

DATE 11/10/2005

# Columbia County Building Permit

PERMIT 000023837

This Permit Expires One Year From the Date of Issue

APPLICANT MIKE TODD

PHONE 755-4387

ADDRESS 129 NE COLBURN AVE

LAKE CITY

FL 32055

OWNER GREGORY "JASON" LITTLE

PHONE 755-3303

ADDRESS 428 SW SAPPHIRE COURT

LAKE CITY

FL 32024

CONTRACTOR MIKE TODD

PHONE 755-4387

LOCATION OF PROPERTY

47S, TL ON WALTER AVE, TL ON LITTLE ROAD, TR ON SAPPHIRE CT, 1 MILE DOWN DIRT ROAD TURN LEFT

TYPE DEVELOPMENT SFD, UTILITY

ESTIMATED COST OF CONSTRUCTION 58200.00

HEATED FLOOR AREA 1164.00

TOTAL AREA 1372.00

HEIGHT .00

STORIES 1

FOUNDATION CONC

WALLS FRAMED

ROOF PITCH 6/12

FLOOR SLAB

LAND USE & ZONING A-3

MAX. HEIGHT 16

Minimum Set Back Requirements:

STREET-FRONT 30.00

REAR 25.00

SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE XOUT

DEVELOPMENT PERMIT NO.

PARCEL ID 01-5S-16-03390-008

SUBDIVISION

LOT

BLOCK

PHASE

UNIT

TOTAL ACRES 1.00

Culvert Permit No.

Culvert Waiver

CGC006209

Contractor's License Number

BK

Applicant/Owner/Contractor

EXISTING

05-0929-N

LU & Zoning checked by

JH

New Resident

Driveway Connection

Septic Tank Number

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

SECTION 14.9 SPECIAL FAMILY LOT PERMIT

## FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power

Foundation

(Footer/Slab)

Under slab rough-in plumbing

Slab

Sheathing/Nailing

Framing

date/app. by

Rough-in plumbing above slab and below wood floor

Electrical rough-in

date/app. by

Heat & Air Duct

date/app. by

Permanent power

date/app. by

C.O. Final

date/app. by

M/H tie downs, blocking, electricity and plumbing

date/app. by

Pool

date/app. by

Reconnection

date/app. by

Pump pole

date/app. by

Utility Pole

date/app. by

M/H Pole

date/app. by

Travel Trailer

date/app. by

Re-roof

date/app. by

MISC. FEES \$ .00

ZONING CERT. FEE \$ 50.00

FIRE FEE \$ .00

WASTE FEE \$

BUILDING PERMIT FEE \$ 295.00

CERTIFICATION FEE \$ 6.86

SURCHARGE FEE \$ 6.86

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$ 25.00

CULVERT FEE \$

TOTAL FEE 383.72

INSPECTORS OFFICE

CLERKS OFFICE

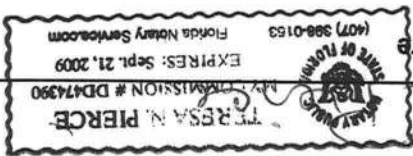
NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY, IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

## This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.



Notary Signature

*Theresa N. Pierce*

NOTARY STAMP/SEAL

Competency Card Number

Contractors License Number *CC000209*

Contractor Signature

*[Signature]*

Personally known  or Produced Identification

this 27 day of October 2005

Sworn to (or affirmed) and subscribed before me

COUNTY OF COLUMBIA

STATE OF FLORIDA

Owner Builder or Agent (Including Contractor)

*[Signature]*

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.  
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.  
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Total Building Height 10'4" Number of Stories 1 Heated Floor Area 1104 Roof Pitch 6/12  
Actual Distance of Structure from Property Lines - Front 45' Side 91.4' Side 91.4' Rear 119.72'  
Total Acreage 1 Lot Size \_\_\_\_\_ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive  
Type of Construction residential single fam Number of Existing Dwellings on Property 0  
dirt road - Turn 1 - Approx 2 miles to property  
Little Road - 3 miles to Sapphire Ct - Turn 2 - 1 mile down  
Driving Directions Turn 1 on 5th on 5th  
Subdivision Name \_\_\_\_\_ Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_  
Property ID Number 01-55-10-03390-008 Estimated Cost of Construction 83,000  
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy  
Mortgage Lenders Name & Address First Federal, Lake City  
Architect/Engineer Name & Address n/a  
Bonding Co. Name & Address n/a  
Fee Simple Owner Name & Address n/a  
Address 139 NE Colburn Ave, Lake City 32055  
Contractors Name Mike Todd Phone 386 755 4387  
911 Address 428 SW Sapphire Ct, Lake City 32024  
Owners Name Gregory "Jason" Little Phone 386 755-3303  
Address 129 NE Colburn Avenue, Lake City 32055  
Applicants Name Mike Todd Phone 386 755 4387

For Office Use Only Application # 0510-96 Date Received 10/31/05 By [Signature] Permit # 23837  
Application Approved by - Zoning Official BK Date 09.11.05 Plans Examiner OK JTH Date 11-9-05  
Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3  
Comments Section 14.9 Special Family Lt Permit

Columbia County Building Permit Application CK# 10956 Revised 9-23-04 383.72

OWNER AGENT: \_\_\_\_\_  
 PREPARED BY: \_\_\_\_\_  
 DATE: 1/20/11  
 Review of plans and specifications covered by the calculation are in compliance with the Florida Energy Code.  
 BUILDING OFFICIAL: \_\_\_\_\_  
 DATE: \_\_\_\_\_  
 Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

1. Compliance package chosen (A-F)
2. New construction or addition
3. Single family detached or Multifamily attached
4. If Multifamily—No. of units covered by this submission
5. Is this a worst case? (yes / no)
6. Conditioned floor area (sq. ft.)
7. Predominant eave overhang (ft.)
8. Glass type and area :
  - a. Clear glass
  - b. Tint, film or solar screen
9. Percentage of glass to floor area
10. Floor type, area or perimeter, and insulation:
  - a. Slab on grade (R-value)
  - b. Wood, raised (R-value)
  - c. Wood, common (R-value)
  - d. Concrete, raised (R-value)
  - e. Concrete, common (R-value)
11. Wall type, area and insulation:
  - a. Exterior: 1. Masonry (Insulation R-value)
  2. Wood frame (Insulation R-value)
12. Ceiling type, area and insulation:
  - a. Under attic (Insulation R-value)
  - b. Single assembly (Insulation R-value)
13. Air Distribution System: Duct insulation, location
  - a. Under attic (Insulation R-value)
  - b. Single assembly (Insulation R-value)
14. Cooling system
 

(Types: central, room unit, package terminal A.C., gas, none)
15. Heating system:
 

(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)
16. Hot water system:
 

(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)

1.	lin. ft.	R=	0
10a.	sq. ft.	R=	
10b.	sq. ft.	R=	
10c.	sq. ft.	R=	
10d.	sq. ft.	R=	
10e.	sq. ft.	R=	
11a-1	sq. ft.	R=	
11a-2	sq. ft.	R=	
11b-1	sq. ft.	R=	13
11b-2	sq. ft.	R=	
12a.	sq. ft.	R=	30
12b.	sq. ft.	R=	116.8
13.	sq. ft.	R=	6
14a.	Type:		Central
14b.	SEER/EER:		13.0
14c.	Capacity:		2 TONS
15a.	Type:		Heat Pump
15b.	HSPF/COP/AFUe:		
15c.	Capacity:		1.8 TONS
16a.	Type:		Electric
16b.	EF:		81

Please Print  
 CK

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.  
 2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.  
 3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.  
 4. Complete page 1 based on the "To Be Installed" column information.  
 5. Read Minimum Requirements for All Packages, Table 6B-2 and check each box to indicate your intent to comply with all applicable items.  
 6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

OWNER: <u>LATE JASON</u>	PERMIT NO.: <u>23837</u>	JURISDICTION NO.: <u>221000</u>
PROJECT NAME: <u>LATE JASON</u>	OFFICE: <u>ClimateCo.</u>	CLIMATE ZONE: <input type="checkbox"/> 1 <input type="checkbox"/> 2 <input type="checkbox"/> 3
AND ADDRESS: _____	PERMITTING: _____	
BUILDER: <u>MIKE TODD CONSTRUCTION</u>		

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescribes in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

FORM 600B-01  
 FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION  
 Residential Component Prescriptive Method B  
 NORTH 1 2 3

MR. JASON LITTLE  
TWN 56  
R 16E

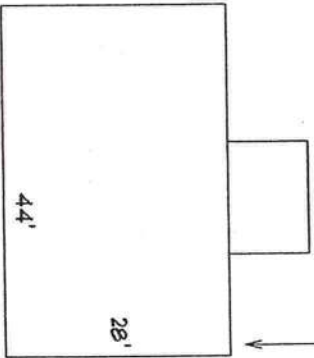
225.24'

192.73

91.41'

DRIVE

WELL



119.72

226.82'

45'

SEPTIC



91.41'

192.72



Inst: 2005009097 Date: 04/18/2005 Time: 16:32  
Doc Stamp-Deed : 0.70  
DC, P. Dewitt Cason, Columbia County B-1043 P-2205

**QUITCLAIM DEED**

THIS QUITCLAIM DEED, executed this 18th day of April, 2005

by first party, Grantor, Gregory Dale Little  
whose post office address is 178 SW Sapphire Ct, Lake City, FL 32024  
to second party, Grantee, Gregory Jason Little  
whose post office address is 178 SW Sapphire Ct, Lake City, FL 32024

WITNESSETH, That the said first party, for good consideration and for the sum of \$10,000 Dollars (\$10,000) paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release and quitclaim unto the said second party forever, all the right, title, interest and claim which the said first party has in and to the following described parcel of land, and improvements and appurtenances thereto in the County of Columbia State of Florida to wit:

SEE ATTACHED DESCRIPTION

[Signatures on following page.]

001.70.7000 12:17 PM

IN WITNESS WHEREOF, The said first party has signed and sealed these presents the day and year first above written. Signed, sealed and delivered in presence of:

Signature of First Party, Grantor  
*Gregory Dale Little*

Print name of First Party  
*Gregory Dale Little*

Signature of First Party, Grantor

Print name of First Party

Signature of Witness  
*M. Virginia Tiner*

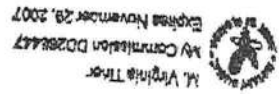
Print name of Witness  
*M. Virginia Tiner*

Signature of Witness  
*Dorothy K. Evans*

Print name of Witness  
*Samantha L. Evans*

WITNESS my hand and official seal

Signature of Notary  
*M. Virginia Tiner*



Affiant Known Produced ID  
Type of ID *FL Drivers License*  
*L 340-28454-335-D*

Signature of Preparer

Print Name of Preparer

Address of Preparer

Initials of First Party

Inst: 200508037 Date: 04/19/2005 Time: 16:32  
Doc Stamp-Dead : 0.70  
DC, P. Dewitt Cason, Columbia County B: 1043 P: 2306

**BOUNDARY SURVEY TIN:**

**THE NW 1/4 OF SECTION 15-3, R16-E**  
Columbia County, Florida.

**DESCRIPTION:**

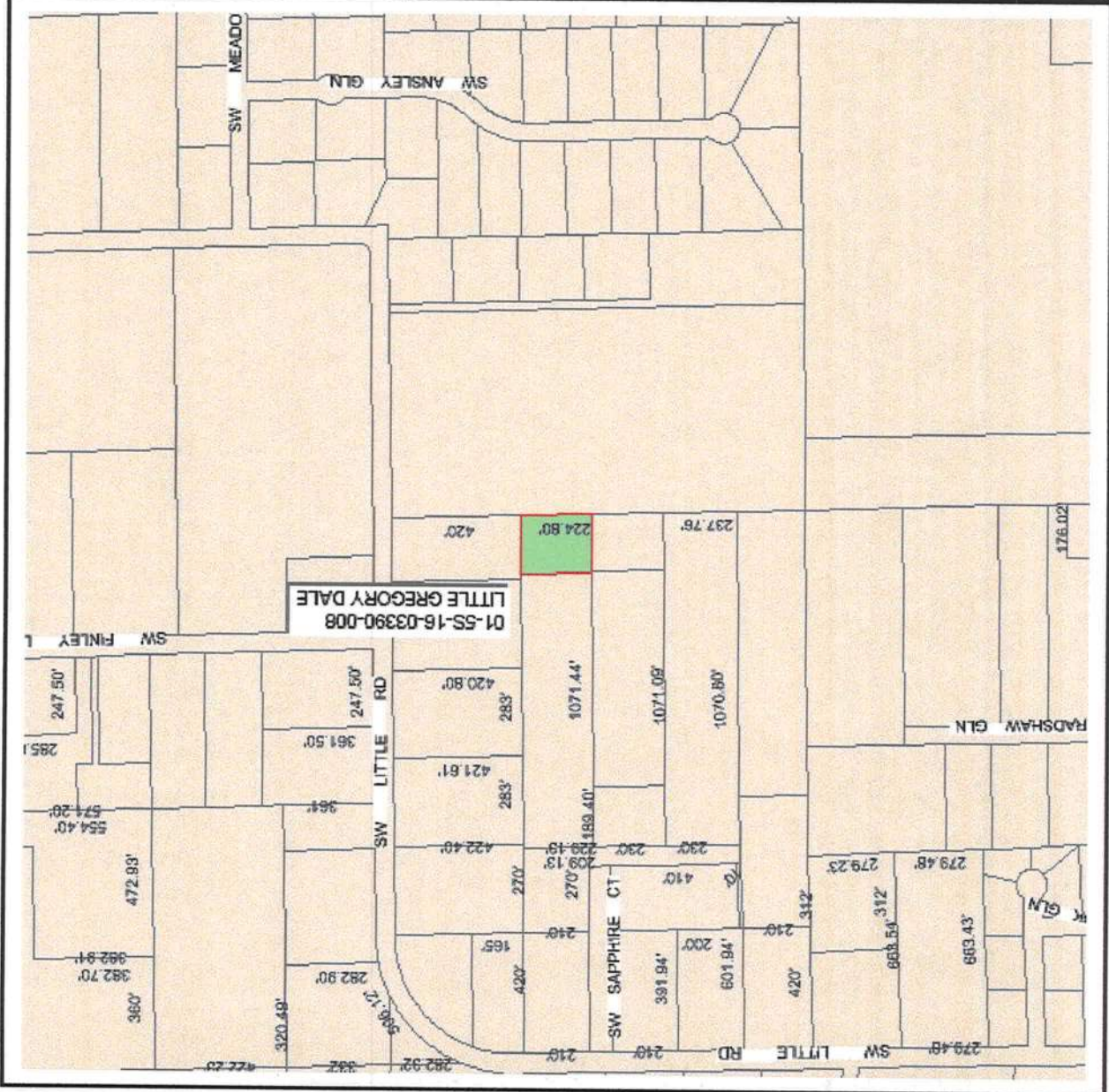
Commence at the S.E. corner of the N.W. 1/4 of Sect. and, Township 5 South, Range 16 East, Columbia County, Florida, Thence Run N 0° 03' 55" W along the East line of said N.W. 1/4, 893.95 feet. thence S 89° 21' 30" W 420 feet to the POINT OF BEGINNING. thence S 89° 21' 30" W 225.24 feet. thence N 0° 31' 31" W 192.72 feet. thence N 89° 21' 30" E 225.82 feet. thence S 0° 03' 24" E 192.73 feet to the Point of Beginning. Containing 1.00 Acres, more or less. Said Parcel being the South 1/2 here of a Parcel of land described in O.R.O. 838, Page 426, Parcel "D", of the Official Records of Columbia County, Florida.

Together with a 25 foot Easement for Egress and Egress, being 25 feet to the left of the following described line: Begin at the N.E. corner of the above described Parcel, thence Run N 0° 03' 24" W 16.63 feet. thence N 0° 01' 27" W 81.80 feet. thence S 89° 20' 22" W 229.48 feet, to the Point of Termination of said line.




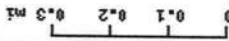
Also:  
Together with Easements for Egress and Egress as described in D.R.O. 838 page 426, Parcel "D" and in O.R.O. 340 page 604, of the Official Records of Columbia County, Florida.

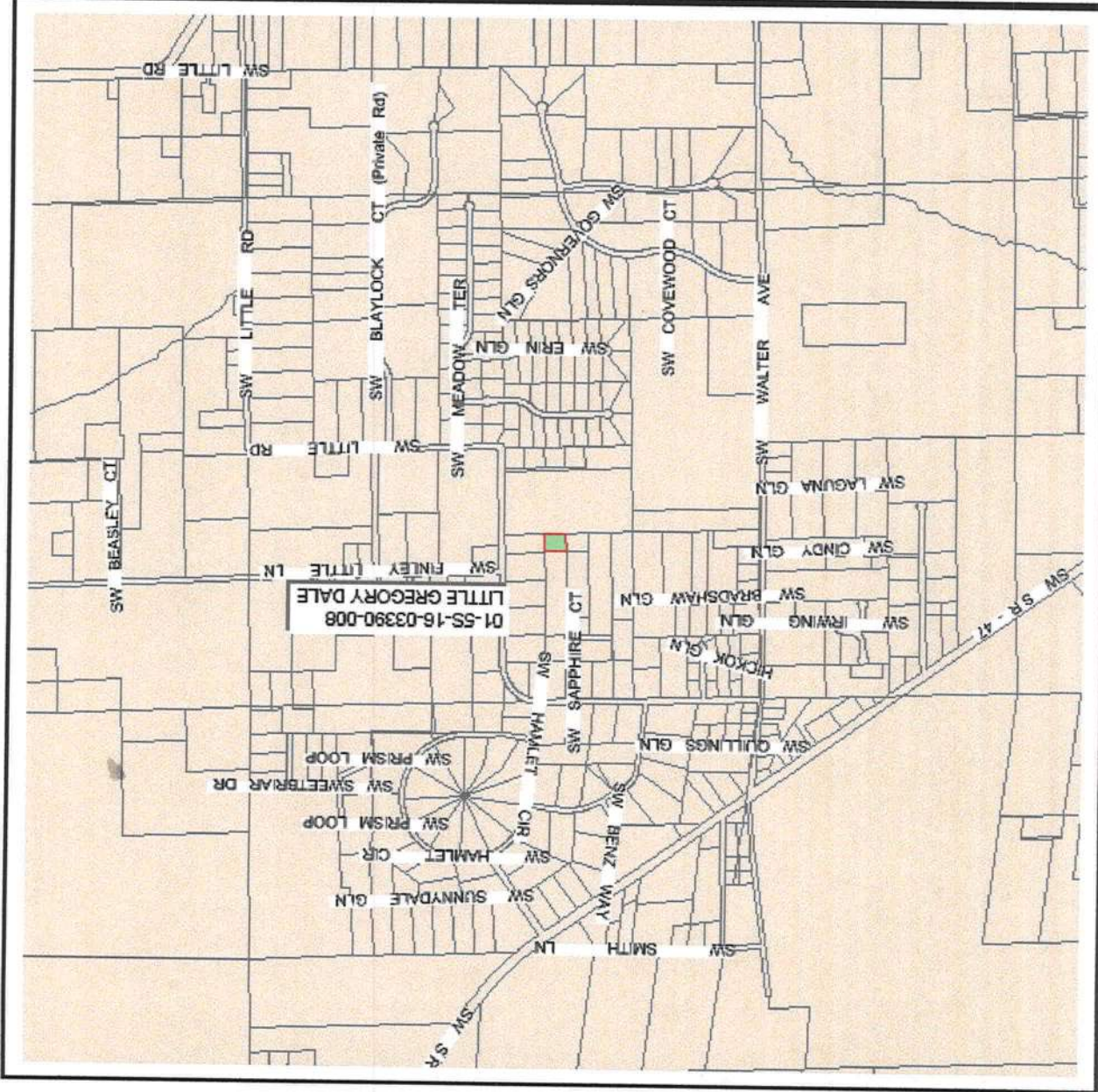
This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values and therefore are subject to change before being finalized for ad valorem assessment purposes.

			Name: LITTLE GREGORY DALE Site: 178 SW SAPPHIRE CT Mail: LAKE CITY, FL 32024 Sales 4/18/2005 \$100,00 V / U Info
COMM SE COR OF NW1/4 OF SEC, RUN N 893.95 FT, W 420 FT FOR POB, CONT W 225.24 FT, NORTH <b>PARCEL: 01-55-16-03390-008 - NO AG ACRE (009900)</b>		LandVal \$10,000.00 BldgVal \$0.00 ApprVal \$10,000.00 JustVal \$10,000.00 Assd \$10,000.00 Exmpt \$0.00 Taxable \$10,000.00	
J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083 <b>Columbia County Property Appraiser</b>		0 220 440 660 ft 	



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

 		Name: LITTLE GREGORY DALE Site: 178 SW SAPHIRE CT Mail: LAKE CITY, FL 32024 Sales 4/18/2005 \$100.00 V / U Info LandVal \$10,000.00 BldgVal \$0.00 AppVal \$10,000.00 JustVal \$10,000.00 Assd \$10,000.00 Exmpt \$0.00 Taxable \$10,000.00
<b>PARCEL: 01-55-16-03390-008 - NO AG ACRE (009800)</b> COMM SE COR OF NW1/4 OF SEC. RUN N 893.95 FT, W 420 FT FOR POB, CONT W 225.24 FT, NORTH J. Doyle Crews, CFA - Lake City, Florida - 386-756-1083 <b>Columbia County Property Appraiser</b>		
 		



**Columbia County Property Appraiser**

Parcel: 01-5S-16-03390-008

DB Last Updated: 9/16/2005

**2005 Proposed Values**

Tax Record    Property Card    Interactive GIS Map    Print

Search Result: 1 of 1

**Owner & Property Info**

<b>Owner's Name</b>	LITTLE GREGORY DALE
<b>Site Address</b>	
<b>Mailing Address</b>	178 SW SAPPHERE CT LAKE CITY, FL 32024
<b>Brief Legal</b>	COM SE COR OF NW1/4 OF SEC, RUN N 893.95 FT, W 420 FT FOR POB, CONT W 225.24 FT, NORTH

<b>Use Desc. (code)</b>	NO AG ACRE (009900)
<b>Neighborhood</b>	1516.00
<b>Tax District</b>	3
<b>UD Codes</b>	
<b>Market Area</b>	01
<b>Total Land Area</b>	1.000 ACRES

**Property & Assessment Values**

<b>Mkt Land Value</b>	cnt: (1)	\$10,000.00
<b>Ag Land Value</b>	cnt: (0)	\$0.00
<b>Building Value</b>	cnt: (0)	\$0.00
<b>XFOB Value</b>	cnt: (0)	\$0.00
<b>Total Appraised Value</b>		\$10,000.00

<b>Just Value</b>	\$10,000.00
<b>Class Value</b>	\$0.00
<b>Assessed Value</b>	\$10,000.00
<b>Exempt Value</b>	\$0.00
<b>Total Taxable Value</b>	\$10,000.00

**Sales History**

<b>Sale Date</b>	4/18/2005	<b>Book/Page</b>	1043/2305	<b>Inst. Type</b>	QC	<b>Sale Vimp</b>	V	<b>Sale Qual</b>	U	<b>Sale RCode</b>	06	<b>Sale Price</b>	\$100.00
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**Building Characteristics**

<b>Bldg Item</b>		<b>Bldg Desc</b>		<b>Year Blt</b>		<b>Ext. Walls</b>		<b>Heated S.F.</b>		<b>Actual S.F.</b>		<b>Bldg Value</b>	
NONE													

**Extra Features & Out Buildings**

<b>Code</b>		<b>Desc</b>		<b>Year Blt</b>		<b>Value</b>		<b>Units</b>		<b>Dims</b>		<b>Condition (% Good)</b>	
NONE													

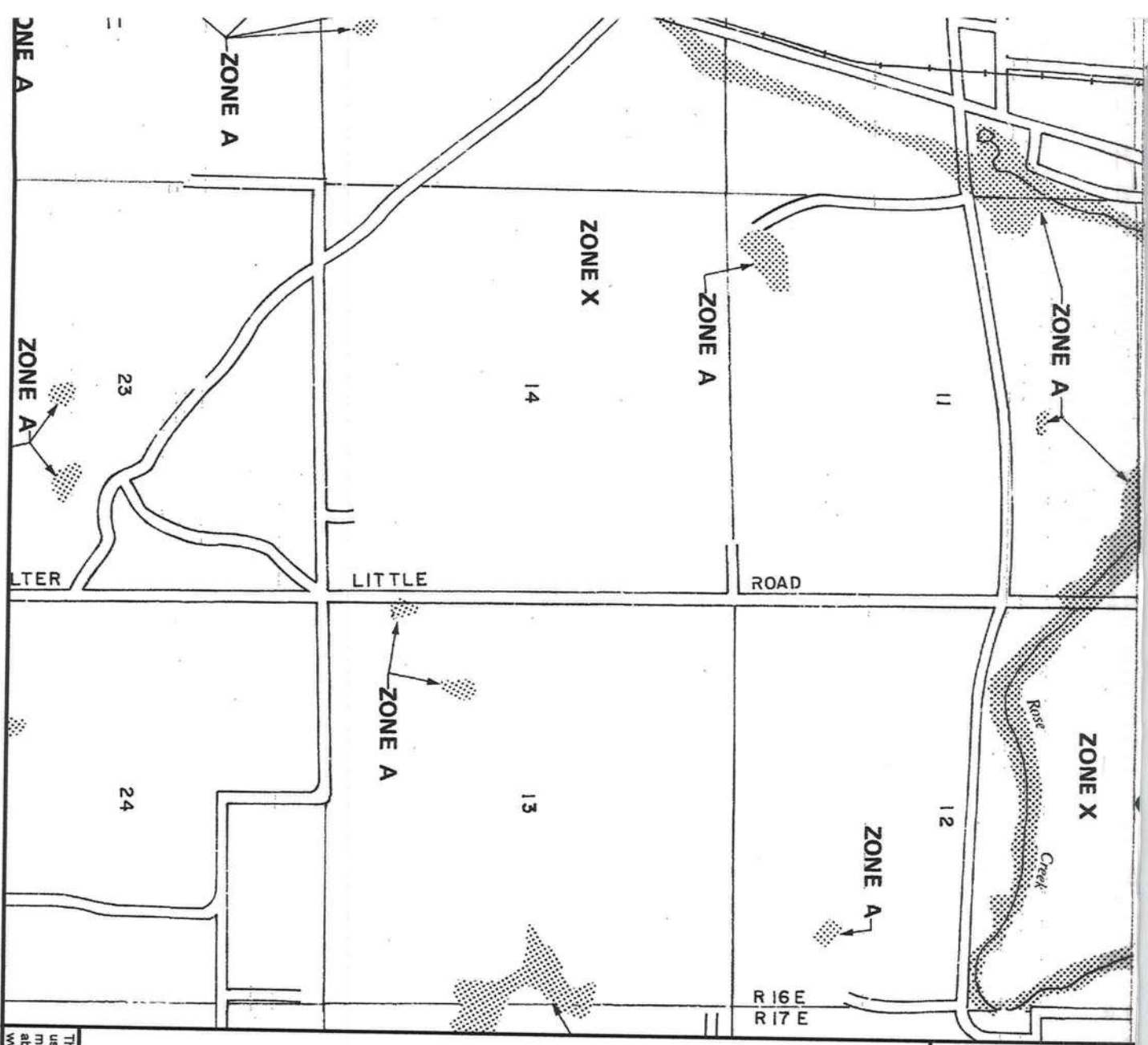
**Land Breakdown**

<b>Lnd Code</b>	009900	<b>Desc</b>	AC NON-AG (MKT)	<b>Units</b>	1.000 AC	<b>Adjustments</b>	1.00/1.00/1.00/1.00/.80	<b>Eff Rate</b>	\$10,000.00	<b>Lnd Value</b>	\$10,000.00
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Columbia County Property Appraiser

DB Last Updated: 9/16/2005

**Disclaimer**



**NATIONAL FLOOD INSURANCE PROGRAM**

**FIRM**  
**FLOOD INSURANCE RATE MAP**

COLUMBIA  
 COUNTY,  
 FLORIDA  
 (UNINCORPORATED AREAS)

PANEL 225 OF 290

PANEL LOCATION

COMMUNITY-PANEL NUMBER  
 120070 0225 B

EFFECTIVE DATE:  
 JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at [www.fema.gov/nfltsd](http://www.fema.gov/nfltsd).

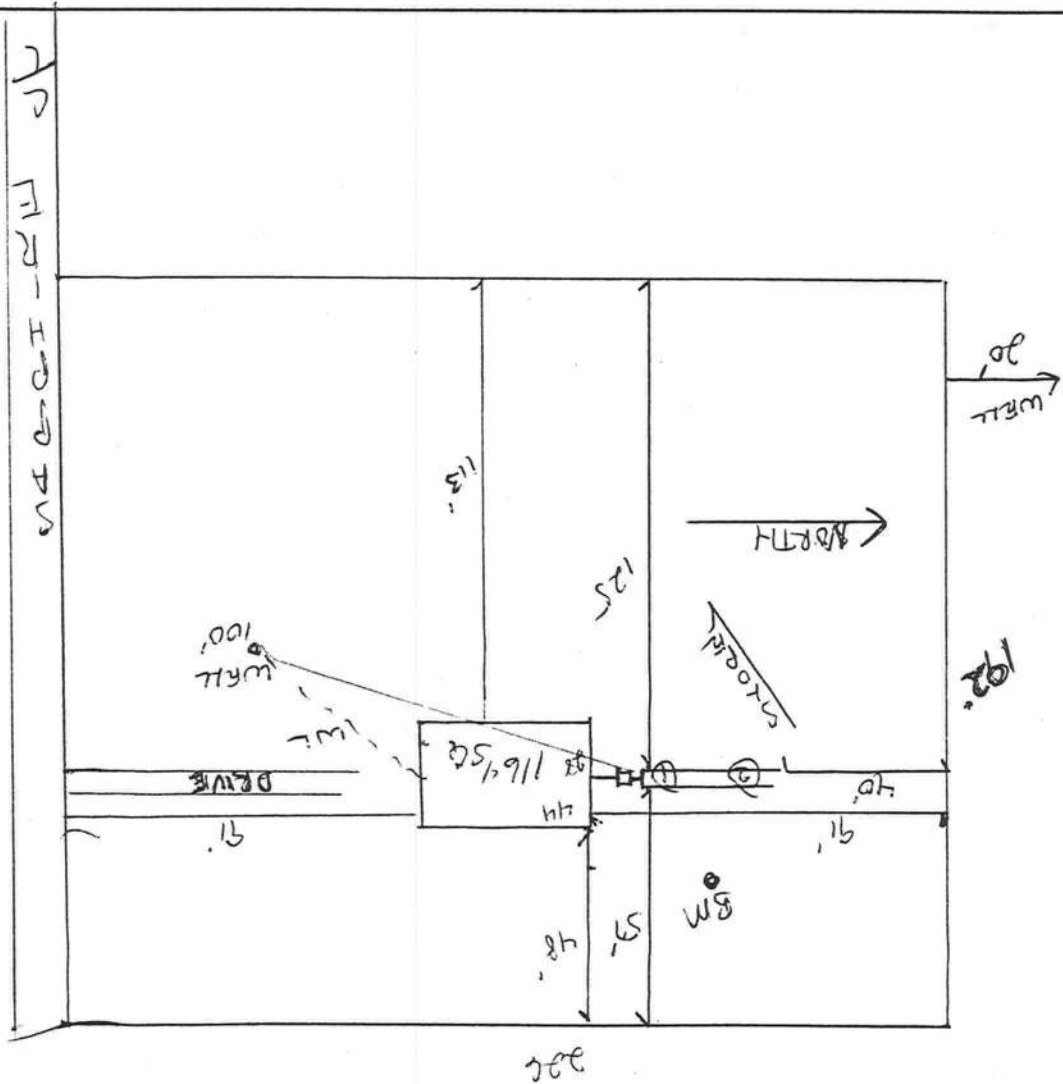
Print Date: 11/4/2005 (printed at scale and type)

STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-09291

----- PART II - SITEPLAN -----

Scale: 1 inch = 50 feet.



Notes:

Site Plan submitted by: Rock 270  
Plan Approved: ✓ Not Approved: \_\_\_\_\_  
By: Mr. J. J. [Signature] County Health Department  
Date: 9-13-05  
MASTER CONTRACTOR

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

This instrument prepared by:  
Michael H. Harrell  
Abstract & Title Services, Inc.  
283 NW Cole Terrace  
Lake City, Florida 32055

INS:2005023398 Date:09/22/2005 Time:10:55  
DC, P. Dewitt Cason, Columbia County B:1059 P:834

ATS# 15229

### NOTICE OF COMMENCEMENT

#### TO WHOM IT MAY CONCERN:

The undersigned hereby give notice that improvements will be made to certain real property and in accordance with Chapter 713.13, Florida Statutes, the following is provided in this Notice of Commencement:

1. Construction of Dwelling, to be made to real property located at SW Sapphire Court, Lake City, Florida 32024, more particularly described as:  
SEE EXHIBIT "A" ATTACHED HERETO AND BY THIS REFERENCE MADE A PART HEREOF.

2. The name and address of the undersigned owner is: Gregory Jason Little, SW Sapphire Court, Lake City, FL 32024.

3. The name and address of the contractor is: Mike Todd, 129 NE Colburn Ave, Lake City, FL 32055.

4. The name and address of surety bond is: N/A

5. LENDER: First Federal Savings Bank of Florida, 2571 US Highway 90 West, Lake City, Florida 32055.

6. Persons within the State of Florida designated by Owner upon whom notices of other documents may be served as provided in Section 713.13(1)(a), Florida Statutes: NONE

7. In addition to himself, Owner designates Paula L. Kehler 2571 US Highway 90 West, Lake City, Florida 32055, is also designated to receive a copy of the Lender's Notice as provided in Section 713.06(2)(b) Florida Statutes.

8. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

\*Owner is used for singular or plural as context requires.

Signed, sealed and delivered in the presence:

Paul Lander WITNESS  
Gregory Jason Little WITNESS

STATE OF FLORIDA MICHAEL H. HARRELL  
COUNTY OF COLUMBIA

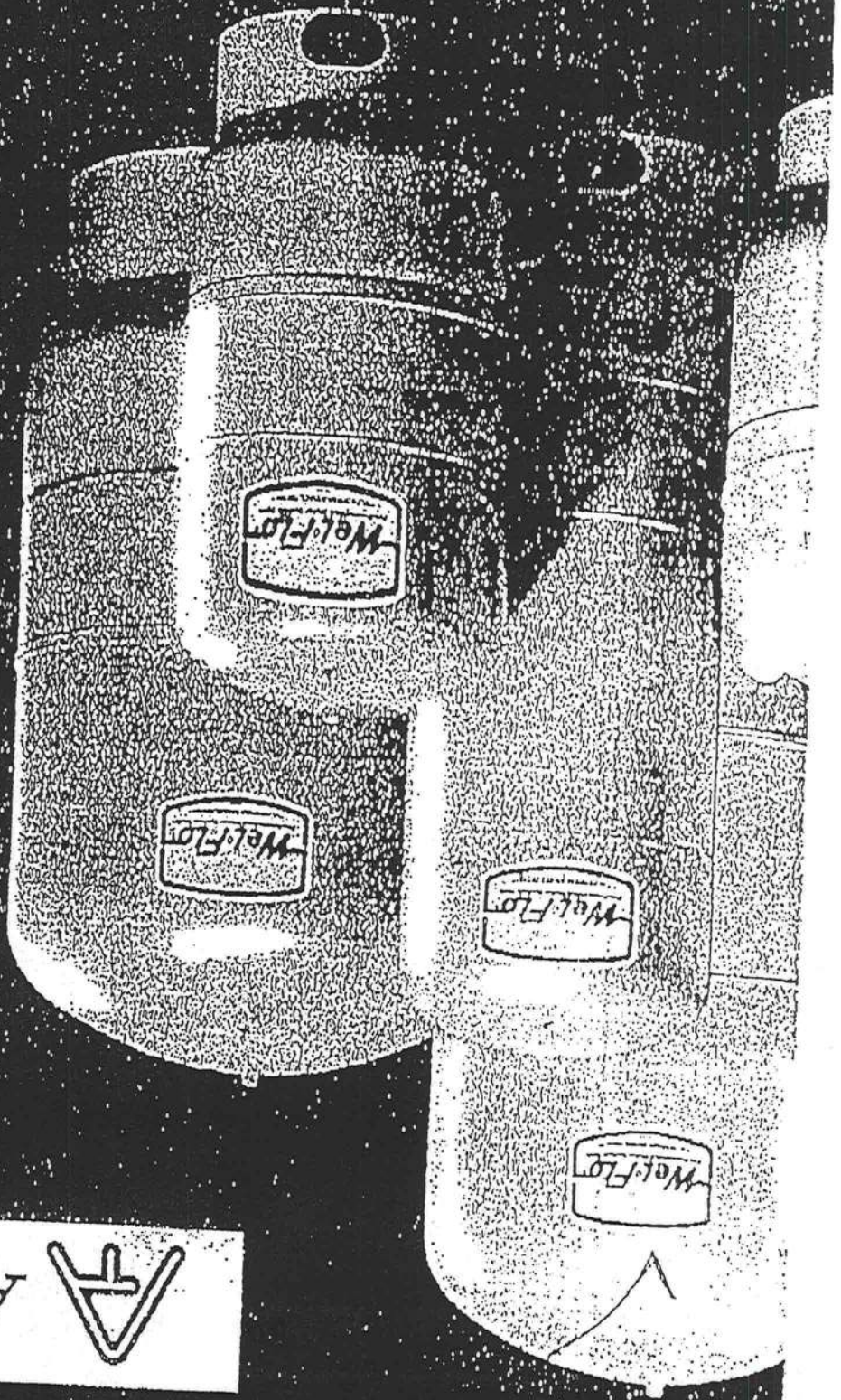
Before me, personally appeared Gregory Jason Little, to me known to be the person(s) described in and who executed the foregoing instrument, and they acknowledged to and before me that they executed said instrument for the purpose therein expressed.

Witness my hand and official seal this 19th day of September, 2005.

NOTARY PUBLIC  
Michael H. Harrell



MAJURU U HARRELL



- Outstanding Value
- Sizes from 14 to 119 Gallons
- Tough Glass Finish
- Proven Diaphragm Design

**WEL-FLO®**  
 Pre-pressurized  
 Water System Tanks

**AMTROL**  
 INC.

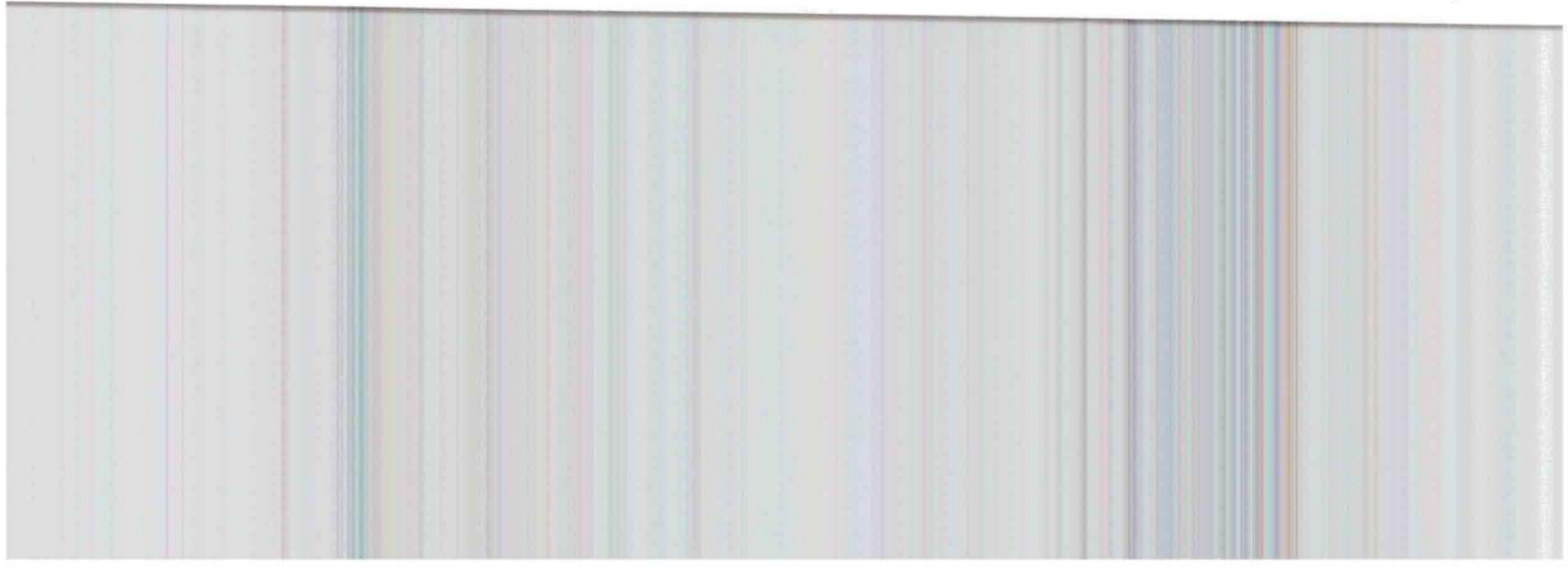
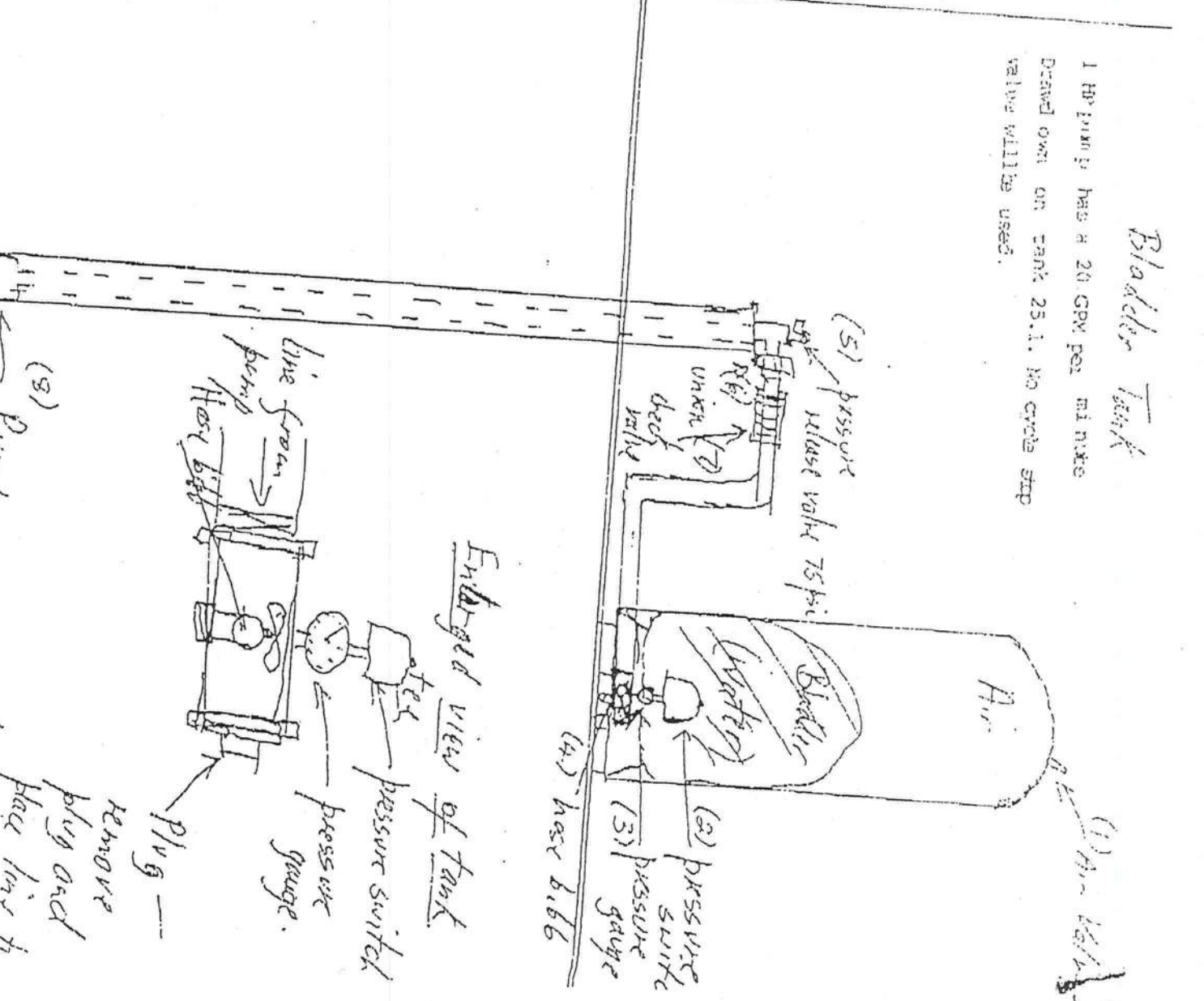


MAR. 13 2002 02:23PM PT

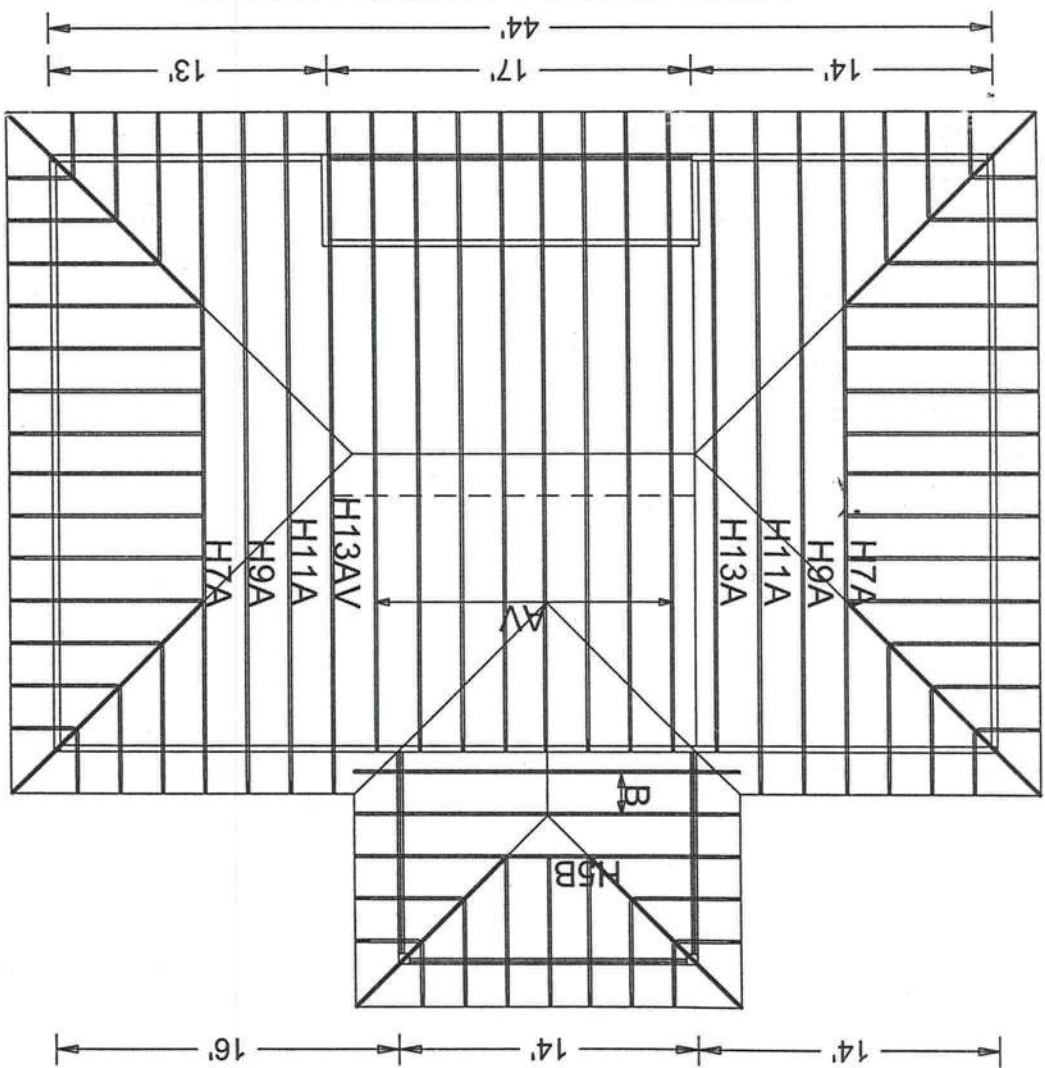
PHONE NO. : 7526677

FROM : LITCH WELLS DRILLING 752-1477

1. Air Valve. Allows air to be put into tank. Must be at or 2 psi below cut-in pressure with tank empty.
2. Pressure switch. Sets cut-in and cut-off pressure for pump.
3. Pressure gauge. Shows actual pressure in tank.
4. Hose bibb. May be used to drain tank or for watering purposes.
5. Pressure release valve. Safety device to prevent explosion of tank.



MIKE TODD / JASON LITTLE  
 Roof Plane Sheathing Area = 1919 sq. ft  
 Total Sheathing Area = 1919 sq. ft  
 Fascia Material = 180 linear ft  
 Valley Flashing Material = 27 linear ft  
 Ridge Cap Material = 26 linear ft  
 Hip Ridge Material = 123 linear ft



28'  
 10'

38'  
 28'  
 10'

14'  
 17'  
 44'  
 13'

14'  
 14'  
 16'

DESIGNED BY:

JOB DESCRIPTION:  
 MIKE TODD / JASON LITTLE

JOB LOCATION:

JOB NO:  
5-308

PAGE NO:  
1 OF 1

Truss Fabricator: Anderson Truss Company  
 Job Identification: 5-308-MIKE TODD / JASON LITTLE  
 Truss Count: 14

Model Code: Florida Building Code 2001

Truss Criteria: ANSI/TPI-1995 (STD) / FBC

Engineering Software: Alpine Software, Version 7.04.

Structural Engineer of Record:

Address:

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-98 - Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer or record, as defined in ANSI/TPI 1-1995 Section 2.2

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB

#	Ref	Description	Drawing#	Date
1	26987--H7A		05196005	07/15/05
2	26988--H9A		05196006	07/15/05
3	26989--H11A		05196007	07/15/05
4	26990--H13A		05196008	07/15/05
5	26991--AV		05196009	07/15/05
6	26992--H13AV		05196010	07/15/05
7	26993--H5B		05196011	07/15/05
8	26994--B		05196012	07/15/05
9	26995--CJ1		05196013	07/15/05
10	26996--HJ7		05196014	07/15/05
11	26997--HJ5		05196015	07/15/05
12	26998--CJ3		05196016	07/15/05
13	26999--CJ5		05196017	07/15/05
14	27000--EJ7		05196018	07/15/05

Seal Date: 07/15/2005

-Truss Design Engineer-

Arthur R. Fisher

Florida License Number: 59687

1950 Marley Drive

Haines City, FL 33844

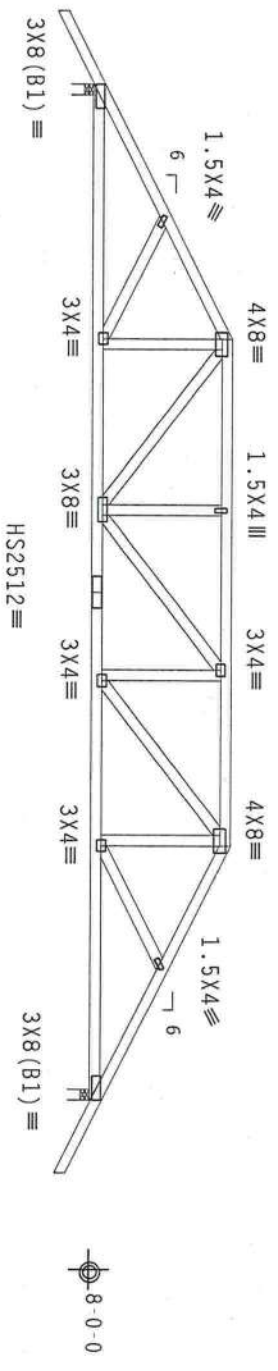



Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load.



7-0-0      14-0-0      7-0-0

28-0-0 Over 2 Supports

R=2311 U=458 W=3.5"

R=2311 U=458 W=3.5"

PLT TYP. 20 Gauge HS, Wave TPI

Design Crit: TPI-1995(STD)/FBC

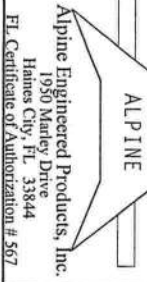
7.0.0

FL/-/3/-/1-/R/-

Scale = .1875"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 509 PROUDMAN BLVD., SUITE 600, HOUSTON, TX 77059) AND WFLA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN HOUSTON, TX 77036) FOR THE LATEST RECOMMENDATIONS. ALWAYS FOLLOW THE BEST PRACTICES INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC RATIONAL DESIGN SPEC. BY AISC AND TPI. APPLY CONNECTOR PLATES MADE OF 20/18/16GA (W, M/S/K) ASTM A575 GRADE 40/60 (W, K/L/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 180A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



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 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

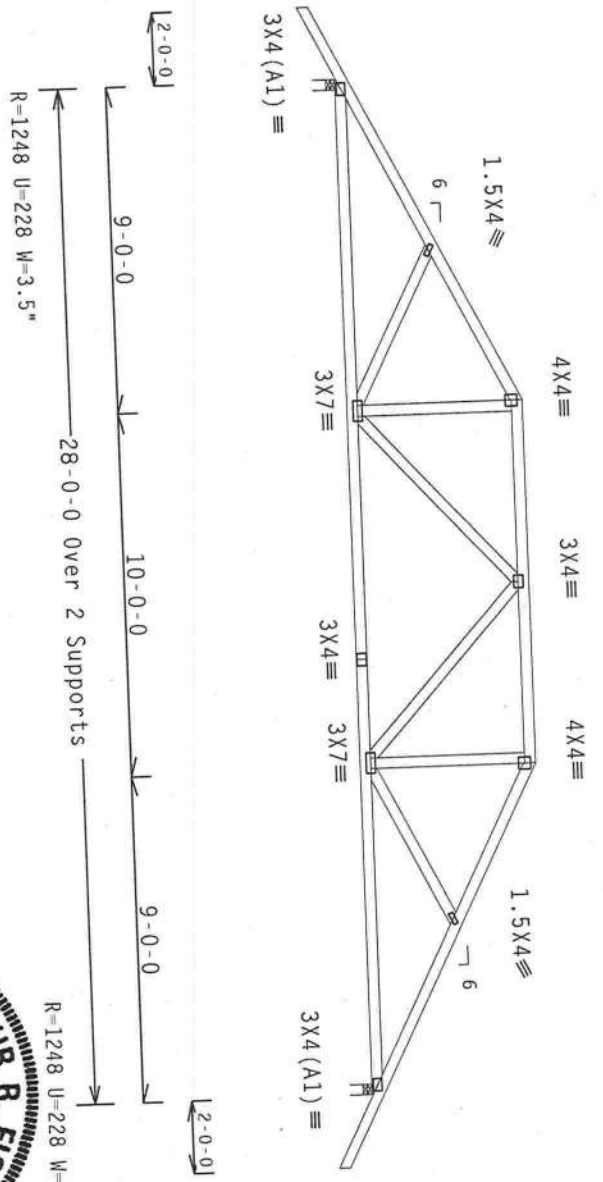


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TC DL	10.0 PSF	DATE	07/15/05
BC DL	10.0 PSF	DRW	HCUSR487 05196005
BC LL	0.0 PSF	HC-ENG	GDL/AF
TOT.LD.	40.0 PSF	SEQN	7309
DUR.FAC.	1.25		
SPACING	24.0"	JREF	150R487_205

5-308-MIKE TODD / JASON LITTLE - H9A)  
 OP Chord 2x4 SP #2 Dense  
 of Chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 10.10 ft mean hgt, ASCE 7-98, URBAN WIND  
 Located within 4.50 ft from roof edge, CAT II, EXP B, Wind TC  
 DL=5.0 psf, Wind BC DL=5.0 psf.

Reflection meets L/360 Tive and L/240 total load.



4-10-3

PLT TYP. Wave TPI

Design Crit: TPI-1995 (STD) / FBC

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING TRANSPORTATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING STORAGE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING INSTALLATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING USE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING REMOVAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING REPAIR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING DEMOLITION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING DISPOSAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING RECYCLING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING REUSE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING REPAIR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING DEMOLITION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING DISPOSAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING RECYCLING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSS FROM DAMAGE DURING REUSE.

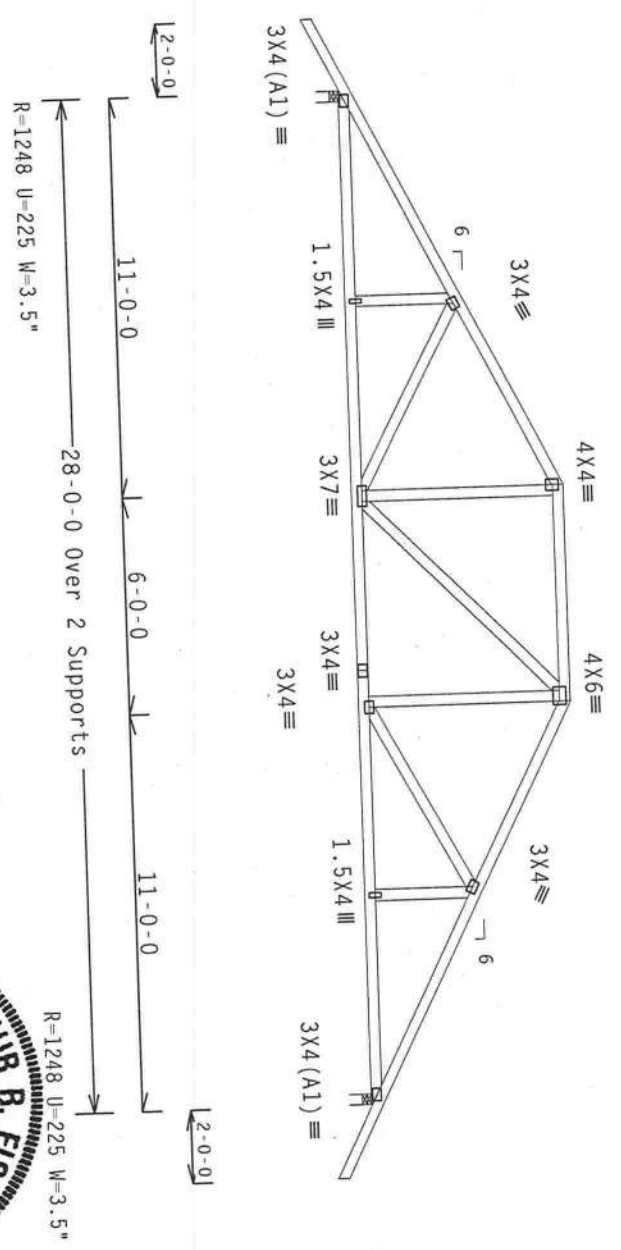


FL / - / 3 / - / - / R / -	Scale = .1875" / Ft.	
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TC DL	10.0 PSF	DATE 07/15/05
BC DL	10.0 PSF	DRW HCUSR487 05196006
BC LL	0.0 PSF	HC-ENG GDL/AF
TOT.LD.	40.0 PSF	SEQN- 7316
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1S0R487_205

5-308-MIKE TODD / JASON LITTLE - H11A)  
 op chord 2x4 SP #2 Dense  
 ot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 10.60 ft mean hgt, ASCE 7-98, CLUSTED BRG, 100%  
 located within 4.50 ft from roof edge, CAT II, EXP B, wind TC  
 DL=5.0 psf, wind BC DL=5.0 psf.

reflection meets L/360 live and L/240 total load.



5-10-3

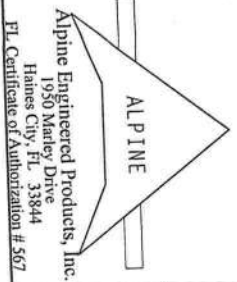
PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.00  
 No. 59687

FL/-/3/-/-/R/-

Scale = .1875" / ft.



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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. APPLY THE DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF MOD (NATIONAL GRADE 40/50 (A, B, C, D, E, F, G, H, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z) CONNECTIONS WITH APPLICABLE PROVISIONS OF MOD (NATIONAL GRADE 40/50 (A, B, C, D, E, F, G, H, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z) PLATES TO EACH FACE OF TRUSS AND, UNLESS SHOWN, BE PER ANNEK A3 OF TPI-2002 SEC. 2. ANY INSPECTION OF PLATES FOLLOWED BY FUNCTIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPANY'S DRAWING INDICATES ACCEPTANCE BY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.



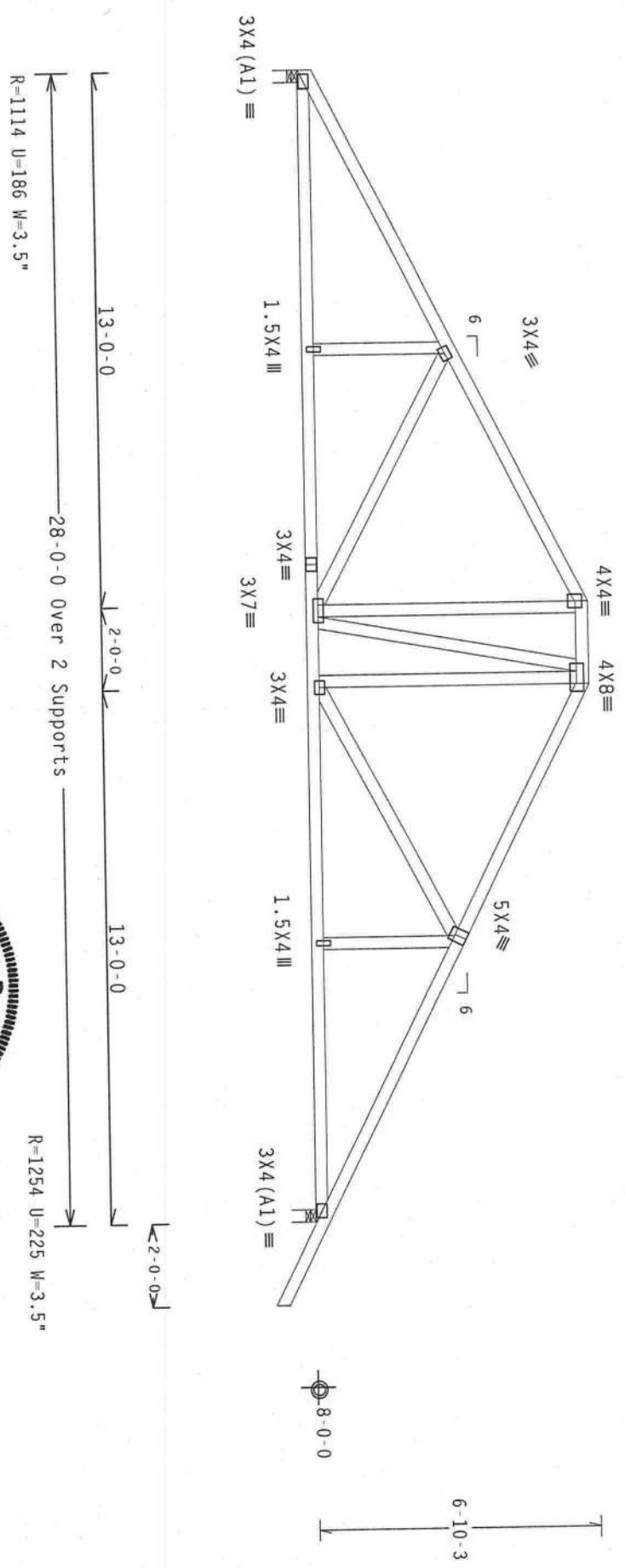
Jul 15 '05

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BC LL	0.0 PSF	HC-ENG	GDL/AF
TOT. LD.	40.0 PSF	SEQN-	7321
DUR. FAC.	1.25		
SPACING	24.0"	JREF	150R487_205

(5-308-MIKE TODD / JASON LITTLE - H13A)  
 Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 11.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not  
 located within 4.50 ft from roof edge, CAT II, EXP B, wind TC  
 DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.



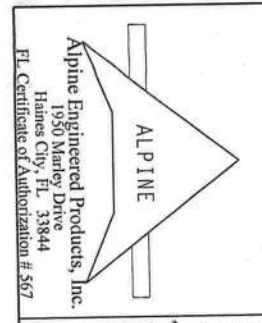
PLT TYP. Wave TPI

Design Cmt: TPI-1995(STD)/FBC

7.0

FL/-/3/-/1/-/R/-

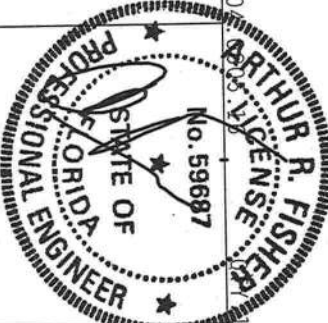
Scale = .25" / Ft.



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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN ACCORDANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR THE WORKMANSHIP WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALL TRUSS CONNECTION PLATES ARE MADE OF 2017B186GA (W-4/S/R) ASTM A653 GRADE 40/50 (W, K/H, S) GALV. STEEL. ALL TRUSS PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIAA AS OF TPI-2002, SEC. 3.1. FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SHALL BE STATED ON THIS DESIGN SHOW. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



JUL 15 '05

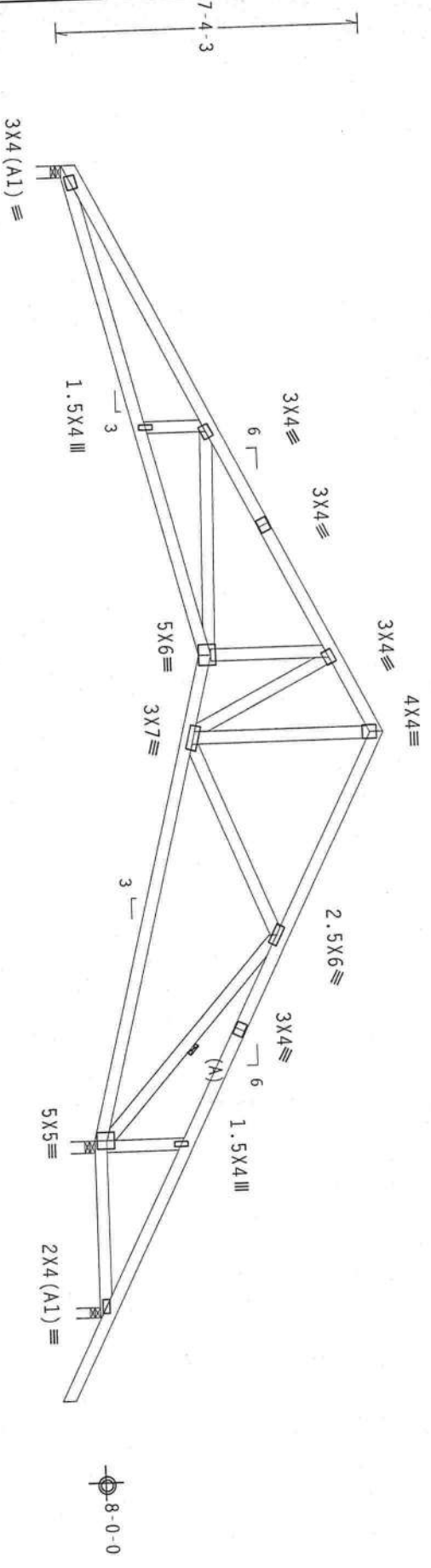
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BC DL	10.0 PSF	DRW HCUSR487 05196008
BC LL	0.0 PSF	HC-ENG GDL/AF
TOT.LD.	40.0 PSF	SEQN- 7338
DUR.FAC.	1.25	
SPACING	24.0"	JREF - 1SOR487_205

5-308-MIKE TODD / JASON LITTLE - AV)  
 op chord 2x4 SP #2 Dense  
 ot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 11.35 ft mean hgt, ASCE 7-99, velocity 139 mph  
 located within 4.50 ft from roof edge, CAT II, EXP B, wind TC  
 DL=5.0 psf, wind BC DL=5.0 psf.  
 Deflection meets L/360 live and L/240 total load.

A) Continuous lateral bracing equally spaced on member.  
 Shim all supports to solid bearing.

\* Negative reaction(s) of -220# MAX. (See below) from a  
 non-wind load case requires special uplift connection.  
 Connection to be designed and furnished by others.



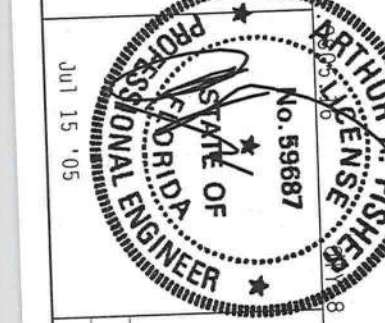
Design Crit: TPI-1995(STD)/FBC  
 R=862 U=180 W=3.5"  
 R=1726 U=264 W=3.5"  
 \*R=174/-220 U=224 W=3"

PLT TYP. Wave TPI  
 Design Crit: TPI-1995(STD)/FBC  
 Scale = .25"/ft.

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR CONNECTOR PLATES MADE OF 20/18/18GA (MIN. THICKNESS LOCATED ON THIS DESIGN, POSITION PER DRAWINGS ON THIS PLATE TO EACH FACE OF TRUSS AND UNLESS OTHERWISE SPECIFIED SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360-10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND INSURANCE. ANY INSPECTION OF PLATES FOLLOWING PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTED PROFESSIONAL ENGINEERING RESPONSIBILITY FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND INSURANCE. ANY INSPECTION OF PLATES FOLLOWING PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTED PROFESSIONAL ENGINEERING RESPONSIBILITY FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.



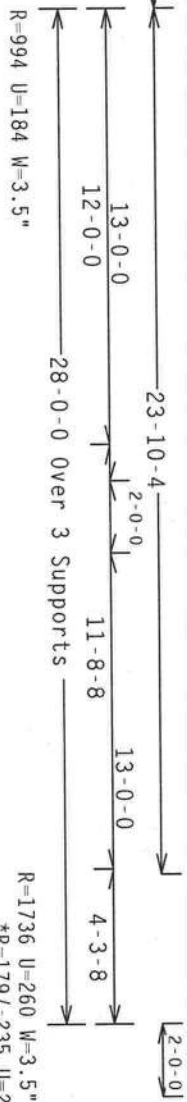
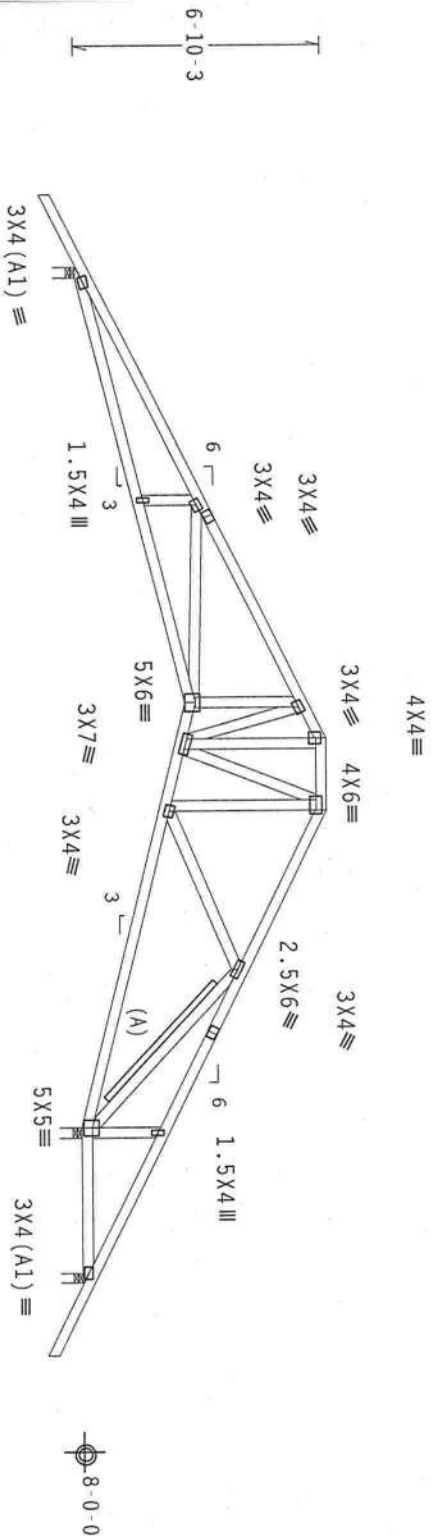
FL/-/3/-/1-/R/-	20.0 PSF	REF R487 - 26991
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TC DL	10.0 PSF	DRW HCUSR487 05196009
BC DL	0.0 PSF	HC-ENG GDL/AF
BC LL	40.0 PSF	SEQN- 7333
TOT.LD.	1.25	
DUR.FAC.	24.0"	JREF- 1S0R487_205
SPACING		

Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

(A) 2x4 SP #3 or better "T" brace. 80% length of web member.  
 Attach with 1ed Box or Gun (0.135"x3.5", min.) nails @ 6" OC.  
 Shim all supports to solid bearing.

110 mph wind, 11.10 ft mean hgt, ASCE 7-98, CLOSED bldg, not  
 located within 4.50 ft from roof edge, CAT II, EXP B, wind TC  
 DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.  
 \* Negative reaction(s) of -235# MAX. (See below) from a  
 non-wind load case requires special uplift connection.  
 Connection to be designed and furnished by others.



PLT TYP. Wave TPI

Design Crit: TPI-1995 (STD)/FBC

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PARTS OF THE INTERNATIONAL BUILDING CODE, 6300 INTERPRET. L.N. D. (BORO RD. DR., SUITE 200, HOUSTON, TX 77056) AND PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. HOUSTON, TX 77056) FOR SAFETY INFORMATION. THE DESIGNER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOD (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (K/H/S/K) ASTM A653 GRADE 40/50 (K/H/S) GALV. STEEL. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SECS.3. THE TRUSS COMPONENTS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEE THE TRUSS COMPONENT DESIGN SHEET. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 1950 Marley Drive  
 Gaines City, FL 33844  
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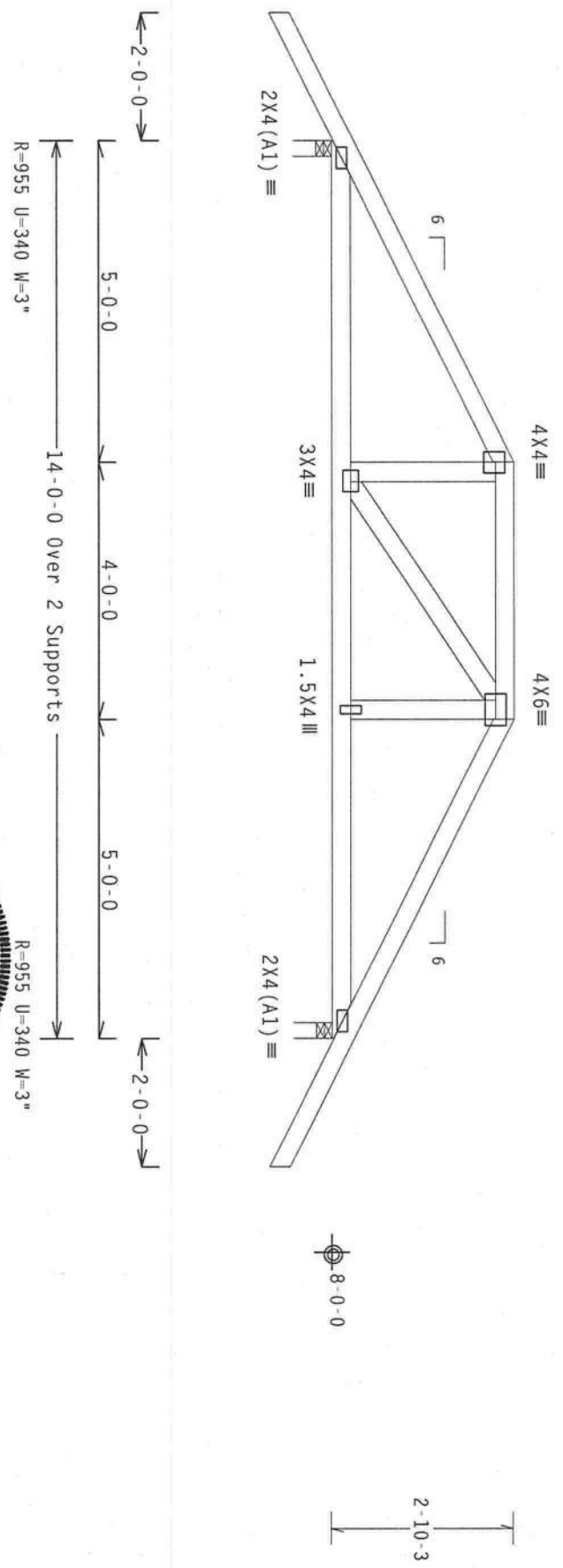
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TC DL	10.0 PSF	DATE 07/15/05
BC DL	10.0 PSF	DRW HCUSR487 05196010
BC LL	0.0 PSF	HC-ENG GDL/AF
TOT.LD.	40.0 PSF	SEQN - 7328
DUR.FAC.	1.25	
SPACING	24.0"	JREF - 150R487_205

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #3  
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 5'-0" jacks with no webs.

Deflection meets L/360 live and L/240 total load.



PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC 7.0

ARTHUR B. FISHER  
Professional Engineer  
No. 59687  
July 15 '05

Scale = .375" / Ft.

**ALPINE**  
Alpine Engineered Products, Inc.  
1950 Marney Drive  
Haines City, FL 33844  
FL Certificate of Authorization #567

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 585 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETRIORATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. CALLING AND BUILDING ON THE ALPINE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC. BY AIA/CES) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/15GA. (20-H/18/S/15) ASTM A573 GRADE 40/60 (40, K/18/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF SP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.



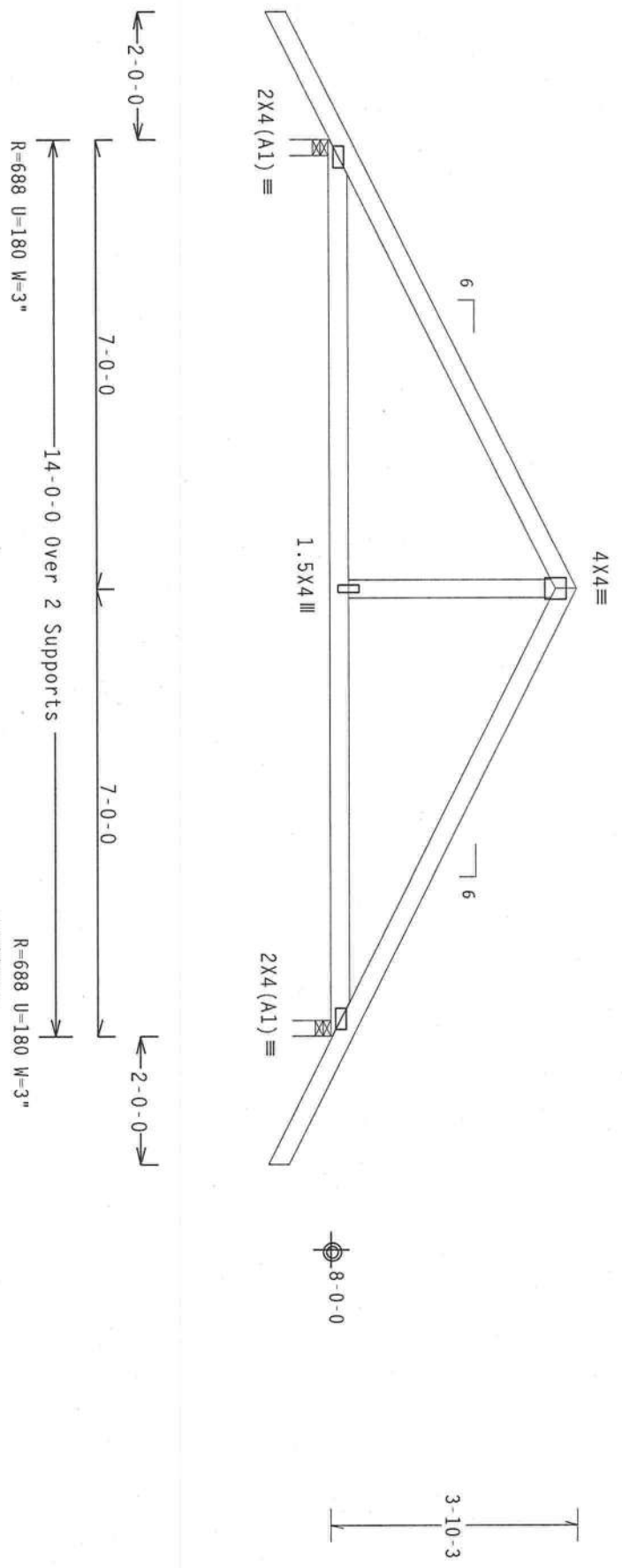
FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL 20.0 PSF	REF R487 - 26993
TC DL 10.0 PSF	DATE 07/15/05
BC DL 10.0 PSF	DRW HCUSR487 05196011
BC LL 0.0 PSF	HC-ENG GDL/AF
TOT. LD. 40.0 PSF	SEQN - 7296
DUR. FAC. 1.25	
SPACING 24.0"	JREF - 1SOR487_205

(5-308-MIKE TODD / JASON LITTLE - B)  
 Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 9.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, Wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load.

THIS DRG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY 18033.PFK.



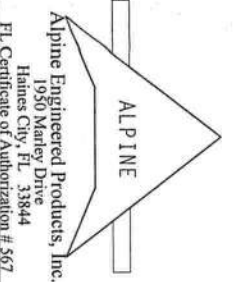
PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.0

FL/-/3/-/1-/R/-

Scale = .375" / Ft.



Alpine Engineered Products, Inc.  
 1930 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization #567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (WOOD PRESERVATION) BOARD OF CONSTRUCTION, 1200 N. WASHINGTON, WASHINGTON, DC 20004-4201, AND THE (WOOD PRESERVATION) BOARD OF CONSTRUCTION, 1200 N. WASHINGTON, WASHINGTON, DC 20004-4201, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ARMY) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/N/S/K) ASTM A653 GRADE 40/60 (W, K/H/S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AM51/TPI 1 SEC. 2.

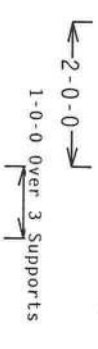


TC LL	20.0 PSF	REF R487-- 26994
TC DL	10.0 PSF	DATE 07/15/05
BC DL	10.0 PSF	DRW HCUSR487 05196012
BC LL	0.0 PSF	HC-ENG GDL/AF
TOT. LD.	40.0 PSF	SEON- 7300
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1S0R487_205

Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load.

110 mph wind, 8.10 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave TPI Design Cnt: TPI-1995 (STD) /FBC

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. CONSULT THE TRUSS MANUFACTURER FOR A COMPLETE SET OF TRUSS AND BRACING DRAWINGS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION AND BRACING OF THE TRUSS. THE TRUSS MANUFACTURER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

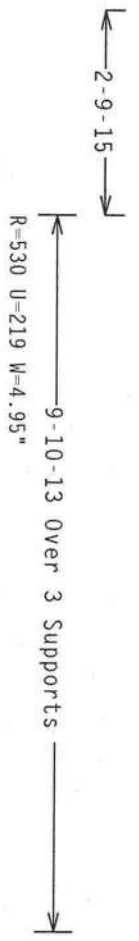
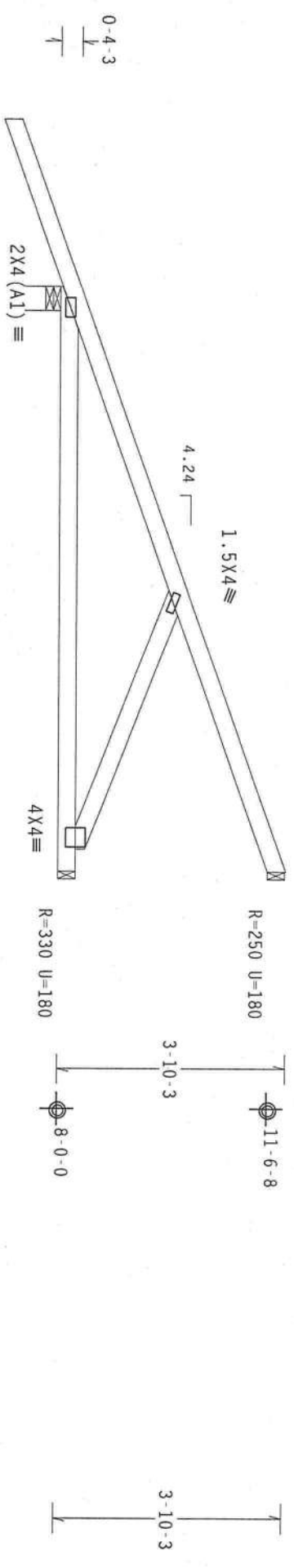
**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FABRICATOR TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (A501, A502, A515, A588, A674, A992, A995, A996, A997, A998, A999, A1000, A1001, A1002, A1003, A1004, A1005, A1006, A1007, A1008, A1009, A1010, A1011, A1012, A1013, A1014, A1015, A1016, A1017, A1018, A1019, A1020, A1021, A1022, A1023, A1024, A1025, A1026, A1027, A1028, A1029, A1030, A1031, A1032, A1033, A1034, A1035, A1036, A1037, A1038, A1039, A1040, A1041, A1042, A1043, A1044, A1045, A1046, A1047, A1048, A1049, A1050, A1051, A1052, A1053, A1054, A1055, A1056, A1057, A1058, A1059, A1060, A1061, A1062, A1063, A1064, A1065, A1066, A1067, A1068, A1069, A1070, A1071, A1072, A1073, A1074, A1075, A1076, A1077, A1078, A1079, A1080, A1081, A1082, A1083, A1084, A1085, A1086, A1087, A1088, A1089, A1090, A1091, A1092, A1093, A1094, A1095, A1096, A1097, A1098, A1099, A1100, A1101, A1102, A1103, A1104, A1105, A1106, A1107, A1108, A1109, A1110, A1111, A1112, A1113, A1114, A1115, A1116, A1117, A1118, A1119, A1120, A1121, A1122, A1123, A1124, A1125, 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Top chord 2x4 SP #2 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Hipjack supports 7'-0" setback jacks with no webs.  
 Provide ( 2 ) 16d common nails (0.162"x3.5"); toe nailed at Top chord.  
 Provide ( 3 ) 16d common nails (0.162"x3.5"); toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, located  
 anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0  
 psf.

Deflection meets L/360 live and L/240 total load.



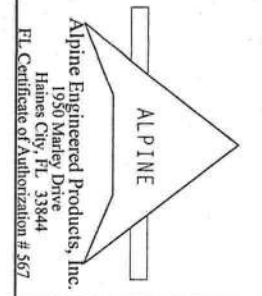
PLT TYP. Wave TPI

Design Cmt: TPI-1995 (STD)/FBC

7.0

FL/-/3/-/R/-

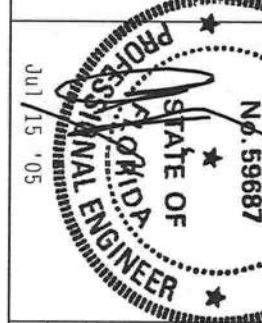
Scale = .375" / Ft.



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 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DECS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 589 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBD CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE RESPONSIBILITY OF THE CONTRACTOR. INSTALLING AND BRACING OF TRUSSES, CONNECTOR PLATES ARE MADE OF 2018/1606 (A/R/S/X) ASIN A653 GRADE 40/50 (A/R/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



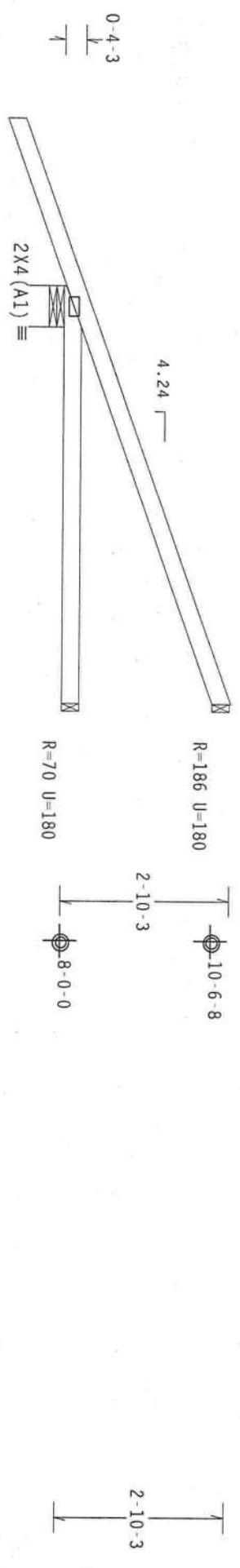
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TOT.LD.	40.0 PSF	SEQN- 7288
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SOR487_205

Top Chord 2x4 SP #2 Dense  
Bot Chord 2x4 SP #2 Dense

Hipjack supports 5-0-0 setback jacks with no webs.

Provide { 2 } 16d common nails (0.162"x3.5"); toe nailed at Top chord.  
Provide { 2 } 16d common nails (0.162"x3.5"); toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
Deflection meets L/360 live and L/240 total load.



← 2-9-15 →  
← 7-0-14 Over 3 Supports →  
R=386 U=180 W=8.485"

PLT TYP. Wave TPI

Design Cmt: TPI-1995(STD)/FBC

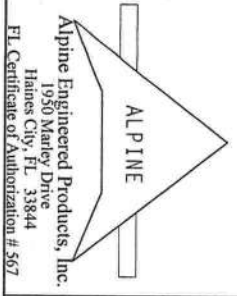
7.0

2 FL/-/3/-/1-/R/-

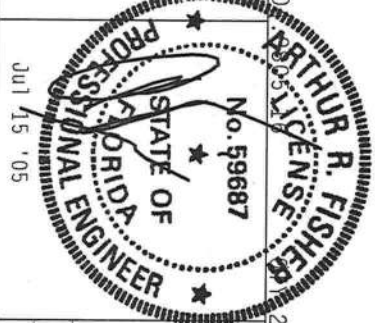
Scale = .375" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 NORTH FORT WORTH, TEXAS, 76102, AND WCA GOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN, HOUSTON, TX 77036. THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OR DAMAGE TO THE TRUSS IN COMPLIANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AF&P) AND TPI-1. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (W/ K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



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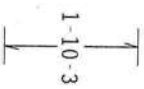
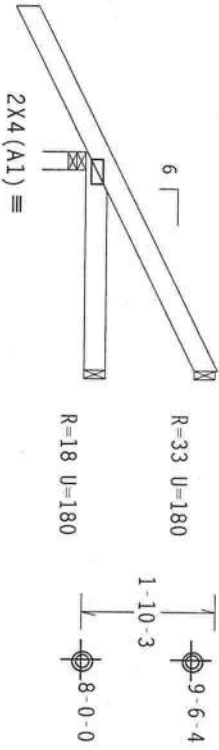


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TC DL	10.0 PSF	DATE	07/15/05
BC DL	10.0 PSF	DRW	HCUSR487 05196015
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TOT.LD.	40.0 PSF	SEQN-	7292
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SOR487 Z05

Top Chord 2x4 SP #2 Dense  
Bot Chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load.

110 mph wind, 8.60 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave TPI

Design Cnt: TPI-1995(STD)/FBC

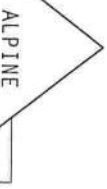
7.04

12 FL/-/3/-/R/-

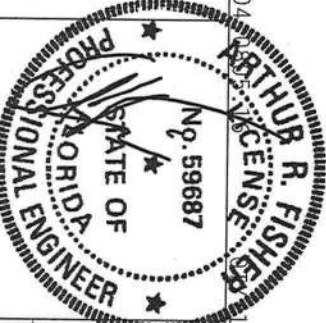
Scale = .375"/ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 503 D'BORRHO DR., SUITE 200, WAUWATON, WI 52779, AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, TOLSON, WI 52779) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN COMPROMISES WITH APPLICABLE PROVISIONS OF AIA (INTERNATIONAL ASSOCIATION OF STRUCTURAL ENGINEERS) AND THE DESIGNER'S PROFESSIONAL LIABILITY INSURANCE POLICY SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI REQUIREMENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN COMPROMISES WITH APPLICABLE PROVISIONS OF AIA (INTERNATIONAL ASSOCIATION OF STRUCTURAL ENGINEERS) AND THE DESIGNER'S PROFESSIONAL LIABILITY INSURANCE POLICY SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.



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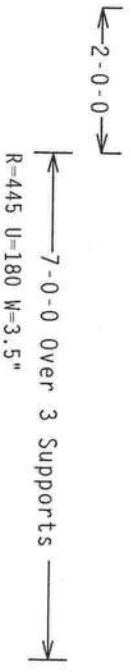
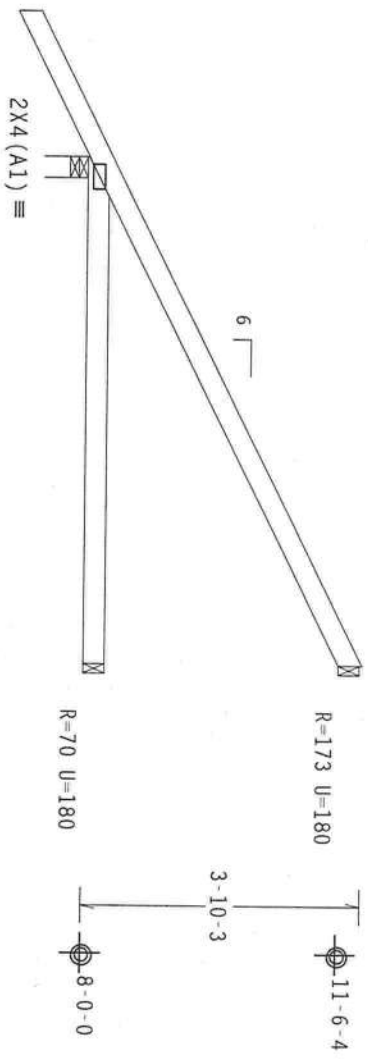
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TOT. LD.	40.0 PSF	SEQN- 7272
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1S0R487_Z05



Top chord 2x4 SP #2 Dense  
Bot chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load.

110 mph wind, 9.60 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



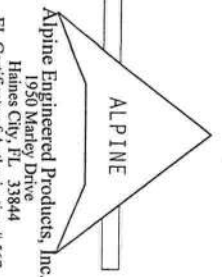
PLT TYP. Wave TPI

Design Crit: TPI-1995(STD)/FBC

7.0

16 FL-/3/-/R/-

Scale = .375"/ft.



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FL Certificate of Authorization #567

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS OR THE INSTALLATION OF THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, DESIGN SPEC. (BY A/R/P/A AND BRACING OF TRUSSES. DESIGN COURTESY WITH APPLICABLE PROVISIONS OF AIAA 6000 GRADE 40/60 (A/R/S) GALV. STEEL. APPLICABLE CONNECTOR PLATES ARE MADE OF 20/18/2166A (A/N/S/R) ASTM A653 GRADE 40/60 (A/R/S) GALV. STEEL. APPLICABLE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487 - 27000
TC DL	10.0 PSF	DATE	07/15/05
BC DL	10.0 PSF	DRW	HCUSR487 05196018
BC LL	0.0 PSF	HC-ENG	GDL/AF
TOT. LD.	40.0 PSF	SEQN-	7281
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1S0R487_Z05

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

### NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

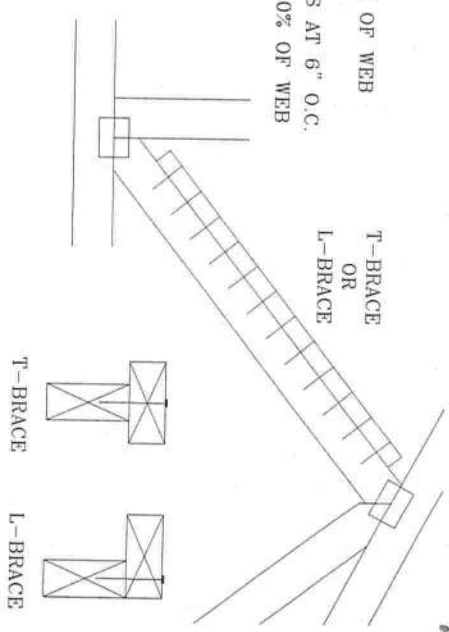
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

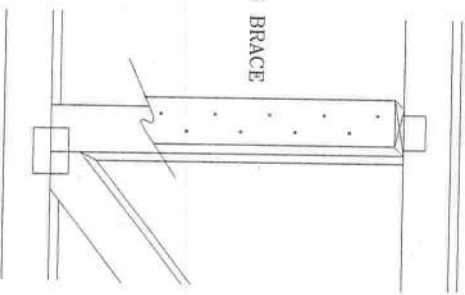
### T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE ATTACH WITH 16d NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH

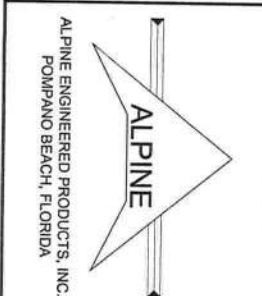


### SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



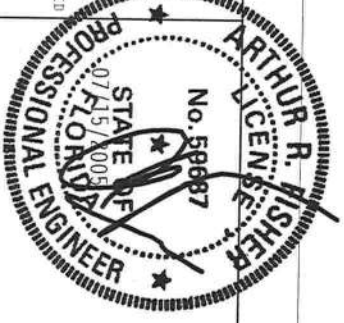
THIS DRAWING REPLACES DRAWING 579,640



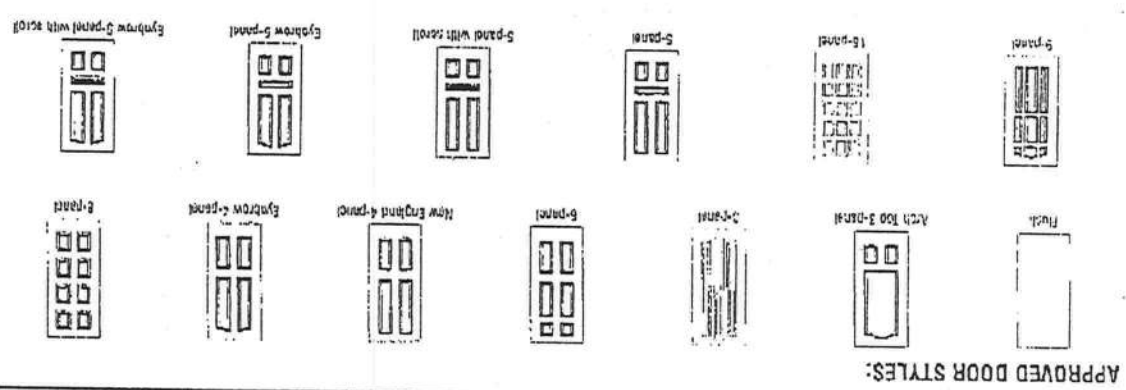
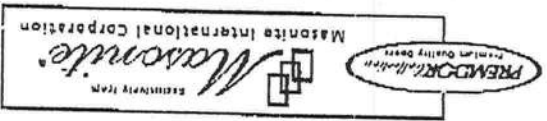
ALPINE ENGINEERED PRODUCTS, INC.  
POMPANO BEACH, FLORIDA

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE PLATE INSTITUTE, 583 DUNDRAFF DR., SUITE 200, MADISON, WI 53719, AND VITA (WOOD TRUSS CONSTRUCTION) PUBLISHED BY THE NATIONAL WOOD TRUSS ASSOCIATION, 6300 ENTERPRISE LN, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPRUDENT\*\*** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN AND FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE BRACING PROVISIONS OF THIS DESIGN. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND THE TRUSS COMPONENTS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND THE TRUSS COMPONENTS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND THE TRUSS COMPONENTS.



TC	LL	PSF	REF	CLB	SUBST.
TC	DL	PSF	DATE	11/26/03	
BC	DL	PSF	DRWG	BRCLBSUB1103	
BC	LL	PSF	-ENG	MLH/KAR	
TOT.	LD.	PSF			
DUR.	FAC.				
SPACING					



**MINIMUM ASSEMBLY DETAIL:**  
Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02.

**MINIMUM INSTALLATION DETAIL:**  
Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

**Large Missile Impact Resistance**  
Design Pressure +66.0/-66.0  
Maximum unit size = 3' x 6'  
Single Door  
Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by AISC 7-national. Initial water impact tested breakout design is used.

**APPROVED ARRANGEMENT:**

Note: Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Take Data Review Certificate #1026417 and OCF/101 Report Verification Matrix #1026417-001 provides additional information - available from the IIS/WH website (www.cslab.com), the website (www.cslab.com), the website (www.cslab.com) or the Masonite technical center.

**WOOD-EDGE STEEL DOORS**

X  
Opaque Inswing Unit

CDP-W1-JH4101-N2

Label 17, 2003  
For additional information on Johnson's products, visit our website at [www.johnsonentry.com](http://www.johnsonentry.com) or call 1-800-368-3333.



2

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

*Kurt A. Balthazor*

Kurt Balthazor, P.E. - License Number 56533  
State of Florida, Professional Engineer

Test Data Report Form  
Masonite International Corporation  
Masonite is a registered trademark of Masonite International Corporation.  
Masonite website ([www.masonite.com](http://www.masonite.com))  
Masonite (www.masonite.com) information - available from 7:15 AM to 5:00 PM, Monday through Friday.  
Masonite International Corporation  
Masonite International Corporation

TESTED IN ACCORDANCE WITH  
MIAMI-DADE BCCO  
PA201, PA202 & PA203  
COMPANY NAME  
CITY, STATE

**PRODUCT COMPLIANCE LABELING:**

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.  
Door panels constructed from 26-gauge 0.017" thick steel skins. Both sides constructed from wood.  
Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.  
Frame constructed of wood with an extruded aluminum threshold.

**CERTIFIED TEST REPORTS:**

NCTL 210-2185-1, 2, 3

**WOOD-EDGE STEEL DOORS**

X  
Opaque Inswing Unit

COP-VL-JH4101-02

130 Perry Court  
 York, PA 17402-9405  
 phone: 717.764.7700  
 fax: 717.764.4129  
 www.lesstest.com

AAE:ajp

Adam A. Fedor, Technician

For ARCHITECTURAL TESTING, INC.

Reference should be made to Report No. 01-37589.01 for complete test specimen description and data.

Results	
Title of Test	Test Specimen #1
Overall Design Pressure	30 psf
Operating Force	20 lb max.
Air Infiltration	0.27 cfm/ft <sup>2</sup>
Water Resistance	5.25 psf
Structural Test Pressure	±45.0 psf
Degazing	Passed
Forced Entry Resistance	Grade 10
	Test Specimen #2
	47 psf
	N/A
	N/A
	6.0 psf
	±70.5 psf
	N/A
	N/A

MI HOME PRODUCTS, INC.  
 SERIES/MODEL: 450  
 TYPE: Aluminum Single Hung Window  
 RATING: H-C30 54 x 90; H-C45 52 x 72\*

Rendered to:

AA/A/N/W/W/D/A 101/I/S-2-97  
 TEST REPORT SUMMARY

Architectural Testing



MI HOME PRODUCTS, INC.



AAMA/NWDA 101/S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INCORPORATED  
 650 West Market Street  
 Gratz, Pennsylvania 17030-0370

Report No: 01-37589.01  
 Test Date: 06/29/00  
 Report Date: 09/11/00  
 Expiration Date: 06/29/04

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted to witness tests on a Series/Model 450, aluminum single hung window at the MI Home Products in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 H-C30 54 x 90; Test Specimen #2 H-C40 52 x 72\*. Test specimen descriptions and results are reported herein.

**General Note:** An asterisk (\*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWDA 101/S.2-97, Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Class Doors.

Test Specimen Description

Series/Model: 450

Type: Aluminum Single Hung Window

Test Specimen #1 H-C30 54 x 90

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court  
 York, PA 17402-9405  
 phone: 717.764.7700  
 fax: 717.764.4129  
 www.testall.com

Test Specimen Description: (Continued)

Test Specimen #: 11-C40 52 x 72\*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" thick clear annealed glass and an Intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

Description	Quantity	Location
0.210" high by 0.270" backed polypropylene with center fin	Row	Fixed mcleing rail
0.250" high by 0.187" backed polypropylene with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam-filled vinyl bulb gasket	Row	Bottom rail
0.400" high by 1/2" square polypropylene dust plug	4	One on each sash corner

Frame Construction: The main frame was constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners. The fixed mcleing rail was connected of an extruded aluminum member with coped, butted and sealed ends fastened with two screws each.



Test Specimen Description: (Continued)

Sash Construction: The sash members were constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners fastened with one screw each.

Screen Construction: The screen was constructed of rolled aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Plastic snap latch	1	Midsparn of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail
Drainage: Sloped sill		

Reinforcement: No reinforcement was utilized.

Installation: The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood fast back utilizing the integral nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blindstopped with wood members and secured with #8 x 3" screws every 24" on center.



Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force Test Specimen #1: H-C30 54 x 90	20 lbs	45 lbs max.
	Air Infiltration per ASTM E 283 (See Note #1) @ 1.57 psf (25 mph)	0.27 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max.

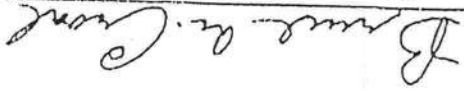
Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/NWWDA 101/1.S. 2-97 for air infiltration.

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) @ 45.0 psf (exterior) @ 45.0 psf (interior)	0.03" 0.04"	0.22" max. 0.22" max.
2.2.1.6.2	De-glazing Test per ASTM E 987 In operating direction at 70 lbs Meeting rail Bottom rail In remaining direction at 50 lbs Left stile Right stile	0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
	Water Resistance per ASTM E 547 (with and without screen) WTP = 4.5 psf No leakage	No leakage	No leakage
	Lock Manipulation Test Test A1 through A5 Test A7 Lock Manipulation Test	No entry No entry No entry	No entry No entry No entry
	Forced Entry Resistance per ASTM E 588-97 Type: A Grade: 10	No entry	No entry

Adam A. Fodor  
Technician



Bruce W. Cronk  
Director - Product/Physical Testing



FOR ARCHITECTURAL TESTING, INC.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

Paragraph	Title of Test - Test Method	Results	Allowed
4.1.1	Water Resistance per ASTM E 547 (with and without screen) WTP - 5.25 psf	No leakage	Allowed
Optional Performance			
Test Specimen #1: (Continued)			
4.1.2	Water Resistance per ASTM E 547 and 331 (with and without screen) WTP - 6.0 psf	No leakage	Allowed
Optional Performance			
Test Specimen #2: 11-C140 52 X 72*			
4.1.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed masonry rail) (Loads held for 33 seconds) (@ 47.0 psf (exterior) (@ 47.0 psf (interior)	0.04" 0.03"	N/A N/A
	(Loads held for 10 seconds) (@ 70.5 psf (exterior) (@ 70.5 psf (interior)	0.07" 0.04"	0.21" max. 0.21" max.



TAMKO Roofing Products, Inc.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

All testing was performed by Florida State certified independent labs.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

TO: OUR FLORIDA CUSTOMERS:

January 31, 2002

**TAMKO**  
ROOFING PRODUCTS

LAMAR BOOZER  
 900 EAST PUTNAM STREET  
 LAKE CITY, FLORIDA 32055

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

CLIENT INFORMATION:  
 NAME: MIKE TODD  
 ADDRESS: 129 COLBURN AVENUE  
 LAKE CITY, FL 32055

PROJECT: CUSTOM  
 CLIENT: LITTLE  
 DATE: 05-05-05

DESIGNER: L. BOOZER

BLDG. LOAD	AREA	SEN. LOSS	LAT. GAIN	SEN. GAIN	TOTAL
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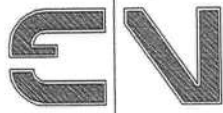
3-C WINDOW DBL PANE CLR GLS METL FR	85	2,773	0	2,964	2,964
12 - D WALL R-11 + ½" ASPHLT BRD (R-1.3)	929	3,344	0	1,828	1,828
11 - C DOOR METAL POLYSTYRENE CORE	60	1,269	0	693	693
16 - G CEILING R - 30 INSULATION	1,040	1,545	0	1,545	1,545
22 - A SLAB ON GRADE NO EDG INSUL	135	4,921	0	0	0
SUBTOTALS FOR STRUCTURES					
PEOPLE	12	0	0	3,600	3,600
APPLIANCES	0	0	800	1,800	2,600
DUCTWORK	0	692	0	1,243	1,243
INFILTRATION W.CFM: 0.0	0	0	0	0	0
VENTILATION W.CFM: 0.0	0	0	0	0	0

SENSIBLE GAIN TOTAL	13,673				
TEMP. SWING MULTIPLIER	X 1.00				
BUILDING LOAD TOTALS	14,544	800	13,673	14,473	

SUPPLY CFM AT 20 DEGREE DT: 622  
 SQUARE FT. OF ROOM AREA: 1,164  
 SQUARE FOOT PER TON: 0.598

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 14,544 MBH  
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 1,206 TONS

CALCULATIONS ARE BASED IN 7<sup>TH</sup> EDITION OF ACCA MANUAL J.  
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.



**NICHOLAS  
PAUL  
GEISLER  
ARCHITECT**  
1758 NW Brown Road  
Lake City, FL 32055  
386/755-9021  
N.C.A.R.B. Certified

**FLORIDA BUILDING CODE SECTION 1606**

**COMPLIANCE SUMMARY**

**PROJECT:** LITTLE RESIDENCE, COLUMBIA COUNTY, FL (110 WIND ZONE)

**TYPE OF CONSTRUCTION**

ROOF: Gable Construction, Wood Trusses @ 24" O.C., SYP  
WALLS: 2x4 Wood Studs @ 16" O.C.  
FLOOR: 4" Thk. Conc. Slab, w/ Fibermesh concrete additive  
FOUNDATION: Continuous Footer/Stemwall  
EDGE STRIP: 3.0 ft. END ZONE: 6.0 ft.

**ROOF DECKING**

MATERIAL: 7/16" O.S.B.  
SHEET SIZE: 48"x96" Sheets Placed Perpendicular to Roof Framing  
FASTENERS: 8d Common Nails @ 4" O.C. Ends, 8" O.C. Interior

**SHEAR WALLS**

MATERIAL: 7/16" O.S.B. "Windstorm Sheathing"  
SHEET SIZE: 48"x97 1/8" Sheets Placed Vertical  
FASTENERS: 8d Common Nails @ 4" O.C. Edges, 8" O.C. Interior  
DRAGSTRUT: Dbl. Top Plate Nailed w/ 12d Nails @ 16" O.C.  
WALL STUDS: S-P-F Nr. 2 and better, 2x4 Studs @ 16" O.C.

**HURRICANE UPLIFT CONNECTORS**

TRUSS CLIPS: "SEMCO" HDPT2  
WALL TENSION: 1/2" CDX plywd. w/ 8d Common Nails @ 4" O.C. Edges,  
8" O.C. Interior for all exterior non-shear walls  
HOLD-DOWN CONNECTORS: A307 Bolts, within 6" of corners  
WALL SILL: 1/2" x 10" A.B., w/ 2" washers @ 48" o.c., 7" embedment  
CORNER HOLD-DOWN DEVICE: "SIMPSON" PHD2-SDS3, Ea. Corner

**FOOTINGS AND FOUNDATIONS**

HOUSE FOOTINGS: 20"x10" Continuous w/ 2 - #5 Rebars  
HOUSE STEMWALL: 8" CMU w/ #5 Rebar DOWELS Gd. 40, @ 72" O.C.  
CONCRETE: Fb = 2500 p.s.i. or greater

**PREPARER'S CERTIFICATION**

I hereby certify that the attached Wind Load Design and Analysis  
calculations are in compliance with the Florida Building Code,  
Section 1606, to the best of my knowledge and belief.

Nicholas Paul Geisler, Architect AR0007005

Date: 12/2/2005

AL7005 12/2/2005  
mcd

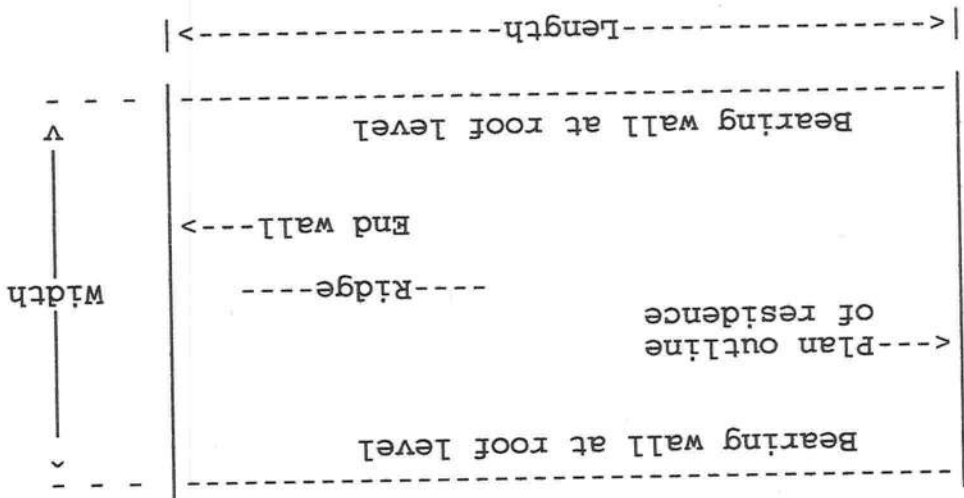
Assume that this building is an 'Enclosed building' per code 1606.2.3.

\*\*\*\* DEGREE OF ENCLOSURE \*\*\*\*

Wind velocity = 110 mph

Roof cross slope = 6 / 12

Length along bearing walls out to out of studs (block) = 44 feet  
Width along end walls out to out of studs (block) = 28 feet  
Roof overhang in long dir. from outer face of stud (block) = 2 feet gen.  
Roof overhang at short end wall from outer face of stud (block) = 2 feet  
Height of exterior wall to top of plate (tie bm) on long side = 8 feet



permanent construction  
simple rectangular building

\*\*\*\* GENERAL INPUT DATA \*\*\*\*

W I N D   D E S I G N  
Version 1.0  
Copyright 1998---EDA Software, Inc.  
Based on the Standard Building Code, 1997 edition

Project name: LITTLE  
Location : COLUMBIA COUNTY

Data entry by: MT      Date: 9 / 22 / 05

\*\*\* STRUCTURAL FRAMING INPUT DATA \*\*\*

\*\*\* Roof Structural Data \*\*\*

Member number 1  
Common truss--supported by exterior walls only  
Span length out to out of supports = 28 feet  
Roof cross slope = 6 / 12  
Truss spacing = 24 inches  
Overhang = 2 feet

\*\*\* Wall Structural Data \*\*\*

Spacing of wall studs = 16 inches  
Total thickness of plates = .375 feet  
Wall stud number 1 is 8 feet high out to out of plates  
\*\*\* Wall Structural Data \*\*\*

Spacing of wall studs = 16 inches  
Total thickness of plates = .375 feet  
Wall stud number 2 is 8 feet high out to out of plates  
Wall stud number 3 is 8 feet high out to out of plates  
Wall stud number 4 is 8 feet high out to out of plates  
Wall stud number 5 is 8 feet high out to out of plates  
Wall stud number 6 is 8 feet high out to out of plates  
Wall stud number 7 is 8 feet high out to out of plates  
Wall stud number 8 is 8 feet high out to out of plates

COEFFICIENTS AND PRESSURES

Main Wind Force Resisting Systems  
 actual wind pressure = Velocity pressure x Use factor x Coefficient

Wind velocity is 110 mph

Mean roof height is 11.87268 feet

Velocity pressure is 24.7 psf

Use factor is 1.0

roof cross slope is 6 on 12, which equals 26.56505 degrees to horizontal  
 and zone width is 6 feet

For illustration see Code, Figure 1606.2B1.

-----  
 Coefficient      Actual Pressure (psf)

Zone	Coefficient	Actual Pressure (psf)
End zone:		
Windward wall (1E)	.7	17.29
Windward roof (2E)	-1	-24.7
Leeward roof (3E)	-1	-24.7
Leeward wall (4E)	-.95	-23.47
Interior zone:		
Windward wall (1)	.4	9.88
Windward roof (2)	-.75	-18.53
Leeward roof (3)	-.75	-18.53
Leeward wall (4)	-.7	-17.3

TOTAL WIND FORCES ON ENTIRE BUILDING  
 Main Wind Force Resisting Systems

Forces Transverse:	Value
Lateral forces (pounds):	
Windward wall	= 3860 inward
Leeward wall	= 6157 outward
Uplift forces (pounds):	
Windward roof	= 12451 upward
Leeward roof	= 12451 upward
Windward overhang	= 3261 upward
Forces Longitudinal:	
Lateral forces (pounds):	
Windward wall	= 3860 inward
Leeward wall	= 1092 outward

COEFFICIENTS AND PRESSURES  
Roof components

Actual wind pressure = Velocity pressure x Use factor x Coefficient  
 Velocity pressure = 24.7 psf  
 Use factor = 1.0  
 End zone width = 6 feet  
 Edge strip width = 3 feet

Common truss 1 --Span 28 ft., Spacing 24 in., Slope 6/12, Overhang 2 ft.  
 Member located in interior zone: Tributary area = 261.3333 square feet  
 Interior area : Coefficient = -1.1  
 Eave and ridge areas: Coefficient = -1.2  
 Overhang area : Coefficient = -1.55  
 Wind uplift at bearing = 949 pounds  
 Pressure = -27.171 psf  
 Pressure = -29.641 psf  
 Pressure = -38.285 psf

Common truss 2 --Span 28 ft., Spacing 24 in., Slope 6/12, Overhang 2 ft.  
 Member located in end zone: Tributary area = 261.3333 square feet  
 Interior area : Coefficient = -1.8  
 Eave and ridge areas: Coefficient = -1.8  
 Overhang area : Coefficient = -1.6  
 Wind uplift at bearing = 1408 pounds  
 Pressure = -44.46 psf  
 Pressure = -44.46 psf  
 Pressure = -39.52 psf

COEFFICIENTS AND PRESSURES  
Wall components

Actual wind pressure = Velocity pressure x Use factor x Coefficient  
 Velocity pressure = 24.7 psf  
 Use factor = 1.0  
 Edge strip width = 3 feet

Wall stud number 1 --Stud height 7.625 feet, Spacing 16 inches  
 Stud located in interior zone: Tributary area = 19.38021 square feet  
 Coefficient = -1.297  
 Pressure = -32.036 psf  
 Outward wind force on stud = 325 pounds

Wall stud number 2 --Stud height 7.625 feet, Spacing 16 inches  
 Stud located in end zone: Tributary area = 19.38021 square feet  
 Coefficient = -1.493  
 Pressure = -36.878 psf  
 Outward wind force on stud = 374 pounds

ROOF LOADING--Roof Number 1 (pounds per square foot)

Roof cross slope = 6 inches per foot	
-----	
Fiberglass shingles 220 # per square and 1 layer of 15 # felt	= 2.35
No insulation	
7/16 in. roof sheathing, 1 layer	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
-----	
Total roof unit weight on slope	= 5.875147
Cosine of roof cross slope	= .8944272
-----	
Roof unit weight on horizontal	= 6.568614
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
=====	
Total	= 10.36862
Roof Unit Dead Load = 11 psf	

Roof dead load supported generally by wall = 156.6606 pif

ROOF LOADING--Roof Number 2 (pounds per square foot)

Roof cross slope = 6 inches per foot	
-----	
Fiberglass shingles 220 # per square and 1 layer of 15 # felt	= 2.35
No insulation	
7/16 in. roof sheathing, 1 layer	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
-----	
Total roof unit weight on slope	= 5.875147
Cosine of roof cross slope	= .8944272
-----	
Roof unit weight on horizontal	= 6.568614
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
=====	
Total	= 10.36862
Roof Unit Dead Load = 11 psf	

Roof dead load supported generally by wall = 156.6606 pif

ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS (pounds)

Roof member number 1 --Span 28 feet, Slope 6 /12, interior zone----- 313

Roof member number 2 --Span 28 feet, Slope 6 /12, end zone----- 313

EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 8 ft. out to out plates

3--2 in. x 4 in. plates	= 2.865625
2 in. x 4 in. studs at 16 in. spacing	= 5.462598
R-13 Insulation	= 1.90625
Brick veneer siding	= 10.5
1/2 in. Gypsum board--Total 1 layer---	= 16
-----	
Total	= 37

Exterior wall Dead Load = 37 plf

SUMMARY OF HURRICANE ANCHOR ANALYSIS

All values of forces are in pounds. Resistances were increased by 60 percent.

End zone width = 6 feet

Code: C = Compliance

N = Non-compliance

SEMCO hurricane anchors

Member 1 --Common truss--Span 28 feet, at 24 in. oc--in interior zone:

Uplift = 949 Dead = 313 Net = 636 Model Special, Resistance = 1248 C

Model HDPT2--all nails per mfr.--data supplied by operator, not EDA

Member 2 --Common truss--Span 28 feet, at 24 in. oc--in end zone:

Uplift = 1408 Dead = 313 Net = 1095 Model Special, Resistance = 1248 C

Model HDPT2--all nails per mfr.--data supplied by operator, not EDA

\*\*\* TRANSVERSE SHEARWALL DIMENSIONS \*\*\*

Transverse Shearwall Segment Analysis:

Segment	Left End	H	L	H/L	Right End	H	L	H/L
Segment ST1	LEFT END	96 in.	326 in.	.294	MASTER BDRM	96 in.	48 in.	2 < 3.5
Segment ST2	MASTER BDRM	96 in.	48 in.	H/L = 2 < 3.5	BEDROOM 2	96 in.	48 in.	H/L = 2 < 3.5
Segment ST3	BEDROOM 2	96 in.	48 in.	H/L = 2 < 3.5	MASTER BEDROOM	96 in.	186 in.	H/L = .516 < 3.5
Segment ST4	MASTER BEDROOM	96 in.	186 in.	H/L = .516 < 3.5	MASTER BATH	96 in.	42 in.	H/L = 2.285 < 3.5
Segment ST5	MASTER BATH	96 in.	42 in.	H/L = 2.285 < 3.5	PANTRY	96 in.	36 in.	H/L = 2.666 < 3.5
Segment ST6	PANTRY	96 in.	36 in.	H/L = 2.666 < 3.5				

\*\*\* ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE \*\*\*

shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine Lumber, spaced at 24 in.

Sheathing is Oriented Strand Board, 7/16 inch thick

Sheathing has no intermediate blocking

Fasteners on panel ends are 8d common nails spaced 4 in.

Fasteners in panel interior are 8d common nails spaced 8 in.

Total lateral wind force on building = 10017 pounds

Total force transferred through diaphragm to shearwalls = 5008 pounds

Total length of shearwalls = 57.16666 feet

MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 15.9 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 87 plf

Allowable diaphragm force per unit length of shearwall = 314 plf

\*\*\* Summary of Analysis \*\*\*

Roof sheathing diaphragm satisfies code requirements.

\*\*\* LONGITUDINAL SHEARWALL DIMENSIONS \*\*\*

Longitudinal Shearwall Segment Analysis:

Segment S11, BEDROOM 2, H = 96 in., L = 48 in., H/L = 2 < 3.5
Segment S12, BEDROOM 2, H = 96 in., L = 48 in., H/L = 2 < 3.5
Segment S13, MASTER BDRM, H = 96 in., L = 42 in., H/L = 2.285 < 3.5
Segment S14, MASTER BDRM, H = 96 in., L = 42 in., H/L = 2.285 < 3.5
Segment S15, LAUNDRY, H = 96 in., L = 66 in., H/L = 1.454 < 3.5
Segment S16, DINING, H = 96 in., L = 102 in., H/L = .941 < 3.5
Segment S17, KITCHEN, H = 96 in., L = 78 in., H/L = 1.23 < 3.5
Segment S18, KITCHEN, H = 96 in., L = 114 in., H/L = .842 < 3.5

\*\*\* ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL \*\*\*

Shear analysis applies along supporting shearwalls. Roof trusses are Southern pine lumber, spaced at 24 in. Sheathing is Oriented Strand Board, 7/16 inch thick. Sheathing has no intermediate blocking. Fasteners on panel ends are 8d common nails spaced 4 in. Fasteners in panel interior are 8d common nails spaced 8 in.

Total lateral wind force on building = 4952 pounds  
 Total force transferred through diaphragm to shearwalls = 2476 pounds  
 Total length of shearwalls = 45 feet  
 MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 7.9 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 55 plf  
 Allowable diaphragm force per unit length of shearwall = 314 plf

\*\*\* Summary of Analysis \*\*\*

Roof sheathing diaphragm satisfies code requirements.

\*\*\* ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL \*\*\*

Interior zone (area R1)  
Roof trusses are Southern Pine Lumber, spaced at 24 inches  
Sheathing is 7/16 inch with no intermediate blocking  
Size of sheathing is 48 inches by 96 inches  
Fasteners along end trusses are 8d nails spaced 4 in.  
Fasteners along int. trusses are 8d nails spaced 8 in.  
Total outward wind force on sheathing = 824 pounds  
Total withdrawal resistance of 47 common nails = 3569 pounds  
Fastening of roof sheathing satisfies code requirements.

Edge strip (area S1) width = 3 feet  
Roof trusses are Southern Pine Lumber, spaced at 24 inches  
Sheathing is 7/16 inch with no intermediate blocking  
Size of sheathing is 48 inches by 96 inches  
Fasteners along end trusses are 8d nails spaced 4 in.  
Fasteners along int. trusses are 8d nails spaced 8 in.  
Total outward wind force on sheathing = 1269 pounds  
Total withdrawal resistance of 47 common nails = 3569 pounds  
Fastening of roof sheathing satisfies code requirements.

End zone (areas Se and C) width = 6 feet  
Roof trusses are Southern Pine Lumber, spaced at 24 inches  
Sheathing is 7/16 inch with no intermediate blocking  
Size of sheathing is 48 inches by 96 inches  
Fasteners along end truss are 8d nails spaced 4 in.  
Fasteners along wall are 8d nails spaced 4 in.  
Fasteners along int. trusses are 8d nails spaced 8 in.  
Total outward wind force on sheathing = 1705 pounds  
Total withdrawal resistance of 47 common nails = 3569 pounds  
Fastening of roof sheathing satisfies code requirements.

\*\*\* ANALYSIS OF WALL STUD NUMBER 1 \*\*\*

2 in. x 4 in. single stud at 16 in. spacing  
Stud height is 7.625 feet--located in interior zone  
Spruce--Pine--Fir Lumber--Number 1--Number 2 grade  
Sheathing is 7/16 inch rated OSB, span rating 24/16  
Exterior finish is brick veneer

Total outward force on stud = 325 pounds  
Stud moment = 309 ft-lb.

Stresses:

Stud bending vert : Actual = 1213 psi Allowable = 2415 psi (adjusted)  
Stud shear : Actual = 42 psi Allowable = 112 psi (adjusted)  
Stud tensile : Actual = 51 psi Allowable = 1020 psi (adjusted)  
Interaction bending and tension actual/allowable stress ratio total = .5522774  
Sheathing bending hor : Actual = 178 psi Allowable = 222 psi (adj.)

Deflections:

Stud : Actual = .27 in. Allowable = .5083 in.

\*\*\* Summary of Analysis \*\*\*

Wood wall studs COMPLY with code requirements.

\*\*\* ANALYSIS OF WALL STUD NUMBER 2 \*\*\*

2 in. x 4 in. single stud at 16 in. spacing  
Stud height is 7.625 feet--located in end zone  
Spruce--Pine--Fir Lumber--Number 1--Number 2 grade  
Sheathing is 7/16 inch rated OSB, span rating 24/16  
Exterior finish is brick veneer

Total outward force on stud = 374 pounds  
Stud moment = 356 ft-lb.

Stresses:

Stud bending vert : Actual = 1396 psi Allowable = 2415 psi (adjusted)  
Stud shear : Actual = 49 psi Allowable = 112 psi (adjusted)  
Stud tensile : Actual = 51 psi Allowable = 1020 psi (adjusted)  
Interaction bending and tension actual/allowable stress ratio total = .6280538  
Sheathing bending hor : Actual = 205 psi Allowable = 222 psi (adj.)

Deflections:

Stud : Actual = .3107 in. Allowable = .5083 in.

\*\*\* Summary of Analysis \*\*\*

Wood wall studs COMPLY with code requirements.

T R A N S V E R S E D R A G S T R U T N A I L A N A L Y S I S

Top plate is 2 in. x 4 in. Lumber  
Fasteners are 12d common nails  
Top plate lumber is Spruce--Pine--Fir  
Approximate nail spacing = 16 inches

Total lateral force on building = 10017 pounds  
Force applied at top of walls = 5008 pounds  
Total dragstrut length = 58 feet  
Shear per unit dragstrut length = 86 pounds per linear foot

Actual shear on each nail = 114 pounds  
Allowable shear on each nail = 126 pounds

Dragstrut nailing satisfies Code requirements.

L O N G I T U D I N A L D R A G S T R U T N A I L A N A L Y S I S

Top plate is 2 in. x 4 in. Lumber  
Fasteners are 12d common nails  
Top plate lumber is Spruce--Pine--Fir  
Approximate nail spacing = 16 inches

Total lateral force on building = 4952 pounds  
Force applied at top of walls = 2476 pounds  
Total dragstrut length = 45 feet  
Shear per unit dragstrut length = 55 pounds per linear foot

Actual shear on each nail = 73 pounds  
Allowable shear on each nail = 126 pounds

Dragstrut nailing satisfies Code requirements.

\*\*\*\* LONGITUDINAL SHEARWALL ANALYSIS \*\*\*\*

Wall framing is 2 in. x 4 in. studs at 16 in. spacing  
Wall stud framing lumber is Spruce--Pine--Fir  
Shear siding is Oriented Strand Board -- 7/16 inch thick , outside  
Wall sheathing has all edges nailed  
Fasteners: 8d common nails along edges at 4 in.  
Fasteners: 8d common nails in interior at 12 in.  
Total lateral force on building = 10017 pounds  
Force applied at top of walls = 5008 pounds  
Accumulated total shearwall length = 57.1666 feet  
Actual unit shear on shearwalls = 87 plf  
Allowable unit shear on shearwalls = 322 plf  
Shearwall COMPLIES with code.

\*\*\*\* TRANSVERSE SHEARWALL ANALYSIS \*\*\*\*

Wall framing is 2 in. x 4 in. studs at 16 in. spacing  
Wall stud framing lumber is Spruce--Pine--Fir  
Shear siding is Oriented Strand Board -- 7/16 inch thick , outside  
Wall sheathing has all edges nailed  
Fasteners: 8d common nails along edges at 4 in.  
Fasteners: 8d common nails in interior at 12 in.  
Total lateral force on building = 4952 pounds  
Force applied at top of walls = 2476 pounds  
Accumulated total shearwall length = 45 feet  
Actual unit shear on shearwalls = 55 plf  
Allowable unit shear on shearwalls = 352 plf  
Shearwall COMPLIES with code.

\*\*\* ANALYSIS OF SHEATHING FASTENERS \*\*\*

Wall framing is Spruce--Pine--Fir lumber  
Sheathing is 7/16 inch Oriented Strand Board  
Sheathing extends from bottom of bottom plate to top of top plate  
Fasteners are 8d common nails at 4 inch spacing

Total uniform wind uplift in first story at top of wall level = 359 plf  
Uniform dead loads per linear foot:

Roof = 156.6606 plf

Total = 156.6606 plf

Total uniform dead load in first story at top of wall level = 156 plf  
Net wind uplift in first story at top of wall level = 203 plf

Total uplift force on each nail = 67 pounds  
Allowable shear on each nail = 97 pounds (increased for wind)  
Sheathing to plate fastening satisfies all code requirements.

\*\*\* ANALYSIS OF SHEATHING FASTENERS \*\*\*

Wall framing is Spruce--Pine--Fir lumber  
Sheathing is 7/16 inch Oriented Strand Board  
Sheathing extends from bottom of bottom plate to top of top plate  
Fasteners are 8d common nails at 4 inch spacing

Total uniform wind uplift in first story at floor level = 359 plf  
Uniform dead loads per linear foot:

Roof = 156.6606 plf

Wall = 36.73447 plf

Total = 193.3951 plf

Total uniform dead load in first story at floor level = 193 plf  
Net wind uplift in first story at floor level = 166 plf

Total uplift force on each nail = 55 pounds  
Allowable shear on each nail = 97 pounds (increased for wind)  
Sheathing to plate fastening satisfies all code requirements.

\*\*\* ANALYSIS OF FOUNDATION ANCHORAGE \*\*\*

Reinforced concrete foundation, strength 2500 psi  
Anchor bolts are 1/2 inch A307, with 2 inch round washer at 48 inches  
Anchor bolts are L-shaped  
Embedment is 7 in. from poured surface to inside of bolt at L-bend

Total uniform wind uplift on foundation = 359 plf  
Uniform dead loads in plf:

Roof = 156.6606  
Wall = 36.73447

Total = 193.3951 plf

Total uniform dead load times 2/3 = 128 plf  
Net uplift force on foundation = 231 plf

Total uplift force on each anchor bolt = 924 pounds  
Safe tension value of each anchor bolt = 1634 pounds (increased for wind)  
Anchor bolt tension value is governed by washer failure

\*\*\* Summary of Analysis \*\*\*

Foundation anchorage complies with code requirements.

\*\*\* ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS \*\*\*

Length of shearwall segment = 42 inches  
Hold-down is SIMPSON PHD2-SDS3 (data by operator, not EDA), each wall  
Normal anchor bolt spacing = 48 inches  
Minimum distance, hold-down to anchor bolt = 41 in.

Bearing wall: Distance from corner to hold-down device = 7 inches  
Bearing wall: Distance from corner to first interior anchor bolt = 48 inches  
Net uplift force on foundation = 231 pounds per linear foot  
Tributary distance to corner device = 2.291667 feet  
Net uplift on corner hold-down device = 529 pounds

Uplift tension due to shearwall action in a transverse shearwall segment:  
Distance from corner to hold-down device = 7 inches  
Distance from corner to first interior anchor bolt = 48 inches  
Total shear from shearwall segment = 306 pounds  
Height of wall = 8 feet  
Uniform dead load times 2/3 = 24 pounds per linear foot  
Shearwall moment at bottom of wall = 2453 foot-pounds  
Additional tension at corner device = 1008 pounds  
Total uplift tension on corner hold-down devices = 1537 pounds  
Allowable tension on corner hold-down devices = 5776 pounds

\*\*\* Summary of Analysis \*\*\*

Corner hold-down device COMPLIES with Code.

\*\*\*\* ANALYSIS OF FOUNDATION \*\*\*\*

Stemwall is 8 inch concrete masonry, filled with grout, 16 inches high  
Footing is 20 inches wide by 10 inches deep (including slab)  
Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 359 pounds per linear foot  
Uniform dead loads in pounds per linear foot:

Roof = 156.6606 plf  
Wall = 36.73447 plf

Total = 513.3951 plf

Total uniform dead load times 2/3 = 342 pounds per linear foot  
Net uplift force at top of foundation = 17 pounds per linear foot  
Weight of stemwall footing earth x 2/3 = 261 pounds per linear foot  
Net uplift at bottom of footing = 0 pounds per linear foot

\*\*\*\* Summary of Analysis \*\*\*\*  
Foundation is stable.

\*\*\*\* ANALYSIS OF REINFORCING STEEL \*\*\*\*

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 359 pounds per linear foot  
Uniform dead loads in pounds per linear foot:

Roof = 156.6606 plf  
Wall = 36.73447 plf  
Brick = 320 plf

Total = 513.3951 plf

Total uniform dead load times 2/3 = 342 pounds per linear foot  
Net uplift force on foundation = 17 pounds per linear foot  
Weight of concrete block stemwall x 2/3 = 81 pounds per linear foot  
Net uplift at top of footing = 0 pounds per linear foot

Total uplift force on each re-bar = 0 pounds  
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)

\*\*\*\* Summary of Analysis \*\*\*\*

Reinforcing steel satisfies all code requirements.

\*\*\*\* SUMMARY OF REINFORCING DATA \*\*\*\*

Foundation wall data:

Wall is composed of 8 inch concrete masonry, fully grouted.  
Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers.  
Minimum required lap splice for Number 5 bar is 25 inches.  
Minimum required clearance for Number 5 bar is 1.5 inches.  
For poured floor slabs--Number 5 bar extends to 1 inch below top of slab.  
Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing.

Footing data:

Footing is continuous, 20 inches wide by 10 inches deep.  
Footing concrete is 2500 psi  
Footing reinforcing is Grade 40 steel, 2--# 5 longitudinal.  
Minimum required splice length = 16 inches  
Reinforcing steel shall have cover as follows:  
Top-----6 inches  
Sides-----3 inches  
Bottom----3 inches

**COLUMBIA COUNTY BUILDING DEPARTMENT**

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR  
FLORIDA BUILDING CODE 2001  
ONE (1) AND TWO (2) FAMILY DWELLINGS  
ALL REQUIREMENTS ARE SUBJECT TO CHANGE  
EFFECTIVE MARCH 1, 2002**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

- WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.
1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
  2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
  3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

**GENERAL REQUIREMENTS:** Two (2) complete sets of plans containing the following:

Applicant  Plans Examiner

All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.

**Site Plan including:**

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

**Wind-load Engineering Summary, calculations and any details required**

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC

- a. Basic wind speed (MPH)
- b. Wind importance factor (I) and building category
- c. Wind exposure - if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- d. The applicable internal pressure coefficient
- e. Components and Cladding. The design wind pressure in terms of psf (kN/m<sup>2</sup>), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional

**Elevations including:**

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

*Sec 2 NOTE*

*PH*

- 1. All materials making up wall
- 2. Block size and mortar type with size and spacing of reinforcement
- 3. Lintel, tie-beam sizes and reinforcement
- 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
- 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
- 7. Fire resistant construction (if required)
- 8. Fireproofing requirements
- 9. Shoe type of termite treatment (termicide or alternative method)
- 10. Slab on grade
  - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
  - b. Must show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement treated wood will be placed
- 11. Indicate where pressure treated wood will be placed
- 12. Provide insulation R value for the following:
  - a. Attic space
  - b. Exterior wall cavity
  - c. Crawl space (if applicable)

a) Masonry wall

**Wall Sections including:**

- 1. Rafter size, species and spacing
  - 2. Attachment to wall and uplift
  - 3. Ridge beam sized and valley framing and support details
  - 4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
- 1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
  - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

a) Truss package including:

**Roof System:**

- a) Location of all load-bearing wall with required footings indicated as standard
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

**Foundation Plan including:**

- f) Must show and identify accessibility requirements (accessible bathroom handrails)
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and hearth
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with egress windows in bedrooms to be shown
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed
- b) Shear walls
- a) Rooms labeled and dimensioned

**Floor Plan including:**

SEE NOTE 3

**b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
- a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
- b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:

- a. Attic space
- b. Exterior wall cavity
- c. Crawl space (if applicable)
- c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

**Floor Framing System:**

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

**Electrical layout including:**

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms

**HVAC information**

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom
- Energy Calculations** (dimensions shall match plans)
- Gas System Type** (LP or Natural) Location and BTU demand of equipment

**Disclosure Statement for Owner Builders**

\*\*\*Notice Of Commencement Required Before Any Inspections Will Be Done

**Private Potable Water**

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

See Note 4

**THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS**

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. Development permit cost is \$50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

**ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE - TIME WILL NOT ALLOW THIS - PLEASE DO NOT ASK**

**GENERAL CONTRACTORS**  
**OF**

**OCCUPANCY**

**COLUMBIA COUNTY, FLORIDA**

**Department of Building and Zoning Inspection**

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 01-5S-16-03390-008 Building permit No. 000023837

Use Classification SFD, UTILITY

Fire: 29.60

Permit Holder MIKE TODD

Waste: 61.25

Owner of Building GREGORY "JASON" LITTLE

Total: 90.85

Location: 428 SW SAPPHIRE CT

Date: 05/22/2006

*[Signature]*

Building Inspector

**POST IN A CONSPICUOUS PLACE**  
*(Business Places Only)*



# Notice of Treatment

11776

**Applicator:** Florida Pest Control & Chemical Co. (www.flapest.com)

Address: Bama Ave

City: Lake City Phone: 752-1706

**Site Location:** Subdivision \_\_\_\_\_

Lot # \_\_\_\_\_ Block# \_\_\_\_\_ Permit # 23837

Address 428 SW Sapphire Ct

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
<input type="checkbox"/> Premise	Imidacloprid	0.1%
<input type="checkbox"/> Termidor	Fipronil	0.12%
<input checked="" type="checkbox"/> Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

**Type treatment:**

Soil

Wood

Dwelling

1372

429

4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line \_\_\_\_\_.

1-17-06

Date

0800

Time

F254 Gummy

Print Technician's Name

Remarks: \_\_\_\_\_

Applicator - White

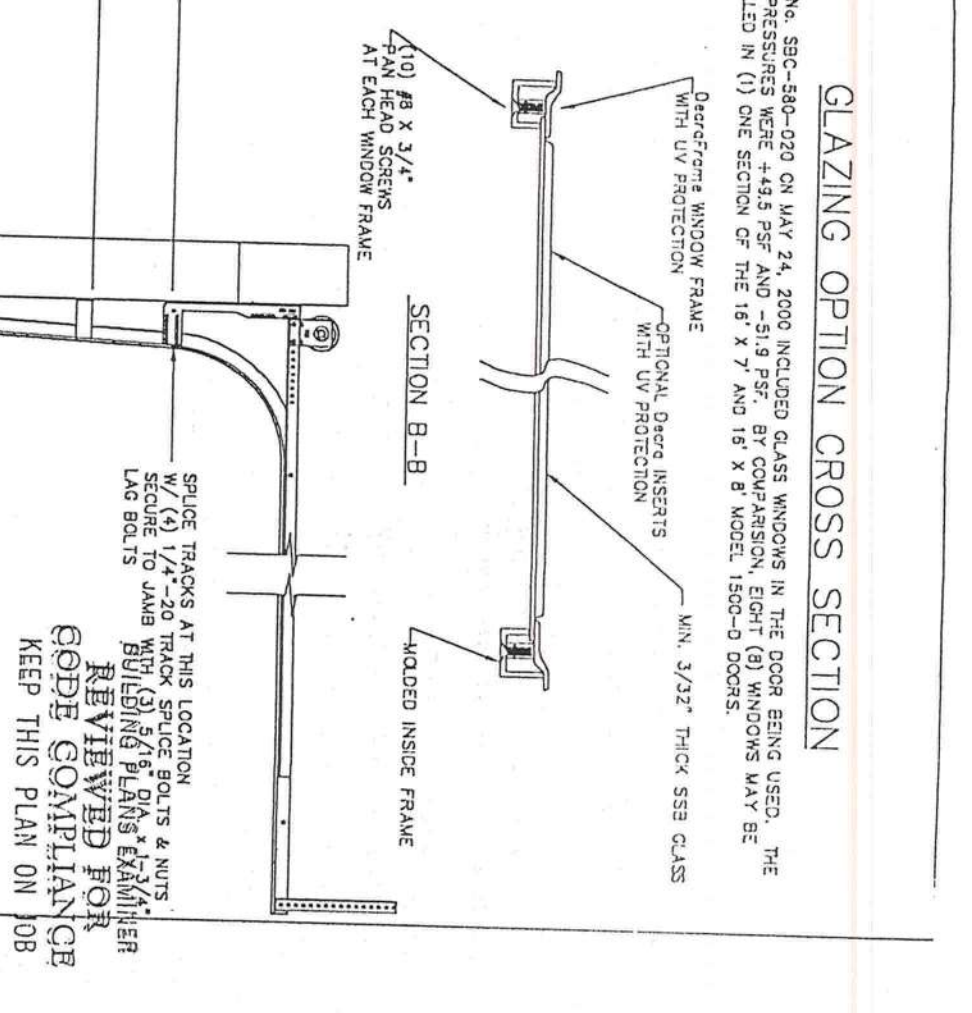
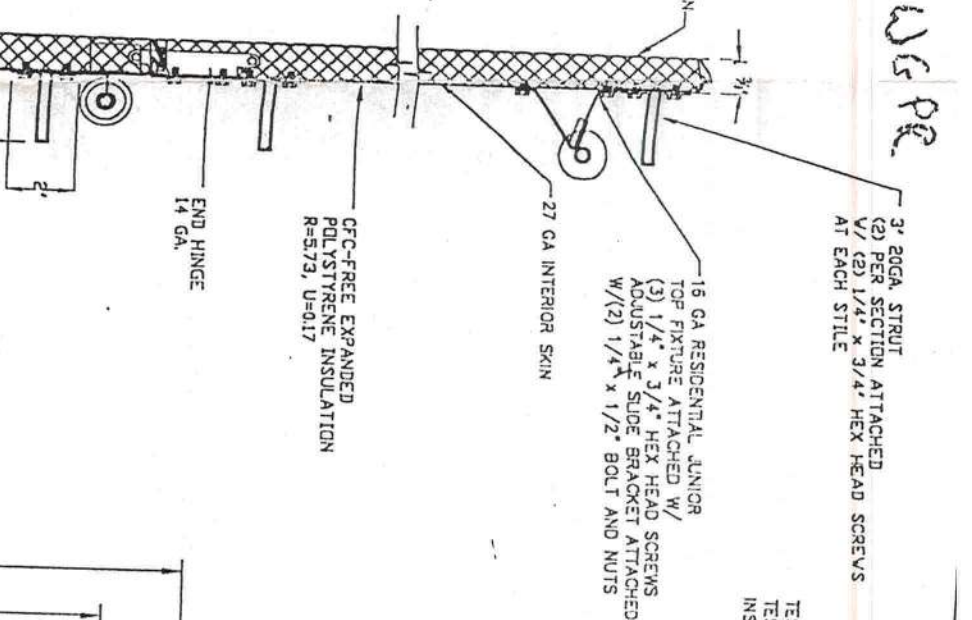
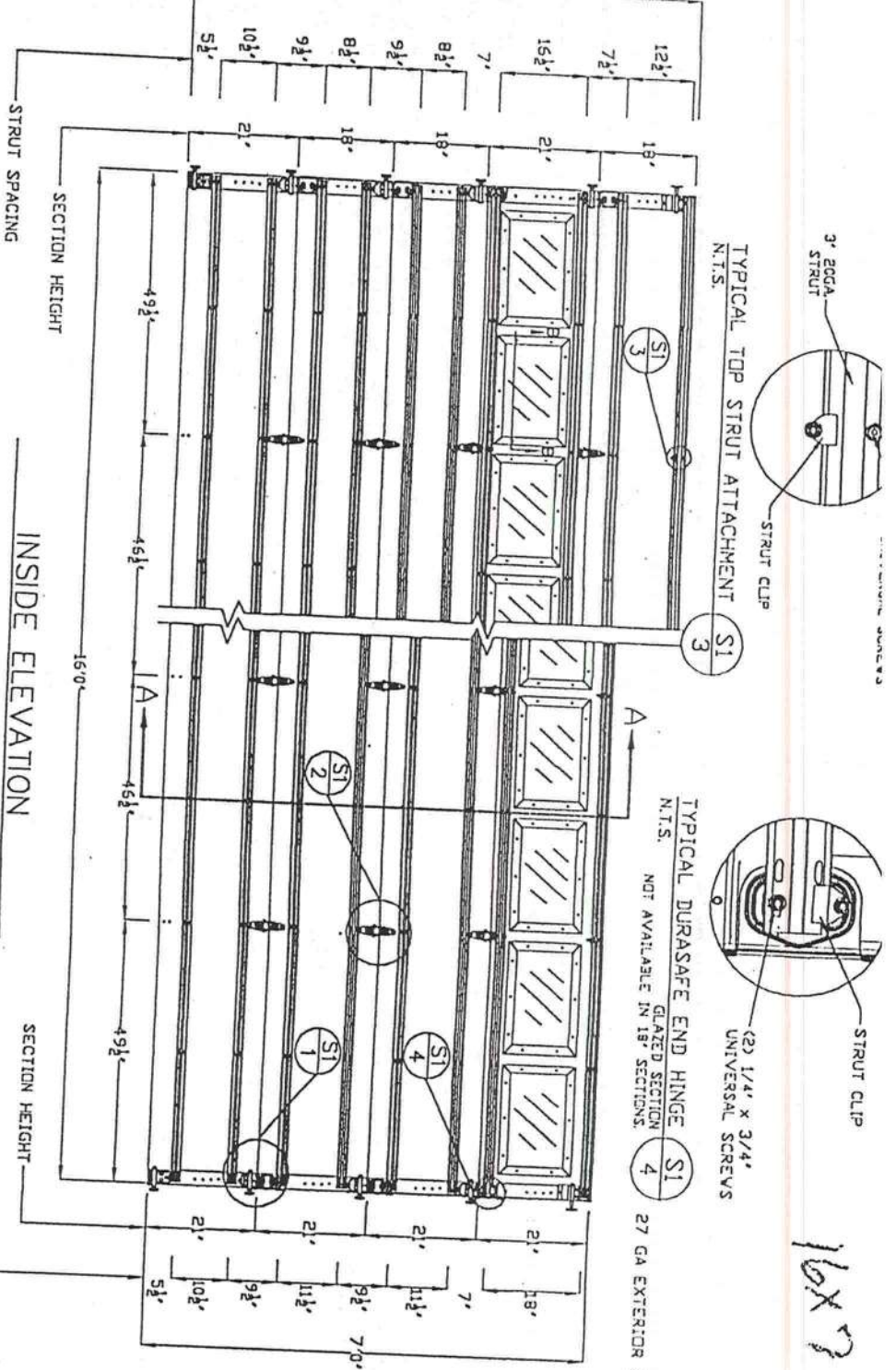
Permit File - Canary

Permit Holder - Pink

16x7 UG PR

GLAZING OPTION CROSS SECTION

TEST No. S9C-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISON, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.



TRACK CONFIGURATION FOR 6'6\"/>

JAMB BRACKET LOCATIONS		A	B	C	D	E	S
6'-6"	4'	21-1/2"	39"	57"			70"
7'-0"	4'	21-1/2"	42"	63"			76"
7'-6"	4'	18-1/2"	36"	54"			82"
8'-0"	4'	21-1/2"	39"	57"			88"

SPECIFICATIONS AND NOTES

- DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASHA.
- DOOR SECTIONS SHALL BE 27 GA. UNK. (0.016") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY C-40 GALVANIZATION.
- DOORS UP TO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
- DOORS UP TO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
- SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADS.
- THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1608 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
  - A. BASIC WIND SPEED OF 110 MPH
  - B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
  - C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
  - D. USE FACTOR OF 1.0
  - E. EXPOSURE RATING OF C

12 GA. GALV. STEEL JAMB BRACKET ATTACHED TO TRACK SPURCE BOLT & NUT TRACK SPURCE BOLT & NUT

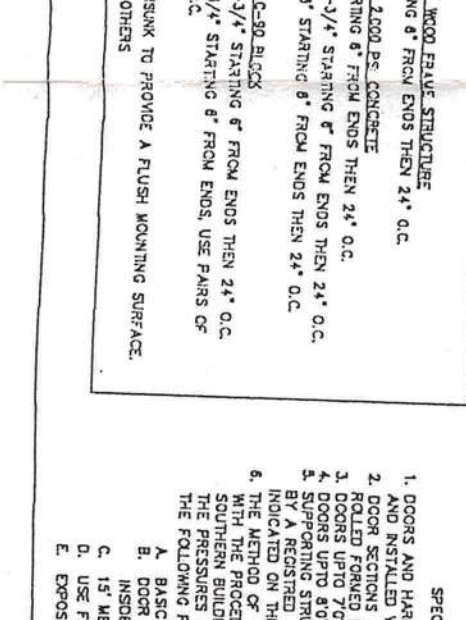
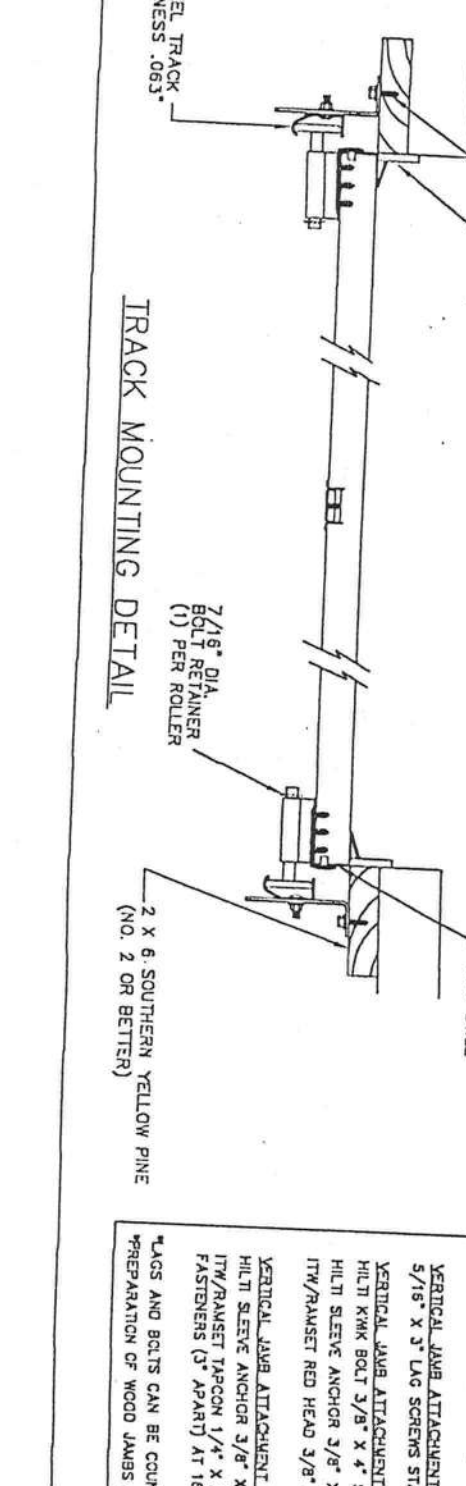
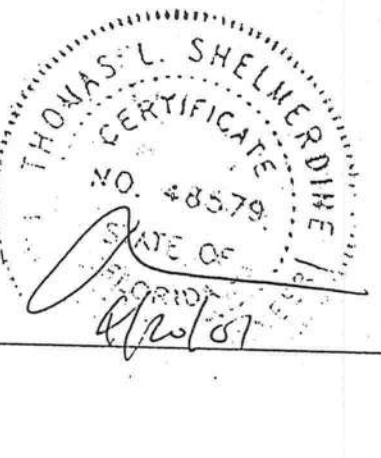
EXAMINER SIGNATURE: *Tom Shepherd*

Inspector Signature: *Tom Shepherd*

License No. *10001520*

MAY 17 2001

REVIEWED FOR CODE COMPLIANCE KEEP THIS PLAN ON JOB



DESIGNER & ENGINEER

**Amarr**

5801 GARY CENTER BLVD. VENTNOR, MO. 63158

MODEL #1500 WeatherGuard

DATE: 05/17/01

SCALE: 1/8\"/>