32024-
Owner:	Spec House
Climate Zone:	North
Builder:	Johnny Johnson
Permitting Office:	Columbia
Permit Number:	23925
Jurisdiction Number:	221008

1. New construction or existing	Single family or multi-family	1
2. Single family or multi-family	Single family	1
3. Number of units, if multi-family	1	4
4. Number of Bedrooms	4	Yes
5. Is this a worst case?	Yes	3060 ft²
6. Conditioned floor area (ft²)	Single Pane	Double Pane
7. Glass area & type	0.0 ft²	0.0 ft²
a. Clear glass, default U-factor	0.0 ft²	0.0 ft²
b. Default tint, default U-factor	0.0 ft²	0.0 ft²
c. Labeled U-factor or SHGC	0.0 ft²	0.0 ft²
8. Floor types	R=0.0, 277.0(p) ft	
a. Slab-On-Grade Edge Insulation		
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 302.0 ft²	
b. Face Brick, Wood, Exterior	R=13.0, 1806.0 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 3112.0 ft²	
b. N/A		
c. N/A		
d. N/A		
e. N/A		
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 310.0 ft	
b. N/A		
12. Cooling systems		
a. Central Unit		
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump		
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance		
b. N/A		
c. Conservation credits		
e. (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits		
(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.12	Total as-built points: 40870	Total base points: 42615	PASS
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DATE: 9/28/05 Ben Sparks Ben Sparks I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.	DATE: 9/28/05 Ben Sparks Ben Sparks I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.	DATE: _____ BUILDING OFFICIAL: _____ Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.
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Residential Whole Building Performance Method A - Details

PERMIT #:

BASE		AS-BUILT	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		Overhang Ornt Len Hgt Area X SPM X SOF = Points Type/SC	
.18	3060.0	20.04	11038.0
Double, Clear S 9.0 8.0 20.0 35.87 0.50 SW 1.5 8.0 36.0 40.16 0.95 NW 99.0 8.5 10.0 25.97 0.52 SW 12.5 7.5 45.0 40.16 0.41 SE 21.8 8.0 15.0 42.75 0.38 SE 1.5 9.0 15.0 42.75 0.96 SW 1.5 9.0 36.0 40.16 0.96 SW 1.5 6.0 8.0 40.16 0.89 SW 1.5 7.0 15.0 40.16 0.92 NW 1.5 7.0 30.0 25.97 0.95 NE 1.5 7.0 20.0 29.56 0.94 NE 6.5 9.0 30.0 29.56 0.66 NE 6.5 3.5 28.0 29.56 0.48 NE 0.0 0.0 30.0 29.56 1.00 NE 0.0 0.0 14.0 29.56 1.00 SE 1.5 6.0 16.0 42.75 0.88 SE 1.5 6.0 8.0 42.75 0.88 Double, Clear 360.6		Double, Clear S 9.0 8.0 20.0 35.87 0.50 SW 1.5 8.0 36.0 40.16 0.95 NW 99.0 8.5 10.0 25.97 0.52 SW 12.5 7.5 45.0 40.16 0.41 SE 21.8 8.0 15.0 42.75 0.38 SE 1.5 9.0 15.0 42.75 0.96 SW 1.5 9.0 36.0 40.16 0.96 SW 1.5 6.0 8.0 40.16 0.89 SW 1.5 7.0 15.0 40.16 0.92 NW 1.5 7.0 30.0 25.97 0.95 NE 1.5 7.0 20.0 29.56 0.94 NE 6.5 9.0 30.0 29.56 0.66 NE 6.5 3.5 28.0 29.56 0.48 NE 0.0 0.0 30.0 29.56 1.00 NE 0.0 0.0 14.0 29.56 1.00 SE 1.5 6.0 16.0 42.75 0.88 SE 1.5 6.0 8.0 42.75 0.88 Double, Clear 360.6	
WALL TYPES Area X BSPM = Points		Type R-Value Area X SPM = Points	
Adjacent	302.0	0.70	211.4
Exterior	1806.0	1.70	3070.2
Base Total: 2108.0 3281.6		As-Built Total: 2108.0 813.3	
DOOR TYPES Area X BSPM = Points		Type R-Value Area X SPM = Points	
Adjacent	20.0	2.40	48.0
Exterior	50.0	6.10	305.0
Base Total: 70.0 353.0		As-Built Total: 70.0 237.0	
CEILING TYPES Area X BSPM = Points		Type R-Value Area X SPM X SCM = Points	
Under Attic	3060.0	1.73	5293.8
Base Total: 3060.0 5293.8		As-Built Total: 3112.0 5383.8	

SUMMER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 6, Sub: Bethany Place, Plat: , Lake City, FL, 32024- PERMIT #:

BASE					AS-BUILT				
FLOOR TYPES					Type				
Area X BSPM = Points					R-Value Area X SPM = Points				
Slab	277.0(p)	-37.0	-10249.0		Slab-On-Grade Edge Insulation	0.0	277.0(p)	-41.20	-11412.4
Raised	0.0	0.00							
Base Total:					As-Built Total:				
INFILTRATION Area X BSPM = Points					Area X SPM = Points				
Summer Base Points:					Summer As-Built Points:				
Total Summer X System = Cooling Points					Total X Cap X Duct X System X Credit = Cooling Points				
					(DM x DSM x AHU)				
40960.0	0.4266	17473.6	36441.6	1.000	1.000	(1.090 x 1.147 x 1.00)	0.325	1.000	14809.3

Residential Whole Building Performance Method A - Details

PERMIT #:

BASE										AS-BUILT									
GLASS TYPES										AS-BUILT									
.18 X Conditioned X BWPM = Points Floor Area										Type/SC Ornt Len Hgt Area X WPM X WOF = Points Overhang									
.18										.18									
3060.0										376.0									
12.74										3.76									
7017.2										8946.3									

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 6, Sub: Bethany Place, Plat: , Lake City, FL, 32024-
PERMIT #:

BASE					AS-BUILT				
FLOOR TYPES Area X BWPM = Points					Type R-Value Area X WPM = Points				
Slab	277.0(p)	8.9	2465.3		Slab-On-Grade Edge Insulation	0.0	277.0(p)	18.80	5207.6
Raised	0.0	0.00	0.0						
Base Total: 2465.3					As-Built Total: 277.0 5207.6				
INFILTRATION Area X BWPM = Points					Area X WPM = Points				
3060.0 -0.59 -1805.4					3060.0 -0.59 -1805.4				
Winter Base Points: 22564.5					Winter As-Built Points: 26038.7				
Total Winter X System = Heating Points					Total X Cap X Duct X System X Credit = Heating Points				
Multiplier = Points					Component Ratio Multiplier Multiplier Multiplier (DM x DSM x AHU)				
22564.5	0.6274	14157.0	26038.7	1.00	1.250	0.467	1.000	1.000	15200.0
22564.5	0.6274	14157.0	26038.7	1.00	1.250	0.467	1.000	1.000	15200.0

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 6, Sub: Bethany Place, Plat: , Lake City, FL, 32024- PERMIT #:

BASE		AS-BUILT				
WATER HEATING						
Number of Bedrooms X Multiplier = Total		Tank	EF	Number of Bedrooms	Tank X Multiplier X Credit = Total	
		Volume		Ratio		
4		40.0	0.89	4	1.00	2715.15
2746.00					1.00	10860.6
10984.0		As-Built Total:				

Code Compliance Checklist
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 6, Sub: Bethany Place, Plat: , Lake City, FL, 32024-
PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: 3 cfm/sq.ft. window area; 5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; penetrations; between wall panels & top/bottom plates; between walls and floor.	
Floors	606.1.ABC.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members.	
Ceilings	606.1.ABC.1.2.3	EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Multi-story Houses	606.1.ABC.1.2.5	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Additional Infiltration reqts	606.1.ABC.1.3	Air barrier on perimeter of floor cavity between floors.	
		Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.6

The higher the score, the more efficient the home.

Spec House, Lot: 6, Sub: Bethany Place, Plat: , Lake City, FL, 32024-

- 1. New construction or existing
- 2. Single family or multi-family
- 3. Number of units, if multi-family
- 4. Number of Bedrooms
- 5. Is this a worst case?
- 6. Conditioned floor area (ft²)
- 7. Glass area & type
 - a. Clear glass, default U-factor
 - b. Default tint, default U-factor
 - c. Labeled U-factor or SHGC
- 8. Floor types
 - a. Slab-On-Grade Edge Insulation
 - b. N/A
 - c. N/A
- 9. Wall types
 - a. Frame, Wood, Adjacent
 - b. Face Brick, Wood, Exterior
 - c. N/A
 - d. N/A
 - e. N/A
- 10. Ceiling types
 - a. Under Attic
 - b. N/A
 - c. N/A
- 11. Ducts
 - a. Sup: Unc. Ret. Unc. AH: Garage
 - b. N/A

- 12. Cooling systems
 - a. Central Unit
 - b. N/A
 - c. N/A
- 13. Heating systems
 - a. Electric Heat Pump
 - b. N/A
 - c. N/A
- 14. Hot water systems
 - a. Electric Resistance
 - b. N/A
 - c. N/A
- 15. HVAC credits
 - c. Conservation credits
 - (HR-Heat recovery, Solar
 - DHP-Dedicated heat pump)
 - HVAC credits
 - (CF-Ceiling fan, CV-Cross ventilation,
 - HF-Whole house fan,
 - PT-Programmable Thermostat,
 - MZ-C-Multizone cooling,
 - MZ-H-Multizone heating)

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (HEEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs, Energy Gauge Division, version: FLR2PB v3.4)

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02853-306

Building permit No. 000023925

Use Classification SFD, UTILITY

Fire: 18.17

Permit Holder JOHN NORRIS

Waste: 0.00

Owner of Building JOHN JOHNSON

Total: 18.17

Location: 208 SW BETHANY PLACE(RUSSWOOD EST., LOT 6)

Date: 09/12/2006

Ferry Dick

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)





Cal-Tech Testing, Inc.

- Engineering
 - Geotechnical
 - Environmental
- LABORATORIES

P.O. Box 1625 • Lake City, FL 32056
4784 Rosselle Street • Jacksonville, FL 32254
2230 Greensboro Highway • Quincy, FL 32351

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 381-8901 • Fax (904) 381-8902
Tel. (850) 442-3495 • Fax (850) 442-4008

September 7, 2006

John Johnson
204 S. W. Dusty Glen
Lake City, Florida 32024

Reference: Existing Residence

Russwood Estates, Unit 3, Lot 6
Columbia County, Florida
Cal-Tech Project No. 05-575

Dear Mr. Johnson,

Cal-Tech Testing, Inc. has completed an investigation to verify the as-built finished floor elevation of the residence at the referenced location.

Based upon the U.S.G.S. "Lake Cite West" quadrangle map, the center of the cul-de-sac adjacent the lot has an elevation of approximately 99.5 feet. Using this elevation as a reference, the finished floor elevation was found to be approximately 99.82 feet. You initially proposed to place the finished floor elevation at approximately 99.3 feet; therefore, the as-built floor elevation is roughly 6 inches above the elevation initially proposed.

It is our opinion this finished floor elevation is sufficient to substantially reduce the likelihood of flooding at the site. Additionally, the finished floor elevation is approximately 6 inches above the recommended minimum floor elevation.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.

Geotechnical Engineer

9/7/06

52612

ATTENTION: New Pressure-Treated Woods Require Additional Corrosion Resistance!

CHEMICAL CHANGES IN PRESSURE-TREATED WOODS

Pressure-treated wood is a building material most commonly used in decks, fences and homes to protect against insects and fungal decay. For a number of years Chromated Copper Arsenate (CCA-C) has been the primary chemical treatment used in pressure-treated wood. Recently, the treated wood industry voluntarily decided to phase out CCA-C pressure-treated wood for residential and general consumer use due to the negative publicity focused on the use of arsenic in CCA-C. As of December 31, 2003, CCA-C will no longer be produced for residential or general consumer use. Several new pressure-treated wood alternatives have been created to replace CCA-C. Although most of the new treatments no longer contain arsenic, many of the new preservatives are generally more corrosive to steel connectors, anchors and fasteners.

SIMPSON STRONG-TIE CONNECTORS, ANCHORS AND FASTENERS

Simpson Strong-Tie, the leader in structural systems technology and product innovation, conducted testing to measure the corrosion of its products when used in contact with the new pressure-treated woods. Based on the test results, the standard G60 galvanized connectors and Simpson Strong-Tie standard G90 galvanized connectors and fasteners are no longer appropriate to use with many of the new types of pressure-treated wood. The new treated wood products require connectors, anchors and fasteners with greater resistance to corrosion.

SIMPSON STRONG-TIE SOLUTION

Stainless Steel connectors provide the most resistance to corrosion and should be used whenever practical in corrosive environments. If Stainless Steel is not practical, Simpson Strong-Tie recommends its ZMAX™ (G185) galvanized products with most types of pressure-treated wood as a minimum standard to help protect against corrosion. ZMAX™ products are coated with a thicker layer of zinc than our standard products. This additional zinc coating improves the product's corrosion resistance. The chart on the back of this page provides a list of the most common varieties of pressure-treated wood and indicates which Simpson Strong-Tie product coating is recommended for each type of treated wood. (Pressure treated wood chemical manufacturers also recommend G185 as the minimum coating to help protect against corrosion. Visit the website of the chemical manufacturer for more information.)

OUTDOOR APPLICATIONS AND CORROSION

You should be aware that the new pressure treated wood chemicals are not the only form of corrosion that can affect connectors. When connectors are used in outdoor applications in which they are exposed to the elements (rain, water, soil, fertilizers, etc.), corrosion also occurs. Whenever connectors, anchors or fasteners are used in outdoor applications, a minimum of ZMAX™ (G185) galvanized connectors should be used.

PLEASE NOTE: Many of the new Pressure Treated Woods use chemicals that are corrosive to steel. By selecting connectors that offer greater corrosion resistance (Stainless Steel, Post Hot-Dip Galvanized, or ZMAX™) you can extend the service life of your connectors. However, corrosion will still occur. You should perform periodic inspection of your connectors and fasteners to insure their strength is not being adversely affected by corrosion. In some cases, it may be necessary to have a local professional perform the inspections. Because of the many variables involved, Simpson Strong-Tie cannot provide estimates on service life of connectors, anchors or fasteners.

USE ZMAX FOR ADDED CORROSION RESISTANCE!



Look for the teal color on our box labels to identify products that have extra corrosion resistance.



Visit www.strongtie.com/info for additional critical information on this topic.

Strong-Tie's Web site, www.strongtie.com

Simpson Strong-Tie manufactures a variety of connectors, anchors and fasteners. For the full line of product offerings, call 1-800-999-5099 or visit Simpson

- ☐ Refer to the treated wood producer to verify suitability.
- ☐ Make sure all pressure-treated wood is dry before installation.
- ☐ Always use connectors and fasteners made with the same type of finish. Use Stainless Steel fasteners with Stainless Steel connectors. Use hot-dip galvanized fasteners with ZMAX™ and post hot-dip galvanized connectors.
- ☐ Match the type of pressure-treated wood with the appropriate Simpson Strong-Tie product coating. (Refer to the above chart for specific recommendations)
- ☐ Determine the type of pressure-treated wood you're using. (List not intended to be comprehensive)

CONSTRUCTION CHECKLIST:

A CRITICAL NOTE ABOUT FASTENERS AND CONNECTORS
Use **ONLY** Stainless Steel fasteners with Stainless Steel connectors.
Use **ONLY** hot-dip galvanized fasteners with ZMAX™ and post hot-dip galvanized connectors.

These coatings or metals provide additional corrosion protection.

Recommendations shown are based on accelerated testing per AWWA Standard E12-94, and may or may not have a relation to actual service life.

Simpson Strong-Tie Product Finishes	Standard* (G90)	ZMAX™ (G185)	Post Hot-Dip** Galvanized (HDG)	SST300™ (Stainless Steel)
Untreated Wood	✓	✓	✓	✓
Chromated Copper Arsenate (CCA-C)	✓	✓	✓	✓
DOT Sodium Borate (SBX)	✓	✓	✓	✓
Alkaline Copper Quat ACO-C and ACO-D (Carbonate)	✓	✓	✓	✓
Copper Azole CBA-A and CA-B	✓	✓	✓	✓
Other Borate (Non-DOT)	✓	✓	✓	✓
Ammoniacal Copper Zinc Arsenate (ACZA)	✓	✓	✓	✓
Other Pressure-Treated Woods	✓	✓	✓	✓

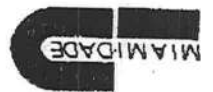
SIMPSON STRONG-TIE PRODUCT RECOMMENDATIONS WITH PRESSURE-TREATED WOOD

The following chart was created based on testing performed by Simpson Strong-Tie. If the pressure treated wood you are using does not appear by name, then we recommend you use the "Other Pressure-Treated Woods" column and use only Stainless Steel connectors and fasteners.



*The Simpson Strong-Tie standard G90 galvanized connectors as well as other U.S. standard 660 galvanized connectors used with untreated wood and CCA-C treated wood are still considered appropriate.

**Simpson Post Hot-Dip Galvanized connectors meet ASTM A123 standards. Simpson HDG fasteners meet ASTM A153 standards.



BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

PCT Industries
1070 Technology Drive
Nokomis, FL 34274

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (in Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series HR-201 Aluminum Horizontal Sliding Window with STD Meeting Rail
APPROVAL DOCUMENT: Drawing No. 268-1, titled "Alum. Horiz. Roller (STD, MTC, Rail)", sheets 1 through 9, prepared, signed and sealed by Robert Clark, P.E. dated 5/10/02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

The submitted documentation was reviewed by Raul Rodriguez



COMPARATIVE ANALYSIS - TABLE 1. A

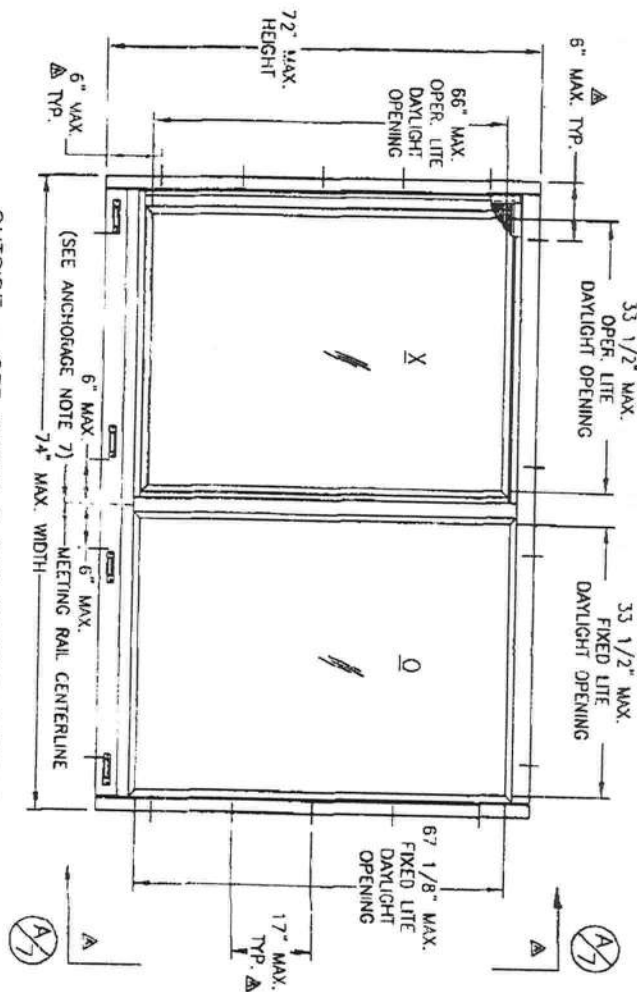
GLASS TYPE: A, DSB - 1/8" ANNEALED (FTL-3341) 3/16" TEMPERED (FTL-3339) 1/2" IG (3/16A, 3/16S, 1/8A (FTL-3340))
B, 3/16" ANNEALED (FTL-3339) D, 1/2" IG - 3/16A, 3/16S, 1/8A (FTL-3340)

WINDOW WIDTH	WINDOW HEIGHT									
	NEG	POS	NEG	POS	NEG	POS	NEG	POS	NEG	POS
26,500 (4)	-75.0	56.0	-75.0	56.0	-75.0	56.0	-67.9	56.0	-53.0	53.0
37,000 (4)	A	-75.0	56.0	-75.0	56.0	-67.9	56.0	-53.0	53.0	45.7
	B	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
44,000 (4)	A	-135.0	56.0	-135.0	56.0	-118.8	56.0	-92.8	56.0	45.7
	B	-135.0	56.0	-135.0	56.0	-118.8	56.0	-92.8	56.0	45.7
53,125 (4)	A	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
	B	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
63,000 (4)	A	-75.0	56.0	-75.0	56.0	-67.9	56.0	-53.0	53.0	45.7
	B	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
74,000 (4)	A	-135.0	56.0	-135.0	56.0	-118.8	56.0	-92.8	56.0	45.7
	B	-135.0	56.0	-135.0	56.0	-118.8	56.0	-92.8	56.0	45.7
74,000 (4)	A	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
	B	-120.0	56.0	-120.0	56.0	-101.9	56.0	-78.5	56.0	45.7
74,000 (4)	A	-56.8	56.0	-51.2	51.2	-43.2	43.2	-36.7	36.7	27.1
	B	-56.8	56.0	-51.2	51.2	-43.2	43.2	-36.7	36.7	27.1
74,000 (4)	A	-99.0	56.0	-99.0	56.0	-95.4	56.0	-80.6	56.0	51.6
	B	-99.0	56.0	-99.0	56.0	-95.4	56.0	-80.6	56.0	51.6
74,000 (4)	A	-84.9	56.0	-84.9	56.0	-81.8	56.0	-68.1	56.0	44.3
	B	-84.9	56.0	-84.9	56.0	-81.8	56.0	-68.1	56.0	44.3
74,000 (4)	A	-38.8	38.8	-38.8	38.8	-38.8	38.8	-38.8	38.8	24.3
	B	-38.8	38.8	-38.8	38.8	-38.8	38.8	-38.8	38.8	24.3
74,000 (4)	A	-58.2	56.0	-58.2	56.0	-58.2	56.0	-58.2	56.0	45.0
	B	-58.2	56.0	-58.2	56.0	-58.2	56.0	-58.2	56.0	45.0
74,000 (4)	A	-58.2	56.0	-58.2	56.0	-58.2	56.0	-58.2	56.0	45.0
	B	-58.2	56.0	-58.2	56.0	-58.2	56.0	-58.2	56.0	45.0
74,000 (4)	A	-28.5	28.5	-27.6	27.6	-27.6	27.6	-24.7	24.7	18.6
	B	-28.5	28.5	-27.6	27.6	-27.6	27.6	-24.7	24.7	18.6
74,000 (4)	A	-42.7	42.7	-41.4	41.4	-41.4	41.4	-38.5	38.5	33.5
	B	-42.7	42.7	-41.4	41.4	-41.4	41.4	-38.5	38.5	33.5
74,000 (4)	A	-49.8	49.8	-48.3	48.3	-48.3	48.3	-46.5	46.5	39.1
	B	-49.8	49.8	-48.3	48.3	-48.3	48.3	-46.5	46.5	39.1
74,000 (4)	A	-21.9	21.9	-20.0	20.0	-20.0	20.0	-20.0	20.0	20.0
	B	-21.9	21.9	-20.0	20.0	-20.0	20.0	-20.0	20.0	20.0
74,000 (4)	A	-32.9	32.9	-30.0	30.0	-30.0	30.0	-30.0	30.0	30.0
	B	-32.9	32.9	-30.0	30.0	-30.0	30.0	-30.0	30.0	30.0
74,000 (4)	A	-38.4	38.4	-35.0	35.0	-35.0	35.0	-35.0	35.0	35.0
	B	-38.4	38.4	-35.0	35.0	-35.0	35.0	-35.0	35.0	35.0
74,000 (4)	A	-32.9	32.9	-30.0	30.0	-30.0	30.0	-30.0	30.0	30.0
	B	-32.9	32.9	-30.0	30.0	-30.0	30.0	-30.0	30.0	30.0

TABLE NOTE: NUMBERS IN () PARENTHESES ARE MINIMUM QUANTITY OF ANCHORS PER SIDE.

NON-IMPACT WINDOW NOTES:

- GLAZING: 1/8" ANNEALED, 3/16" ANNEALED, 3/16" TEMPERED, 1/2" IG (3/16A, 3/16S, 1/8A)
- DESIGN PRESSURE RATING: SEE TABLE 1.
 - NEGATIVE DESIGN LOADS BASED ON TESTED PRESSURE AND GLASS TABLES ASTM E 1300-98.
 - POSITIVE DESIGN LOADS BASED ON WATER TEST PRESSURE AND GLASS TABLES ASTM E 1300-98.
- CONFIGURATIONS: OX, XO
- REFERENCE TEST REPORTS: FTL-3338, FTL-3339, FTL-3340 & FTL-3341
- SHUTTER REQUIREMENT: MIAMI-DADE COUNTY APPROVED SHUTTERS ARE REQUIRED.
- NARROW JOINT SEAM SEALANT IS USED ON ALL CORNERS AND AT EACH FIXED MEETING RAIL.
- ANCHORAGE:
SINGLE ROW OF FASTENERS LOCATED AS FOLLOWS AND PER SHEET 9.
HEAD & SILL:
MAX. 6" FROM CORNERS.
MAX. 6" ON EACH SIDE OF MEETING RAIL CENTERLINE.
JAMBS:
MAX. 6" FROM CORNERS.
MAX. 17" SPACING.
- SEE SHEET 2 FOR GLAZING DETAILS.
- PRESSURES UNDER 40 P.S.F. NOT APPLICABLE IN MIAMI-DADE COUNTY.
- SEE SHEET 7 FOR CORNER CONSTRUCTION INFORMATION. A



OUTSIDE - SEE SHEET 2 FOR SECTION DETAILS

 P. J. Clark, P.E. 5/16/02		PCT INDUSTRIES	
1070 TECHNOLOGY DRIVE NOKOMIS, FL 34275 P.O. BOX 1559 NOKOMIS, FL 34274		Title: ALUM. HORIZ. ROLLER (STD. MTC. RAIL)	
Revised By: FK	Date: 4/23/02	Revisions: ADD ANCHOR DIM.	
Drawn By: F.K.	Date: 1/11/02		
Series/Model: HR-201	Score: NTS	Sheet: 1 of 9	Drawing No.: 268-1
		Rev: A	

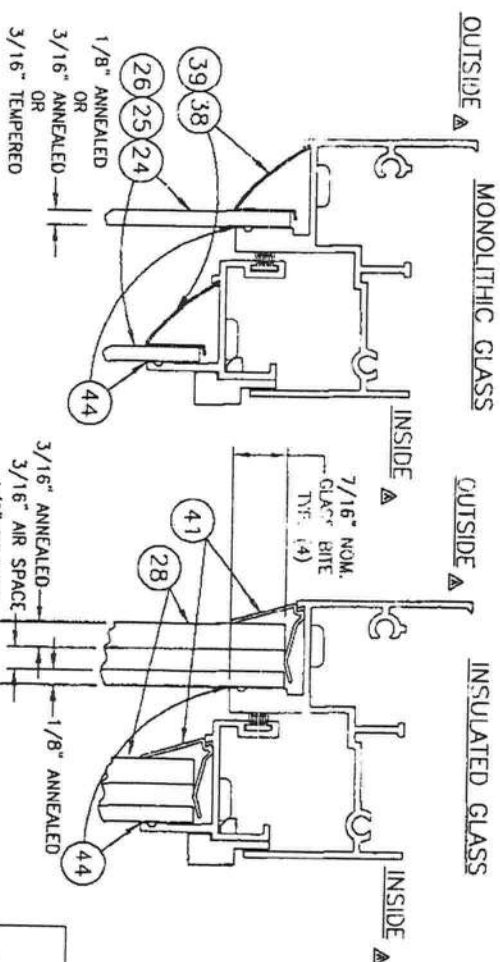
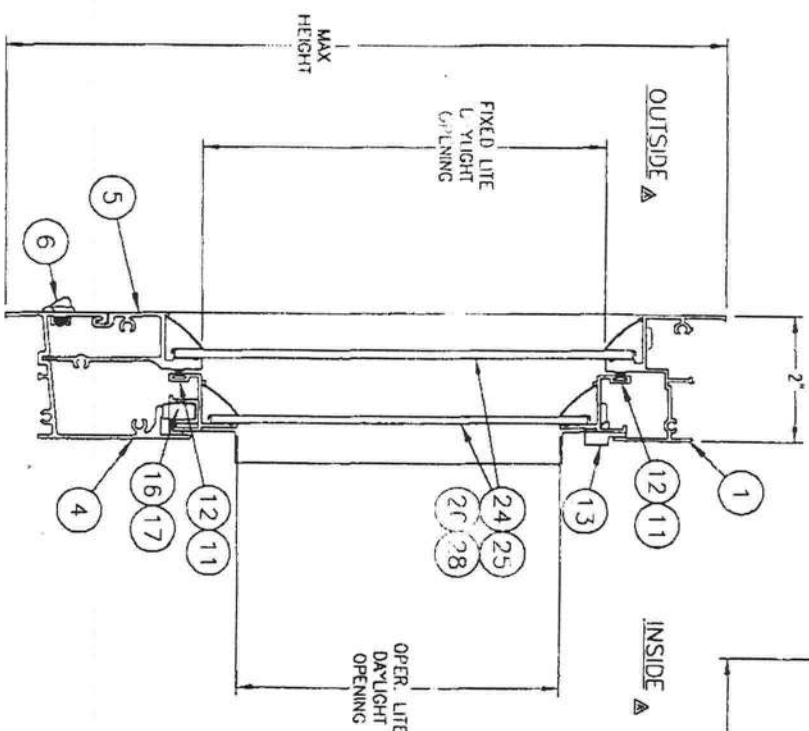
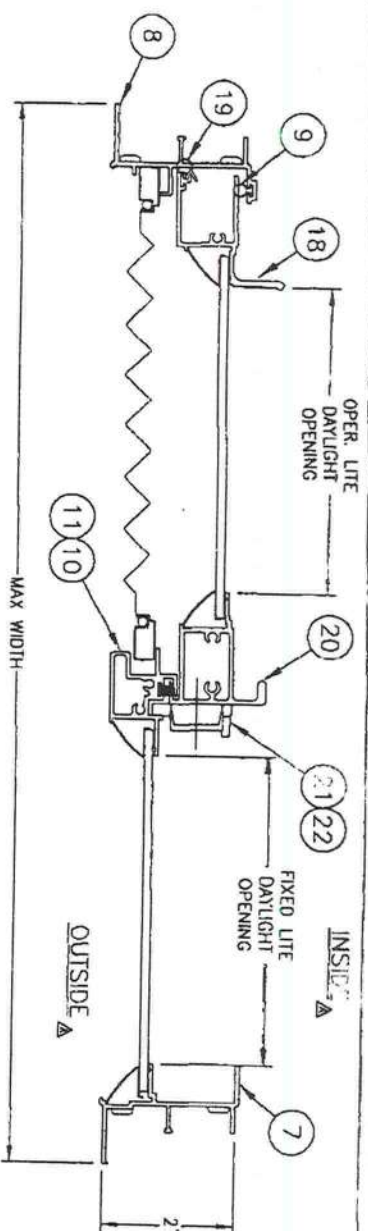
Approved as complying with the Florida Building Code

Date: **5-20-02**

Drawn: **P. J. Clark**

Checked: **P. J. Clark**

By:



HORIZONTAL SECTION

VERTICAL SECTION

TEST REPORTS:

FTL-3338
FTL-3339
FTL-3340
FTL-3341

Robert L. Clark, P.E.
PE #39712
Structural

PGI
INDUSTRIES

100, 200, 300, 400, 500, 600, 700, 800, 900, 1000, 1100, 1200, 1300, 1400, 1500, 1600, 1700, 1800, 1900, 2000, 2100, 2200, 2300, 2400, 2500, 2600, 2700, 2800, 2900, 3000, 3100, 3200, 3300, 3400, 3500, 3600, 3700, 3800, 3900, 4000, 4100, 4200, 4300, 4400, 4500, 4600, 4700, 4800, 4900, 5000, 5100, 5200, 5300, 5400, 5500, 5600, 5700, 5800, 5900, 6000, 6100, 6200, 6300, 6400, 6500, 6600, 6700, 6800, 6900, 7000, 7100, 7200, 7300, 7400, 7500, 7600, 7700, 7800, 7900, 8000, 8100, 8200, 8300, 8400, 8500, 8600, 8700, 8800, 8900, 9000, 9100, 9200, 9300, 9400, 9500, 9600, 9700, 9800, 9900, 10000, 10100, 10200, 10300, 10400, 10500, 10600, 10700, 10800, 10900, 11000, 11100, 11200, 11300, 11400, 11500, 11600, 11700, 11800, 11900, 12000, 12100, 12200, 12300, 12400, 12500, 12600, 12700, 12800, 12900, 13000, 13100, 13200, 13300, 13400, 13500, 13600, 13700, 13800, 13900, 14000, 14100, 14200, 14300, 14400, 14500, 14600, 14700, 14800, 14900, 15000, 15100, 15200, 15300, 15400, 15500, 15600, 15700, 15800, 15900, 16000, 16100, 16200, 16300, 16400, 16500, 16600, 16700, 16800, 16900, 17000, 17100, 17200, 17300, 17400, 17500, 17600, 17700, 17800, 17900, 18000, 18100, 18200, 18300, 18400, 18500, 18600, 18700, 18800, 18900, 19000, 19100, 19200, 19300, 19400, 19500, 19600, 19700, 19800, 19900, 20000, 20100, 20200, 20300, 20400, 20500, 20600, 20700, 20800, 20900, 21000, 21100, 21200, 21300, 21400, 21500, 21600, 21700, 21800, 21900, 22000, 22100, 22200, 22300, 22400, 22500, 22600, 22700, 22800, 22900, 23000, 23100, 23200, 23300, 23400, 23500, 23600, 23700, 23800, 23900, 24000, 24100, 24200, 24300, 24400, 24500, 24600, 24700, 24800, 24900, 25000, 25100, 25200, 25300, 25400, 25500, 25600, 25700, 25800, 25900, 26000, 26100, 26200, 26300, 26400, 26500, 26600, 26700, 26800, 26900, 27000, 27100, 27200, 27300, 27400, 27500, 27600, 27700, 27800, 27900, 28000, 28100, 28200, 28300, 28400, 28500, 28600, 28700, 28800, 28900, 29000, 29100, 29200, 29300, 29400, 29500, 29600, 29700, 29800, 29900, 30000, 30100, 30200, 30300, 30400, 30500, 30600, 30700, 30800, 30900, 31000, 31100, 31200, 31300, 31400, 31500, 31600, 31700, 31800, 31900, 32000, 32100, 32200, 32300, 32400, 32500, 32600, 32700, 32800, 32900, 33000, 33100, 33200, 33300, 33400, 33500, 33600, 33700, 33800, 33900, 34000, 34100, 34200, 34300, 34400, 34500, 34600, 34700, 34800, 34900, 35000, 35100, 35200, 35300, 35400, 35500, 35600, 35700, 35800, 35900, 36000, 36100, 36200, 36300, 36400, 36500, 36600, 36700, 36800, 36900, 37000, 37100, 37200, 37300, 37400, 37500, 37600, 37700, 37800, 37900, 38000, 38100, 38200, 38300, 38400, 38500, 38600, 38700, 38800, 38900, 39000, 39100, 39200, 39300, 39400, 39500, 39600, 39700, 39800, 39900, 40000, 40100, 40200, 40300, 40400, 40500, 40600, 40700, 40800, 40900, 41000, 41100, 41200, 41300, 41400, 41500, 41600, 41700, 41800, 41900, 42000, 42100, 42200, 42300, 42400, 42500, 42600, 42700, 42800, 42900, 43000, 43100, 43200, 43300, 43400, 43500, 43600, 43700, 43800, 43900, 44000, 44100, 44200, 44300, 44400, 44500, 44600, 44700, 44800, 44900, 45000, 45100, 45200, 45300, 45400, 45500, 45600, 45700, 45800, 45900, 46000, 46100, 46200, 46300, 46400, 46500, 46600, 46700, 46800, 46900, 47000, 47100, 47200, 47300, 47400, 47500, 47600, 47700, 47800, 47900, 48000, 48100, 48200, 48300, 48400, 48500, 48600, 48700, 48800, 48900, 49000, 49100, 49200, 49300, 49400, 49500, 49600, 49700, 49800, 49900, 50000, 50100, 50200, 50300, 50400, 50500, 50600, 50700, 50800, 50900, 51000, 51100, 51200, 51300, 51400, 51500, 51600, 51700, 51800, 51900, 52000, 52100, 52200, 52300, 52400, 52500, 52600, 52700, 52800, 52900, 53000, 53100, 53200, 53300, 53400, 53500, 53600, 53700, 53800, 53900, 54000, 54100, 54200, 54300, 54400, 54500, 54600, 54700, 54800, 54900, 55000, 55100, 55200, 55300, 55400, 55500, 55600, 55700, 55800, 55900, 56000, 56100, 56200, 56300, 56400, 56500, 56600, 56700, 56800, 56900, 57000, 57100, 57200, 57300, 57400, 57500, 57600, 57700, 57800, 57900, 58000, 58100, 58200, 58300, 58400, 58500, 58600, 58700, 58800, 58900, 59000, 59100, 59200, 59300, 59400, 59500, 59600, 59700, 59800, 59900, 60000, 60

ALUM. HORIZ. ROLLER (STD. MTG. RAIL)

DATE: 10/10/2011
TIME: 10:10
PAGE: 1
NO. 268-1
REV: A

1-997	6	7	CIN	107-111
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1-997

Approved as complying with the
Florida Building Code
Date Jan 17, 2007
NOA# 07-071310
Miami Dade Product Control
Division
By: [Signature]

COMPARATIVE ANALYSIS - TABLE 2

CLASS TYPE: A. DSB - 1/8" ANNEALED (FTL-3343) B. 3/16" ANNEALED (FTL-3343) C. 3/16" TEMPERED (FTL-3342) D. 1/2" IG - 1/8A, 1/4S, 1/8A (FTL-3344)

WINDOW WIDTH	WINDOW HEIGHT							
	NEG	POS	NEG	POS	NEG	POS	NEG	POS
26,000 (2)			38,375 (3)		44,000 (3)		50,625 (4)	
26,500 (4)	A. -75.0	56.0	-75.0	56.0	-75.0	56.0	-66.1	56.0
	B. -120.0	56.0	-120.0	56.0	-120.0	56.0	-88.2	56.0
	C. -135.0	56.0	-135.0	56.0	-135.0	56.0	-110.2	56.0
37,000 (4)	D. -75.0	56.0	-75.0	56.0	-75.0	56.0	-75.0	56.0
	A. -75.0	56.0	-58.5	56.0	-51.0	56.0	-47.0	56.0
	B. -120.0	56.0	-120.0	56.0	-102.4	56.0	-86.0	56.0
	C. -135.0	56.0	-135.0	56.0	-128.1	56.0	-107.6	56.0
44,000 (4)	D. -75.0	56.0	-75.0	56.0	-75.0	56.0	-58.0	56.0
	A. -75.0	56.0	-51.2	51.2	-43.2	43.2	-36.7	36.7
	B. -113.1	56.0	-101.6	56.0	-88.8	56.0	-75.5	56.0
	C. -135.0	56.0	-135.0	56.0	-113.4	56.0	-94.4	56.0
53,125 (4)	D. -75.0	56.0	-75.0	56.0	-75.0	56.0	-66.1	56.0
	A. -58.2	56.0	-47.9	47.9	-40.4	40.4	-33.6	33.6
	B. -77.6	56.0	-77.6	56.0	-70.5	56.0	-59.1	56.0
	C. -97.1	56.0	-97.0	56.0	-93.0	56.0	-83.0	56.0
63,000 (4)	D. -67.9	56.0	-67.9	56.0	-67.9	56.0	-58.1	49.8
	A. -42.7	42.7	-41.4	41.4	-37.3	37.3	-32.2	32.2
	B. -56.9	49.8	-55.2	48.3	-51.8	48.3	-39.5	38.7
	C. -71.2	56.0	-69.0	56.0	-68.0	56.0	-60.0	56.0
74,000 (4)	D. -49.8	42.7	-48.3	41.4	-49.3	41.4	-38.7	33.2
	A. -32.9	32.9	-30.0	30.0	-30.0	30.0	-29.5	29.5
	B. -43.9	38.4	-40.0	35.0	-40.0	35.0	-36.7	35.0
	C. -54.8	49.4	-50.0	45.0	-50.0	45.0	-50.0	45.0
	D. -38.4	32.9	-35.0	30.0	-35.0	30.0	-35.0	30.0

TABLE NOTE: NUMBERS IN () PARENTHESES ARE MINIMUM QUANTITY OF ANCHORS PER SIDE.

NON-IMPACT WINDOW NOTES:

- GLAZING: 1/8" ANNEALED, 3/16" ANNEALED, 3/16" TEMPERED, 1/2" IG (1/8A, 1/4S, 1/8A)
- DESIGN PRESSURE RATING: SEE TABLE 2.
 - NEGATIVE DESIGN LOADS BASED ON TESTED PRESSURE AND GLASS TABLES ASTM E 1300-98.
 - POSITIVE DESIGN LOADS BASED ON WATER TEST PRESSURE AND GLASS TABLES ASTM E 1300-98.
- CONFIGURATIONS: OX, XO
- REFERENCE TEST REPORTS: FTL-3342, FTL-3343, FTL-3344 & FTL-3345.
- SHUTTER REQUIREMENT: MIAMI-DADE COUNTY APPROVED SHUTTERS ARE REQUIRED.
- NARROW JOINT SEAM SEALANT IS USED ON ALL CORNERS AND AT EACH FIXED MEETING RAIL.
- ANCHORAGE:

SINGLE ROW OF FASTENERS LOCATED AS FOLLOWS AND PER SHEET 9.

HEAD & SILL:

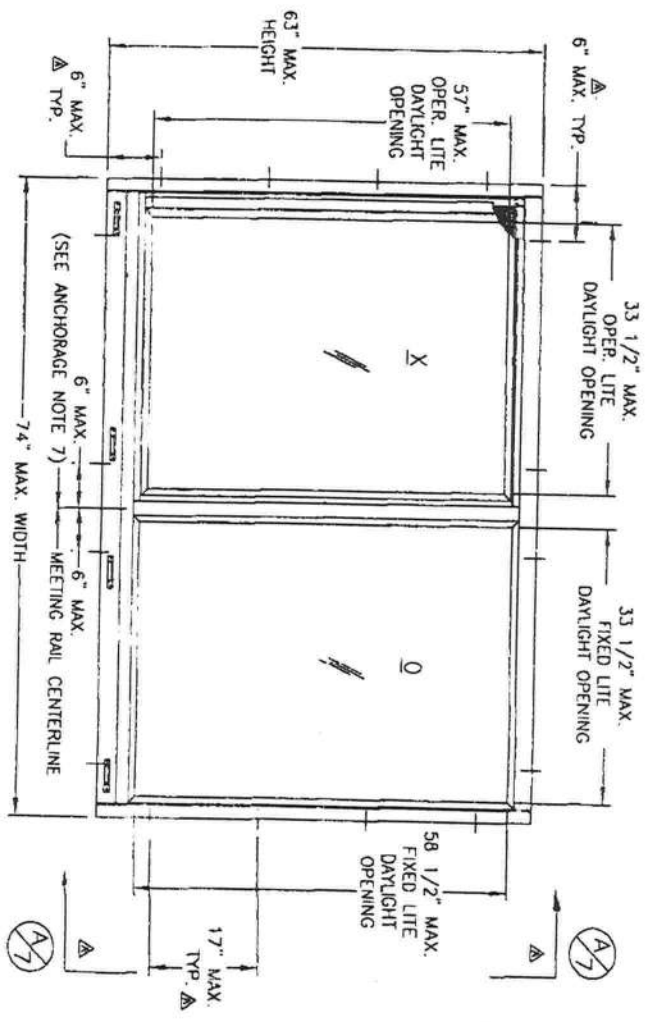
MAX. 6" FROM CORNERS.

MAX. 6" ON EACH SIDE OF MEETING RAIL CENTERLINE.

JAMBES:

MAX. 6" FROM CORNERS.

MAX. 17" SPACING.
- SEE SHEET 4 FOR GLAZING DETAILS.
- PRESSURES UNDER 40 P.S.F. NOT APPLICABLE IN MIAMI-DADE COUNTY.
- SEE SHEET 7 FOR CORNER CONSTRUCTION INFORMATION.



OUTSIDE - SEE SHEET 4 FOR SECTION DETAILS

Robert L. Clark, P.E.
PE #39712
Structural

PGI INDUSTRIES

1070 TECHNOLOGY DRIVE
NOKOMIS, FL 34275
P.O. BOX 1525
NOKOMIS, FL 34274

Title:	OX, XO ELEVATION, 74WX63H		
Serial/Model:	Scale:	Sheet:	Drawing No.
HR-201	NTS	3 of 9	268-1

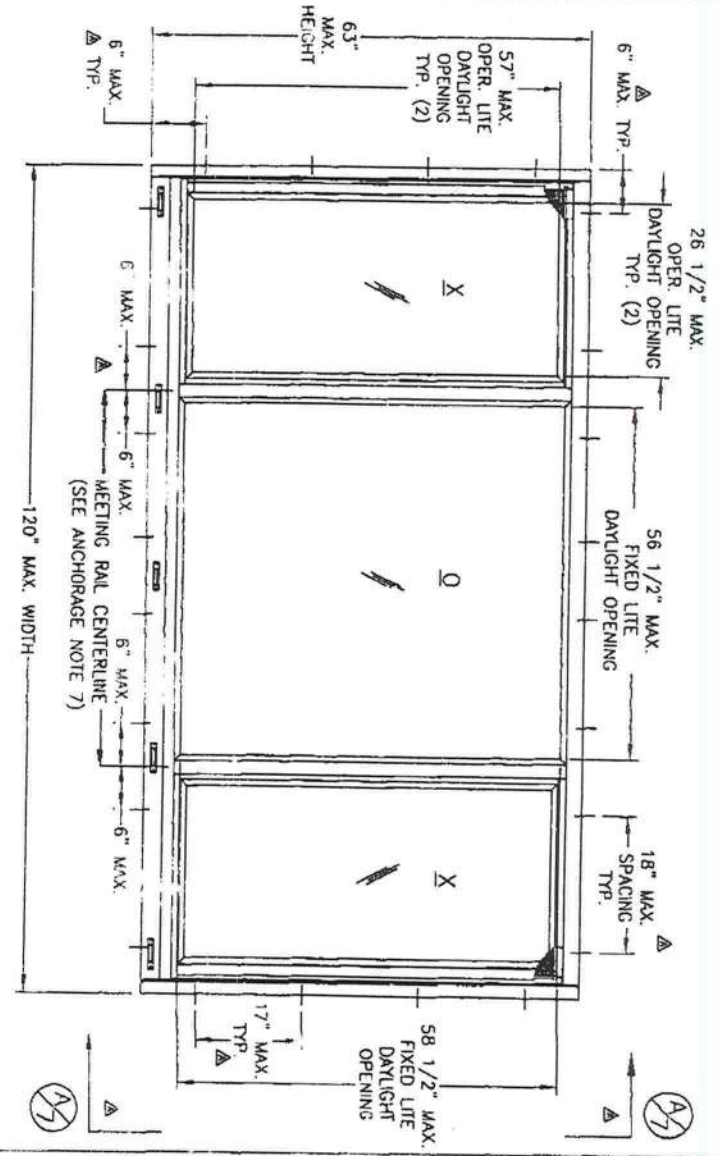
Approved as complying with the
Florida Building Code
Date: 3-26-27
NOAH 2-2-27
Miami Dade Product Control
By: *[Signature]*

COMPARATIVE ANALYSIS TABLE 3. A

GLASS TYPE: A. 3/16" ANNEALED (FTL-3336) B. 3/16" TEMPERED (FTL-3335)

WINDOW WIDTH	WINDOW HEIGHT											
	26.000 (2)			38.375 (3)			44.000 (3)			50.625 (4)		
	NEG	POS	NEG	POS	NEG	POS	NEG	POS	NEG	POS	NEG	POS
53.125 (6)	A. -135.0	56.0	-84.5	56.0	-73.0	56.0	-61.2	56.0	-49.6	49.6	56.0	56.0
	B. -135.0	56.0	-110.0	56.0	-91.2	56.0	-75.8	56.0	-57.8	56.0	56.0	56.0
	C. -75.0	56.0	-75.0	56.0	-75.0	56.0	-62.7	56.0	-49.6	49.6	56.0	56.0
74.000 (7)	A. -78.9	56.0	-64.3	56.0	-56.2	56.0	-48.3	48.3	-37.4	37.4	56.0	56.0
	B. -92.0	56.0	-92.0	56.0	-75.5	56.0	-61.3	56.0	-45.3	45.3	56.0	56.0
	C. -75.0	56.0	-71.3	56.0	-63.9	56.0	-52.5	52.5	-38.9	38.9	56.0	56.0
90.000 (7)	A. -53.3	53.3	-52.4	52.4	-48.4	48.4	-43.3	43.3	-34.4	34.4	56.0	56.0
	B. -62.2	56.0	-62.2	56.0	-62.2	56.0	-55.6	55.6	-40.1	40.1	56.0	56.0
	C. -53.3	53.3	-53.3	53.3	-53.3	53.3	-47.4	47.4	-34.4	34.4	56.0	56.0
106.375 (8)	A. -38.2	38.2	-38.2	38.2	-38.2	38.2	-38.0	38.0	-31.5	31.5	56.0	56.0
	B. -44.5	44.5	-44.5	44.5	-44.5	44.5	-44.5	44.5	-36.7	36.7	56.0	56.0
	C. -38.2	38.2	-38.2	38.2	-38.2	38.2	-38.2	38.2	-31.5	31.5	56.0	56.0
111.000 (8)	A. -35.2	35.2	-35.1	35.1	-35.1	35.1	-35.1	35.1	-30.6	30.6	56.0	56.0
	B. -41.1	41.1	-40.9	40.9	-40.9	40.9	-40.9	40.9	-36.1	36.1	56.0	56.0
	C. -35.2	35.2	-35.1	35.1	-35.1	35.1	-35.1	35.1	-30.8	30.8	56.0	56.0
120.000 (9)	A. -30.5	30.5	-30.0	30.0	-30.0	30.0	-30.0	30.0	-28.2	28.2	56.0	56.0
	B. -35.5	35.5	-35.0	35.0	-35.0	35.0	-35.0	35.0	-35.0	35.0	56.0	56.0
	C. -30.5	30.5	-30.0	30.0	-30.0	30.0	-30.0	30.0	-28.7	28.7	56.0	56.0

TABLE NOTE: NUMBERS IN PARENTHESES ARE MINIMUM QUANTITY OF ANCHORS PER SIDE.



OUTSIDE - SEE SHEET 6 FOR SECTION DETAILS

NON-IMPACT WINDOW NOTES:

1. GLAZING: 3/16" ANNEALED, 3/16" TEMPERED AND 1/2" I.G. (1/8A, 1/4S, 1/8A).
2. DESIGN PRESSURE RATING: SEE TABLE 3.
 - A. NEGATIVE DESIGN LOADS BASED ON TESTED PRESSURE AND GLASS TABLES ASTM E 1300-98.
 - B. POSITIVE DESIGN LOADS BASED ON WATER TEST PRESSURE AND GLASS TABLES ASTM E 1300-98.
3. CONFIGURATIONS: XOX
4. REFERENCE TEST REPORTS: FTL-3335, FTL-3336 & FTL-3337.
5. SHUTTER REQUIREMENT: MIAMI-DADE COUNTY APPROVED SHUTTERS ARE REQUIRED.
6. NARROW JOINT SEAM SEALANT IS USED ON ALL CORNERS AND AT EACH FIXED MEETING RAIL.
7. ANCHORAGE:

SINGLE ROW OF FASTENERS LOCATED AS FOLLOWS AND PER SHEET 9.

HEAD & SILL:

MAX. 6" FROM CORNERS.

MAX. 6" ON EACH SIDE OF MEETING RAIL CENTERLINE.

MAX. 18" SPACING.

JAMBS:

MAX. 6" FROM CORNERS.

MAX. 17" SPACING.
8. SEE SHEET 6 FOR GLAZING DETAILS.
9. PRESSURES UNDER 40 P.S.F. NOT APPLICABLE IN MIAMI-DADE COUNTY.
10. SEE SHEET 7 FOR CORNER CONSTRUCTION INFORMATION. A

Robert L. Clark, P.E.
PE #39712
Structural

5/10/02

PCT INDUSTRIES

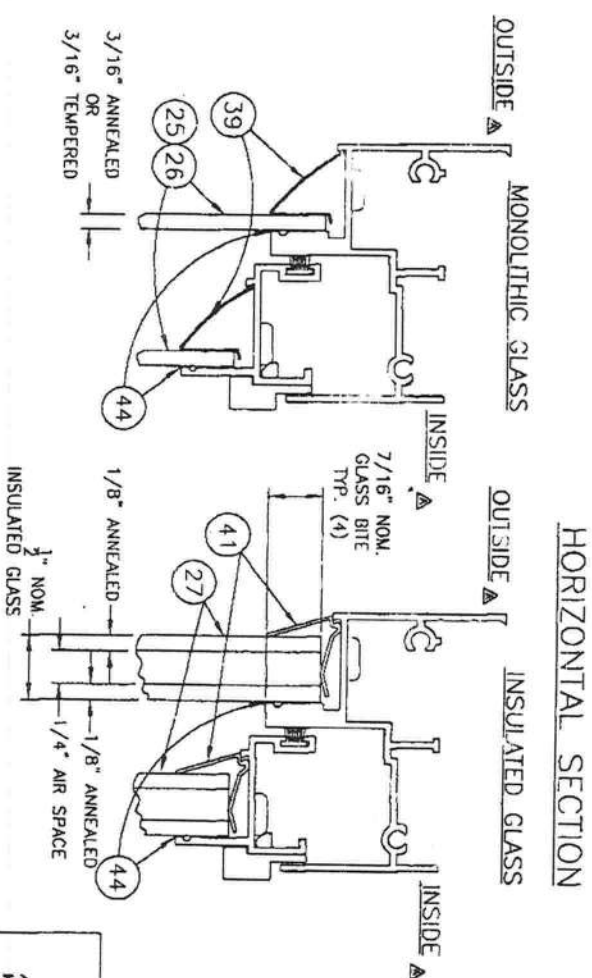
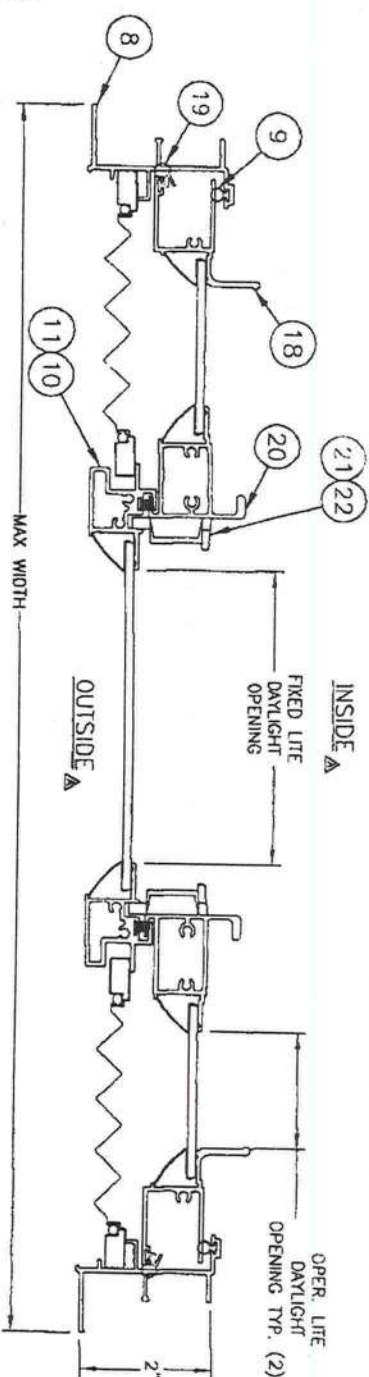
1070 TECHNOLOGY DRIVE
NOKOMIS, FL 34275
NOKOMIS, FL 34274

Revised By:	Date:	Revisions:
Revised By:	Date:	Revisions:
Revised By:	Date:	Revisions:

Description:
XOX ELEVATION, 120Wx63H

Approved as complying with the
Florida Building Code
Date: 5-10-02, 2nd
NOAR #6-2313-1st
Miami Dade English Council
Division

File:	Scale:	Sheet:	Drawing No.	Rev:
ALUM. HORIZ. ROLLER (STD. MTG. RAIL)	NTS	5 of 9	268-1	A



VERTICAL SECTION

FIL-3335
 FIL-3336
 FIL-3337

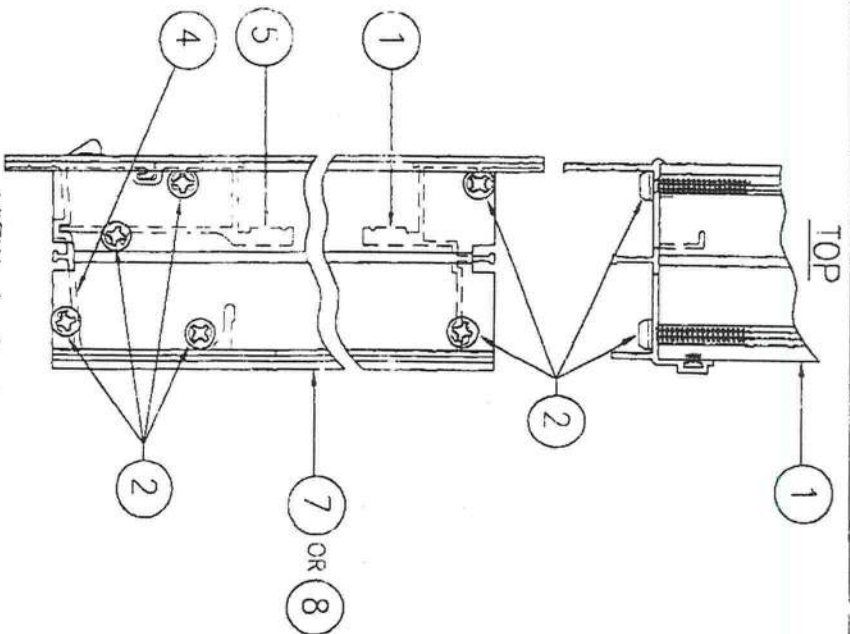
Pt. Bar
5/10/02

1070 TECHNOLOGY DRIVE
NOKOMIS, FL 34275
P.O. BOX 1529
NOKOMIS, FL 34274

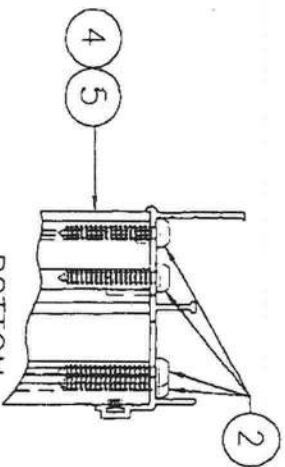
GLAZING DETAILS

Revised By:	Date:	Revisions:
Revised By: FK	Date: 4/23/02	Revisions: ADD INT/EXT DESIG.
Drawn By F.K.	Date: 7/11/02	

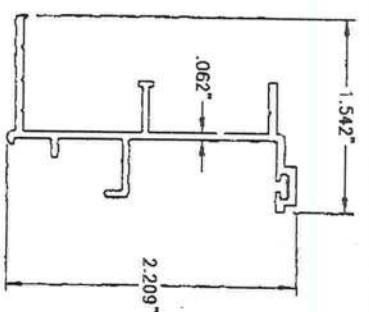
Title:		ALUM. HORIZ. ROLLER (STD. MTG. RAIL)	
Series/Model:	Scale:	Sheet:	Drawing No.
HR-201	NTS	6 of 9	268-1
			Rev
			A



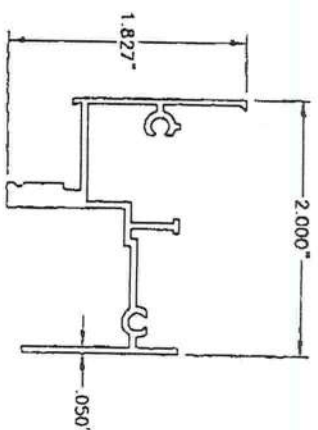
VIEW A-A
CORNER CONTRUCTION



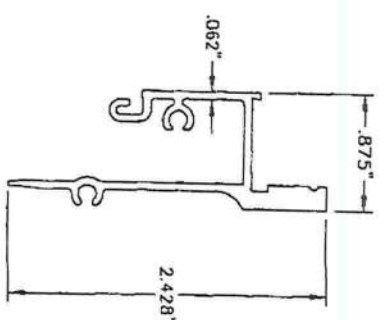
BOTTOM



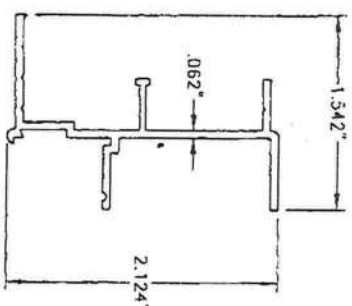
8 FRAME JAMB(OPERABLE)
MAT'L: 6063-T5
DWG# 219



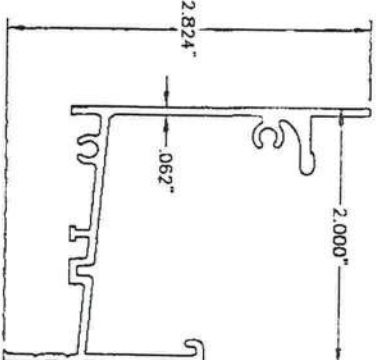
1 FRAME HEAD
MAT'L: 6063-T6
DWG# 215B



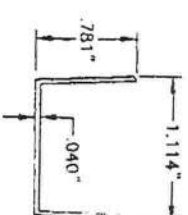
5 SILL ADAPTER
MAT'L: 6063-T6
DWG# 265B



7 FRAME JAMB(FIXED)
MAT'L: 6063-T6
DWG# 218A



4 FRAME SILL
MAT'L: 6063-T5
DWG# 264



3 SASH STOP
MAT'L: 6063-T5
DWG# 110

TEST REPORTS:
FTL-3335 FTL-3338 FTL-3342
FTL-3336 FTL-3339 FTL-3343
FTL-3337 FTL-3340 FTL-3344
FTL-3341 FTL-3345

Robert L. Clark, P.E.
5/16/02
PE #39712
Structural

PCT
INDUSTRIES

1070 TECHNOLOGY DRIVE
NOKOMIS, FL 34275
P.O. BOX 1528
NOKOMIS, FL 34274

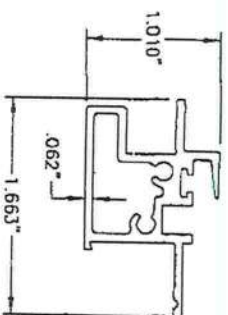
Revised By:	Date:	Revisions:
Revised By:	Date:	Revisions:
Drawn By:	Date:	Revisions:
Check By:	Date:	Revisions:
F.K.	4/23/02	ADD CORNER DET.
F.K.	11/11/02	

EXTRUSION PROFILES & CORNER DETAIL

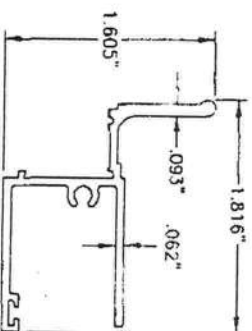
Title: ALUM. HORIZ. ROLLER (STD. MTG. RAIL)
Series/Model: HR-201
Scale: NTS
Sheet: 7 of 9
Drawing No.: 268-1
Rev: A

Approved as complying with the
Florida Building Code, Part 5,
Date: 3-17-02
NOKOMIS 01-01-02
Miami Dade Technical Center
Drawn by: *Robert L. Clark*

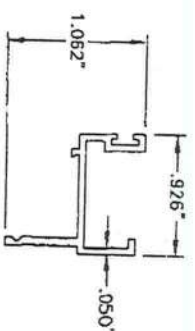
Item	Dwg #	Description	Mat'l	P.C.T. #
1	215B	Large Frame Head	6063-T6 ▲	6215W.B.T
2	1155	#8 x 1.000, QUAD Pn., SMS	Fastener	781PQA
3	110	Sash Stop	6063-T5	66387
4	264	Main Frame Sill	6063-T5	6264
5	265B	Main Frame Sill Adapter	6063-T6 ▲	6265BW.B.T
6	1298	Weep Hole Cover	Plastic	71298
7	218A	Flange Frame Jamb (Fixed, XO ONLY)	6063-T6 ▲	6128W.B.T
8	219	Flange Frame Jamb (Operable)	6063-T5	61088
9	1003	Wasp., Bulb Vinyl	Vinyl	66601
10	220B	Fixed Meeting Rail	6063-T6 ▲	6220W.B.T
11	1072	Wasp., 187 x .200, Fin Seal	Whitstrip	67919G
12	223A	Sash Top & Bottom Rail	6063-T6 ▲	6223W.B.T
13	242	Sash Top Guide (Anti-rack)	Plastic	42105
14	227	End Cap	Plastic	42114
15		#8 x .25, QUAD Pn., SMS	Fastener	78X14PQA
16	225	Roller Housing & Guide	Plastic	4212N
17	276	Rollers, Brass	Brass Part	78RWHL2
18	221A	Sash Side Rail	6063-T6 ▲	6221W.B.T
19	1001	Wasp., Bulb Vinyl	Whitstrip	64664
20	222A	Sash Meeting Rail	6063-T6	6222W.B.T
21	1096	Sweep Latch	Cast Part	71096
22	1016	#5 x .625, Ph. Fl., SM ⁵	Fastener	7858
24		DSB Glass	Glass	
25		3/16" Annealed Glass	Glass	
26		3/16" Tempered Glass	Glass	
27		1/2" Insulated DSB/DSB	Glass	
28		1/2" Insulated DSB/3/16" Annealed		
29	1265-3	3/32 x 1/4 x 1 Setting Block	Rubber	712653K
30	1267	1/8 x 1/2 x 1 Setting Block	Rubber	71267N
31	1014	Screen Frame	Aluminum	61011
32	1013	Screen Corner Key	Plastic	41903
33	1073	Screen Spring	Spring Steel	78835M
34	1278	Screen Pull	Plastic	70409K
35	1618	Screen Spine	Rubber	68155K
36		Screen Cloth		61816
38	2718	Glazing Bead (SSB/DSB)	Rollform Bd.	65196
39	2719	Glazing Bead - (3/16")	Rollform Bd.	65016
40	1263	Glazing Bead - (1/4")	Rollform Bd.	65133
41	241	Glazing Bead - (1/2" Insulated)	Plastic	6V748
42	1274	Glazing Bead - (for Grill Kit - SSB/DSB)	Plastic	6GB1274
43	1237	Glazing Bead - (for Grill Kit - 3/16")	Plastic	6GB1237
44		Silicone Adhesive Dow 899 or Equal		



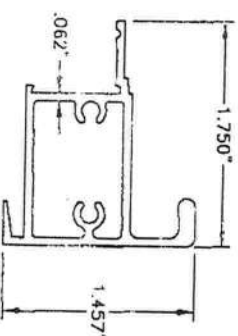
10 FIXED MEETING RAIL
MAT'L: 6063-T6 ▲
DWG# 2208



13 SASH SIDE RAIL
MAT'L: 6063-T5 ▲
DWG# 221A



12 SASH TOP & BOT. RAIL
MAT'L: 6063-T6 ▲
DWG# 223A



20 SASH MEETING RAIL
MAT'L: 6063-T6
DWG# 222A

TEST REPORTS:
FTL-3335 FTL-3338 FTL-3342
FTL-3336 FTL-3339 FTL-3343
FTL-3337 FTL-3340 FTL-3344
FTL-3341 FTL-3345

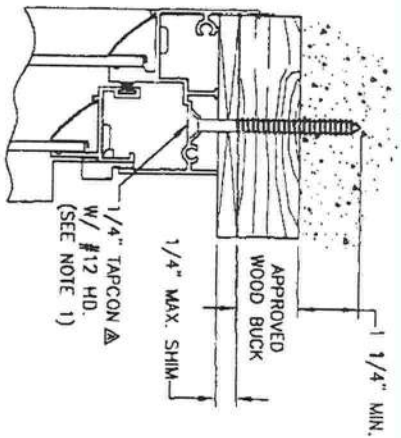
Approved as complying with the
Florida Building Code
Date: 3-26-87, Rev. 2
NOKAL 87-03-01
Missal Date Product Control
Division

PCT
INDUSTRIES

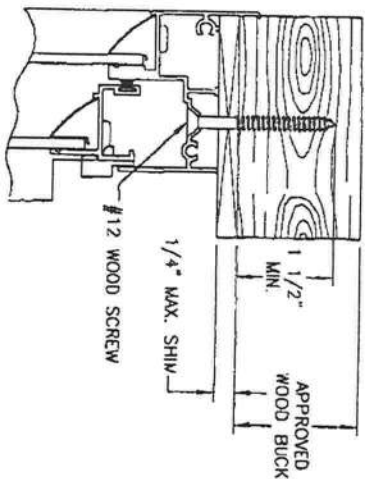
Robert L. Gryn, P.E.
PE 139712
Sturtevant

1070 TECHNOLOGY DRIVE
NOKOLUS, FL 34275
P.O. BOX 1529
NOKOLUS, FL 34274

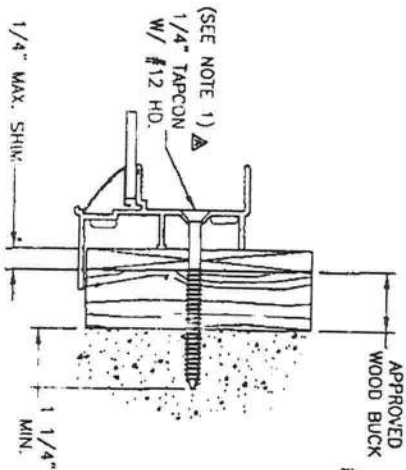
Title: EXTRUSION PROFILES & B.O.M.
ALUM. HORIZ. ROLLER (STD. MTG. RAIL)
Series/Model: HR-201
Scale: NTS
Sheet: 8 of 9
Drawing No.: 268-1
Rev: A



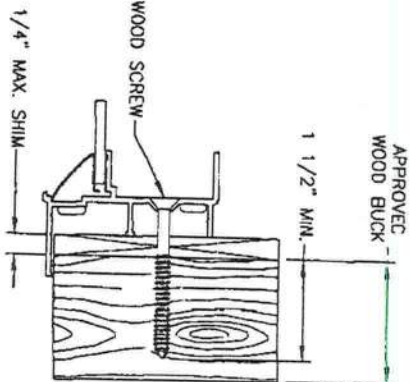
TYP. HEAD SECTION
TO CONCRETE



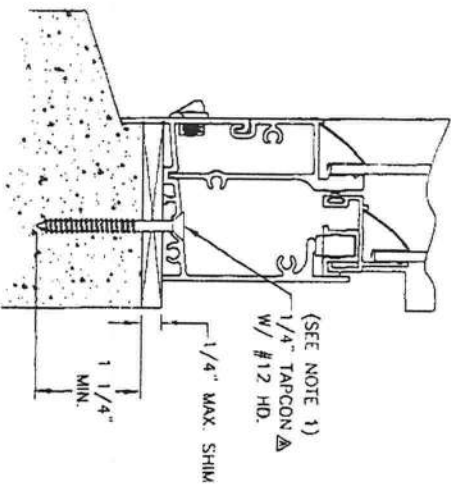
TYP. HEAD SECTION
TO WOOD



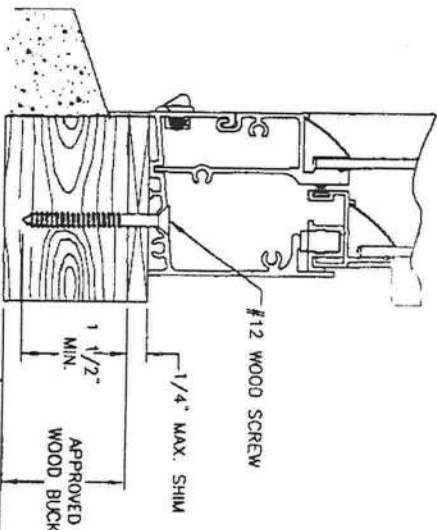
TYP. JAMB SECTION
TO CONCRETE



TYP. JAMB SECTION
TO WOOD



TYP. SILL SECTION
TO CONCRETE



TYP. SILL SECTION
TO WOOD

NOTE:
1. USE MIAMI-DADE COUNTY APPROVED ELCO OR ITW TAPCONS.
(415# MINIMUM SHEAR RATING AT 1 1/4" EMBEDMENT) Δ

Robert L. Clark, P.E.
PE #38712
Structural

PCT
INDUSTRIES

1070 TECHNOLOGY DRIVE
NORCOMES, FL 34275
P.O. BOX 1529
NORCOMES, FL 34274

ANCHORAGE DETAILS

Title:			
ALUM. HORIZ. ROLLER (STD. MTG. RAIL)	Scale:	Sheet:	Drawing No.
HR-201	NTS	9 of 9	268-1
			Rev. A

TEST REPORTS:
FTL-3335 FTL-3338 FTL-3342
FTL-3336 FTL-3339 FTL-3343
FTL-3337 FTL-3340 FTL-3344
FTL-3341 FTL-3345

Approved as complying with the
Florida Building Code
Date: 05-24-02 By: [Signature]
NOLAN 05-23-02
Principal Design Engineer

002

The active ingredient in Timber-Saver PT, Disodium Octaborate Tetrahydrate or DOT, is the most widely accepted form of borates used for treatment of forest products. DOT is manufactured from naturally occurring boron, which is widely used in a variety of applications in agriculture, cleaning products and detergents, and in wood preservation.

Product Attributes

TimberSaver® PT

Use for

Applications for TimberSaver PT treated products include:

- Framing Lumber • Interior
- Studs
- Sill Plates
- Floor Joists
- Roof Rattlers
- Trusses
- Plywood
- Furring Strips
- Flooring
- Moldings
- Interior Wood
- Trim

TimberSaver PT Protects Against These Wood Destroying Insects and Decay Causing Fungi.



- Formosan Termites*
- Subterranean Termites (Coptotermes, Reticulitermes, Heterotermes)
- Dampwood Termites (Zootermopsis)
- Drywood Termites (Kaloterms, Incisitermes)
- Carpenter Ants (Componotus)
- Powderpost Beetles (Lyctidae)
- Furniture Beetles (Anobiidae)
- Longhorn Beetles (Cerambycidae)
- Brown Rot Fungi
- White Rot Fungi
- Wet Rot Fungi

Borate preservatives have been and continue to be a key weapon used in controlling Formosan termites in Hawaii and other high hazard areas throughout the world. TimberSaver PT borate-treated lumber and plywood is protected against this aggressive termite species when the higher 0.42 ppi (DOT) retention is specified.

sites. TimberSaver PT

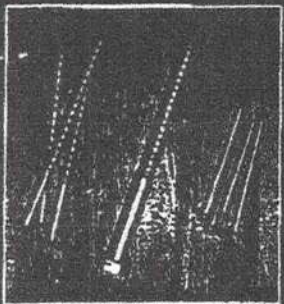
products should be stored out of ground contact, either indoors or wrapped in plastic to protect against exposure to liquid water.

With the exception of Southern Pine, all end cut surfaces and field cuts of any type must receive an application of TimberSaver solution by brushing, spraying, dipping, or flooding.

Lumber and plywood treated with TimberSaver PT must be protected from exposure

to the weather while in transit and while being stored at retail yards and job

wood construction practices.



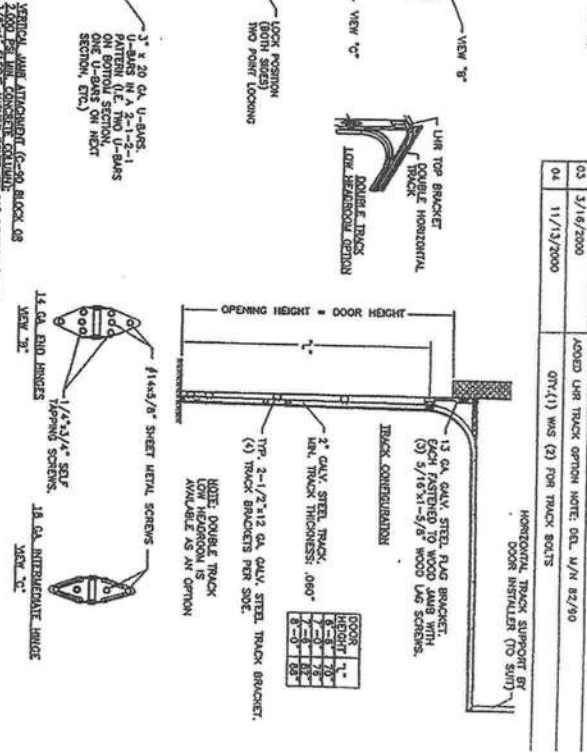
TimberSaver PT

borate treated wood can be sawn, nailed, drilled, stained and assembled using standard fastener systems typically used in general

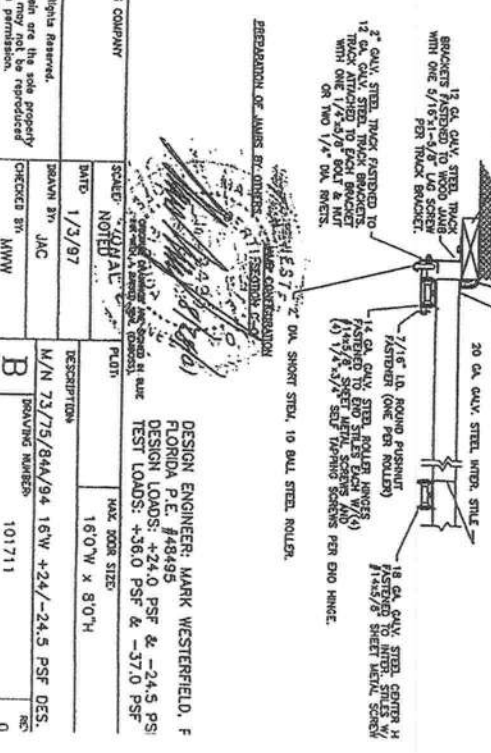
TimberSaver® PT

Handling and Use

(5) INTERMEDIATE STILES FOR 16' WIDE DOOR ATTACHED W/ 105-L-LOC (TOP & BOT) AND URETHANE ADHESIVE (ALONG CENTER)



INSIDE ELEVATIONS



15-24 GAL. CIV. STREET TRACK
BRACKETS FASTENED TO ROAD
WITH ONE 1/6" x 1-1/2" x 3/8" UG SCREW
PER TRACK BRACKET.

CLORAN BUILDING PRODUCTS COMPANY
312 WALNUT STREET
SUITE 1600
CINCINNATI, OH 45202
(613) 381-0000

Cloran Corporation All Rights Reserved.

The arrangements depicted herein are the sole property of Cloran Corporation and may not be reproduced in any form without its written permission.

1. The first step is to identify the problem or question that needs to be answered. This involves understanding the context and the specific requirements of the task.

DESIGN ENGINEER: MARK WESTERFIELD, F
FLORIDA P.E. #48495
DESIGN LOADS: +24.0 PSF & -24.5 PSF
TEST LOADS: +36.0 PSF & -37.0 PSF

ASTM HERITAGE 30 AR®

LAMINATED ASPHALT SHINGLES

Manufactured in Tuscaloosa, AL.

ASTM HERITAGE 30 AR® shingles feature a double-layer fiberglass mat construction with a random-cut sawtooth design.

The two layers of mat are coated with asphalt and then laminated together and surfaced with mineral granules that will help protect against discoloration caused by algae. A self-sealing strip of asphalt helps provide added wind resistance.



USES

ADVANTAGES

- 30 year limited warranty, 5 year FULL START, limited transferability, winds up to 70 MPH
- Affordable upgrade from 3-tab shingles
- Superior fire resistance compared to organic shingles
- Rustic beauty of wood shakes
- Shadowtone feature adds depth and dimensional appearance
- Algae resistant granules to protect against discoloration in areas where extreme humidity is a problem
- 10 year limited warranty against discoloration caused by certain algae growth

CERTIFICATIONS

UL Class A Fire Rating
UL Wind Resistant

Fed. Spec.: Exceeds SS-S-001534, Class A, Type I
ASTM D 3018, Type I
ASTM E 108, Class A
ASTM D 3161, Type I (modified to 110 mph)
ASTM D 3462

COLORS

Classic Heritage Colors:

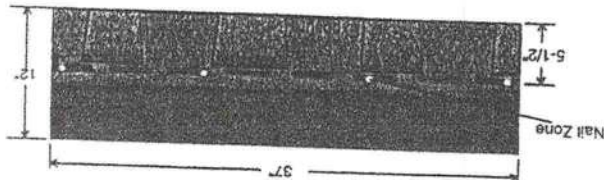
- Weathered Wood
- Rustic Cedar
- Rustic Hickory
- Driftwood
- Oxford Grey
- Shadow Grey
- Desert Sand
- Rustic Black
- Olde English Pewter
- Glacier White
- Rustic Evergreen

PRODUCT DATA*



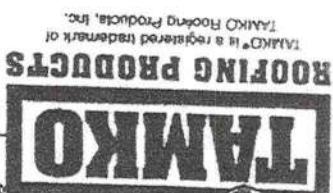
Shingle size 12" X 37"
Exposure 5"
Shingles per square 78
Bundles per square 3

*All values stated as nominal



CAUTION: The National Institute for Occupational Safety and Health (NIOSH) has concluded that fumes of heated asphalt are a potential occupational carcinogen. Do not heat or burn this product.

Visit our Web Site at www.tamko.com



Central District
Northeast District
Southeast District
Southwest District
Western District
220 West 4th St., Joplin, MO
4500 Tamko Dr., Frederick, MD
2300 35th St., Tuscaloosa, AL
7910 S. Central Exp., Dallas, TX
5300 East 43rd Ave., Denver, CO
64801 800-641-4691
21701 800-368-2055
35401 800-228-2656
75216 800-443-1834
80216 800-530-8868

MI HOME PRODUCTS - PRIME ALUMINUM WINDOWS - INSTALLATION INSTRUCTIONS FOR "NAIL FIN" PRODUCTS

MI Home Products appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition - proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1. Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin.

2. Set unit plumb and square into opening and make sure that there is $3/16" \pm 1/16"$ clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit before any and all fasteners are set.

3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DPs (design pressures) up to 35, do not exceed 24" spacing of additional screws. For DPs from 35.1 to 50, do not exceed 18". Install load bearing shim adjacent to each anchor. Use shim where space exceeds 1/16".

4. Flash over head and caulk outside perimeter in accordance with code requirements and good installation practices.

5. Fill voids between frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of expanding aerosol type insulating foam, which can bow the frame, waives all stated warranties.

6. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks and interlocks are also clear. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent as you would your automobile.

CAUTION -

MI Home Products or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. MI Home Products window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing near doors, bathtubs, and shower enclosures. Also be aware of emergency egress code requirements.

Corporate Headquarters:
650 West Market St.
Gatz, PA 17030-0370
(717) 365-3300



AAMA/NWDA 101/L.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650/850
TYPE: H-C30 54 x 90; H-C40 52 x 72*

Summary of Results		
Title of Test	Test Specimen #1	Test Specimen #2
AAMA Rating	H-C30 54 x 90	H-C40 52 x 72*
Uniform Load Deflection Test Pressure	35.0 psf	47.0 psf
Operating Force	20 lb max.	N/A
Air Infiltration	0.27 cfm/ft ²	N/A
Water Resistance Test Pressure	5.25 psf	6.0 psf
Uniform Structural Load Test Pressure	45.0 psf	70.5 psf
De-glazing	Passed	N/A
Forced Entry Resistance	Grade 10	N/A

Reference should be made to ATI Report No. 01-37589.02 for complete test specimen description and data.

Allen M. Rumpf
24 JUNE 2000

Architectural Testing

TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gatz, Pennsylvania 17030-0370

Report No: 01-37589.02
Test Date: 06/15/00
Thru: 06/29/00
Report Date: 06/06/02
Expiration Date: 06/29/04

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on two Series/Model 450/650/850, aluminum single hung windows at their facility in Elizabethtown, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: H-C30 54 x 90; Test Specimen #2: H-C40 52 x 72*.

General Note: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/LS-2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 450/650/850

Type: Aluminum Single Hung Window

Test Specimen #1: Gateway Performance Specimen H-C30 54 x 90 rating

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Active Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.architest.com

Alan M. Rumm
28 JUNE 2002



Test Specimen Description: (Continued)

Test Specimen #2: H-C40 52 x 72*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Active Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" clear annealed glass and an intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

Description	Quantity	Location
0.210" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam filled vinyl bulb gasket	1 Row	Bottom rail
0.400" high by 1/2" square polypile dust plug	4	One on each sash corner

Frame Construction: Series/Model 450 frame was constructed of thermally broken extruded aluminum with coped, butted and sealed corners. The fixed meeting rail was constructed of an extruded aluminum member with coped, butted and sealed ends fastened with two #8 x 1/4" screws. Series/Model 650 frame was constructed of extruded aluminum.

Sash Construction: The Series/Model 450 sash members were constructed of thermally broken extruded aluminum members with coped, butted and sealed corners fastened with one #8 x 1-1/4" screw. Series/Model 650 sash was constructed of extruded aluminum.

Screen Construction: The screen was constructed of rolled-aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

Allen M. Rosen
29 June 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Plastic snap latch	1	Midspace of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail
Drainage: Sloped sill		

Reinforcement: No reinforcement.

Installation: The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood test buck utilizing the nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blind stopped with wood members and secured with #8 x 3" screws every 24" on center.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
-----------	-----------------------------	---------	---------

Test Specimen #1: Gateway Performance Specimen H-C30 54 x 90

2.2.1.6.1 Operating Force 20 lbs 45 lbs max

Air Infiltration (ASTM E 283)
@ 1.57 psf (25 mph)

0.27 cfm/ft² 0.30 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/T.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547)
(with and without screen)
WTP = 4.5 psf

No leakage No leakage

2.1.4.1 Uniform Load Deflection (ASTM E 330-97)
(Measurements reported were taken on the fixed meeting rail)

(Loads were held for 33 seconds)
@ 35.0 psf (positive) 0.27"
@ 35.0 psf (negative) 0.73"*
0.30" max 0.30" max

* Exceeds L/175 for deflection but meets all other test requirements.

Alan M. Runt
29 JUNE 79

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
-----------	-----------------------------	---------	---------

Test Specimen #1: Gateway Performance Specimen H-C30 54 x 90 (Continued)

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the fixed meeting rail) (Loads were held for 10 seconds) @ 45.0 psf (positive) @ 45.0 psf (negative)	0.03" 0.04"	0.21" max 0.21" max
---------	---	----------------	------------------------

2.2.1.6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs	Meeting rail Bottom rail	0.06"/12% 0.06"/12%
-----------	--	-----------------------------	------------------------

2.1.8	Forced Entry Resistance (ASTM F 588-97) In remaining direction at 50 lbs	Left stile Right stile	0.06"/12% 0.06"/12%
-------	---	---------------------------	------------------------

Type: A Grade: 10	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Alan M. Hunter
7 & 10/11/15 7:00 PM

Test Results

Paragraph Title of Test - Test Method Results Allowed

Test Specimen #1: Gateway Performance Specimen H-C30 54 x 90 (Continued)

Optional Performance

4.3 Water Resistance (ASTM E 547)

(with and without screen)
WTP = 5.25 psf

No leakage No leakage

Test Specimen #2: H-C40 52 x 72*

Optional Performance

4.3 Water Resistance (ASTM E 547 and ASTM E 331)

(with and without screen)
WTP = 6.0 psf

No leakage No leakage

Uniform Load Deflection (ASTM E 330)

(Measurements reported were taken on the fixed meeting rail)

@ 47.0 psf (positive) 0.04" @ 47.0 psf (negative) 0.30" max

(Loads were held for 33 seconds)

Uniform Load Structural (ASTM E 330)

(Measurements reported were taken on the fixed meeting rail)

@ 70.5 psf (positive) 0.07" @ 70.5 psf (negative) 0.21" max

(Loads were held for 10 seconds)

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

Mark A. Hess
Technician

MAH:baw
01-37589.02

Allen N. Reeves, P.E.
Director - Engineering Services
24 JUNE 2002

Current Issue Date: 06/06/02

DOCUMENT CONTROL APPENDUM #01-37589.00



Report No.: 01-37589.01

Requested by: Scott Gill, MI Home Products, Inc.
Purpose: AAMA/NWDA 101/I.S.2-97 testing on Series/Model 450, aluminum single hung window.
Issued Date: 09/11/00
Comments: Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-37589.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Revised Report No. 01-37589.01.
Issued Date: 06/06/02
Comments: Added Series/Model 650/850. Florida P.E. seal required on report
Certification copy to John Smith at Associated Laboratories, Inc.

Allen M. Rimm
24 June 2007

AAMA/NWDA 101/1.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650

TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft ²
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
De-glazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete description and data.



Architectural Testing

AAMA/NWDA 101/S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.

P.O. Box 370

650 West Market Street

Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethtown, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-1/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com



Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping

Description	Quantity	Location
0.230" high by 0.270" backed polypropylene with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypropylene with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stile's screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midsparn of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max
Air Infiltration (ASTM E 283-91)			
@ 1.57 psf (25 mph)			
0.16 cfm/ft ²			
0.3 cfm/ft ² max			

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)
(with and without screen)
WTP = 2.86 psf

No leakage



Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
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2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max 0.41" max
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2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max 0.29" max
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2.2.6.2	Dejacking Test (ASTM E 987-88) In operating direction at 70 lbs		
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	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%

	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%

2	8	Forced Entry Resistance (ASTM F 588-97)	
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Type: A
Grade: 10

Lock Manipulation Test

No entry

No entry

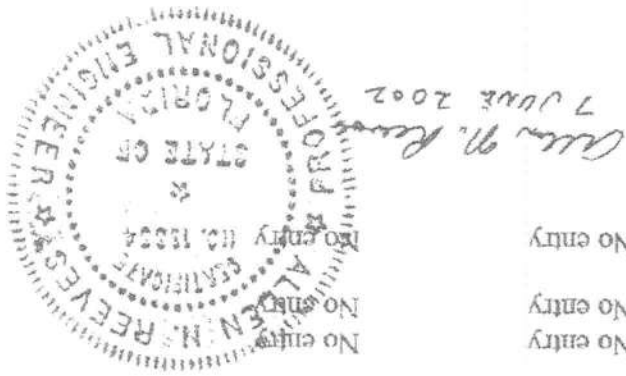
Test A1 through A5

No entry
Test A7

Lock Manipulation Test

No entry

No entry



Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
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Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Loads were held for 52 seconds) @ 35.3 psf (positive) @ 47.2 psf (negative)	0.46"* 0.67"*	0.41" max 0.41" max
	*Exceeds L/175 for deflection, but meets all other test requirements.		

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 53.0 psf (positive) @ 52.5 psf (negative)	0.03" 0.02"	0.29" max 0.29" max
--	--	----------------	------------------------

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

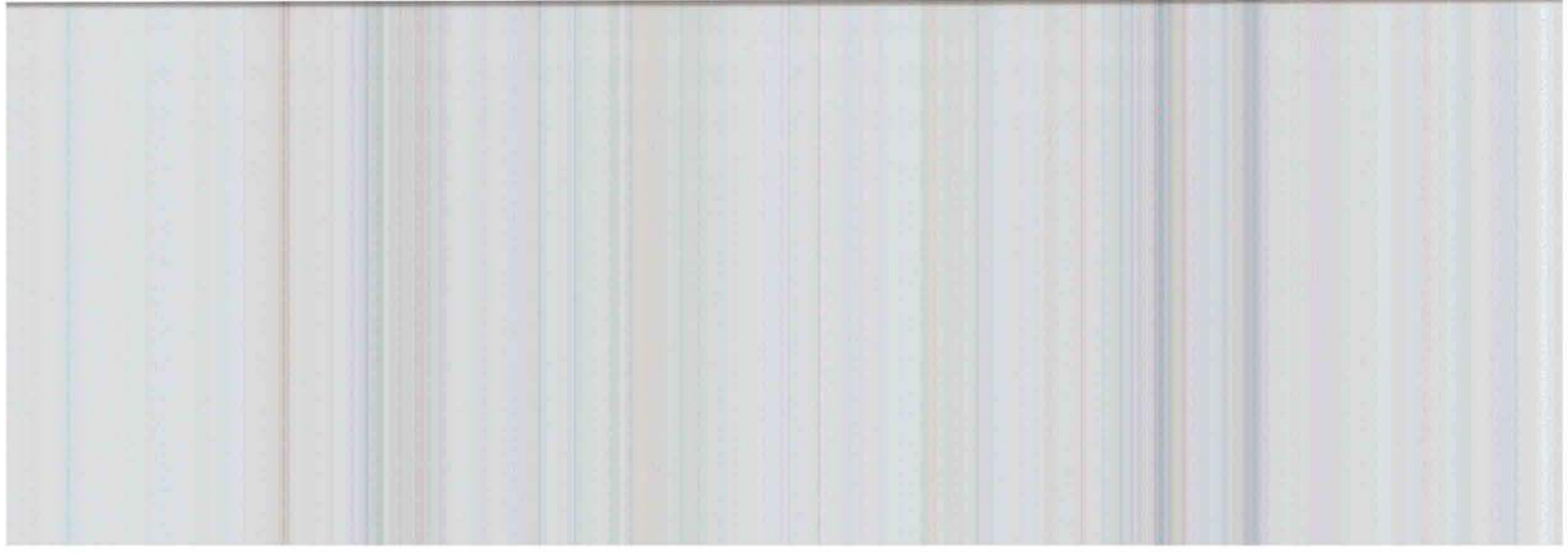
For ARCHITECTURAL TESTING, INC.

Mark A. Hess
Technician

MAH:mb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002





STRUCTURAL TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

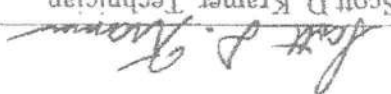
SERIES/MODEL: 650

TYPE: Twin Aluminum Single Hung Window

Title of Test	Results
Overall Design Pressure	35.0 psf
Operating Force	18 lb max.
Air Infiltration	0.29 cfm/ft ²
Water Resistance	5.25 psf
Structural Test Pressure	70.5 psf

Reference should be made to Report No. 01-36060.02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Scott D. Kramer, Technician

SDK:nlb/baw



Architectural Testing

STRUCTURAL TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gatz, Pennsylvania 17030-0370

Report No: 01-36060.02
Test Date: 11/04/99
Report Date: 03/26/02
Expiration Date: 11/04/03

Project Summary: Architectural Testing, Inc. (ATI) was contracted to perform tests on a Series/Model 650, twin aluminum single hung window at MI Home Products' test facility in Elizabethville, Pennsylvania. Test specimen description and results are reported herein.

Test Specification The test specimen was evaluated in accordance with the following:

ASTM E 283-91, Standard Test Method for Determining the Rate of Air Leakage Through Exterior Windows, Curtain Walls, and Doors Under Specified Pressure Differences Across the Specimen

ASTM E 330-97, Standard Test Method for Structural Performance of Exterior Windows, Curtain Walls, and Doors by Uniform Static Air Pressure Difference

ASTM E 547-96, Standard Test Method for Water Penetration of Exterior Windows, Curtain Walls, and Doors by Cyclic Static Air Pressure Differential

Test Specimen Description

Series/Model: 650

Type: Twin Aluminum Single Hung Window

Overall Size: 5' 10-1/4" wide by 5' 0" high

Active Size (2): 2' 8-3/4" wide by 2' 6-1/4" high

Fixed Daylight Opening Size (2): 2' 6-1/4" wide by 2' 3" high

Screen Size (2): 2' 7-3/4" wide by 2' 4-1/4" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.architest.com



Allen H. Rumm
28 MARCH 2002

Test Specimen Description: (Continued)

Finish: All aluminum was painted white.

Glazing Details: Both the active sash and fixed lites utilized 5/8" thick insulating glass fabricated from two sheets of 3/32" thick clear annealed glass and a desiccant filled metal spacer system. The active sash were channel glazed with a flexible wedge gasket. The fixed lites were interior glazed, back bedded with single sided adhesive foam tape and held-in-place with PVC snap-in glazing beads.

Weatherstripping:

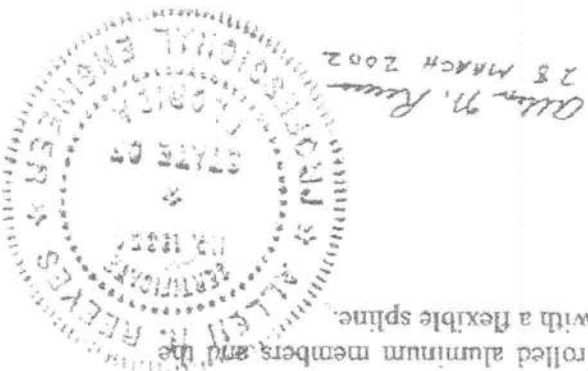
Description	Quantity	Location
0.270" backed by 0.190" high polypile with center fin	1 Row	Fixed meeting stile
3/8" high vinyl wrapped foam bulb	1 Row	Bottom rail
0.187" backed by 0.250" high polypile with center fin	2 Rows	Stiles
1/4" high polypile dust plug	2 Rows	Ends of bottom rail, top of each stile

Frame Construction: Frame was constructed of extruded aluminum members and all corners were coped, butted, sealed, and fastened with two screws per corner. The fixed meeting rail was attached to the jambs with a plastic clip and two screws per end.

Mullion Construction: The mullion was constructed of an extruded aluminum member. It was fastened to the head and sill with four screws per end. All screw heads were sealed as well as the butt joint at the sill.

Sash Construction: The sash were constructed of extruded aluminum members and all corners were coped, butted, and fastened with one screw per corner.

Screen Construction: The screen was constructed of rolled aluminum members and the corners were keyed. The screen mesh was held-in-place with a flexible spline.



Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Plastic tilt latches	4	Ends of interior meeting rail
Metal pivot bars	4	Ends of the bottom rails
Metal sweep lock	2	Midspace of interior meeting rail
Metal keeper	2	Midspace of fixed meeting rail
Sash stops	4	One per jamb
Block and tackle balance system	4	One per jamb
Spring loaded latch pins	2	6" from ends of screen top rail
Drainage:		
Sloped sill		Sill
1/4" wide by 3/16" high weepslot	4	Ends of exterior vertical sill leg

Installation: The test unit was installed into the 2" x 8" nominal Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin and 1" roofing nails. Five per top, bottom, and sides of the nail fin were evenly spaced. The nail fin was bedded in a silicone sealant.



Allen H. Wells
28 MARCH 2002



Allen N. Reeves, P.E.
Director -- Engineering Services
28 MARCH 2002

SDK:mlb/baw
01-36060.02

Scott D. Kramer
Technician

For ARCHITECTURAL TESTING, INC.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (load held for 10 seconds) @ 70.5 psf (exterior) @ 70.5 psf (interior)			0.060"	0.040"	0.24" max	0.24" max
Note: No end measurements were taken on the member measured. The measurements stated above include displacement as well as bending. Only permanent sets were recorded. not deflection measurements. This statement applies to all uniform load tests performed.						
Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (load held for 33 seconds) @ 47.0 psf (exterior) @ 47.0 psf (interior)			0.010"	0.015"	0.24" max	0.24" max
Water Resistance per ASTM E 547-96 (with and without screen) WTP = 5.25 psf No leakage						
Air Infiltration per ASTM E 283-91 @ 0.56 psf (15 mph) @ 1.57 psf (25 mph)			0.15 cfm/ft ²	0.29 cfm/ft ²	0.30 cfm/ft ²	0.30 cfm/ft ²

Title of Test - Test Method	Results	Allowed
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The results are tabulated as follows:

Test Results



**AAMA/NWDA 101/1.S.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650

TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft ²
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test description and data.

For ARCHITECTURAL TESTING, INC.



Architectural Testing



AAMA/NWDA 101/LS-2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gatz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/LS-2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650

Type: Aluminum Picture Window

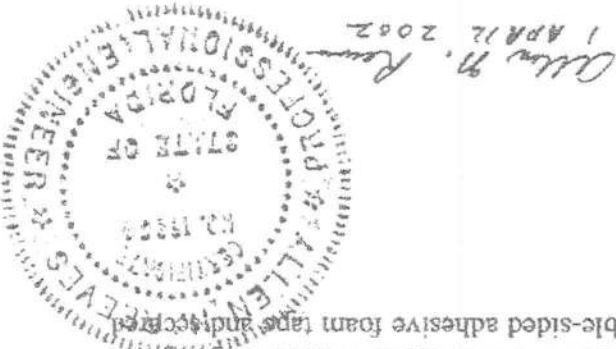
Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish: All aluminum was white

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com



Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.04 cfm/ft ²	0.3 cfm/ft ² max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/LS-2-97 for air infiltration.

Water Resistance (ASTM E 547-00)
WTP = 2.86 psf
No leakage

2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.01" 0.01"	0.41" max 0.41" max
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2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.0" 0.01"	0.29" max 0.29" max
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Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
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Forced Entry Resistance (ASTM F 588-97)

Type: D

Grade: 10

Hand and Tool Manipulation Test

No entry

Optional Performance

4.3 Water Resistance (ASTM E 547-00)

WTP = 8.25 psf

No leakage

No leakage

Uniform Load Deflection (ASTM E 330-97)

(Measurements reported were taken on the jamb)

(Loads were held for 33 seconds)

@ 45.0 psf (positive)

0.02"

0.41" max.

@ 47.2 psf (negative)

0.02"

0.41" max.

Uniform Load Structural (ASTM E 330-97)

(Measurements reported were taken on the jamb)

(Loads were held for 10 seconds)

@ 67.5 psf (positive)

0.01"

0.29" max.

@ 70.8 psf (negative)

0.02"

0.29" max.

For ARCHITECTURAL TESTING, INC

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

Mark A. Hess
Technician
MAH:mb
01-41135.01

Mark A. Hess

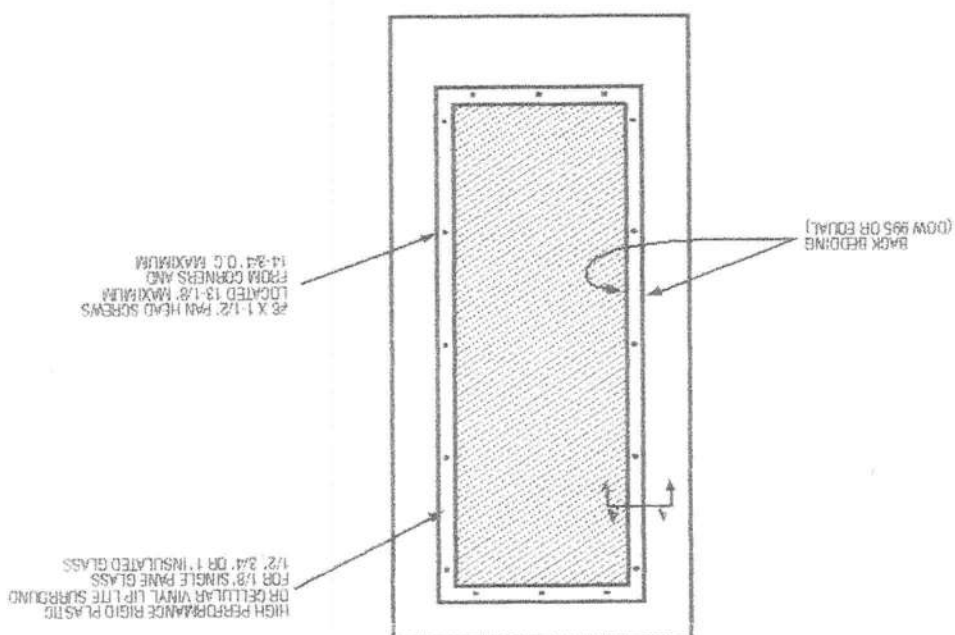
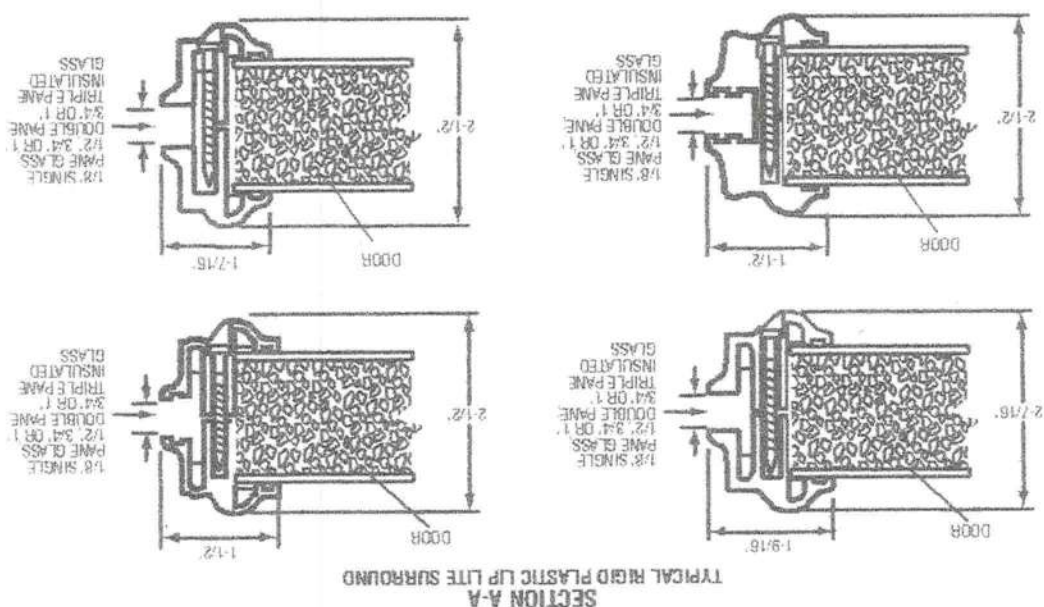
Allen N. Reeves, P.E.
Director - Engineering Services
1 April 2002

Allen N. Reeves



Master Data Review (mailto:masterdatareview@us.ibm.com) and COPIES Report Validation (mailto:masterdatareview@us.ibm.com) available 11/5/2011 00:02:00Z to 11/5/2011 00:02:00Z
Additional information available 11/5/2011 00:02:00Z to 11/5/2011 00:02:00Z
Website (www.us.ibm.com) of the Business Technical Center

*Class inserts to be submitted by Intertek Testing Services/IL Semko or approved validation service.



GLASS INSERT IN DOOR
OR SIDELITE PANEL

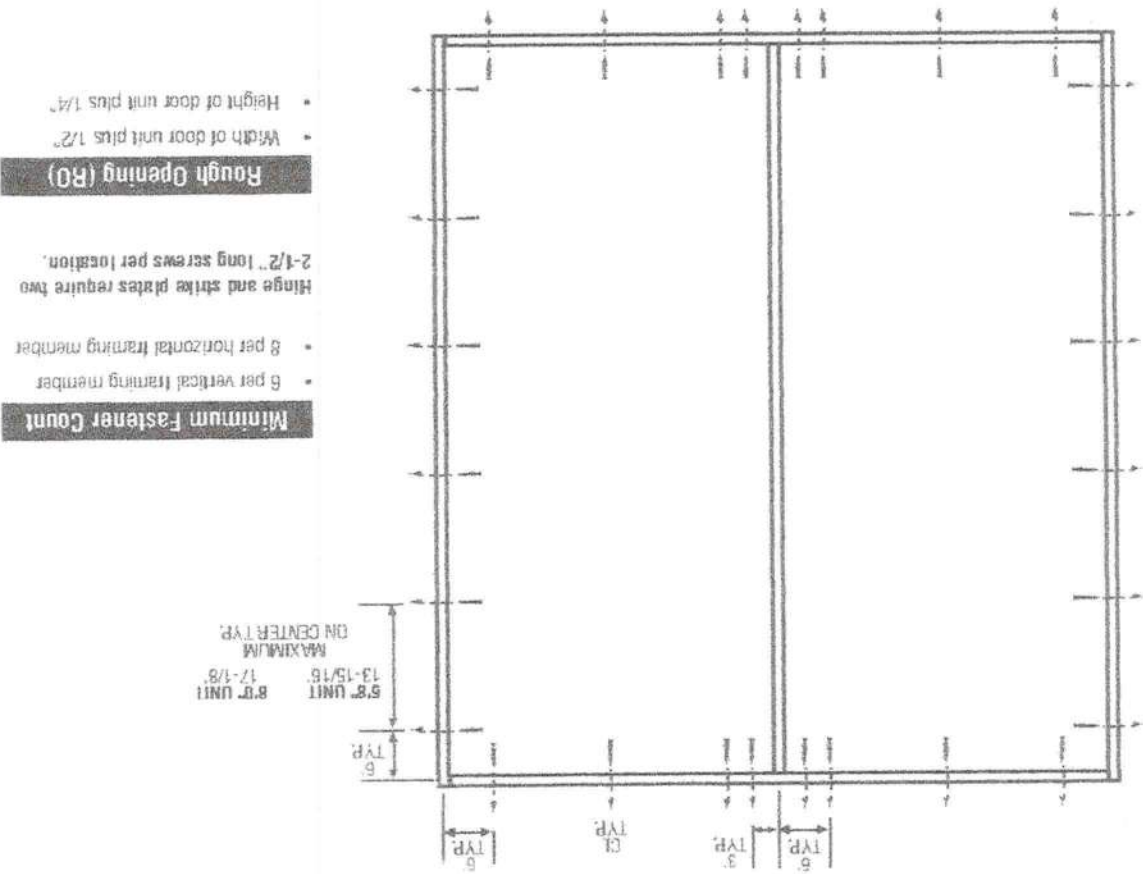
MAD-WL-MA0041-02

MID-WL-MA0001-02

Unit
XX

MID-WL-MA0002-02

DOUBLE DOOR



Best Data Review Details #202647A, #202647B, #202647C and CD-Test Report Validation
Additional information - available from the ITSPWH website (www.itspwh.com), the Masonite
website (www.masonite.com) or the Masonite technical center

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
 - UNITS COVERED BY COP DOCUMENT 3147, 3167, 3242*, 3247, 3262* or 3267
 - Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANS/AF & PA MDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Bode County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

- Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
- The wood screw shear design values come from Table 11.3A of ANS/A- & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELO Dade County approvals respectively, each with minimum 1-1/4" embedment.
- Wood bucks by others, must be anchored properly to transfer loads to the structure.

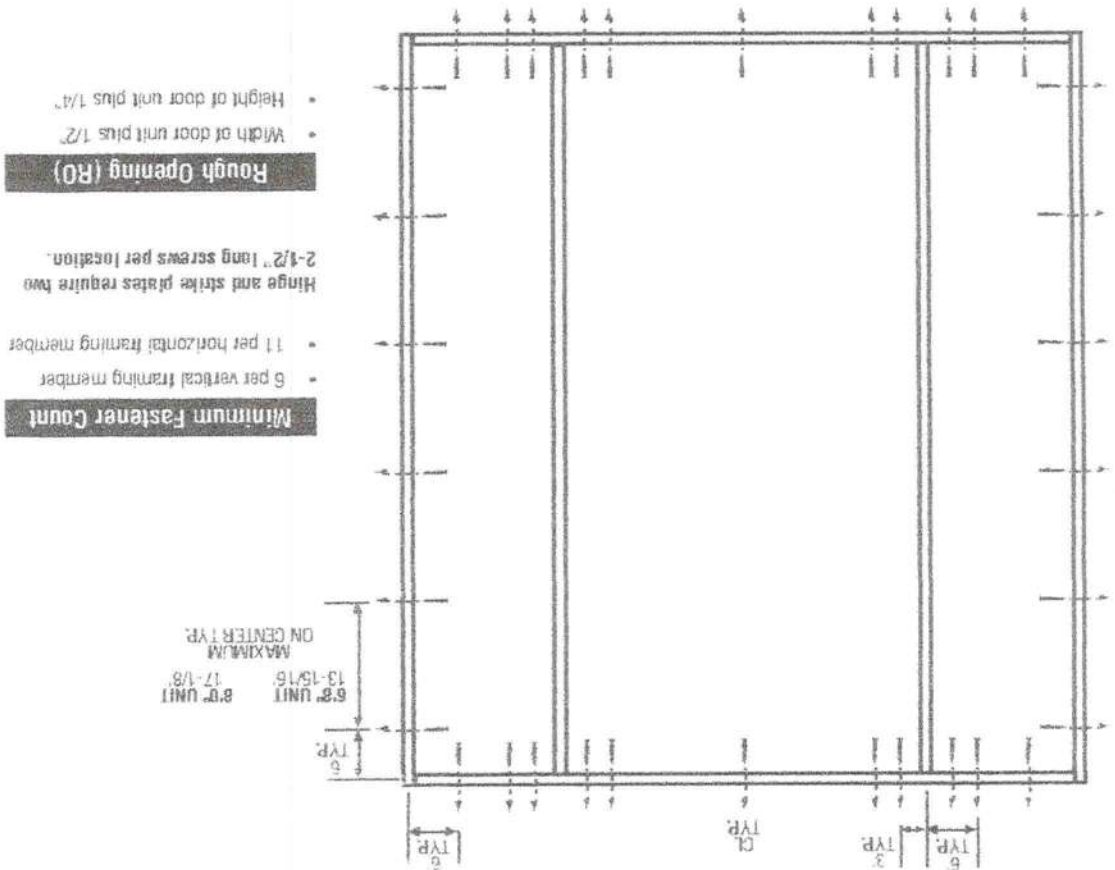
NOTES:

*Based on required Design Pressure - see COP sheet for details.

- * Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
 - * UNITS COVERED BY COP DOCUMENT 3244*, 3249, 3264* or 3269
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

Latching Hardware:

Blackrock Energy
 Test Data Reporting Certificate #32624478 • 43824478 • 43824478 and CD-R from Report Validation
 Matrix #32624478-001-002-003 • #32624478-002-003 • #32624478-001-002-003 provides
 additional information available from the ILS Web website (www.ilsweb.com). The Massport
 website (www.massport.com) or the Massport technical center.



SINGLE DOOR WITH 2 SIDE LITES

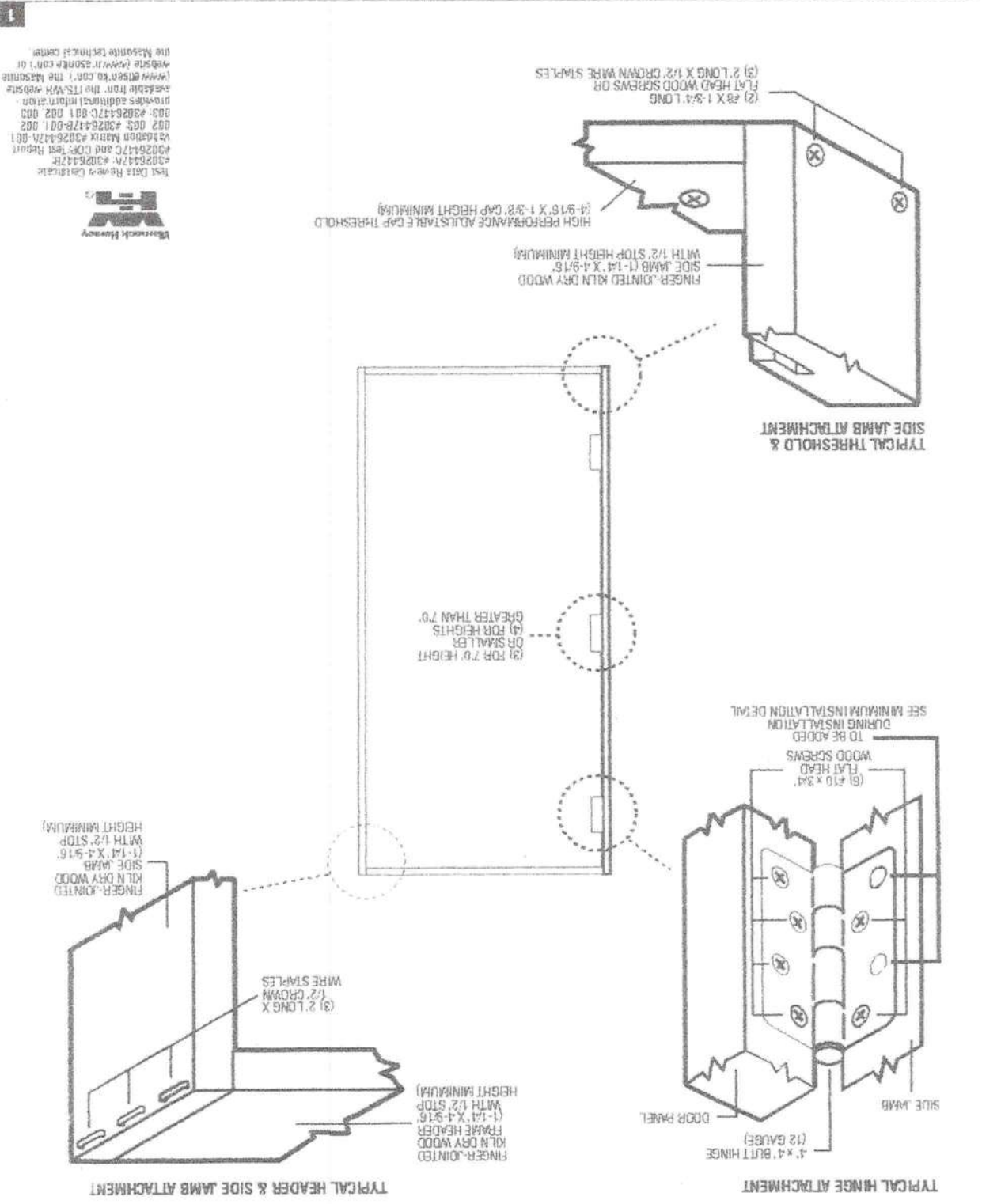
OXO
Unit

MID-WT-MA0004-02

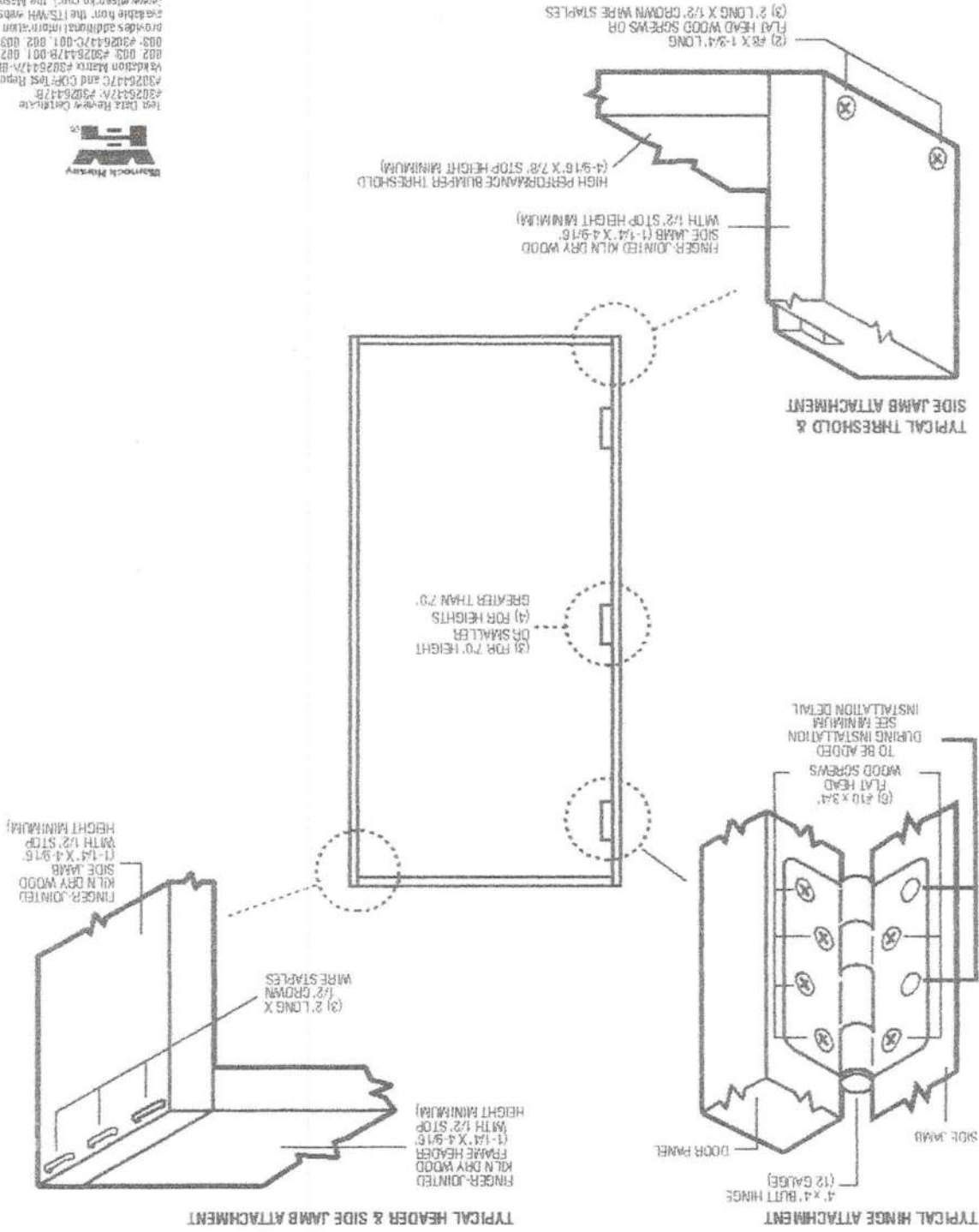
X
Unit

MAD-WL-MA0001-02

INSWING UNIT WITH SINGLE DOOR

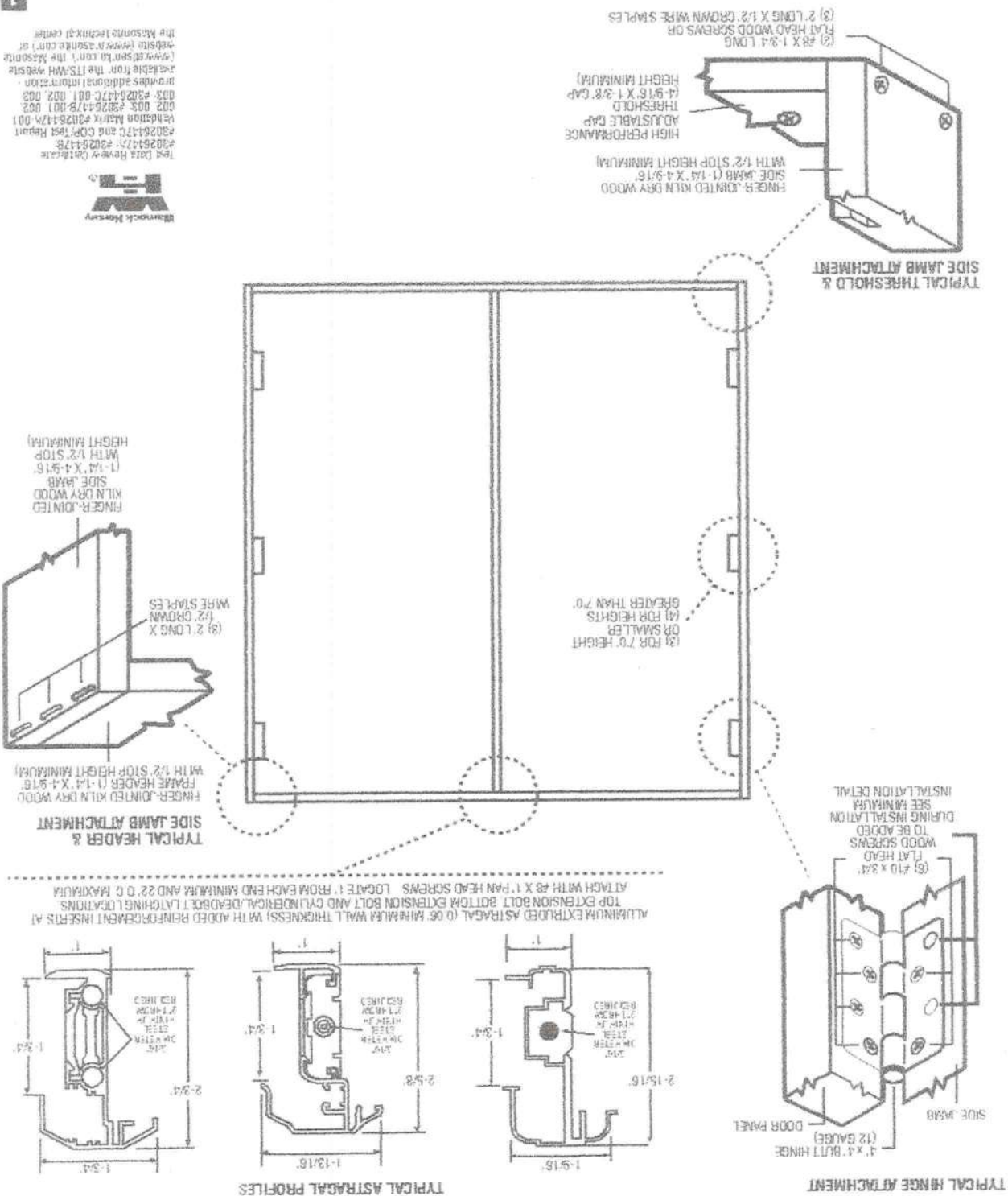


OUTSWING UNITS WITH SINGLE DOOR

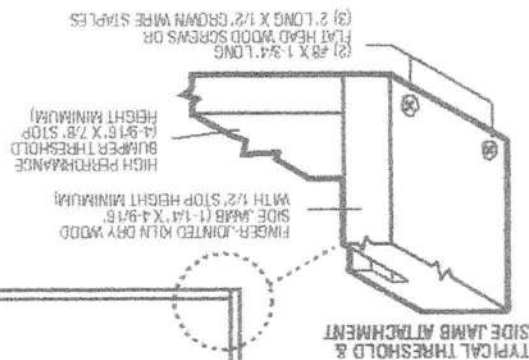
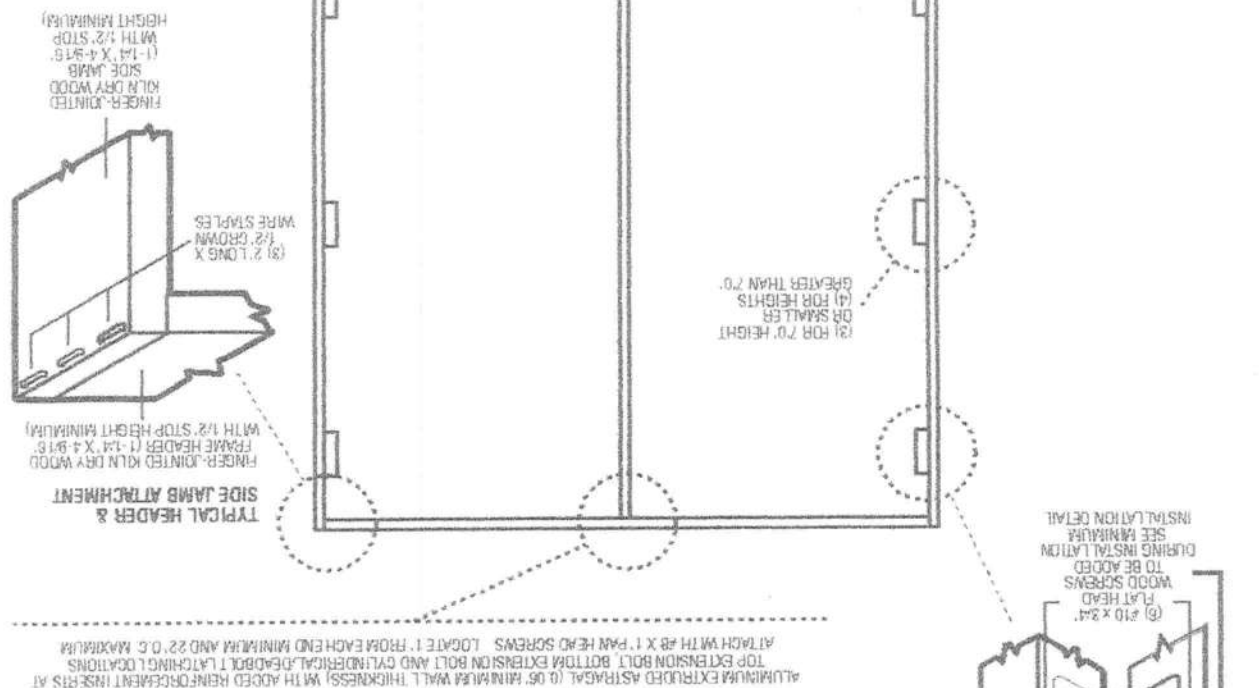
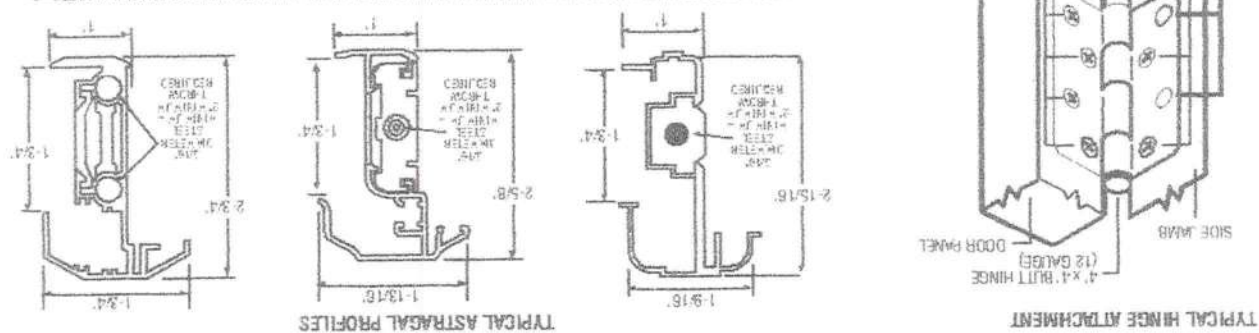


1
 the Masonite Technical Center
 10000 Highway 100, Suite 100
 Mason, Ohio 44869
 216/338-1000
 Fax 216/338-1001
 Telex 338100
 Cable 338100
 10000 Highway 100, Suite 100
 Mason, Ohio 44869
 216/338-1000
 Fax 216/338-1001
 Telex 338100
 Cable 338100

INSWING UNIT WITH DOUBLE DOOR



OUTSWING UNITS WITH DOUBLE DOOR



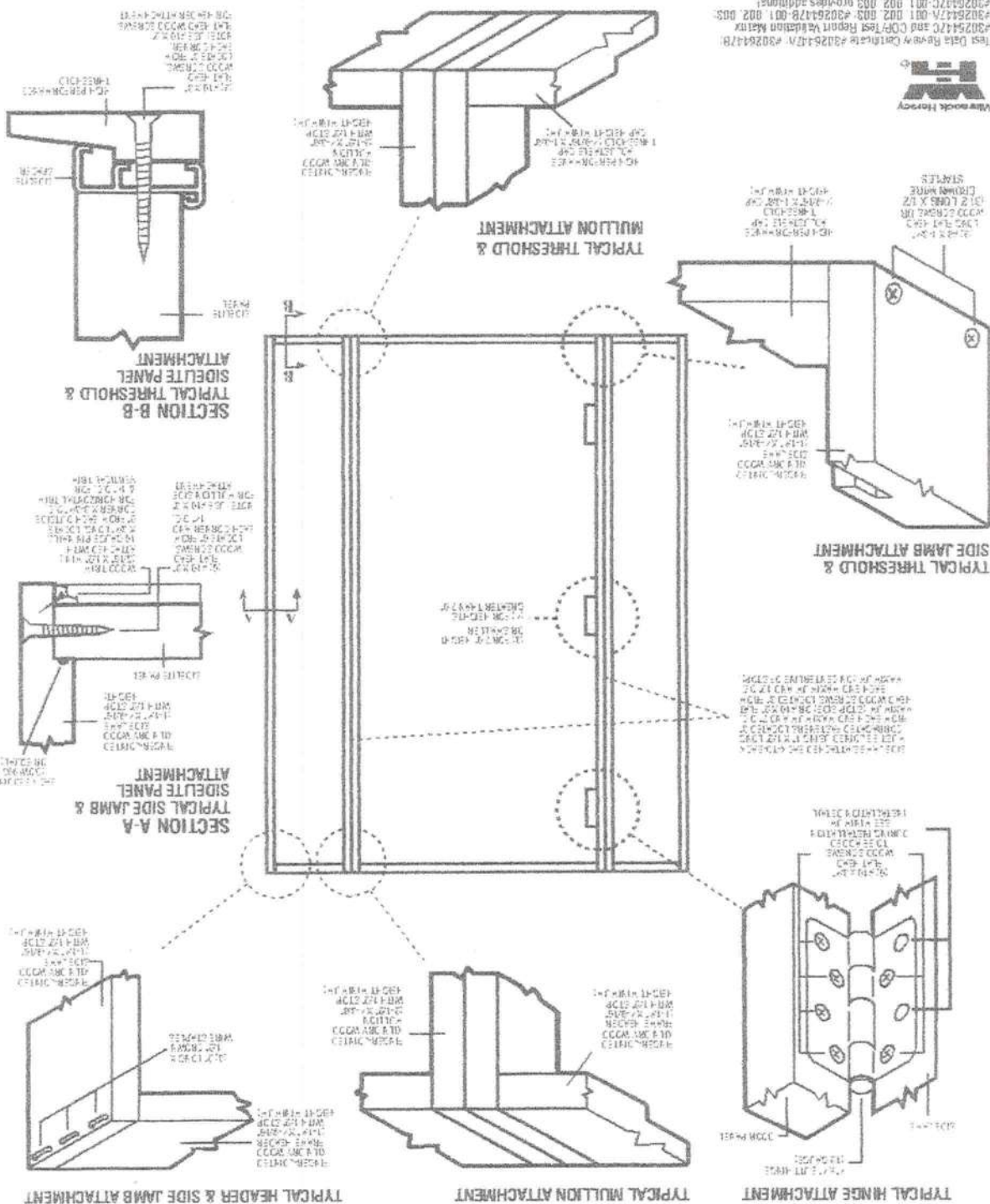
30264177: 30264172E
 3026417C and C07058 Repeat
 3026417C 3026417A 001
 002 003 3026417C 001 002 003
 provides additional information -
 available from the ILS-WHM website
 (www.ils-wm.com) the Mesonite
 Website (www.mesonite.com) or
 the Mesonite Technical Center



MAD-WL-MA0007-02

INSWING UNIT WITH SINGLE DOOR & TWO SIDELITES (BOXED CONSTRUCTION)

OXO
Unit

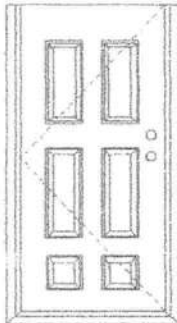


COP-WL-JH4101-02

X
Opaque Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+66.0/-66.0

Forced water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and wind load resistant requirements for a specific building design and geographic location is determined by ASCE 7 manual, state or local building codes specify the edition required.

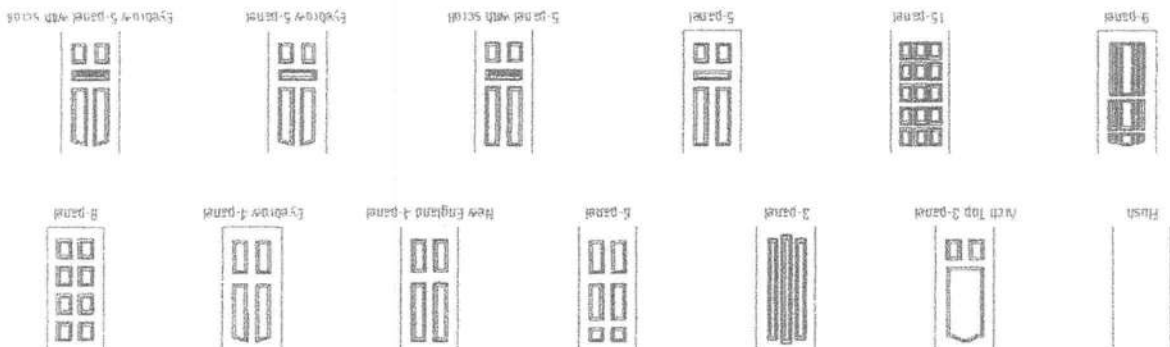
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCITL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).
Kurt A. Balhazor
State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 55533

Test Data Review Certificate #0026-177
and CO-Test Report Validation Matrix
#0026-177-001 provides additional
information available from the ILSWH
website (www.ilswh.com). The
website website (www.ilswh.com) is
in the Masonite technical center.



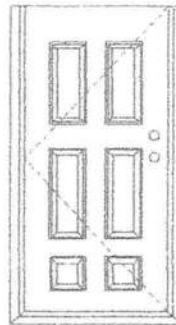
Johnson
Entrysystems
June 17, 2002
Our continuing program of product improvement requires us to change and perfect
products without notice.

COP-WL-JH4121-02

X
Opaque Outswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Maximum unit size = 3'0" x 6'8"

Design Pressure

+66.0/-66.0

limited water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and wind gust resistant requirements for a specific building design and geographic location is determined by ASCE 7, national code or local building codes specify the edition required.

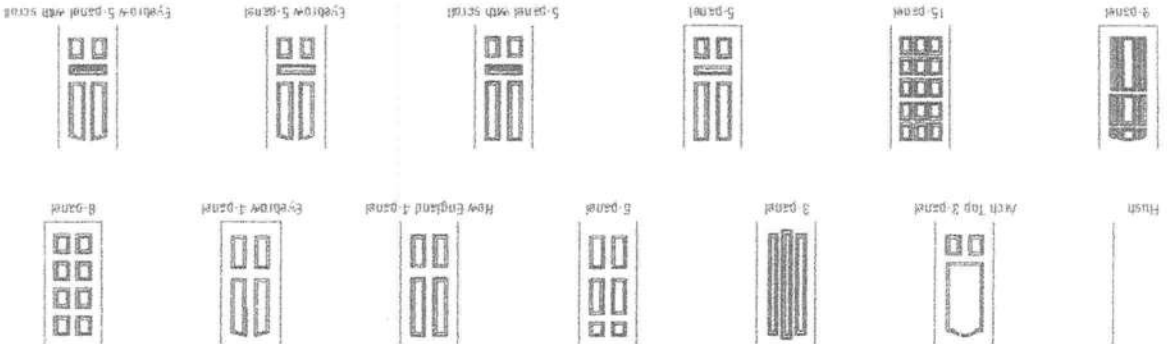
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



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EntrySystems™

June 17, 2002
Our continuing program of product development, research, design and product
improvement is subject to change without notice.



COP-WL-JH4121-02

X
Opaque Outswing Unit

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16256

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

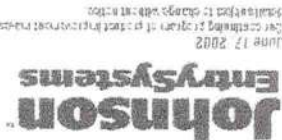
TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt A. Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Blackrock Hursey
Test Data Review Certificate #30264-17A
and COE Test Report Validation Matrix
#30264-17A-001 provides additional
information - available from the ITS-VH
website (www.itsvh.com). The
Masonite website (www.masonite.com) or the Masonite technical center.



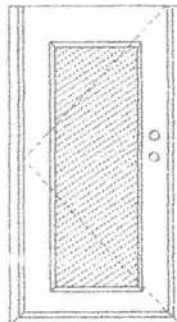
June 17, 2002
Our continuing program of product development makes modifications, design and product
improvements to changes without notice.

COP-WL-JH4141-02

X
Glazed Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this
report as long as the panel used does not
exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+40.5/-40.5

Leaked water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Test Data Review (Certificate #3026-47n
and COP Test Report Validation Matrix
#3026-47n-001) provides additional
information - available from The ITI-WH
website (www.itiwh.com) the
Masonite website (www.masonite.com)



MINIMUM ASSEMBLY DETAIL:

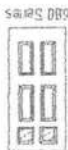
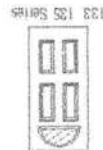
Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02 and
MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

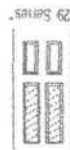
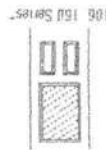
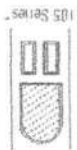
Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



1/2 GLASS:



This glass kit may also be used in the following door styles: 5-panel 5-panel Eyebrow 5-panel Eyebrow 5-panel with screen

Johnson
Entrysystems

June 12, 2002
For additional information or product literature, please contact your local distributor.
Subject to change without notice.



This Data Review Certificate #002547A
and COP Test Report Validation Matrix
#302647A-001 provides additional
information available from the TSMW
website (www.tsmw.com). The
Masonite website (www.masonite.com)
or the Masonite technical center



State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 56533

Kurt Balhazor

To the best of my knowledge and ability the above side-hinged
exterior door unit conforms to the requirements of the 2001 Florida
Building Code, Chapter 17 (Structural Tests and Inspections).

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY STATE

PRODUCT COMPLIANCE LABELING:

Frame constructed of wood with an extruded aluminum threshold.
plastic lip life surround.
cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior
Door panels constructed from 26-gauge 0.017" thick steel skins. Both skins constructed from wood.
Evaluation report NCTL-210-2794-1
Unit Tested in Accordance with Miami-Dade BCCO PA202
Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.
NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

CERTIFIED TEST REPORTS:

3/4 GLASS:				FULL GLASS:			
APPROVED DOOR STYLES:							
	100 Series		110 Series		120 Series		130 Series
	140 Series		150 Series		160 Series		170 Series

WOOD-EDGE STEEL DOORS

COP-WL-JH4141-02

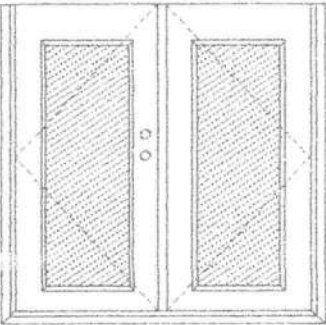
X
Glazed Inswing Unit

COP-WL-JH4142-02

XX
Glazed Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'3".

Test Data Review Certificate #3026417A and COP Test Report Validation Matrix #3026417A-B01 provides additional information - available from the ITS.MWH website (www.itsmwh.com). The Massengill website (www.massengill.com) or the Massengill technical center.



Double Door

Maximum unit size = 6'0" x 6'8"

Design Pressure

+40.5/-40.5

Leak test water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7 national code or local building codes specify the edition required.

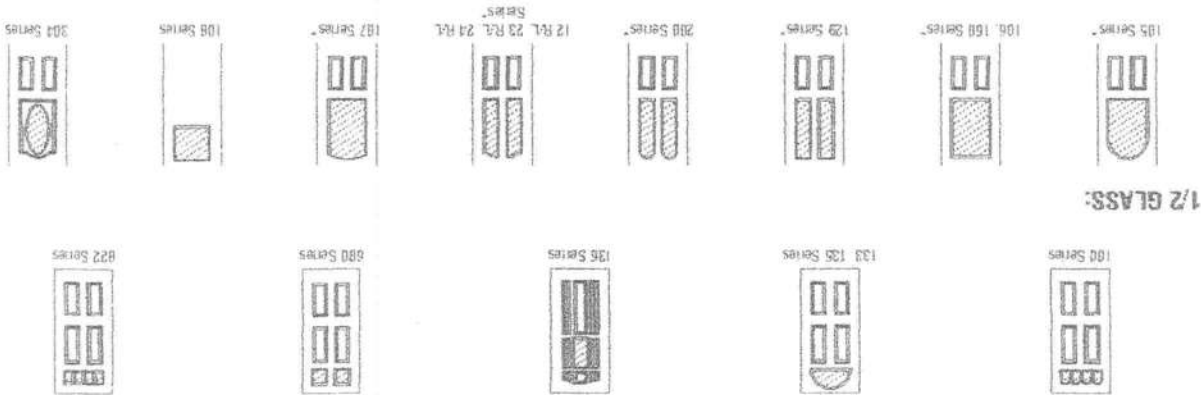
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:



1/2 GLASS:

This glass kit may also be used in the following door styles: 5 panel, 5 panel with scarf, Eyebrow 5 panel, Eyebrow 5 panel with scroll.

Johnson
EntrySystems

JUNE 17, 2002
This document is subject to change without notice.
The following figures are for informational purposes only. Design and product details are subject to change without notice.



Test Data Review Center #3026-17/A
and COP Test Report Validation Matrix
#3026-17/A-001 provides additional
information - available from the ITSC/VH
website (www.itsc.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.



To the best of my knowledge and ability the above side-hinged
exterior door unit conforms to the requirements of the 2001 Florida
Building Code, Chapter 17 (Structural Tests and Inspections).
Kurt J. Balhazor
State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 56533

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY STATE

PRODUCT COMPLIANCE LABELING:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3
Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.
Unit Tested in Accordance with Miami-Dade BCCO PA202
Evaluation report NCTL-210-2794-1
Door panels constructed from 26-gauge 0.017" thick steel skins. Both skins constructed from wood.
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior
cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid
plastic lip like surround.
Frame constructed of wood with an extruded aluminum threshold.

CERTIFIED TEST REPORTS:

APPROVED DOOR STYLES:	
3/4 GLASS:	
400 Series	
410 Series	
450 Series	
FULL GLASS:	
109 Series	
114, 120, 122 Series	
152 Series	
149 Series	
300 Series	

WOOD-EDGE STEEL DOORS

COP-WL-JH4142-02

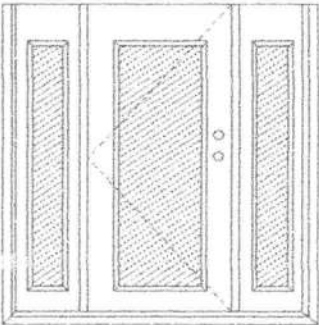
XX
Glazed Inswing Unit

COP-WL-JH4144-02

OXO
Glazed Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure

+40.5/-40.5

Unit: water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7 national code or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

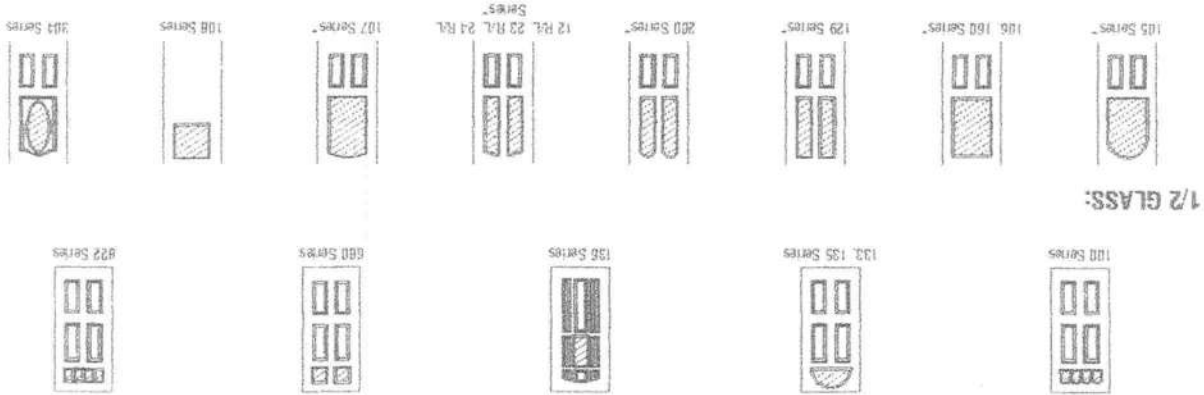
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:

1/4 GLASS:



1/2 GLASS:

Test Data Review Certificate #3026-17A
and CDR Test Report Validation Matrix
#3026-17A-001 provides additional
information - available from the ITSS/VH
website (www.itsvh.com). The
Masonite website (www.masonite.com) or
the Masonite Technical Center



To the best of my knowledge and ability the above side-hinged
exterior door unit conforms to the requirements of the 2001 Florida
Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt Balhazor

State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 56533

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY STATE

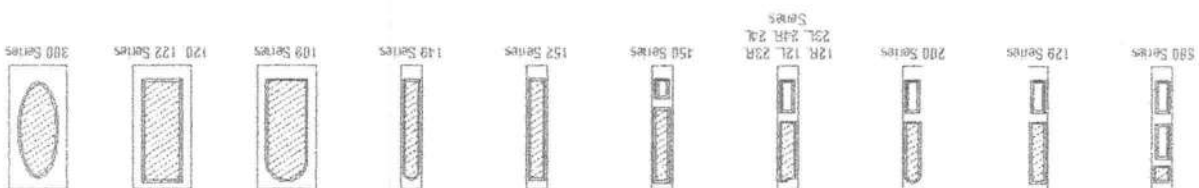
PRODUCT COMPLIANCE LABELING:

Unit Tested in Accordance with Miami-Dade BCCO PA202
Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.
Evaluation report NCTL-210-2794-1
Door panels constructed from 26-gauge 0.017" thick steel skins. Both sills constructed from wood.
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior
cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass
mounted in a rigid plastic lip lite surround.
Frame constructed of wood with an extruded aluminum threshold.

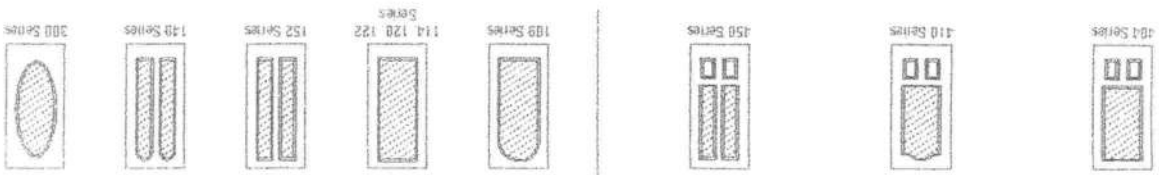
CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

APPROVED SIDELITE STYLES:



APPROVED DOOR STYLES:



FULL GLASS:

WOOD-EDGE STEEL DOORS

COP-WL-JH4144-02

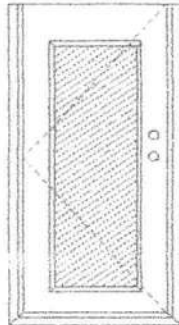
OXO
Glazed Inswing Unit

COP-WL-JH4161-02

X
Glazed Outswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and wind resistance requirements for a specific building design and geographic location is determined by ASCE 7, National State or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

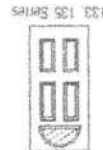
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series

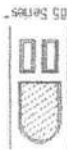


680 Series



822 Series

1/2 GLASS:



105 Series



106, 108 Series



129 Series



200 Series



12 R.L. 23 R.L. 24 R.L. Series



107 Series



108 Series



301 Series

This glass kit may also be used in the following door styles: 5-panel 5-panel with scroll; hybrid 5-panel; hybrid 5-panel with scroll.

Johnson
Entrysystems

June 17, 2002
Not a contract. It is subject to change without notice.
Subject to change without notice.



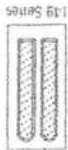
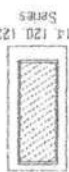
X
Glazed Outswing Unit

COP-WL-JH4161-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3
Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.
Unit Tested in Accordance with Miami-Dade BCCO PA202

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood.
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior
cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid
plastic lip life surround.
Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged
exterior door unit conforms to the requirements of the 2001 Florida
Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt J. Balhazor

State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 56533



Test Data Review Certificate #J026147N
and CCR# Test Report Validation Matrix
#J026147N-001 provides additional
information - accessible from the ITS.WH
website (www.itswh.com). The
Masonite website (www.masonite.com) or the Masonite technical center.

Johnson
Entrysystems

June 12, 2002
Our continuing program of product improvement studies, designs and product
development is subject to change without notice.

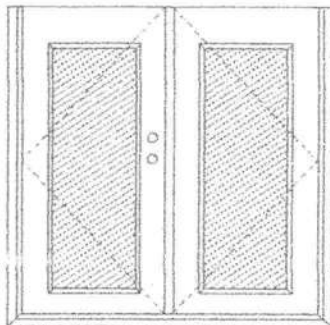


COP-WL-JH4162-02

XX
Glazed Outswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Test Data Review Certificate #2026417A and COP/Ret Report Valuation Matrix #2026417A-B01 provides additional information - available from the TTS-WH website (www.ttswh.com). The Masonite website (www.masonite.com) or the Masonite technical center.



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure

+40.5/-40.5

Let air/water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

factual design pressure and in part resistant requirements for a specific building design and geographic location is determined by ASCE 7-national state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

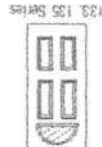
Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

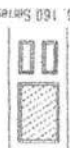
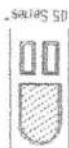
Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



1/2 GLASS:



*This glass kit may also be used in the following door styles: 5-panel, 5-panel with scroll, Eyebrow, 5-panel, Eyebrow, 5-panel with scroll.

Test Data Review Certificate #3026-137
#3026-137-001 provides additional
information available from the ITS/WH
website (www.itswh.com). The
Masonite website (www.masonite.com) at the Masonite technical center.



State of Florida, Professional Engineer
Kurt Balhazor, P.E. - License Number 56533

Kurt Balhazor

To the best of my knowledge and ability the above side-hinged
exterior door unit conforms to the requirements of the 2001 Florida
Building Code, Chapter 17 (Structural Tests and Inspections).

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

PRODUCT COMPLIANCE LABELING:

NCITL 210-1897-7, 8, 9, 10, 11, 12, NCITL 210-1864-5, 6, 7, 8, NCITL 210-2178-1, 2, 3
Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.
Unit Tested in Accordance with Miami-Dade BCCO PA202.
Evaluation report NCITL-210-2794-1
Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood.
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior
cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid
plastic lip lite surround.
Frame constructed of wood with an extruded aluminum bumper threshold.

CERTIFIED TEST REPORTS:

APPROVED DOOR STYLES:		FULL GLASS:	
	101 Series		109 Series
	110 Series		111, 120, 122 Series
	150 Series		152 Series
	159 Series		300 Series

WOOD-EDGE STEEL DOORS

COP-WL-JH4162-02

XX
Glazed Outswing Unit

NPT 1 0 0000

Project Information for:	
L136166	
JOHN JOHNSON" OWNER B1 Date:	10/12/2005
LOT 6 RUSSWOOD EST.	Start Number:
N/A	
COLUMBIA COUNTY	
83	
Truss Page Count:	
Truss Design Load Information (UNO)	
Design Program: Mittek 5.2 / 6.2	
Building Code:	FBC2004
Wind	ASCE 7-98
Roof (psf):	42
Floor (psf):	55
Wind Standard:	110
Wind Speed (mph):	
Note: See individual truss drawings for special loading conditions	
Building Designer, responsible for Structural Engineering: (See attached)	
Owner Builder	
Address: N/A	
SAINT AUGUSTINE, FLORIDA 32084	
Designer:	84
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987	
Company:	Structural Engineering and Inspections, Inc. EB 9196
Address:	16105 N. Florida Ave, Ste B, Lutz, FL 33549
Notes:	
1. Truss Design Engineer is responsible for the individual trusses as components only.	
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2	
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.	

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	101205525	10/12/2005	41	T12	101205565	10/12/2005
2	CJ3	101205526	10/12/2005	42	T13	101205566	10/12/2005
3	CJ3A	101205527	10/12/2005	43	T14	101205567	10/12/2005
4	CJ5	101205528	10/12/2005	44	T15	101205568	10/12/2005
5	CJ5A	101205529	10/12/2005	45	T16	101205569	10/12/2005
6	EJ2	101205530	10/12/2005	46	T17	101205570	10/12/2005
7	EJ5	101205531	10/12/2005	47	T18	101205571	10/12/2005
8	EJ5A	101205532	10/12/2005	48	T19	101205572	10/12/2005
9	EJ7	101205533	10/12/2005	49	T20	101205573	10/12/2005
10	EJ7A	101205534	10/12/2005	50	T21	101205574	10/12/2005
11	EJ7B	101205535	10/12/2005	51	T22	101205575	10/12/2005
12	HJ2	101205536	10/12/2005	52	T23	101205576	10/12/2005
13	HJ6	101205537	10/12/2005	53	T24	101205577	10/12/2005
14	HJ7	101205538	10/12/2005	54	T25	101205578	10/12/2005
15	HJ9	101205539	10/12/2005	55	T26	101205579	10/12/2005
16	HJ9A	101205540	10/12/2005	56	T27	101205580	10/12/2005
17	PB15	101205541	10/12/2005	57	T28	101205581	10/12/2005
18	PB15A	101205542	10/12/2005	58	T29	101205582	10/12/2005
19	PB15B	101205543	10/12/2005	59	T30	101205583	10/12/2005
20	PB19	101205544	10/12/2005	60	T31	101205584	10/12/2005
21	PB19A	101205545	10/12/2005	61	T32	101205585	10/12/2005
22	PB19B	101205546	10/12/2005	62	T33	101205586	10/12/2005
23	PB19C	101205547	10/12/2005	63	T34	101205587	10/12/2005
24	PB19D	101205548	10/12/2005	64	T35	101205588	10/12/2005
25	PB19E	101205549	10/12/2005	65	T36	101205589	10/12/2005
26	PB19F	101205550	10/12/2005	66	T37	101205590	10/12/2005
27	PB19G	101205551	10/12/2005	67	T38	101205591	10/12/2005
28	T01	101205552	10/12/2005	68	T39	101205592	10/12/2005
29	T01G	101205553	10/12/2005	69	T40	101205593	10/12/2005
30	T02	101205554	10/12/2005	70	T41	101205594	10/12/2005
31	T03	101205555	10/12/2005	71	T42	101205595	10/12/2005
32	T04	101205556	10/12/2005	72	T43	101205596	10/12/2005
33	T05	101205557	10/12/2005	73	T44	101205597	10/12/2005
34	T06	101205558	10/12/2005	74	T45	101205598	10/12/2005
35	T07	101205559	10/12/2005	75	T46	101205599	10/12/2005
36	T08	101205560	10/12/2005	76	T47	101205600	10/12/2005
37	T09	101205561	10/12/2005	77	T48	101205601	10/12/2005
38	T10	101205562	10/12/2005	78	T49	101205602	10/12/2005
39	T10G	101205563	10/12/2005	79	T50	101205603	10/12/2005
40	T11	101205564	10/12/2005	80	V04	101205604	10/12/2005

Job	Truss	CJ3	MONO TRUSS	Qty	14	JOHN JOHNSON	Dwg.#101205526
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Builders FirstSource, Lake City, FL 32055
6,200 s Jul 13 2005 Mittek Industries, Inc. Mon Oct 10 18:55:15 2005 Page 1

Scale = 1/16"

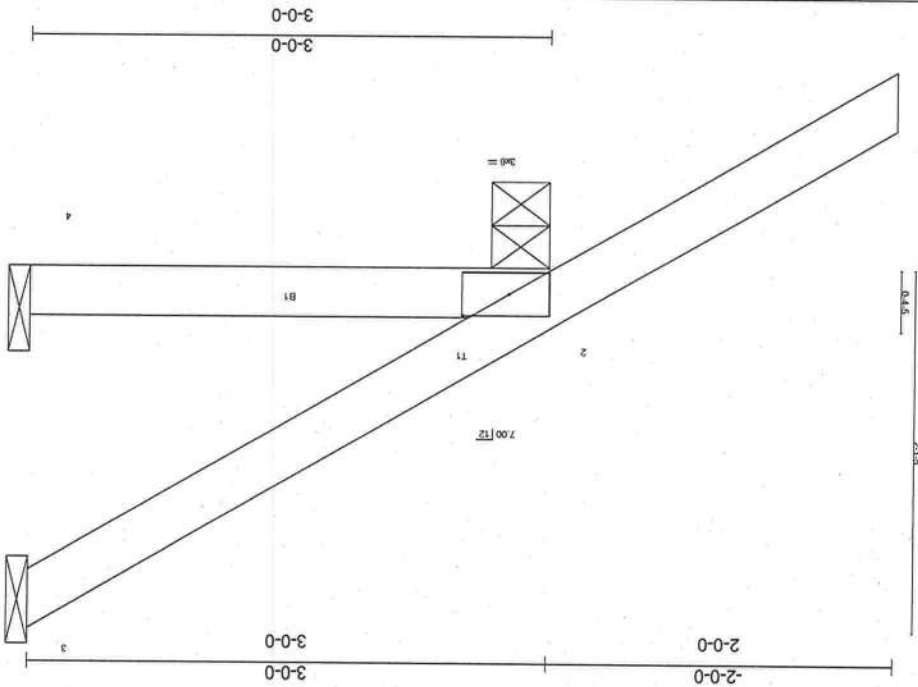


Plate Offsets (X,Y): [2,0,3,3,0,1-8]

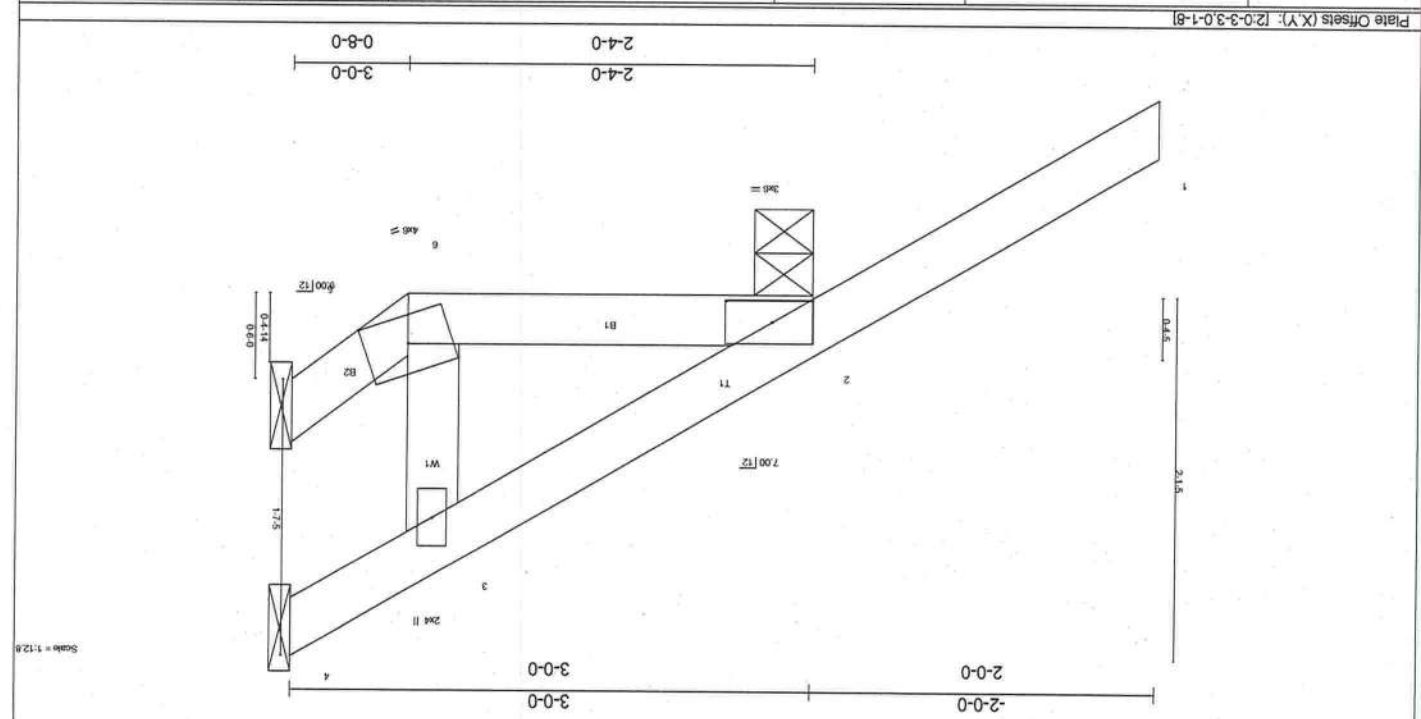
LOADING (psf)	SPACING	Code FBC2004/TP12002	Rep Stress Incr	Lumber Increase	Plates Increase	CSI	DEFL	VERT (TL)	VERT (BL)	HORIZ (TL)	BRACING
BCDL 5.0	2'-0"	YES	1.25	1.25	1.25	0.29	0.01	0.01	0.01	0.00	TOP CHORD
BCLL 10.0						0.08	0.01	0.01	0.01	0.00	VERT (TL)
TCCL 20.0						0.29	0.01	0.01	0.01	0.00	VERT (BL)
TCCL 20.0						0.29	0.01	0.01	0.01	0.00	HORIZ (TL)
BCDL 5.0						0.00	0.01	0.01	0.01	0.00	DEFL

LUMBER TOP CHORD 2 X 4 SYP No.2
BRACING TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horiz 2=154(load case 5)
Max Uplift 3=30(load case 6), 2=237(load case 5), 4=33(load case 3)
Max Grav 3=31(load case 3), 2=279(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-65/15
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft, TCCL=4.2psf, BCDL=1.60, DCL=1.60. This truss is designed for C-C for members and forces, and for Exterior(2) zone, porch left and right exposed; Lumber DCL=1.60 plate grip DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3, 237 lb uplift at joint 2 and 33 lb uplift at joint 4.
LOAD CASE(S) Standard

[illegible]

NUMBER	TOP CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2	WEBS	2 X 4 SYP No.3
			BRACING			
			TOP CHORD			
			BOT CHORD			
			Structural wood sheathing directly applied or 3-0-0 oc purlins.			
			Rigid ceiling directly applied or 6-0-0 oc bracing.			

REACTIONS

(lb/in²) ± 462 /Mechanical, $2=279$ /0/4-0, $5=9$ /Mechanical
Max Horiz $\pm 2=154$ /load case 5)
Max Uplift $\pm 4=24$ /load case 5), $2=202$ /load case 5)

FORCES (b) - Maximum Compression/Maximum Tension
 $1-2=5/4$, $2-3=4/12$, $3-4=15/29$
 $2-6=5/0$, $5-6=4/4$
WEBS
 $3-6=0/42$

NOTES

1) Wind: ASCE 7-02: 11 mph (3-second gust); $h=20$ ft; TCDL=4.2psf; BCDL=4.2psf
2) Exterior (2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for (2) zone connections
3) Provide mechanical connection (by others) of truss to bearing plate cap

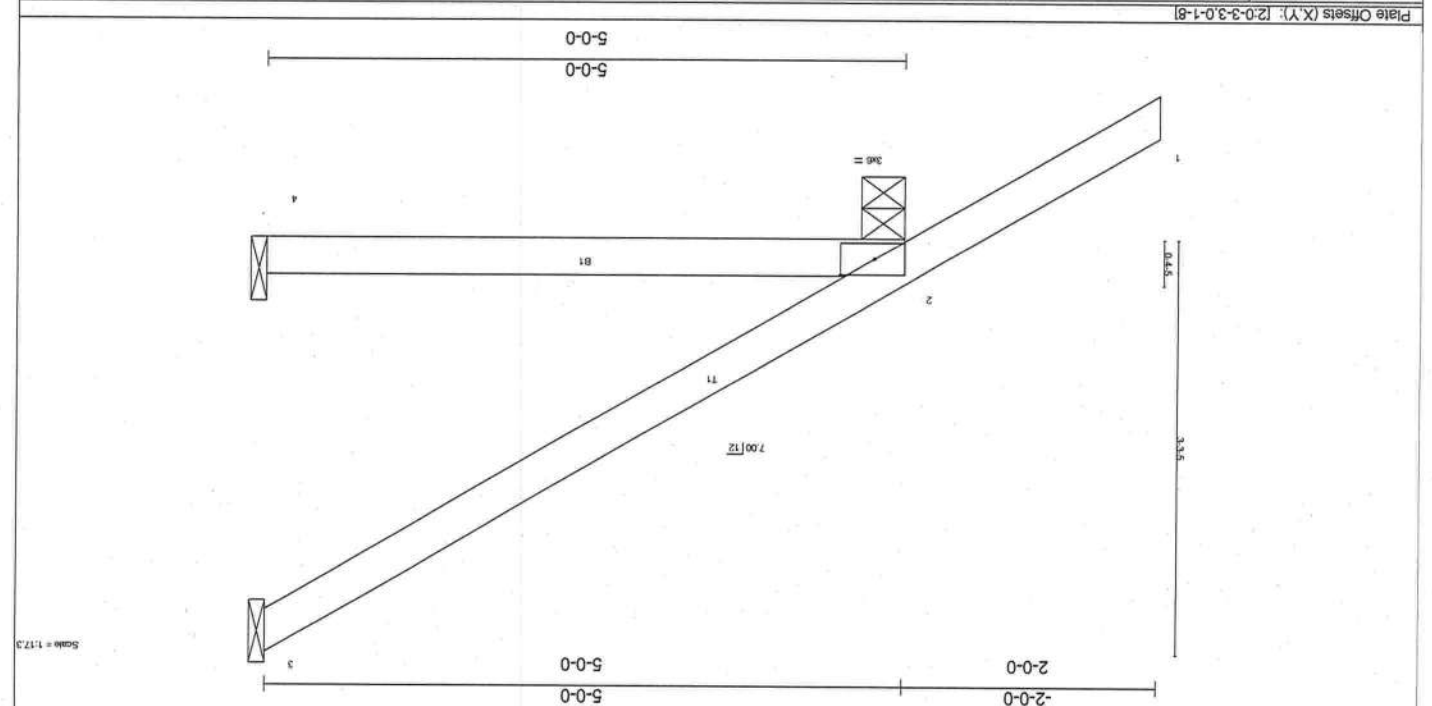
LOAD CASE(S) Standard

LOAD CASE(S) Standard

1) Wind: ASCE 7-02, 10mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp. B; enclosed; MWFRS gable end zone and C-C
2) Exterior(2) zone; Lumber DOL=1.60 plate gip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 4 and 202 lb uplift at joint 2.

Job	Truss	CJS	MONO TRUSS	Qty	4	Ply	JOHN JOHNSON	Dwg.#101205528
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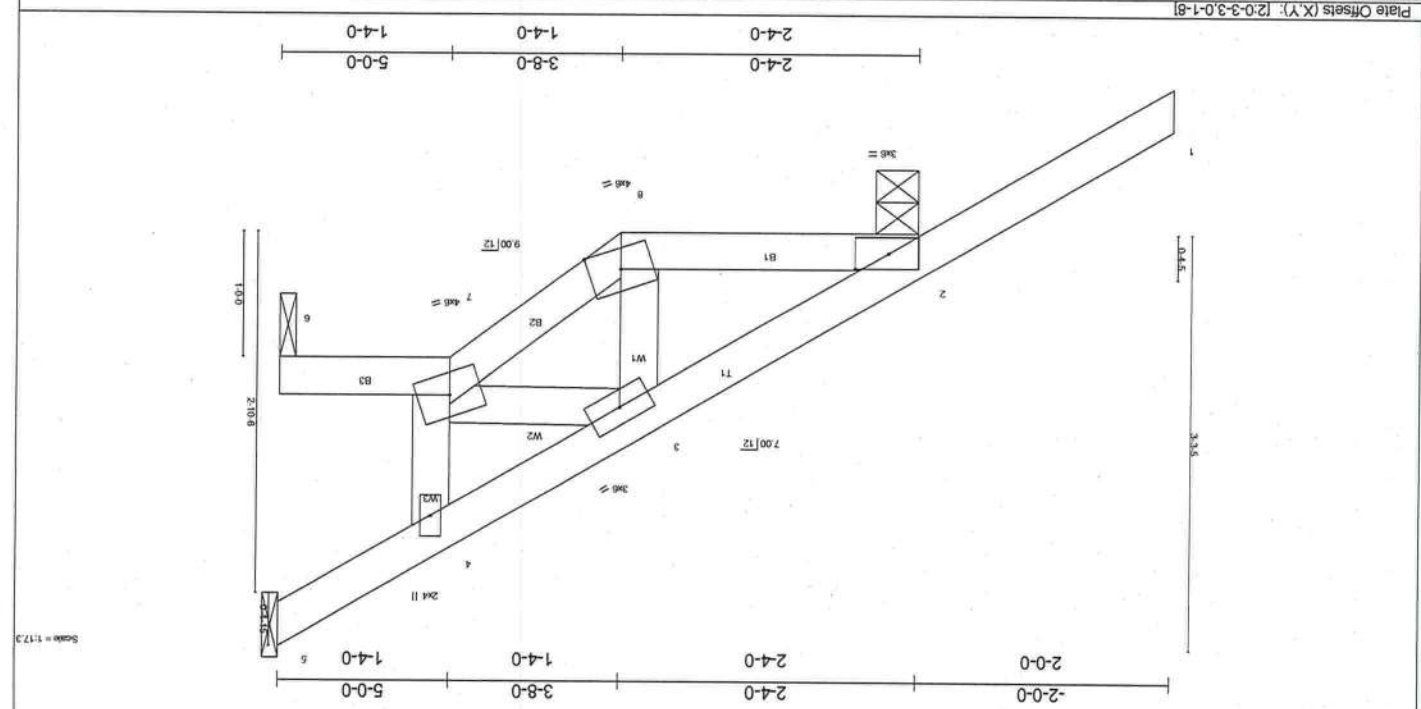


LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER
TC/L 20.0	2'-0"	TC 0.29	Vert(TL) 0.09	TOP CHORD	TOP CHORD 2 X 4 SYP No.2
TC/DL 7.0		BC 0.24	Vert(TL) 0.07	BOT CHORD	BOT CHORD 2 X 4 SYP No.2
BC/L 10.0		WB 0.00	Horz(TL) -0.00		
BC/DL 5.0					

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horiz Z=207(load case 5)
Max Uplift=95(load case 5), 2=-252(load case 5), 4=-56(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-81/40
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TC/DL=4.2psf; BC/DL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3, 252 lb uplift at joint 2 and 56 lb uplift at joint 4.
LOAD CASE(S) Standard

[illegible]

TOP CHORD 2 X 4 SYP No.2	TOP CHORD	2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3		
LUMBER		
BRACING		
TOP CHORD	Structural wood sheathing directly applied or 5-0-0 cc purlins	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing.	

REACTIONS (lb/size) 5=155/Mechanical, 2=344/0-4-0, 6=19/0-1-8
Max Hot 2=307/0-2-5

Max Uplift = -85 (load case 5), 2 = -192 (load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-215/0, 3-4=-90/0, 4-5=-52/75
 BOT CHORD 1-2=54/0, 2-3=0/215, 3-4=90/0, 4-5=52/75

2-8=-50/130, 1-8=-58/166, 6-7=-0/0
3-8=-41/44, 3-7=-109/42, 4-7=-35/156
WEBS

NOTES

1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior (2) zone; Lumber DCL=1.60 plate grip DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Refer to girders (3) for truss to truss connections.

3) Provide mechanical connection (by others) of truss to bearing plate at joint(s).

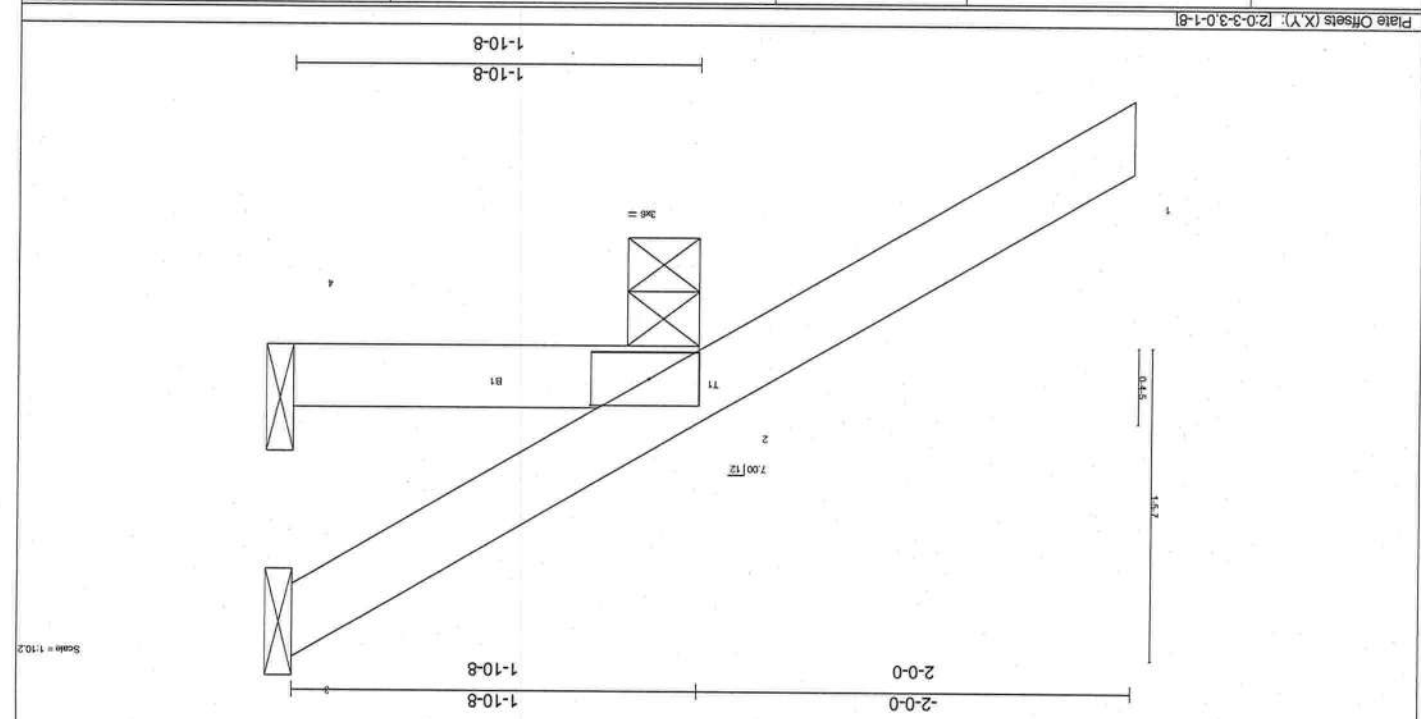
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 5 and 192 lb uplift at joint 2.

LOAD CASES) Standard

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L136166	Truss	EJ2	JACK	1	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205530
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Builders FirstSource, Lake City, FL 32055
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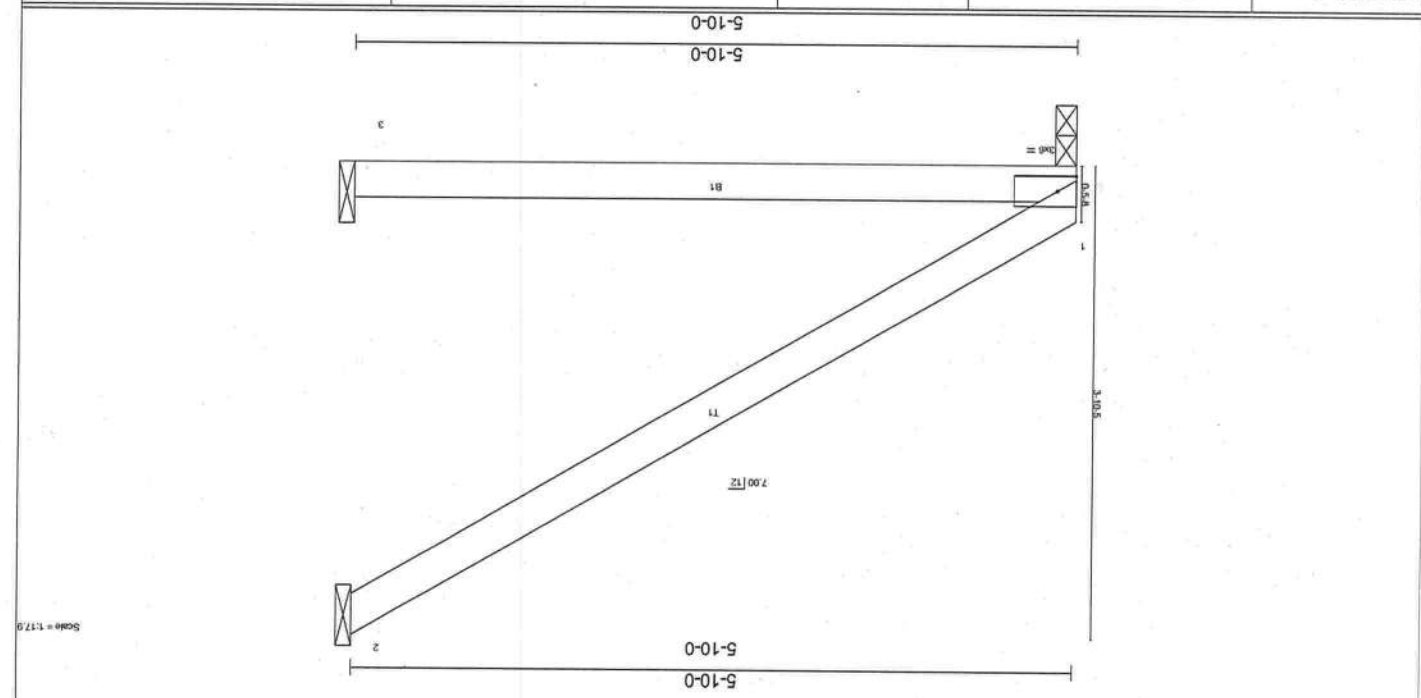


LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER	REACTIONS	FORCES	NOTES
BCDL 5.0 BCLL 10.0 TCCL 7.0 TCLL 20.0	Code FBC2004/TP12002 Rep Stress Incr YES Lumber Increase 1.25 Plates Increase 1.25 2-0-0	WB 0.00 BC 0.02 TC 0.26 CSI (Matrix)	Horz(TL) -0.00 Vert(TL) -0.00 In (loc) 3 L/delt 180 L/d 240	TOP CHORD BOT CHORD Rigid ceiling wood sheathing directly applied or 1-10-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.	TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	(lb/size) 2-24/8-0-4-0, 4=27/Mechanical Max Horiz Z=125(load case 5) Max Uplift Z=213(load case 1), 3=12(load case 5) Max Grav Z=248(load case 1), 4=27(load case 1), 3=35(load case 5)	(lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/53, 2-3=-58/22 BOT CHORD 2-4=0/0	1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4, 2psf; BCDL=3, 0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) Refer to girder(s) for truss to truss connections. 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2 and 12 lb uplift at joint 3.

Job	Truss	EJSA	JACK	Qty	4	Ply	JOHN JOHNSON	Dwg.#101205532
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	PLATES	GRIP
BCDL 5.0	2-0-0	TC 0.40	0.17	1-3	240	MT20	244/190
BCLL 10.0	Rep Stress Incr YES	WB 0.00	0.15	1-3	>398		
TCDL 7.0	Lumber Increase 1.25	BC 0.34	0.15	1-3	>465		
TCLL 20.0	Plates Increase 1.25		0.17	1-3	>398		
				2	n/a		
				n/a	n/a		

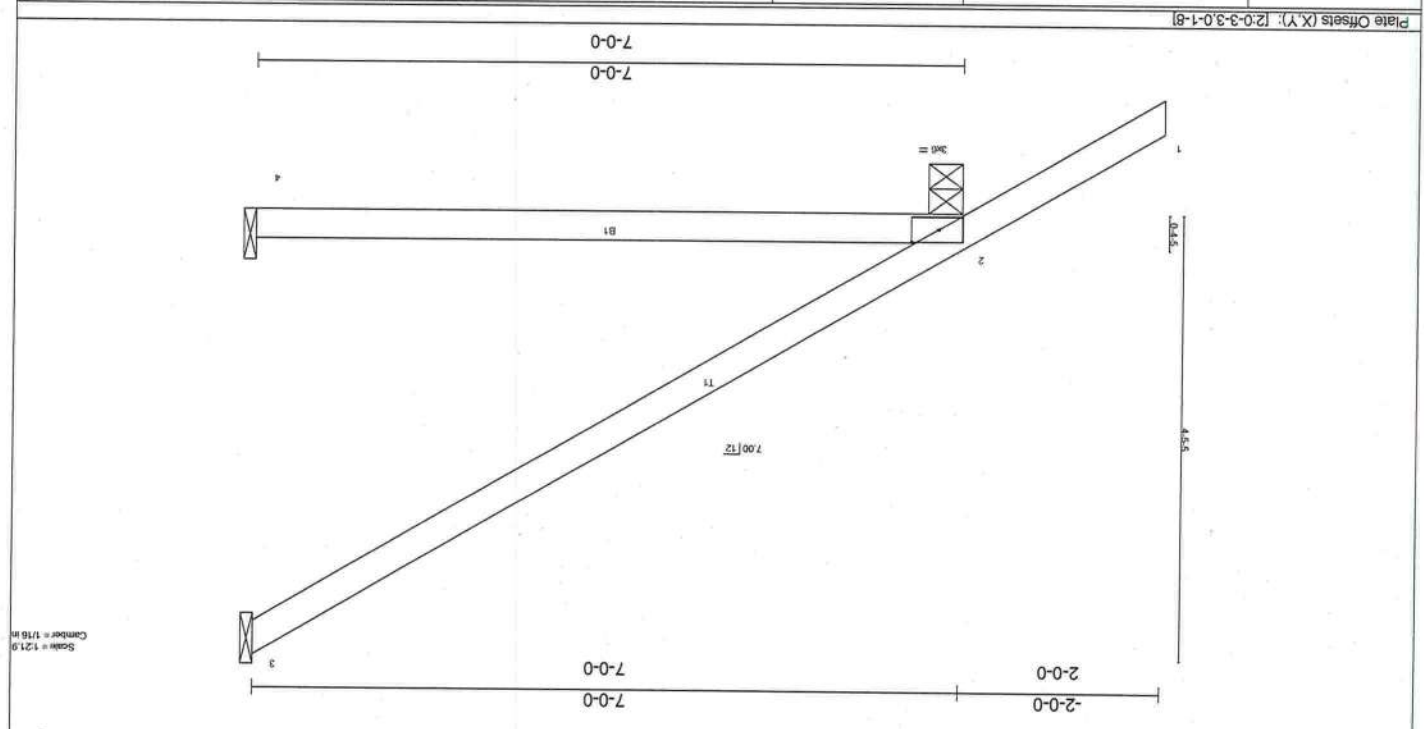
LUMBER	TOP CHORD 2 X 4 SYP No.2	BRACING	TOP CHORD	Structural wood sheathing directly applied or 5-10-0 oc purlins.
	BOT CHORD 2 X 4 SYP No.2		BOT CHORD	Rigid ceiling wood sheathing directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=239/0-2-0, 2=154/Mechanical, 3=65/Mechanical
 Max Horiz 1=162(load case 5)
 Max Uplift 1=-112(load case 5), 2=-157(load case 5), 3=-67(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-114/64
 BOT CHORD 1-3=0/0

NOTES
 1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 1.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 1, 157 lb uplift at joint 2 and 67 lb uplift at joint 3.

Job	L136166	Truss Type	MONO TRUSS	Qty	27	Ply	JOHN JOHNSON	Dwg. #101205533
Builders FirstSource, Lake City, FL 32055								
6:200 s Jul 13 2005 Mike Industries, Inc. Mon Oct 10 18:55:19 2005 Page 1								
Job Reference (optional)								



LOADING (psf)	SPACING	PLATES INCREASE	REPS INCR	CODE FB/C2004/FP12002	CSI	DEFL	BRACING
BCDL 5.0	2'-0"	1.25	YES		WB 0.00	Horz(TL) -0.00	TOP CHORD
BCLL 10.0		1.25			BC 0.35	Vert(TL) -0.20	Structural wood sheathing directly applied or 6'-0" oc purlins.
TCCL 7.0		1.25			TC 0.44	Vert(LL) -0.12	
TCLL 20.0						In (oc) 3	
						Idell 24	
						L/d 240	
						n/a 180	
						n/a	
						Weight: 26 lb	
							PLATES MT20
							GRIP 244/190

REACTIONS (lb/size) 3=161/Mechanical, 2=420/0-4-0, 4=104/Mechanical
 Max Horiz 2=261 (load case 5)
 Max Uplift 3=-146 (load case 5), 2=-198 (load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-111/64
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft: TCCL=4.2psf: BCDL=3.0psf: Category II: Exp B: enclosed: MWFRS gable end zone and C-C Exterior (2) zone: Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 3 and 198 lb uplift at joint 2.

LOAD CASE(S) Standard



TOP CHORD 2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3		

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=389/6, 3-4=-114/0, 4-5=-70/65
BOT CHORD 2-8=-116/296, 7-8=-85/265, 6-7=-1/0
WEBS 3-8=0/23, 3-7=-294/123, 4-7=0/83

NOTES

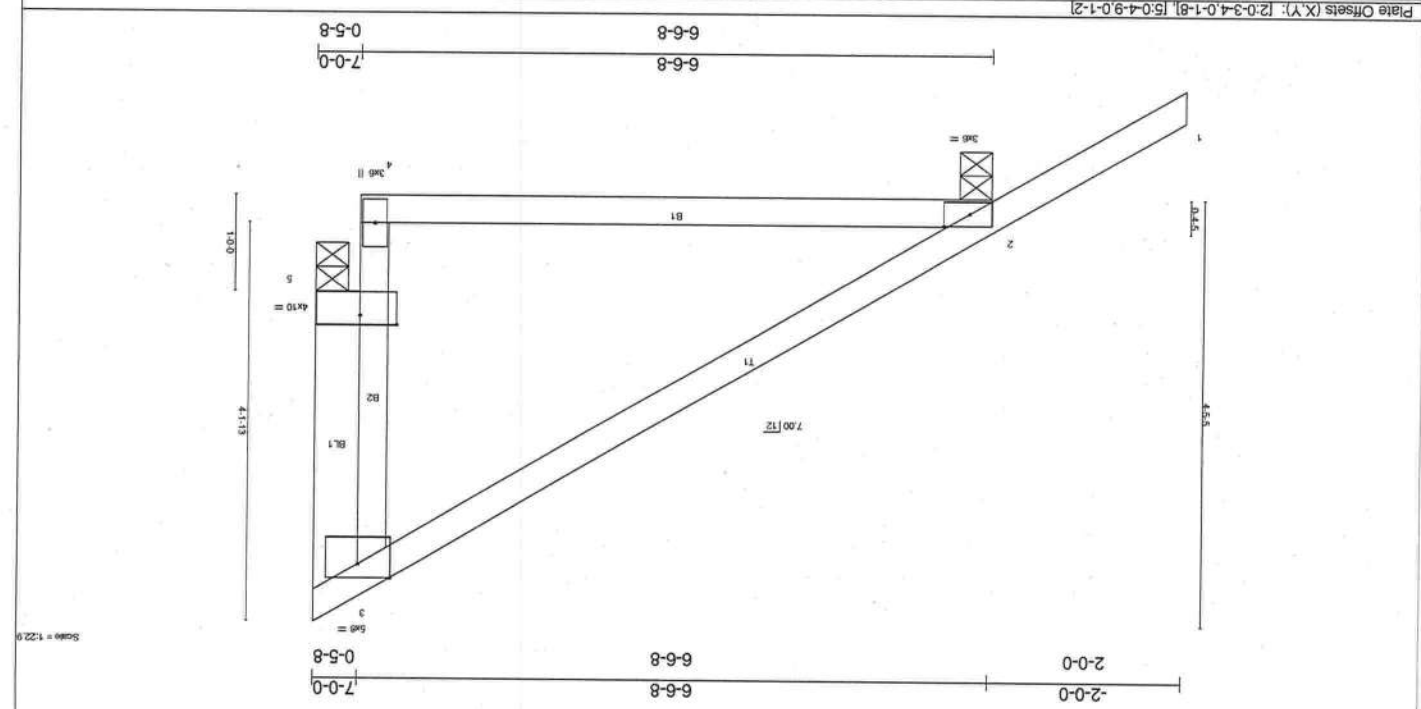
1) Wind: ASCE 7-02, 10mph (3-second gust); $n=20$; $TCDL=4.2psf$; $BCLD=3.0psf$; Category II; Exp. B; enclosed; MWFRS gable end zone and C-C
Exterior(2) zone; Lumber $DCL=1.60$ plate grip $DCL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girders (or truss to truss connection)
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 5, 198 lb uplift at joint 2 and 32 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	EJ7B	L136166
Truss Type	SPECIAL		
Qty	1		
Ply	1		
JOHN JOHNSON			
Dwg.#101205535			

Builders FirstSource, Lake City, FL 32055
 6:200 s JUL 13 2005 Mittek Industries, Inc. Tue Oct 11 17:18:01 2005 Page 1

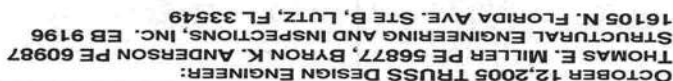


LOADING (psf)	SPACING	Code FBC2004/FP12002	CSI	DEFL	BRACING
TCLL 20.0	2.0-0	Plates Increase 1.25	TC 0.29	Vert(TL) -0.04	TOP CHORD
TCCL 7.0		Lumber Increase 1.25	BC 0.39	Vert(TL) -0.07	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BCLL 10.0		Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	Rigid ceiling wood sheathing directly applied or 10-0-0 oc bracing.
BCDL 5.0				in (loc) 5	BOT CHORD

LUMBER	2 X 4 SYP No.2
TOP CHORD	2 X 4 SYP No.2 *Except*
BOT CHORD	B2 2 X 4 SYP No.3
OTHERS	2 X 6 SYP No.1D

REACTIONS	(lb/size) 5=254/0-4-0, 2=399/0-4-0
	Max Horiz 2=253(load case 5)
	Max Uplift 5=-136(load case 5), 2=-191(load case 5)
FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/54, 2-3=-169/11
BOT CHORD	4-5=0/108, 3-5=-140/160, 2-4=-38/75

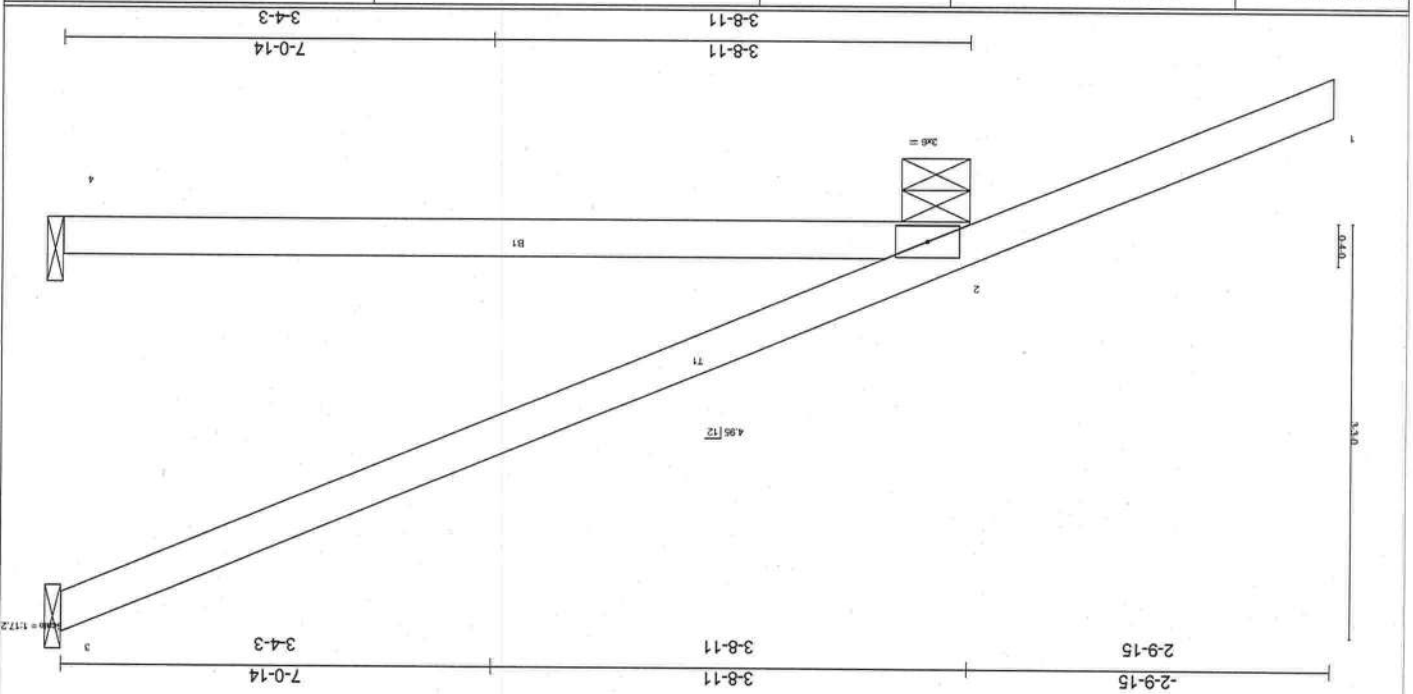
NOTES
 1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 2) Bearing at joint(s): Lumber DCL=1.60 plate gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 136 lb uplift at joint 5 and 191 lb uplift at joint 2.
 LOAD CASE(S) Standard



Job	Truss	Truss Type	MONO TRUSS	Qty	5	Ply	JOHN JOHNSON	Dwg.#1010205538
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Builders FirstSource, Lake City, Fl 32055

6,200 s Jul 13 2005 Mike Industries, Inc. Mon Oct 10 18:55:21 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	TOP CHORD	BRACING	Weight: 27 lb
TCLL 20.0	Plates Increase 1.25	TC 0.55	in (loc)	TOP CHORD	Structural wood sheathing directly applied or 10'-0" oc bracing.	PLATES MT20 244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.27	Vert(TL) -0.13	TOP CHORD		
BCLL 10.0	Rep Stress Incr NO	WB 0.00	Horz(TL) -0.00	BOT CHORD		
BCDL 5.0	Code FBC2004/TP12002					

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=190/Mechanical, 2=381/0-6-7, 4=110/Mechanical

Max Horiz 2=195(load case 4)
Max Uplift 3=-153(load case 4), 2=-243(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/58, 2-3=-8/51

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 153 lb uplift at joint 3 and 243 lb uplift at joint 2.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

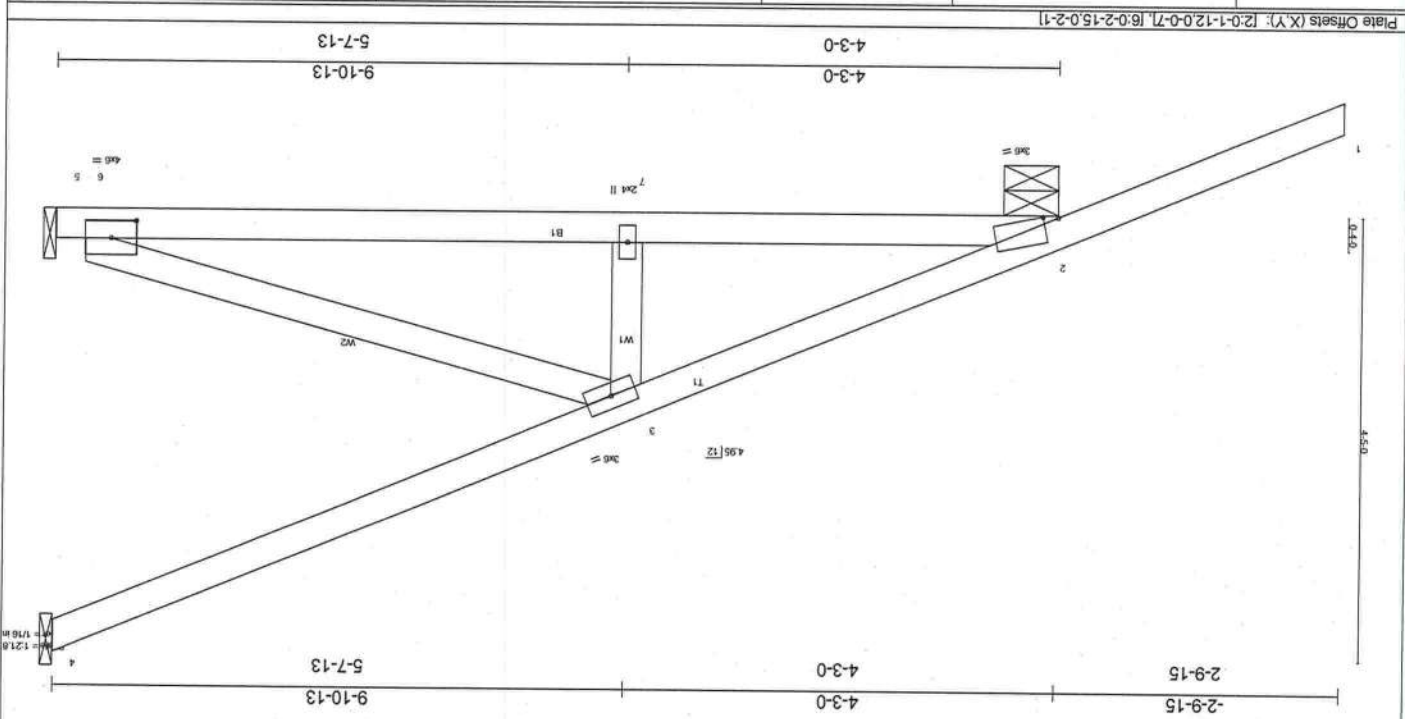
(1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (psf)

Vert 1-2=-54

Trapezoidal Loads (psf)

Vert: 2=-4(F=25, B=25)-10-3=-95(F=21, B=21), 2=0(F=15, B=15)-10-4=-53(F=-12, B=-12)



LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
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	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
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	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	TCLL	20.0		Plates Increase 1.25								
LUMBER	BCDL	5.0	SPACING 2-0-0	Code FBC2004/TP12002	(Matkx)	DEFL in (loc)	Vert(LL) 6-7	Vert(TL) -0.09	Horz(TL) 0.01	n/a	n/a	Weight: 46 lb
	BCLL	10.0		Rep Stress Incr NO								
LOADING (psf)	TCCL	7.0	SPACING 2-0-0	Lumber Increase 1.25	CSL	in (						

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBBS	2 X 4 SYP No.3
BRACING	
TOP CHORD	
BOT CHORD	
Structural wood sheathing directly applied or 6-0-0 oc purfins.	
Rigid ceiling directly applied or 10-0-0 oc bracing.	

REACTIONS (lb/size) 4=268/Mechanical, 2=537/0-6-7, 5=374/Mechanical

Max Horiz 2=318(load case 4)
Max Uplift4=-248(load case 4), 2=-264(load case 4), 5=-70(load case 4)

Category	Value
WEBS	3-7=0/178, 3-6=-737/1301
BOT CHORD	2-7=-286/699, 6-7=-286/699, 5-6=0/0
TOP CHORD	1-2=0/56, 2-3=-775/58, 3-4=-126/75
JOINT	1-2=0/56, 2-3=-775/58, 3-4=-126/75

NOTES
(1) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate and DOL=1.60

(2) Refer to girder(s) for truss to truss connections.
(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 4, 264 lb uplift at joint 2 and 70 lb uplift at joint 5.

(t) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 Standard
 (1) Regular: Lumber increase=1.25, Plate increase=1.25
 LOAD CASE(S)

Uniform Loads (psi)
Vert. 1-2=54
Trapezoidal Loads (psi)
A-B=54 to 124.5
C-D=124.5 to 195
E-F=195 to 265.5
G-H=265.5 to 336
I-J=336 to 406.5
K-L=406.5 to 477
M-N=477 to 547.5
O-P=547.5 to 618
Q-R=618 to 688.5
S-T=688.5 to 759
U-V=759 to 829.5
W-X=829.5 to 900
Y-Z=900 to 970.5
AA-AB=970.5 to 1041
AC-AD=1041 to 1111.5
AE-AF=1111.5 to 1182
AG-AH=1182 to 1252.5
AI-AJ=1252.5 to 1323
AK-AL=1323 to 1393.5
AM-AN=1393.5 to 1464
AO-AP=1464 to 1534.5
AQ-AR=1534.5 to 1605
AS-AT=1605 to 1675.5
AU-AV=1675.5 to 1746
AW-AX=1746 to 1816.5
AY-AZ=1816.5 to 1887
BA-BB=1887 to 1957.5
BC-BD=1957.5 to 2028
BE-BF=2028 to 2098.5
BG-BH=2098.5 to 2169
BI-BJ=2169 to 2239.5
BK-BL=2239.5 to 2310
BM-BN=2310 to 2380.5
BO-BP=2380.5 to 2451
BQ-BR=2451 to 2521.5
BS-BT=2521.5 to 2592
BU-BV=2592 to 2662.5
BW-BX=2662.5 to 2733
BY-BZ=2733 to 2803.5
CA-CB=2803.5 to 2874
CC-CD=2874 to 2944.5
CE-CF=2944.5 to 3015
CG-CH=3015 to 3085.5
CI-CJ=3085.5 to 3156
CK-CL=3156 to 3226.5
CM-CN=3226.5 to 3297
CO-CP=3297 to 3367.5
CQ-CR=3367.5 to 3438
CS-CT=3438 to 3508.5
CU-CV=3508.5 to 3579
CW-CX=3579 to 3649.5
CY-CZ=3649.5 to 3720
DA-DB=3720 to 3790.5
DC-DD=3790.5 to 3861
DE-DF=3861 to 3931.5
DG-DH=3931.5 to 4002
DI-DJ=4002 to 4072.5
DK-DL=4072.5 to 4143
DM-DN=4143 to 4213.5
DO-DP=4213.5 to 4284
DQ-DR=4284 to 4354.5
DS-DT=4354.5 to 4425
DU-DV=4425 to 4495.5
DW-DX=4495.5 to 4566
DY-DZ=4566 to 4636.5
EA-EB=4636.5 to 4707
EC-ED=4707 to 4777.5
EE-EF=4777.5 to 4848
EG-EH=4848 to 4918.5
EI-EJ=4918.5 to 4989
EK-EL=4989 to 5059.5
EM-EN=5059.5 to 5130
EO-EP=5130 to 5200.5
EQ-ER=5200.5 to 5271
ES-ET=5271 to 5341.5
EU-EV=5341.5 to 5412
EW-EX=5412 to 5482.5
EY-EZ=5482.5 to 5553
FA-FB=5553 to 5623.5
FC-FD=5623.5 to 5694
FE-FG=5694 to 5764.5
FH-FI=5764.5 to 5835
FI-FJ=5835 to 5905.5
FK-FL=5905.5 to 5976
FM-FN=5976 to 6046.5
FO-FP=6046.5 to 6117
FQ-FR=6117 to 6187.5
FS-FT=6187.5 to 6258
FU-FV=6258 to 6328.5
FW-FX=6328.5 to 6400
FY-FZ=6400 to 6470.5
GA-GB=6470.5 to 6541
GC-GD=6541 to 6611.5
GE-GF=6611.5 to 6682
GG-GH=6682 to 6752.5
GI-GJ=6752.5 to 6823
GK-GL=6823 to 6893.5
GM-GN=6893.5 to 6964
GO-GP=6964 to 7034.5
GQ-GR=7034.5 to 7105
GS-GT=7105 to 7175.5
GU-GV=7175.5 to 7246
GW-GX=7246 to 7316.5
GY-GZ=7316.5 to 7387
HA-HB=7387 to 7457.5
HC-HD=7457.5 to 7528
HE-HF=7528 to 7598.5
HG-HI=7598.5 to 7669
HI-HJ=7669 to 7739.5
HK-HL=7739.5 to 7810
HM-HN=7810 to 7880.5
HO-HP=7880.5 to 7951
HQ-HR=7951 to 8021.5
HS-HT=8021.5 to 8092
HU-HV=8092 to 8162.5
HW-HX=8162.5 to 8233
HY-HZ=8233 to 8303.5
IA-IB=8303.5 to 8374
IC-ID=8374 to 8444.5
IE-IF=8444.5 to 8515
IG-IH=8515 to 8585.5
II-IL=8585.5 to 8656
IM-IN=8656 to 8726.5
IO-IP=8726.5 to 8797
IQ-IR=8797 to 8867.5
IS-IT=8867.5 to 8938
IU-IV=8938 to 9008.5
IW-IX=9008.5 to 9079
IY-IZ=9079 to 9149.5
JA-JB=9149.5 to 9220
JC-JD=9220 to 9290.5
JE-JF=9290.5 to 9361
JG-JH=9361 to 9431.5
JI-JJ=9431.5 to 9502
JK-JL=9502 to 9572.5
JM-JN=9572.5 to 9643
JO-JP=9643 to 9713.5
JQ-JR=9713.5 to 9784
JS-JT=9784 to 9854.5
JU-JV=9854.5 to 9925
JW-JX=9925 to 9995.5
JY-JZ=9995.5 to 10066
KA-KB=10066 to 10136.5
KC-KD=10136.5 to 10207
KE-KF=10207 to 10277.5
KG-KH=10277.5 to 10348
KI-KJ=10348 to 10418.5
KK-KL=10418.5 to 10489
KM-KN=10489 to 10559.5
KO-KP=10559.5 to 10630
KQ-KR=10630 to 10700.5
KS-KT=10700.5 to 10771
KU-KV=10771 to 10841.5
KW-KX=10841.5 to 10912
KY-KZ=10912 to 10982.5
LA-LB=10982.5 to 11053
LC-LD=11053 to 11123.5
LE-LF=11123.5 to 11194
LG-LH=11194 to 11264.5
LI-LJ=11264.5 to 11335
LK-LL=11335 to 11405.5
LM-LN=11405.5 to 11476
LO-LP=11476 to 11546.5
LQ-LR=11546.5 to 11617
LS-LT=11617 to 11687.5
LU-LV=11687.5 to 11758
LW-LX=11758 to 11828.5
LY-LZ=11828.5 to 11900
MA-MB=11900 to 11970.5
MC-MD=11970.5 to 12041
ME-MF=12041 to 12111.5
MG-MH=12111.5 to 12182
MI-MJ=12182 to 12252.5
MK-ML=12252.5 to 12323
MN-MN=12323 to 12393.5
MO-MP=12393.5 to 12464
MQ-MR=12464 to 12534.5
MS-MT=12534.5 to 12605
MU-MV=12605 to 12675.5
MW-MX=12675.5 to 12746
MY-MZ=12746 to 12816.5
NA-NB=12816.5 to 12887
NC-ND=12887 to 12957.5
NE-NF=12957.5 to 13028
NG-NH=13028 to 13098.5
NI-NJ=13098.5 to 13169
NK-NL=13169 to 13239.5
NM-NN=13239.5 to 13310
NO-NP=13310 to 13380.5
NQ-NR=13380.5 to 13451
NS-NT=13451 to 13521.5
NU-NV=13521.5 to 13592
NW-NX=13592 to 13662.5
NY-NZ=13662.5 to 13733
OA-OB=13733 to 13803.5
OC-OD=13803.5 to 13874
OE-OF=13874 to 13944.5
OG-OH=13944.5 to 14015
OI-OJ=14015 to 14085.5
OK-OL=14085.5 to 14156
OM-ON=14156 to 14226.5
OO-OP=14226.5 to 14297
OQ-OR=14297 to 14367.5
OS-OT=14367.5 to 14438
OU-OV=14438 to 14508.5
OW-OX=14508.5 to 14579
OY-OZ=14579 to 14649.5
PA-PB=14649.5 to 14720
PC-PD=14720 to 14790.5
PE-PF=14790.5 to 14861
PG-PH=14861 to 14931.5
PI-PJ=14931.5 to 15002
PK-PL=15002 to 15072.5
PM-PN=15072.5 to 15143
PO-PP=15143 to 15213.5
PQ-PR=15213.5 to 15284
PS-PT=15284 to 15354.5
PU-PV=15354.5 to 15425
PW-PX=15425 to 15495.

Veri: $z=4(f-z_5)$, $D=z_5$, $10\text{-}4=-134(f=40)$, $B=40$, $Z=0(f=15)$, $B=15$, $10\text{-}5=-14(f=22)$, $B=22$

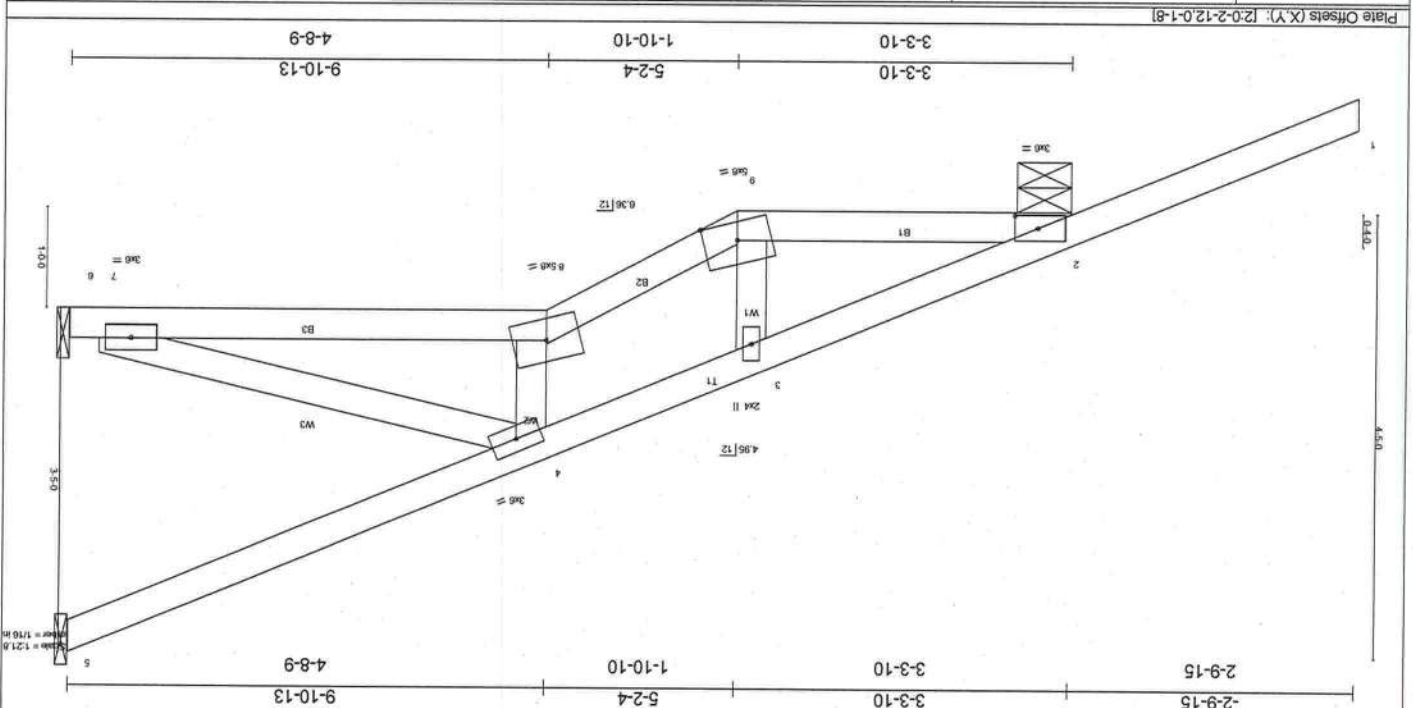
OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 56872, RYRON K. ANDERSON PE 60987

STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#101205540

Job Reference (optional)

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LOADING	SPACING	CSF	DEFL	PLATES	Weight:
(psi)	2-0-0	(Matrix)	in (loc)	L/d	lb
TCLL 20.0	Plates increase 1.25	TC 0.59	-0.15	>761	MT20
TCLL 7.0	Lumber increase 1.25	BC 0.66	7-8	>470	GRIP
BCLL 10.0	Rep Stress Incr NO	WB 0.30	0.10	n/a	244/190
BCLL 5.0	Code FBC2004/TP12002	Horz(TL) Vert(TL)	6	n/a	

LUMBER	2 X 4 SYP No.2	
BOT CHORD	2 X 4 SYP No.2	
WEBS	2 X 4 SYP No.3	
BRACING		
TOP CHORD		Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD		Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=263/Mechanical, 2=537/0-6-7, 6=379/Mechanical
Max Horiz 2=15(lb/case 4)
Max Uplift 5=-226(lb/case 4), 2=-264(lb/case 4), 6=-92(lb/case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

NOTES
1) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 5, 264 lb uplift at joint 2 and 92 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2-4=(F=25, B=25)+10-5=-134(F=-40, B=-40), 2-0(F=15, B=15)+10-9=-24(F=3, B=3), 9-24(F=3, B=3)+10-8=-38(F=-4, B=-4), 8-4(-6=-74(F=-22, B=-22), 4-10-6=-74(F=-22, B=-22)

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

L136166	PB15	PIGgyBACK		4	Qty	JOHN JOHNSON	Dwg.#101205541	Job Reference (optional)	6,200 s Jul 13 2005 Miller Industries, Inc. Mon Oct 10 18:55:23 2005 Page 1	Builders FirstSource, Lake City, Fl 32055
Job	Tuss	Tuss Type								

LOAD CASE(S) Standard

7) Wind: ASCE 7-02, 10mph (3-second gust); h=20ft, TCDF=4.2psf, BCLD=3.0psf; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. Surfact.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 1, 23 lb uplift at joint 7, 60 lb uplift at joint 9 and 198 lb uplift at joint 10 and 193 lb uplift at joint 8.

NOTES

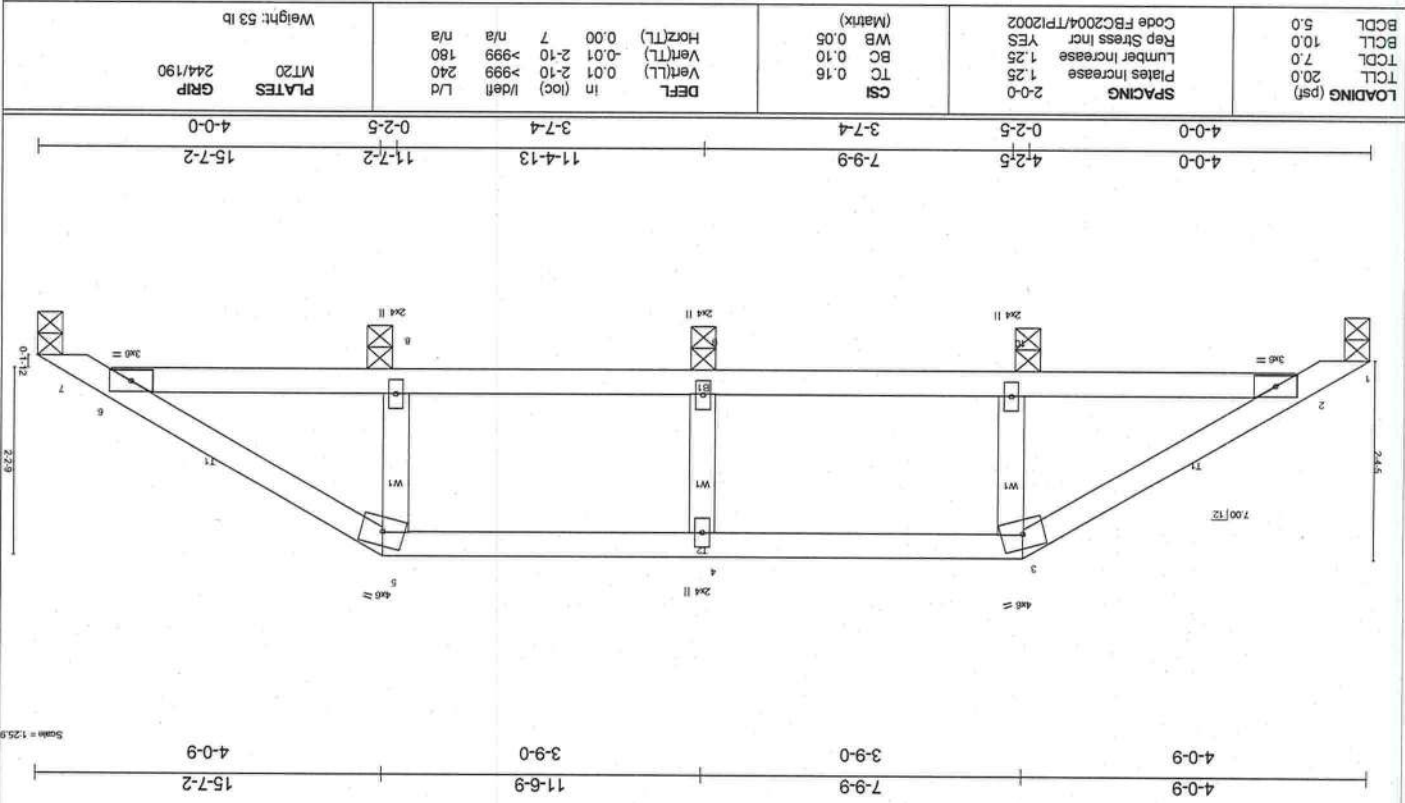
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=157/160, 2-3=126/157, 3-4=42/153, 4-5=6/153, 5-6=63/157, 6-7=34/15
BOT CHORD 2-1=87/100, 9-10=87/100, 8-9=87/100, 6-8=87/100
WEBS 4-9=309/90, 3-10=228/206, 5-8=228/203

REACTIONS

REACTIONS

COMBLES	TOP CHORD	2 X 4 SYP No.2
	TOP CHORD	2 X 4 SYP No.2
	WEBS	2 X 4 SYP No.3
BRACING	TOP CHORD	
	BOT CHORD	
Structural wood sheathing directly applied or 5-0-0 oc purlins.		
Rigid ceiling directly applied or 5-0-0 oc bracing.		

[illegible]



LOADING (psf)	SPACING	PLATES INCREASE	CS	DEFL	IN (LOC)	VERT (LL)	HORIZ (TL)	WEBS	TOP CHORD	BOT CHORD	BRACING
20.0	2-0-0	1.25	0.16	0.01	2-10	>999	180	n/a	Structural wood sheathing directly applied or 6-0-0 oc purlins.	Rigid ceiling directly applied or 6-0-0 oc bracing.	Weight: 53 lb
TCDL	7.0	1.25	0.10	0.01	2-10	>999	180	n/a			
TCDL	7.0	1.25	0.10	0.01	2-10	>999	180	n/a			
BCL	10.0	1.25	0.10	0.01	2-10	>999	180	n/a			
BCL	5.0	1.25	0.10	0.01	2-10	>999	180	n/a			

REACTIONS (lb/size) 1=60/0-3-8, 7=60/0-3-8, 10=423/0-3-8, 8=423/0-3-8, 9=293/0-3-8
 Max Horiz 1=79(load case 3)
 Max Uplift 1=20(load case 5), 7=37(load case 3), 10=146(load case 5), 8=133(load case 6), 9=170(load case 3)
 Max Grav 1=80(load case 9), 7=80(load case 10), 10=423(load case 1), 8=423(load case 1), 9=307(load case 9)

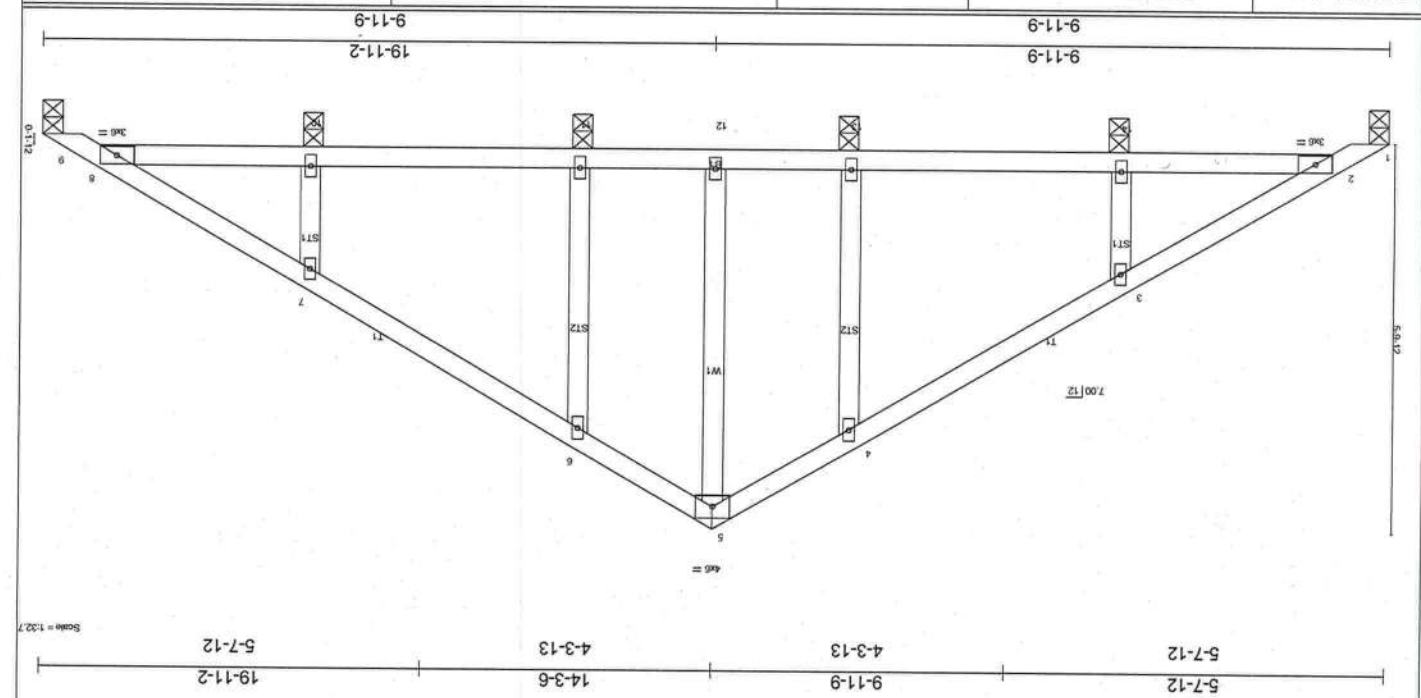
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=67/76, 2-3=-72/208, 5-6=-58/208, 6-7=-35/22, 3-4=-20/153, 4-5=-20/153
 BOT CHORD 2-10=-131/99, 9-10=-153/113, 8-9=-153/113, 6-8=-131/99
 WEBS 3-10=-291/177, 5-8=-291/177, 4-9=-208/191

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone: Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 7 considers parallel to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 37 lb uplift at joint 7, 146 lb uplift at joint 10, 133 lb uplift at joint 8 and 170 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	L136166	Truss type	PIGGYBACK	Qty	2	Job Reference (optional)	JOHN JOHNSON	Dwg.#101205544
	PB19			Ply	1			

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	Weight: 85 lb
TCLL 20.0	Plates Increase 1.25	TC 0.16	DEFL	in (loc)	L/d	PLATES GRIP
TCCL 7.0	Lumber Increase 1.25	BC 0.10	Ver(TL)	2-14 >999	180	MT20 244/190
BCCL 10.0	Rep Stress Incr YES	WB 0.07	Horz(TL)	9 n/a	n/a	
BCDL 5.0	Code FBC2004/TP12002					

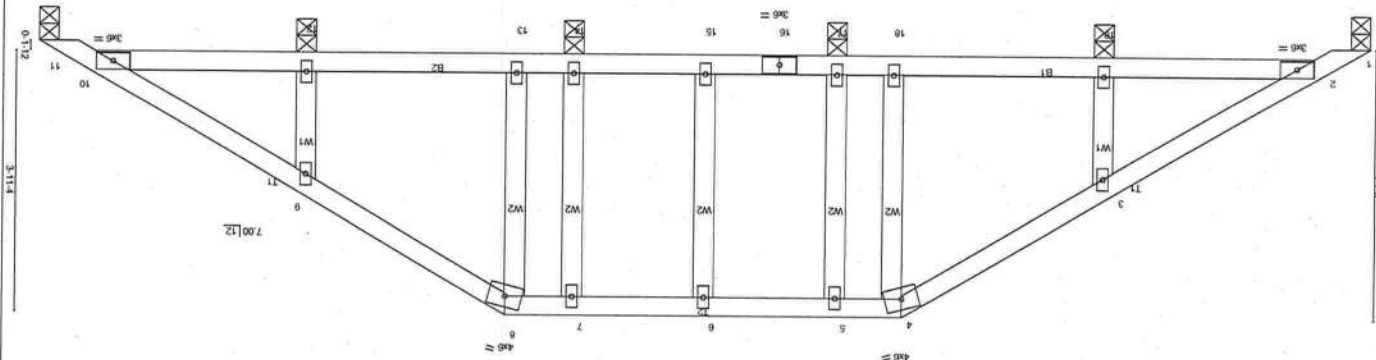
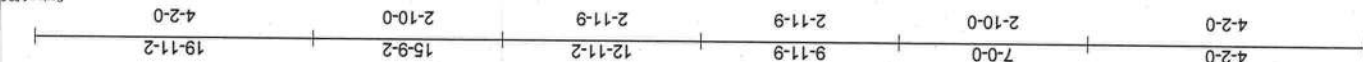
LUMBER	BRACING	OTHERS
TOP CHORD 2 X 4 SYP No.2	TOP CHORD	WEBS
BOT CHORD 2 X 4 SYP No.2	BOT CHORD	OTHERS
2 X 4 SYP No.3		2 X 4 SYP No.3

REACTIONS (lb/size) 1=110/0-3-8, 9=110/0-3-8, 13=335/0-3-8, 14=366/0-3-8, 10=366/0-3-8
 Max Horz 1=199(load case 3)
 Max Uplift=29(load case 5), 13=134(load case 5), 14=201(load case 5), 11=122(load case 6)
 Max Grav 1=110(load case 9), 9=110(load case 10), 13=335(load case 9), 10=370(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=206/195, 2-3=171/97, 3-4=86/81, 4-5=43/89, 5-6=43/84, 6-7=50/61, 7-8=145/67, 8-9=49/0
 BOT CHORD 2-14=26/173, 12-13=26/173, 11-12=26/173, 10-11=26/173, 8-10=26/173
 WEBS 5-12=30/5, 4-13=206/171, 3-14=229/213, 6-11=206/165, 7-10=229/210

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) All plates are 2x4 MT20 unless otherwise indicated.
 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 134 lb uplift at joint 13, 201 lb uplift at joint 14, 122 lb uplift at joint 11 and 195 lb uplift at joint 10.

LOAD CASES Standard
 OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)		SPACING		CSI		DEFL		BRACING		WEIGHT: 90 lb	
TCLL	20.0	Plates Increase	1.25	TC	0.15	in (loc)	L/d	15	>999	MT20	244/190
TCCL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	0.01	15	>999	GRIP	
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	1	n/a		
BCDL	5.0	Code FBC2004/TP2002									

LUMBER TOP CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
Rigid ceiling wood sheathing directly applied or 6-0-0 oc purlins.

REACTIONS (lb/size)

Max Uplift 11=139(load case 3)
Max Horiz 11=29(load case 5), 17=183(load case 5), 19=160(load case 3), 12=191(load case 6)
Max Grav 11=92(load case 10), 19=357(load case 9), 14=357(load case 10), 12=357(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 8-9=49/87, 9-10=122/123, 10-11=141/137, 11-12=41/116, 12-13=60/106, 13-14=60/106, 14-15=60/106, 15-16=60/106, 16-17=60/106, 17-18=60/106, 18-19=57/104, 19-20=57/104
WEBS 3-19=216/185, 4-18=66/49, 5-17=167/96, 6-16=40/59, 7-15=167/101, 8-14=66/58, 9-13=216/189

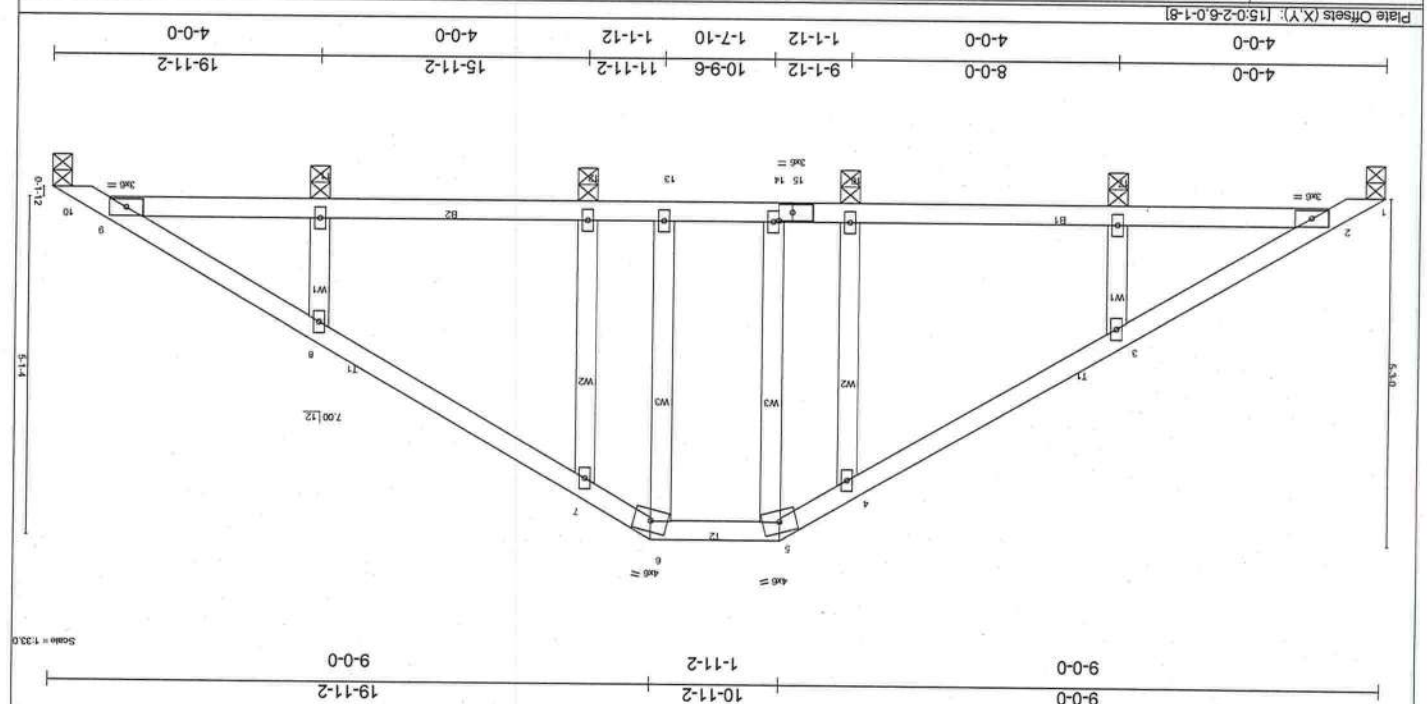
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Bearing at joint(s) 11, 1 considers parallel to grain value using ANSIT/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 11, 25 lb uplift at joint 1, 183 lb uplift at joint 19, 134 lb uplift at joint 17, 160 lb uplift at joint 14 and 191 lb uplift at joint 12.

LOAD CASE(S) Standard

Job	L136166	Truss	Truss Type	HIP CAP	Qty	1	Ply	JOHN JOHNSON	Dwg. #101205547
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LOADING (psf)	SPACING	CSI	DEFL	BRACING	PLATES	Weight: 91 lb
TCL 20.0	Plates Increase 1.25	TC 0.16	in (loc) 0.01	Structural wood sheathing directly applied or 6-0-0 oc purlins.	MT20	
TCOL 7.0	Lumber Increase 1.25	BC 0.10	Vert(TL) -0.01		GRIP	244/190
BCL 10.0	Rep Stress Incr YES	WB 0.07	Horiz(TL) -0.01			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)				

LUMBER	TOP CHORD	2 X 4 SYP No.2	WEBS	2 X 4 SYP No.3
REACTIONS	Max Horiz 10=-180(load case 3)			
	Max Uplift 10=-44(load case 4), 1=-8(load case 4), 17=-201(load case 5), 16=-100(load case 5), 12=-16(load case 6), 11=-205(load case 6)			
	Max Gray 10=109(load case 10), 1=109(load case 9), 17=367(load case 9), 16=350(load case 9), 12=350(load case 1), 11=367(load case 10)			
FORCES	(lb) - Maximum Compression/Maximum Tension			
	TOP CHORD 6-37/102, 7-8=40/122, 8-9=154/137, 9-10=-179/185, 1-2=-48/9, 2-3=-123/101, 3-4=40/95, 4-5=-37/106, 5-6=0/92			
	BOT CHORD 2-17=48/154, 16-17=48/154, 14-15=-48/154, 13-14=-47/153, 11-12=47/153, 9-11=-47/153			
	WEBS 6-13=-25/9, 5-14=-25/25, 3-17=-227/214, 4-16=-215/152, 7-12=-215/160, 8-11=-227/215			
NOTES				
	1) Unbalanced roof live loads have been considered for this design.			
	2) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft, TCCL=4.2psf, BCCL=3.0psf, Category II: Exp B; enclosed; MWFRS gable end zone and C-C			
	3) Provide adequate drainage to prevent water ponding.			
	4) All plates are 2x4 MT20 unless otherwise indicated.			
	5) Bearing at joint(s) 10, 1 considers parallel to grain value using ANSIT/TP1 angle to grain formula. Building designer should verify capacity of bearing surface.			
	6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 10, 8 lb uplift at joint 1, 201 lb uplift at joint 17, 100 lb uplift at joint 16, 116 lb uplift at joint 12 and 205 lb uplift at joint 11.			
LOAD CASE(S) Standard				



OTHERS	2 X 4 SYP No.3
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Max Horz 1 = 155 (load case 3)
Max |n|/|t| = 371 (load case 5) 11 = 193 (load case 6) 14 = 160 (load case 5) 15 = 154 (load case 5) 13 = 314 (load case 5)

TOP CHORD 1-2=-139/145, 2-3=-57/37, 3-4=-43/33, 4-5=-43/33, 5-6=-45/79, 6-7=-46/94, 7-8=-26/106, 8-9=-107/101, 9-10=-46/22

1) Unbalanced roof live loads have been considered for this design.

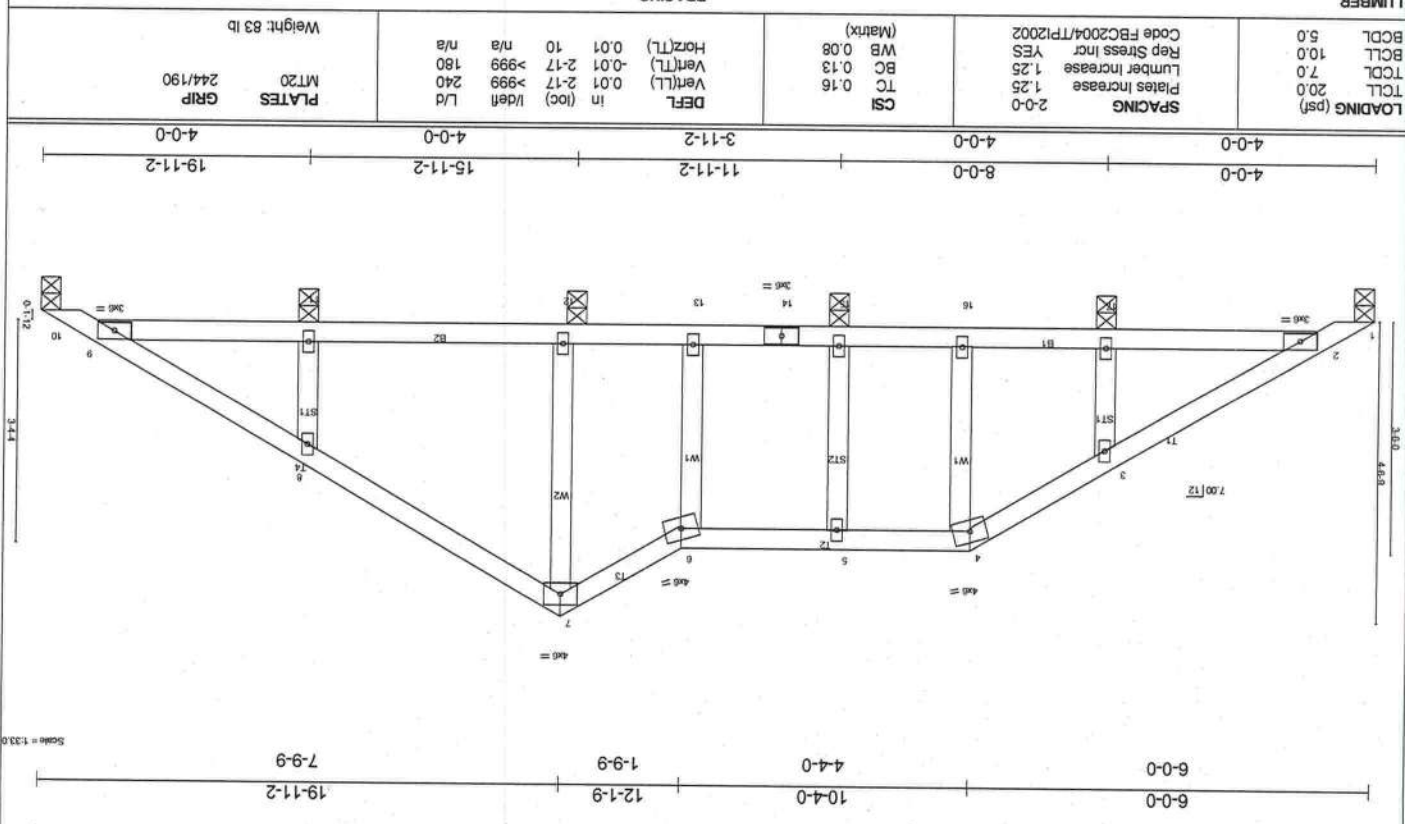
4) All plates are 2x4 M120 unless otherwise indicated.
5) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSIT/PI 1 angle to grain formula. Building designer should verify capacity of bearing.

LOAD CASE(S) Standard

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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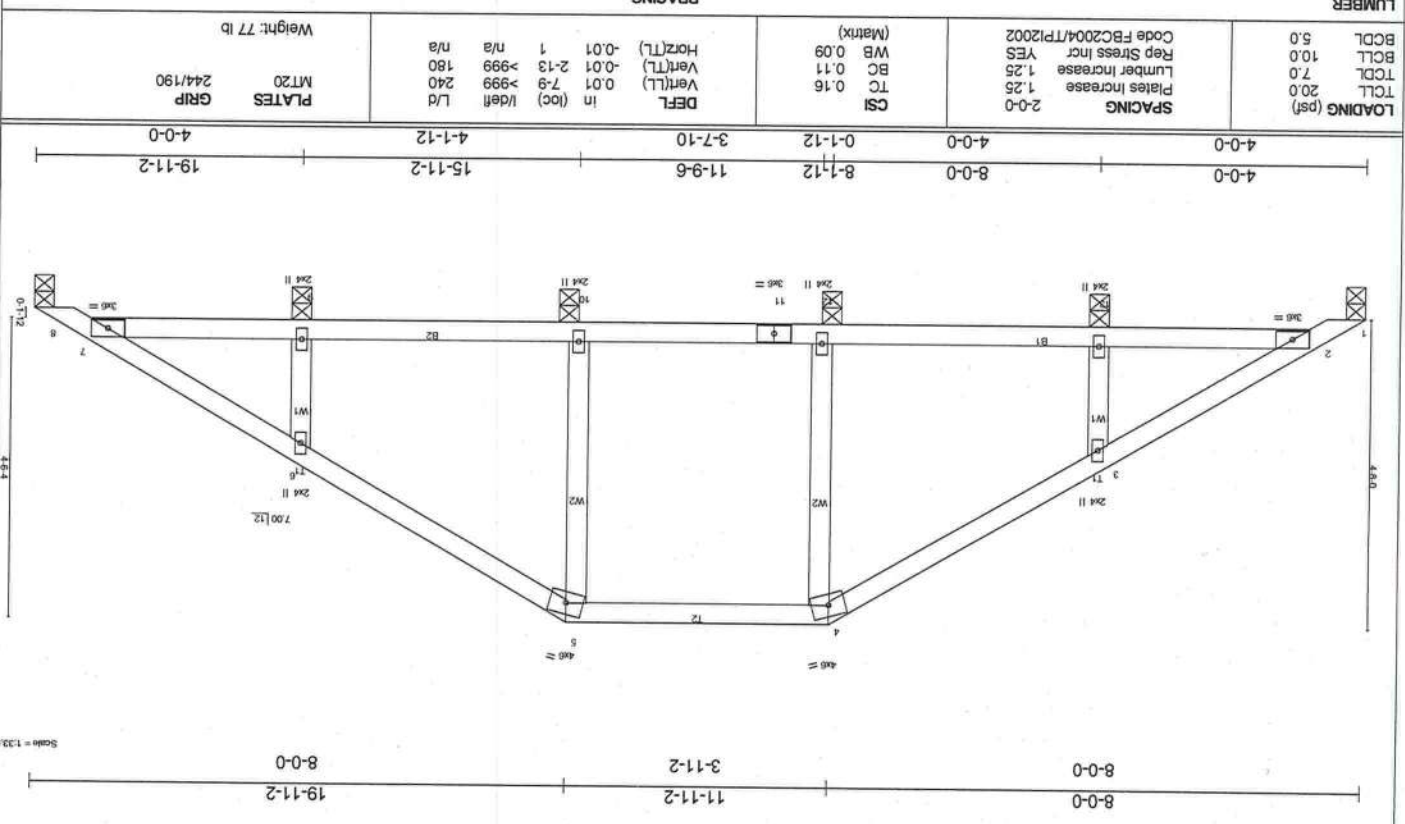
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L136166	Truss	PB19F	VALLEY	1	Qty	JOHN JOHNSON	Dwg.#1010205550
Truss Type	VALLEY	1	Qty	JOHN JOHNSON	1	Ply	JOHN JOHNSON	Dwg.#1010205550
Job Reference (optional)	6:200 S JUL 13 2005 Mitek Industries, Inc. Tue Oct 11 17:22:37 2005 Page 1							



LOADING (psf)	SPACING	CSI	DEFL	IN (in)	VERT (in)	HORIZ (in)	TOP CHORD	BOT CHORD	BRACING
LUMBER	2 X 4 SYP No.2	2.0-0	0.16	0.01	0.01	0.01	Structural wood sheathing directly applied or 6-0-0 oc purlins.	Structural wood sheathing directly applied or 6-0-0 oc purlins.	Weight 83 lb
WEBS	2 X 4 SYP No.3	0.08	0.13	0.01	0.01	0.01			
OTHERS	2 X 4 SYP No.3	0.08	0.13	0.01	0.01	0.01			

REACTIONS	FORCES	NOTES	LOAD CASE(S)
(lb/size) 1=82/0-3-8, 10=84/0-3-8, 11=356/0-3-8, 17=380/0-3-8, 15=348/0-3-8, 12=372/0-3-8	Max Horiz 1=155(load case 3) Max Uplift 1=23(load case 6), 10=144(load case 6), 11=186(load case 6), 17=184(load case 6), 15=145(load case 5), 12=83(load case 5) Max Grav 1=83(load case 9), 10=96(load case 10), 11=359(load case 10), 17=380(load case 10), 15=356(load case 9), 12=372(load case 1)	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 10mph (3-second gust): h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C 3) Provide adequate drainage to prevent water ponding. 4) All plates are 2x4 MT20 unless otherwise indicated. 5) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 14 lb uplift at joint 10, 186 lb uplift at joint 11, 184 lb uplift at joint 17, 145 lb uplift at joint 15 and 83 lb uplift at joint 12.	Standard



LOADING (psf)	SPACING	CS	DEFL	IN (loc)	IDEFL	L/D	PLATES	GRIP	WEIGHT: 77 lb
TCLL 20.0	Plates Increase 1.25	TC 0.16	Vert(TL) -0.01	1	n/a	240	MT20	244/190	
TCCL 7.0	Lumber Increase 1.25	BC 0.11	Horz(TL) -0.01	2-13	>999	180			
BCLL 10.0	Rep Stress Incr YES	WB 0.09		n/a	n/a	n/a			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)							

REACTIONS (lb/size) 8=76/0-3-8, 1=76/0-3-8, 10=374/0-3-8, 12=374/0-3-8, 13=362/0-3-8, 9=362/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 5-6=47/151, 6-7=123/154, 7-8=160/165, 1-2=38/16, 2-3=62/154, 3-4=8/151, 4-5=1/109

BOT CHORD 2-13=84/100, 11-12=93/103, 10-11=93/103, 9-10=84/102, 7-9=84/102

WEBS 5-10=262/169, 4-12=262/119, 3-13=221/214, 6-9=221/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C

3) Provide adequate drainage to prevent water ponding.

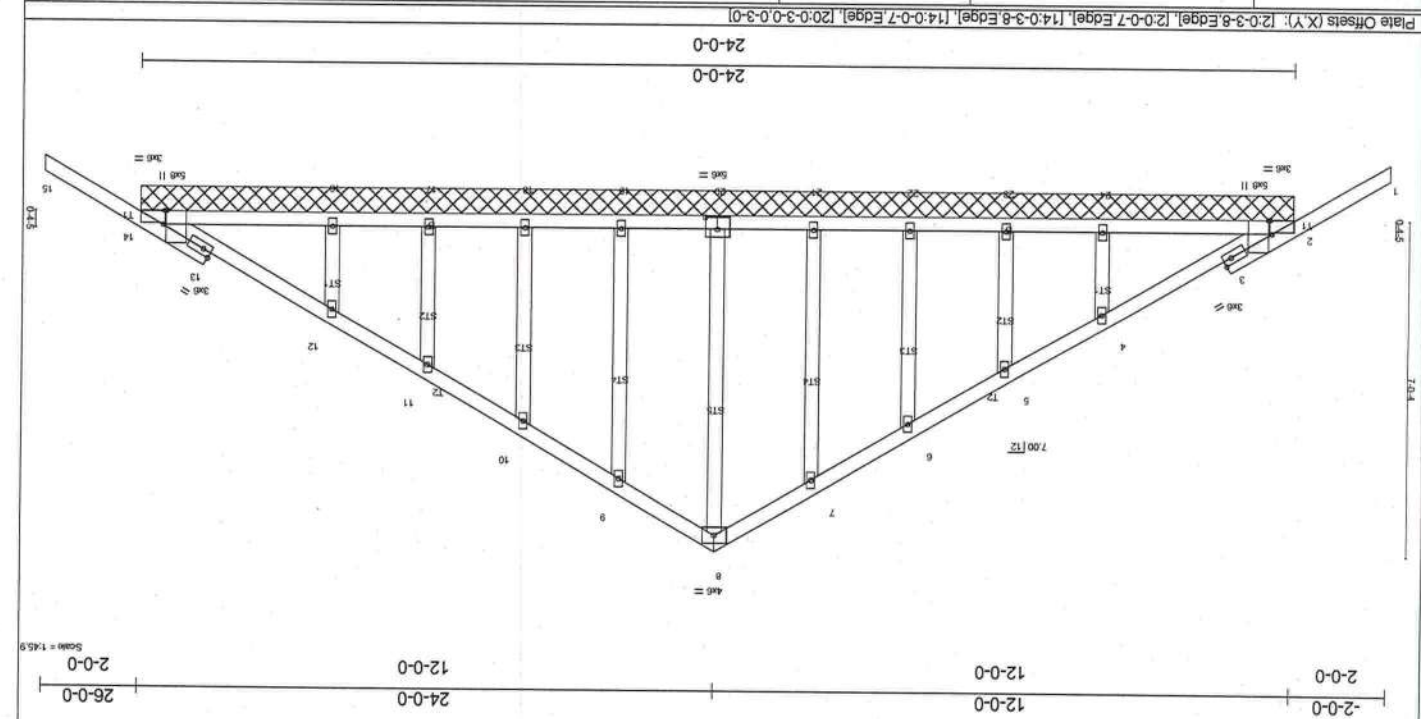
4) Bearing at joint(s) 8, 1 considers parallel to grain value using ANSIT/TP 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 8, 24 lb uplift at joint 1, 142 lb uplift at joint 10, 93 lb uplift at joint 12, 202 lb uplift at joint 13 and 206 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	L136166	Truss	T01G	COMMON	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205553
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Builders FirstSource, Lake City, FL 32055
 6:200 s Jul 13 2005 Mitek Industries, Inc. Mon Oct 10 18:55:31 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	VERT(TL)	HORIZ(TL)	BRACING	OTHERS	LUMBER
TCLL 20.0	Plates increase 1.25	TC 0.50	in (loc)	-0.03	0.01	TOP CHORD	2 X 4 SYP No.2	TOP CHORD 2 X 4 SYP No.2
TCCL 7.0	Lumber increase 1.25	BC 0.08	15	-0.05	14	Structural wood sheathing directly applied or 6'-0\"	2 X 4 SYP No.2	BOT CHORD 2 X 4 SYP No.2
BCLL 10.0	Rep Stress Incr NO	WB 0.21	n/r	15	n/a	Rigid ceiling directly applied or 6'-0\"	OTHERS	
BCCL 5.0	Code FBC2004/TP12002	(Matrix)	L/d	n/a	n/a	Weight: 139 lb		

REACTIONS	FORCES	WEBS	NOTES
(lb/size) 2=525/24-0-0, 14=525/24-0-0, 20=319/24-0-0, 21=287/24-0-0, 22=303/24-0-0, 23=221/24-0-0, 24=460/24-0-0, 18=287/24-0-0, 19=287/24-0-0, 17=221/24-0-0, 16=460/24-0-0, 15=291/24-0-0, 14=256/24-0-0, 13=256/24-0-0, 12=256/24-0-0, 11=256/24-0-0, 10=256/24-0-0, 9=256/24-0-0, 8=256/24-0-0, 7=256/24-0-0, 6=256/24-0-0, 5=256/24-0-0, 4=256/24-0-0, 3=256/24-0-0, 2=256/24-0-0, 1=256/24-0-0	Max Horiz Z=239(load case 3) Max Uplift Z=253(load case 6), 14=275(load case 6), 21=133(load case 5), 22=144(load case 5), 23=136(load case 5), 24=159(load case 5), 19=130(load case 6), 18=146(load case 6), 17=221/24-0-0, 16=460/24-0-0, 15=291/24-0-0, 14=256/24-0-0, 13=256/24-0-0, 12=256/24-0-0, 11=256/24-0-0, 10=256/24-0-0, 9=256/24-0-0, 8=256/24-0-0, 7=256/24-0-0, 6=256/24-0-0, 5=256/24-0-0, 4=256/24-0-0, 3=256/24-0-0, 2=256/24-0-0, 1=256/24-0-0	8-20=256/13, 7-21=233/144, 6-22=237/158, 5-23=184/141, 4-24=347/187, 9-19=233/141, 10-18=237/159, 11-17=184/138, 12-16=347/192	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mitek "Standard Gable End Detail". 4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2'-0-0 oc. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2, 275 lb uplift at joint 14, 133 lb uplift at joint 21, 144 lb uplift at joint 22, 136 lb uplift at joint 23, 159 lb uplift at joint 24, 130 lb uplift at joint 19, 146 lb uplift at joint 18, 133 lb uplift at joint 17 and 166 lb uplift at joint 16. 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)	Standard	Uniform Loads (plf)	Notes
1) Regular Lumber Increase=1.25		Vert 1-8=-114(F=60), 8-15=-114(F=60), 2-14=-30	

Dwg.#101205554

Job Reference (optional)

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LOADING (psf)	20.0 7.0 10.0 5.0	SPACING	2-0-0	Code FBCC2004/TP12002	(Matrix)	CS I	0.26	TC	1.25	Lumber increase	1.25	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.05	6	n/a	Weight: 120 lb
TCLL	20.0	Plates Increase	1.25			DEF L	In (loc)	-0.09	240					BC	0.44	Ver(TL)	-0.14	180	M/T20	
TCDL	7.0							-0.09	240					BC	0.44	Ver(TL)	-0.14	180	GRIP	244/190
BCLL	10.0							-0.09	240					BC	0.44	Ver(TL)	-0.14	180	PLATES	
BCLD	5.0							-0.09	240					BC	0.44	Ver(TL)	-0.14	180		

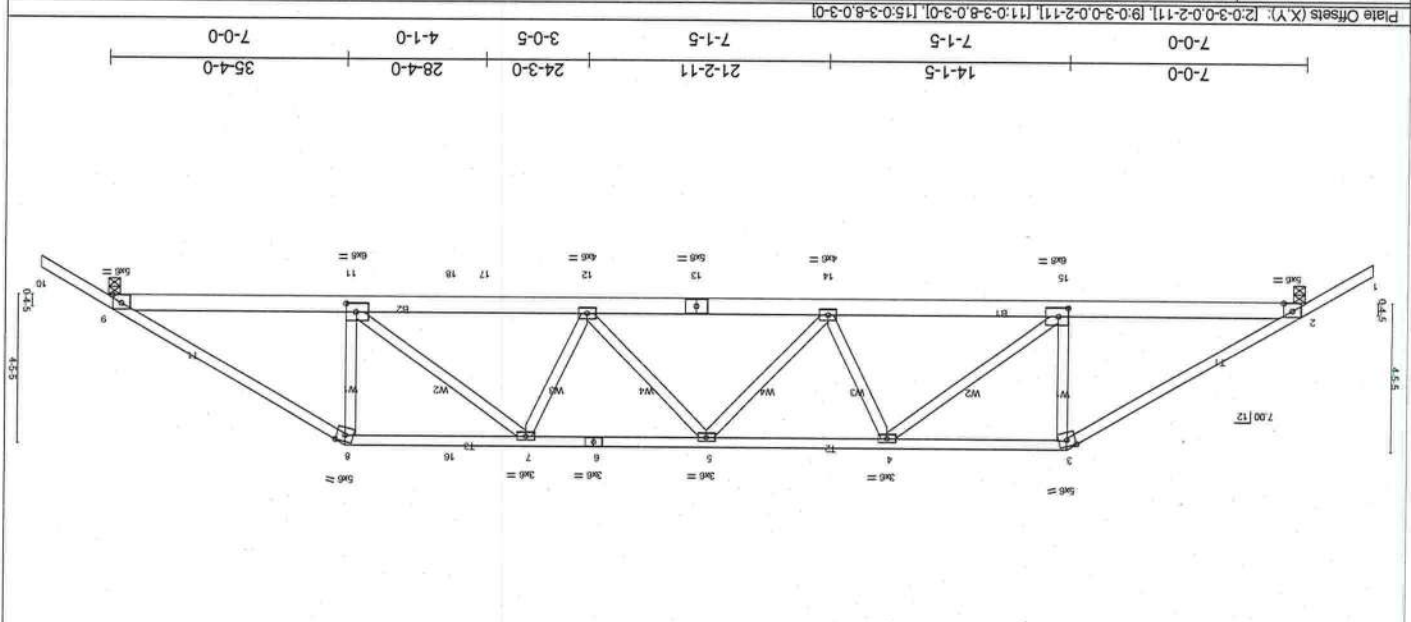
Structural wood sheathing directly applied or 4-8-9 oc purlins, Rigid ceiling directly applied or 9-11-3 oc bracing.

REACTIONS (lb/size) 6=990/Mechanical, 2=1117/0-4-0
Max Horz 2=270(load case 4)
Max Up/lift 6=-317(load case 6), 2=-448(load case 5)

FORCES (b) - Maximum Compression: Tension
TOP CHORD 1.2-0/54, 2.3-=-1558/505, 3.4-=-1073/445, 4.5-=-1074/447, 5.6-=-1584/539
BOT CHORD 2.9-=-383/1263, 8.9-=-383/1263, 7.8-=-374/1292, 6.7-=-374/1292
WEBS 3.9-0/193, 3.8-=-499/265, 4.8-=-239/698, 5.8-=-534/309, 5.7-0/210

(1) Unbalanced roof live loads have been considered for this design.
(2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20ft$; TCDFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate gnp DCL=1.60, This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
(3) Refer to girders(s) for truss to truss connections.
(4) Provide mechanical connections (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 6 and 448 lb uplift at joint 2.

LOAD CASE(S) Standard



Dwg. #101205556

JOHN JOHNSON

Ply

Qty

1

1

HIP

Truss Type

T04

L136166

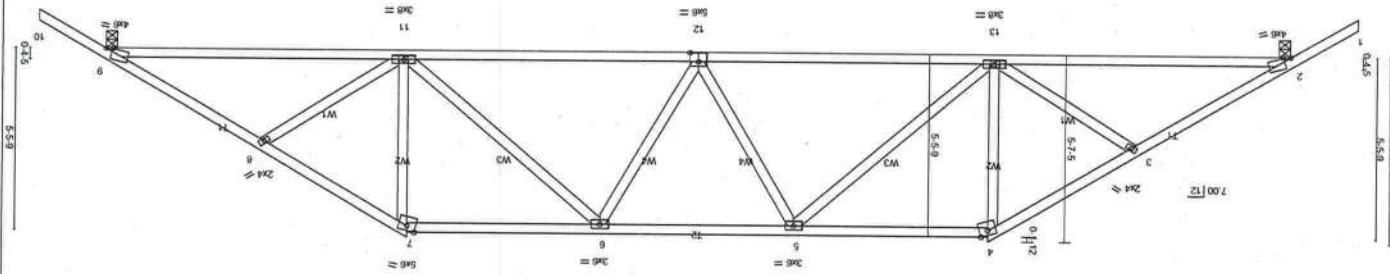
Builders FirstSource, Lake City, Fl 32055

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2-0-0	4-8-13	9-0-0	14-9-5	20-6-11	26-4-0	30-7-3	35-4-0	37-4-0
2-0-0	4-8-13	4-3-3	5-9-5	5-9-5	5-9-5	4-3-3	4-8-13	2-0-0

Scale = 1/8" = 1'-0"

Center = 1/8" = 1'-0"



9-0-0	17-8-0	26-4-0	35-4-0	9-0-0
9-0-0	8-8-0	8-8-0	9-0-0	9-0-0

Plate Offsets (X,Y): [2-0-2-6-0-0-11], [9-0-2-6-0-0-11], [12-0-3-0-0-3-4]

LOADING (psf)	SPACING	CS	DEFL	BRACING	RECTIONS	FORCES	NOTES
BCDL 5.0	2-0-0	CS 0.28	Vert(L) -0.24	TOP CHORD	Max Uplift=537 (load case 5), 9=537 (load case 6)	Max Horiz=188 (load case 4)	(1) Unbalanced roof live loads have been considered for this design.
BCLL 10.0		WB 0.85	Vert(TL) -0.40	BOT CHORD			(2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C.
TCDL 7.0		BC 0.67	Horz(TL) 0.13				(3) Provide adequate drainage to prevent water ponding.
TCLL 20.0			In (loc) 9				(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2 and 537 lb uplift at joint 9.
Plates Increase 1.25			L/d 240				
Lumber Increase 1.25			>999				
Rep Stress Incr YES			n/a				
Code FBC2004/TP12002			n/a				

LUMBER 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
Structural wood sheathing directly applied or 3-10-8 oc purlins.
Rigid ceiling directly applied or 6-7-3 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=2416/892, 3-4=2238/846, 4-5=1878/80, 5-6=2482/1004, 6-7=1878/80, 7-8=2238/846, 8-9=2416/892, 9-10=0/54
BOT CHORD 2-13=-772/2022, 12-13=-924/2409, 9-11=-868/2409, 9-11=-805/2022
WEBS 3-13=-149/179, 4-13=-223/817, 5-13=-777/470, 5-12=-28/194, 6-12=-28/194, 6-11=-777/470, 7-11=-223/817, 8-11=-149/179

NOTES
(1) Unbalanced roof live loads have been considered for this design.
(2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C.
(3) Provide adequate drainage to prevent water ponding.
(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2 and 537 lb uplift at joint 9.

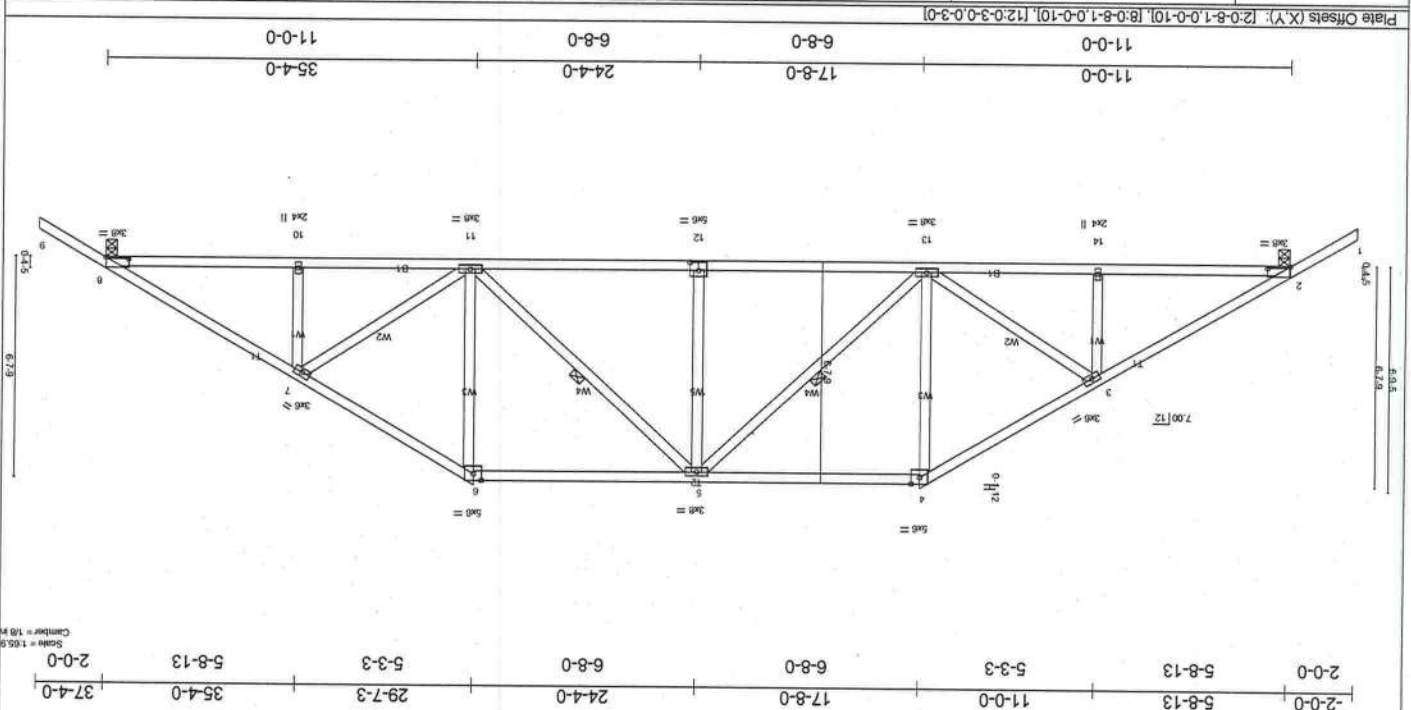
LOAD CASE(S) Standard

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	T05	HIP	City	JOHN JOHNSON	Dwg.#101205557
Builders FirstSource, Lake City, FL 32055						
Job Reference (optional)						

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	TCLL 20.0	TCDL 7.0	BCLL 10.0	BCDL 5.0
	SPACING			
	2-0-0	Plates increase 1.25	Lumber increase 1.25	Rep Stress Incr Code FBC2004/TP12002
	CSI	TC 0.30	BC 0.50	WB 0.24
	(Matrx)			
		Vert(LL) >99	Vert(TL) >99	Horz(TL) 0.12
		in (loc)	11-12	8
		I/dell	L/d	n/a
		MT20	PLATES	GRIP
		244/190	Weight: 198 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied or 3-9-15 oc purlins. Rigid ceiling directly applied or 7-8-2 oc bracing. 1 Row at midptl 5-13, 5-11
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REACTIONS (lb/size) 2=1587/0-4.0, 8=1587/0-4.0
Max Horiz 2=-228(load case 3)
Max Uplift 2=-555(load case 5), 8=-555(load case 6)

TOP CHORD
BOT CHORD
WEBS

NOTES

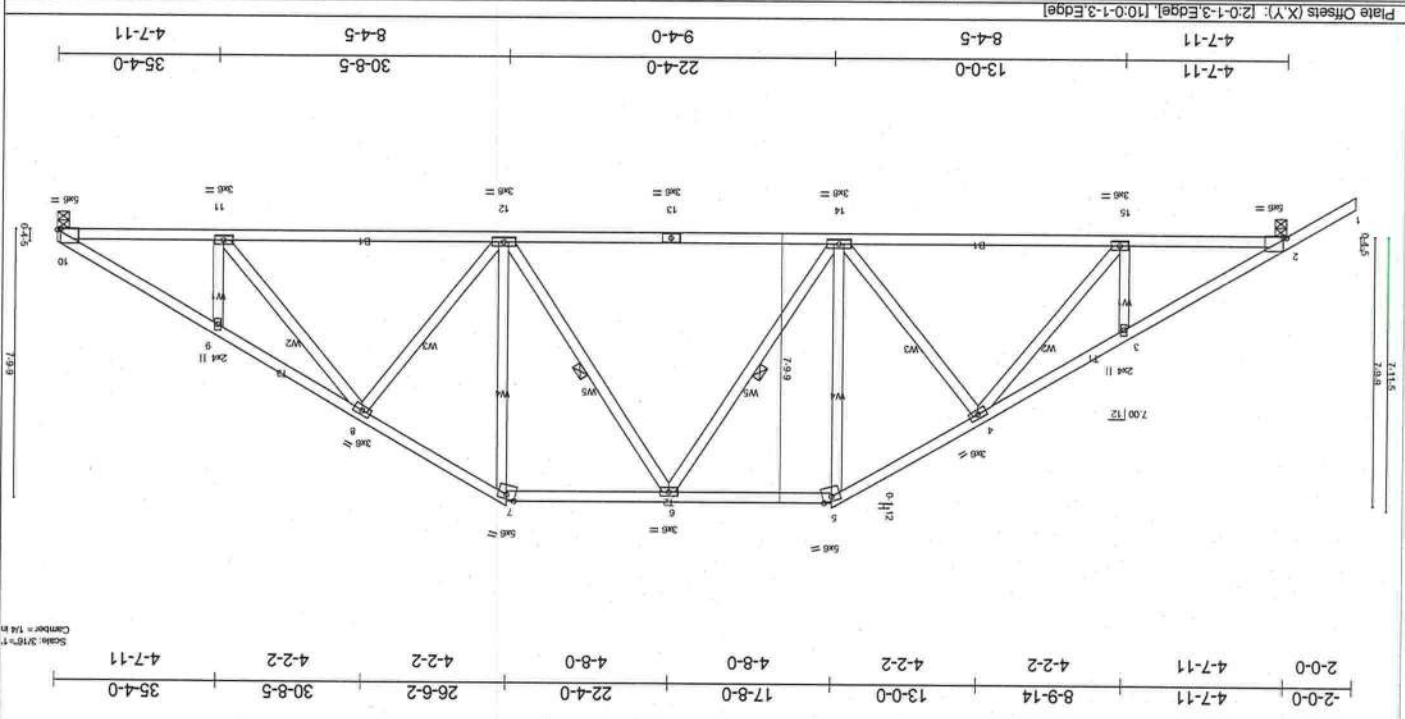
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 11 mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $B/CDL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS gable end zone and C-C Exterior (2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 555 lb uplift at joint 8.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

Job	L136166	Truss	T06	HIP	1	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205558
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Builders FirstSource, Lake City, FL 32055
 6.200 s Jul 13 2005 Mike Industries, Inc. Tue Oct 11 17:35:15 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	BRACING	WEBS	TOP CHORD	BOT CHORD	WEBS	1 Row at midpt	6-14, 6-12	Weight 207 lb
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(L) -0.37	In (loc) 11-12	180	0.11	10	n/a	n/a	L/D	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.88	Vert(TL) -0.60	11-12	>697	0.60	11-12	n/a	n/a	L/D	GRIP 244/190
BCLL 10.0	Rep Stress Incr NO	WB 0.35	Horz(TL) 0.11	10	n/a	0.11	10	n/a	n/a	L/D	
BCDL 5.0	Code FBC2004/TP12002	(Matrix)									

LUMBER
 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.10
 WEBS 2 X 4 SYP No.3

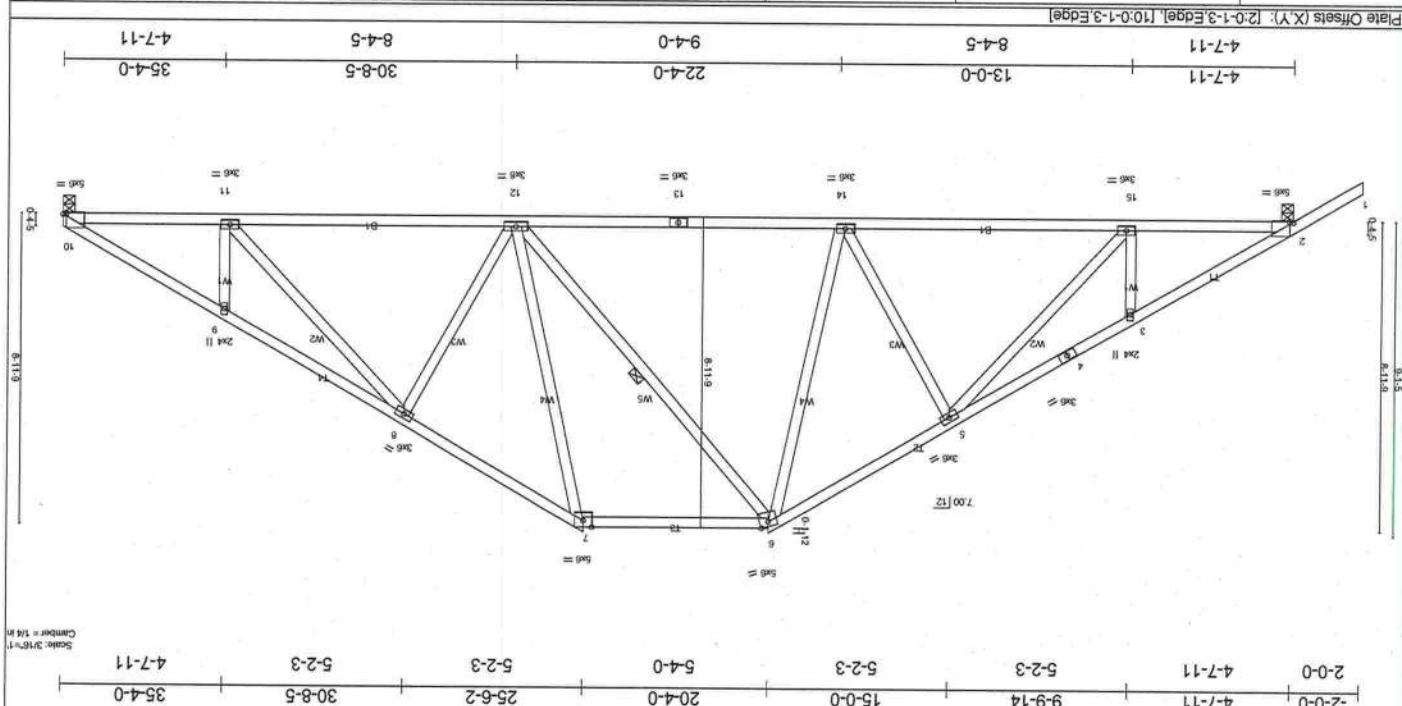
REACTIONS (lb/size)
 10=1776/0-4-0, 2=1691/0-4-0
 Max Horz 2=289/load case 4)
 Max Uplift 10=561(load case 6), 2=610/load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=2724/916, 3-4=2671/1023, 4-5=-2104/868, 5-6=-1770/792, 6-7=-1928/858, 7-8=-2289/944, 8-9=-3194/1281, 9-10=-3255/1168
 BOT CHORD 2-15=-705/2261, 14-15=-624/1986, 13-14=-574/1942, 12-13=-715/2223, 10-11=-926/2713
 WEBS 3-15=-149/193, 4-15=-180/445, 4-14=-337/259, 5-14=-268/789, 6-14=-436/349, 6-12=-146/211, 7-12=-306/884, 8-12=-462/319, 8-11=-359/789, 9-11=-131/204

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 561 lb uplift at joint 10 and 610 lb uplift at joint 2.
 5) In the L/OAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=-54, 5-7=-54, 7-10=-54, 2-12=-30, 11-12=-80(F=-50), 10-11=-30

1.736166	Tuss	Tuss Type	HIP	JOHN JOHNSON	Dwg. #101020555	1	Job Reference (optional)	6,200 s Jul 13 2005 Mitek Industries, Inc. Tue Oct 11 17:37:59 2005 Page 1	Builders FirstSource, Lake City, Fl 32055
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[illegible]

BRACING
 TOP CHORD
 BOT CHORD
 WEBS
 Structural wood sheathing directly applied or 3-4-3 cc putlins.
 Rigid ceiling directly applied or 6-11-0 cc bracing.
 1 Row at midpt
 6-12

REACTIONS

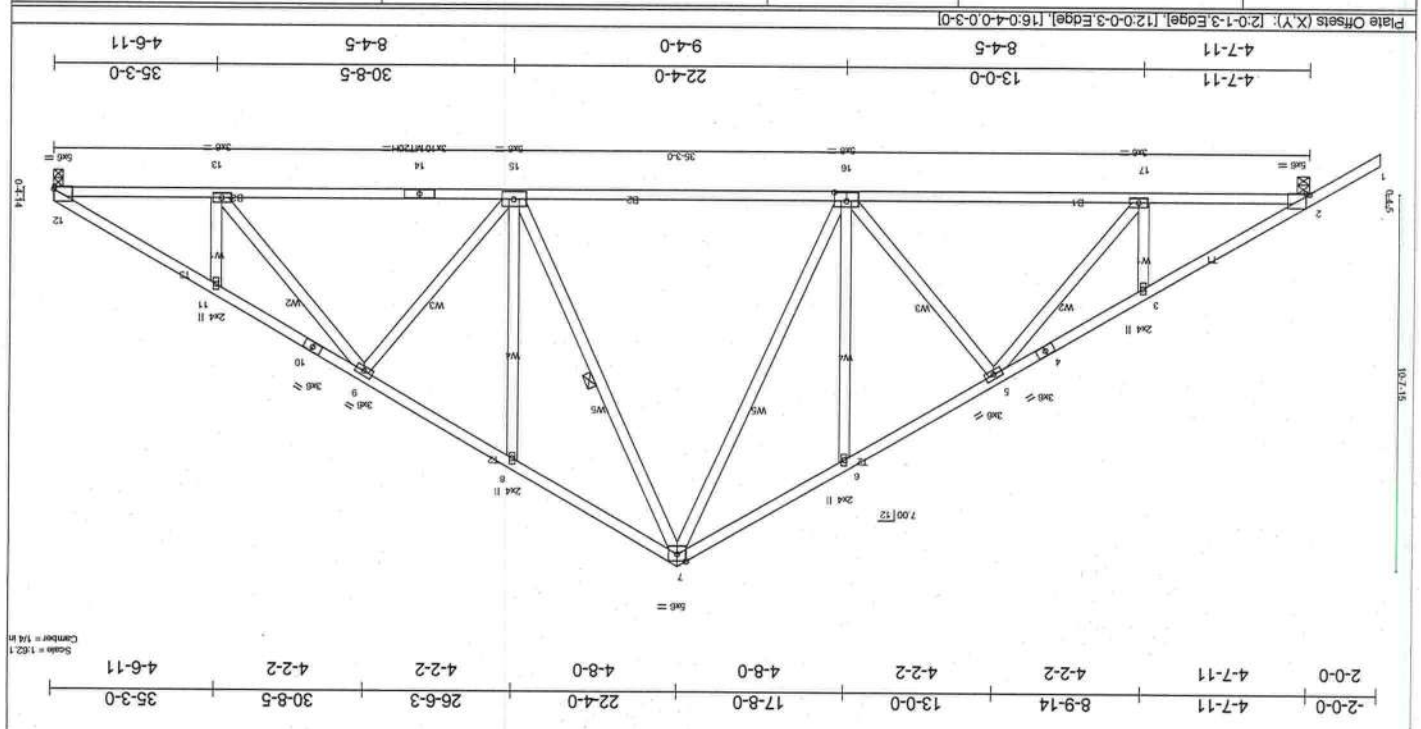
(b) - Maximum Compression/Maximum Tension

OP CHORD	NOT CHORD	NEBS
1-2=5/4, 2-3=27/32, 3-4=2696/1017, 4-5=2631/1032, 5-6=2090/879, 6-7=1724/816, 7-8=2275/956, 8-9=3224/1291, 9-10=3272/1161	2-15=712/2273, 14-15=574/1941, 13-14=402/1629, 12-13=402/1629, 11-12=667/2153, 10-11=923/2730	3-15=1180/220, 5-15=220/514, 5-14=394/309, 6-14=274/813, 6-12=236/321, 7-12=294/851, 8-12=251/364, 8-11=406/875, 9-11=1622/2229

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 11 mph (3-second gust);
 Exterior(2) zone; Lumber OCL=1.60 plate gnp DCL=1.60.
 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 10 and 624 lb uplift at joint 2.
 Standard
 Regular, Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-6=-54, 6-7=-54, 7-10=-54, 2-12=-30, 11-12=-80(F=-50), 10-11=-30

Vertical Load (plf)
Regular, Linear Increase = 1.25, Plate Increase = 1.25
Vert: 1-6=-54, 6-7=-54, 7-10=-54, 2-12=30, 11-12=80(F=-50), 10-11=-30



LOADING (psi)	SPACING	CS	DEFL	BRACING	LUMBER	REACTIONS	FORCES	NOTES
BCDL 5.0	2-0-0	WB 0.93	in (loc)	TOP CHORD	TOP CHORD 2 X 4 SYP No. 2	Max Horiz 2=385 (load case 4)	Max Uplift 12=589 (load case 6), 2=637 (load case 5)	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 110 mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension (2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) All plates are MT20 plates unless otherwise indicated. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 589 lb uplift at joint 12 and 637 lb uplift at joint 2. 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
BCDL 10.0	Rep Stress Incr NO	BC 0.90	Vert (TL)	BOT CHORD	BOT CHORD 2 X 4 SYP No. 1D "Except"			
TCFL 20.0	Plates Increase 1.25	TC 0.29	in (loc)	WEBS	WEBS 2 X 4 SYP No. 3			
TCCL 7.0	Lumber Increase 1.25							
BCDL 5.0	Code FBC2004/TP12002							

Weight: 217 lb
 MT20H 187/143
 MT20 244/190
 GRIP 187/143

1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 110 mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension (2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) All plates are MT20 plates unless otherwise indicated.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 589 lb uplift at joint 12 and 637 lb uplift at joint 2.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOADING (psf)	20.0	TCL	7.0	TCDL	10.0	BCDL	5.0
SPACING	2.0-0	Plates/increase	1.25	TC	0.28	CS	
		Lumber/increase	1.25	BC	0.35	WB	0.08
		Rep Stress Incr	YES	Ver(FL)	-0.06	Horz(FL)	0.01
		Code FBC2004/TP12002		Ver(FL)	-0.10	Horz(FL)	0.01
				4	n/a	4	n/a
				180	240	180	240
				L/den	L/den	L/den	L/den
				GRIP	GRIP	GRIP	GRIP
				MT20	MT20	MT20	MT20
				244/190	244/190	244/190	244/190
				Weight: 50 lb	Weight: 50 lb	Weight: 50 lb	Weight: 50 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/ft²) $\leftarrow$ 508/0-4.0, $\leftarrow$ 264/5-10-4.0
(lb/ft²) $\leftarrow$ 2156/1-2.0
Max Uplift $\leftarrow$ 161 (load case 6), $\leftarrow$ 302 (load case 5)

FORCES

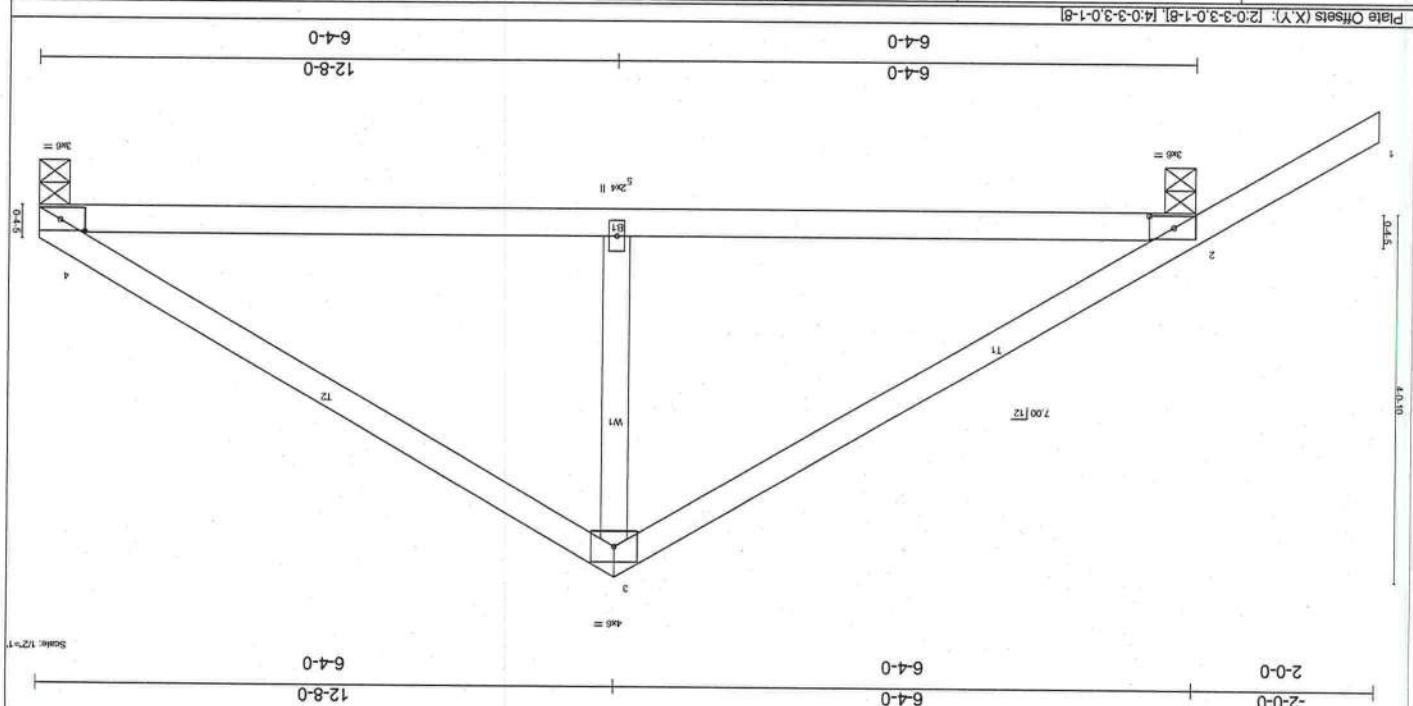
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2/0/54, 2-3 $\leftarrow$ 668/226, 3-4 $\leftarrow$ 663/219
BOT CHORD 2-5 $\leftarrow$ 96/505, 4-5 $\leftarrow$ 96/505
WEBS 3-5 $\leftarrow$ 0/235

NOTES

(1) Unbalanced roof live loads have been considered for this design.
(2) Wind: ASCE 7 zone: 12 mph (-3-second gust); h=20ft; TCDL=4.2psf
Exterior(2) zone: Lumber DOL=1.60 plate gup DOL=1.60. This truss
(3) Provide mechanical connection (by others) of truss to bearing plate

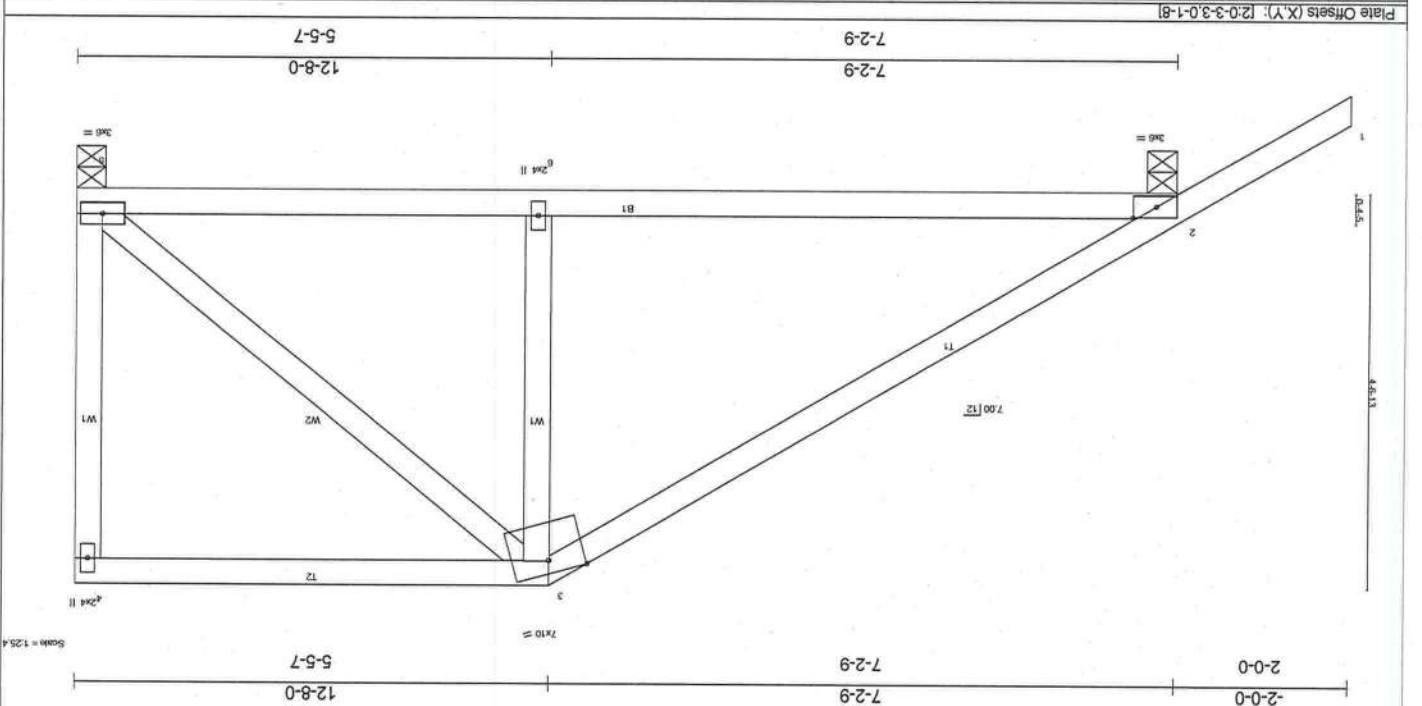
LOAD CASE(S) Standard

1) Unbraced roof live loads have been considered for this design.
2) Wind: ASCE 7-10mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp. B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DDL=1.60 plate gnp DDL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 4 and 302 lb uplift at joint 2.



Job	L136166	Truss	T11	Truss Type	MONO HIP	Qty	1	Plty	JOHN JOHNSON	Dwg.#101205564
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LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2004/TP12002	CSI	DEFL	Vert(LL)	Horz(TL)
20.0	2.0-0	1.25	1.25	YES	2.0-0	TC 0.41	0.09	-0.14	0.01
7.0						BC 0.34	2.6	2.6	5
10.0						WB 0.42			
5.0									

LUMBER	TOP CHORD	2 X 4 SYP No.2	2 X 4 SYP No.3	WEBS	2 X 4 SYP No.3
BRACING	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.			

REACTIONS (lb/size) 5=509/0-4-0, 2=646/0-4-0
 Max Horiz 2=269(load case 5)
 Max Uplift 5=-172(load case 4), 2=-282(load case 5)
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=602/156, 3-4=-19/26, 4-5=-110/138
 BOT CHORD 2-6=-216/437, 5-6=-215/445
 WEBS 3-6=0/225, 3-5=-547/262
NOTES
 1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft, TCFL=4.2psf, BCFL=3.0psf, Category II, Exp B; enclosed, MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Provide adequate drainage to prevent water ponding.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 5 and 282 lb uplift at joint 2.
LOAD CASE(S) Standard

Job	L136166	Truss	T12	MONO HIP	Truss Type	JOHN JOHNSON	Ply	1	Qty	1	Job Reference (optional)	Dwg.#101205565
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Scale = 1/8" = 1'-0"

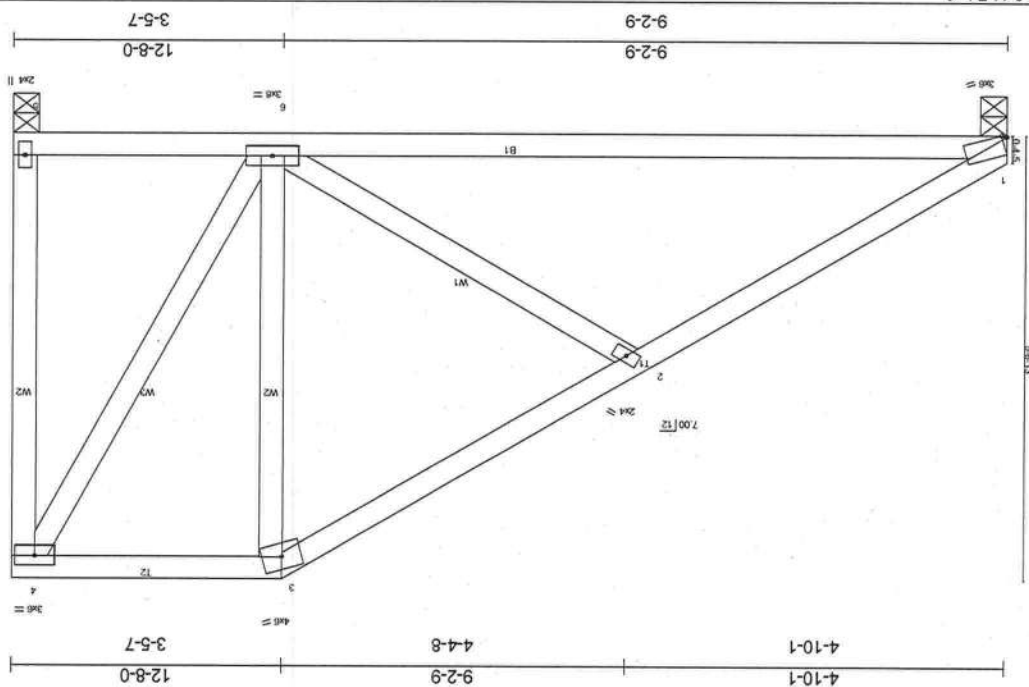


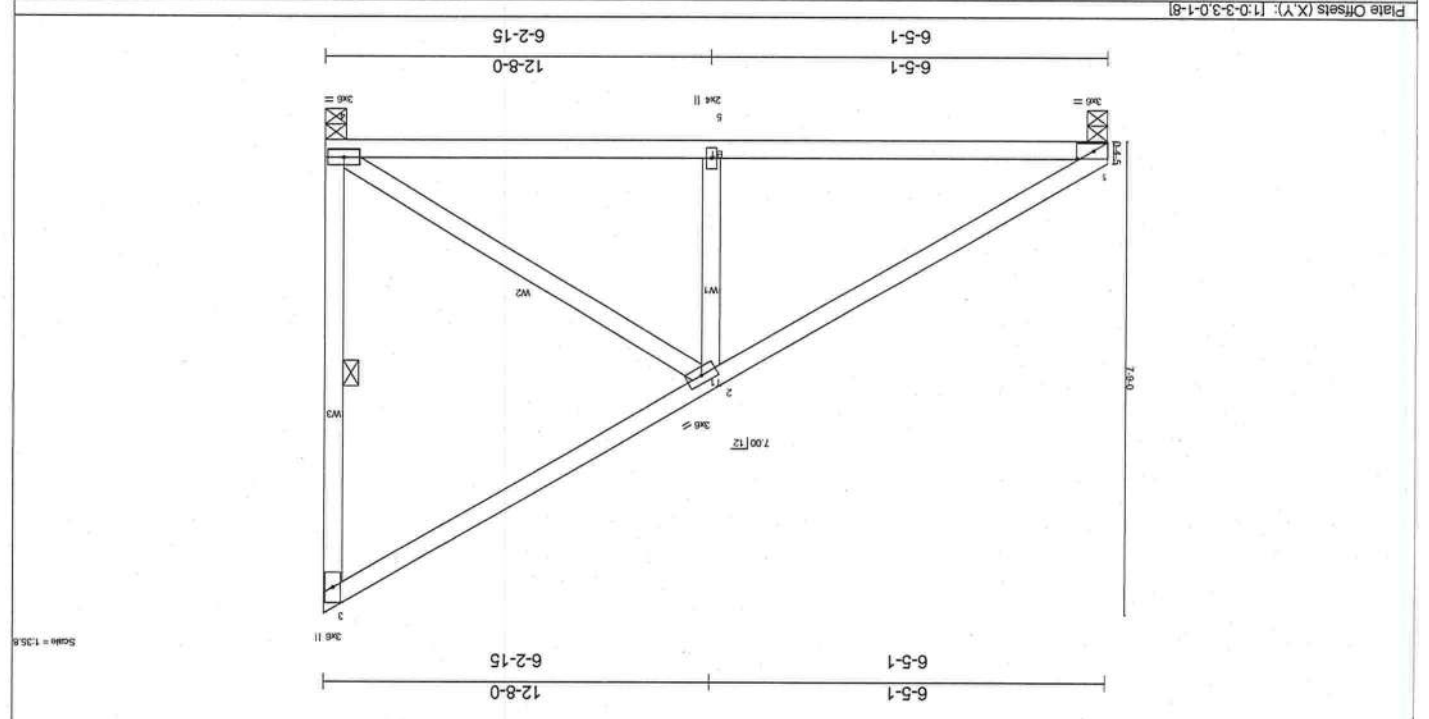
Plate Offsets (X,Y): 11:0-0-11, Edge

LOADING (psf)	SPACING	Code FBC2004/TP12002	Rep Stress Incr	Lumber Increase	Plates Increase	2-0-0	CSI	DEFL	BRACING	TOP CHORD	BOT CHORD	Weight: 72 lb
BCDL 5.0	2-0-0	YES	1.25	1.25	1.25	0.29	0.17	0.01	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	2 X 4 SYP No.2	2 X 4 SYP No.3	
BCLL 10.0												
TCDL 7.0												
TCLL 20.0												

LUMBER	TOP CHORD	BOT CHORD	WEBS	BRACING	TOP CHORD	BOT CHORD	Weight: 72 lb
2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3		Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	2 X 4 SYP No.3	2 X 4 SYP No.3	

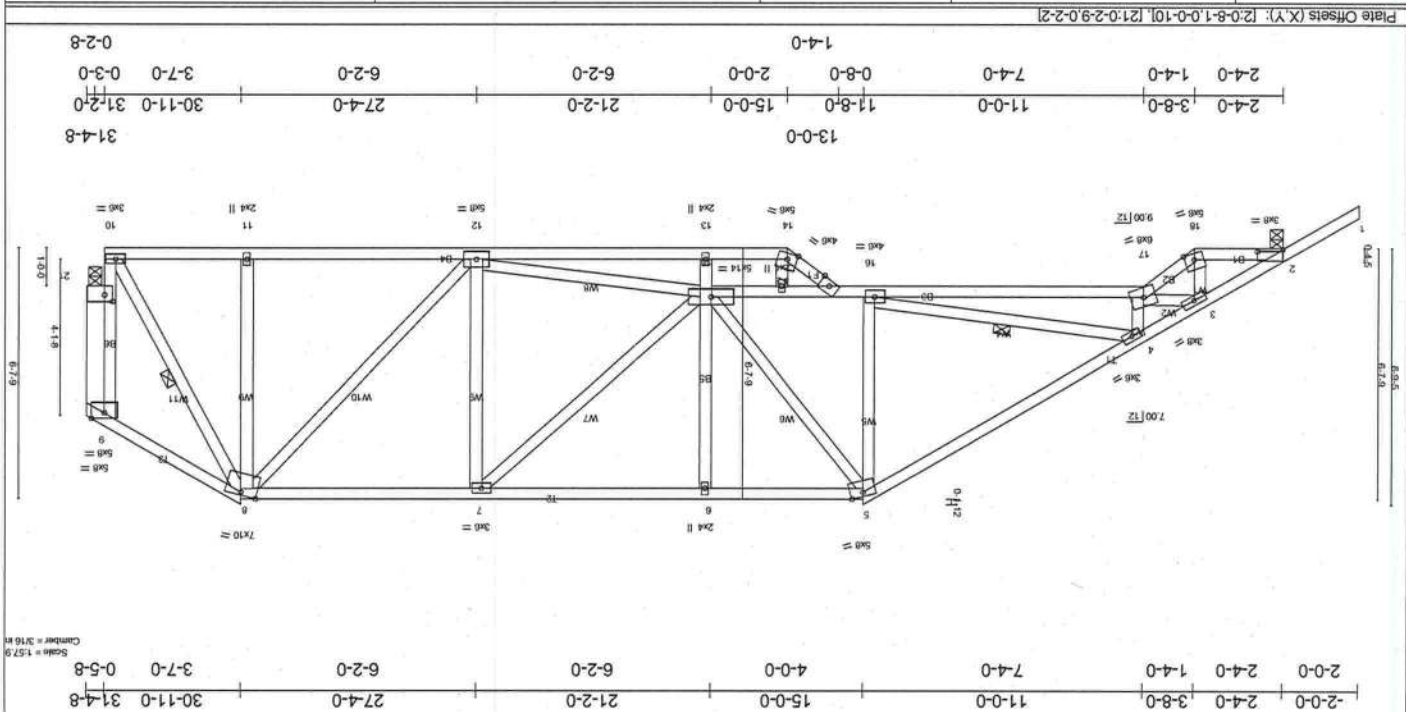
REACTIONS	(lb/size)	1-519/0-4-0, 5-519/0-4-0	Max Horiz 1=251 (load case 5)	Max Uplift 1=-135 (load case 5), 5=-211 (load case 5)	FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD	BOT CHORD	WEBS
					1-2=-641/190, 2-3=-385/93, 3-4=-270/127, 4-5=-537/269	1-6=-341/528, 5-6=-7/1	2-6=-299/253, 3-6=-50/131, 4-6=-244/536	

NOTES	LOAD CASE(S)	Standard
1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.		
2) Provide adequate drainage to prevent water ponding.		
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 1 and 211 lb uplift at joint 5.		



LOADING (psf)	SPACING	Code FBC2004/TP12002	(Matrix)	DEFL	Weight: 67 lb
BCDL 5.0	2-0-0	Rep Stress Incr YES	WB 0.53	Horz(TL) 0.01	MT20
BCLL 10.0		Lumber Increase 1.25	BC 0.34	Vert(TL) -0.10	GRIP
TCCL 7.0		Plates Increase 1.25	TC 0.30	in (loc) 4	244/190
TCLL 20.0				l/d/rf 180	
				L/d 240	

LUMBER	TOP CHORD	2 X 4 SYP No.2	WEBS	2 X 4 SYP No.3	REACTIONS	(lb/size)	1=519/0-4-0, 4=519/0-4-0	Max Horiz 1=339(load case 5)	Max Uplift1=-95(load case 5), 4=-297(load case 5)	FORCES	(lb) - Maximum Compression/Maximum Tension	TOP CHORD	1-2=-676/59, 2-3=-116/50, 3-4=-131/133	BOT CHORD	1-5=-299/516, 4-5=-299/516	WEBS	2-5=0/226, 2-4=-596/339	NOTES	1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 1 and 297 lb uplift at joint 4.	LOAD CASE(S)	Standard
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LOADING (psf)	20.0	TCLL	7.0	BCLL	10.0	BCDL	5.0
SPACING							
2-0-0	Plates Increase	1.25	Lumber Increase	1.25	Rep Stress Incr	YES	Code FBC2004/TP12002
CS	TC	0.42	BC	0.66	WB	0.64	(Matbx)
DEFL	in (loc)	-0.25	Vert(LL)	-0.41	Horz(TL)	0.16	21
l/d _{eff}	180	>999	16-17	>887	n/a	n/a	
PLATES	MT20	244/190	GRIP	Weight: 224 lb			

ITEM	DESCRIPTION	QTY	UNIT	PRICE	TOTAL
LUMBER	TOP CHORD	2 X 4 SYP No. 2			
	BOT CHORD	2 X 4 SYP No. 2 *except			
		B5 2 X 4 SYP No. 3, B6 2 X 4 SYP No. 3			
WEBS		2 X 4 SYP No. 3			
OTHERS		2 X 6 SYP No. 1D			
BRACING	TOP CHORD				
	BOT CHORD				
WEBS					
	1 Row at midpt				
		4-16, 8-10			
	Rigid ceiling directly applied or 5-4-5 oc bracing.				
	Structural wood sheathing directly applied or 2-1-1 oc purlins.				

REACTIONS (lb/size) 2=1442/0-4-0, 21=1327/0-4-0
Max Horiz 2=300(load case 5)
Max Up lift 2=497(load case 5), 21=408(load case 3)

TOP CHORD	BOT CHORD	WEBS
1-2-0/54, 2-3-2=213/615, 3-4-4=402/14102, 4-5-2=2168/771, 5-6-2=1928/772, 6-7-2=1930/778, 8-9-2=1419/605, 9-10-2=58/71	2-18=660/657, 17-18=737/2027, 16-17=1354/3400, 15-16=690/1788, 13-15=0/161, 6-15=291/268, 13-14=0/0, 12-13=48/135	11-12=241=239/615, 4-17-2=813/1345, 10-21=450/1252, 9-21=619/186, 3-18=1193/434, 3-17=813/1345, 4-16=1824/1111, 5-16=105/525, 5-15=315/375, 7-15=252/669, 7-12=832/479, 8-12=485/1111, 8-11=0/152, 8-10=1351/1818

LOAD CASE(S) Standard

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-10, 11mph (3-second gust); h=20ft, TCFL4, 2psf. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Exterior (2) zone: Lumber DOL=1.60 plate girth DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (4) Bearing at joint(s): 21 consider parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 497 lb uplift at joint 2 and 408 lb uplift at joint 21.

LOAD CASE(S) Standard



NAME	ADDRESS	TELEPHONE	DATE	TIME	REMARKS
...	...	...	...	...	...

OTHERS	2 X 6 SYP NO.1D	JOINTS	1 Brace at Jt(s): 16
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Max UpliftZ=-571(load case 5), 21=-345(load case 6)

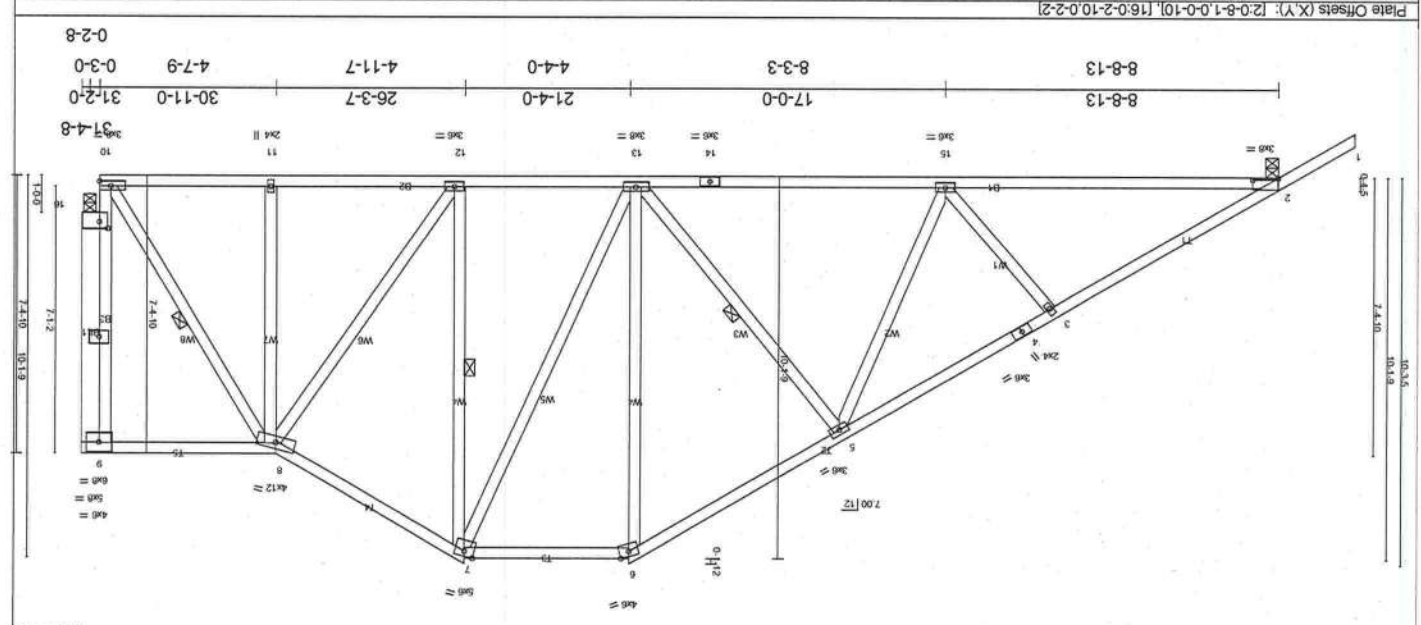
8-10=-10/1/348, 8-13=-149/420, 9-13=-368/819, 9-12=0/164, 9-11=-1298/437

Exterior(2) zone: Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

LOAD CASE(S) Standard

Job	L136166	Truss	T20	SPECIAL	1	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205573
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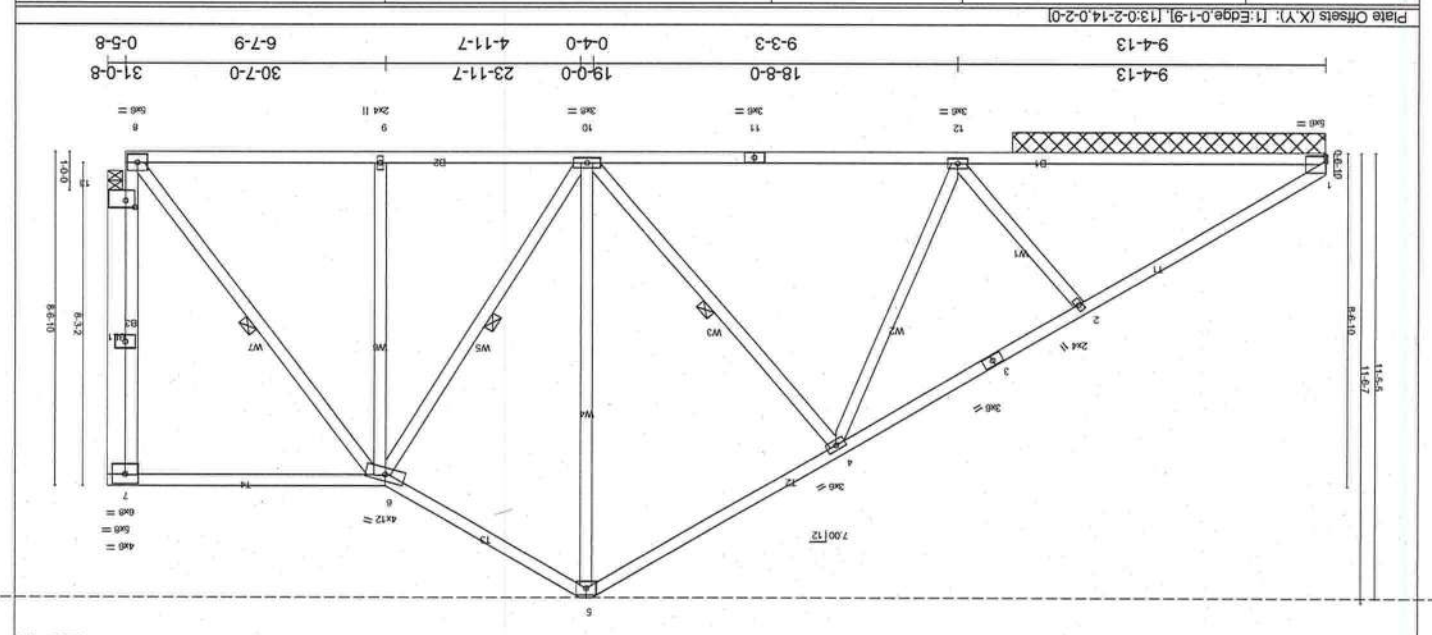


LOADING (psf)	SPACING	CSI	DEFL	BRACING	WEBS	OTHERS	REACTIONS	FORCES	NOTES
BCDL 5.0	Code FBC2004/TP12002	WB 0.44	Horz(TL) 0.05	TOP CHORD	1 Row at midpt	2 X 6 SYP No.10	(lb/size) 2=1406/0-4-0, 16=1294/0-4-0	(lb) - Maximum Compression/Maximum Tension	1) Unbalanced roof live loads have been considered for this design.
BCLL 10.0	Rep Stress Incr YES	BC 0.55	Vert(TL) -0.26	BOT CHORD	Rigid ceiling directly applied or 7-1-13 oc bracing. Except:	WEBS 2 X 4 SYP No.3	Max Horiz 2=439(load case 5)	BOT CHORD 10-16=-426/1166, 9-16=-127/129, 2-15=-783/1710, 14-15=-590/1359, 13-14=-590/1359, 12-13=-325/657, 11-12=-281/715	2) Wind: ASCE 7-02, 110mph (3-second gust); n=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; Lumber DCL=1.60 plate gfp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
TCCL 7.0	Lumber Increase 1.25	TC 0.27	in (loc) 16	Structural wood sheathing directly applied or 4-1-10 oc purlins.	5-13, 7-12, 8-10	WEBS 2 X 4 SYP No.3	Max Uplift2=-520(load case 5), 16=-384(load case 5)	TOP CHORD 1-2=0/54, 2-3=-207/1656, 3-4=-1873/625, 4-5=-1795/646, 5-6=-1238/625, 6-7=-1004/509, 7-8=-1051/454, 8-9=-171/12	3) Provide adequate drainage to prevent water ponding.
TCLL 20.0	Plates Increase 1.25		L/d	Weight: 233 lb					4) Bearing at joint(s) 16 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
									5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 520 lb uplift at joint 2 and 384 lb uplift at joint 16.

REACTIONS	FORCES	NOTES
(lb/size) 2=1406/0-4-0, 16=1294/0-4-0	(lb) - Maximum Compression/Maximum Tension	1) Unbalanced roof live loads have been considered for this design.
Max Horiz 2=439(load case 5)	BOT CHORD 10-16=-426/1166, 9-16=-127/129, 2-15=-783/1710, 14-15=-590/1359, 13-14=-590/1359, 12-13=-325/657, 11-12=-281/715	2) Wind: ASCE 7-02, 110mph (3-second gust); n=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; Lumber DCL=1.60 plate gfp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
Max Uplift2=-520(load case 5), 16=-384(load case 5)	TOP CHORD 1-2=0/54, 2-3=-207/1656, 3-4=-1873/625, 4-5=-1795/646, 5-6=-1238/625, 6-7=-1004/509, 7-8=-1051/454, 8-9=-171/12	3) Provide adequate drainage to prevent water ponding.
		4) Bearing at joint(s) 16 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
		5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 520 lb uplift at joint 2 and 384 lb uplift at joint 16.

Job	L16166	Truss Type	T21	Truss	JOHN JOHNSON	Dwg.#101205574
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Job Reference (optional)	1	Qty	1	Ply	1	6.200 s Jul 13 2005 Mittek Industries, Inc. Tue Oct 11 17:58:20 2005 Page 1
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LOADING (psf)	TOLL 20.0	TCOLL 7.0	BCOLL 10.0	BCDL 5.0
LUMBER	2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 6 SYP No.1D
OTHERS	2 X 6 SYP No.1D	2 X 6 SYP No.1D	2 X 6 SYP No.1D	2 X 6 SYP No.1D

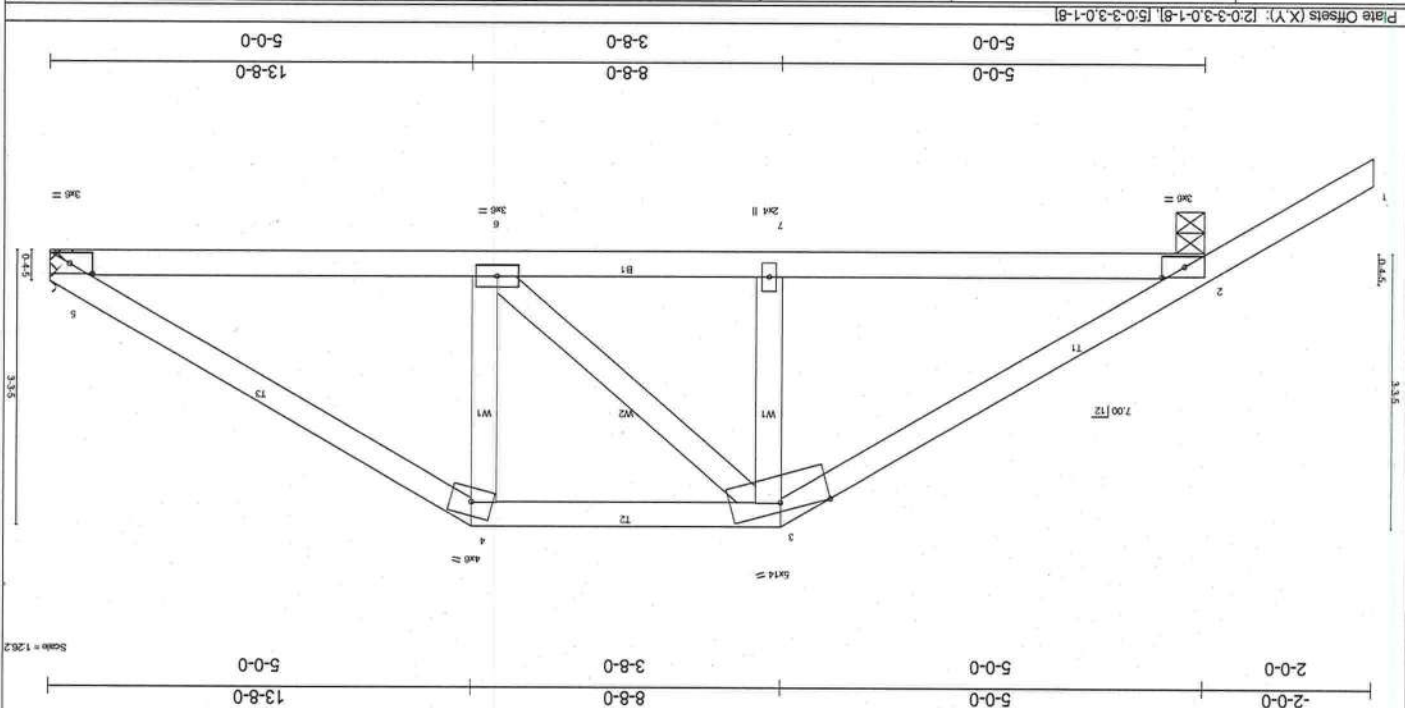
LUMBER	TOP CHORD	2 X 4 SYP No.2
	BOT CHORD	2 X 4 SYP No.2 "Except"
		B3 2 X 4 SYP No.3
WEBS		2 X 4 SYP No.3
OTHERS		2 X 6 SYP No.1D

BRACING	TOP CHORD	Structural wood sheathing directly applied or 3'-10"-14 oc purlins.
	BOT CHORD	Rigid ceiling directly applied or 6'-8"-6 oc bracing. Except:
		7-13
WEBS		1 Row at midpt
		4'-10, 6'-10, 6'-8

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Bearing at joint(s) 13 considers parallel to grain value using ANSIT/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 1 and 421 lb uplift at joint 13.

LOAD CASE(S) Standard



LOADING (psf)	20.0	TCLL	20.0	Plates Increase	1.25	TC	0.28	CSI	DEFL	in (loc)	l/defl	L/d	PLATES MT20 GRIP	244/190	Weight: 61 lb
	7.0	TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.04	5-6	>999	180			
	10.0	BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	-0.07	5-6	>999	160			
	5.0	BCCL	5.0	Code FBC2004/TP12002						5	n/a	n/a			

BRACING	TOP CHORD	BOT CHORD
LUMBER	TOP CHORD 2 X 4 SYP No.2	BOT CHORD 2 X 4 SYP No.2
	WEBS 2 X 4 SYP No.3	
Structural wood sheathing directly applied or 5-1-3 oc purlins.		Rigid ceiling directly applied or 8-2-9 oc bracing.

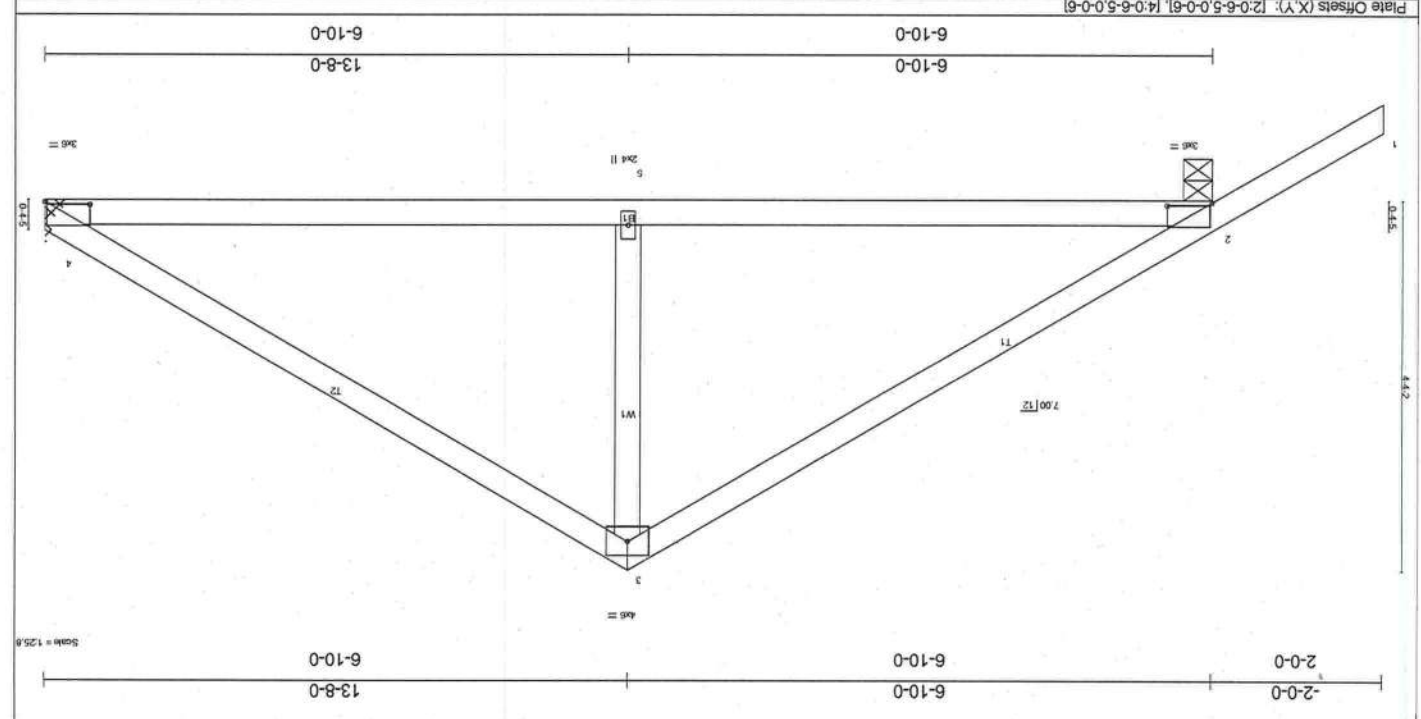
REACTI0NS
(b)size= 56877Mechanical, Z=1034/0-4-0
Max Horiz Z=129/0(load case 3)
Max Up/lfift=5=408(load case 5), Z=547(load case 4)
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-5=-142/656, 3-6=-121/0/649, 4-5=-144/
BOT CHORD 1-2=7=884/1-60, 6-7=590/1-60, 5-6=5-10/191
WEBS 3-7=-138/401, 3-6=-119/104, 4-6=-186/447

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 5 and 547 lb uplift at joint 2.
6) Girder carries hip and with 5'-0" end seabck.
7) Girder(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 170 lb up at 8'-8"-0", and 245 lb down and 170 lb up at 5'-0"-0" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
1) Regular: Lumber increase=1.25, Plate increase=1.25
Vent: 1-3=-54, 3-4=-91(F=-37), 4-5=-54, 2-7=-30, 6-7=-50(F=-20), 5-6=-30
Concentrated Loads (lb)
Vent: 7=-245(F) 6=-245(F)

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L136166	Truss	T23	COMMON	Truss Type	JOHN JOHNSON	Qty	2	Ply	1	Job Reference (optional)	6.200 s Jul 13 2005 Milltek Industries, Inc. Tue Oct 11 17:59:34 2005 Page 1
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Dwg.#101205576

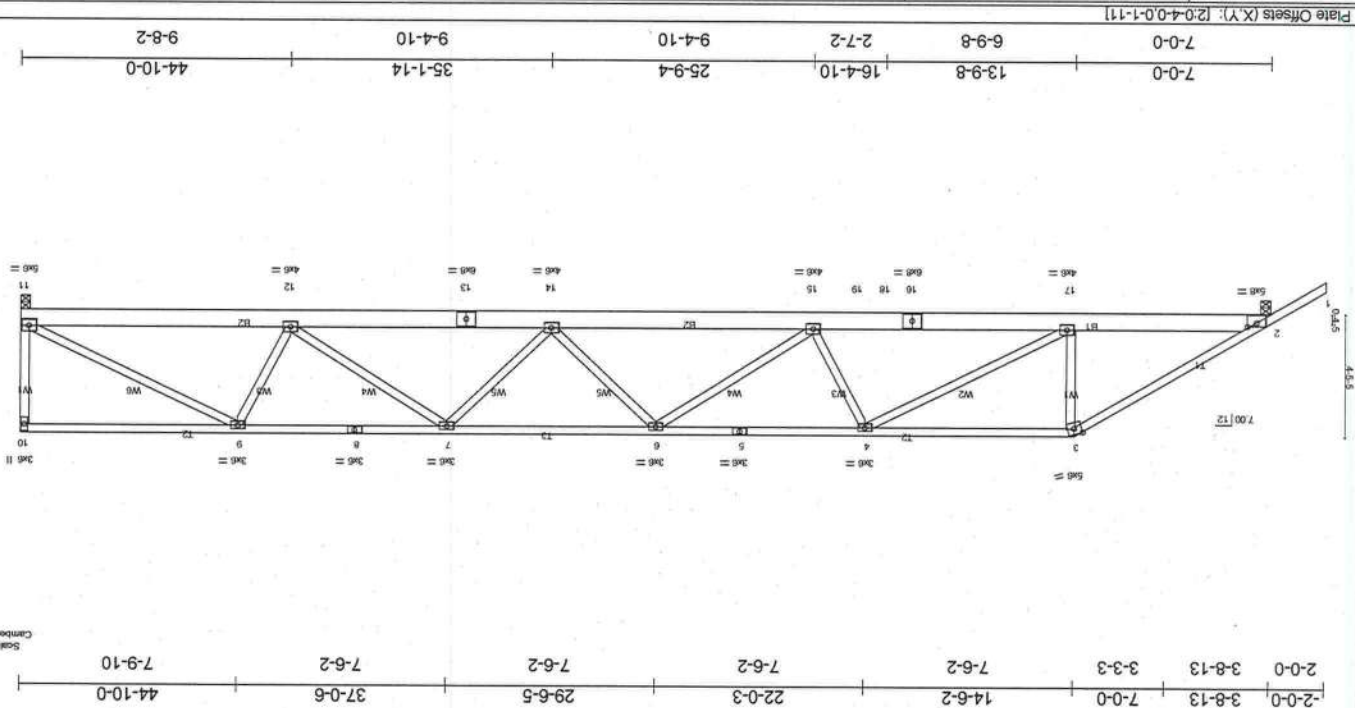


LOADING (psf)	20.0	SPACING	2-0-0	Code FBC2004/TP12002	Rep Stress Incr	YES	WB	0.08	DEFL	0.01	Weight: 53 lb
TCDL	7.0	Lumber Increase	1.25	Lumber Increase	1.25	BC	0.42	Horz(TL)	-0.14	4-5	PLATES
BCDL	5.0	Plates Increase	1.25	TC	0.33	CSI	0.06	Vert(TL)	-0.09	>999	MT20
										L/d	GRP
										L/d	
										L/d	

LUMBER	TOP CHORD 2 X 4 SYP No.2	BRACING	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
	BOT CHORD 2 X 4 SYP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	WEBS 2 X 4 SYP No.3			

REACTIONS (lb/size) 4=551/Mechanical, 2=687/0-4-0
 Max Horiz 2=166/load case 4)
 Max Uplift4=-176(load case 6), 2=-314(load case 5)
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-729/247, 3-4=-725/240
 BOT CHORD 2-5=-109/554, 4-5=-109/554
 WEBS 3-5=0/257

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 3) Refer to girder(s) for truss to truss connections.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 4 and 314 lb uplift at joint 2.



LOADING (psf)	SPACING	CSF	DEFL	L/d_{eff}	PLATES	Weight: 876 lb
20.0	Plates increase 1.25	TC 0.64	in (loc) -0.36	240	MT20	
7.0	Lumber Increase 1.25	BC 0.48	vert(TL) -0.58	180		
10.0	Rep Stress Incr NO	WB 0.72	Horz(TL) 0.10	n/a		
5.0	Code FBC2004/TP12002	(Matrix)		n/a		
					GRIP	244/190

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 8 SYP No.1D
WEBS	2 X 4 SYP No.3
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 cc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing.

REACTIONS (lb/size) 11=4173/0-4-0, 2=3788/0-4-0

Max Uplift11=-2381(load case 2), 2=-1620(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/39, 2-3=-69868/3343, 3-4=-8068/2937, 4-5=-11460

3-17=-1580/3100, 4-17=-5161/3080, 4-15=-911/2025, 6

NOTES

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 cc.

Bottom ends connected as follows: 2 X 6 - 2 foms at 0-7-0 cc.

have been provided to distribute only loads noted as (E) or (B). Unless

Wind: ASCE 7-02; 110 mph (3-second gust); $h=20$ ft; TCFL=4.2 psf; BO

(4) Provide adequate drainage to prevent water ponding.

(5) Girder carries hip end with 0-0-0 right side setback 14-9-8 left side setback 14-9-8

chord. The design/selection of such connection device(s) is the responsibility of the designer.

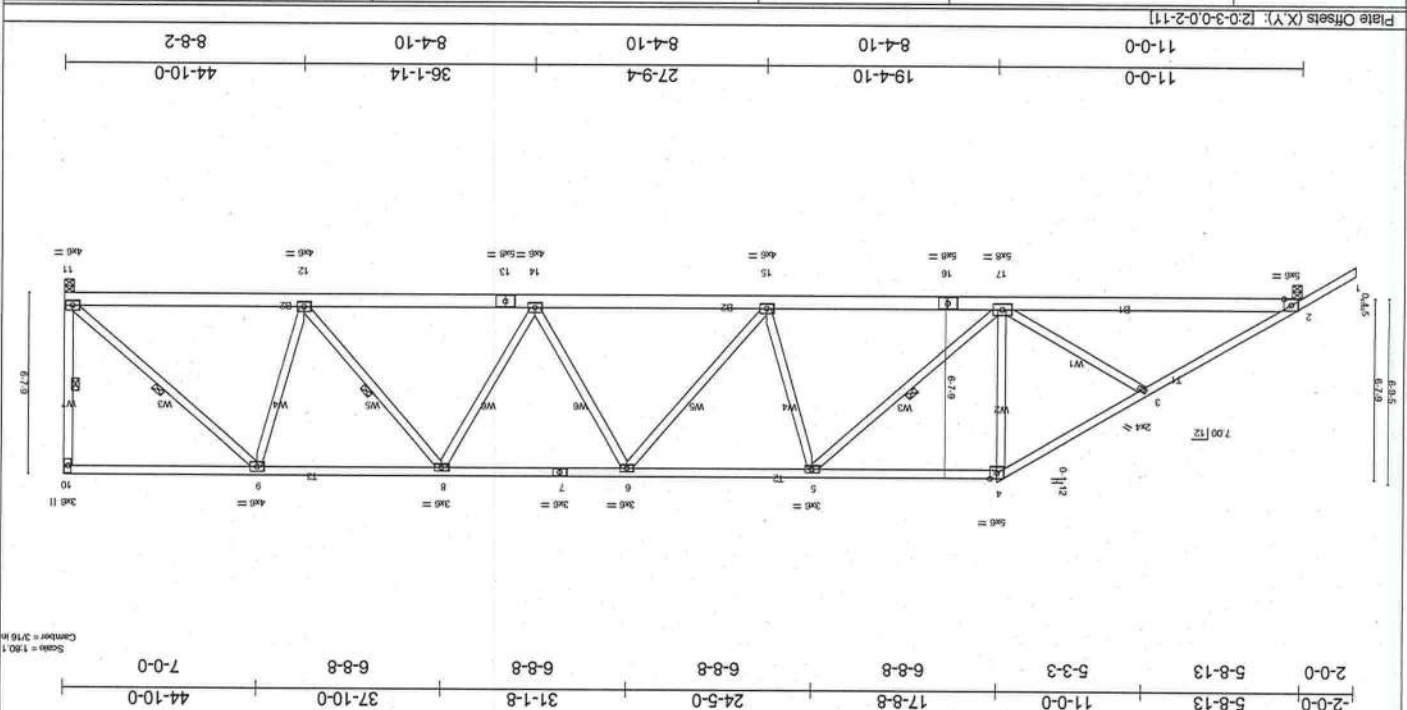
OAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25

Vert: 1-3=-54, 3-4=-54, 4-10=-117 (F=-63), 2-19=-30, 11-19=-6

Vert: 18=-1157(F)

Dwg.#101205578



LOADING	(psf)	TCLL	7.0	BCLL	5.0
		TCDL	20.0	BCLL	10.0
		SPACING	2-0-0		
		Plates increase	1.25		
		Lumber increases	1.25		
		Rep Stress Incr	YES		
		Code FBC2004/TPJ2002			
		CSL	0.51		
		TC	0.51		
		WB	0.97		
		Horz(T/L)	0.11		
		Vert(L/L)	-0.27		
		DEFLL	In (loc)		
		I/dell	n/a		
		>99%	180		
		>99%	240		
		MT20	GRIP		
		Weight:	292 lb		

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	2 X 4 SYP No.1D	WEBS	2 X 4 SYP No.3
LUMBER					
BRACING					
TOP CHORD					
BOT CHORD					
WEBS					
1 Row at midpt					
10-11, 5-17, 8-12, 9-11					
Rigid ceiling directly applied or 7-3 oc bracing					
Structural wood sheathing directly applied or 3-3 oc purlins, except end verticals.					

REACTIONS (lb/size) 11=1867/0-4-0, 2=1990/0-4-0
Max Horiz 2=367 (load case 5)
Max Up/lift 11=700 (load case 6), 2=633 (load case 5)

FORCES (b) - Maximum Compression/Tension
TOP CHORD -1.20/57, 2.3=-319/1182, 3.4=-293/1099, 4.5=-246/1006, 5.6=-326/1307, 6.7=-313/4123, 7.8=-313/4123, 8.9=-208/808, 9.10=-39/18, 10.11=-172/121
BOT CHORD 2.17=-1203/2864, 16.17=-1306/2193, 19.16=-1306/2193, 14.15=-1347/3304, 13.14=-1145/2827, 12.13=-1145/2827, 11.12=-173/1763
WEBS 3.17=-277/262, 4.17=-318/1111, 5.17=-1073/433, 5.15=-39/337, 6.15=-120/107, 6.14=-360/241, 8.14=-186/648, 8.12=-172/1190/538, 9.12=-367/11232, 9.11=-233/4940

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110 mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) for lifting water ponding.

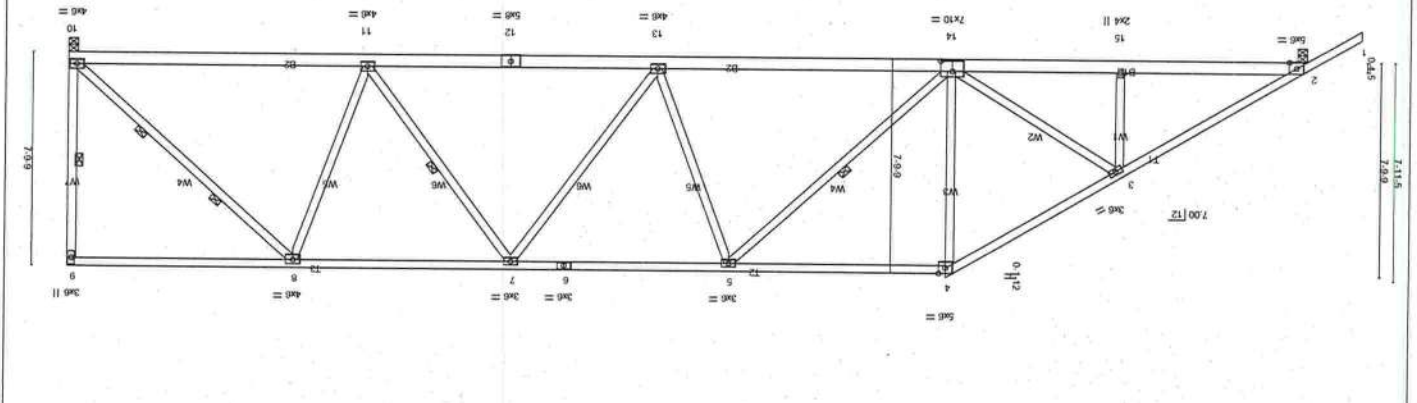
700 lb uplift at joint 11 and 633 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	L136166	Truss	127	MONO HIP	1	JOHN JOHNSON	Dwg.#101205580
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2-0-0	6-8-13	13-0-0	20-10-10	28-9-4	36-7-14	44-10-0	8-2-2
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6-8-13	13-0-0	23-6-3	34-0-5	44-10-0	10-9-11
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LOADING (psf)	SPACING	CS	DEFL	BRACING	WEBS
BCDL 5.0	Code FBC2004/TP12002	WB 0.95	Horz(TL) 0.10	Structural wood sheathing directly applied or 3-2-10 oc purlins, except end	2 Rows at 1/3 pts
BCLL 10.0	Rep Stress Incr YES	BC 0.39	Vert(TL) -0.41	1 Row at midpt	9-10, 5-14, 7-11
TCDL 7.0	Lumber Increase 1.25	TC 0.67	Vert(UL) -0.25	Rigid ceiling directly applied or 7-8-12 oc bracing.	8-10
TCLL 20.0	Plates Increase 1.25		In (loc) 10	Verticals	
	2-0-0		L/d		

LUMBER	TOP CHORD	WEBS	TOP CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 4 SYP No.3

REACTIONS	Max Horiz 2-421 (load case 5)	Max Uplift 10=-698 (load case 6), 2=-645 (load case 5)
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FORCES (lb)	TOP CHORD	WEBS	TOP CHORD	WEBS
1-2=0/57, 2-3=-3238/1114, 3-4=-2774/1054, 4-5=-2318/976, 5-6=-2811/1121, 6-7=-2811/1121, 7-8=-2067/802, 8-9=-4319, 9-10=-200/141	1-2=0/57, 2-3=-3238/1114, 3-4=-2774/1054, 4-5=-2318/976, 5-6=-2811/1121, 6-7=-2811/1121, 7-8=-2067/802, 8-9=-4319, 9-10=-200/141	1-2=0/57, 2-3=-3238/1114, 3-4=-2774/1054, 4-5=-2318/976, 5-6=-2811/1121, 6-7=-2811/1121, 7-8=-2067/802, 8-9=-4319, 9-10=-200/141	1-2=0/57, 2-3=-3238/1114, 3-4=-2774/1054, 4-5=-2318/976, 5-6=-2811/1121, 6-7=-2811/1121, 7-8=-2067/802, 8-9=-4319, 9-10=-200/141	1-2=0/57, 2-3=-3238/1114, 3-4=-2774/1054, 4-5=-2318/976, 5-6=-2811/1121, 6-7=-2811/1121, 7-8=-2067/802, 8-9=-4319, 9-10=-200/141

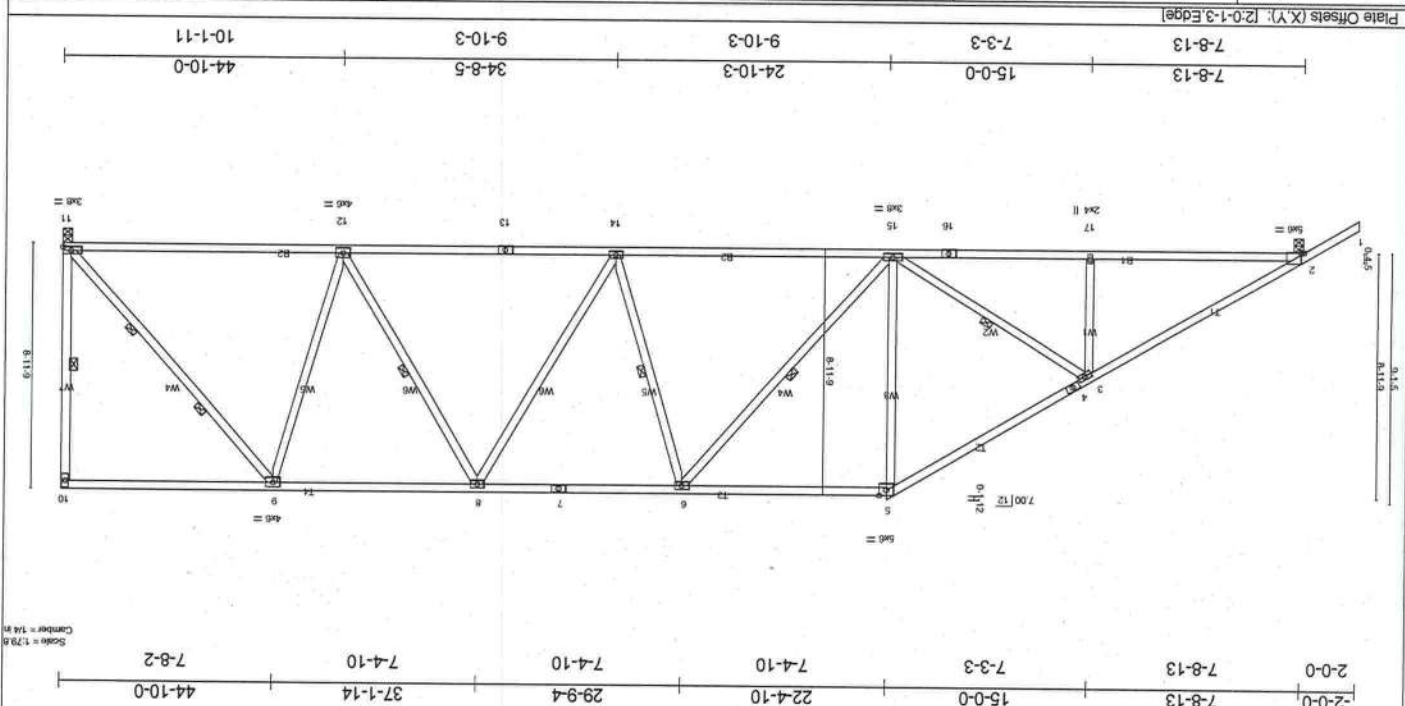
NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 698 lb uplift at joint 10 and 645 lb uplift at joint 2.

LOAD CASE(S) Standard

MONO HIP	1	Qty	JOHN JOHNSON	1	Ply	Job Reference (optional)	Dwg.#101205581
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	20.0 TCLL	7.0 TCOL	10.0 BCLL	5.0 BCDL
SPACING	2-0-0	Plates increase Lumber increase Rep Stress Incr	Code FBC2004/TP12002	(Matrix)
CSI	0.63	T/C	0.81	WB
	0.95	BC	0.95	
DEFLL	in (loc)	Vert(LLL) Vert(TLL) Horz(TLL)	0.16 0.61 0.16	n/a n/a n/a
L/d	>999	240	180	272 lb
GRIP	MT20	244/190		

BRACING	Structural wood sheathing directly applied or 2-10-4 cc purlins, except end verticals.	9-11
TOP CHORD	Rigid ceiling directly applied or 5-9-7 cc bracing.	10-11, 1-9-15, 6-15, 6-14, 8-12
BOT CHORD	1 Row at midpt	2 Rows at 1/3 pts
WEBS		

REACTIONS (lb/size) 11=1867/0-4-0, 2=1990/0-4-0
Max Horz 2=473(load case 5)
Max Uplift 11=-697(load case 6), 2=-654(load case 5)

FORCES (b) - Maximum Compression/Maximum Tension
1-2=0/5/4, 2-3=3178/1074, 3-4=2597/957, 4-5=2581/986, 5-6=2151/926, 6-7=2363/967, 7-8=2383/967, 8-9=1690/668, 9-10=38/9, 10-11=186/129, 11-12=184/264, 12-13=184/264, 13-14=1184/2640, 14-15=1184/2640, 15-16=1184/2640, 16-17=1184/2640, 17-18=1184/2640, 18-19=1184/2640, 19-20=1184/2640, 20-21=1184/2640, 21-22=1184/2640, 22-23=1184/2640, 23-24=1184/2640, 24-25=1184/2640, 25-26=1184/2640, 26-27=1184/2640, 27-28=1184/2640, 28-29=1184/2640, 29-30=1184/2640, 30-31=1184/2640, 31-32=1184/2640, 32-33=1184/2640, 33-34=1184/2640, 34-35=1184/2640, 35-36=1184/2640, 36-37=1184/2640, 37-38=1184/2640, 38-39=1184/2640, 39-40=1184/2640, 40-41=1184/2640, 41-42=1184/2640, 42-43=1184/2640, 43-44=1184/2640, 44-45=1184/2640, 45-46=1184/2640, 46-47=1184/2640, 47-48=1184/2640, 48-49=1184/2640, 49-50=1184/2640, 50-51=1184/2640, 51-52=1184/2640, 52-53=1184/2640, 53-54=1184/2640, 54-55=1184/2640, 55-56=1184/2640, 56-57=1184/2640, 57-58=1184/2640, 58-59=1184/2640, 59-60=1184/2640, 60-61=1184/2640, 61-62=1184/2640, 62-63=1184/2640, 63-64=1184/2640, 64-65=1184/2640, 65-66=1184/2640, 66-67=1184/2640, 67-68=1184/2640, 68-69=1184/2640, 69-70=1184/2640, 70-71=1184/2640, 71-72=1184/2640, 72-73=1184/2640, 73-74=1184/2640, 74-75=1184/2640, 75-76=1184/2640, 76-77=1184/2640, 77-78=1184/2640, 78-79=1184/2640, 79-80=1184/2640, 80-81=1184/2640, 81-82=1184/2640, 82-83=1184/2640, 83-84=1184/2640, 84-85=1184/2640, 85-86=1184/2640, 86-87=1184/2640, 87-88=1184/2640, 88-89=1184/2640, 89-90=1184/2640, 90-91=1184/2640, 91-92=1184/2640, 92-93=1184/2640, 93-94=1184/2640, 94-95=1184/2640, 95-96=1184/2640, 96-97=1184/2640, 97-98=1184/2640, 98-99=1184/2640, 99-100=1184/2640, 100-101=1184/2640, 101-102=1184/2640, 102-103=1184/2640, 103-104=1184/2640, 104-105=1184/2640, 105-106=1184/2640, 106-107=1184/2640, 107-108=1184/2640, 108-109=1184/2640, 109-110=1184/2640, 110-111=1184/2640, 111-112=1184/2640, 112-113=1184/2640, 113-114=1184/2640, 114-115=1184/2640, 115-116=1184/2640, 116-117=1184/2640, 117-118=1184/2640, 118-119=1184/2640, 119-120=1184/2640, 120-121=1184/2640, 121-122=1184/2640, 122-123=1184/2640, 123-124=1184/2640, 124-125=1184/2640, 125-126=1184/2640, 126-127=1184/2640, 127-128=1184/2640, 128-129=1184/2640, 129-130=1184/2640, 130-131=1184/2640, 131-132=1184/2640, 132-133=1184/2640, 133-134=1184/2640, 134-135=1184/2640, 135-136=1184/2640, 136-137=1184/2640, 137-138=1184/2640, 138-139=1184/2640, 139-140=1184/2640, 140-141=1184/2640, 141-142=1184/2640, 142-143=1184/2640, 143-144=1184/2640, 144-145=1184/2640, 145-146=1184/2640, 146-147=1184/2640, 147-148=1184/2640, 148-149=1184/2640, 149-150=1184/2640, 150-151=1184/2640, 151-152=1184/2640, 152-153=1184/2640, 153-154=1184/2640, 154-155=1184/2640, 155-156=1184/2640, 156-157=1184/2640, 157-158=1184/2640, 158-159=1184/2640, 159-160=1184/2640, 160-161=1184/2640, 161-162=1184/2640, 162-163=1184/2640, 163-164=1184/2640, 164-165=1184/2640, 165-166=1184/2640, 166-167=1184/2640, 167-168=1184/2640, 168-169=1184/2640, 169-170=1184/2640, 170-171=1184/2640, 171-172=1184/2640, 172-173=1184/2640, 173-174=1184/2640, 174-175=1184/2640, 175-176=1184/2640, 176-177=1184/2640, 177-178=1184/2640, 178-179=1184/2640, 179-180=1184/2640, 180-181=1184/2640, 181-182=1184/2640, 182-183=1184/2640, 183-184=1184/2640, 184-185=1184/2640, 185-186=1184/2640, 186-187=1184/2640, 187-188=1184/2640, 188-189=1184/2640, 189-190=1184/2640, 190-191=1184/2640, 191-192=1184/2640, 192-193=1184/2640, 193-194=1184/2640, 194-195=1184/2640, 195-196=1184/2640, 196-197=1184/2640, 197-198=1184/2640, 198-199=1184/2640, 199-200=1184/2640, 200-201=1184/2640, 201-202=1184/2640, 202-203=1184/2640, 203-204=1184/2640, 204-205=1184/2640, 205-206=1184/2640, 206-207=1184/2640, 207-208=1184/2640, 208-209=1184/2640, 209-210=1184/2640, 210-211=1184/2640, 211-212=1184/2640, 212-213=1184/2640, 213-214=1184/2640, 214-215=1184/2640, 215-216=1184/2640, 216-217=1184/2640, 217-218=1184/2640, 218-219=1184/2640, 219-220=1184/2640, 220-221=1184/2640, 221-222=1184/2640, 222-223=1184/2640, 223-224=1184/2640, 224-225=1184/2640, 225-226=1184/2640, 226-227=1184/2640, 227-228=1184/2640, 22

LOAD CASE(S) Standard

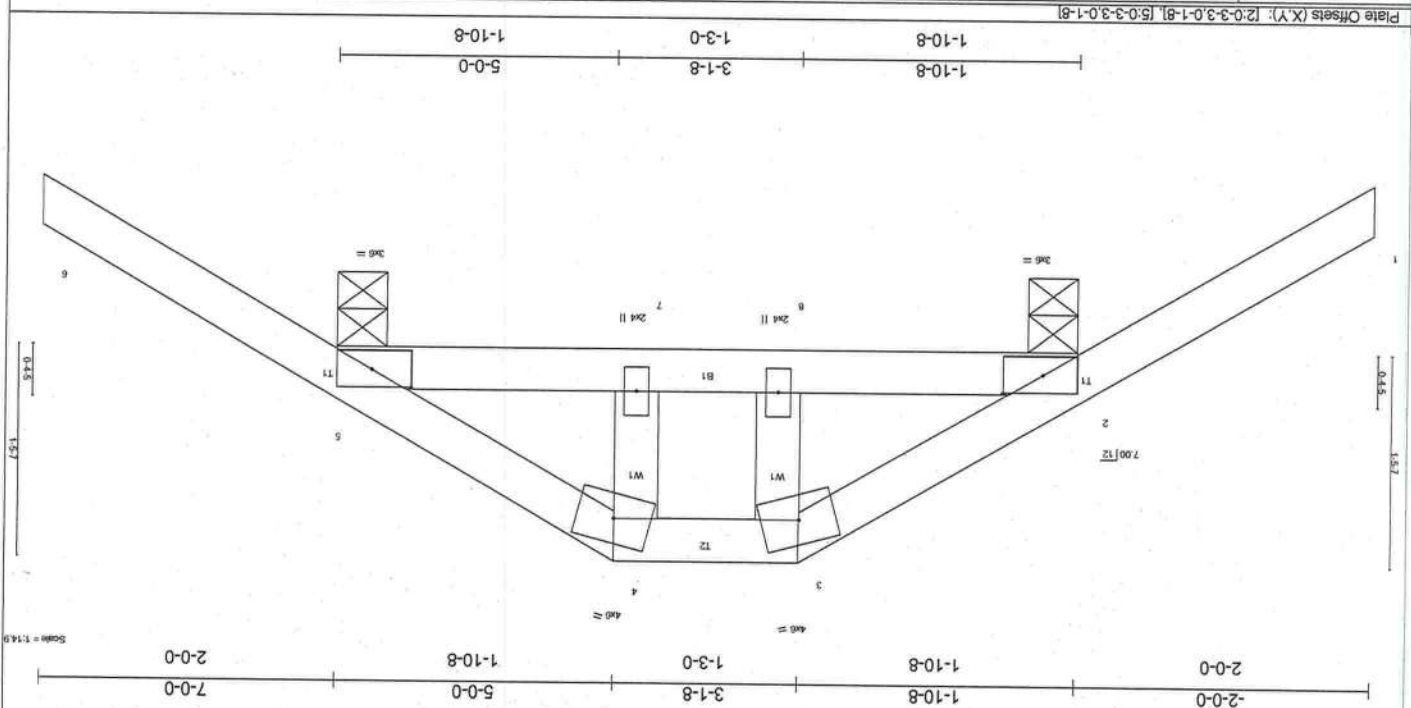
- (1) Unstabilized routine loads have been considered for this design.
- (2) Wind: ASCE 7-02, 11 mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60, plate gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Provide adequate drainage to prevent water ponding.
- (4) All plates are 3x5 MT20 unless otherwise indicated.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 697 lb uplift at joint 11 and 654 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss type	Qty	Ply	JOHN JOHNSON	Dwg.#:101205583
L136166	T30	HIP	1	1		
Builders FirstSource, Lake City, FL 32055						
Job Reference (optional)						

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	TCDL 20.0	BCLL 7.0	BCLD 10.0	BCDL 5.0
	SPACING 2-0-0			
	Plates Increase 1.25	Lumber Increase 1.25	Rep Stress Incr NO	Code FBC2004/TP12002
	CSI	TC 0.28	WB 0.04	(Matrix)
	DEFL In (in)	Ver(T/L) -0.00	Horz(T/L) 0.00	
	I/dell	>999 240	>999 180	n/a
	L/D	MT20	GRIP	Weight: 26 lb
		244/190		

TOP CHORD 2 X 4 SYP No.2
TOP CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
BOT CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

Max Horiz 2=60(load case 4)
Max Uplift12=-279(load case 4), 5=-279(load case 5)
Max Grav 2=471(load case 8), 5=471(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension

OP CHORD 1-2=0/54, 2-3=321/99, 3-4=244/80, 4-5=321/99, 5-6=0/54
BOT CHORD 2-8=63/231, 7-8=64/244, 5-7=59/231
WEBS 3-8=43/119, 4-7=43/119

NOTES

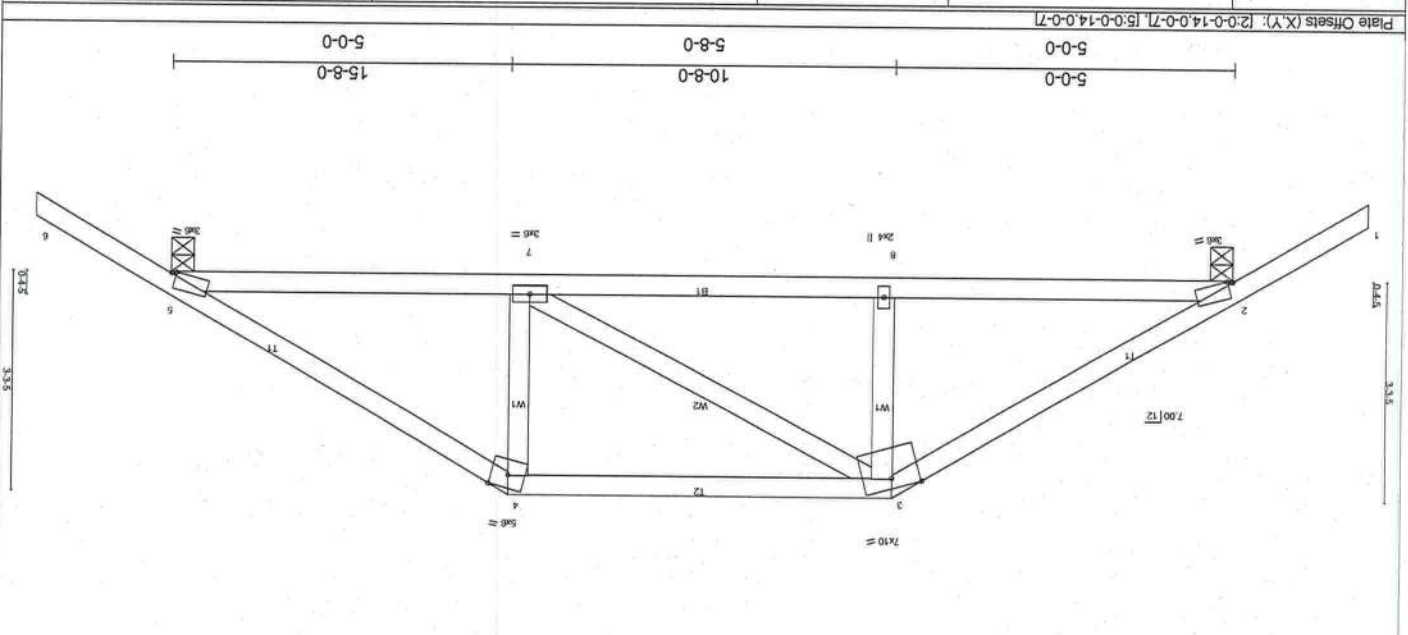
2) Unbraced roof live loads have been considered for this design.
Wind: ASCE 7-02, 10mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category III; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 279 lb uplift at joint 2 and 279 lb uplift at joint 5.
5) Girders carry tie-in span(s): 5'-0-0 from 0'-0 to 5'-0-0
6) Girder carries tie-in span(s): 1'-10-8 right side setback, and 2'-0-0 end setback.
7) Hangar(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 13 lb down and 9 lb up at 3'-1-8, and 13 lb down 9 lb up at 1'-10-8 on bottom chord.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
(1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (plf)
Vert 1-3=-54, 3-4=-50(F=4), 4-6=-54, 2-8=-90(F=-60), 7-8=-88(F=-58), 5-7=-90(F=-60)
Concentrated Loads (lb)
Vert 8=-13(F) 7=-13(F)

LOAD CASE(S) Standard

Uniform Loads (pl)
Regular: Lumber increase=1.25, Plate increase=1.25
Vert. 1.3=54, 3.4=50(f=4), 4.6=54, 2.8=90(f=60), 7.8=88(f=58), 5.7=90(f=60)
Concentrated Loads (lb)
Vert. 8=13(f) 7=13(f)

Job	L136166	Truss	T31	HIP	JOHN JOHNSON	Qty	1	Ply	1	Job Reference (optional)	6/200 s Jul 13 2005 Mike Industries, Inc. Tue Oct 11 18:14:49 2005 Page 1
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Builders FirstSource, Lake City, FL 32055	2-0-0	5-0-0	5-0-0	5-0-0	10-8-0	15-8-0	17-8-0	2-0-0	Scale = 1/32
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LOADING (psf)	SPACING	CSI	DEFL	BRACING	WEBS	REACTIONS	FORCES	NOTES
BCDL 5.0	2-0-0	WB 0.16	Horz(TL) 0.04	TOP CHORD	2 X 4 SYP No. 3	Max Horiz 2=109(load case 2)	Max Uplift 2=601(load case 4), 5=601(load case 5)	(1) Unbalanced roof live loads have been considered for this design.
BCL 10.0	Rep Stress Incr NO	BC 0.43	Vert(TL) -0.13	Structural wood sheathing directly applied or 4-5-14 cc purlins.	BOT CHORD 2 X 4 SYP No. 2			(2) Wind: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber
TCCL 7.0	Lumber Increase 1.25	TC 0.42	in (loc) 5	Rigid ceiling directly applied or 7-4-7 cc bracing.				(3) Provide adequate drainage to prevent water ponding.
TCCL 20.0	Plates Increase 1.25		L/d					(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 5.
								(5) Girder carries hip end with 5-0-0 end setback.
								(6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 170 lb up at 10-8-0, and 245 lb down and 170 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
								(7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

WEBS	3-8=-124/461, 3-7=-105/105, 4-7=-152/493	REACTIONS	(b) - Maximum Compression/Maximum Tension	NOTES
BOT CHORD	2-8=-724/1416, 7-8=-730/1439, 5-7=-830/1417	Max Horiz 2=109(load case 2)	TOP CHORD	(1) Unbalanced roof live loads have been considered for this design.
TOP CHORD	1-2=0/54, 2-3=-1718/800, 3-4=-1440/736, 4-5=-1719/800, 5-6=0/54	Max Uplift 2=601(load case 4), 5=601(load case 5)	WEBS	(2) Wind: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber
				(3) Provide adequate drainage to prevent water ponding.
				(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 5.
				(5) Girder carries hip end with 5-0-0 end setback.
				(6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 170 lb up at 10-8-0, and 245 lb down and 170 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
				(7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
 Concentrated Loads (lb)
 Vert: 8=-245(F) 7=-245(F)

REACTIONS (lb/size) 2=1164/0-4-0, 5=1164/0-4-0
 Max Horiz 2=109(load case 2)
 Max Uplift 2=601(load case 4), 5=601(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-1718/800, 3-4=-1440/736, 4-5=-1719/800, 5-6=0/54
 BOT CHORD 2-8=-724/1416, 7-8=-730/1439, 5-7=-830/1417
 WEBS 3-8=-124/461, 3-7=-105/105, 4-7=-152/493

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber
 (3) Provide adequate drainage to prevent water ponding.
 (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 5.
 (5) Girder carries hip end with 5-0-0 end setback.
 (6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 170 lb up at 10-8-0, and 245 lb down and 170 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 (7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
 Concentrated Loads (lb)
 Vert: 8=-245(F) 7=-245(F)

BRACING
 TOP CHORD
 Structural wood sheathing directly applied or 4-5-14 cc purlins.
 Rigid ceiling directly applied or 7-4-7 cc bracing.
 BOT CHORD
 Weight: 73 lb
 MT20 244/190
 GRIP

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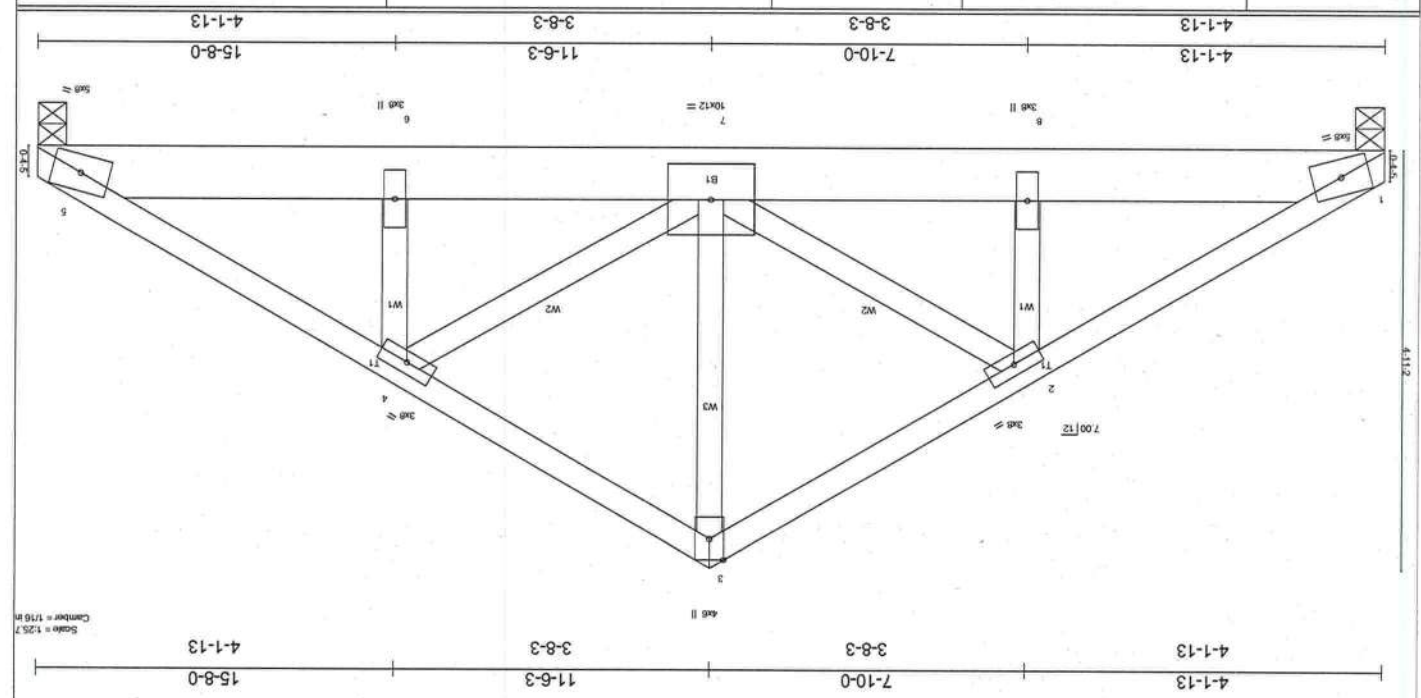


LOAD CASES) Standard

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-02, 110mph (3-second gust); $h=20ft$; TCDF=4.2psf; BCLD=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Exterior(2) zone: Lumber DCL=1.60 plate gird. PDL=1.60.
- (4) Provide adequate drainage to prevent water ponding.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 1 and 335 lb uplift at joint 6.

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#101205586



LOADING (psf)	SPACING	CS	DEFL	VERT (TL)	VERT (LL)	DEFL	TOP CHORD	BOT CHORD	BRACING
BCLD 5.0	Code FBC2004/TP12002	WB 0.98	in (loc)	0.04	-0.18	5	Structural wood sheathing directly applied or 10-0-0 oc purlins.	Rigid ceiling directly applied or 10-0-0 oc bracing.	
BCLD 10.0	Rep Stress Incr	BC 0.65	in (loc)	6-7	>999	180			
TCCL 7.0	Plates Increase	TC 0.36	in (loc)	6-7	>999	240			
TCLL 20.0	Plates Increase		in (loc)	6-7	>999	240			

LUMBER	TOP CHORD	BOT CHORD	WEBS	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS	NOTES
2 X 4 SYP No.2	2 X 8 SYP No.10	2 X 4 SYP No.3	Max Horiz 1=155(load case 3)	Max Uplift 1=-2340(load case 4), 5=-2340(load case 5)	(lb) - Maximum Compression/Maximum Tension	1-2=-9002/3447	1-8=-2984/7752	2-7=-2697/1120	2-8=-946/2562
						2-3=-6362/2480	2-7=-2923/7752	3-7=-2697/1120	3-8=-2348/6106
						4-5=-9002/3447	4-6=-2923/7752	4-7=-2697/1120	4-8=-950/2562

NOTES

1) 2-ply truss to be connected together with 0.13"x3" Nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCCL=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2340 lb uplift at joint 1 and 2340 lb uplift at joint 5.

5) Girder carries tie-in span(s): 36-2-0 from 0-0-0 to 15-8-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

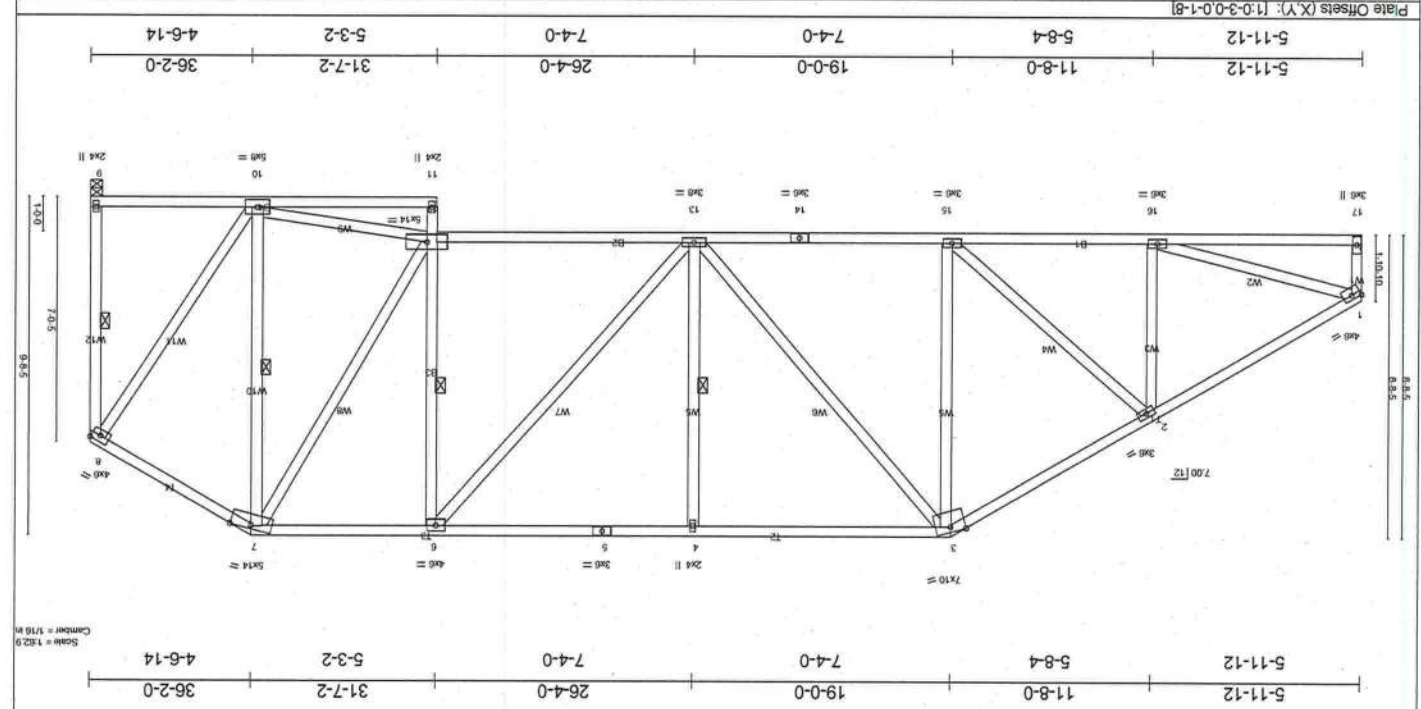
Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 1-5=-740(F=-710)

Dwg.#101205588

Job	Truss	T36	SPECIAL	JOHN JOHNSON	Dwg.#101205589
Qty	Ply	2	1		
Job Reference (optional)					

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LOADING (psf)	SPACING	CSI	DEFL	BRACING	WEBS	RECTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
TOLL 20.0	2.00	TC 0.56	Vert(TL) -0.15	Structural wood sheathing directly applied or 4-6-6 oc purlins, except end and verticals.	1 Row at midpt	Max Horiz 17=227(loud case 5)	Max Uplift 17=-441(loud case 4), 9=-490(loud case 3)	1-2=-1809/662, 2-3=-1738/729, 3-4=-1653/763, 4-5=-1652/763, 5-6=-1652/763, 6-7=-1283/607, 7-8=-812/351, 1-17=-1411/545.	16-17=-301/158, 15-16=-669/1492, 14-15=-626/1443, 13-14=-626/1443, 12-13=-554/1297, 11-12=0/74, 6-12=-763/501, 10-11=-45/15.	9-10=-8/14
TCDL 7.0	1.25	BC 0.43	Vert(TL) -0.24	and 2-0-0 oc purlins (4-7-2 max.), 3-7.	6-12					
BCLL 10.0	YES	WB 0.99	Horz(TL) 0.07	Rigid ceiling directly applied or 7-8-14 oc bracing. Except.	4-13, 7-10, 8-9					
BCLD 5.0	Code FBC2004/TP12002	(Matrix)	In (loc) 9	Weight: 266 lb						

RECTIONS (lb/size) 17=1507/Mechanical, 9=1507/0-4-0
Max Horiz 17=227(loud case 5)
Max Uplift 17=-441(loud case 4), 9=-490(loud case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1809/662, 2-3=-1738/729, 3-4=-1653/763, 4-5=-1652/763, 5-6=-1652/763, 6-7=-1283/607, 7-8=-812/351, 1-17=-1411/545.
BOT CHORD 16-17=-301/158, 15-16=-669/1492, 14-15=-626/1443, 13-14=-626/1443, 12-13=-554/1297, 11-12=0/74, 6-12=-763/501, 10-11=-45/15.
WEBS 9-10=-8/14
2-16=219/160, 2-15=-206/183, 3-15=-83/247, 3-13=-331/428, 4-13=-423/369, 6-13=-257/547, 10-12=-201/637, 7-12=-597/1251, 7-10=-932/442, 1-16=-430/1387, 8-10=-444/145

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 17 and 490 lb uplift at joint 9.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

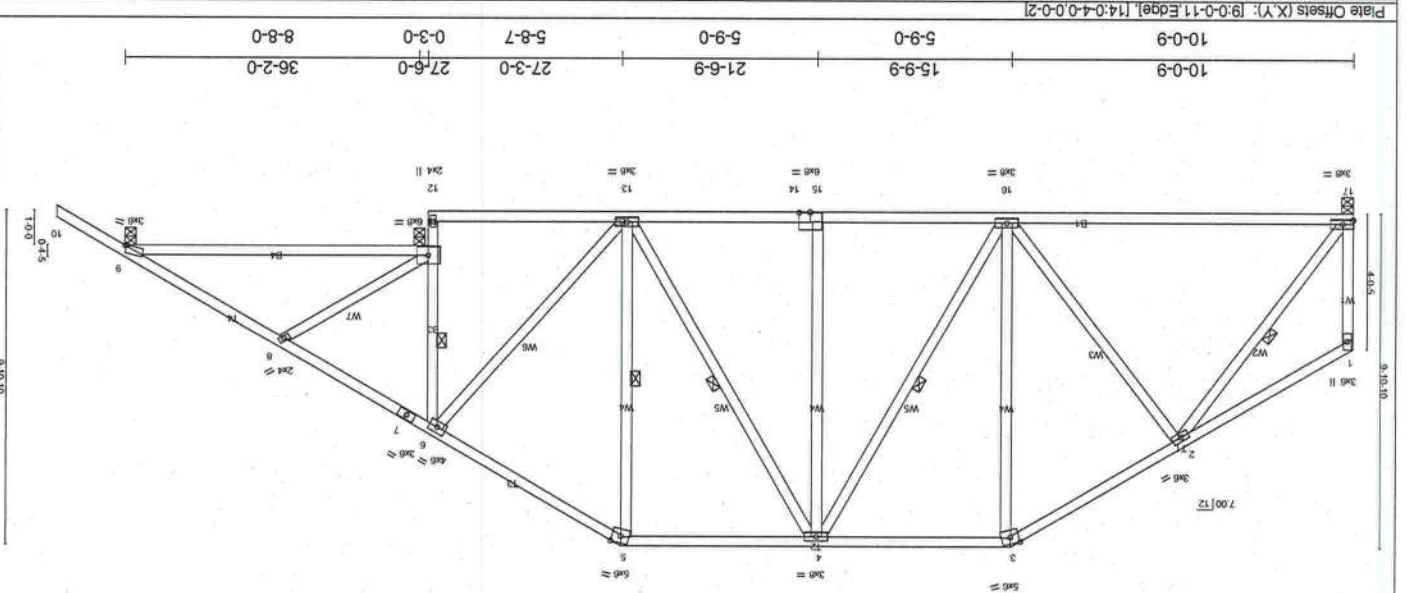
October 12, 2005 TRIBUS Design Engineer

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L136166	Truss	T39	SPECIAL	JOHN JOHNSON	Dwg.#101205592
Qty	1	Ply	1			
Job Reference (optional)	6,200 S JUL 13 2005 Mitek Industries, Inc. Tue Oct 11 18:20:13 2005 Page 1					

Builders FirstSource, Lake City, FL 32055

5-2-1	10-0-9	15-9-9	21-6-9	27-3-0	31-5-11	36-2-0	38-2-0
5-2-1	4-10-9	5-9-0	5-9-0	5-8-7	4-2-11	4-8-5	2-0-0



LOADING (psf)	SPACING	CSI	DEFL	BRACING	WEBS	RECTIONS
BCLL 10.0	Code FBC2004/TP12002	WB 0.45	Horz(TL) 0.03	Structural wood sheathing directly applied or 6-0-0 oc bracing. Except:	1 Row at midpt	Max Gray 11=1613(load case 1), 9=416(load case 10), 17=1102(load case 1)
TCLL 20.0	Rep Stress Incr YES	BC 0.57	Vert(TL) 0.15	Rigid ceiling directly applied or 10-0-0 oc bracing. Except:	6-11	Max Horiz 17=313(load case 3)
TCCL 7.0	Lumber Increase 1.25	TC 0.80	in (loc) 0.20		6-0-0 oc bracing: 12-13.	Max Uplift 11=555(load case 3), 9=388(load case 6), 17=328(load case 5)
BCLL 5.0	Plates Increase 2.00		9-11 >695		1 Row at midpt	
			180		4-16, 4-13, 5-13, 2-17	

WEBS	2 X 4 SYP No.3	TOP CHORD	2 X 4 SYP No.2	WEBS	2 X 4 SYP No.3
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.3

RECTIONS (lb/size) 11=1613/0-4-0, 9=413/0-4-0, 17=1102/0-4-0
 Max Horiz 17=313(load case 3)
 Max Uplift 11=555(load case 3), 9=388(load case 6), 17=328(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-171/87, 2-3=-926/460, 3-4=-746/451, 4-5=-487/372, 5-6=-638/360, 6-7=-86/171, 7-8=-101/103, 8-9=-191/210, 9-10=0/54,
 BOT CHORD 1-17=-189/120
 WEBS 2-16=-161/235, 3-16=-371/59, 4-16=-92/214, 4-15=0/129, 4-13=-545/328, 5-13=-48/81, 6-13=-360/878, 8-11=-262/242, 2-17=-947/396

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCCL=4.2psf; BCLL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS
 for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 525 lb uplift at joint 11, 388 lb uplift at joint 9 and 328 lb uplift at joint 17.

LOAD CASE(S) Standard

The drawing illustrates a roof truss system with various members and joints. The truss is supported by a wall on the left and a column on the right. The roof slope is indicated as 12.4:10. The truss members are labeled with numbers 1 through 14. The joints are labeled with letters A through L. The drawing includes a detailed view of the truss structure, showing the connections between members and the supports. The drawing also includes a table of offsets for the truss members, which is used to determine the vertical and horizontal positions of the members relative to the supports. The table is as follows:

Member	Offset (ft)
1	0.0
2	0.0
3	0.0
4	0.0
5	0.0
6	0.0
7	0.0
8	0.0
9	0.0
10	0.0
11	0.0
12	0.0
13	0.0
14	0.0

[illegible]

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 "Except"
	B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
TOP CHORD	Structural wood sheathing directly applied or 5-8-1 cc purlins, except
	2-0-0 cc purlins (6-0-0 max.): 3-5, 6-8
BOT CHORD	Rigid ceiling directly applied or 6-0-0 cc bracing.
WEBS	1 Row at midpt
JOINTS	1 Brace at Jt(s): 20

REACTIONS (lb/size) 2=533/0-4-0, 13=1441/0-4-0, 11=362/0-4-0, 19=2008/0-4-0
Max Horiz 2=383(load case 4)

Max Uplift = -393 (load case 5), $\Gamma_3 = 530$ (load case 3), $\Gamma_1 = 281$ (load case 6), $\Gamma_9 = 826$ (load case 4)
Max Grav = 2535 (load case 9), $\Gamma_3 = 1441$ (load case 1), $\Gamma_1 = 362$ (load case 10), $\Gamma_9 = 2008$ (load case 1)

FORCES (lb)	TOP CHORD	BOT CHORD	WEBS
1-2=0/154, 2-3=-353/282, 3-4=-208/303, 4-5=-305/274, 5-6=-1055/446, 6-7=-821/471, 7-8=-835/464, 8-9=-168/265, 9-10=-98/34,	10-11=-24/0/48, 11-12=-0/40	12-13=-27/21/27, 13-14=-33/30/129, 14-15=-26/37/85, 15-16=-44/16/320, 16-17=-43/0/14	17-15=-480/282, 18-15=-161/581, 19-14=-1186/414, 10-13=-82/74
Max/min Compression/Max/min Tension			

NOTES

1) Unlabeled roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extent(2) zone; porch left and right exposed; Lumber DOL=1.60 plate gnp DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

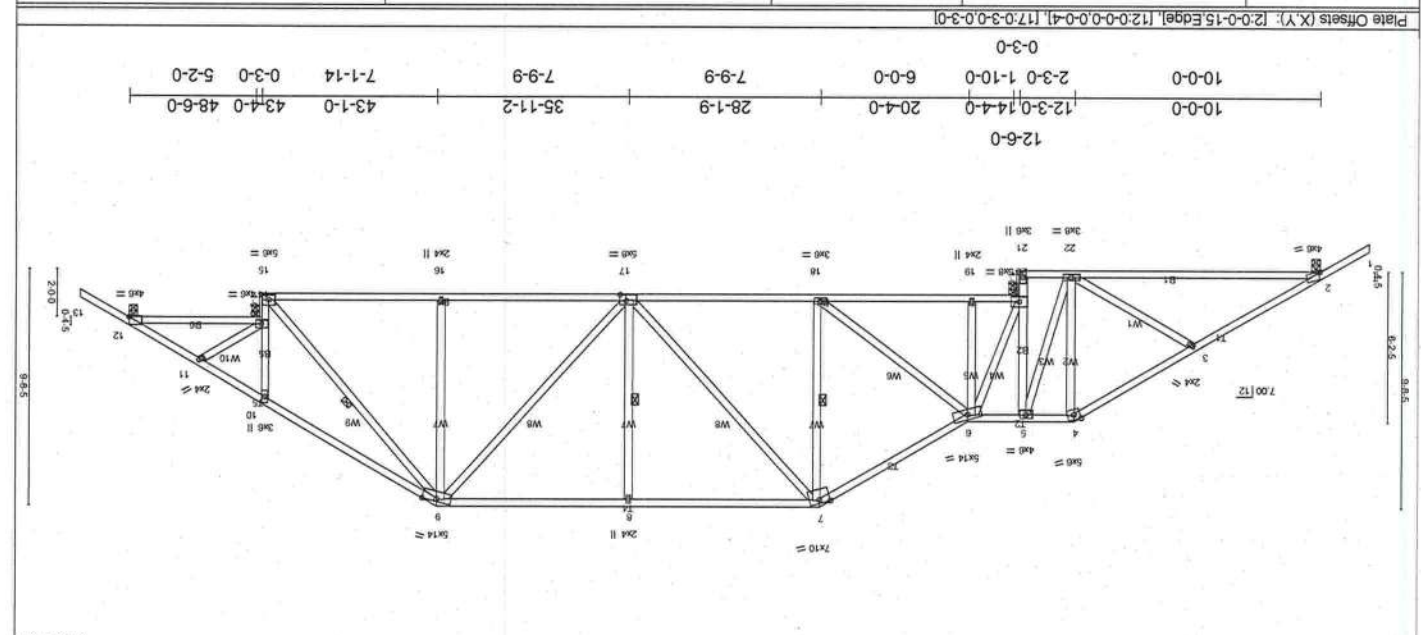
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 393 lb uplift at joint 2, 530 lb uplift at joint 13, 261 lb uplift at joint 11 and 826 lb uplift at joint 19.

5) Design assumes 4x2 (fla orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASES) Standard

Job	L136166	Truss	T142	Truss Type	SPECIAL	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205595
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Builders FirstSource, Lake City, FL 32055	6:200 s Jul 13 2005 Mittek Industries, Inc. Tue Oct 11 18:23:35 2005 Page 1	Job Reference (optional)	1	2-0-0	5-2-13	10-0-0	4-9-3	2-3-0	2-1-0	20-4-0	28-1-9	35-11-2	43-1-0	45-6-11	48-6-0	50-6-0	2-0-0
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LOADING (psf)	SPACING	CSI	DEFLECT	BRACING	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
20.0	2-0	0.43	0.35	Structural wood sheathing directly applied or 5-9-14 oc purlins, except	Max Horiz 2=354(load case 4), 20=1956/0-4-0, 14=1456/0-4-0, 12=363/0-4-0	(lb) - Maximum Compression/Maximum Tension	1-2=0/54, 2-3=-335/324, 3-4=-100/246, 4-5=-43/245, 5-6=-211/275, 6-7=-1057/507, 7-8=-1155/616, 8-9=-1149/616, 9-10=-170/266, 10-11=-99/34, 11-12=-18/49, 12-13=0/54, 13-14=-306/257, 14-15=-307/1028, 15-16=-243/837, 16-17=-317/293, 17-18=-292/510, 18-19=-187/121, 19-20=-615/757, 20-21=-1376/516, 21-22=-615/757, 22-23=-317/293, 23-24=-292/510, 24-25=-187/121, 25-26=-615/757, 26-27=-1376/516, 27-28=-615/757, 28-29=-317/293, 29-30=-292/510, 30-31=-187/121, 31-32=-615/757, 32-33=-1376/516, 33-34=-615/757, 34-35=-317/293, 35-36=-292/510, 36-37=-187/121, 37-38=-615/757, 38-39=-1376/516, 39-40=-615/757, 40-41=-317/293, 41-42=-292/510, 42-43=-187/121, 43-44=-615/757, 44-45=-1376/516, 45-46=-615/757, 46-47=-317/293, 47-48=-292/510, 48-49=-187/121, 49-50=-615/757, 50-51=-1376/516, 51-52=-615/757, 52-53=-317/293, 53-54=-292/510, 54-55=-187/121, 55-56=-615/757, 56-57=-1376/516, 57-58=-615/757, 58-59=-317/293, 59-60=-292/510, 60-61=-187/121, 61-62=-615/757, 62-63=-1376/516, 63-64=-615/757, 64-65=-317/293, 65-66=-292/510, 66-67=-187/121, 67-68=-615/757, 68-69=-1376/516, 69-70=-615/757, 70-71=-317/293, 71-72=-292/510, 72-73=-187/121, 73-74=-615/757, 74-75=-1376/516, 75-76=-615/757, 76-77=-317/293, 77-78=-292/510, 78-79=-187/121, 79-80=-615/757, 80-81=-1376/516, 81-82=-615/757, 82-83=-317/293, 83-84=-292/510, 84-85=-187/121, 85-86=-615/757, 86-87=-1376/516, 87-88=-615/757, 88-89=-317/293, 89-90=-292/510, 90-91=-187/121, 91-92=-615/757, 92-93=-1376/516, 93-94=-615/757, 94-95=-317/293, 95-96=-292/510, 96-97=-187/121, 97-98=-615/757, 98-99=-1376/516, 99-100=-615/757	1 Row at midpt 6-0 oc bracing Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 2-0 oc purlins (5-5-12 max.); 4-6, 7-9	Max Uplift=402(load case 3), 12=-282(load case 6) Max Gray 2=509(load case 9), 20=1956(load case 4), 14=1456(load case 5), 12=363(load case 10)

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

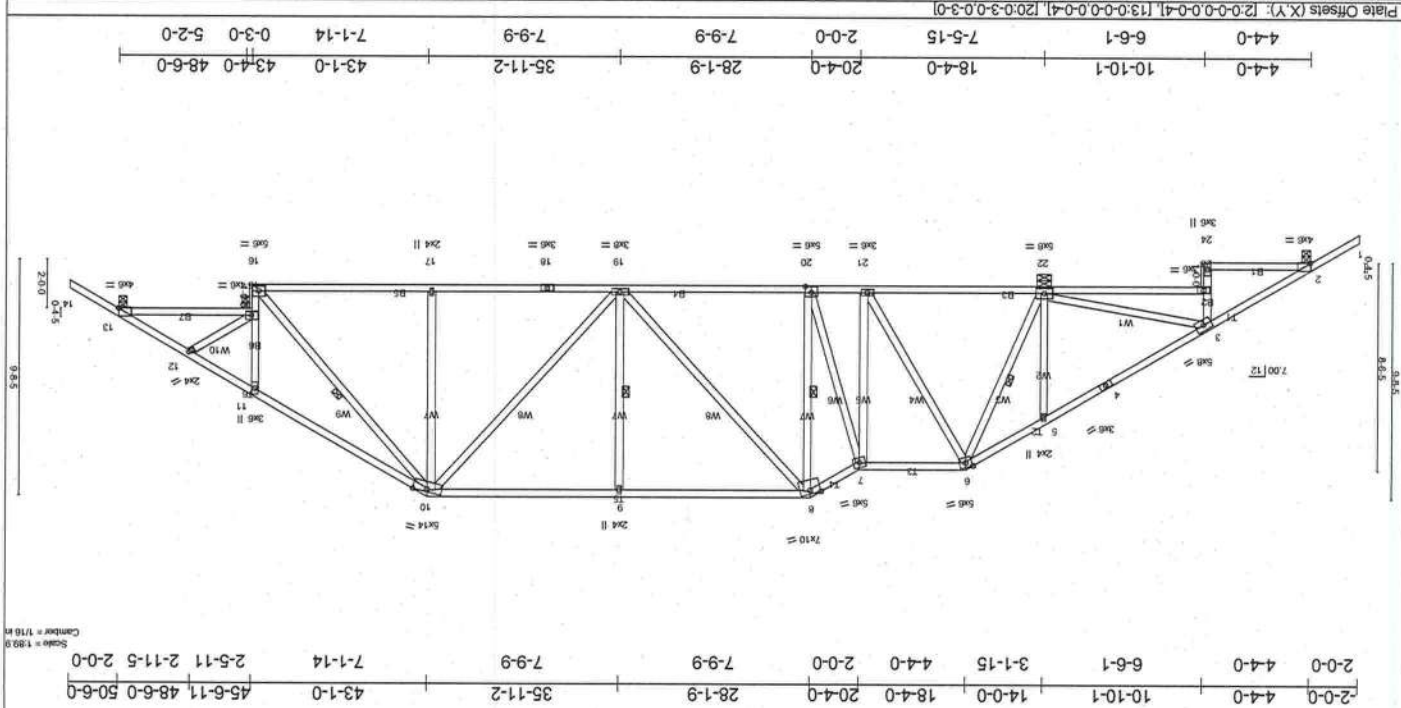
TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3

TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3



LOADING (psf)	SPACING	CSJ	DEFL	L/d	PLATES	Weight: 318 lb
20.0	Plates Increase 1.25	TC 0.44	in (loc) -0.12	>999	MT20	
TCLL	Lumber Increase 1.25	BC 0.66	Ver(UL) -0.20	180	GRP	244/190
TCDL	Rep Stress Incr YES	WB 0.83	Horz(TL) 0.04	n/a		
BCLL						
BCDL						

WEBS	WEBS	WEBS
1 Row at midpt 6-0-0 oc bracing: 2-24, 6-22, 8-20, 9-19, 10-16	2 X 4 SYP No.3 2 X 4 SYP No.3	2 X 4 SYP No.3 2 X 4 SYP No.3
BOT CHORD	BOT CHORD	BOT CHORD
2-0-0 oc purlins (5-4-14 max.): 6-7, 8-10, Rigid ceiling directly applied or 10-0-0 oc bracing. Except:	2 X 4 SYP No.2 "Except" 2 X 4 SYP No.3	2 X 4 SYP No.2 2 X 4 SYP No.3
TOP CHORD	TOP CHORD	TOP CHORD
BRACING	BRACING	BRACING
Structural wood sheathing directly applied or 5-8-1 oc purlins, except:		

REACTIONS (lb/size) 2-3/13/0-4-0, 15=1490/0-4-0, 13=345/0-4-0, 22=2132/0-7-3
Max Horz 2=353/0ad case 4)
Max Uplift2=214/0ad case 4)
Max Gray2=316/0ad case 9), 15=1490/0ad case 1), 13=349/0ad case 10), 22=2132/0ad case 1)
Max Gray2=316/0ad case 9), 15=1490/0ad case 1), 13=349/0ad case 10), 22=2132/0ad case 1)

FORCES (lb)	TOP CHORD	BOT CHORD	WEBS
10-1-1=149-220/83, 11-12-6=68/39, 12-13=158/41, 13-14=50/54	2-24-1=140/07, 2-23-2=219/194, 4-5=56/65/57, 4-5=518/626, 5-6=416/653, 6-7=687/432, 7-8=989/504, 8-9=1171/603, 9-10=1171/603	2-24-1=140/07, 2-23-2=219/194, 4-5=56/65/57, 4-5=518/626, 5-6=416/653, 6-7=687/432, 7-8=989/504, 8-9=1171/603, 9-10=1171/603	8-9=164-64/269, 9-10=164-64/269, 10-11=164-64/269, 11-12=164-64/269, 12-13=164-64/269, 13-14=164-64/269, 14-15=164-64/269, 15-16=164-64/269, 16-17=164-64/269, 17-18=164-64/269, 18-19=164-64/269, 19-20=164-64/269, 20-21=164-64/269, 21-22=164-64/269, 22-23=164-64/269, 23-24=164-64/269, 24-25=164-64/269, 25-26=164-64/269, 26-27=164-64/269, 27-28=164-64/269, 28-29=164-64/269, 29-30=164-64/269, 30-31=164-64/269, 31-32=164-64/269, 32-33=164-64/269, 33-34=164-64/269, 34-35=164-64/269, 35-36=164-64/269, 36-37=164-64/269, 37-38=164-64/269, 38-39=164-64/269, 39-40=164-64/269, 40-41=164-64/269, 41-42=164-64/269, 42-43=164-64/269, 43-44=164-64/269, 44-45=164-64/269, 45-46=164-64/269, 46-47=164-64/269, 47-48=164-64/269, 48-49=164-64/269, 49-50=164-64/269, 50-51=164-64/269, 51-52=164-64/269, 52-53=164-64/269, 53-54=164-64/269, 54-55=164-64/269, 55-56=164-64/269, 56-57=164-64/269, 57-58=164-64/269, 58-59=164-64/269, 59-60=164-64/269, 60-61=164-64/269, 61-62=164-64/269, 62-63=164-64/269, 63-64=164-64/269, 64-65=164-64/269, 65-66=164-64/269, 66-67=164-64/269, 67-68=164-64/269, 68-69=164-64/269, 69-70=164-64/269, 70-71=164-64/269, 71-72=164-64/269, 72-73=164-64/269, 73-74=164-64/269, 74-75=164-64/269, 75-76=164-64/269, 76-77=164-64/269, 77-78=164-64/269, 78-79=164-64/269, 79-80=164-64/269, 80-81=164-64/269, 81-82=164-64/269, 82-83=164-64/269, 83-84=164-64/269, 84-85=164-64/269, 85-86=164-64/269, 86-87=164-64/269, 87-88=164-64/269, 88-89=164-64/269, 89-90=164-64/269, 90-91=164-64/269, 91-92=164-64/269, 92-93=164-64/269, 93-94=164-64/269, 94-95=164-64/269, 95-96=164-64/269, 96-97=164-64/269, 97-98=164-64/269, 98-99=164-64/269, 99-100=164-64/269, 100-101=164-64/269, 101-102=164-64/269, 102-103=164-64/269, 103-104=164-64/269, 104-105=164-64/269, 105-106=164-64/269, 106-107=164-64/269, 107-108=164-64/269, 108-109=164-64/269, 109-110=164-64/269, 110-111=164-64/269, 111-112=164-64/269, 112-113=164-64/269, 113-114=164-64/269, 114-115=164-64/269, 115-116=164-64/269, 116-117=164-64/269, 117-118=164-64/269, 118-119=164-64/269, 119-120=164-64/269, 120-121=164-64/269, 121-122=164-64/269, 122-123=164-64/269, 123-124=164-64/269, 124-125=164-64/269, 125-126=164-64/269, 126-127=164-64/269, 127-128=164-64/269, 128-129=164-64/269, 129-130=164-64/269, 130-131=164-64/269, 131-132=164-64/269, 132-133=164-64/269, 133-134=164-64/269, 134-135=164-64/269, 135-136=164-64/269, 136-137=164-64/269, 137-138=164-64/269, 138-139=164-64/269, 139-140=164-64/269, 140-141=164-64/269, 141-142=164-64/269, 142-143=164-64/269, 143-144=164-64/269, 144-145=164-64/269, 145-146=164-64/269, 146-147=164-64/269, 147-148=164-64/269, 148-149=164-64/269, 149-150=164-64/269, 150-151=164-64/269, 151-152=164-64/269, 152-153=164-64/269, 153-154=164-64/269, 154-155=164-64/269, 155-156=164-64/269, 156-157=164-64/269, 157-158=164-64/269, 158-159=164-64/269, 159-160=164-64/269, 160-161=164-64/269, 161-162=164-64/269, 162-163=164-64/269, 163-164=164-64/269, 164-165=164-64/269, 165-166=164-64/269, 166-167=164-64/269, 167-168=164-64/269, 168-169=164-64/269, 169-170=164-64/269, 170-171=164-64/269, 171-172=164-64/269, 172-173=164-64/269, 173-174=164-64/269, 174-175=164-64/269, 175-176=164-64/269, 176-177=164-64/269, 177-178=164-64/269, 178-179=164-64/269, 179-180=164-64/269, 180-181=164-64/269, 181-182=164-64/269, 182-183=164-64/269, 183-184=164-64/269, 184-185=164-64/269, 185-186=164-64/269, 186-187=164-64/269, 187-188=164-64/269, 188-189=164-64/269, 189-190=164-64/269, 190-191=164-64/269, 191-192=164-64/269, 192-193=164-64/269, 193-194=164-64/269, 194-195=164-64/269, 195-196=164-64/269, 196-197=164-64/269, 197-198=164-64/269, 198-199=164-64/269, 199-200=164-64/269, 200-201=164-64/269, 201-202=164-64/269, 202-203=164-6

NOTES

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: S-SC 7-02: 11 mph (3-second gust); n=20ft; TCCL-4-25ft; BCL-4-25ft; DCL-4-25ft; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Provide adequate drainage to prevent water ponding.
- (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2, 591 lb uplift at joint 15, 277 lb uplift at joint 13 and 973 lb uplift at joint 22.
- (5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

[illegible]

Job Reference (optional)	6 300 5 111 13 2005 MIT 05 12
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REACTIONS (lb/size) 2=269/0-4-0, 15=1736/0-4-0, 13=329/0-4-0, 22=1947/0-7-3
Max Horiz 2=383(load case 4)
Max Uplift2=-194(load case 5), 15=-640(load case 3), 13=-279(load case 6), 22=-858(load case 4)

Max Uplift=2=194(road case 5), 15=-640(road case 3), 13=-279(road case 6), 22=-858(road case 4)
Max Grav=2=271(road case 7), 15=1736(road case 9), 13=332(road case 1), 22=1947(road case 1)

Wavelength (nm)	Bot Chord	Webbs
10-17	2296/875	2296/875
11-16	1560/555	1560/555
12-15	72/2	72/2
13-14	230/602	230/602
14-13	1667/855	1667/855
15-12	320/1024	320/1024
16-11	46/9	46/9
17-10	328/341	328/341
18-9	13/15	13/15
19-8	598/1483	598/1483
20-7	639/1541	639/1541
21-6	289/747	289/747
22-5	1071/147	1071/147
23-4	654/9	654/9
24-3	93/56	93/56
25-2	293/404	293/404
26-1	777/1446	777/1446

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2gst; BCDL=3.0gst; Lumber DOL=1.60 plate gip DOL=1.60. This truss is designed for C-C for members and forces, and for Exterior(2) zone; porch left and right exposed; MWFRS gable end zone and C-C. MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

6) Design assumes 4x2 (flat orientation) purlins at cc spacing indicated, fastened to truss TC w/ 2-10d nails.

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:

Job	L136166	Truss	T47	Truss Type	SPECIAL	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205600
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2-0-0	4-4-0	10-2-0	16-0-0	22-7-12	29-3-7	35-11-2	40-6-0	4-6-14
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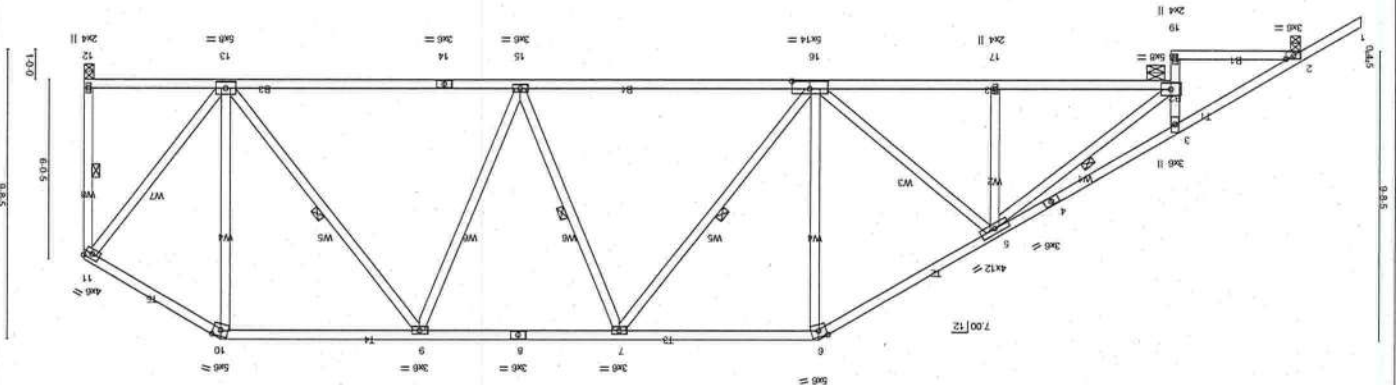


Plate Offsets (X,Y): [2-0-3-4-0-1-8], [16-0-7-0-0-3-0]

LOADING (psf)	SPACING	CS	DEFL	BRACING	LUMBER	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
BCDL 5.0	Code FBC2004/TP12002	WB 0.57	Horz(TL) 0.06	Structural wood sheathing directly applied or 4-6-11 cc purlins, except end and verticals	TOP CHORD 2 X 4 SYP No.2	Max Gray 2=289(load case 9), 18=1688(load case 5), 12=496(load case 3)	TOP CHORD - Maximum Compression/Maximum Tension	1-2=0/54, 2-3=288/124, 3-4=271/82, 4-5=159/97, 5-6=1780/716, 6-7=1481/685, 7-8=1581/698, 8-9=1581/698, 9-10=759/385, 10-11=922/380, 11-12=1482/585	12-13=108, 13-14=610/1398, 14-15=610/1640, 15-16=720/1640, 16-17=655/1537, 17-18=655/1537, 18-19=576/2, 19-20=378/319, 20-21=1482/585	5-18=1795/694, 5-17=0/142, 5-16=1781/66, 6-16=121/513, 7-16=371/335, 7-15=171/184, 9-15=116/507, 9-13=1068/543, 10-13=231/88, 11-13=458/1208

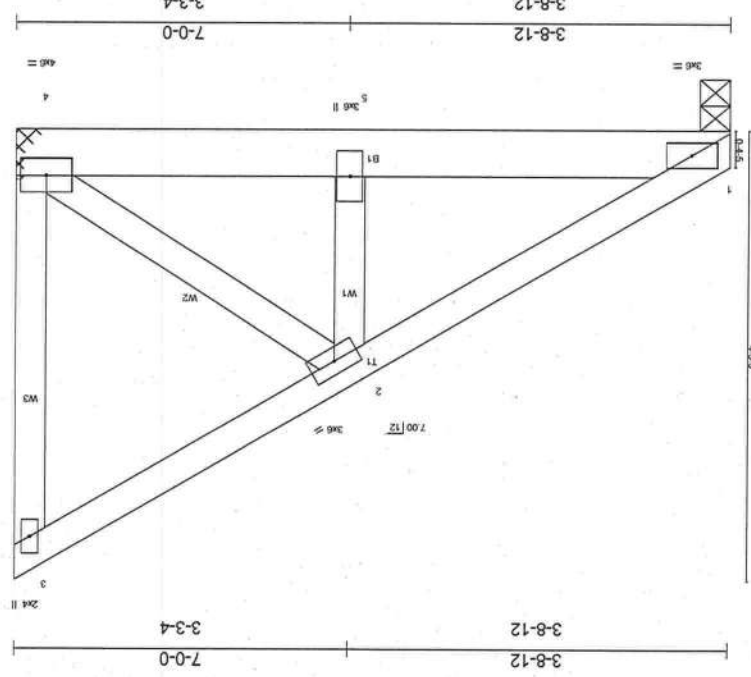
WEBS	TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-6-11 cc purlins, except end and verticals	2 X 4 SYP No.2	2 X 4 SYP No.3

REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
(lb/size) 2=289/0-4-0, 18=1688/0-7-3, 12=1516/0-4-0	Max Gray 2=289(load case 9), 18=1688(load case 5), 12=496(load case 3)	1-2=0/54, 2-3=288/124, 3-4=271/82, 4-5=159/97, 5-6=1780/716, 6-7=1481/685, 7-8=1581/698, 8-9=1581/698, 9-10=759/385, 10-11=922/380, 11-12=1482/585	12-13=108, 13-14=610/1398, 14-15=610/1640, 15-16=720/1640, 16-17=655/1537, 17-18=655/1537, 18-19=576/2, 19-20=378/319, 20-21=1482/585	5-18=1795/694, 5-17=0/142, 5-16=1781/66, 6-16=121/513, 7-16=371/335, 7-15=171/184, 9-15=116/507, 9-13=1068/543, 10-13=231/88, 11-13=458/1208

NOTES	FORCES	TOP CHORD	BOT CHORD	WEBS
1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 2, 684 lb uplift at joint 18 and 496 lb uplift at joint 12. 5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.	Max Gray 2=289(load case 9), 18=1688(load case 5), 12=496(load case 3)	1-2=0/54, 2-3=288/124, 3-4=271/82, 4-5=159/97, 5-6=1780/716, 6-7=1481/685, 7-8=1581/698, 8-9=1581/698, 9-10=759/385, 10-11=922/380, 11-12=1482/585	12-13=108, 13-14=610/1398, 14-15=610/1640, 15-16=720/1640, 16-17=655/1537, 17-18=655/1537, 18-19=576/2, 19-20=378/319, 20-21=1482/585	5-18=1795/694, 5-17=0/142, 5-16=1781/66, 6-16=121/513, 7-16=371/335, 7-15=171/184, 9-15=116/507, 9-13=1068/543, 10-13=231/88, 11-13=458/1208

LOAD CASE(S)	Standard
1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 2, 684 lb uplift at joint 18 and 496 lb uplift at joint 12. 5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.	Standard

LOADING (psf)	TCLL	20.0	TCLL	1.25	Plates Increase	2-0-0	CSI	0.16	TC	0.25	DEFL	in (loc)	l/defl	L/D	240	180	180	244/190	MTT20	GRIP	Weight: 84 lb
	TCDL	7.0	Lumber Increase	1.25			BC	0.22	VB	0.03	Vert(TL)	-0.02	1-5	>999	180	180	180	244/190	MTT20	GRIP	Weight: 84 lb
	BCLL	10.0	Rep Stress Incr	NO			WB	0.30	Horz(TL)	0.01	4	n/a	n/a	n/a	180	180	180	244/190	MTT20	GRIP	Weight: 84 lb
	BCDL	5.0	Code FBC2004/FP12002												180	180	180	244/190	MTT20	GRIP	Weight: 84 lb



Scale = 1:216

Builders FirstSource, Lake City, FL 32055
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Job	136166	T48	MONO TRUSS	1	Qty	JOHN JOHNSON	Dwg.#101205601
Truss Type				1	Ply		

Dwg.#101205601

REACTIONS

Max Horiz 1=183(load case 4)
Max Uplift1=651(load case 4), 4=-759(load case 4)

FORCES (1b)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1948/656, 2-3=-83/15, 3-4=-64/66
BOT CHORD 1-5=-695/1651, 4-5=-695/1651
WEBS 2-5=-668/1860, 2-4=-1958/822

NOTES

(1) 2-ply truss to be connected together with 0.131"X3"Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 cc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 cc.

2) All loads are considered equally applied to all plies, except if noted. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

3) Wind: ASCE 7-02; 110mph (3-second gust); $h=20$ ft; $TCDF=4.2$ psf
DOL=1.60 plate grip DOL=1.60.

4) Refer to girder(s) for truss to truss connections.

6) Girder carries tie-in span(s): 24-0-0 from 0-0-0 to 7-0-0

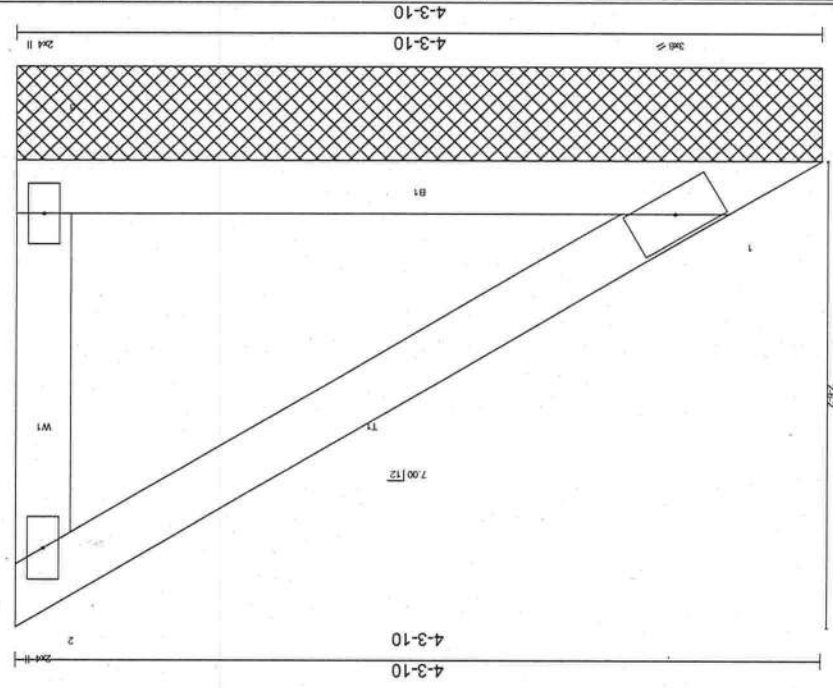
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

Vert: $1.4 = 4.86(F = 4.56)$, $1.3 = .54$

100

Job	Truss	V04	VALLEY	Truss Type	1	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205604
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Builders FirstSource, Lake City, FL 32055 6/2005 s Jul 13 2005 Mittek Industries, Inc. Mon Oct 10 18:56:03 2005 Page 1



LOADING (psf)	SPACING	DEFLECTION	BRACING	WEIGHT
TCLL 20.0	Plates Increase 1.25	DEFL in (loc)	Structural wood sheathing directly applied or 4-3-10 oc purlins, except end verticals.	Weight: 16 lb
TCDL 7.0	Lumber Increase 1.25	Vert(TL) n/a	TOP CHORD	PLATES MT20
BCLL 10.0	Rep Stress Incr YES	Vert(TL) n/a	BOT CHORD	GRIP 244/190
BCDL 5.0	Code FBC2004/TP12002	Horz(TL) 0.00		

LUMBER	RECTIONS	FORCES	NOTES
TOP CHORD 2 X 4 SYP No.2	(lb/size) 1=152/4-3-10, 3=152/4-3-10	(lb) - Maximum Compression/Maximum Tension	1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
BOT CHORD 2 X 4 SYP No.2	Max Horiz 1=97(load case 5), 3=-86(load case 5)		2) Gable requires continuous bottom chord bearing.
WEBS 2 X 4 SYP No.3			3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1 and 66 lb uplift at joint 3.

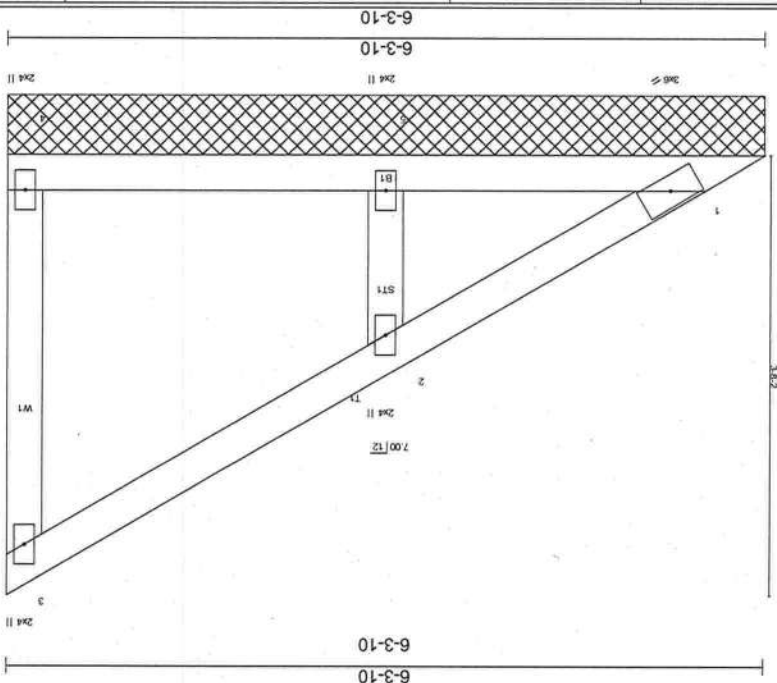
LOAD CASE(S)	Standard
1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C	
2) Gable requires continuous bottom chord bearing.	
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1 and 66 lb uplift at joint 3.	

OCTOBER 12, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L136166	Truss	V06	VALLEY	Truss Type	1	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205605
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6,200 s Jul 13 2005 Mikek Industries, Inc. Mon Oct 10 18:56:04 2005 Page 1

Scale = 1/16" = 1'



LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER
TCLL 20.0	Plates Increase 1.25	TC 0.10	in (loc)	TOP CHORD	2 X 4 SYP No.2
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL) n/a	BOT CHORD	2 X 4 SYP No.2
BCLL 10.0	Rep Stress Incr YES	WB 0.05	Horz(TL) 0.00	Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.	
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Rigid ceiling directly applied or 10'-0-0 oc bracing.	

OTHERS	WEBS	TOP CHORD	BOT CHORD
2 X 4 SYP No.3	2 X 4 SYP No.3	2 X 4 SYP No.2	2 X 4 SYP No.2

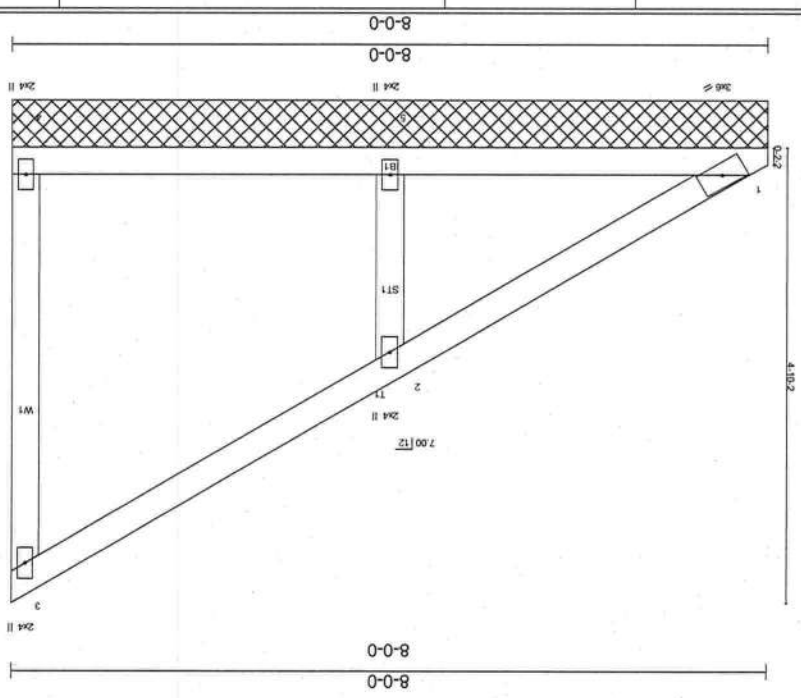
REACTIONS (lb/size) 1=78/6-3-10, 4=98/6-3-10, 5=296/6-3-10
Max Horiz 1=151 (load case 5)
Max Uplift=-56 (load case 5), 5=-167 (load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-161/46, 2-3=-50/24, 3-4=-63/69
BOT CHORD 1-5=0/0, 4-5=0/0
WEBS 2-5=-190/206

NOTES
1) Wind: ASCE 7-02: 110mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
2) Gable requires continuous bottom chord bearing.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 4 and 167 lb uplift at joint 5.

Job	Truss Type	VALLEY	Qty	1	Ply	JOHN JOHNSON	Dwg.#101205607
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LOADING (psf)	SPACING	CSI	DEFL	VERT(L)	VERT(TL)	Horz(TL)
BCDL 5.0	2'-0-0	WB 0.06	in (loc)	n/a	0.00	4
BCLL 10.0		BC 0.10	n/a	n/a	n/a	n/a
TCDL 7.0		TC 0.12	n/a	n/a	n/a	n/a
TCLL 20.0			L/d	999	999	999

LUMBER	BRACING	Weight: 35 lb
TOP CHORD 2 X 4 SYP No.2	TOP CHORD	PLATES MT20 244/190
BOT CHORD 2 X 4 SYP No.2	BOT CHORD	GRIP
WEBS 2 X 4 SYP No.3		
OTHERS 2 X 4 SYP No.3		

REACTIONS (lb/size) 1=129/8-0-0, 4=133/8-0-0, 5=378/8-0-0
 Max Horiz 1=205(load case 5)
 Max Uplift=-76(load case 5), 5=-212(load case 5)

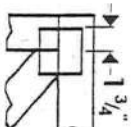
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-193/46, 2-3=-74/31, 3-4=-83/90
 BOT CHORD 1-5=-8/12, 4-5=-8/12
 WEBS 2-5=-237/243

NOTES
 1) Wind: ASCE 7-02, 110mph (3-second gust); n=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Gable requires continuous bottom chord bearing.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 4 and 212 lb uplift at joint 5.

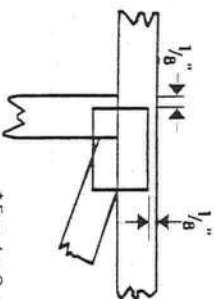
LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

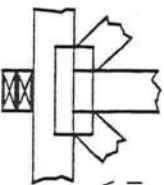
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



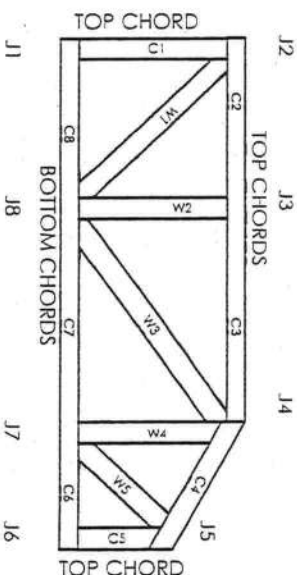
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DLHR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear lightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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Florida Pest Control & Chemical Co.

www.flapest.com

Product to be used: Bora-Care Termiticide (Wood Treatment)

Chemical to be used: 23% Disodium Octaborate Tetrahydrate

Application will be performed onto structural wood at dried-in stage of construction. Bora-Care Termiticide application shall be applied according to EPA registered label directions as stated in the Florida Building Code Section 1816.1

(Information to be provided to local building code offices prior to concrete foundation installation.)

(Address of Treatment or Lot/Block of Treatment)

City

Lake City

208 SW Bithany Place

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address:

City: Phone:

Site Location: Subdivision

Lot #

Block #

Permit #

Address:

Product used

Active Ingredient

% Concentration

☐ Premise

Imidacloprid

0.1%

☐ Termitidor

Fipronil

0.12%

☐ Bora-Care

Disodium Octaborate Tetrahydrate

23.0%

Type treatment:

☐ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

As per Florida Building Code 104.2.6 - If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line

Date

Time

Print Technician's Name

Remarks:

Applicator - White Permit File - Canary Permit Holder - Pink

10/05

©

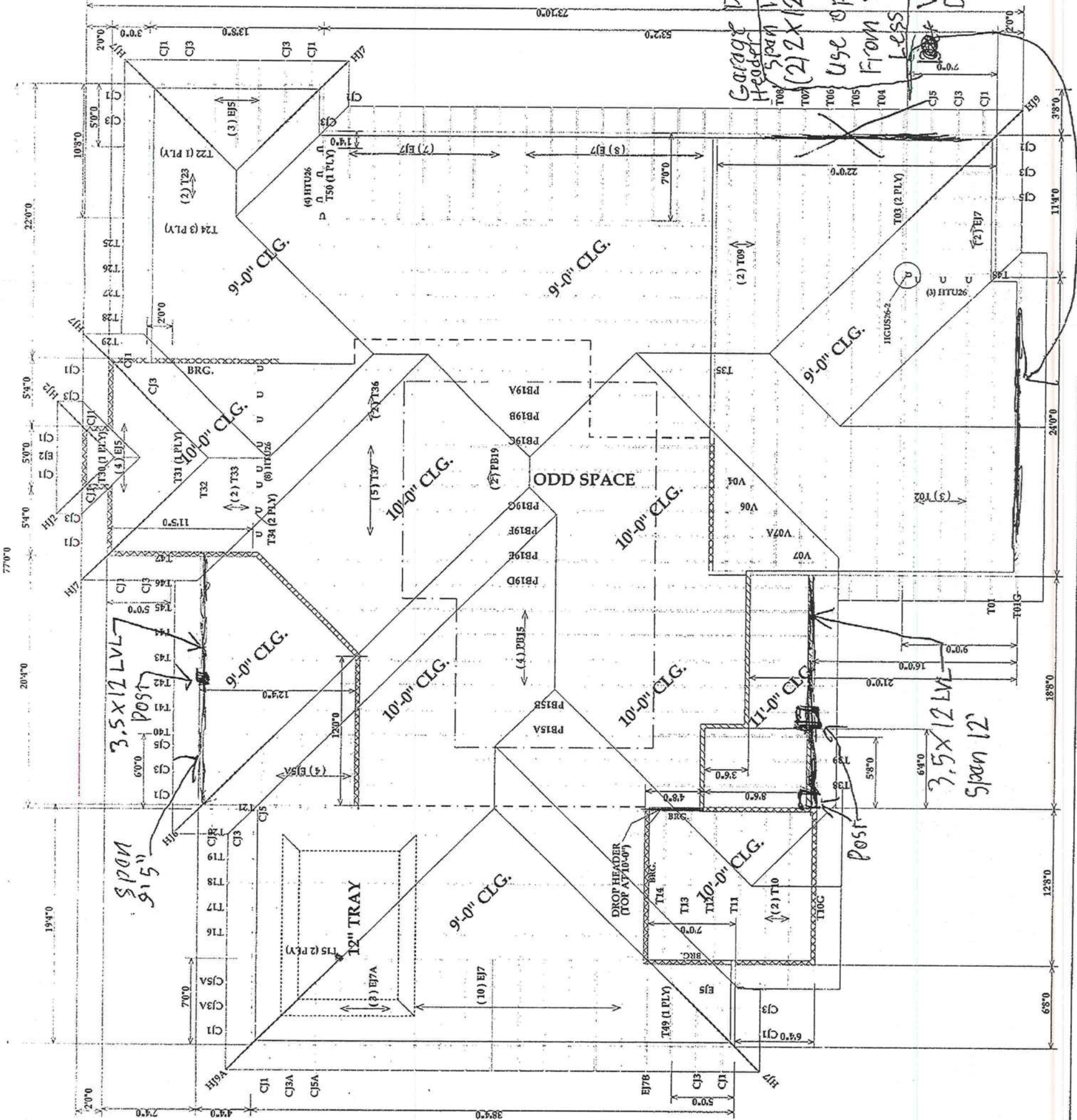
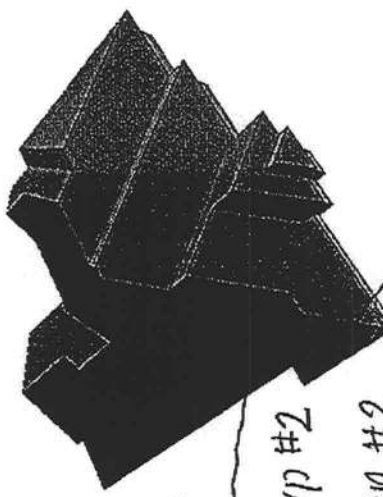
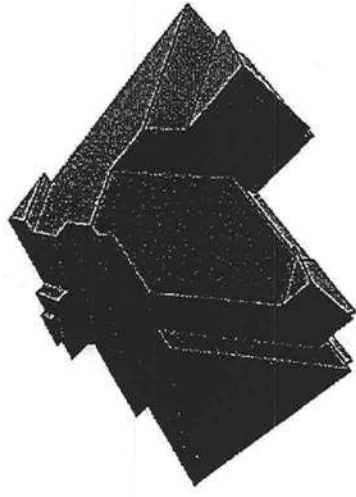
6/05

©

INTERIOR BRGS. HAVE BEEN
ADDED TO TRUSS ROOF

Application #0511-31

John Norris Owner John Johnson
LOT 6 UNIT 3 Russwood Estates



BEARING HEIGHT SCHEDULE	9'-0"
	10'-0"
	11'-0"
EXTERIOR WALL SIZE 2x4 OVERHANG ROOF PITCH(S) 7/12	
NOTES: 1) REFER TO ALL RECOMMENDATIONS FOR HANDLING, INSTALLATION, AND TEMPORARY BRACING. REFER TO ENGINEER DRAWINGS FOR PERMANENT BRACING REQUIRED. 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DEIGNED OR REFER TO DETAIL FOR ALL ALTERNATE BRACING REQUIREMENTS. 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FINISHED BY BUILDER. 4) ALL TRUSSES ARE DESIGNED FOR 2 1/2 HORIZONTAL SPACING, UNLESS OTHERWISE NOTED. 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED. 6) 5/16\"/>	
SHOP DRAWING APPROVAL THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VALLEYS. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVISIONS, AND APPROVALS OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES ARE SET. VERIFY ALL CONDITIONS TO ENSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.	
Builders FirstSource Bunnell PHONE: 904-437-3349 FAX: 904-437-3884 Jacksonville PHONE: 904-772-6100 FAX: 904-772-1973 Lake City PHONE: 904-755-6894 FAX: 904-755-7873 Sanford PHONE: 407-322-0059 FAX: 407-322-5953	
BUILDER: JOHN JOHNSON LEGAL ADDRESS: RUSSWOOD EST. LOT 6 MODEL: CUSTOM REVISION: SCALE: NTS DATE: 10/11/05 DRAWN BY: JCB BPC L136166	

HANGER SCHEDULE
(15) HTU26
(1) HGUS26-2

W-13
Detail

