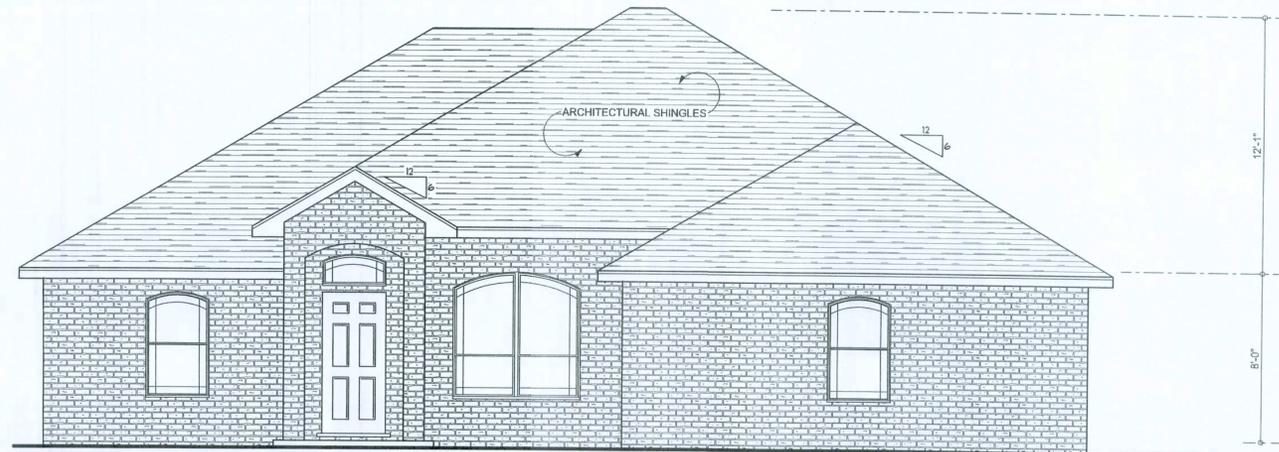


REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN TRADE



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

NOTES:
1. ALL WINDOWS HAVE PRAIRIE STYLE GRILLS
2. BUILDER IS TO VERIFY WINDOW ELEVATION OFF FINISHED FLOOR

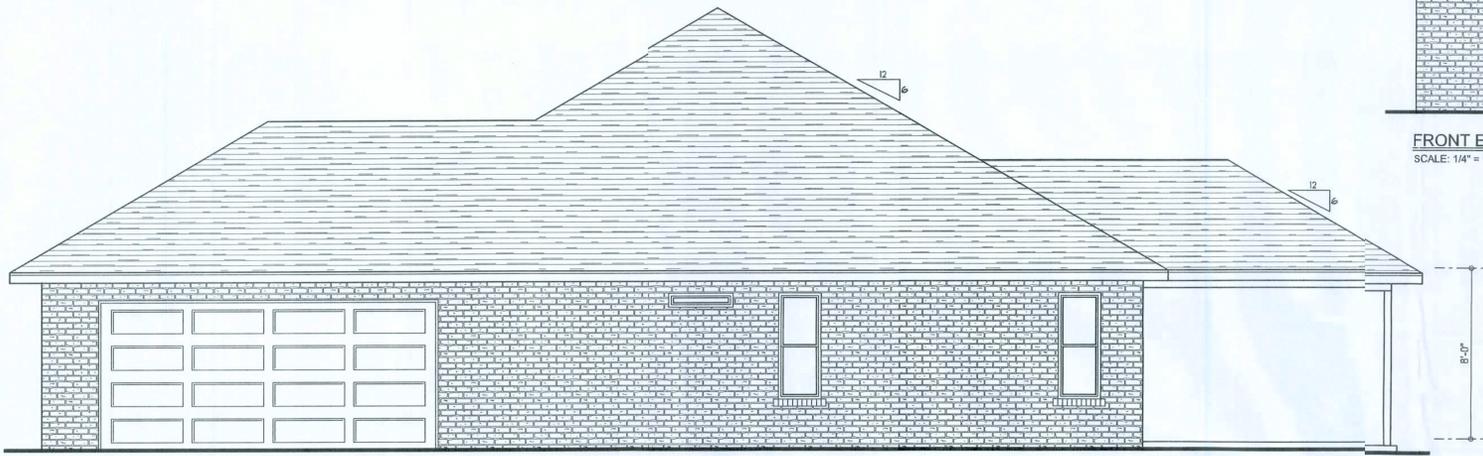
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2308.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
2356 S.F. / 300 x 50% = 3.9 S.F. RIDGE VENT AREA REQUIRED
35 FEET OF RIDGE VENT REQUIRED

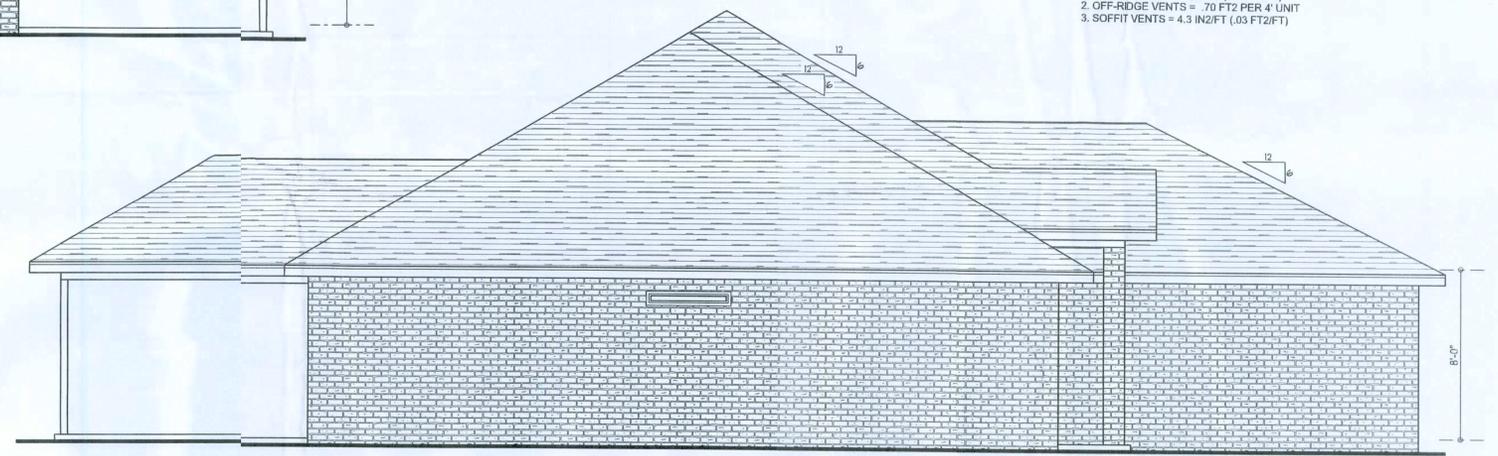
SOFFIT VENT
2356 S.F. / 300 x 50% = 3.9 S.F. SOFFIT VENT AREA REQUIRED
130 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER:
Mark Disosway, P.E.
No. 52915, POB 808, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
Mark Disosway
24 NOV 08
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DESIGNED BY:
Matthew Erkinger

FINALS DATE:
23Nov08

JOB NUMBER:
811113

DRAWING NUMBER:
A1

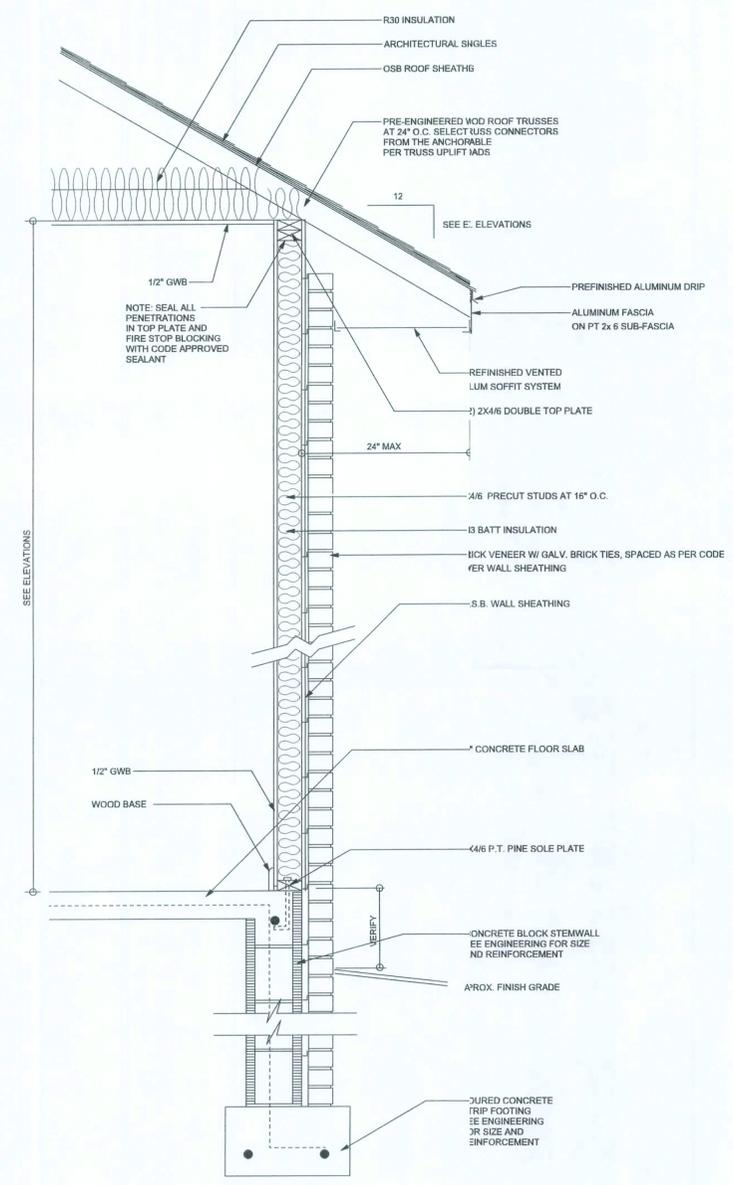
OF 6 SHEETS



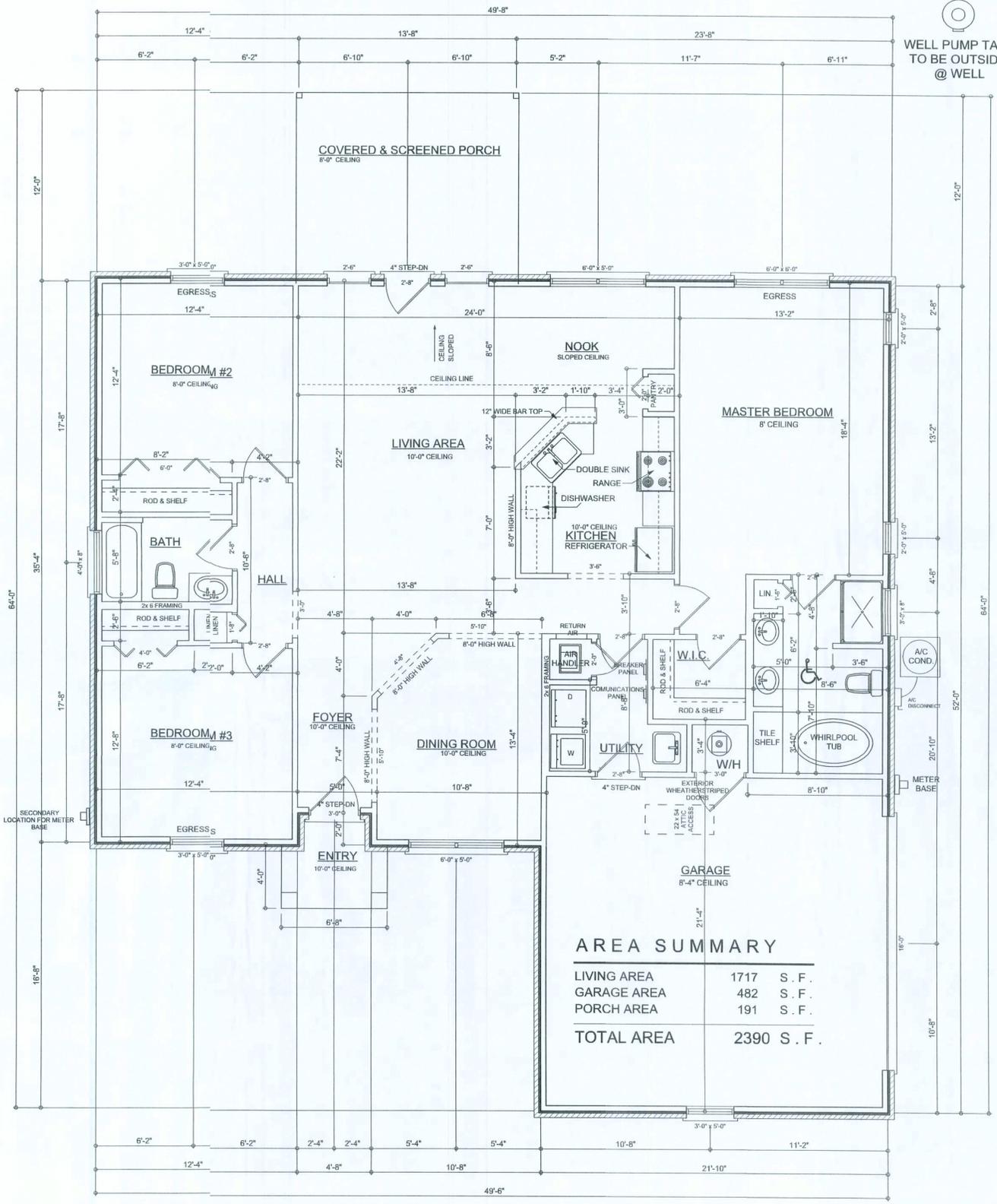
REVISIONS	



WELL PUMP TANK
TO BE OUTSIDE
@ WELL



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'-0"



FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY

LIVING AREA	1717	S . F .
GARAGE AREA	482	S . F .
PORCH AREA	191	S . F .
TOTAL AREA	2390	S . F .

WINDLOAD ENGINEER:
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DIMENSIONS:
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DESIGNED BY:
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FINALES DATE:
23Nov08

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811111

DRAWING NUMBER
A2

OF 6 SHEETS

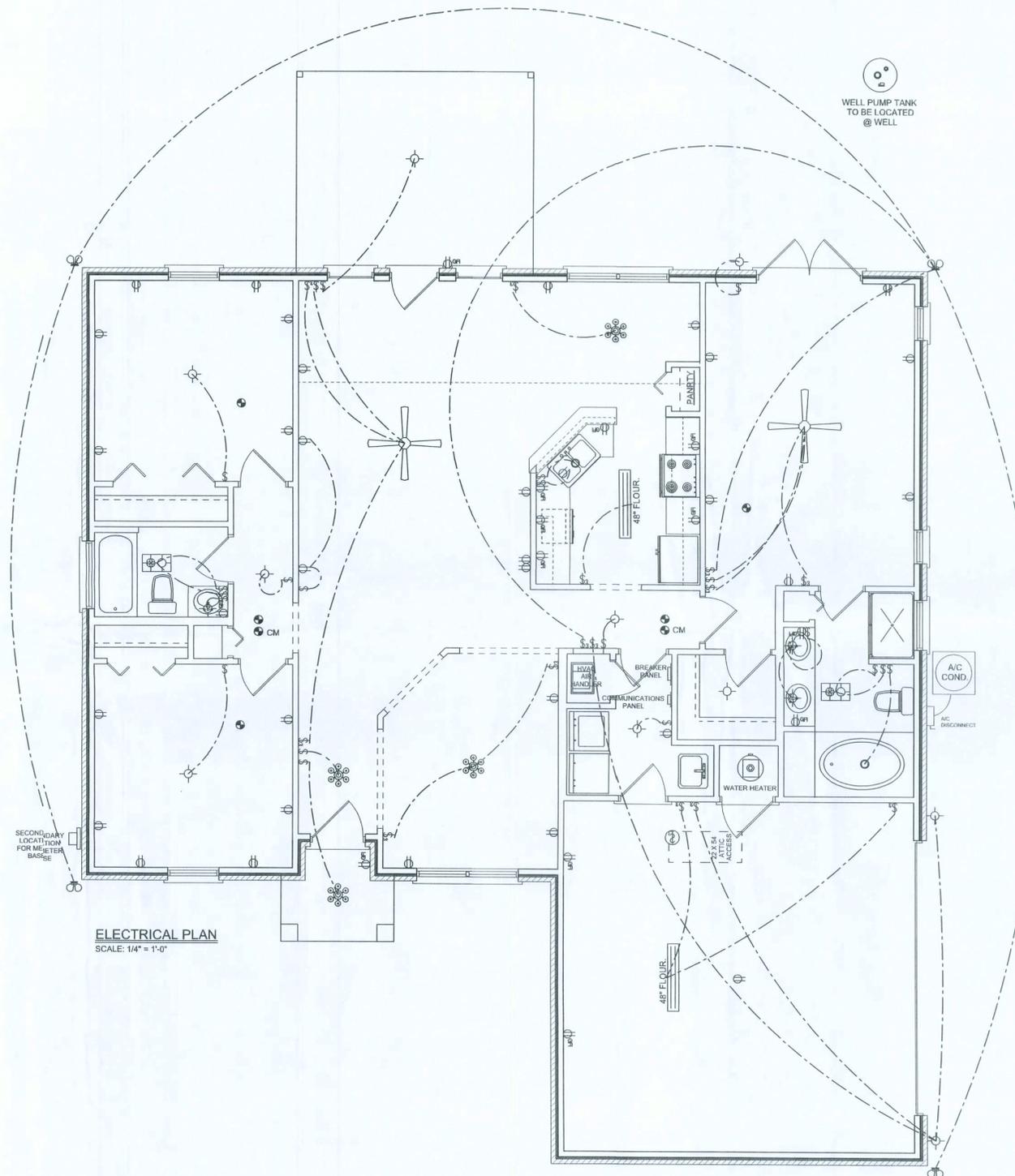
REVISIONS	

SOFTPLAN
ARCHITECTURAL RENDERING

ELECTRICAL LEGED	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2x4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPRATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NATL. ELECTR. CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHA BE INTERLOCKED TOGETHER. INSTALL INSIDE AN NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAE DEVICES OR OUTLETS SHALL BE AS PER THE OWER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 CARBON MONOXIDE ALARMS SHALL BE REQUIRE WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, FIREPLACE, OR ATTACHED GARAGE.



WINDLOAD ENGINEER:
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DIMENSIONS:
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Mark Disoway
24 NOV 08
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Erkinger Builders

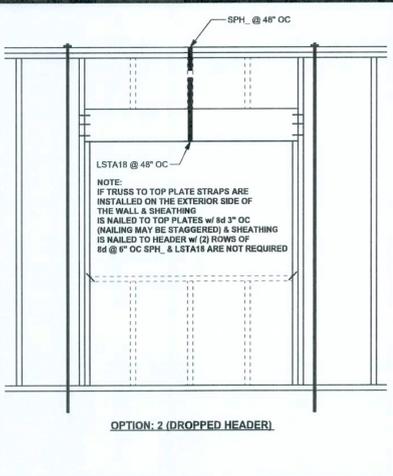
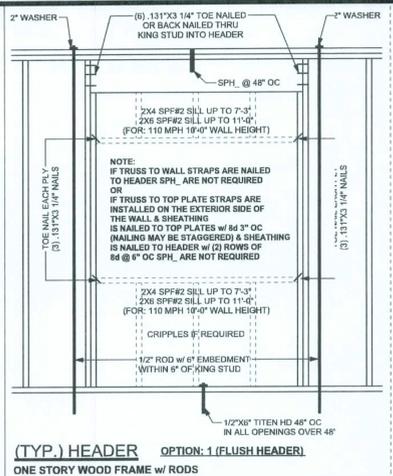
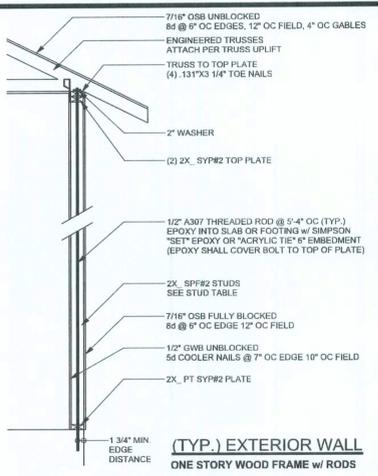
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PRINTED DATE:
November 23 2008

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DESIGNED BY: <i>Matthew Erkinger</i>	
FINALES DATE: 23 Nov 08	
JOB NUMBER: 81113	
DRAWING NUMBER A3	
OF 6 SHEETS	



ANCHOR TABLE
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	285	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12-LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12-MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								
LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LG14-SDS3	4080	3890	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MGT	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10380	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					5-10d x 1 1/2"	5-10d x 1 1/2"
SPH4	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
SP6	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
SPH6	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
LSTA18	1235	1110					14-10d	14-10d
LSTA21	1235	1235					16-10d	16-10d
CS20	1030	1030					14-10d	14-10d
CS16	1705	1705					22-10d	22-10d
STUD ANCHORS								
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT31	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
H2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HT16	4175	3695					32-16d	5/8" ANCHOR
HT22	5260	5250					12-16d	5/8" ANCHOR
ABU44	2300	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

EXTERIOR WALL STUD TABLE
FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-0" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADS. EXAMPLE 16" OC x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PVL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X4 RAFTERS WITH MIN UPLIFT CONNECTION 4x8x8 EACH END, 2X4 RAFTERS TO EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCEMENT SLAB: 6" x 6" W1.4 x W1.4. FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB OR GIRDER SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/RA OR REINFORCING STEEL. CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK AT A GIVEN LINE.

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPICES 48" DB (25" FOR 5BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-06, U.N.O.

GULLAM BEAMS: GLB, 24F-V3SP, F_b = 2400, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (1.11) 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

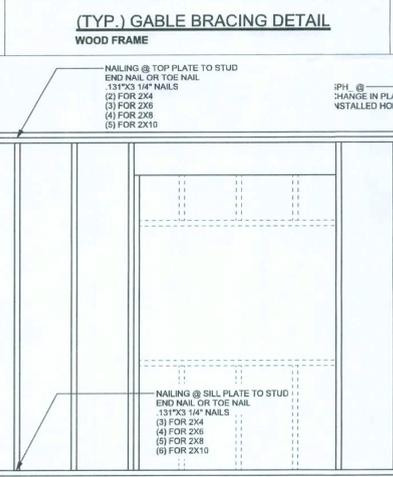
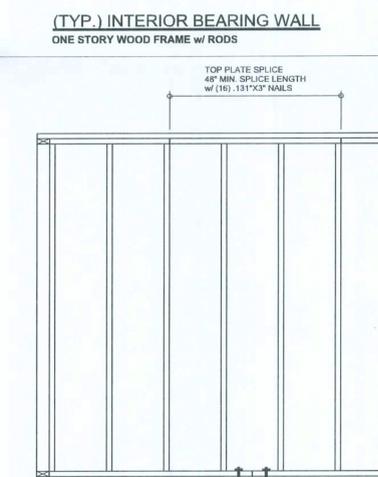
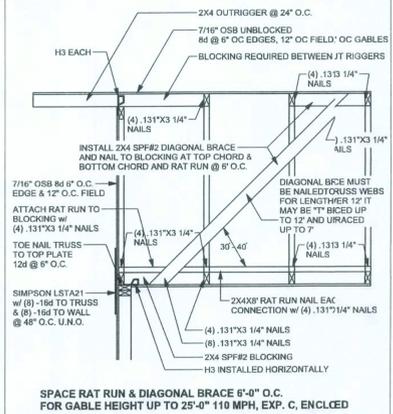
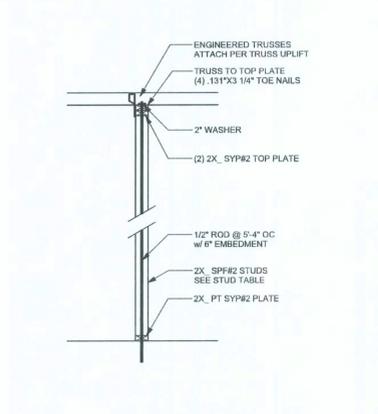
STRUCTURAL CONNECTORS: MANUFACTURER'S AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUDED CMU.

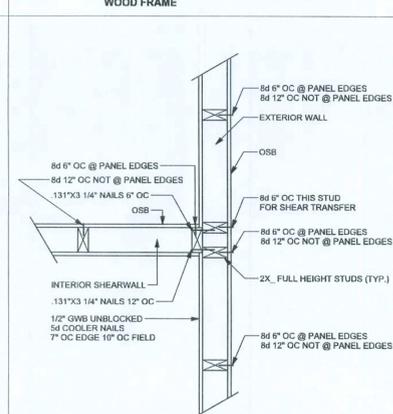
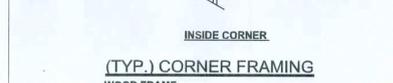
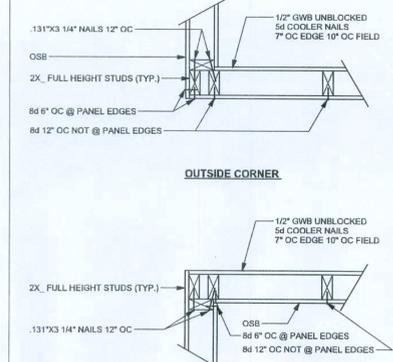
WASHERS: WASHERS USED WITH 1/2" BOLT TO BE 2" x 2" x 9/16". WITH 5/8" BOLTS TO BE 2" x 2" x 3/4". WITH 3/4" BOLTS TO BE 3" x 3" x 5/16". UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

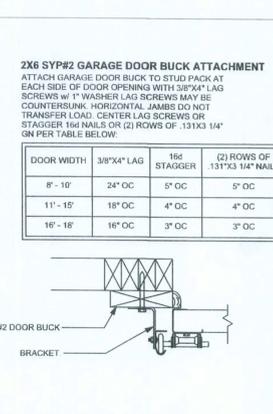
REVISIONS



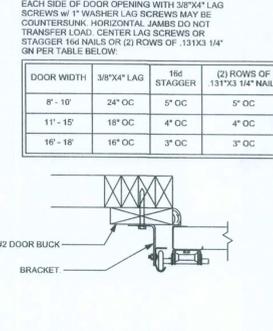
ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL
ONE STORY WOOD FRAME w/ RODS



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.



2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT



ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS. TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS ENGINEER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS. MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT. NOT ON UPPER HALF OF HILL OR ESCARPMENT 6 FT IN EXP. & 3 FT IN EXP. AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50' HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = I
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	15	20
1	19.9	21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 On	40.6			-40.6
3	19.9	25.5	18.1	-21.8
3 On	46.3			-42.4
4	21.8	23.0	18.5	-20.4
5	21.8	-29.1	18.5	-22.8
Doors & Windows		21.8	-29.1	
Wind Case (Zone 5, 10 ft)				
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3-12)
ROOF	20 PSF (FLAT OR <4-12)
	16 PSF (4-12 TO <12-12)
	12 PSF (12-12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER
Mark Disoway, P.E.
No. 53915, POB 608, Lake City, FL 32056,
386-754-5419

BUILDER'S RESPONSIBILITY
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

WINDLOAD ENGINEER
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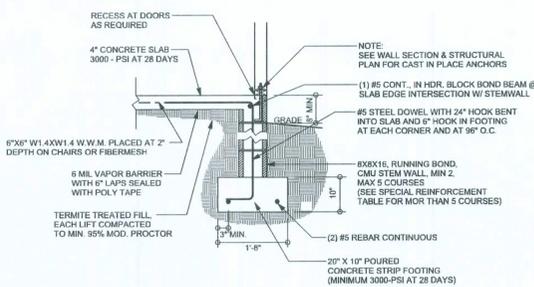
CERTIFICATION: I hereby certify that I have examined this plan, and that to the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

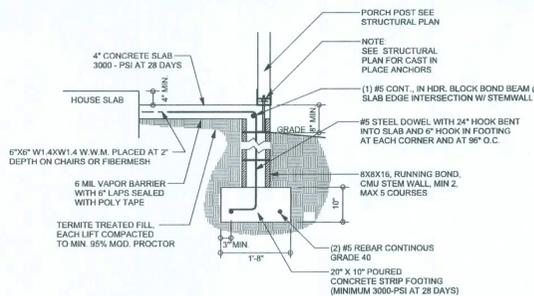
MARK DISOWAY
P.E. 53915
24 NOV 08
SEAL

Erkinger Builders

REVISIONS	



F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

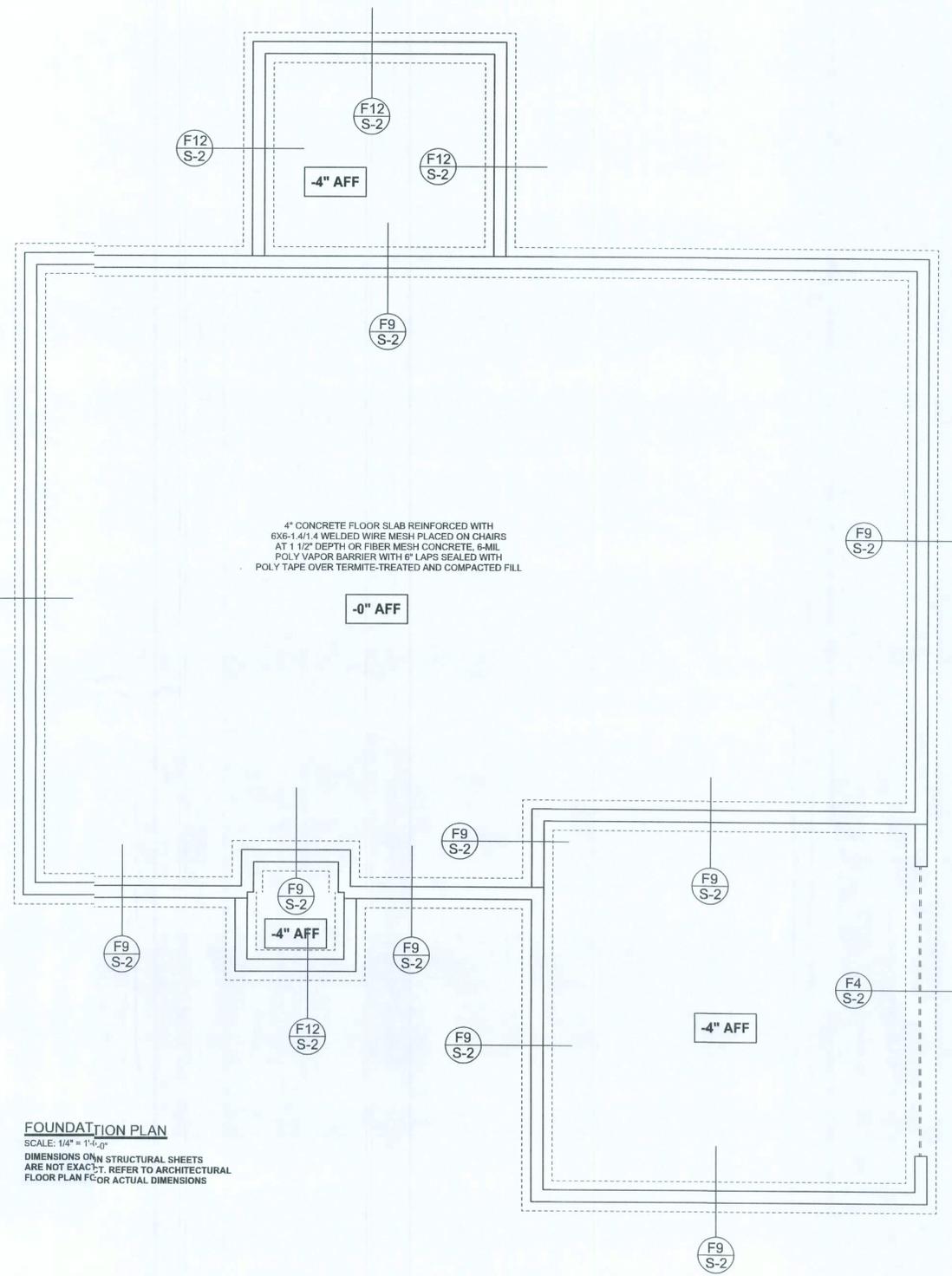


F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

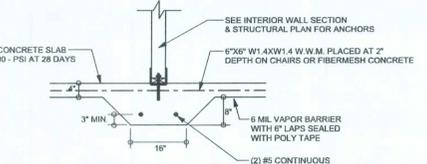
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. A vertical steel is to be placed toward the tension side of the CMU wall away from the pressure, within 2" of the exterior side of the wall. If the wall is over 8' high, add Diaphragm reinforcement at 16" O.C. vertically or a horizontal bond beam with 1#5 continuous mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

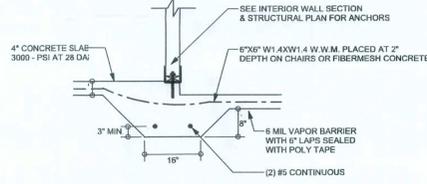
STEM WALL HEIGHT (FEET)	UNBALANCE BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



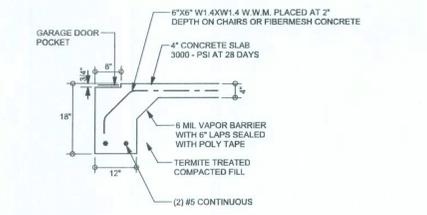
FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER:
Mark Disosway, P.E.
No. 53915, PCB 858, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, including, but not limited to, the engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

Mark Disosway
24 Nov 08
SEAL

Erkinger Builders

Mamie L. James
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PRINTED DATE:
November 23, 2008

DRAWN BY: STRUCTURAL BY:
Javis Disosway

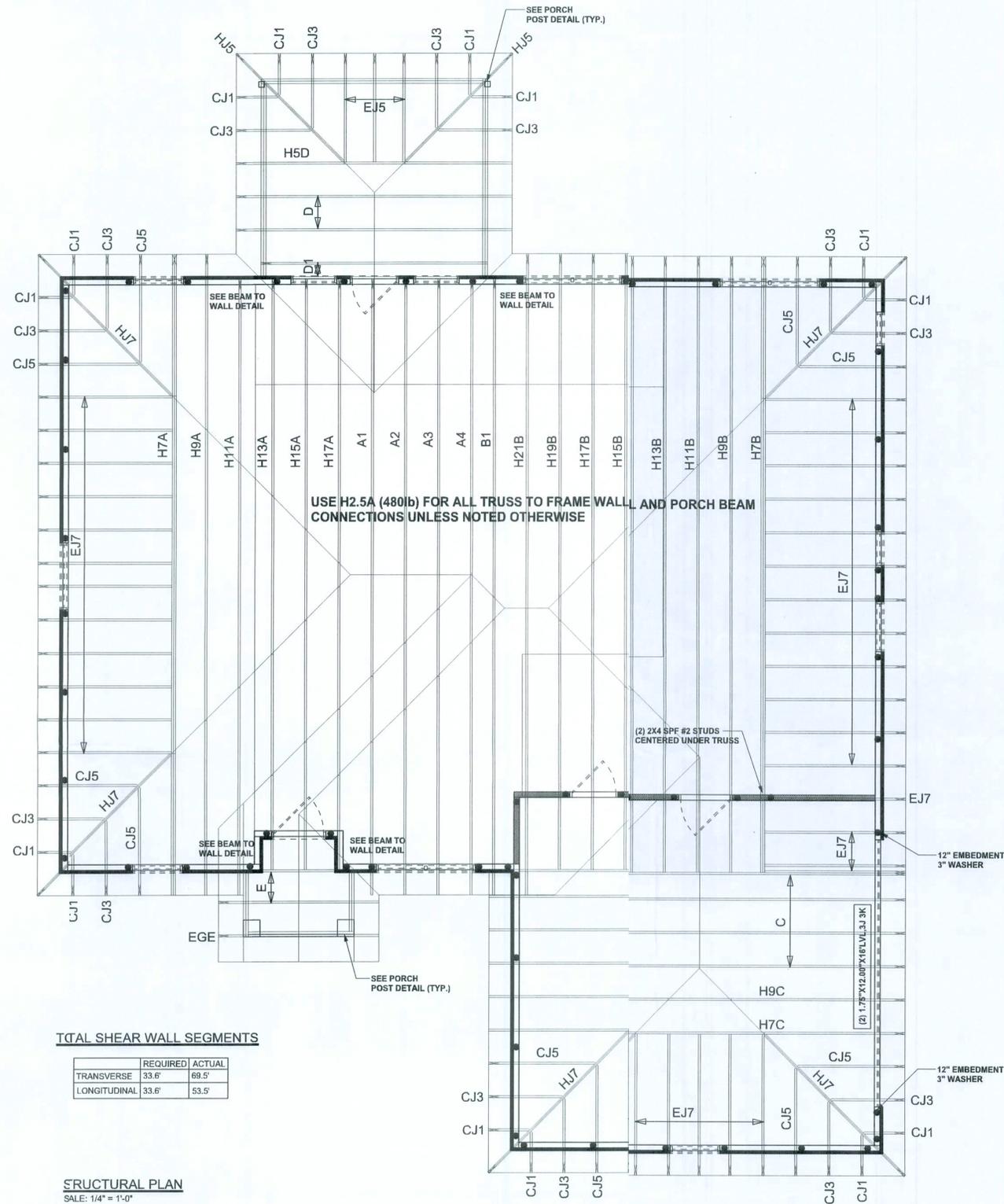
FINALS DATE:
23 Nov 08

JOB NUMBER:
81113

DRAWING NUMBER
S-2
OF 6 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



TOTAL SHEAR WALL SEGMENTS

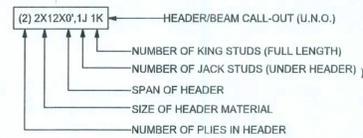
	REQUIRED	ACTUAL
TRANSVERSE	33.6'	69.5'
LONGITUDINAL	33.6'	53.5'

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

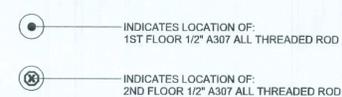
STRUCTURAL PLAN NOTES

- SI-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SI-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SI-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT, REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SI-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03. BCSH-81, BCSH-82, & BCSH-83. BCSH-81, BCSH-82, & BCSH-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



THREADED ROD LEGEND



WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB # 8-270

WINDLOAD ENGINEER:
Mark Disoway, P.E.
No. 53915, PCB 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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24NOV08
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PRINTED DATE:
November 23, 2008

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David Disoway

FINALS DATE:
23Nov08

JOB NUMBER:
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DRAWING NUMBER
S-3

OF 6 SHEETS