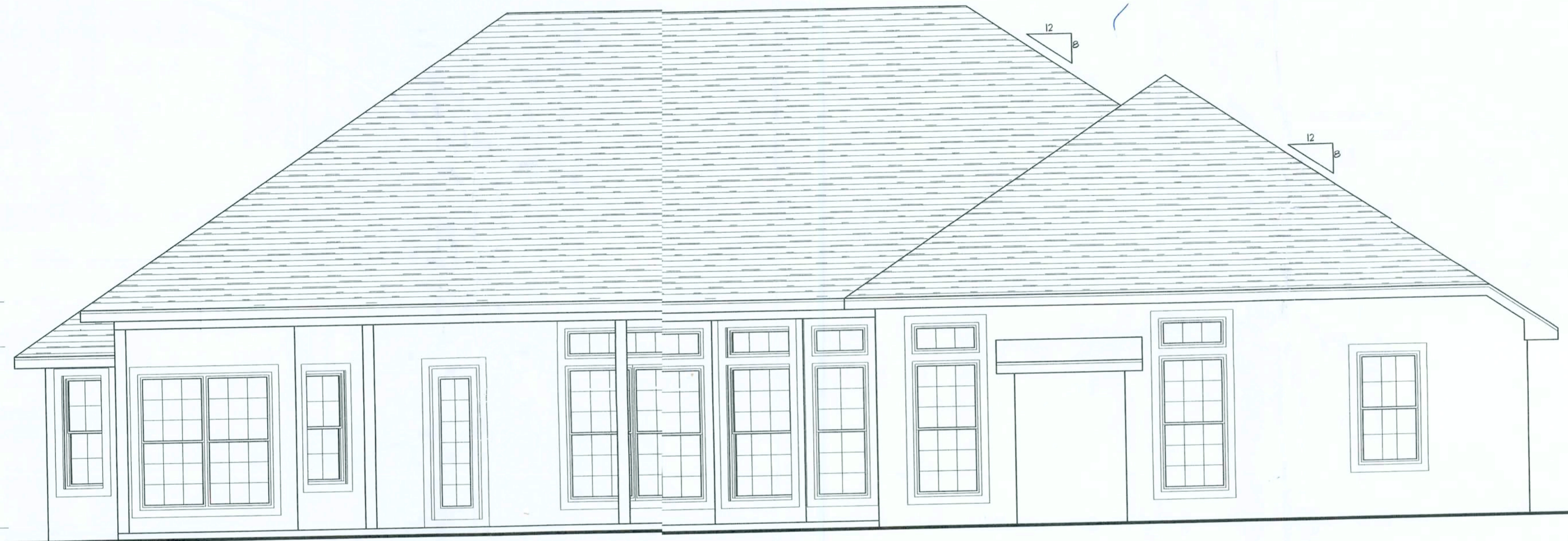


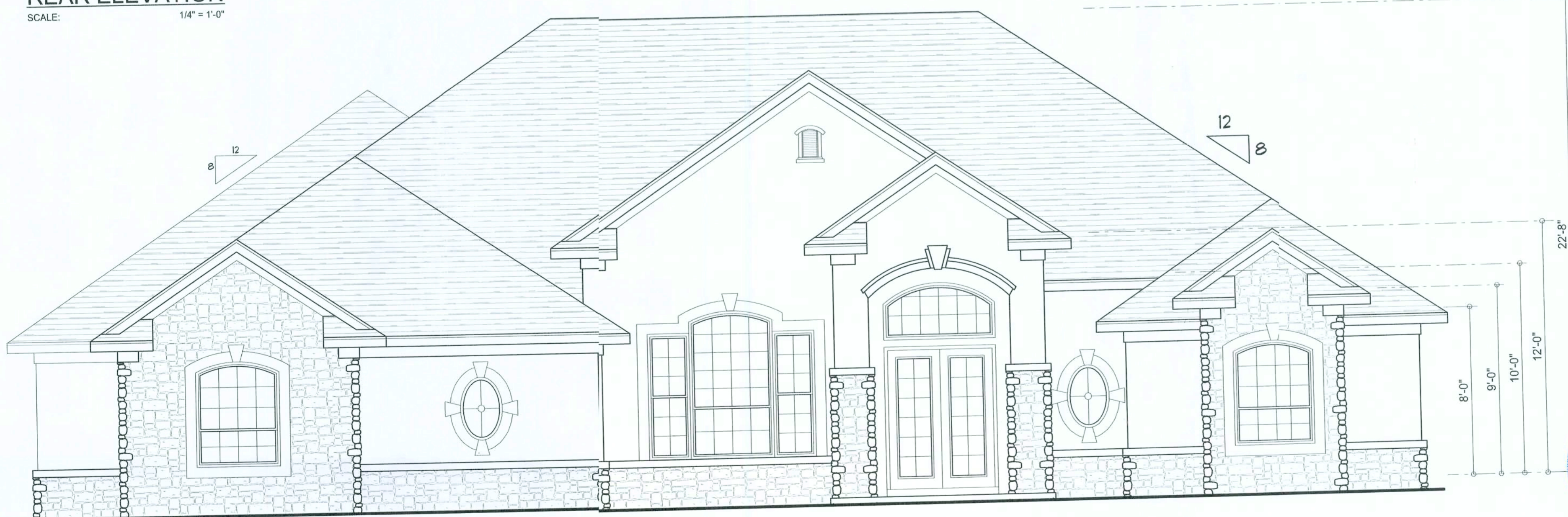
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LEFT ELEVATION
SCALE: 3/16" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

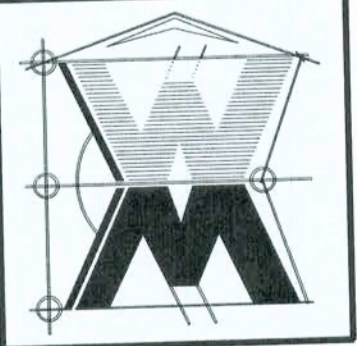
REVISIONS
August 07, 2006

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"
TYPICAL WALL SECTION
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
ROBERT & STEPHANIE MIELE
PROJECT ADDRESS: LOT 54, EMERALD COVE, COLUMBIA COUNTY, FLORIDA 32025
ISAAC CONSTRUCTION, LLC.
2108 US HWY 90 W. Ste. #170 PMB 338 Lake City, Fl. 32055

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(386) 758-8406
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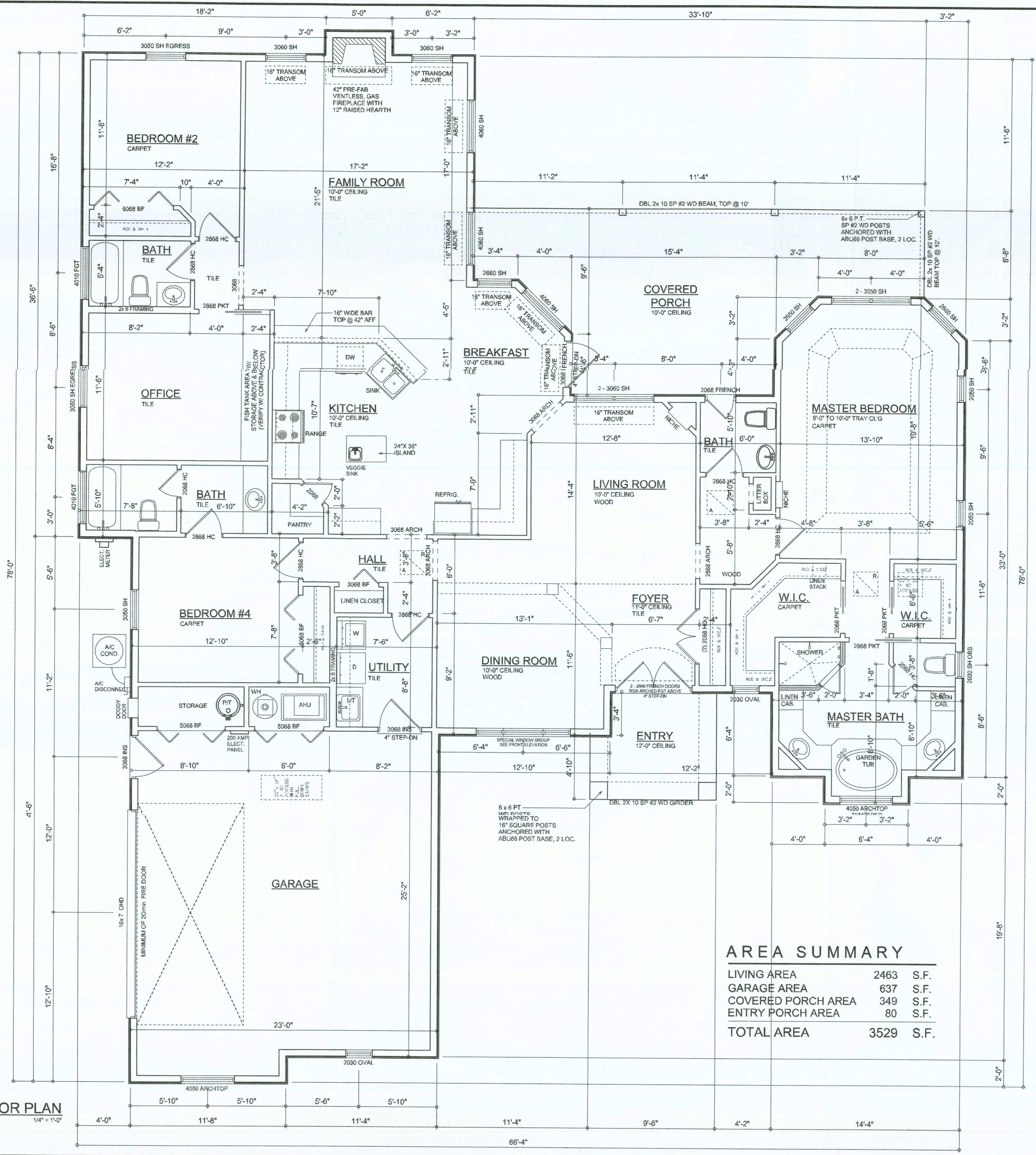
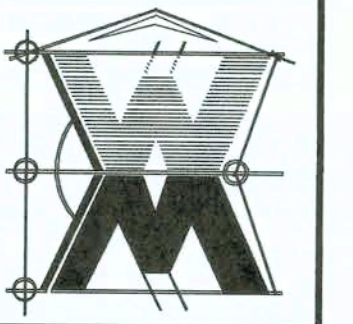


JOB NUMBER
060801

SHEET NUMBER
A.1
OF 3 SHEETS



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AREA SUMMARY

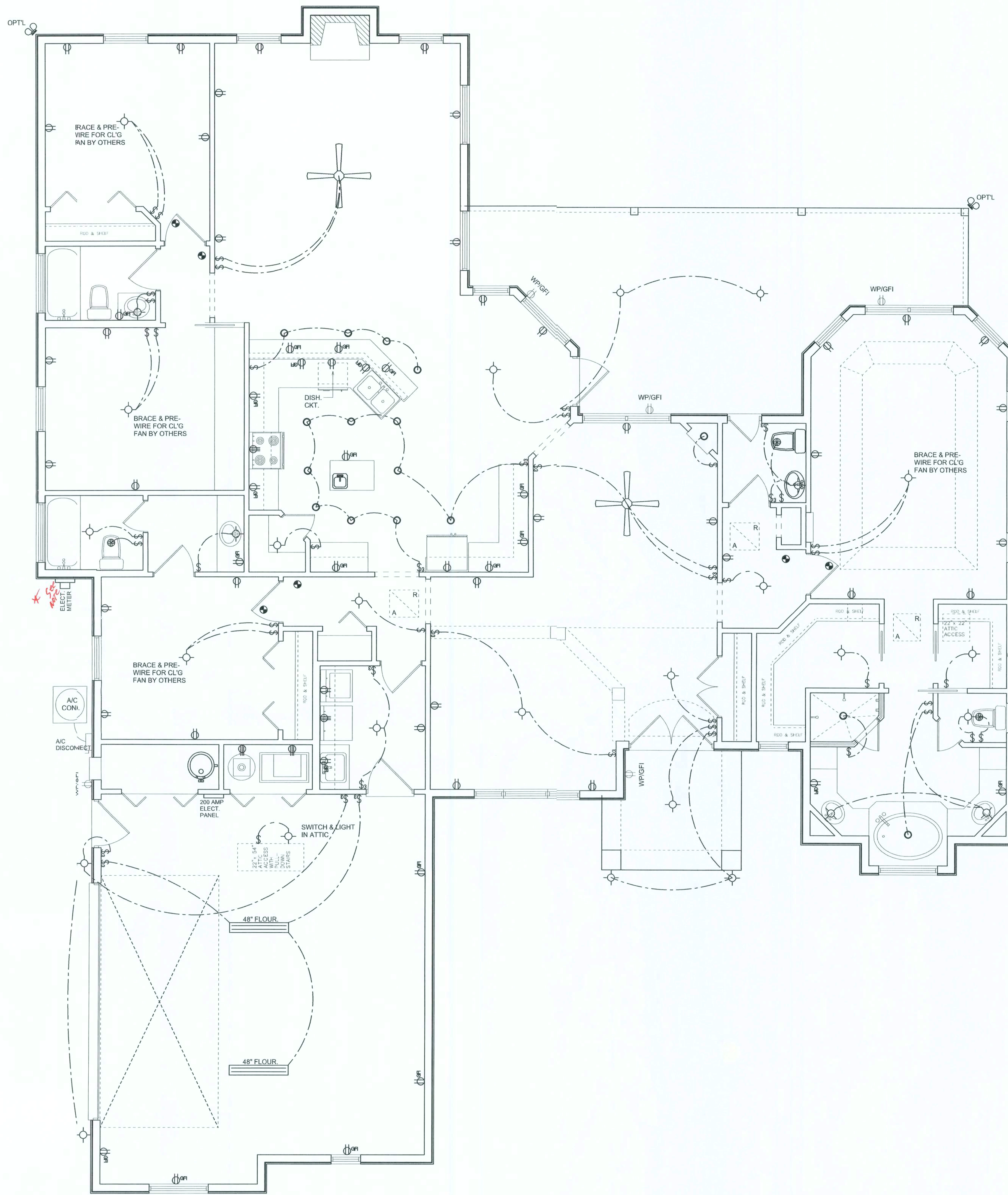
LIVING AREA	2463	S.F.
GARAGE AREA	637	S.F.
COVERED PORCH AREA	349	S.F.
ENTRY PORCH AREA	80	S.F.
TOTAL AREA	3529	S.F.

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

FLOOR PLAN
SCALE: 1/4" = 1'-0"

William Myers



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.
* THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

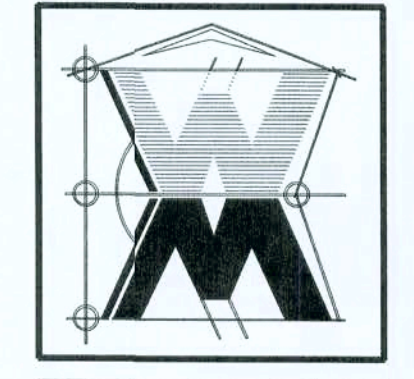
REVISIONS
August 07, 2006

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
ROBERT & STEPHANIE MIELE
PROJECT ADDRESS: LOT 54, EMERALD COVE, COLUMBIA COUNTY, FLORIDA 32025
ISAAC CONSTRUCTION, LLC.
2109 US HWY 90 W. Ste. #170 PMB 338 Lake City, FL 32055

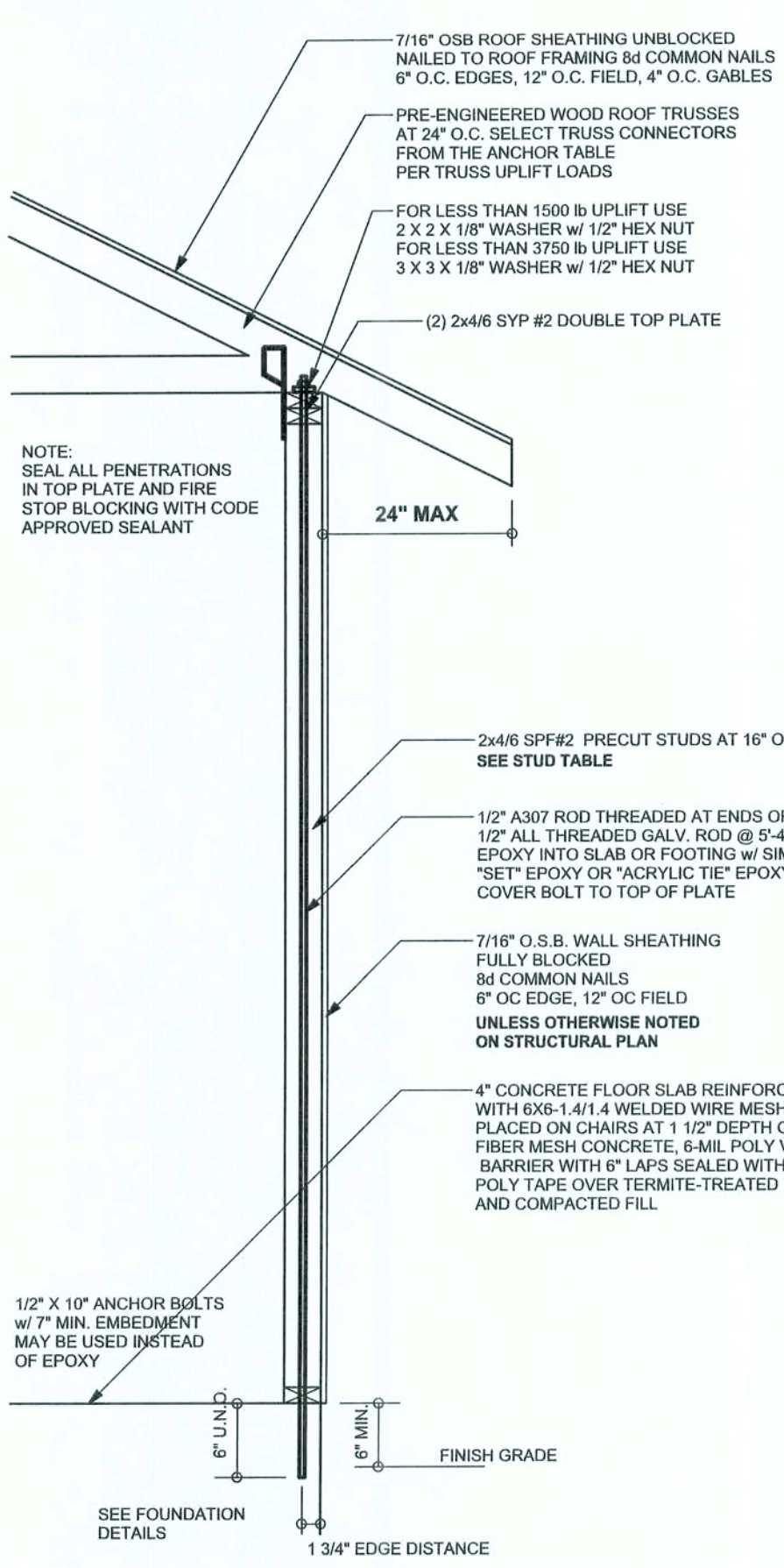
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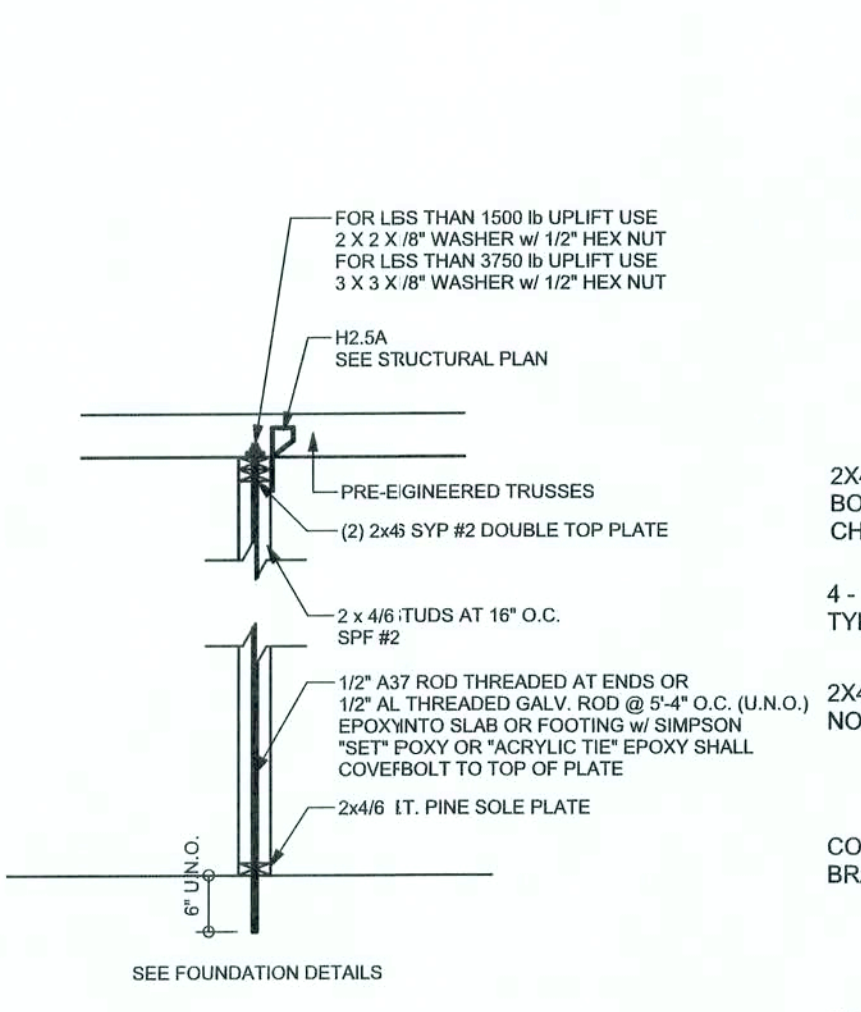
JO3 NUMBER
060801

SHEET NUMBER
A.3
OF 3 SHEETS

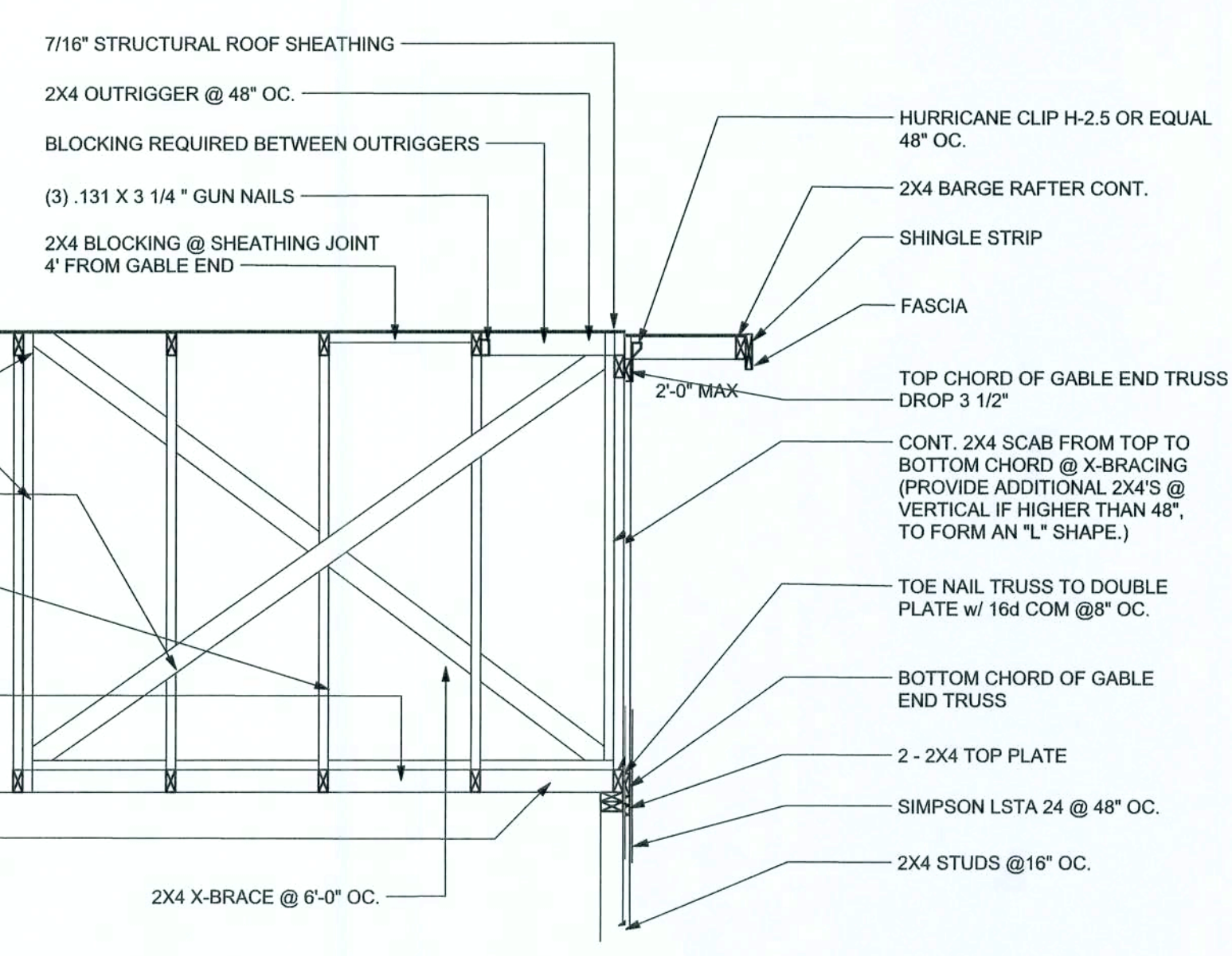
Will Myers



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



TYPICAL GABLE END (X-BRACING)
ALL MEMBERS SHALL BE SYP

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-16d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-16d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG12	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT18	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-18d		1/2" AB
< 2300	< 2300	ABU66	12-18d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415 LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB + 8KSL. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C1116. SUPPLIER TO PROVIDE ASTM C1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/ M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, $f_y = 60$ KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4ksf$, $E = 1800ksf$, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOF ROOFING HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED. FASTENED WITH COMMON NAILS (13D), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURER AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BED OR 12" IN CRACKED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/8"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/16"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/16".

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

REVISIONS

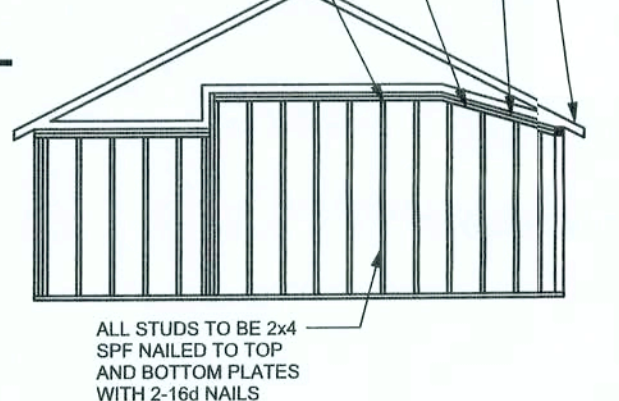
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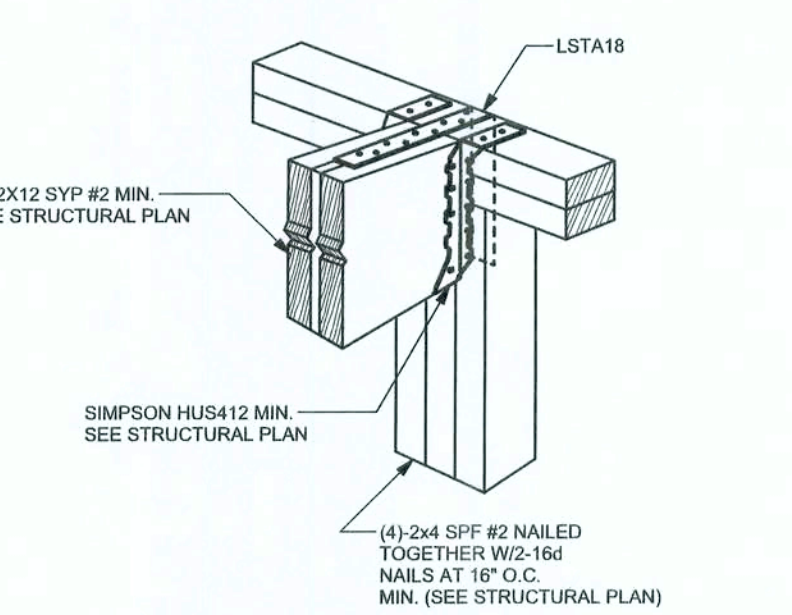
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

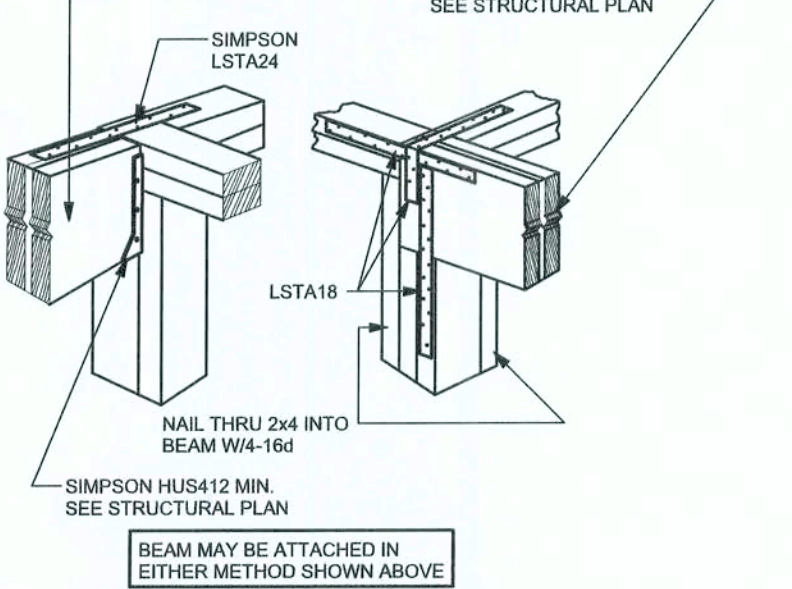
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.



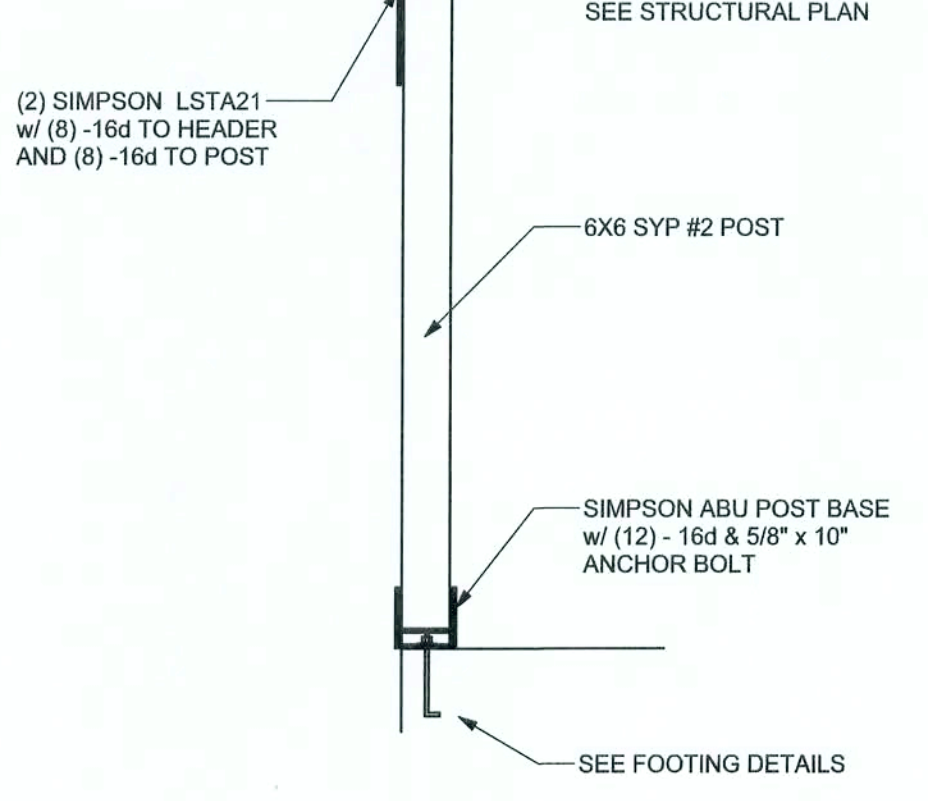
BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

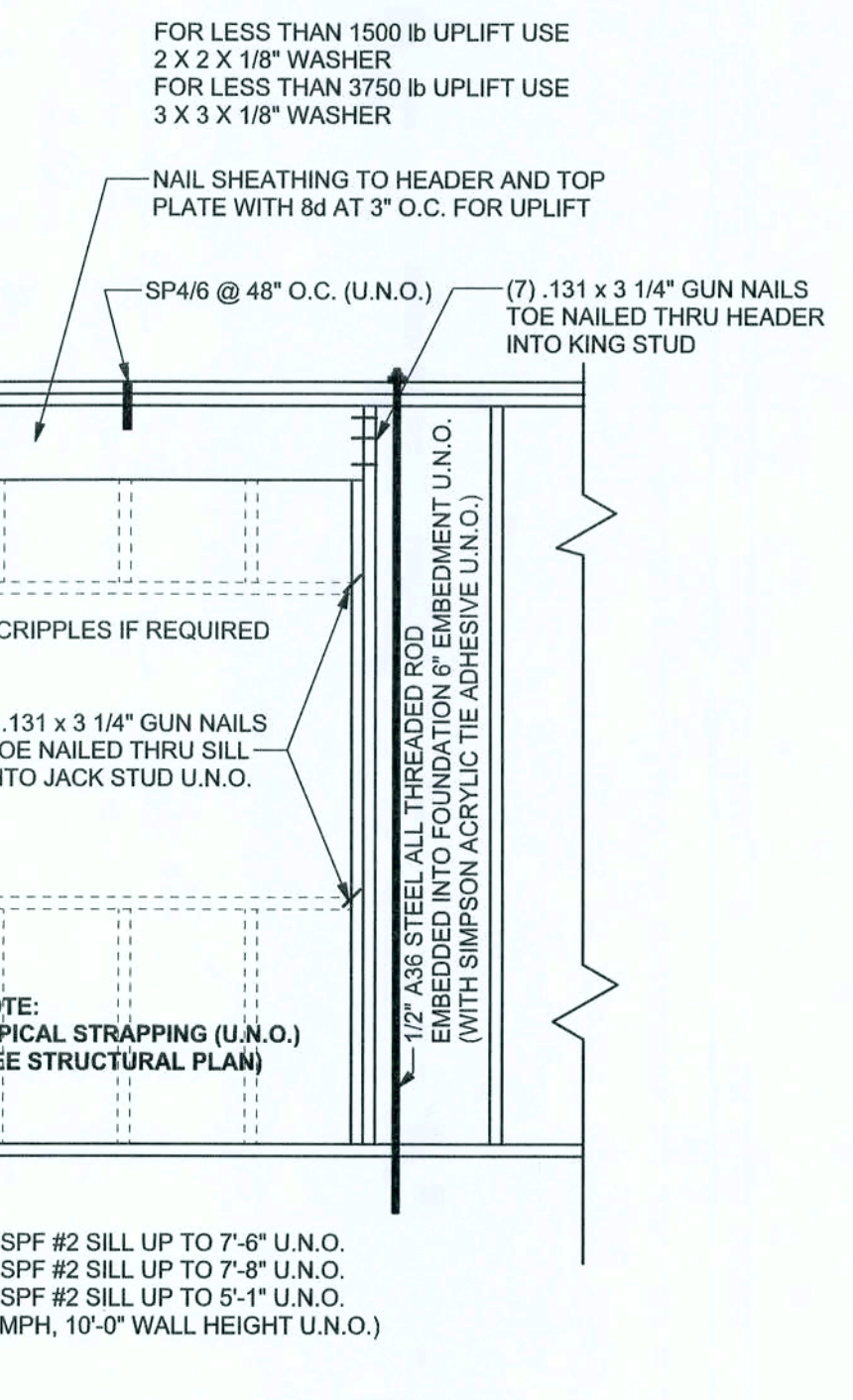


SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

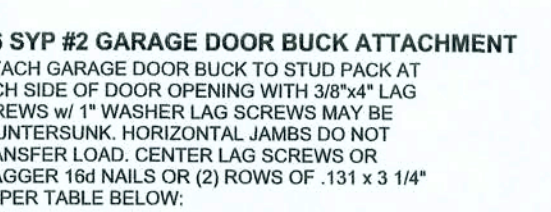
NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SP4/6 @ 48" O.C. ARE NOT REQUIRED



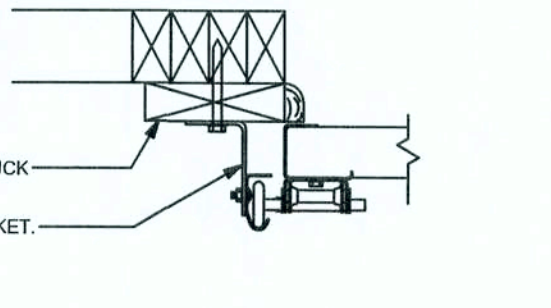
TYPICAL 1 STORY HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200, 1.6
2x10	SYP #2	1050, 1.6
2x12	SYP #2	975, 1.6
GLB	24F-V3 SP	2400, 1.8
LSL	TIMBERSTRAND	1700, 1.7
LVL	MICROLAM	2900, 2.0
PSL	PARALAM	2900, 2.0



DOOR WIDTH	3/8" x 4" LAG STAGGER	(2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:
8' - 10'	24" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1) BASIC WIND SPEED = 110 MPH
2) WIND EXPOSURE = B
3) WIND IMPORTANCE FACTOR = 1.0
4) BUILDING CATEGORY = II
5) ROOF ANGLE = 10-45 DEGREES
6) MEAN ROOF HEIGHT = <30 FT
7) INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)
8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	16.1	-21.8
2 Other	40.6	-	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Other	-88.3	-	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows	21.8	-29.1	
Worst Case (Zone 5, 10 ft ²)			
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3.12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53916, F08 868, Lake City, FL 32056, 386-7545419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53916
Mark Disoway
16 AUG 06
SEAL

Isaac Construction
Robert & Stephanie Mice Residence

ADDRESS:
Lot 54 Emerald Cove S/D
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
August 16, 2006

DRAWN BY: David Disoway

STRUCTURAL BY: David Disoway

FINALS DATE:
16 / Aug / 16

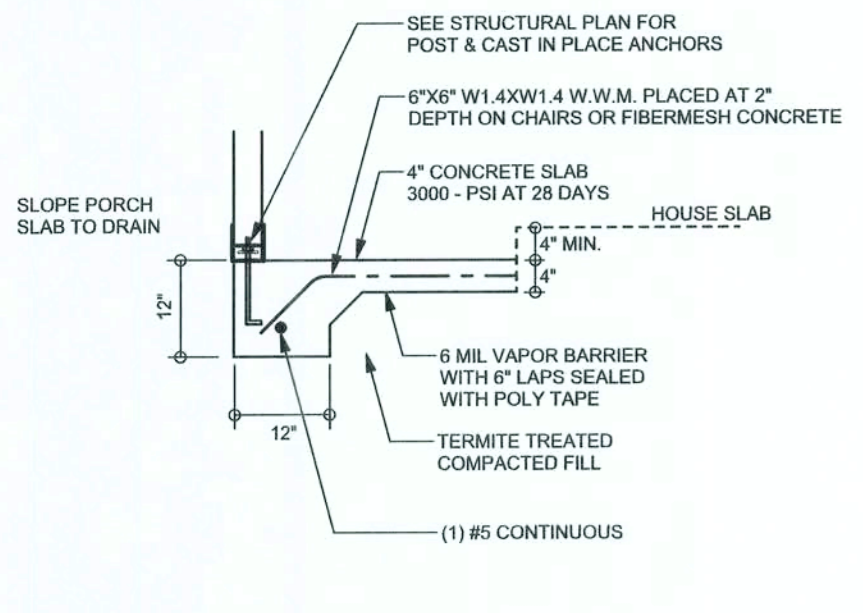
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DRAWING NUMBER

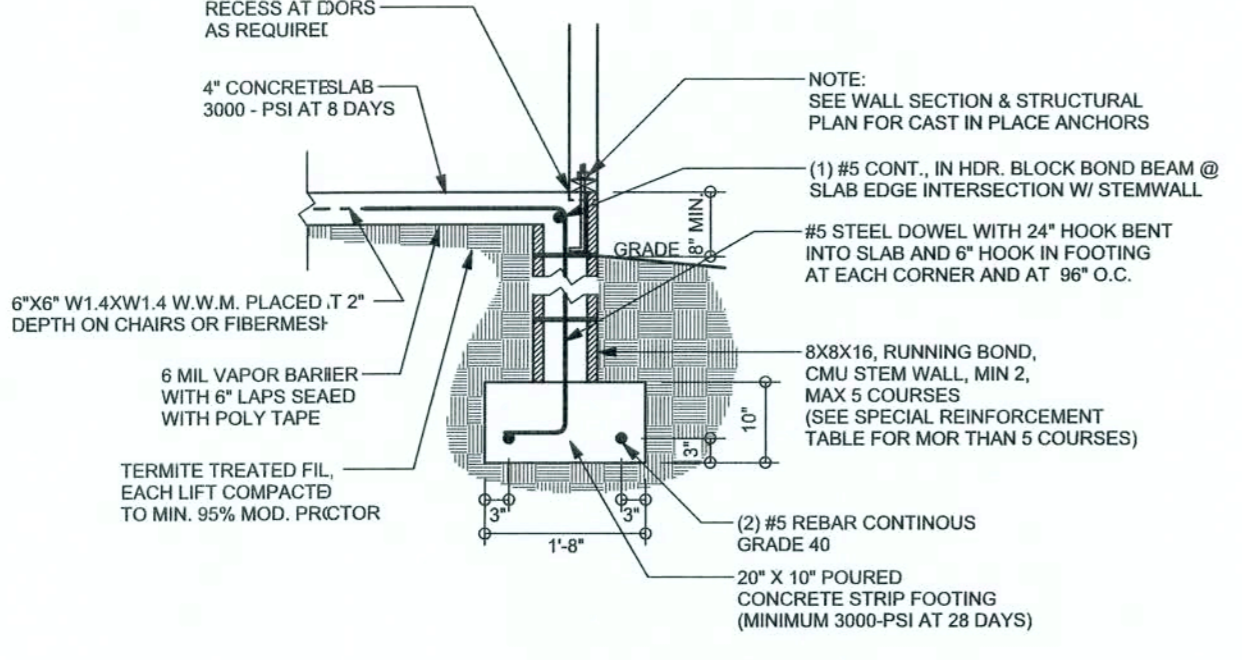
S-1
OF 3 SHEETS

REVISIONS	

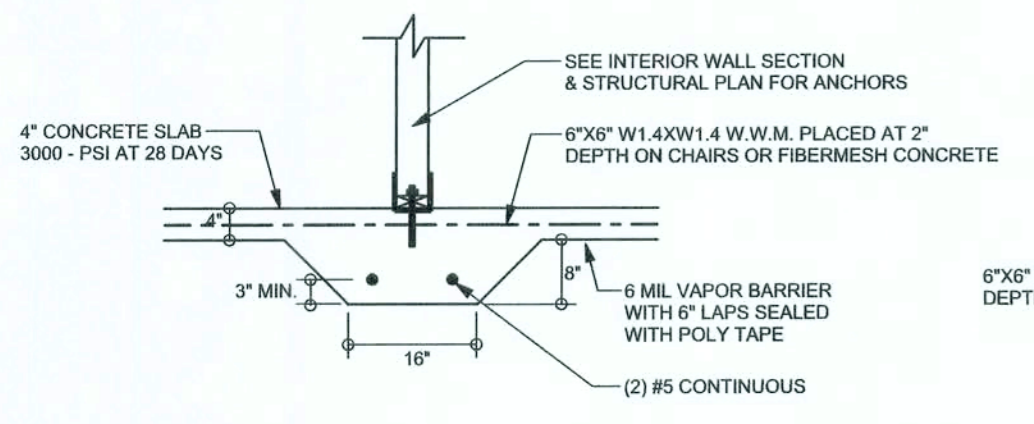
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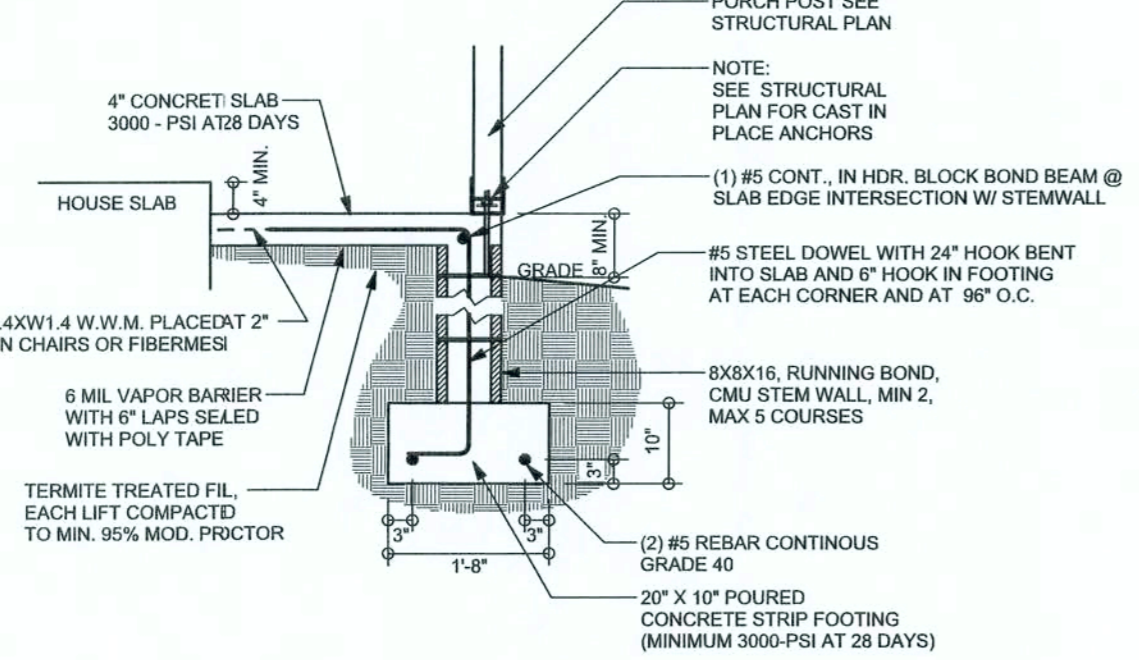
F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



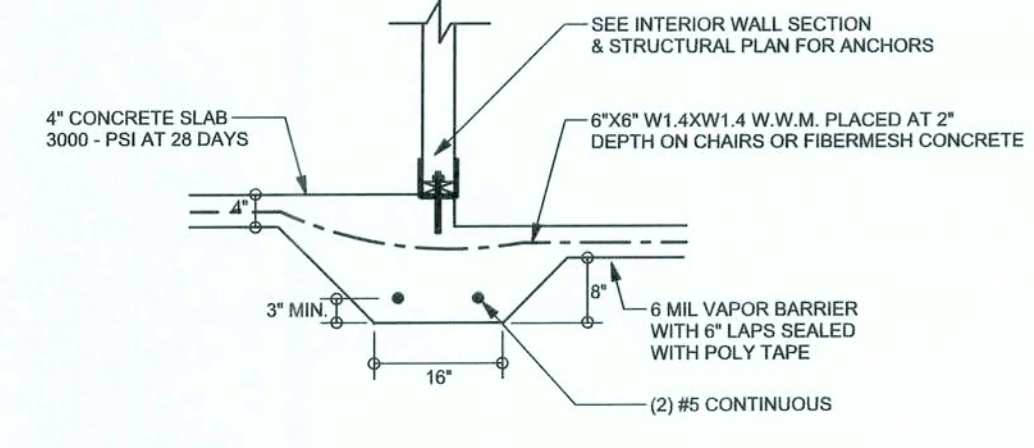
F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



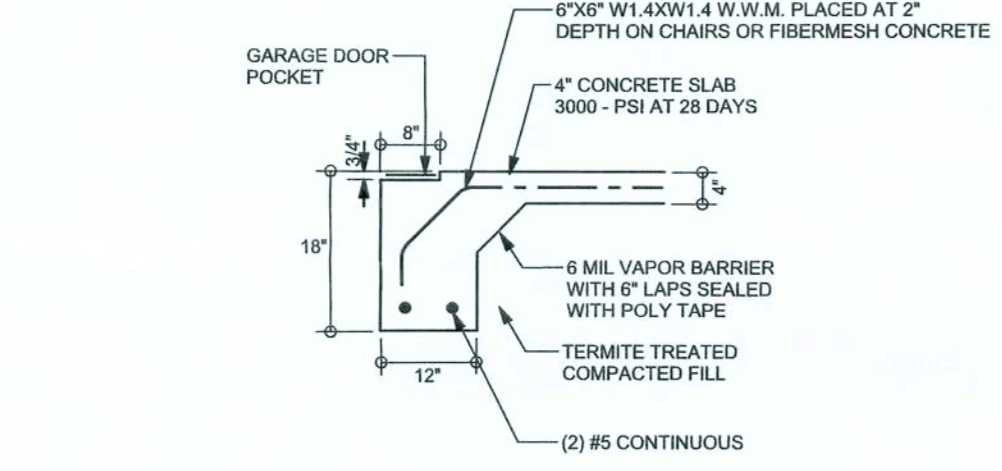
F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



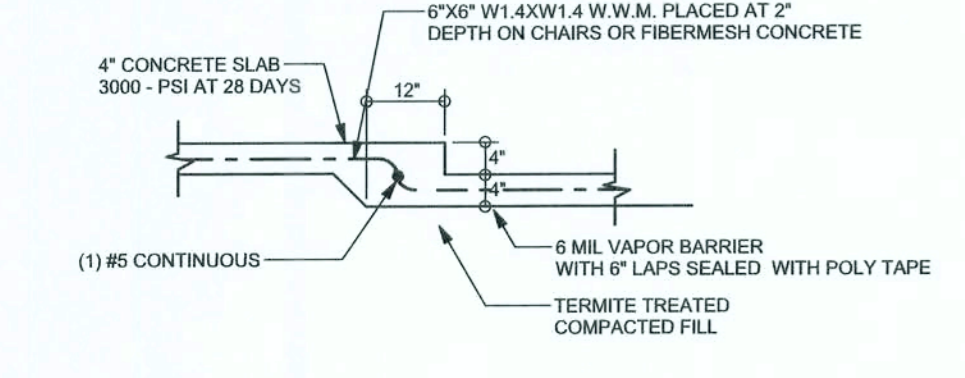
F'2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F13 ALT. STEM WALL GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

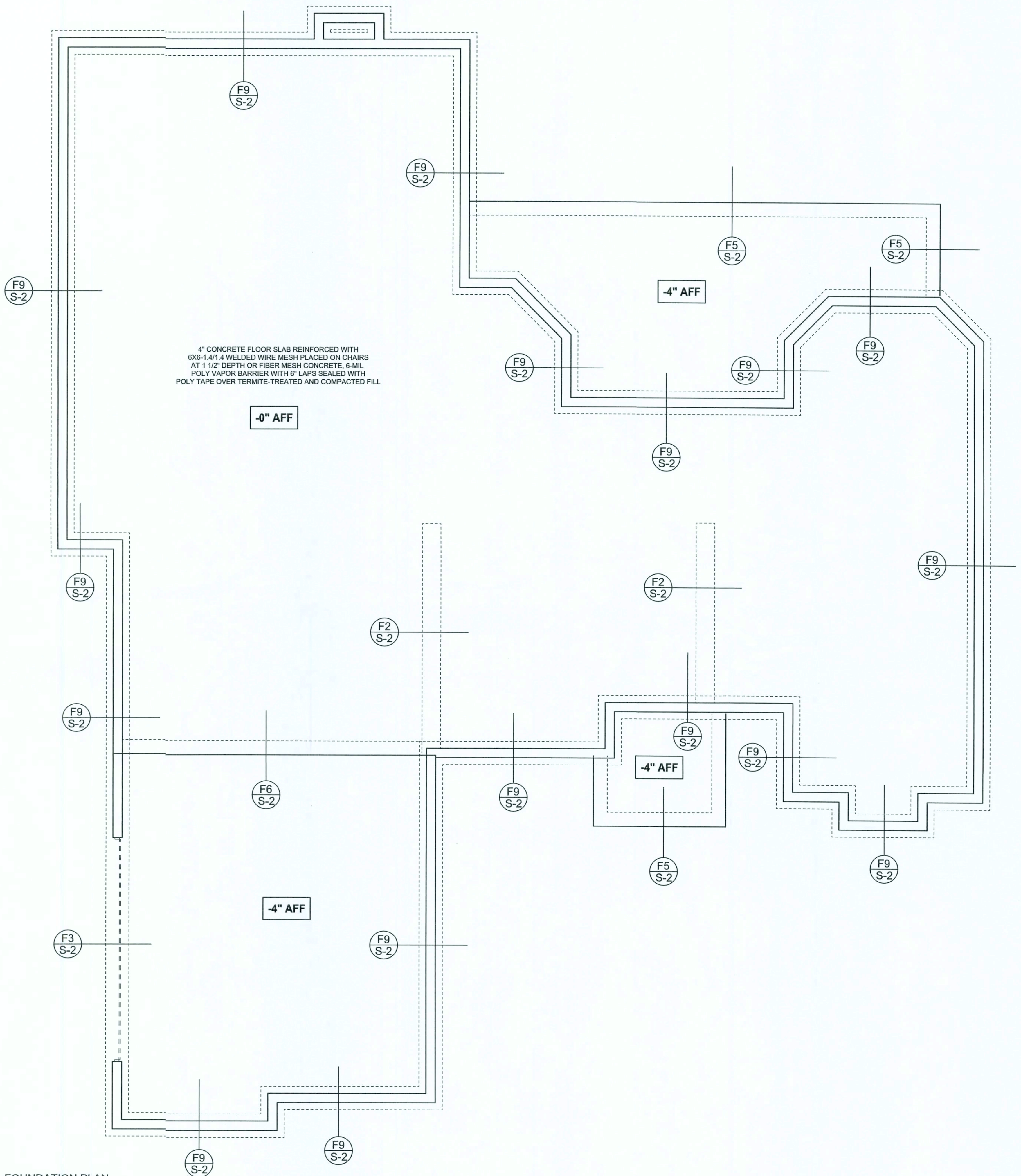


F6 TYPICAL NON-BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced side above top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Outrigger ladder reinforcement at 16"OC vertically or a horizontal bond beam with 180 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNALIGNED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915 PCB 868, Lake City, FL 32056, 386-754-5419
 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.
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 CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.
 LIMITATION: This design is valid for one building, at specified location.
 MARK DISOSWAY
 P.E. 53915
 16/08/06
 SEAL

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PRINTED DATE:
 August 16, 2006
 DRAWN BY: STRUCTURAL BY:
 David Disosway
 FINALS DATE:
 16 / Aug / 06
 JOB NUMBER:
 608032
 DAWING NUMBER
 S-2
 OF 3 SHEETS

NO.	DESCRIPTION	DATE

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-103. BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	1ST FLOOR EXTERIOR WALL
	2ND FLOOR EXTERIOR
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

-
-

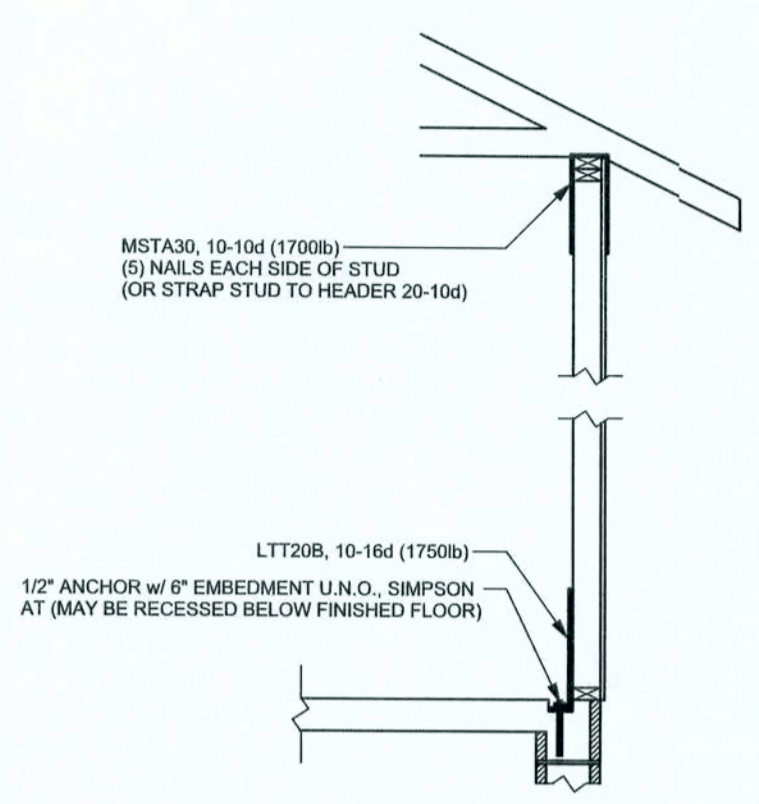
HEADER LEGEND

-
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

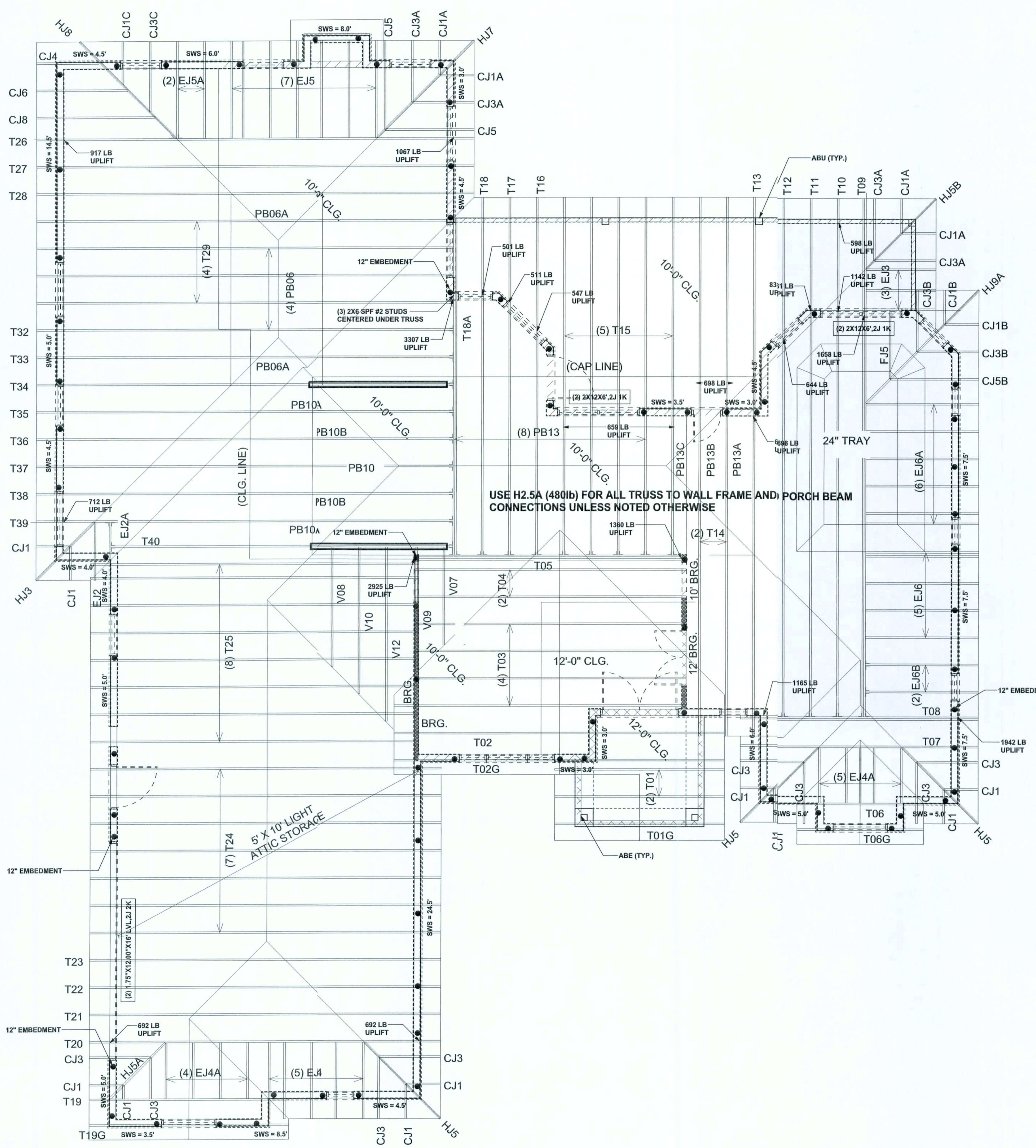
TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	45.2'	106.0'
LONGITUDINAL	39.5'	58.5'



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL.
SCALE: 1/2" = 1'-0"



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,
P.E. No. 53915, ICB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway P.E. for resolution.
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CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the files, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915

Mark Disosway
10/16/2006

SEAL

Isaac Construction

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16 / Aug / 06

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DRAWING NUMBER
S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #L205488