

Columbia County Building Permit Application

CK# 1123

For Office Use Only Application # 061101 Date Received 11/01 By JW Permit # 25256
 Application Approved by - Zoning Official BLK Date 14.11.06 Plans Examiner AKJH Date 11-28-06
 Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments NOC - ~~HEALTH~~
 NOC EH Deed or PA Site Plan State Road Info Parent Parcel # Development Permit

Fax 386-418-1092
 Name Authorized Person Signing Permit LEON JONES Phone 352.316.2715
 Address P.O. BOX 2669
 Owners Name Carlton & Marisa Bussey Phone 352 316-2715
 911 Address 578 SW Bussey Glen, Ft. White, FL 32038
 Contractors Name Leon Jones Construction, Inc. Phone 352 316-5944
 Address P. O. Box 2669, HIGH Springs, Fla. 32655

Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address _____
 Architect/Engineer Name & Address Philip MARTY ESKRIDGE (PE. PAUL STRESINS)
 Mortgage Lenders Name & Address First Horizon Home Loan Corp. 2400 Maitland Cntr Phwy Maitland, Fla. 32756

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
 Property ID Number 18-7-17 10021-010 Estimated Cost of Construction 140,000
 Subdivision Name Bussey Glen Unrecorded Lot 12 Block _____ Unit _____ Phase _____
 Driving Directions US 27 west from HIGH Springs to Bussey Glen, turn right, go 3/4 mi. job on right

Type of Construction Single family dwelling Number of Existing Dwellings on Property Non
 Total Acreage 10 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
 Actual Distance of Structure from Property Lines - Front 144 ✓ Side 88 ✓ Side 687.75 Rear 316.97 ✓
 Total Building Height 20' Number of Stories 1 Heated Floor Area 1550 Roof Pitch 8/12
 TOTAL 2722

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Carlton Bussey
Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
 COUNTY OF COLUMBIA
 Jannette S. Boyd
 MY COMMISSION # DD250332 EXPIRES August 7, 2007
 BONDED THRU TROY FAIR INSURANCE, INC.
 Sworn to (or affirmed) and subscribed before me
 this 16 day of October 20 06
 Personally known or Produced Identification _____

Angela Arthur Kight
 Notary Public, State of Florida
 My comm. exp. Mar. 1, 2008
 Contractor Signature _____
 Contractors License Number CGAA.701 0D 277521
 Competency Card Number _____
 NOTARY STAMP/SEAL

 Notary Signature (Revised Sept. 2006)



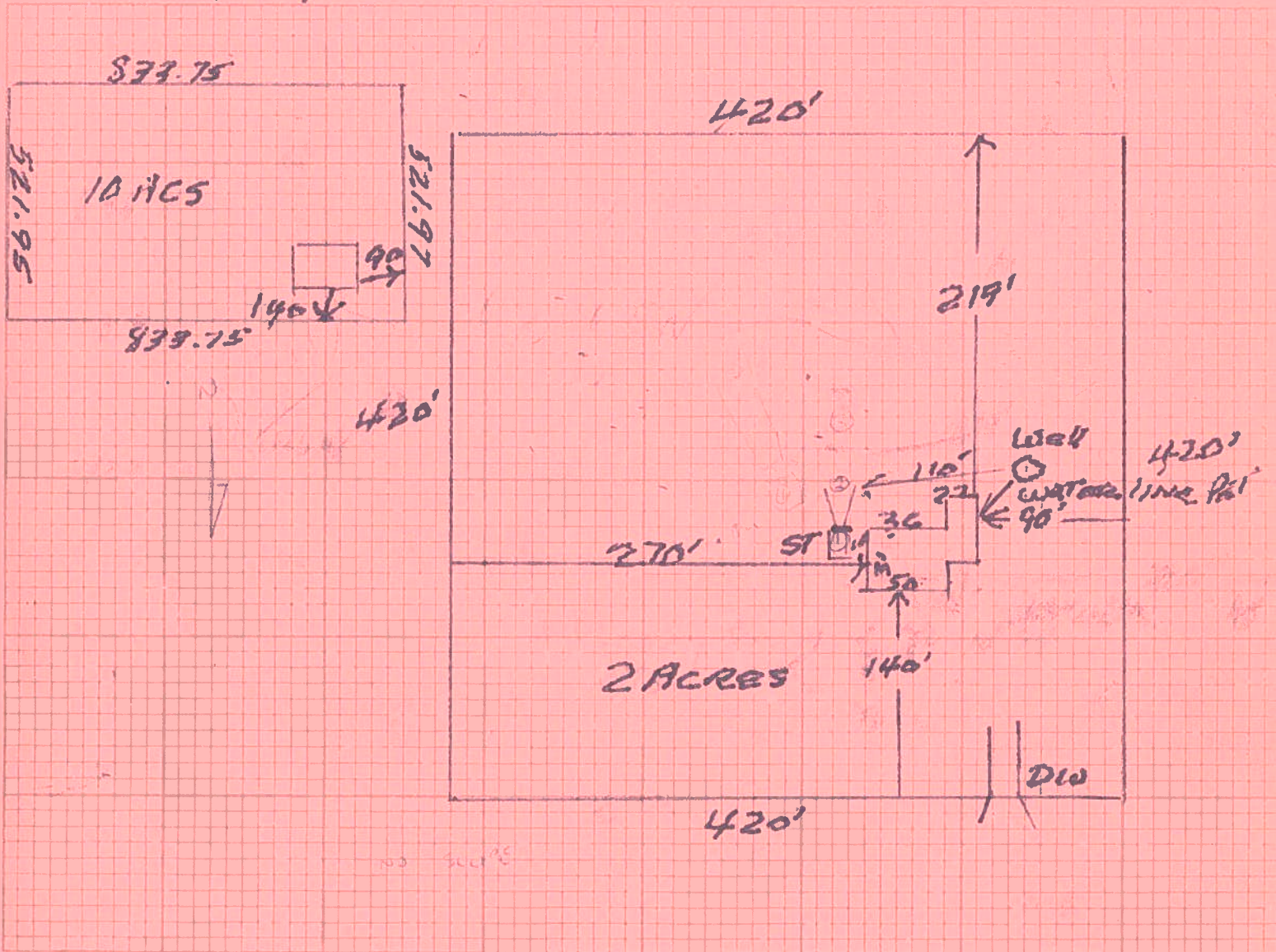
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0965N

----- PART II - SITE PLAN -----

Scale: Each block represents ^{10'} 5 feet and 1 inch = ^{100'} 50 feet.



Notes: _____

REVISED 11/21/6

Site Plan submitted by: MB Jones Signature _____

Contractor Title _____

Plan Approved Not Approved _____

Date 11/15/6

By Proper _____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 11/2/2006 DATE ISSUED: 11/6/2006

ENHANCED 9-1-1 ADDRESS:

578 SW BUSSEY GLN

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

18-7S-17-10021-010

Remarks:

LOCATED ON PRCL #12 BUSSEY GLEN UNR S/D

Address Issued By: _____


Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

479

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

@ CAM112M01 S CamaUSA Appraisal System
 11/01/2006 13:37 Legal Description Maintenance
 Year T Property Sel
 2007 R 18-7S-17-10021-010

Columbia County
 Land 000
 2184 AG 001
 Bldg 000
 Xfea 000
 2184 TOTAL B

BUSSEY CARLTON H & MARISA G

1	COMM SE COR, RUN W 2641.60 FT, N 210 FT, W 166.33 FT, N	2
3	1906.90 FT FOR POB, CONT N 521.97 FT, W 838.75 FT, S	4
5	521.95 FT, E 833.75 FT TO POB (AKA PRCL #12, BUSSEY GLEN UNR)	6
7	ORB 741-1674-1678, 744-005-007 ORB, 992-1887, CORR WD IN ORB	8
9	1007-2422 (SEE NOTE SCREEN)	10
11		12
13		14
15		16
17		18
19		20
21		22
23		24
25		26
27		28

Mnt 2/27/2004 KYLIE

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More



ALACHUA COUNTY LAND SURVEYORS, INC.
Professional Land Surveying

2512 N. E. 1st Blvd. • Suite 300 • Gainesville, FL 32609 • (904) 376-1180

And

Alachua, FL • High Springs, FL • (904) 454-1148



HAROLD L. WISE, P.L.S.

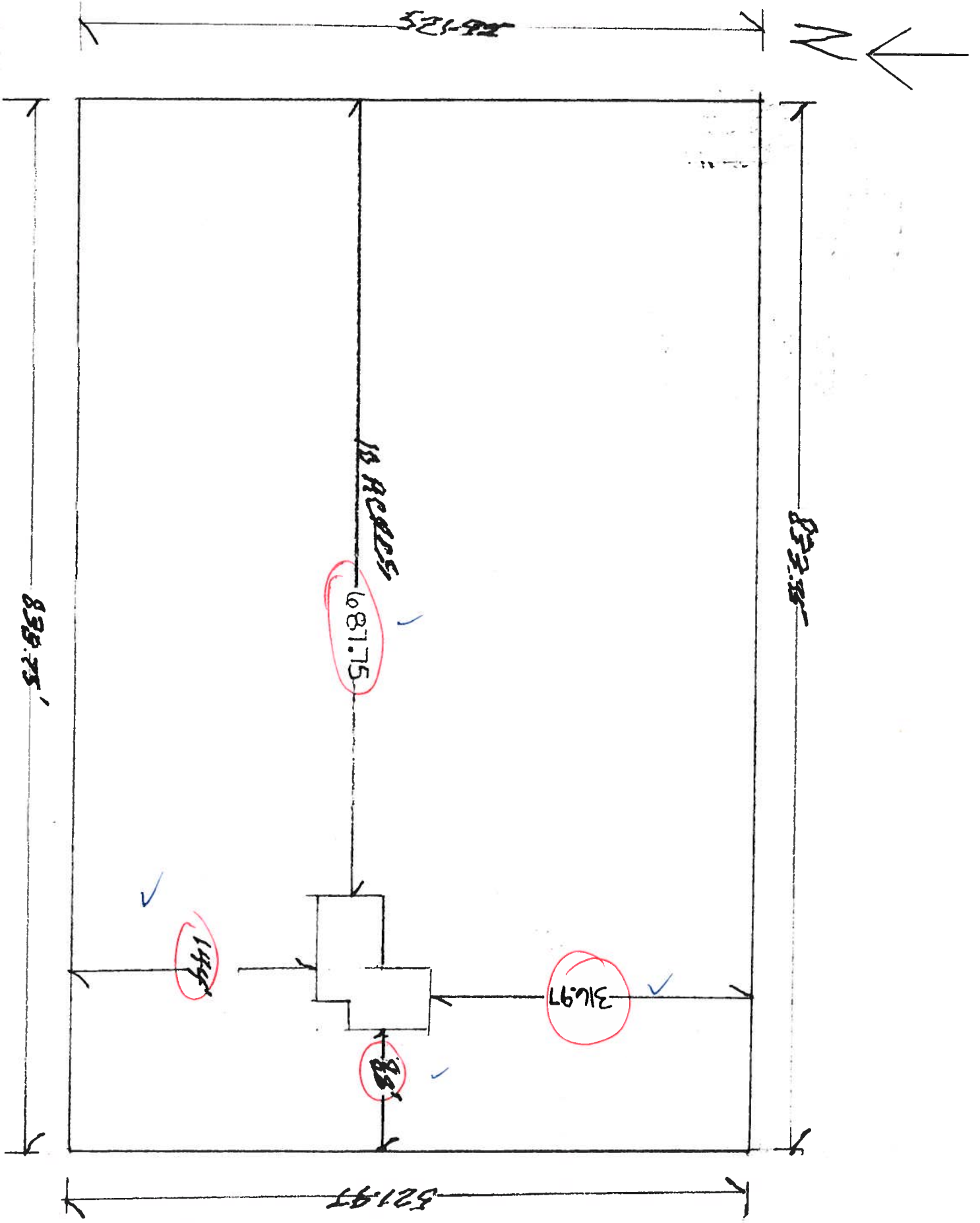
STACY A. HALL, P.L.S.

October 4, 1990
Easement for Parcels
9,11,12,13,14
Job No. 90-50

EASEMENT DESCRIPTION

A tract of land situated in Section 18, Township 7 South, Range 17 East, Columbia County, Florida, said tract of land being more particularly described as follows:

Commence at a concrete monument at the Southeast corner of the aforementioned Section 18, Township 7 South, Range 17 East for the point of reference and run S.88°00'29"W., along the South line of said Section 18, a distance of 2641.60 feet to a concrete monument at the Southeast corner of the Southeast 1/4 of the Southwest 1/4 of said Section 18; thence run N.01°30'02"W., along the East line of said Southeast 1/4 of the Southwest 1/4, a distance of 210.00 feet to a concrete monument at the Northeast corner of the South 210 feet of said Southeast 1/4 of the Southwest 1/4; thence run S.88°00'29"W., along the North line of said South 210 feet, a distance of 166.33 feet; thence run N.03°40'54"W., a distance of 879.75 feet to a steel rod & cap and the True Point of Beginning; thence continue N.03°40'54"W., a distance of 562.18 feet to a concrete monument; thence run N.01°28'59"W., a distance of 986.94 feet to a concrete monument on the North line of the Southwest 1/4 of said Section 18; thence run S.88°01'29"W., along said North line, a distance of 450.02 feet; thence run S.01°28'59"E, a distance of 40.00 feet; thence run N.88°01'29"E., a distance of 410.02 feet to a steel rod & cap; thence run S.01°28'59"E., a distance of 947.36 feet to a steel rod & cap; thence run S.03°40'54"E., a distance of 562.94 feet to a steel rod & cap; thence run N.86°19'06"E., a distance of 40.00 feet to the True Point of Beginning.



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: JONES - BUSSEY Address: City, State: , Owner: BUSSEY Climate Zone: North	Builder: LEON JONES CONSTRUCTION Permitting Office: COLUMBIA COUNTY Permit Number: 25256 Jurisdiction Number: 221000
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1. New construction or existing New <input type="checkbox"/> 2. Single family or multi-family Single family <input type="checkbox"/> 3. Number of units, if multi-family 1 <input type="checkbox"/> 4. Number of Bedrooms 3 <input type="checkbox"/> 5. Is this a worst case? No <input type="checkbox"/> 6. Conditioned floor area (ft ²) 1550 ft² <input type="checkbox"/> 7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor: Description Area (or Single or Double DEFAULT) 7a. (Dble Default) 139.0 ft ² <input type="checkbox"/> b. SHGC: (or Clear or Tint DEFAULT) 7b. (Clear) 223.0 ft ² <input type="checkbox"/> 8. Floor types a. Slab-On-Grade Edge Insulation R=0.0, 142.0(p) ft <input type="checkbox"/> b. N/A <input type="checkbox"/> c. N/A <input type="checkbox"/> 9. Wall types a. Frame, Wood, Exterior R=13.2, 1105.0 ft² <input type="checkbox"/> b. Frame, Wood, Adjacent R=12.9, 160.0 ft² <input type="checkbox"/> c. N/A <input type="checkbox"/> d. N/A <input type="checkbox"/> e. N/A <input type="checkbox"/> 10. Ceiling types a. Under Attic R=30.0, 1550.0 ft² <input type="checkbox"/> b. N/A <input type="checkbox"/> c. N/A <input type="checkbox"/> 11. Ducts a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 48.0 ft <input type="checkbox"/> b. N/A <input type="checkbox"/>	12. Cooling systems a. Central Unit Cap: 36.0 kBtu/hr <input type="checkbox"/> SEER: 13.00 <input type="checkbox"/> b. N/A <input type="checkbox"/> c. N/A <input type="checkbox"/> 13. Heating systems a. PTHP Cap: 36.0 kBtu/hr <input type="checkbox"/> COP: 3.50 <input type="checkbox"/> b. N/A <input type="checkbox"/> c. N/A <input type="checkbox"/> 14. Hot water systems a. Electric Resistance Cap: 40.0 gallons <input type="checkbox"/> EF: 0.93 <input type="checkbox"/> b. N/A <input type="checkbox"/> c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) <input type="checkbox"/> 15. HVAC credits CF, <input type="checkbox"/> (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
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Glass/Floor Area: 0.14	Total as-built points: 20053	PASS
	Total base points: 24552	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Larry Rasmundo a/c
DATE: October 12, 2006

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
 EnergyGauge® (Version: FLRCSB v4.0)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
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BASE	AS-BUILT
GLASS TYPES	
.18 X Conditioned X BSPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X SPM X SOF = Points
.18 1550.0 20.04 5591.2	Double, Clear N 8.0 6.0 90.0 19.20 0.67 1155.8
	Single, Clear S 8.0 8.0 63.0 40.81 0.52 1338.4
	Double, Clear E 1.5 6.0 40.0 42.06 0.91 1535.7
	Single, Clear W 1.5 8.0 21.0 43.84 0.96 880.4
	Double, Clear W 1.5 4.0 9.0 38.52 0.81 282.2
	As-Built Total: 223.0 5192.5
WALL TYPES Area X BSPM = Points	Type R-Value Area X SPM = Points
Adjacent 160.0 0.70 112.0	Frame, Wood, Exterior 13.2 1105.0 1.48 1635.4
Exterior 1105.0 1.70 1878.5	Frame, Wood, Adjacent 12.9 160.0 0.61 96.8
Base Total: 1265.0 1990.5	As-Built Total: 1265.0 1732.2
DOOR TYPES Area X BSPM = Points	Type Area X SPM = Points
Adjacent 21.0 2.40 50.4	Exterior Wood 21.0 6.10 128.1
Exterior 21.0 6.10 128.1	Adjacent Wood 21.0 2.40 50.4
Base Total: 42.0 178.5	As-Built Total: 42.0 178.5
CEILING TYPES Area X BSPM = Points	Type R-Value Area X SPM X SCM = Points
Under Attic 1550.0 1.73 2681.5	Under Attic 30.0 1550.0 1.73 X 1.00 2681.5
Base Total: 1550.0 2681.5	As-Built Total: 1550.0 2681.5
FLOOR TYPES Area X BSPM = Points	Type R-Value Area X SPM = Points
Slab 142.0(p) -37.0 -5254.0	Slab-On-Grade Edge Insulation 0.0 142.0(p) -41.20 -5850.4
Raised 0.0 0.00 0.0	
Base Total: -5254.0	As-Built Total: 142.0 -5850.4
INFILTRATION Area X BSPM = Points	Area X SPM = Points
1550.0 10.21 15825.5	1550.0 10.21 15825.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
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BASE	AS-BUILT
Summer Base Points: 21013.2	Summer As-Built Points: 19759.8
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
21013.2 0.4266 8964.2	<small>(sys 1: Central Unit 36000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)</small> 19760 1.00 (1.09 x 1.147 x 1.00) 0.263 0.950 6161.5 19759.8 1.00 1.250 0.263 0.950 6161.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
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BASE	AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF = Points							
.18 1550.0 12.74 3554.5	Double, Clear	N	8.0	6.0	90.0	24.58	1.02	2259.6
	Single, Clear	S	8.0	8.0	63.0	20.24	2.73	3483.0
	Double, Clear	E	1.5	6.0	40.0	18.79	1.04	778.4
	Single, Clear	W	1.5	8.0	21.0	28.84	1.01	612.7
	Double, Clear	W	1.5	4.0	9.0	20.73	1.05	196.7
	As-Built Total:						223.0	7330.4
WALL TYPES Area X BWPM = Points	Type			R-Value	Area X	WPM	=	Points
Adjacent 160.0 3.60 576.0	Frame, Wood, Exterior			13.2	1105.0	3.36		3712.8
Exterior 1105.0 3.70 4088.5	Frame, Wood, Adjacent			12.9	160.0	3.32		530.4
Base Total: 1265.0 4664.5	As-Built Total:				1265.0			4243.2
DOOR TYPES Area X BWPM = Points	Type				Area X	WPM	=	Points
Adjacent 21.0 11.50 241.5	Exterior Wood				21.0	12.30		258.3
Exterior 21.0 12.30 258.3	Adjacent Wood				21.0	11.50		241.5
Base Total: 42.0 499.8	As-Built Total:				42.0			499.8
CEILING TYPES Area X BWPM = Points	Type			R-Value	Area X	WPM X WCM	=	Points
Under Attic 1550.0 2.05 3177.5	Under Attic			30.0	1550.0	2.05 X 1.00		3177.5
Base Total: 1550.0 3177.5	As-Built Total:				1550.0			3177.5
FLOOR TYPES Area X BWPM = Points	Type			R-Value	Area X	WPM	=	Points
Slab 142.0(p) 8.9 1263.8	Slab-On-Grade Edge Insulation			0.0	142.0(p)	18.80		2669.6
Raised 0.0 0.00 0.0								
Base Total: 1263.8	As-Built Total:				142.0			2669.6
INFILTRATION Area X BWPM = Points					Area X	WPM	=	Points
1550.0 -0.59 -914.5					1550.0	-0.59		-914.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
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BASE			AS-BUILT					
Winter Base Points: 12245.6			Winter As-Built Points: 17006.0					
Total Winter X Points	System = Multiplier	Heating Points	Total X Component (System - Points)	Cap X Ratio	Duct X Multiplier (DM x DSM x AHU)	System X Multiplier	Credit = Multiplier	Heating Points
12245.6	0.6274	7682.9	(sys 1: PTHP 36000 btuh ,EFF(3.5) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 17006.0 1.000 (1.069 x 1.169 x 1.00) 0.286 1.000 6071.9					
12245.6	0.6274	7682.9	17006.0	1.00	1.250	0.286	1.000	6071.9

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.4

The higher the score, the more efficient the home.

BUSSEY, . . .

<p>1. New construction or existing New <input type="checkbox"/></p> <p>2. Single family or multi-family Single family <input type="checkbox"/></p> <p>3. Number of units, if multi-family 1 <input type="checkbox"/></p> <p>4. Number of Bedrooms 3 <input type="checkbox"/></p> <p>5. Is this a worst case? No <input type="checkbox"/></p> <p>6. Conditioned floor area (ft²) 1550 ft² <input type="checkbox"/></p> <p>7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)</p> <p style="margin-left: 20px;">a. U-factor: Description Area</p> <p style="margin-left: 20px;">(or Single or Double DEFAULT) 7a. (Dble Default) 139.0 ft² <input type="checkbox"/></p> <p style="margin-left: 20px;">b. SHGC:</p> <p style="margin-left: 20px;">(or Clear or Tint DEFAULT) 7b. (Clear) 223.0 ft² <input type="checkbox"/></p> <p>8. Floor types</p> <p style="margin-left: 20px;">a. Slab-On-Grade Edge Insulation R=0.0, 142.0(p) ft <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>9. Wall types</p> <p style="margin-left: 20px;">a. Frame, Wood, Exterior R=13.2, 1105.0 ft² <input type="checkbox"/></p> <p style="margin-left: 20px;">b. Frame, Wood, Adjacent R=12.9, 160.0 ft² <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">d. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">e. N/A <input type="checkbox"/></p> <p>10. Ceiling types</p> <p style="margin-left: 20px;">a. Under Attic R=30.0, 1550.0 ft² <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>11. Ducts</p> <p style="margin-left: 20px;">a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 48.0 ft <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p>	<p>12. Cooling systems</p> <p style="margin-left: 20px;">a. Central Unit Cap: 36.0 kBtu/hr <input type="checkbox"/></p> <p style="margin-left: 20px;">SEER: 13.00 <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>13. Heating systems</p> <p style="margin-left: 20px;">a. PTHP Cap: 36.0 kBtu/hr <input type="checkbox"/></p> <p style="margin-left: 20px;">COP: 3.50 <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. N/A <input type="checkbox"/></p> <p>14. Hot water systems</p> <p style="margin-left: 20px;">a. Electric Resistance Cap: 40.0 gallons <input type="checkbox"/></p> <p style="margin-left: 20px;">EF: 0.93 <input type="checkbox"/></p> <p style="margin-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="margin-left: 20px;">c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) <input type="checkbox"/></p> <p>15. HVAC credits CF, <input type="checkbox"/></p> <p style="margin-left: 20px;">(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)</p>
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I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

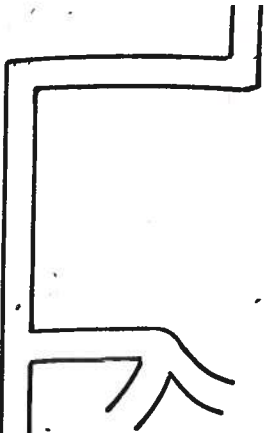
Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.0)



7

0611-01

OAK

RIDGE

ROA

778

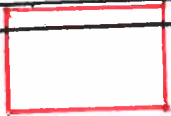
ZONE A

ZONE A

ZONE A

ZONI

18



19

CSX

R16E
R17E

25256

This instrument was prepared by:
Beverly K. Smith, employee of
Vision Title of Alachua County, LLC
4881-1 NW 8th Ave., Gainesville, FL 32605

Please Return to:

Return to:
Vision Title of Alachua County, LLC
4881 NW 8th Avenue, Suite 1
Gainesville, FL 32605

Permit No. _____

NOTICE OF COMMENCEMENT

Loan No. 0059047191

Inst:200604289 Date:10/12/2006 Time:11:23
DC, P. Dewitt Cason, Columbia County B:1098 P:2080

State of Florida
County of Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: See attached Schedule A

(legal description of the property, and street address if available)

BUSSEY GLN
FORT WHITE Florida 32038

2. General description of improvement: Construction of Single Family Residence

3. Owner information:

a. Name and address: CARLTON BUSSEY

BUSSEY GLN
FORT WHITE Florida 32038

b. Interest in property: fee simple

c. Name and address of fee simple titleholder (if other than Owner):

4. a. Contractor's name and address:

LEON JONES CONSTRUCTION
7717 NW 19TH STREET
ALACHUA, FL 32615

b. Contractor's phone number: 386/418-4690

c. Fax number (optional, if service by fax is acceptable):

5. Surety

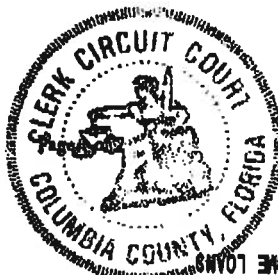
a. Name and address: N/A

b. Phone number:

c. Amount of bond: \$

d. Fax number (optional, if service by fax is acceptable):

Notice of Commencement 0605



STATE OF FLORIDA COUNTY OF COLUMBIA
I HEREBY CERTIFY that the above and foregoing
is a true copy of the original filed in this office
P. DEWITT CASON CLERK OF COURTS

By: [Signature]

Deputy Clerk PH/022K

Date 10-12-06

3pgs

0059047191

- 6. a. Lender's name and address: First Horizon Home Loans
200 E Robinson Ste 400
Orlando, FL 32801
- b. Lender's phone number: (866) 684-8672
- c. Fax number (optional, if service by fax is acceptable): _____

- 7. a. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: _____

(name and address)
- b. Phone numbers of designated persons: _____
- c. Fax number (optional, if service by fax is acceptable): _____

- 8. a. In addition to himself or herself, Owner designates First Horizon Home Loans (address above)
& Vision Title of Alachua County, * to receive a copy of the Lienor's Notice as provided
in Section 713.13(1)(b), Florida Statutes. 4881-1 NW 8th Ave., Gainesville, FL 32605
- b. Phone number of person or entity designated by owner: _____
- c. Fax number (optional, if service by fax is acceptable): _____

- 9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

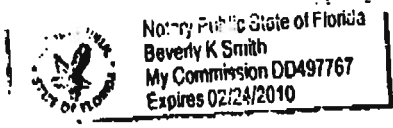
Carlton Bussey
(Signature of Owner) **CARLTON BUSSEY**

Sworn to (or affirmed) and subscribed before me this 10 day of Oct, 2006 by
Carlton Bussey
(name of person making statement)

Beverly K. Smith
(Signature of Notary Public - State of Florida)
Beverly K. Smith

(Print, Type, or Stamp Commissioned Name of Notary Public)

Personally Known _____ OR Produced Identification
Type of Identification Produced drivers license



Inst:2006024289 Date:10/12/2006 Time:11:23

DC,P.Dewitt Cason,Columbia County B:1098 P:2081

Schedule "A"

File Number: GV06-470

A tract of land situated in Section 18, Township 7 South, Range 17 East, Columbia county, Florida, said tract of land being more particularly described as follows:

Commence at a concrete monument at the Southeast corner of the aforementioned Section 18, Township 7 South, Range 17 East, for the Point of Reference and run South 88° 00' 29" West, along the South line of said Section 18, a distance 2641.60 feet to a concrete monument at the Southeast corner of the Southeast ¼ of the Southwest ¼ of said Section 18; thence run North 01° 30' 02" West, along the East line of said Southeast ¼ of the Southwest ¼, a distance of 210.00 feet to a concrete monument at the Northeast corner of the South 210 feet of said Southeast ¼ of the Southwest ¼; thence run South 88° 00' 29" West, along the North line of said South 210 feet, a distance of 166.33 feet; thence run North 03° 40' 54" West, a distance of 1441.93 feet to a concrete monument; thence run North 01° 28' 59" West, a distance of 464.97 feet to a steel rod and cap on the true Point of Beginning; thence continue North 01° 28' 59" West, a distance of 521.97 feet to a concrete monument on the North line of the Southwest ¼ of Section 18; thence run South 88° 01' 29" West, along said North line, a distance of 838.75 feet to a steel rod and cap; thence run South 02° 01' 55" East, a distance of 521.95 feet to a steel rod and cap; thence run North 88° 01' 29" East, a distance of 833.75 feet to the true Point of Beginning.

Subject to the maintained right of way for the County Road (Bussey Road) along the northerly boundary thereof.

Subject to and together with an easement for ingress, egress and public utilities over, under and across a 40 foot wide strip of land being more particularly described as follows:

A Tract of land situated in Section 18, Township 7 South, Range 17 East, Columbia County, Florida, said tract of land being more particularly described as follows

Commence at a concrete monument at the Southeast corner of the aforementioned Section 18, Township 7 South, Range 17 East for the Point of Reference and run South 88° 00' 29" West, along the South line of said Section 18, a distance of 2641.60 feet to a concrete monument at the Southeast corner of the Southeast ¼ of the Southwest ¼ of said Section 18; thence run North 01° 30' 02" West, along the East line of said Southeast ¼ of the Southwest ¼, a distance of 210.00 feet to a concrete monument at the Northeast corner of the South 210.00 feet of said Southeast ¼ of the Southwest ¼; thence run South 88° 00' 29" West, along the North line of said South 210.00 feet, a distance of 166.33 feet; thence run North 03° 40' 54" West, a distance of 879.75 feet to a steel rod and cap and the true Point of Beginning; thence continue North 03° 40' 54" West, a distance of 562.18 feet to a concrete monument; thence run North 01° 28' 59" West, a distance of 986.94 feet to a concrete monument on the North line of the Southwest ¼ of Section 18; thence run South 88° 01' 29" West, along said North line, a distance of 450.02 feet; thence run South 01° 28' 59" East, a distance of 40.00 feet; thence run North 88° 01' 29" East, a distance of 410.02 feet to a steel rod and cap; thence run South 01° 28' 59" East, a distance of 947.36 feet to a steel rod and cap; thence run South 03° 40' 54" East, a distance of 562.94 feet to a steel rod and cap; thence run North 86° 19' 06" East a distance of 40.00 feet to the true Point of Beginning.

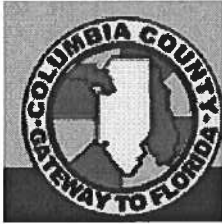
Inst: 2006024289 Date: 10/12/2006 Time: 11:23

File Number: GV06-470

_____ DC, P. Dewitt Cason, Columbia County B: 1098 P: 2082

Legal Description with Non Homestead
Closer's Choice

C.H.B.
M.G.S.



From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0611-01**
Applicant Leon Jones, contractor, Leon Jones Owners, Carlton & Marisa Bussey
Property ID 18-7s-17-10021-010

On the date of November 2, 2006 application 0611-01 and plans for construction of single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0611-01 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.

- 2.** Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.

- 3.** The window opening sizes are not legible on the plans; please show compliance with Section R310.1.1 of the Florida Residential Building Code. All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m²).
Exception: Grade floor openings shall have a minimum net clear opening of 5 square feet (0.465 m²): R310.1.2 Minimum opening height. The minimum net clear opening height shall be 24 inches (610 mm): R310.1.3 Minimum opening width. The minimum net clear opening width shall be 20 inches (508 mm). One emergency escape and rescue openings is required in each bedroom.

Code compliance for the garage area:

- 4.** An attics access pull down stair is shown installed in the garage ceiling. The stair device shall comply with sections R309.1 and R309.2 of the Florida Residential Building Code which requires that the garage shall be separated from the residence. Openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. If attic access is from the garage provide the product information from the pull down stairs manufacture which verifies compliance with these codes requirement.

- 5.** The plans show the placement of an air handling unit in the garage within a mechanical closet. Section R309.1.1 of the Florida Residential Building Code requires that ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. An approved method would be to provide 1 3/8 inches (35 mm) inch thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors for the opening which provides access to the mechanical closet from the garage. Please indicate on the plans which compliance method will be used.

- 6.** The entry door from the garage into the residence is required by sections R309.1 of the Florida Residential Building Code to be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 and 3/8 inches (35 mm) thick, or 20-minute fire-rated doors. Please verify on the plans compliance of this code requirement.

- 7.** The garage has a sixteen foot opening for an overhead door. Please have Mr. Eskridge engineer the required header for this opening which will support of the truss system and show a detail of attachment of the header to the shear walls and foundation.

- 8.** An attic storage area is shown on the plans and is labeled on the plans as a bonus room. Due to not having a means of egress which compiles with the residential code this room can only be show on the plans as an attic storage area, not as habitable space. Show on the plans the sub flooring material type and thickness for the attic storage area.

- 9.** Two dormers are shown on the elevation plan. Please have Mr. Eskridge engineer the required attachment methods of the dormers to the trusses. Also provide the size, types of material which will be used to construct these dormers.

Electrical plans code requirements:

- 10.** The electrical panel is not show plans please verify the location and amperage rating of this electrical panel. Show compliance with the following requirements of the National Electrical Code. A means of disconnect that provides overcurrent protection shall be installed on the exterior of structures to serve as a disconnecting means from the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

11. Please show compliance with the R313 of the Florida Residential Building Code, Smoke alarms: Smoke alarms shall be installed in the following locations:

1. In each sleeping room.
2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
3. On each additional story of the dwelling, including basements but not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with all intervening doors closed.

All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

12. Show on the electrical plans as required by the National Electrical Code article 210.12 that all branch circuits supplying outlets installed within a bedroom shall be protected by Arc-Fault interrupter devices.

13. Show all the electrical receptacles as required by the National Electrical Code (bathrooms, kitchens counters, garage and outdoor) to be Ground fault circuits receptacles.

14. Please show compliance with exception of section R303.3 Bathrooms: Water closet compartments and other similar rooms shall be provided with aggregate glazing area in windows of not less than 3 square feet (0.279 m²), one-half of which must be openable.

Exception: The glazed areas shall not be required where artificial light and a mechanical ventilation system are provided. The minimum ventilation rates shall be 50 cfm (23.6 L/s) for intermittent ventilation or 20 cfm (9.4 L/s) for continuous ventilation. Ventilation air from the space shall be exhausted directly to the outside.

Thank you:



Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

MARTY R. ESKRIDGE

14952 MAIN STREET ALACHUA FL 32615 PH: 386-462-1340

November 6, 2006

Job: M. L. Jones Construction

Re: Dormer Strapping

Dear Sir:

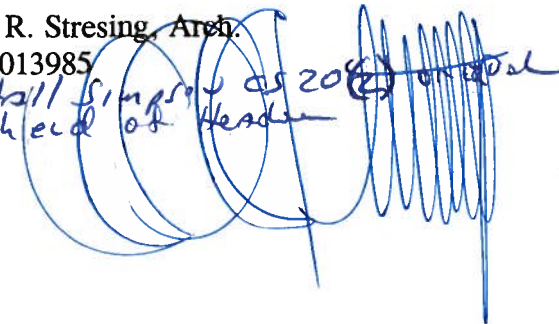
The windload analysis has been reviewed as to the dormer strapping. Install Simpson CS16 or equal @ 32" o.c. attaching the dormer walls to the top chord of the truss below. Install Simpson H-10 or equal on each truss attaching the trusses to the dormer wall. See the original windload analysis for the nail size and pattern.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Paul R. Stresing, Arch.

AR0013985

Install Simpson CS 208 on each end of Header



MARTY R. ESKRIDGE

14952 MAIN STREET ALACHUA FL 32615 PH: 386-462-1340

November 6, 2006

Job: M. L. Jones Construction

Re: Dormer Strapping

Dear Sir:

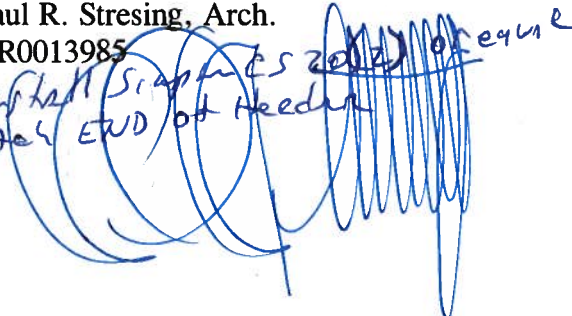
The windload analysis has been reviewed as to the dormer strapping. Install Simpson CS16 or equal @ 32" o.c. attaching the dormer walls to the top chord of the truss below. Install Simpson H-10 or equal on each truss attaching the trusses to the dormer wall. See the original windload analysis for the nail size and pattern.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Paul R. Stresing, Arch.

AR0013985

*Install Strapping (CS 2012) of equal
ON EACH END of Header*



2003.1 Allowable Stress Design

NOTE: THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT.

- PROMOTE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
- DO NOT CUT, NOTCH OR DRILL GANG-LAM.
- SHIM ALL BEARINGS FOR FULL CONTACT.
- VERIFY DIMENSIONS BEFORE CUTTING.
- THE GANG-LAM IS TO BE USED AS A ROOF BEAM ONLY.
- MAKE PROVISION FOR ADEQUATE DRAINAGE.
- PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF GANG-LAM, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PILES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STRAGGER ROWS NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1) OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM	TO	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	220 PLF	00-00-00	17-00-00	1.25	
UNIFORM	ROOF	DEAD	TOP	220 PLF	00-00-00	17-00-00	0.90	
UNIFORM	BEAM	WEIGHT	TOP	14 PLF	00-00-00	17-00-00	0.90	

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR LP-ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN GANG-LAM LVL OR LPI JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE GANG-LAM LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR GANG-LAM LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

2 BEAMS 178" X 14,000 GI 28650F3-1.9E
DESIGN CONSISTS OF 2 - PILES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA :

LIVE LOAD	20	PSF
DEAD LOAD	20	PSF
TOTAL LOAD	40	PSF
ROOF LEFT SPAN CARR.	20.00	FT
ROOF RIGHT SPAN CARR.	2.00	FT
DEFLECTION CRITERIA :		
LIVE LOAD DEFL.	1 / 240	
TOTAL LOAD DEFL.	1 / 180	

CODE COMPLIANCES :

ICBO	REPORT #
ER-5004	
L.A. City	RR-25167
CMC	11518-R
MISCOINSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	HR 1214D

SUPPORT REACTIONS (LBS) :

CASE **B E A R I N G N U M B E R**

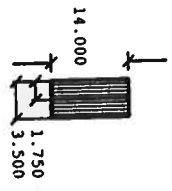
1	3859	3859
2	1989	1989

MIN BEARING SIZES (IN-SX)

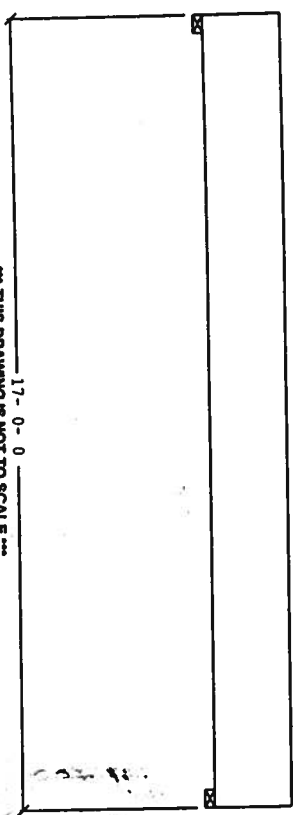
6-0 6-0

MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE

LIVE LOAD	0.24"	0.50"
DEAD LOAD	0.39"	0.50"
TOTAL LOAD	0.50"	1.10"



CROSS SECTION



Handing & Erection

Temporary and permanent bracing for holding component straight and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designer of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is approved and sealed, the structural design is approved as shown. Gang-Lam LVL and CTR, LPI based on data provided by the customer. Gang-Lam LVL and CTR, LPI joints are made without camber and will develop load by code. Continuous contact with concrete must be provided as required by code. Continuous load support is assumed (year, load and have an Architect's or Engineer's seal on-site inspection. This drawing is for informational purposes only and should be considered an Engineering document.

Gang-Lam LVL and CTR, LPI Joist Specifications

Supports and connections for Gang-Lam LVL and CTR, LPI Joists to be specified by the designer. Specific details shall be provided in plan lines shall be spaced a minimum of 4" for 10d and 3" for 8d.

Do not cut, notch, drill or alter Gang-Lam LVL and CTR, LPI Joists except as shown in published material from LP. Any use of Gang-Lam LVL and CTR, LPI Joists contrary to the limits set forth hereon, negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR.

LP is a trademark of Louisiana-Pacific Corporation.

Software Provided By:
LP Engineered Wood Products

2706 Highway 421 North
Wilmington, NC 28401
Local 910 782 8878
National 800 999 8105

DWG # _____
SHEET # _____

06/21/08 SBC11

Mydoitbest.com Home

Home | My Admin | Contacts | Site Map | Logout

Site Search:

Advanced Catalog Search Catalog Search: All Full Text

Current Member:
5831 -
ALACHUA FARM & LUMBER
INC

Item Information

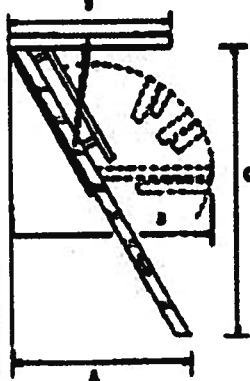
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**22X105 FIRE ATTIC STAIR - 180882
BET89F**

Express Catalog

- Advanced Search
- Regular Shipments
- Shopping Cart
- Pending Orders
- Bulk Order
- Order History
- Preferences
- Help
- Switch View

Item Details



Vendor: CENTURY STAIRS (2904)

Drop Ship: 0.000

Status: Norm

UPS: Y

UPC: 00000628428996

Unit Cube: 8.7049

Page: 16-1C08

Unit Weight: 56.0000

Country: UNITED STATES OF AMERICA

Top 20 %:

Return List and Destroy
Goods:

Other: INCOM Item

INCOM B
Code:

Product Type: 4440 Attic, Stairways, Stringers, Folding, Wooden Stairs

Products

Choose One...

Programs & Services

Choose One...

Online Tools

- Catalog
- Claims
- Commodities Catalog
- Customer Rewards
- Dataset
- eFLASH
- Image Search
- InfoPlus
- Invoices
- Lumber Catalog
- MSDS
- October Market

Warehouse Inventory as of 11/6/2006 11:47:34 AM

CAPE GIRARDEAU, 4
MO :

DIXON, IL : 10

MEDINA, OH : 2

WACO, TX : 4

LEXINGTON, SC : 29

WOODBURN, OR : 13

MONTGOMERY, 14
NY :

Description

Folding Attic Stairs.

Stairways come completely assembled and ready-to-install with instructions. Full width piano hinge across the frame at the head of the stairs. 3/16" stayrods under treads with large washers and painted or plated hardware to prevent rust. The springs expand uniformly under load and are hand tool adjustable for safety and reliability. Windsor. All treads, stiles and well frames are constructed of 1 x 4 kiln dried southern pine. Press info button below for rough opening sizes.

Windsor/Fireguard. All treads, stiles and well frames are constructed of 1 x 4 kiln dried southern pine, includes fire retardant door. Press info button below for rough opening sizes.

Premium. All treads, stiles, and well frames are constructed of 1 x 6 kiln dried southern pine, plus heavy-duty hardware and springs. Designed for heavier home usage and loads. Press info button below for rough opening sizes.

Spacemaker. All treads, stiles and well frames are constructed of 1 x 5 kiln dried southern pine. Press info button below for rough opening sizes.

Supreme Aluminum. All aluminum stair construction with heavy-duty steps and stiles. Double riveted treads provide added stability and security. Adjustable stile and pivotal floor pad make installation easy. Stairway has full width piano hinge and trifold design with heavy-duty side hinges. Reduced maintenance, termite and rust-resistant with sanded plywood door. Press info button below for rough opening sizes.

NOTE: Windsor 250 lb. load limit. Premium and Supreme 300 lb. load limit. Also available with insulated panel. Contact the Do it Best Corp. Millwork Department for details. Press info button below for rough opening sizes.

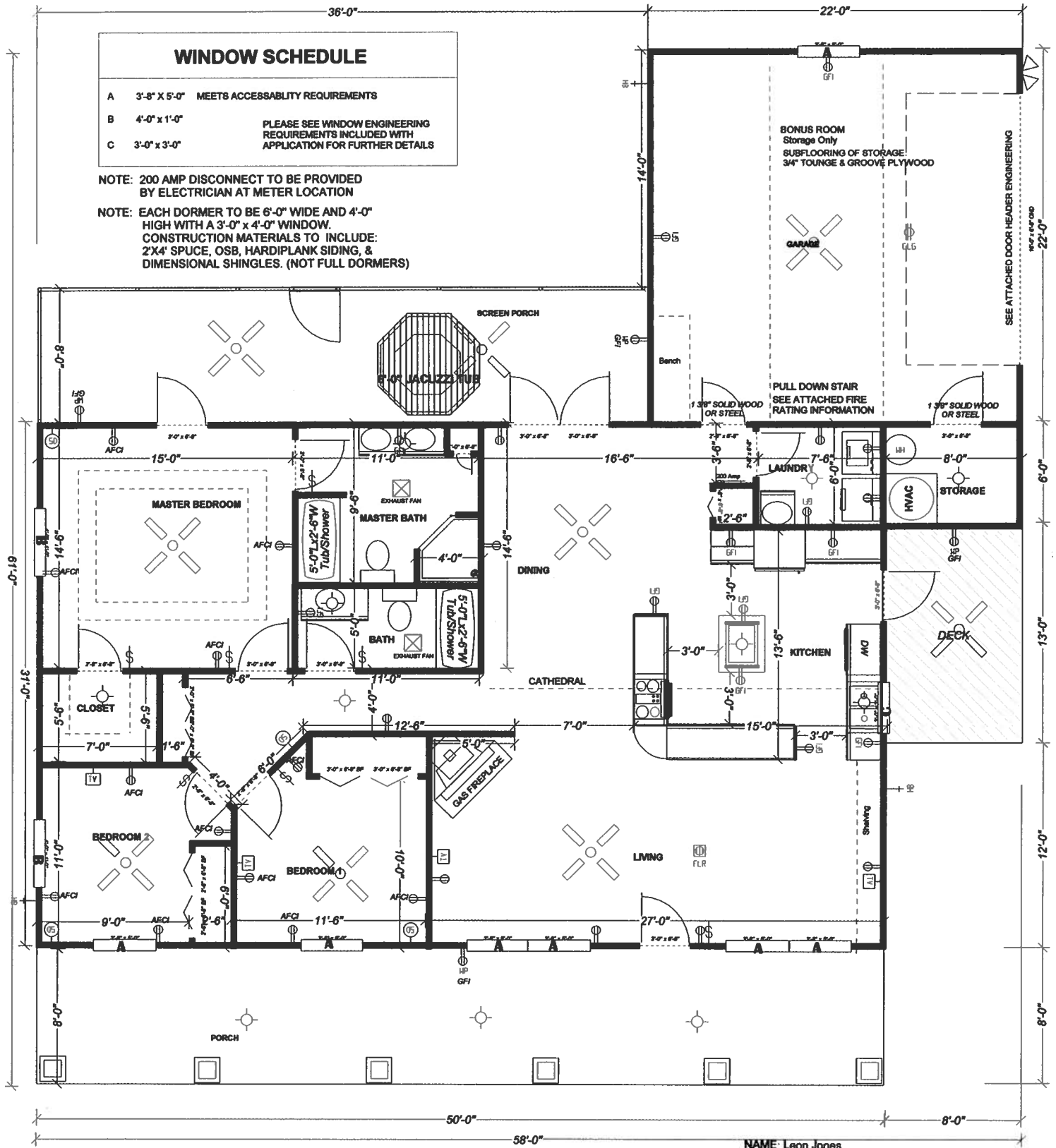
Costs

WINDOW SCHEDULE

- A 3'-8" x 5'-0" MEETS ACCESSABILITY REQUIREMENTS
- B 4'-0" x 1'-0" PLEASE SEE WINDOW ENGINEERING REQUIREMENTS INCLUDED WITH APPLICATION FOR FURTHER DETAILS
- C 3'-0" x 3'-0"

NOTE: 200 AMP DISCONNECT TO BE PROVIDED BY ELECTRICIAN AT METER LOCATION

NOTE: EACH DORMER TO BE 6'-0" WIDE AND 4'-0" HIGH WITH A 3'-0" x 4'-0" WINDOW. CONSTRUCTION MATERIALS TO INCLUDE: 2'X4' SPUCE, OSB, HARDPLANK SIDING, & DIMENSIONAL SHINGLES. (NOT FULL DORMERS)



NAME: Leon Jones
 SIGN: _____
 HEATED sq: 1550
 GARAGE sq: 484
 PORCHES sq: 688
 DECK sq: 104



**ANSI/AAMA/NWDA 101/L.S.2-97
TEST REPORT**

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 450/480/650/680

PRODUCT TYPE: Aluminum Picture Window

Title	Summary of Results
Rating	F-R50 96 x 72
Air Infiltration	0.02 cfm/ft²
Water Resistance Test Pressure	9.0 psf
Uniform Load Deflection Test Pressure	±50.0 psf
Uniform Load Structural Test Pressure	±75.0 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 56249.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com



ANSI/AAMA/NWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.: 56249.01-122-47
Test Date: 03/03/05
Through: 03/04/05
Report Date: 08/25/05
Expiration Date: 03/04/09

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 450/480/650/680, aluminum picture window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for an F-R50 96 x 72 rating. Test specimen description and results are reported herein.

Test Specification: The test specimen was evaluated in accordance with ANSI/AAMA/NWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 450/480/650/680

Product Type: Aluminum Picture Window

Overall Size: 7' 11-3/4" wide by 6' 0" high

Daylight Opening Size: 7' 9-1/8" wide by 5' 9-1/4" high

Overall Area: 48.0 ft²

Finish: All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The lite was interior glazed onto a silicone bedding and secured with aluminum snap in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Glazing beads

Frame Construction: The frame was constructed of thermally improved extruded aluminum. The corners were coped, butted, sealed, and secured with two #6 x 7/8" hex head screws through the head and sill into the jambs.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured with #6 x 1-5/8" drywall screws located in corners and 10" on center. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.02 cfm/ft ²	0.30 cfm/ft ² max.
<i>Note #1: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWDA 101/I.S.2-97 for air infiltration.</i>			
2.1.3	Water Resistance per ASTM E 547 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the head) (Loads were held for 52 seconds)		
	15.0 psf (positive)	0.01"	See Note #2
	15.0 psf (negative)	0.02"	See Note #2

Note #2: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the head) (Loads were held for 10 seconds)		
	22.5 psf (positive)	0.01"	0.37" max.
	22.5 psf (negative)	0.01"	0.37" max.

Test Results: (Continued)

2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: D	Grade: 10	
	Hand and Tool Manipulation Test	No entry	No entry
	<u>Optional Performance</u>		
4.3	Water Resistance per ASTM E 547 (with and without screen)		
	9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the head) (Loads were held for 52 seconds)		
	50.0 psf (positive)	0.04"	See Note #2
	50.0 psf (negative)	0.03"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the head) (Loads were held for 10 seconds)		
	75.0 psf (positive)	0.01"	0.37" max.
	75.0 psf (negative)	0.02"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:



Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician



Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MAH:jld

Revision Log

<u>Rev. #</u>	<u>Date</u>	<u>Page(s)</u>	<u>Revision(s)</u>
0	08/25/05	N/A	Original report issue



**NAN YA PLASTICS CORP.
PLASTPRO INC.**

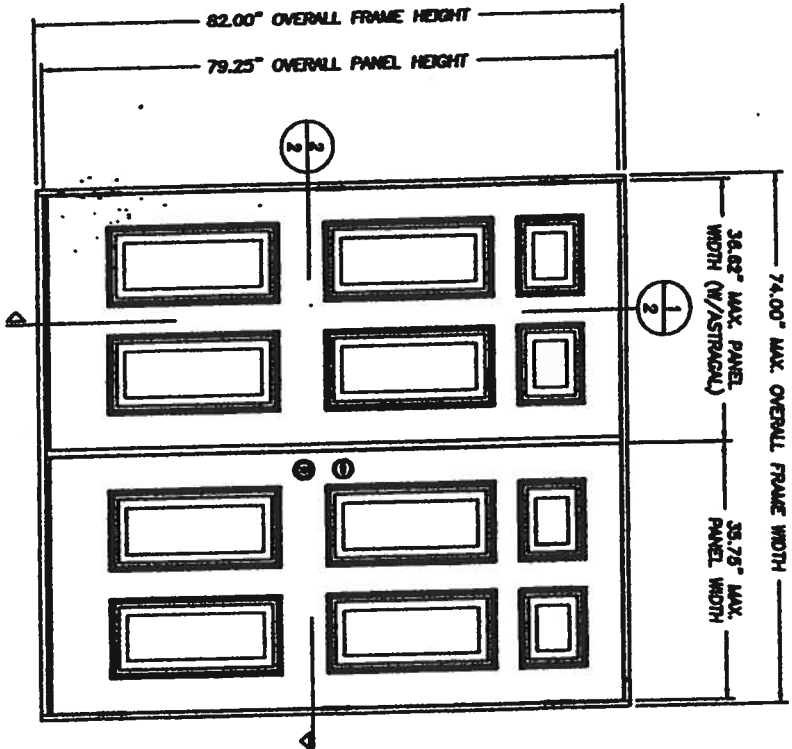
9 PEACH TREE HILL ROAD
LIVINGSTON, NEW JERSEY 07039
PH: 800-779-0861 FAX: 973-758-4001

**DISTINCTION SERIES
6'0" X 6'8" FIBERGLASS DOOR
INSWING / OUTSWING**

GENERAL NOTES

1. THIS PRODUCT HAS BEEN EVALUATED AND IS IN COMPLIANCE WITH THE FLORIDA BUILDING CODE EXCLUDING THE "HIGH VELOCITY, HURRICANE ZONE".
2. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL PRESSING OR STUCCO.
3. WHEN USED IN WIND-BORNE DEBRIS REGIONS THIS PRODUCT COMPLIES WITH SECTION 1609.1.4 OF THE FLORIDA BUILDING CODE AS AN IMPACT RESISTANT PRODUCT AND DOES NOT REQUIRE THE USE OF AN EXTERNAL IMPACT RESISTANT COVERING. THIS PRODUCT DOES NOT MEET THE REQUIREMENTS FOR ENHANCED PROTECTION OF ESSENTIAL FACILITIES AS DEFINED IN ASTM E1986.
4. FOR 2X STUD FRAMING CONSTRUCTION, ANCHORING OF THESE UNITS SHALL BE THE SAME AS THAT SHOWN FOR 2X BLOCK MASONRY CONSTRUCTION.
5. CONDITIONS NOT COVERED BY THIS DRAWING ARE SUBJECT TO FURTHER ENGINEERING ANALYSIS.

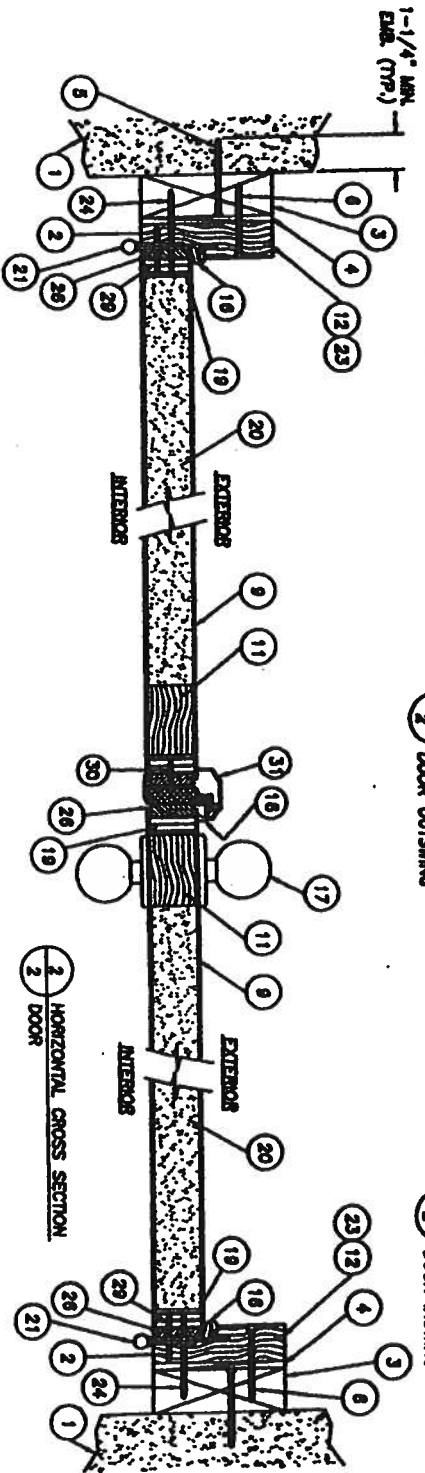
SHEET	DESCRIPTION
1	TYPICAL ELEVATIONS, DESIGN PRESSURES AND GENERAL NOTES
2	VERTICAL AND HORIZONTAL CROSS SECTIONS
3	ANCHORING
4	DETAIL OF MATERIALS AND COMPONENTS



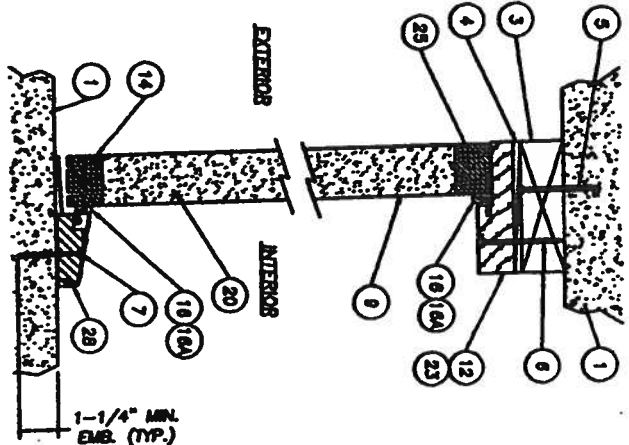
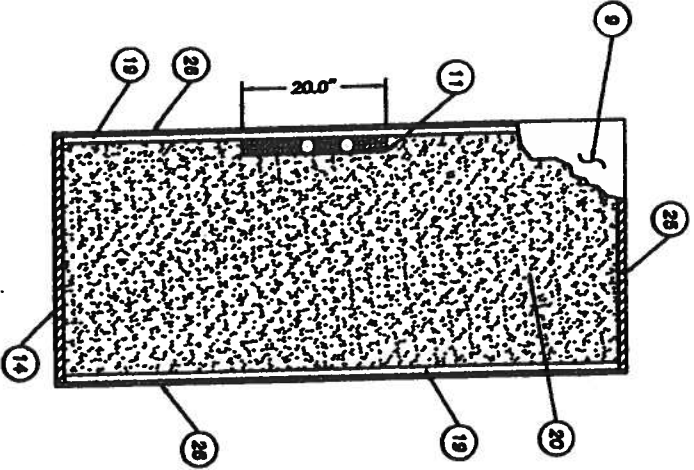
DESIGN PRESSURE RATINGS

IN-SWING		OUT-SWING	
POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
80.0 PSF	80.0 PSF	80.0 PSF	80.0 PSF

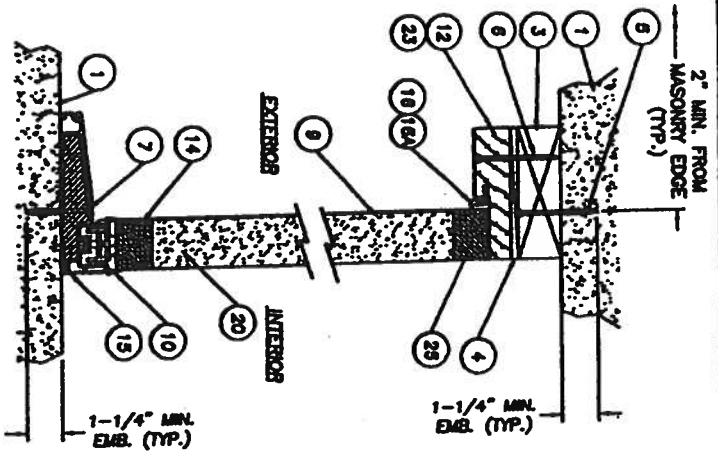
PRODUCT: DISTINCTION SERIES 6'0" X 6'8" FIBERGLASS DOOR INSWING / OUTSWING		Documents Prepared By: <i>RW</i> BUILDING CONSULTANTS, INC. P.O. Box 230 Verona N.J. 07093 Phone No. 943.688.9197 Florida Board of Professional Engineers Certificate Of Authorization No. 0812 <i>Robert W. Hines</i> Resident V. Hines, P.E. No. 04108	
DATE: 8/10/05 REVISIONS: REVERSE TO 2004 FIG		PROJECT: TYPICAL ELEVATIONS, DESIGN PRESSURES & GENERAL NOTES	
DATE: 09/21/05 SCALE: N.T.S. BY: EW CHECKED BY: BWH PROJECT NO.: FL-650	SHEET: 1 OF 4		



1-1/4" MIN. EMB. (TYP.)



1-1/4" MIN. EMB. (TYP.)



NO.	DATE	REVISIONS
1	8/08/05	REVISE TO 2004 FBC

PRODUCT:
 DISTINCTION SERIES
 6'0" x 6'6" FIBERGLASS
 INSWING/OUTSWING
 PART OR ASSEMBLY:
 VERTICAL AND HORIZONTAL
 CROSS SECTIONS

Documents Prepared by
RW BUILDING CONSULTANTS, INC.
 P.O. Box 230 Vero Beach, FL 32908
 Phone No. 888.688.6167
 Florida Board of Professional Engineers
 Certificate of Registration No. 00113
 9/2/05
 David R. Wright, P.E. No. 05128

DATE 08/21/05
 SCALE N.T.S.
 DESIGNED BY
 CHECKED BY
 DRAWN BY
 FL-550
 SHEET 2 OF 4



NAN YA PLASTICS CORP.
PLASTPRO INC.

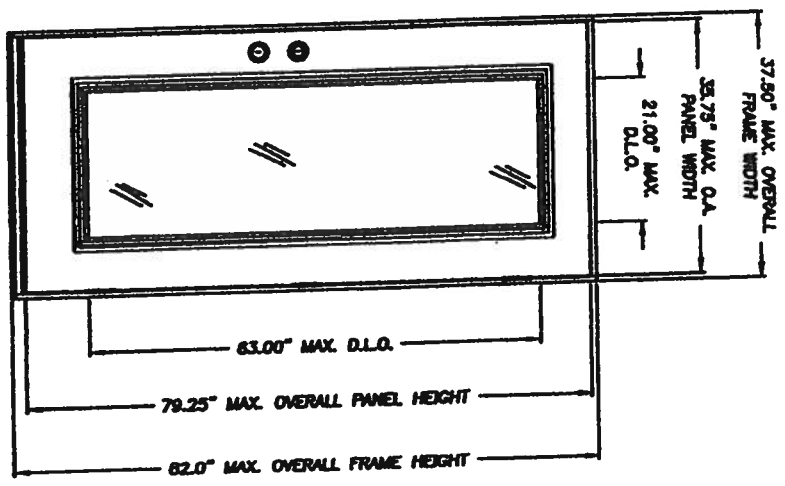
9 PEACH TREE HILL ROAD
LIVINGSTON, NEW JERSEY 07039
PH: 800-779-0661 FAX: 973-758-4001

DISTINCTION SERIES
3'0" x 6'8" GLAZED
SINGLE
FIBERGLASS DOOR
INSWING / OUTSWING

GENERAL NOTES

1. THIS PRODUCT HAS BEEN EVALUATED AND IS IN COMPLIANCE WITH THE FLORIDA BUILDING CODE EXCLUDING THE "HIGH VELOCITY HURRICANE ZONE".
2. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS, ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
3. WHEN USED IN AREAS REQUIRING WIND-BORNE DEBRIS PROTECTION THIS PRODUCT IS REQUIRED TO BE PROTECTED WITH AN IMPACT RESISTANT COVERING THAT COMPLIES WITH SECTION 1608.1.4 OF THE FLORIDA BUILDING CODE.
4. FOR 2X STUD FRAMING CONSTRUCTION, ANCHORING OF THESE UNITS SHALL BE THE SAME AS THAT SHOWN FOR 2X BRICK MASONRY CONSTRUCTION.
5. CONDITIONS NOT COVERED BY THIS DRAWING ARE SUBJECT TO FURTHER ENGINEERING ANALYSIS.

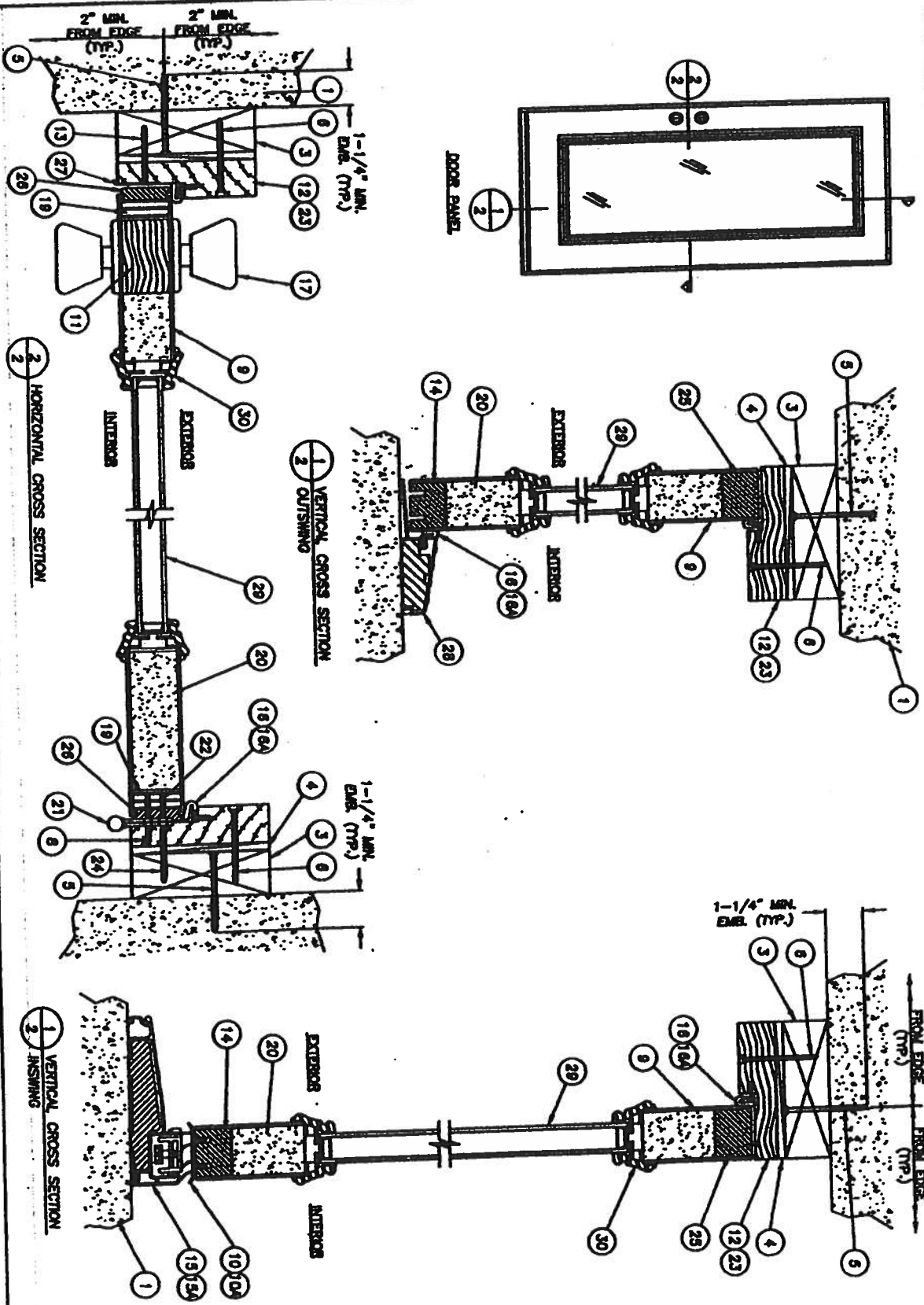
TABLE OF CONTENTS	
SHEET /	DESCRIPTION
1	TYPICAL ELEVATIONS, DESIGN PRESSURES & GENERAL NOTES
2	VERTICAL & HORIZONTAL CROSS SECTIONS
3	BACK & DOOR ANCHORING
4	BILL OF MATERIALS, GLAZING DETAILS & COMPONENTS



SINGLE DOOR INSWING ELEVATION
VIEWED FROM INTERIOR

TYPE	OVERALL FRAME DIMENSION	OVERALL DAYLIGHT DIMENSION	GLASS TYPE	DESIGN PRESSURE (PSF)
INSWING	37.50" x 82.00"	21.00" x 63.00"	1/8" TEMP.	+50.0
OUTSWING	37.50" x 82.00"	21.00" x 63.00"	- AR - 1/8" TEMP.	-50.0

<p>REVISIONS</p> <table border="1"> <tr> <th>NO.</th> <th>DATE</th> <th>BY</th> <th>DESCRIPTION</th> </tr> <tr> <td> </td> <td> </td> <td> </td> <td> </td> </tr> </table>	NO.	DATE	BY	DESCRIPTION					<p>PRODUCT: DISTINCTION SERIES 3'0" x 6'8" FIBERGLASS DOOR INSWING / OUTSWING</p> <p>PANEL OR ASSEMBLY: TYPICAL ELEVATIONS, DESIGN PRESSURES & GENERAL NOTES</p>	<p>Drawings Prepared For:</p> <p>RW BUILDING CONSULTANTS, INC. P.O. Box 230 Venice Fl. 33588 Phone No.: 613.688.9177</p> <p>Florida Board of Professional Engineers Certificate Of Authorization No. 9013</p> <p><i>Edward J. ...</i> Member of Florida P.E. No. 51158</p>
NO.	DATE	BY	DESCRIPTION							



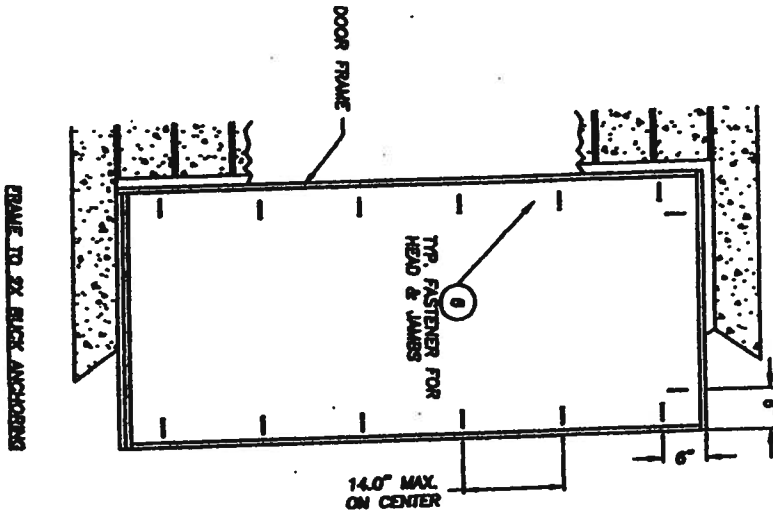
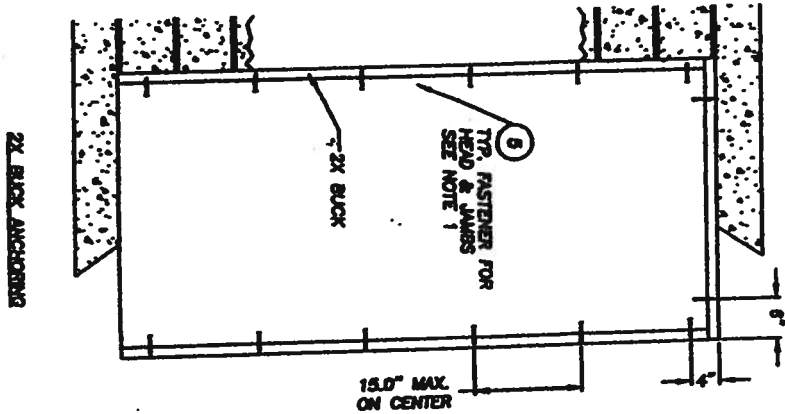
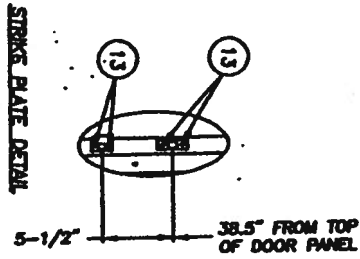
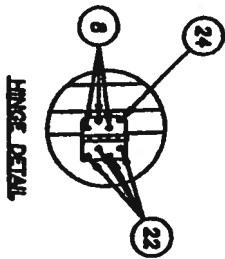
NO.	DATE	REVISIONS
1	12/02/05	REVISED TO 2004 FIG
2	08/22/05	

PRODUCT:
 DISTINCTION SERIES
 50" x 64" FIBERGLASS DOOR
 INSULATED / OUTSWING
PART OR ASSEMBLY:
 VERTICAL & HORIZONTAL
 CROSS SECTION

Documents Prepared by
RW BUILDING CONSULTANTS, INC.
 P.O. Box 230 Venice, FL 33598
 Phone 615.622.9197
 Florida Board of Professional Engineers
 Certificate of Professional No. 0013
Wendell R. Williams 8/2/05
 Wendell R. Williams, P.E., No. 0013

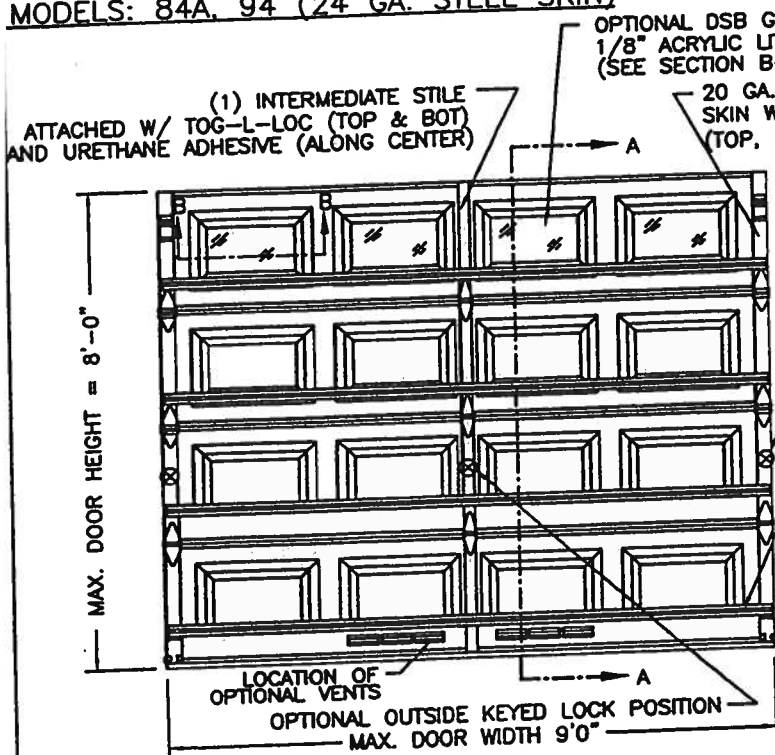
DRAWN BY: N.T.S.
 CHECKED BY: E.V.
 DATE: 08/22/05
 PROJECT NO.: FL-051
 SHEET NO.: 2 OF 4

- NOTES**
- 3/16" TAPCONS REQUIRE A MINIMUM 2.0" CLEARANCE TO MASONRY EDGES AND A MINIMUM 2.25" CLEARANCE TO ADJACENT TAPCONS.

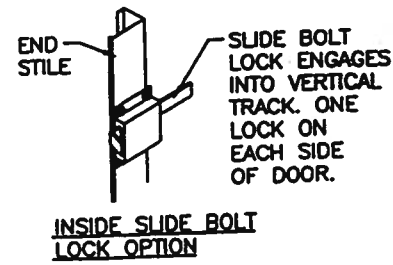


REVISIONS 1 11/01/08 CHANGE TO 2004 FIG DATE		PRODUCT DISTINCTION SERIES 30" x 68" REFRIGERASS DOOR INSULATED / GLASSING PART OR ASSEMBLY BUCK & DOOR ANCHORING		Drawings Prepared by RW BUILDING CONSULTANTS, INC. P.O. Box 230 Valrico FL 33596 Phone No. 813.988.9177 Florida Board of Professional Engineers Certificate Of Authorization No. 0613 [Signature] 11/05 Ronald E. Hines, P.E. SEC. 1105	
DATE 08/22/08 DESIGNED BY N.T.S. CHECKED BY WWH DRAWING NO. FL-001	SHEET 3 OF 4				

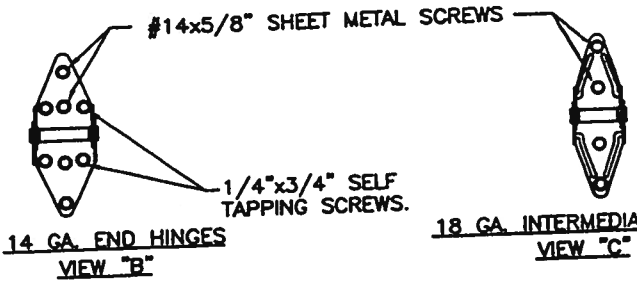
MODELS: 73, 75 (25 GA. STEEL SKIN)
MODELS: 84A, 94 (24 GA. STEEL SKIN)



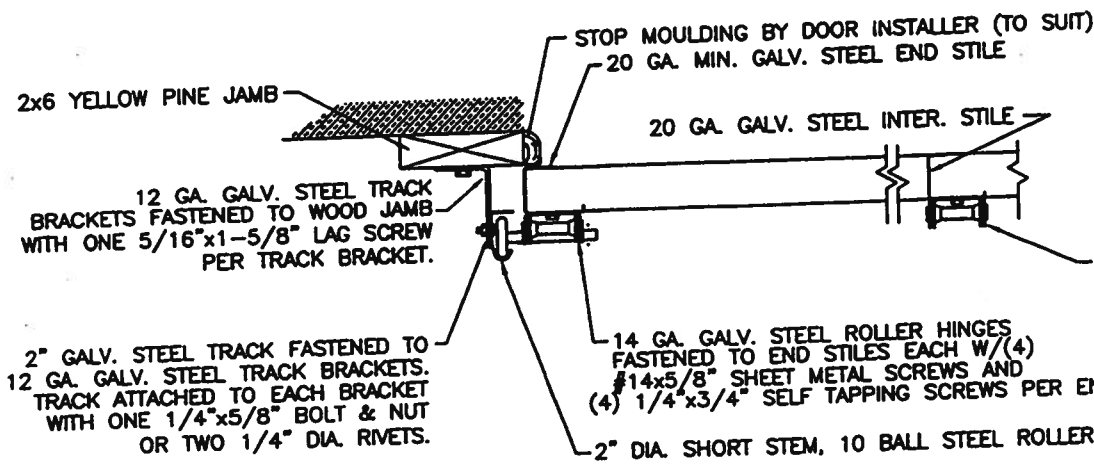
25 GA. MIN. STEEL SKIN COATED WITH GALV., BAKED-ON PRIMER, AND A BAKED-ON POLYESTER TOP COAT APPLIED TO BOTH SIDES OF STEEL SKIN.



NOTE 1: DOORS UP TO 7'-0" HIGH CONSIST OF (4) SECTIONS (SHOWN). DOORS OVER 7'-0" HIGH CONSIST OF (5) SECTIONS (NOT SHOWN).



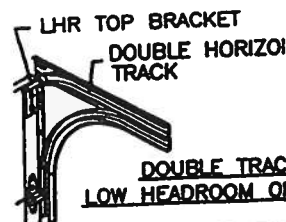
VERTICAL JAMB ATTACHMENT (C-90 BLOCK OR 2,000 PSI MIN. CONCRETE COLUMN):
 3/8"x4" SLEEVE ANCHOR BOLTS ON 24" CENTERS (BOTH C-90 BLOCK OR 2,000 PSI MIN. CONCRETE) OR 1/4"x4" TAPCON SCREWS ON 24" CENTERS (2,000 PSI MIN. CONCRETE). WASHERS INCLUDED WITH SLEEVE ANCHORS, 1" O.D. WASHERS REQUIRED WITH TAPCONS. ANCHORS MAY BE COUNTERSUNK (BUT NOT REQUIRED) TO PROVIDE A FLUSH MOUNTING SURFACE. HORIZONTAL JAMBS DO NOT TRANSFER LOAD.



VERTICAL JAMB ATTACHMENT
 1/2"x3" LAG SCREWS ON WASHER REQUIRED. LAG S (BUT NOT REQUIRED) TO SURFACE. HORIZONTAL JAW

PREPARATION OF JAMBS BY OTHERS.

JAMB CONFIGURATION SECTION C-C

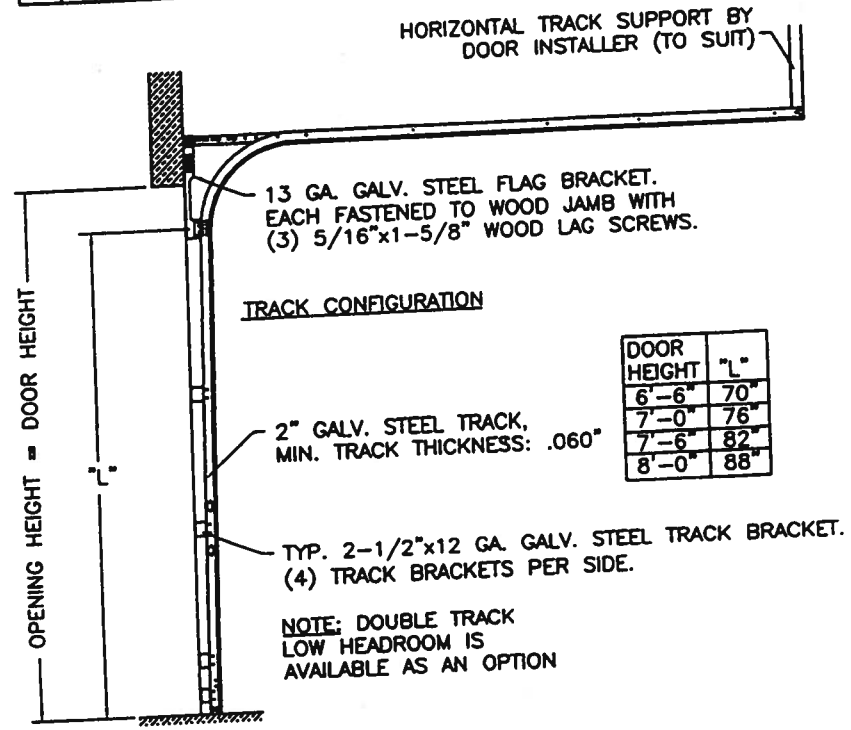
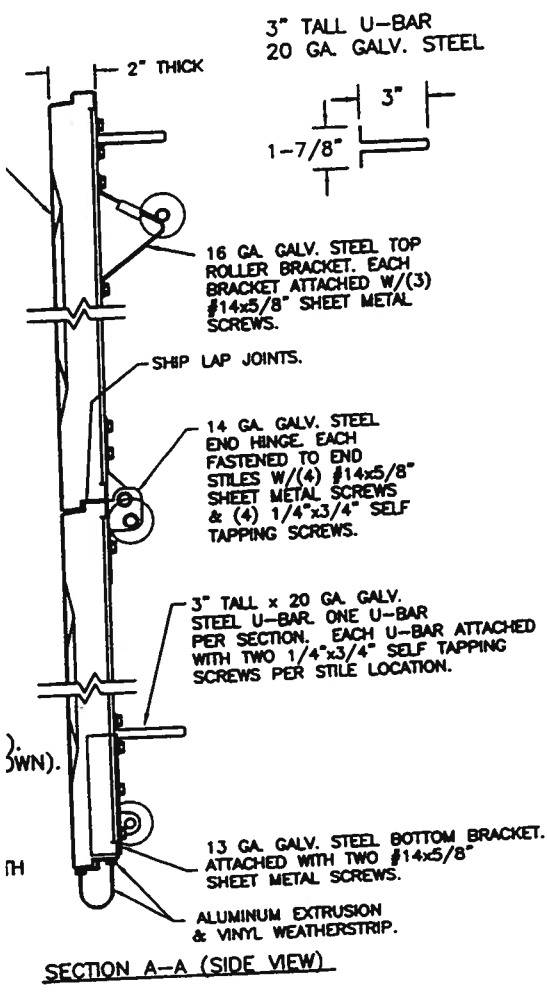


REPORT#9606C

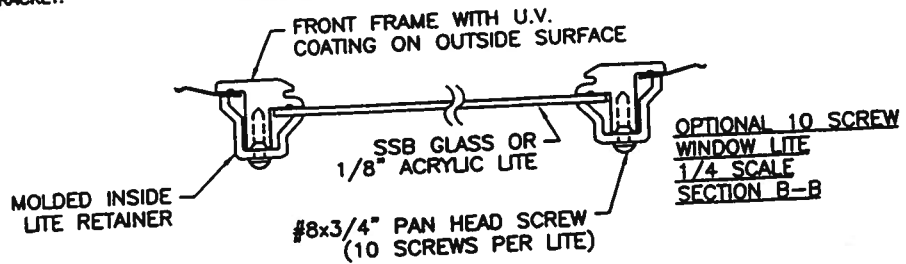


NOTE 2: THE DESIGN OF THE SUPPORTING STRUCTURAL ELEMENTS SHALL BE THE RESPONSIBILITY OF THE PROFESSIONAL OF RECORD FOR THE BUILDING OR STRUCTURE AND IN ACCORDANCE WITH CURRENT BUILDING CODES FOR THE LOADS LISTED ON THIS DRAWING.

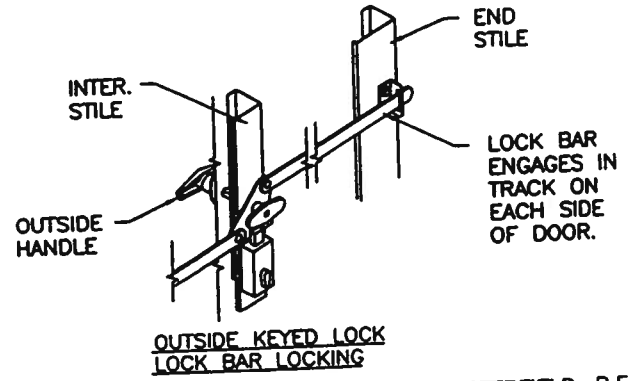
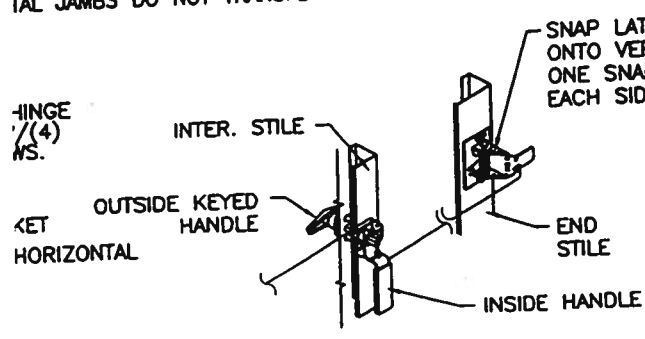
REV	DATE	DESCRIPTION
2	9/96	REDRAWN ON ONE PAGE
03	12/1999	ADDED JAMB ATTACHMENT INFORMATION
04	1/2000	DESIGN LOADS WERE +/-24.2; ADD 50 KSI STRUT SPEC.
05	3/16/2000	ADDED LHR TRACK OPTION NOTE; DEL. M/N 82/90
06	11/2000	QTY. (1) WAS (2) TRACK BOLTS



DOOR HEIGHT	"L"
6'-6"	70"
7'-0"	76"
7'-6"	82"
8'-0"	88"



ATTACHMENT (WOOD FRAME BUILDINGS):
 WS ON 24" CENTERS. 1" O.D.
 LAG SCREWS MAY BE COUNTERSUNK
 TO PROVIDE A FLUSH MOUNTING
 METAL JAMBS DO NOT TRANSFER LOAD.



E TRACK
ROOM OPTION

OUTSIDE KEYED LOCK
SNAP LATCH LOCK OPTION

DESIGN ENGINEER: MARK WESTERFIELD, P.E.
 FLORIDA P.E. #48495
 DESIGN LOADS: +25.0 P.S.F. & -32.0 P.S.F.
 TEST LOADS: +37.5 P.S.F. & -48.0 P.S.F.



CLOPAY BUILDING PRODUCTS COMPANY
 312 WALNUT STREET, SUITE 1600
 CINCINNATI, OHIO 45202
 (513) 381-4800

Clipay Corporation All Rights Reserved.

ORIGINAL DRAWINGS ARE SIGNED IN BLUE
 INK WITH A RAISED SEAL (EMBOSSED).

SCALE:	NOTED	PLUT:	MAX. DOOR SIZE:
DATE:	9/23/96	DESCRIPTION:	9'W x 8'H
DRAWN BY:	MWW	73/75/84A/94, 9'W +25/-32	REV:
CHECKED BY:			B
		DRAWING NUMBER:	300159

May 12 04 06:52a

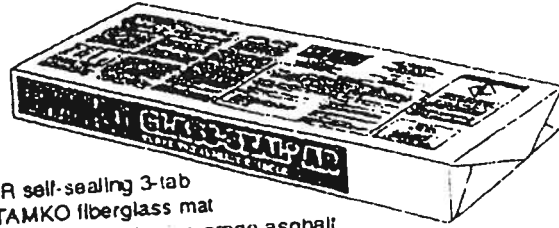
ATTN: LEON JONES

TAMKO Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

PRODUCT DATA

Manufactured in Tuscaloosa, AL



TAMKO ELITE GLASS-SEAL® AR self-sealing 3-tab shingles are made with a tough TAMKO fiberglass mat coated on both sides with a thick layer of weathering-grade asphalt and surfaced with ceramic granules.

USES

For application to roof decks with inclines of not less than 2 inches per foot. For slopes between 2 inches and 4 inches per foot, refer to wrapper instructions.

ADVANTAGES

- 25-year limited warranty, 3-year FULL START, limited transferability, winds up to 60 MPH
- Superior fire resistance compared to organic shingles.
- 10-year Algae-Resist (AR) limited warranty that provides for cleaning or discoloration caused by certain algae growth that may occur in areas with high humidity.

CERTIFICATIONS

UL Class A Fire Rating
 UL Wind Resistant
 ASTM D 3018, Type I
 Miami Dade County Florida NOA 02-0130-03
 Expiration Date: 04/1/07

ASTM D 3167, Type I (modified to 110 mph)
 ASTM D 3462
 ASTM E 108 Class A
 TAS 100-95 Wind and Wind Driven Rain

Fed. Spec.: Exceeds SS-S-001634,
 Class A, Type I

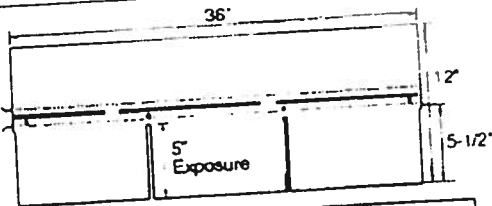
COLORS

- Glacier White
- Grey Blend
- Rustic Hickory
- Pastel Red
- Shadow Grey
- Rustic Black
- Weathered Wood
- Olde English Pewter
- Pastel Green
- Empire Green Blend
- Driftwood
- Rustic Cedar
- Oxford Grey
- Pastel Brown
- Tile Red Blend
- Desert Sand

PRODUCT DATA

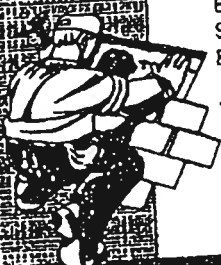
Shingle size 12" X 36"
 Exposure 5"
 Shingles per square 80
 Bundles per square 3

Nail Zone



*All values stated as nominal

CAUTION: The National Institute for Occupational Safety and Health (NIOSH) has concluded that fumes of heated asphalt are a potential occupational carcinogen. Do not heat or burn this product.



TAMKO

ROOFING PRODUCTS

Makers of ELITE GLASS-SEAL® and registered trademarks of TAMKO Roofing Products, Inc.

Visit our Web Site at www.tamko.com

Central District	220 West 4th St., Joplin, MO	6480	800-641-4891
Northeast District	4600 Tamko Dr., Frederick, MD	2170	800-368-2055
Southeast District	2300 36th St., Tuscaloosa, AL	3540	800-228-2656
Southwest District	7910 S. Central Exp., Dallas, TX	75216	800-443-1834
Western District	5300 East 43rd Ave., Denver, CO	80216	800-530-8858



- Series 3540 Single Hung and Fixed Windows
- Series 8540 Single Hung and Fixed Windows

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" VINYL FIN WINDOWS

1. Storage: Do not lay windows flat, lean multiple units against poles, or store in the sun before installing.
2. Handle units one at a time in the closed and locked position. Place a continuous bead of adhesive caulk such as silicone or urethane on the back side of "nail fins" (mounting flanges) before placing in opening.
3. Place shims under corners of sill. In the closed and locked position, set unit into opening and make sure that there is $3/16" \pm 1/16"$ clearance around the frame. Starting at the center of the longest frame member, place #8 sheet metal or wood screws (with a minimum of 1" penetration into the framing) in every other pre-punched slot which are on 4" to 5" centers (max. spacing 10"). Make sure that screws are driven in straight in order to avoid twisting or bowing. Make sure that the head and sill are straight and level and the jambs are straight and plumb. Check operation of unit frequently as fasteners are set.
4. Note: Adherence to the above screw spacing and caulk requirements will allow this product to be used for design pressures (DP's) up to and including ± 47.2
5. Caulk entire perimeter of the fin to mounting surface joint. Also caulk over screw heads and unused slots. Note: This step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jamb 2nd., head 3rd).
6. Fill voids between window frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows. The use of expanding aerosol type insulating foam, which can bow the frame, voids all stated warranties. The use of muriatic acid for brick clean-up may damage the coil spring sash balance system. Windows must be masked off to avoid muriatic acid exposure, which will void the warranty.
7. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.
8. For structures finished in brick or stone, allow $1/4"$ gap under the sill, then caulk this joint when complete. Also caulk the head and jamb joints in the same manner.
9. If one or more holes are field drilled (by others) in any area of the window sill for any purpose (such as security systems), the warranty will be void. Adding holes can cause water leaks and interior damage.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress, structural performance, and energy performance.

Headquarters: M.I. Home Products 650 West Market St. Gratz, PA 17030 (717) 365-3300 www.mihp.com

JL 2 21
JUNE 26, 2003

**AAMA/NWDA 101/I.S.2-97
TEST REPORT**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 450/650/850 Drop In Glazing
TYPE: Aluminum ~~Single Hung Window~~**

Title	Summary of Results
AAMA Rating	H-LC30 53 x 90
Operating Force	24 lb max.
Air Infiltration	0.11 cfm/ft ²
Water Resistance Test Pressure	6.75 psf
Uniform Load Deflection Test Pressure	+32.8 psf -47.2 psf
Uniform Load Structural Test Pressure	+49.2 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-42487.01 for complete test specimen description and data.



Architectural Testing

AAMA/NWVDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-42487.01
Test Date: 08/14/02
And: 08/15/02
Report Date: 10/02/02
Expiration Date: 08/15/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 450/650/850 Drop In Glazing, aluminum single hung window at their facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-LC30 53 x 90 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWVDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 450/650/850 Drop In Glazing

Type: Aluminum Single Hung Window

Overall Size: 4' 5-1/8" wide by 7' 5-5/8" high

Interior Sash Size: 4' 2-3/4" wide by 3' 8-7/8" high

Fixed Daylight Opening Size: 4' 0" wide by 3' 5-3/8" high

Screen Size: 4' 0-3/4" wide by 3' 8-3/4" high

Finish: The unit was white.

Glazing Details: The specimen utilized 5/8" thick, sealed insulating glass constructed from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.190" high by 0.187" polypile with center fin	1 Row	Fixed meeting rail interlock
0.190" high by 0.187" polypile with center fin	2 Rows	Interior sash stiles
1/4" vinyl foam-filled bulb seal	1 Row	Interior sash bottom rail
5/8" wide by 7/8" long polypile plug	4 Pieces	Interior sash, all corners

Frame Construction: The frame was constructed of extruded aluminum. Each corner was coped, butted, sealed, and fastened with two #8 x 1" screws per corner through the head and sill into jamb screw boss. End caps were utilized on the ends of the meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was then secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum. Each corner was coped, butted, and fastened with one #8 x 1-1/4" screw per corner.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock	2	Interior sash, 6-1/2" from top rail ends
Spring-loaded coil balance	2	One per jamb
Plastic tilt latch	2	Interior sash top rail ends
Metal tilt latch pin	2	Interior sash bottom rail ends
Screen spring-loaded retainer pin	2	6-3/4" from rails on stiles

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The specimen was installed into a #2 2 x 8 Spruce-Pine-Fir wood buck. #8 x 1-5/8" drywall screws were placed 3" from corners and 15" on center around nailing fin. Polyurethane was used as a sealant around the exterior perimeter.

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	24 lbs	35 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.11 cfm/ft ²	0.3 cfm/ft ² max.
2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 3.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds) @ 25.0 psf (positive) @ 25.0 psf (negative)	0.64"* 0.54"*	0.29" max. 0.29" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

**Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 37.5 psf (positive) @ 37.5 psf (negative)	0.04" 0.03"	0.20" max. 0.20" max.
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Test Results:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.2	De-glazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Interior sash meeting rail	0.12"/25%	0.50"/100%
	Interior sash bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Interior sash right stile	0.06"/12%	0.50"/100%
	Interior sash left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

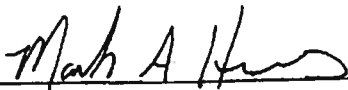
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 32.8 psf (positive)	0.85"*	0.29" max.
	@ 47.2 psf (negative)	0.87"*	0.29" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 49.2 psf (positive)	0.09"	0.20" max.
	@ 70.8 psf (negative)	0.12"	0.20" max.

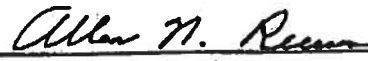
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC:



Mark A. Hess
Technician

MAH:nlb
01-42487.01



Allen N. Reeves, P.E.
Director - Engineering Services

11 OCTOBER 2002



DUCT SYSTEM SUMMARY

Entire House

LARRY RESMONDO A/C

Job: BUSSEY RESIDENCE
10/12/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: LEON JONES CONSTRUCTION
FL

	HEATING	COOLING
External Static Pressure:	0.10 in H2O	0.10 in H2O
Pressure Losses:	0.25 in H2O	0.25 in H2O
Available Static Pressure:	-0.2 in H2O	-0.2 in H2O
Friction Rate:	0.100 in/100ft	0.100 in/100ft
Actual AVF:	1200 cfm	1200 cfm

Total Effective Length (TEL): 230 ft

Supply Branch Detail Table

Name	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trnk
WHOLE HOUSE	4310	3754	200	200	0.100	453	9	0x 0	VIFx	st1A
WHOLE HOUSE-A	4306	3754	200	200	0.100	453	9	0x 0	VIFx	st1
WHOLE HOUSE-B	4306	3754	200	200	0.100	453	9	0x 0	VIFx	st1
WHOLE HOUSE-C	4306	3754	200	200	0.100	453	9	0x 0	VIFx	st1
WHOLE HOUSE-D	4306	3754	200	200	0.100	453	9	0x 0	VIFx	st1
WHOLE HOUSE-E	4306	3754	200	200	0.100	453	9	0x 0	VIFx	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	1200	1200	679	18	0 x 0	RectFbg	
st1A	Peak AVF	200	200	367	10	0 x 0	RectFbg	st1

Return Branch Detail Table

Name	Diffus Sz (in)	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trunk
rb1	0 x 0	25839	22524	1200	1200	0.100	550	20	0x 0	VIFx	

Bold/italic values have been manually overridden



RIGHT-J BUILDING ANALYSIS REPORT

Entire House

LARRY RESMONDO A/C

Job: BUSSEY RESIDENCE
10/12/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

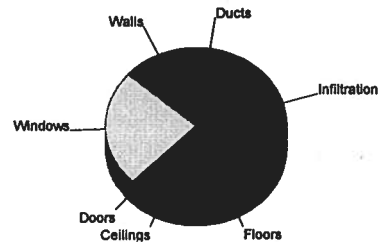
For: LEON JONES CONSTRUCTION
FL

Design Information

	Htg	Clg	Method	Infiltration	Simplified Average
Outside db (°F)	33	92	Construction quality		0
Inside db (°F)	70	75	Fireplaces		
Design TD (°F)	37	17			
Daily range	-	M			
Inside humidity (%)	-	50			
Moisture difference (gr/lb)	-	52			

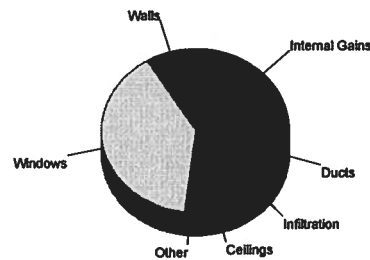
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	2.8	3527	13.7
Windows	27.8	6209	24.0
Doors	17.0	715	2.8
Ceilings	1.2	1893	7.3
Floors	30.0	4256	16.5
Infiltration	30.2	8007	31.0
Ducts		1230	4.8
Total		25836	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	1.6	1975	8.8
Windows	39.8	8877	39.4
Doors	9.5	398	1.8
Ceilings	1.1	1688	7.5
Floors	0.0	0	0.0
Infiltration	6.9	1839	8.2
Ducts		2048	9.1
Internal gains		5700	25.3
Total		22525	100.0



Cooling at 84 % SHR = 2.2 ton

Cooling at 70 % SHR = 2.6 ton

Overall U-Value = 0.141 Btuh/ft²-°F

Cooling air flow = 554 cfm/ton

Cooling at 400 cfm/ton = 3.0 ton

Data entries checked.



RIGHT-J LOAD AND EQUIPMENT SUMMARY

Entire House

LARRY RESMONDO A/C

Job: BUSSEY RESIDENCE
10/12/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: LEON JONES CONSTRUCTION
FL

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Building heat loss	25836 Btuh
Ventilation air	0 cfm
Ventilation air loss	0 Btuh
Design heat load	25836 Btuh

Sensible Cooling Equipment Load Sizing

Structure	22525 Btuh
Ventilation	0 Btuh
Design temperature swing	3.0 °F
Use mfg. data	n
Rate/swing multiplier	0.97
Total sens. equip. load	21849 Btuh

Infiltration

Method	Simplified	
Construction quality	Average	
Fireplaces	0	
	Heating	Cooling
Area (ft ²)	1550	1550
Volume (ft ³)	14725	14725
Air changes/hour	0.8	0.4
Equiv. AVF (cfm)	197	98

Latent Cooling Equipment Load Sizing

Internal gains	690 Btuh
Ventilation	0 Btuh
Infiltration	3448 Btuh
Total latent equip. load	4138 Btuh
Total equipment load	25987 Btuh

Heating Equipment Summary

Make	RUUD AIR COND
Trade	Ruud UPMB Series
	UPMB-036JA
Efficiency	8.0 HSPF
Heating input	0 Btuh
Heating output	0 Btuh
Heating temp rise	0 °F
Actual heating fan	1200 cfm
Heating air flow factor	0.046 cfm/Btuh
Space thermostat	

Cooling Equipment Summary

Make	RUUD AIR COND
Trade	Ruud UPMB Series
	UPMB-036JA
	UBHJ-21+RCHJ-36A2
Efficiency	13.0 SEER
Sensible cooling	24500 Btuh
Latent cooling	10500 Btuh
Total cooling	35000 Btuh
Actual cooling fan	1200 cfm
Cooling air flow factor	0.053 cfm/Btuh
Load sensible heat ratio	84 %

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RIGHT-J SHORT FORM Entire House

LARRY RESMONDO A/C

Job: BUSSEY RESIDENCE
10/12/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: LEON JONES CONSTRUCTION
FL

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	92	Method	Average
Inside db (°F)	70	75	Construction quality	0
Design TD (°F)	37	17	Fireplaces	
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-036JA

Efficiency 8.0 HSPF

Heating input 0 Btuh
Heating output 0 Btuh
Heating temperature rise 0 °F
Actual heating fan 1200 cfm
Heating air flow factor 0.046 cfm/Btuh

Space thermostat

COOLING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-036JA
UBHJ-21+RCHJ-36A2

Efficiency 13.0 SEER

Sensible cooling 24500 Btuh
Latent cooling 10500 Btuh
Total cooling 35000 Btuh
Actual cooling fan 1200 cfm
Cooling air flow factor 0.053 cfm/Btuh

Load sensible heat ratio 84 %

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
WHOLE HOUSE	1550	25836	22525	1200	1200
Entire House	1550	25836	22525	1200	1200
Ventilation air		0	0		
Equip. @ 0.97 RSM			21849		
Latent cooling			4138		
TOTALS	1550	25836	25987	1200	1200

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

RESIDENTIAL WIND DESIGN & ANALYSIS

FBC SECTION 1609 \ *NO COPIES ARE TO BE PERMITTED*

PREPARED FOR:

M. L. JONES CONSTRUCTION

PREPARED BY:

**MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329**

SUMMARY

OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/131 ea
 Interior zone spacing: Interior 8 in. Periphery 4 in.
 Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
 Interior stud spacing 16 in. Composite (yes or no) Y
 End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
59' Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.
63' Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 327 pounds per linear foot

Wall tension transferred by: Siding nails 8d/131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
 Size 1/2 in. Shape L Washer 2" Embedment 7 in.
 Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.
 Monolithic footing: Depth 20 in. Bottom width 12 in.

Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
 Interior Footings: 16" W X 10" D

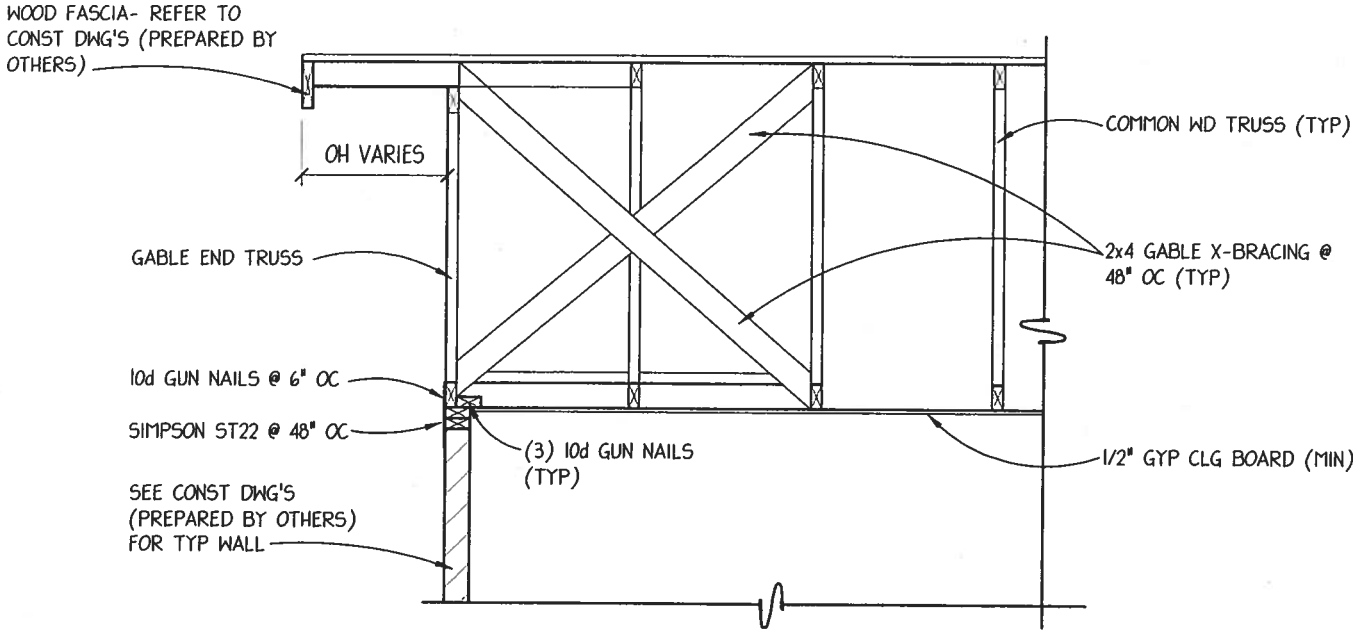
Porch Columns: 4x4XP 5/8" @ 120" o.c. MAX

Porch Column Fasteners: Super CB44/CC44 or Equal

NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, **NOT** a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

[Handwritten Signature]
 1/16/06



THIS WIND ANALYSIS ASSUMES IDEAL SOIL CONDITIONS W/MIN 2500 PSI SOIL BRG CAPACITY & 95% DRY PROCTOR DENSITIES

GABLE END DETAIL

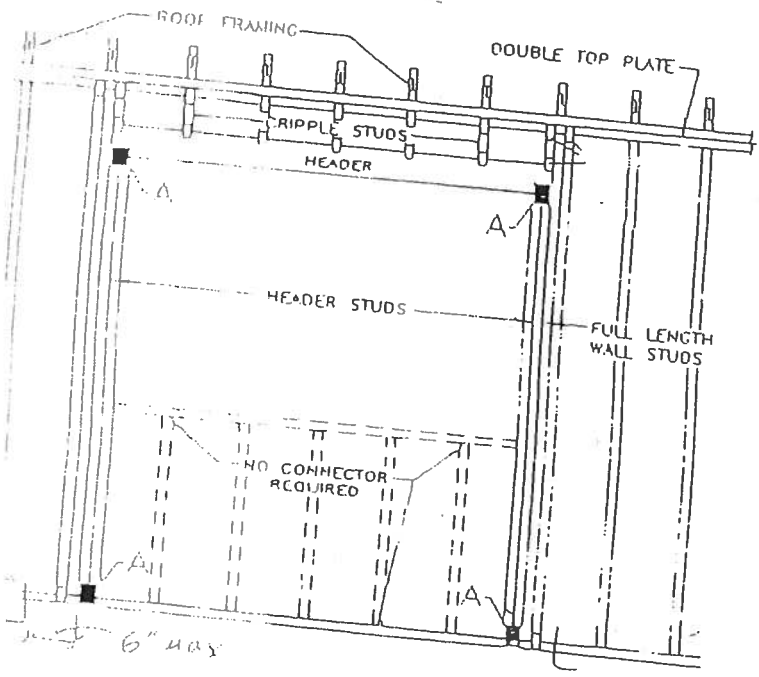
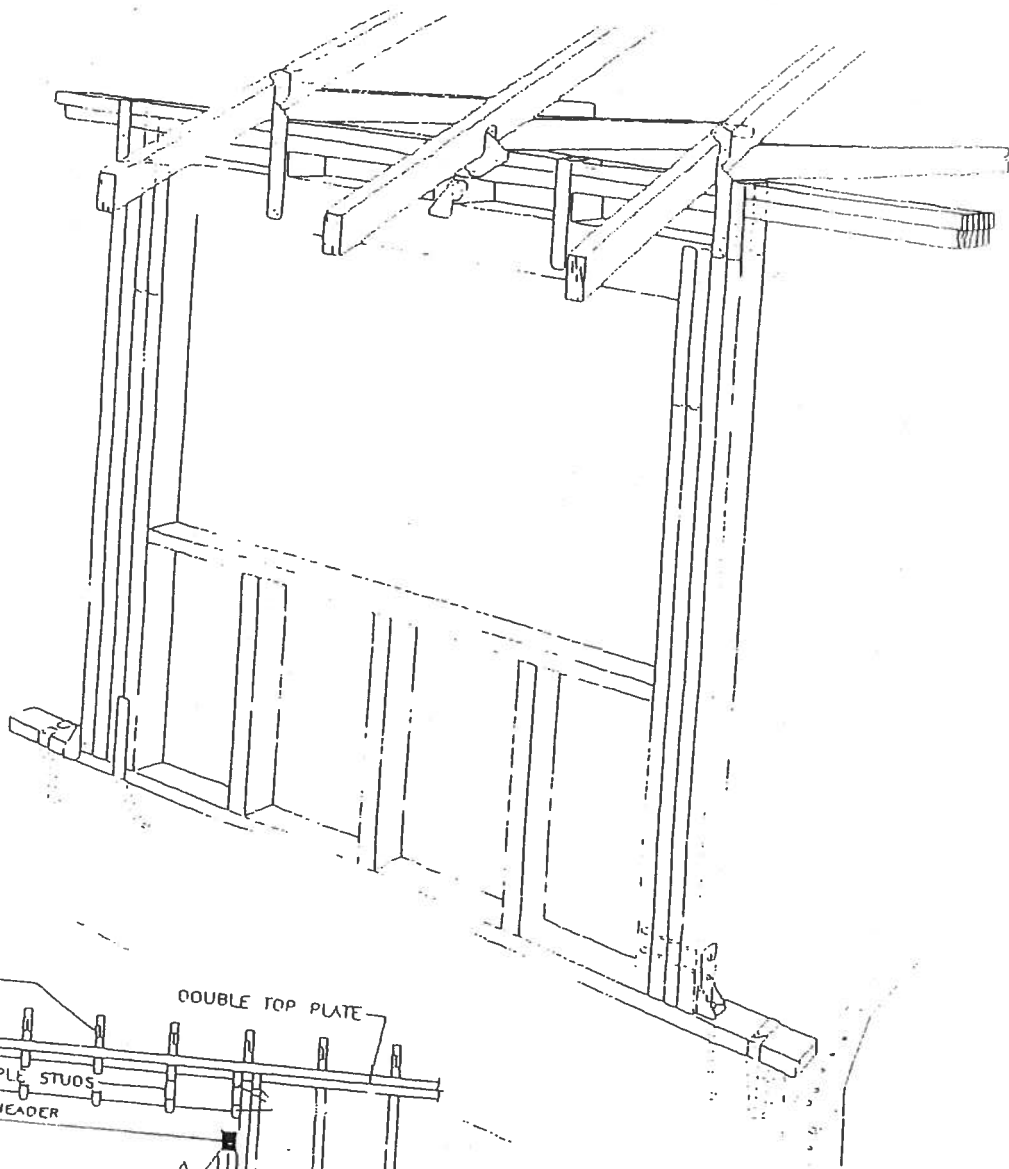
SCALE: 1/2" = 1'-0"

NOTE:
 INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WITH THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN ERECTION & BRACING SHEET.

Handwritten signature and date:
 AMW/3985
 10/16/06

Q:\SUPPORT\DETAILS\WINDLOADS\02-GABLE END.DWG 06/22/06 15:27

TYPICAL GABLE END DETAILS	PAUL STRESING ASSOCIATES, INC.		DRAWN BY: C.A.D.	SHEET NO.
	14817 MAIN STREET ALACHUA, FLORIDA 32816 E-MAIL: paul@psa.com TELEPHONE: 904-462-8407 FAX: 904-462-8409 REGISTRATION NO. AR0002856 CA NO. AA0002377		DATE: 2006	
			PROJECT FILE WINDLOADS	OF



Total each truss uplift on the header divide by 2 for header anchorage

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700
Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.				

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A
Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors. Strap rafters to truss or at each end with minimum uplift resistance of 450# each end. Strap ridge beam at each end with minimum uplift resistance of 1000#. It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.		

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300
*or per truss engineering Use proper bolt anchors All beams to be sheathed or strapped to Double Top Plate when applicable.				

CRIPPLES | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 6" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE: For nailing into SPF members, multiply table values by .86

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	33.7	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	19.67	ft
Mean Roof Height (Ht)	14.33	ft
Width Perp. to Wind (B)	61.00	ft
Width Parallel to Wind (L)	58.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.25
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
<i>Hurricane Prone Region (V>100 mph)</i>	
Table C6-4 Values	
Alpha =	7.000
zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
l =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$l*(zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8914	
Gust2	$0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))$	0.8609	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V*(5280/3600)$	161.33	ft/s
Vzm	$bm*(zm/33)^{Am}*Vhref$	70.89	ft/s
NF1	$NatFreq*Lzm/Vzm$	4.37	Hz
Rn	$(7.47*NF1)/(1+10.302*NF1)^{1.667}$	0.0552	
Nh	$4.6*NatFreq*Ht/Vzm$	0.93	
Nb	$4.6*NatFreq*B/Vzm$	3.96	
Nd	$15.4*NatFreq*Depth/Vzm$	12.60	
Rh	$1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))$	0.5872	
Rb	$1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))$	0.2207	
Rd	$1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))$	0.0762	
RR	$((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^{0.5}$	0.6361	
gg	$(2*LN(3600*n1))^{0.5}+0.577/(2*LN(3600*n1))^{0.5}$	4.19	
Gust3	$0.925*((1+1.7*lzm*(3.4^2*Q^2+GG^2*RR^2)^{0.5})/(1+1.7*3.4*lzm))$	1.04	

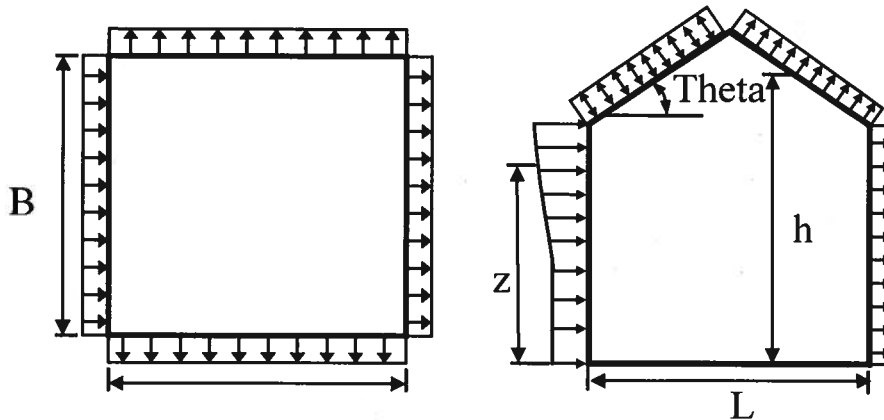
Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft ²	Pressure (lb/ft ²)	
					Windward Wall*	
					+GCpi	-GCpi
19.67	0.70	1.00	1.00	21.70	11.74	18.15
15	0.70	1.00	1.00	21.70	11.74	18.15

Figure 6-3 - External Pressure Coefficients, Cp
Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (15/zg)^{2/\text{Alpha}}$	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 61 ft wall)	-0.50	-10.87	-4.46
Leeward Walls (Wind Dir Parallel to 58 ft wall)	-0.49	-10.71	-4.30
Side Walls	-0.70	-13.93	-7.52
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.05	-4.00	2.41
Windward - Max Positive	0.37	2.53	8.94
Leeward Normal to Ridge	-0.60	-12.40	-5.99
Overhang Top	-0.05	-0.80	-0.80
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.165 ft	-0.90	-17.00	-10.59
Dist from Windward Edge: 7.165 ft to 14.33 ft	-0.90	-17.00	-10.59
Dist from Windward Edge: 14.33 ft to 28.66 ft	-0.50	-10.87	-4.46

ASCE 7-02

10/12/06

Wind Load Design per ASCE 7-02

Dist from Windward Edge: > 28.66 ft	-0.30	-7.80	-1.39
-------------------------------------	-------	-------	-------

* Horizontal distance from windward edge

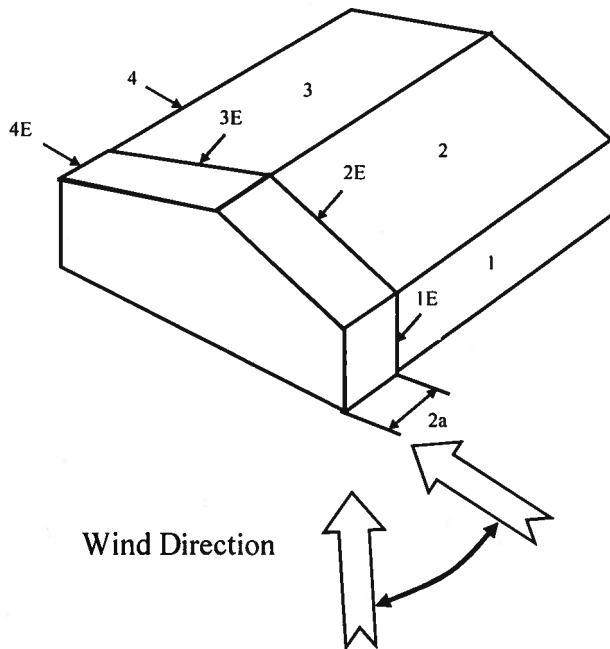
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-02

10/12/06

Wind Load Design per ASCE 7-02

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

$K_h = 2.01 \cdot (15/z_g)^{(2/\alpha)} = 0.57$
 $K_{ht} = \text{Topographic factor (Fig 6-2)} = 1.00$
 $Q_h = 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d = 17.80$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

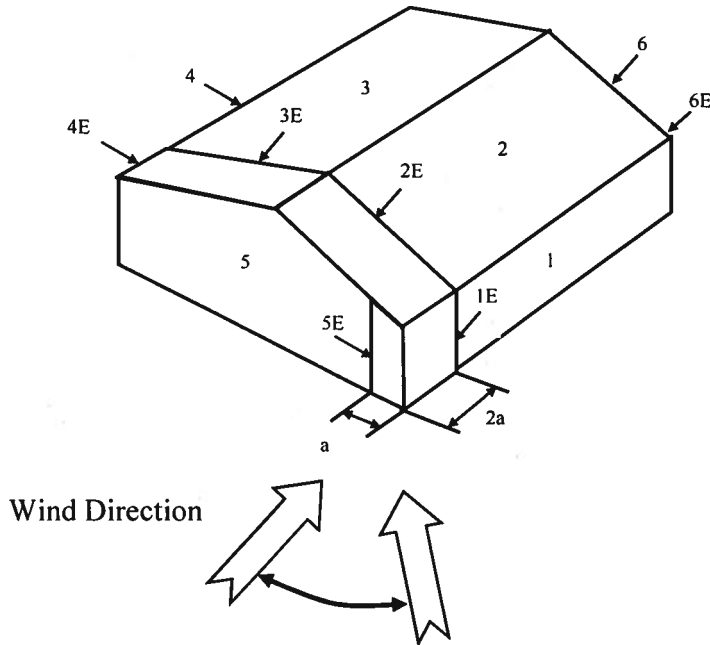


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

ASCE 7-02

10/12/06

Wind Load Design per ASCE 7-02

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

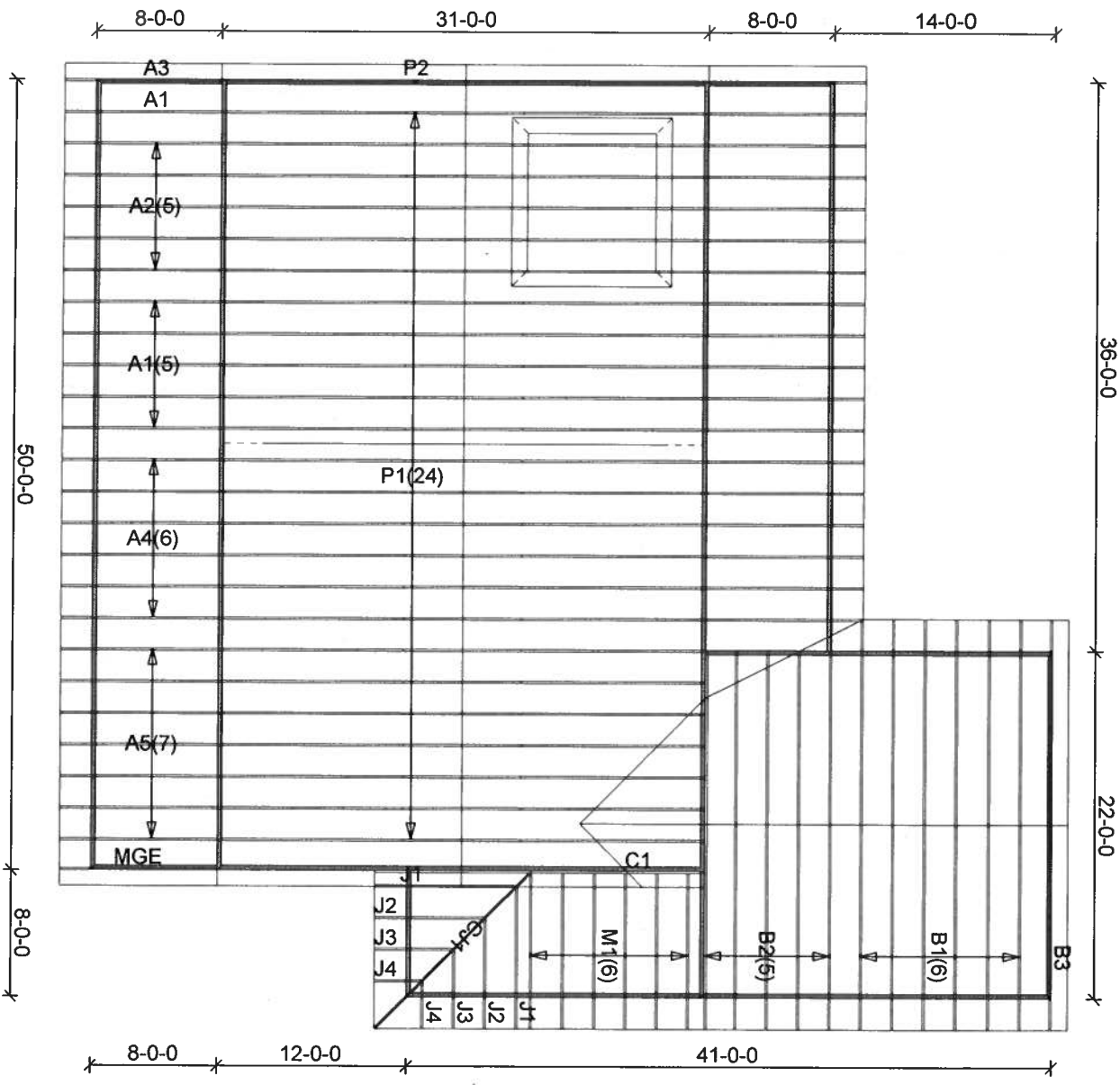
Table 6-8 External Pressure Coefficients for Arched Roofs, C_p

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	C_p		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	C_p	0.13	-1	-0.5
	P (+GCpi) - psf	-1.29	-18.53	-10.87
	P (-GCpi) -psf	5.12	-12.12	-4.46
Roof Springing from Ground	C_p	0.42	-1	-0.5
	P (+GCpi) - psf	3.23	-18.53	-10.87
	P (-GCpi) -psf	3.23	-18.53	-10.87

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, C_f

Variable	Description	Value	
L	Roof dimension normal to wind direction	58.00	ft
B	Roof dimension parallel to wind direction	61.00	ft
L/B	Ratio of L to B	0.951	
Theta	Slope of Roof	33.7	Deg
C_f	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft



LEON JONES CONSTRUCTION

BUSSEY RESIDENCE

110 MPH ASCE WIND LOAD

Mayo T russ Co. Inc.

845 East US 27
 MAYO, FL 32066
 (386)294-3988
 (877)-558-6262

Roof Loading
 TC Live: 20.00 psf
 TC Dead: 10.00 psf
 BC Live: 0.00 psf
 BC Dead: 10.00 psf
 TC Stress Inc: 25.00
 BC Stress Inc: 25.00
 Spacing: 2'-0" o.c.

Account: CONTRACTORS
 Job: JONES-BUSSEY
 Designer: M.MURRAY
 Checker: M.MURRAY
 Date: 10-31-06

Permit Number: _____ Lot Number: _____

Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review

Standard Loading:

T.C. Live	20 psf
T.C. Dead	10 psf
B.C. Live	0 psf
B.C. Dead	10 psf
Total	40 psf

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: JONES-BUSSEY - BUSSEY

ROBBINS ENGINEERING, INC.	P.O. Box 280055 Tampa, FL 33682-0055 Phone: (813) 972-1135	Engineering Index Sheet Index Page 1 of 1
Job Number T06103079	Date 10/29/2006	FBC - 2004 Chapter 16 and 23 Specification Quantity 18
<p>A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)</p>		

ANSI/ASCE 7-02
Wind Speed - 120 MPH
Mean Roof Ht. - 15 FT
Exposure Category - B
Occupancy Factor - 1.00
C and C
Enclosed

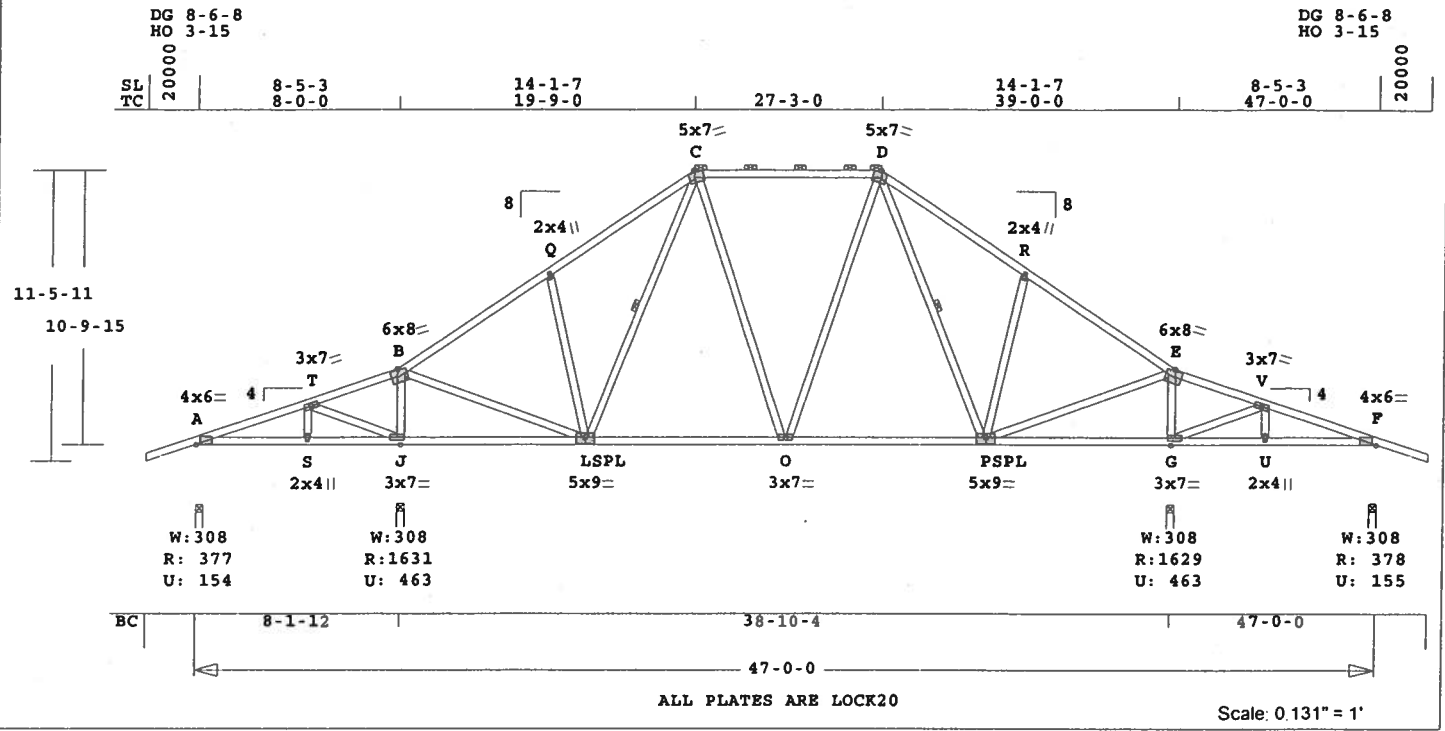
Notes: Refer to individual truss design drawings for special loading conditions.

Date Mark			Date Mark			Date Mark			Date Mark		
1	10/29/06	A1	2	10/29/06	A2	3	10/29/06	A3	4	10/29/06	A4
5	10/29/06	A5	6	10/29/06	B1	7	10/29/06	B2	8	10/29/06	B3
9	10/29/06	C1	10	10/29/06	CJ1	11	10/29/06	J1	12	10/29/06	J2
13	10/29/06	J3	14	10/29/06	J4	15	10/29/06	M1	16	10/29/06	MGE
17	10/29/06	P1	18	10/29/06	P2						

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 361.4 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI -Size-	----Lumber----
TC 0.49	2x 4 SP-#2
BC 0.41	2x 4 SP-#2
WB 0.37	2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 19- 9- 0
TC 2- 0- 0 19- 9- 0 27- 3- 0
TC Cont. 27- 3- 0 47- 0- 0
BC Cont. 0- 0- 0 47- 0- 0
WB 1 rows CLB on L -C
WB 1 rows CLB on D -P
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	377	155	3- 8	1- 8
			Hz = -275	
J	1631	464	3- 8	1-12
G	1629	464	3- 8	1-12
F	378	155	3- 8	1- 8
			Hz = 276	

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -T	0.20		423	T	0.00 0.20
T -B	0.23		248	C	0.03 0.20
B -Q	0.33		1067	C	0.06 0.27
Q -C	0.35		981	C	0.08 0.27
C -D	0.49		820	C	0.01 0.48
D -R	0.35		982	C	0.08 0.27
R -E	0.33		1068	C	0.06 0.27
E -V	0.23		217	T	0.03 0.20
V -F	0.20		345	T	0.00 0.20
-----Bottom Chords-----					
A -S	0.13		369	C	0.04 0.09

	S	-J	0.25	369	C	0.02	0.23
J	-L	0.31	344	T	0.00	0.31	
L	-O	0.41	791	T	0.08	0.33	
O	-P	0.41	791	T	0.08	0.33	
P	-G	0.31	320	T	0.00	0.31	
G	-U	0.25	282	T	0.02	0.23	
U	-F	0.13	282	T	0.04	0.09	

-----Webs-----							
S	-T	0.01	241	C			
T	-J	0.11	795	T			
J	-B	0.16	1300	C			
B	-L	0.37	1134	T			
Q	-L	0.23	398	C			
L	-C	0.04	252	T	1 Br		
C	-O	0.10	168	T			
O	-D	0.10	168	T			
D	-P	0.04	253	T	1 Br		
P	-R	0.23	398	C			
R	-E	0.34	1131	T			
G	-E	0.16	1298	C			
G	-V	0.11	651	T			
U	-V	0.01	149	C			

TL Defl -0.13" in O -P L/999
LL Defl -0.06" in O -P L/999
Shear // Grain in C -D 0.27

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	LOCK	20 Ga,	Gross Area	
Plate	RHS	20 Ga,	Gross Area	
Jt Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	0.1 0.90
T	LOCK	3.0x 7.0	Ctr	0.43
B	LOCK	6.0x 8.0	0.9 0.3	0.86
Q	LOCK	2.0x 4.0	Ctr	0.52
C	LOCK	5.0x 7.0	1.0-3.4	0.74
D	LOCK	5.0x 7.0	1.0-3.4	0.74
R	LOCK	2.0x 4.0	Ctr	0.52
E	LOCK	6.0x 8.0	0.9 0.3	0.86
V	LOCK	3.0x 7.0	Ctr	0.43
F	LOCK	4.0x 6.0	Ctr	0.1 0.90
S	LOCK	2.0x 4.0	Ctr	0.52
J	LOCK	3.0x 7.0	Ctr	0.53
L	LOCK	5.0x 9.0	Ctr	0.5 0.86
O	LOCK	3.0x 7.0	Ctr	0.42
P	LOCK	5.0x 9.0	Ctr	0.5 0.86
G	LOCK	3.0x 7.0	Ctr	0.53
U	LOCK	2.0x 4.0	Ctr	0.52

REVIEWED BY:
Robbins Engineering, Inc.

PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- ---To---
0- 0- 0 8- 1-12
38-10- 4 47- 0- 0
Max comp. force 1300 lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

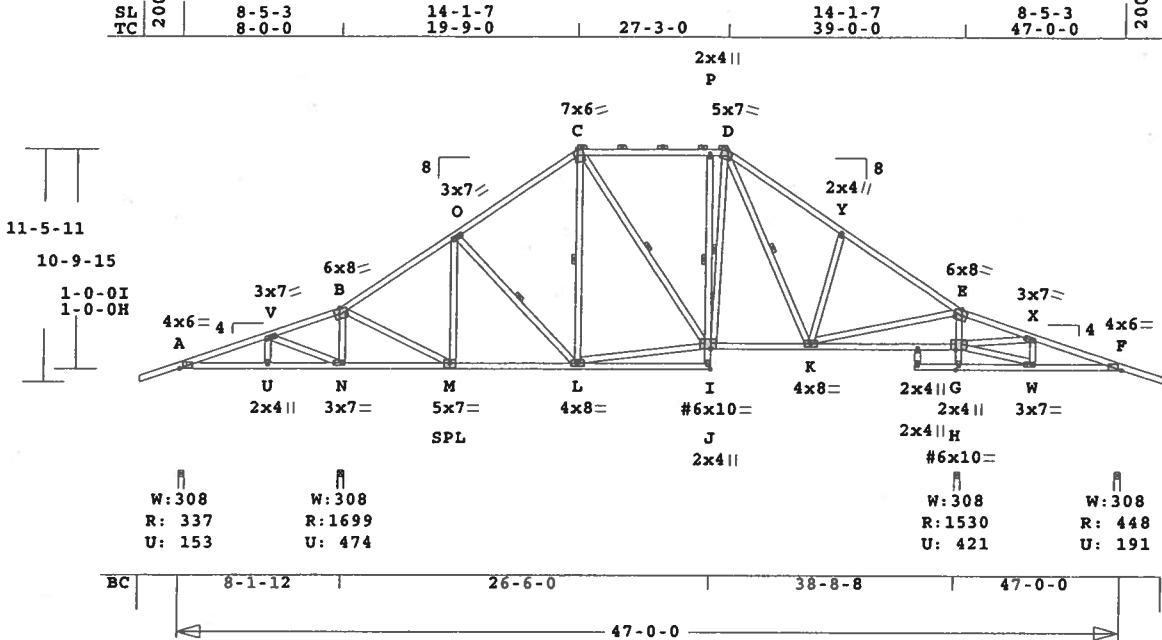


Job JONES-BUSSEY	Mark A2	Quan 5	Type SP	Span 470000	Pl-H1 4	Left OH 2- 0- 0	Right OH 2- 0- 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY

DG 8-6-8
HO 3-15

DG 8-6-8
HO 3-15



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.105" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 408.2 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	Size	Lumber
TC	0.38 2x 4	SP-#2
BC	0.34 2x 4	SP-#2
CW	0.20 2x 4	SP-#2
WB	0.28 2x 4	SP-#2

Brace truss as follows:	O.C.	From	To
TC Cont.	0- 0- 0	19- 9- 0	
TC Cont.	2- 0- 0	19- 9- 0	27- 3- 0
TC Cont.	27- 3- 0	47- 0- 0	
BC Cont.	0- 0- 0	47- 0- 0	

WB 1 rows CLB on O -L
WB 1 rows CLB on L -C
WB 1 rows CLB on C -I
WB 1 rows CLB on I -D
WB 1 rows CLB on D -K
WB 1 rows CLB on I -P
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	338	154	3- 8	1- 8
			Hz =	-276
N	1699	475	3- 8	1-13
G	1530	421	3- 8	1-10
F	449	191	3- 8	1- 8
			Hz =	277

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -V	0.21	395	T	0.00	0.21
V -B	0.26	355	T	0.05	0.21
B -O	0.34	918	C	0.05	0.29
O -C	0.36	971	C	0.07	0.29
C -P	0.33	924	C	0.01	0.32
P -D	0.31	926	C	0.00	0.31
D -Y	0.38	1191	C	0.09	0.29
Y -E	0.36	1287	C	0.07	0.29

E -X	0.19	176	T	0.00	0.19
X -F	0.21	542	T	0.07	0.14
-----Bottom Chords-----					
A -U	0.13	344	C	0.00	0.13
U -N	0.15	344	C	0.01	0.14
N -M	0.16	367	T	0.00	0.16
M -L	0.34	767	T	0.07	0.27
L -J	0.27	15	C	0.00	0.27
I -K	0.32	892	T	0.09	0.23
K -H	0.31	203	T	0.00	0.31
G -W	0.09	120	C	0.00	0.09
W -F	0.15	484	T	0.08	0.07
-----Chord Webs-----					
J -I	0.03	102	T	0.01	0.02
I -P	0.03	328	C	0.01	0.02
H -E	0.14	1204	C	0.00	0.14
G -H	0.20	1500	C	0.00	0.20
-----Webs-----					
U -V	0.02	233	C		
V -N	0.11	793	T		
N -B	0.17	1384	C		
B -M	0.23	1193	T		
M -O	0.21	414	C		
O -L	0.04	169	C	1 Br	
L -C	0.02	154	T	1 Br	
L -I	0.14	805	T		
C -I	0.03	213	T	1 Br	
I -D	0.06	365	T	1 Br	
D -K	0.07	325	T	1 Br	
K -Y	0.17	394	C		
K -E	0.28	1039	T		
H -X	0.09	681	T		
H -W	0.10	578	T		
W -X	0.00	117	C		

TL Defl -0.15" in L -J L/999
LL Defl -0.07" in L -J L/999
Shear // Grain in C -P 0.26

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	0.1	0.90
V	LOCK	3.0x 7.0	Ctr	Ctr	0.43
B	LOCK	6.0x 8.0	0.9	0.3	0.86
O	LOCK	3.0x 7.0	Ctr	Ctr	0.47
C	LOCK	7.0x 6.0	1.3-4.2	0.84	
P	LOCK	2.0x 4.0	Ctr	Ctr	0.40
D	LOCK	5.0x 7.0-1.0-3.4	0.91		
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.52
E	LOCK	6.0x 8.0-0.9	0.3	0.86	
X	LOCK	3.0x 7.0	Ctr	Ctr	0.47
F	LOCK	4.0x 6.0	Ctr	0.1	0.90

U	LOCK	2.0x 4.0	Ctr	Ctr	0.52
N	LOCK	3.0x 7.0	Ctr	Ctr	0.53
M	LOCK	5.0x 7.0	Ctr	0.5	0.86
L	LOCK	4.0x 8.0	Ctr	Ctr	0.69
J	LOCK	2.0x 4.0	Ctr	Ctr	0.65
I#	LOCK	6.0x10.0	0.1	1.4	0.77
K	LOCK	4.0x 8.0	Ctr	Ctr	0.74
H#	LOCK	6.0x10.0	0.2	1.3	0.75
G	LOCK	2.0x 4.0	Ctr	Ctr	0.85
W	LOCK	3.0x 7.0	Ctr	Ctr	0.94

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

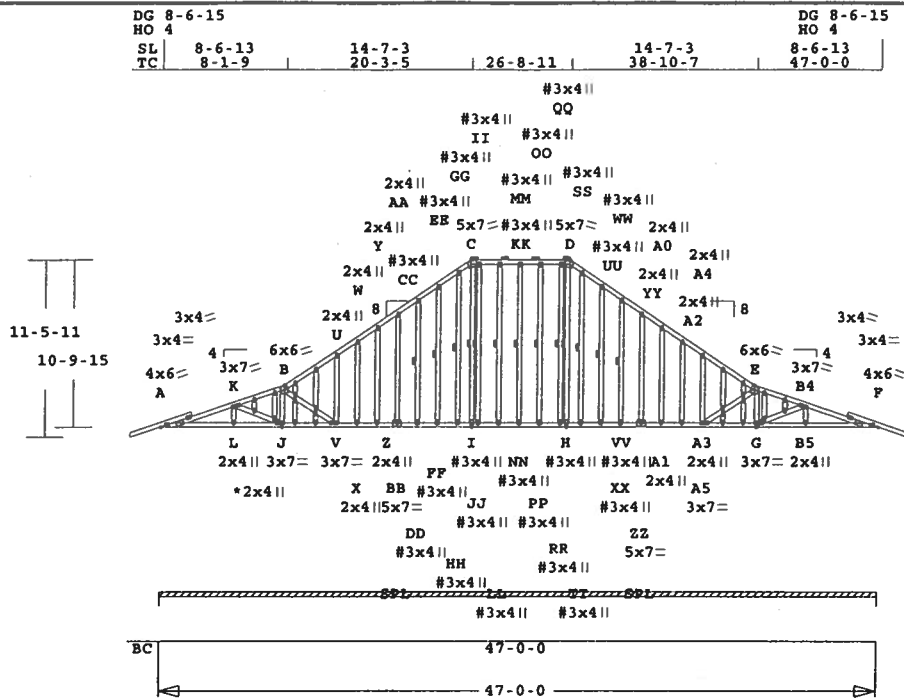


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
JONES-BUSSEY	A2	5	SP	470000	4	2- 0- 0	2- 0- 0	T06103079
U# J#JONES-BUSSEY BUSSEY								

monitor.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 120 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 5.0 psf
 BC Dead Load: 5.0 psf
 User-defined wind-exposed BC
 regions --From-- ---To---
 0- 0- 0 8- 1-12
 38- 8- 8 47- 0- 0
 Max comp. force 1500 Lbs
 Quality Control Factor 1.25

Job JONES-BUSSEY	Mark A3	Quan 1	Type HAWI	Span 470000	P1-H1 4	Left OH 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See * For Typical Gable Plate Size and Placement

Scale: 0.080" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 595.4 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI -Size- ---Lumber---
TC 0.13 2x 4 SP-#2
BC 0.09 2x 4 SP-#2
WB 0.05 2x 4 SP-#2
GW 0.06 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 20- 3- 5
TC 2- 0- 0 20- 3- 5 26- 8- 11
TC Cont. 26- 8- 11 47- 0- 0
BC Cont. 0- 0- 0 47- 0- 0
WB 1 rows CLB on I -II
WB 1 rows CLB on H -QQ
WB 1 rows CLB on DD-CC
WB 1 rows CLB on FF-EE
WB 1 rows CLB on HH-GG
WB 1 rows CLB on JJ-II
WB 1 rows CLB on LL-KK
WB 1 rows CLB on NN-MM
WB 1 rows CLB on PP-OO
WB 1 rows CLB on RR-QQ
WB 1 rows CLB on TT-SS
WB 1 rows CLB on VV-UU
WB 1 rows CLB on XX-WW
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Jt React Uplft Size Req'd
Lbs Lbs In-Sx In-Sx
Cont. Brg 0- 0- 0 to 47- 0- 0
3760 775 Hz = 280

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords-----
A -K 0.13 270 C 0.00 0.13
K -B 0.13 117 T 0.00 0.13
B -U 0.10 172 C 0.02 0.08
U -W 0.09 92 C 0.00 0.09
W -Y 0.01 82 C 0.00 0.01

Y -AA 0.01	69 C	0.00	0.01
AA-CC 0.02	96 T	0.01	0.01
CC-EE 0.03	152 T	0.02	0.01
EE-GG 0.03	212 T	0.02	0.01
GG-C 0.04	236 T	0.03	0.01
C -II 0.02	224 T	0.02	0.00
II-KK 0.03	224 T	0.02	0.01
KK-MM 0.03	224 T	0.02	0.01
MM-OO 0.03	224 T	0.02	0.01
OO-QQ 0.03	224 T	0.02	0.01
QQ-D 0.02	224 T	0.02	0.00
D -SS 0.04	236 T	0.03	0.01
SS-UU 0.03	212 T	0.02	0.01
UU-WW 0.03	152 T	0.02	0.01
WW-YY 0.02	96 T	0.01	0.01
YY-AO 0.01	69 C	0.00	0.01
AO-A2 0.01	82 C	0.00	0.01
A2-A4 0.09	92 C	0.00	0.09
A4-E 0.10	172 C	0.02	0.08
E -B4 0.13	117 T	0.00	0.13
B4-F 0.13	270 C	0.00	0.13

-----Bottom Chords-----
A -L 0.09 8 T 0.00 0.09
L -J 0.07 0 T 0.00 0.07
J -V 0.05 0 T 0.00 0.05
V -X 0.05 0 T 0.00 0.05
X -Z 0.00 0 T 0.00 0.00
Z -BB 0.00 0 T 0.00 0.00
BB-DD 0.00 0 T 0.00 0.00
DD-FF 0.00 0 T 0.00 0.00
FF-HH 0.00 0 T 0.00 0.00
HH-I 0.00 0 T 0.00 0.00
I -JJ 0.00 0 T 0.00 0.00
JJ-LL 0.00 0 T 0.00 0.00
LL-NN 0.00 0 T 0.00 0.00
NN-PP 0.00 0 T 0.00 0.00
PP-RR 0.00 0 T 0.00 0.00
RR-H 0.00 0 T 0.00 0.00
H -TT 0.00 0 T 0.00 0.00
TT-VV 0.00 0 T 0.00 0.00
VV-XX 0.00 0 T 0.00 0.00
XX-ZZ 0.00 0 T 0.00 0.00
ZZ-A1 0.00 0 T 0.00 0.00
A1-A3 0.00 0 T 0.00 0.00
A3-A5 0.05 0 T 0.00 0.05
A5-G 0.05 0 T 0.00 0.05
G -B5 0.07 0 T 0.00 0.07
B5-F 0.09 8 T 0.00 0.09

-----Webs-----
K -J 0.05 378 T
J -B 0.01 147 C
B -V 0.02 158 T
I -C 0.01 48 C 1 Br
H -D 0.01 48 C 1 Br
A5-E 0.02 158 T
G -E 0.01 147 C
G -B4 0.05 378 T
-----Gable Webs-----
L -K 0.00 98 C

V -U 0.06	222 C
X -W 0.02	53 C
Z -Y 0.05	97 C
BB-AA 0.06	96 C
DD-CC 0.02	97 C
FF-EE 0.02	103 C
HH-GG 0.02	62 C
JJ-II 0.01	47 C
LL-KK 0.03	81 C
NN-MM 0.03	82 C
PP-OO 0.03	81 C
RR-QQ 0.01	47 C
TT-SS 0.02	62 C
VV-UU 0.02	103 C
XX-WW 0.02	97 C
ZZ-YY 0.06	96 C
A1-A0 0.05	97 C
A3-A2 0.02	53 C
A5-A4 0.06	222 C
B5-B4 0.00	98 C

TL Defl -0.01" in B5-F L/999
LL Defl 0.00" in B5-F L/999
Shear // Grain in A-K 0.15

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
JONES-BUSSEY	A3	1	HAWI	470000	4	0	0	T06103079
U# J#JONES-BUSSEY BUSSEY								

A LOCK 4.0x 6.0 Ctr Ctr 0.95
 K LOCK 3.0x 7.0 Ctr Ctr 0.43
 B LOCK 6.0x 6.0 Ctr Ctr 0.86
 U LOCK 2.0x 4.0 Ctr Ctr 0.52
 W LOCK 2.0x 4.0 Ctr Ctr 0.52
 Y LOCK 2.0x 4.0 Ctr Ctr 0.52
 AA LOCK 2.0x 4.0 Ctr Ctr 0.52
 CC#LOCK 3.0x 4.0 Ctr Ctr 0.35
 EE#LOCK 3.0x 4.0 Ctr Ctr 0.35
 GG#LOCK 3.0x 4.0 Ctr Ctr 0.35
 C LOCK 5.0x 7.0 1.0-3.4 0.74
 II#LOCK 3.0x 4.0 Ctr Ctr 0.35
 KK#LOCK 3.0x 4.0 Ctr Ctr 0.35
 MM#LOCK 3.0x 4.0 Ctr Ctr 0.35
 OO#LOCK 3.0x 4.0 Ctr Ctr 0.35
 QQ#LOCK 3.0x 4.0 Ctr Ctr 0.35
 D LOCK 5.0x 7.0-1.0-3.4 0.74
 SS#LOCK 3.0x 4.0 Ctr Ctr 0.35
 UU#LOCK 3.0x 4.0 Ctr Ctr 0.35
 WW#LOCK 3.0x 4.0 Ctr Ctr 0.35
 YY LOCK 2.0x 4.0 Ctr Ctr 0.52
 A0 LOCK 2.0x 4.0 Ctr Ctr 0.52
 A2 LOCK 2.0x 4.0 Ctr Ctr 0.52
 A4 LOCK 2.0x 4.0 Ctr Ctr 0.52
 E LOCK 6.0x 6.0 Ctr Ctr 0.86
 B4 LOCK 3.0x 7.0 Ctr Ctr 0.43
 F LOCK 4.0x 6.0 Ctr-0.3 0.95
 L LOCK 2.0x 4.0 Ctr Ctr 0.52
 J LOCK 3.0x 7.0 Ctr Ctr 0.52
 V LOCK 3.0x 7.0 Ctr Ctr 0.46
 X LOCK 2.0x 4.0 Ctr Ctr 0.52
 Z LOCK 2.0x 4.0 Ctr Ctr 0.52
 BB LOCK 5.0x 7.0 Ctr-0.5 0.86
 DD#LOCK 3.0x 4.0 Ctr Ctr 0.35
 FF#LOCK 3.0x 4.0 Ctr Ctr 0.35
 HH#LOCK 3.0x 4.0 Ctr Ctr 0.35
 I# LOCK 3.0x 4.0 Ctr Ctr 0.35
 JJ#LOCK 3.0x 4.0 Ctr Ctr 0.35
 LL#LOCK 3.0x 4.0 Ctr Ctr 0.35
 NN#LOCK 3.0x 4.0 Ctr Ctr 0.35
 PP#LOCK 3.0x 4.0 Ctr Ctr 0.35
 RR#LOCK 3.0x 4.0 Ctr Ctr 0.35
 H# LOCK 3.0x 4.0 Ctr Ctr 0.35
 TT#LOCK 3.0x 4.0 Ctr Ctr 0.35
 VV#LOCK 3.0x 4.0 Ctr Ctr 0.35
 XX#LOCK 3.0x 4.0 Ctr Ctr 0.35
 ZZ LOCK 5.0x 7.0 Ctr-0.5 0.86
 A1 LOCK 2.0x 4.0 Ctr Ctr 0.52
 A3 LOCK 2.0x 4.0 Ctr Ctr 0.52
 A5 LOCK 3.0x 7.0 Ctr Ctr 0.46
 G LOCK 3.0x 7.0 Ctr Ctr 0.52
 B5 LOCK 2.0x 4.0 Ctr Ctr 0.52

Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 5.0 psf
 BC Dead Load: 5.0 psf
 Max comp. force 270 Lbs
 Quality Control Factor 1.25

= Plate Monitor used
 12 Gable studs to be attached
 with 2.0x4.0 plates each end.

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004

WARNING Do Not Cut overframe
 member between outside of
 truss and first tie-plate
 to inside of heel plate.
 Design checked for 10 psf non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.

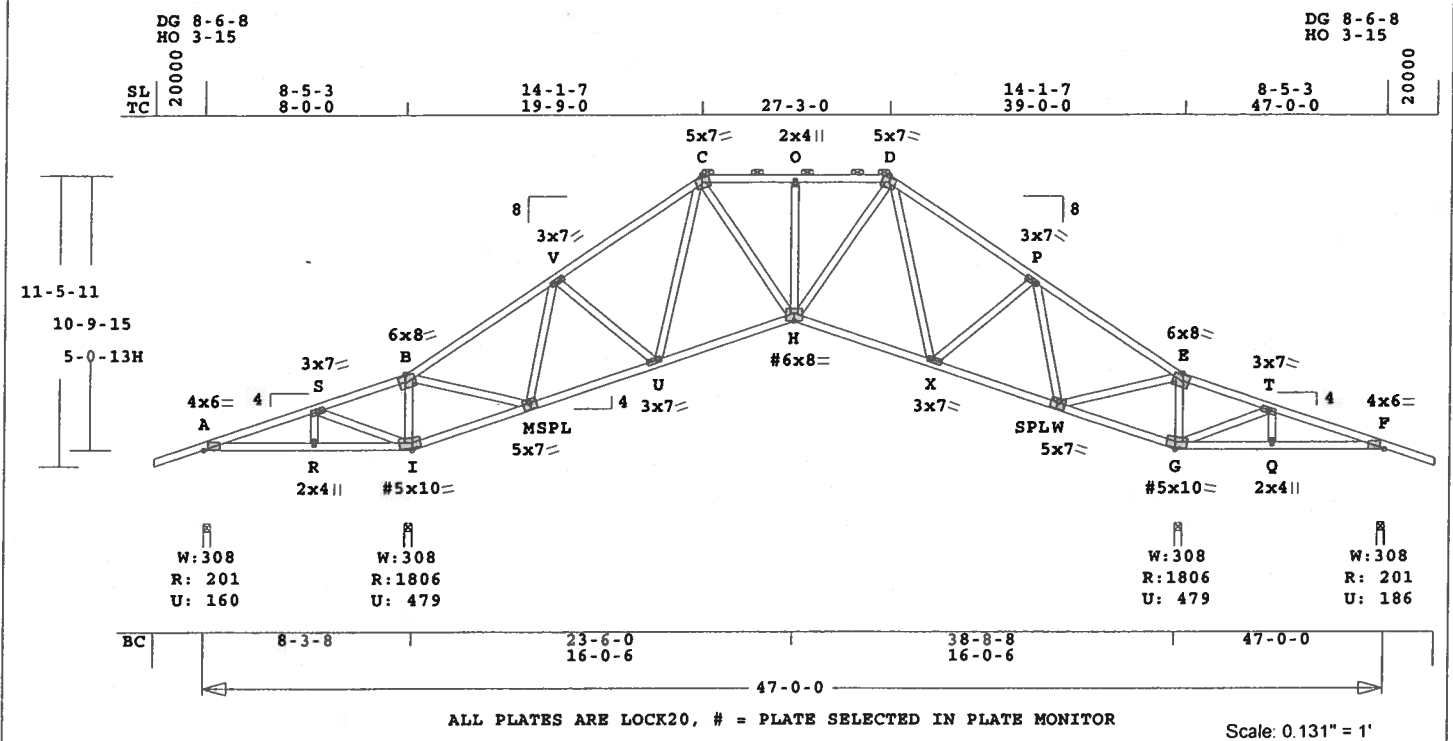
Refer to Gen Det 3 series for
 web bracing and plating.

NOTE: USER MODIFIED PLATES
 This design may have plates
 selected through a plate
 monitor.

Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 120 mph
 Mean Roof Height: 15-0

Job JONES-BUSSEY	Mark A4	Quan 6	Type SP	Span 470000	Pl-H1 4	Left OH 2-0-0	Right OH 2-0-0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 344.9 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI -Size-	----Lumber----
TC	0.42 2x 4 SP-#2
BC	0.47 2x 4 SP-#2
WB	0.28 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 9- 0
TC	2- 0- 0	19- 9- 0 27- 3- 0
TC Cont.	27- 3- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	201	160	3- 8	1- 8
			Hz =	-277
I	1807	479	3- 8	1-15
G	1807	479	3- 8	1-15
F	201	186	3- 8	1- 8
			Hz =	278

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A	-S	0.30	364	T	0.03 0.27
S	-B	0.42	934	T	0.15 0.27
B	-V	0.34	663	C	0.00 0.34
V	-C	0.37	1122	C	0.06 0.31
C	-O	0.29	1268	C	0.01 0.28
O	-D	0.29	1268	C	0.01 0.28
D	-P	0.37	1122	C	0.06 0.31
P	-E	0.34	663	C	0.00 0.34
E	-T	0.42	934	T	0.15 0.27
T	-F	0.30	341	T	0.03 0.27
-----Bottom Chords-----					
A	-R	0.23	335	C	0.00 0.23
R	-I	0.44	335	C	0.00 0.44
I	-M	0.47	864	C	0.00 0.47
M	-U	0.25	744	T	0.12 0.13
U	-H	0.29	995	T	0.16 0.13
H	-X	0.29	995	T	0.16 0.13

X	-W	0.25	744	T	0.12 0.13
W	-G	0.47	864	C	0.00 0.47
G	-Q	0.44	335	C	0.00 0.44
Q	-F	0.23	334	C	0.00 0.23
-----Webs-----					
R	-S	0.04	289	C	
S	-I	0.17	904	T	
I	-B	0.15	1205	C	
B	-M	0.25	1390	T	
M	-V	0.28	808	C	
V	-U	0.05	281	T	
U	-C	0.08	125	C	
C	-H	0.09	531	T	
H	-O	0.04	132	T	
H	-D	0.09	531	T	
D	-X	0.08	125	C	
X	-P	0.05	281	T	
P	-W	0.28	808	C	
W	-E	0.25	1390	T	
G	-E	0.15	1205	C	
G	-T	0.17	830	C	
Q	-T	0.04	259	T	

TL Defl	-0.06"	in G	-Q	L/999
LL Defl	-0.03"	in G	-Q	L/999
Hz Disp	LL	DL	TL	
Jt G	0.07"	0.07"	0.14"	
Shear // Grain	in S	-B	0.21	

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 Ctr 0.1 0.90
S LOCK 3.0x 7.0 Ctr Ctr 0.43
B LOCK 6.0x 8.0 0.9 0.3 0.86
V LOCK 3.0x 7.0 Ctr Ctr 0.55
C LOCK 5.0x 7.0 0.6-3.7 0.97
O LOCK 2.0x 4.0 Ctr Ctr 0.52
D LOCK 5.0x 7.0-0.6-3.7 0.97
P LOCK 3.0x 7.0 Ctr Ctr 0.55
E LOCK 6.0x 8.0-0.9 0.3 0.86
T LOCK 3.0x 7.0 Ctr Ctr 0.43
F LOCK 4.0x 6.0 Ctr 0.1 0.90
R LOCK 2.0x 4.0 Ctr Ctr 0.52
I# LOCK 5.0x10.0-1.2 3.5 0.84
M LOCK 5.0x 7.0 0.2-0.5 0.86
U LOCK 3.0x 7.0 Ctr Ctr 0.46
H# LOCK 6.0x 8.0 Ctr Ctr 0.6 0.75
X LOCK 3.0x 7.0 Ctr Ctr 0.46
W LOCK 5.0x 7.0-0.2-0.5 0.86
G# LOCK 5.0x10.0 0.9 3.4 0.79
Q LOCK 2.0x 4.0 Ctr Ctr 0.52

= Plate Monitor used
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.

NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate
monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
JONES-BUSSEY	A4	6	SP	470000	4	2- 0- 0	2- 0- 0	T06103079
U# J#JONES-BUSSEY BUSSEY								

User-defined wind-exposed BC
regions --From-- ---To---
0- 0- 0 8- 3- 8
38- 8- 8 47- 0- 0
Max comp. force 1268 Lbs
Quality Control Factor 1.25

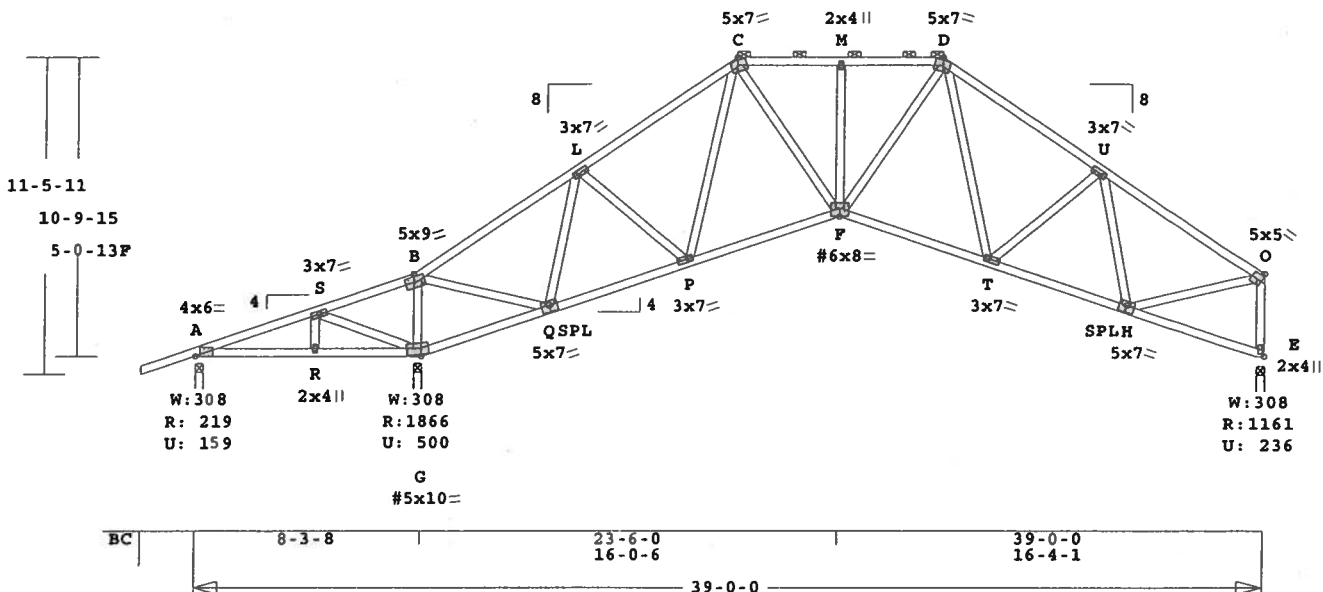
Job JONES-BUSSEY	Mark A5	Quan 7	Type SP	Span 390000	P1-H1 4	Left OH 2- 0- 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY

DG 8-6-8
HO 3-15

DG 15-11-12
HO 2-11-15

SL TC	20000	8-5-3 8-0-0	14-1-7 19-9-0	27-3-0	14-1-7 39-0-0
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ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.142" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 300.5 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	-Size-	---	Lumber	----
TC	0.43	2x 4	SP-#2	
BC	0.50	2x 4	SP-#2	
WB	0.30	2x 4	SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 9- 0
TC	2- 0- 0	19- 9- 0 27- 3- 0
TC Cont.	27- 3- 0	39- 0- 0
BC Cont.	0- 0- 0	39- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	220	160	3- 8	1- 8
			Hz =	-250
G	1867	500	3- 8	2- 0
E	1161	236	3- 8	1- 8
			Hz =	332

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A	-S	0.29	299	T	0.02 0.27
S	-B	0.41	885	T	0.14 0.27
B	-L	0.35	803	C	0.04 0.31
L	-C	0.38	1302	C	0.07 0.31
C	-M	0.33	1516	C	0.01 0.32
M	-D	0.30	1516	C	0.01 0.29
D	-U	0.43	1474	C	0.08 0.35
U	-O	0.42	1262	C	0.00 0.42
-----Bottom Chords-----					
A	-R	0.23	317	C	0.00 0.23
R	-G	0.46	317	C	0.00 0.46
G	-Q	0.50	810	C	0.00 0.50
Q	-P	0.28	876	T	0.14 0.14
P	-F	0.33	1159	T	0.19 0.14
F	-T	0.29	1253	T	0.20 0.09
T	-H	0.29	1232	T	0.20 0.09
H	-E	0.14	305	T	0.00 0.14

-----Webs-----			
R	-S	0.04	262 T
S	-G	0.17	842 C
G	-B	0.16	1278 C
B	-Q	0.26	1455 T
Q	-L	0.30	852 C
L	-P	0.05	313 T
P	-C	0.10	153 C
C	-F	0.12	683 T
F	-M	0.04	123 T
M	-D	0.09	538 T
D	-T	0.09	235 T
T	-U	0.06	200 T
U	-H	0.19	557 C
H	-O	0.20	1101 T
E	-O	0.14	1118 C WindLd

TL Defl	-0.06"	in R	-G	L/999
LL Defl	-0.03"	in R	-G	L/999
Hz Disp	LL	DL	TL	
Jt E	0.08"	0.07"	0.15"	
Shear // Grain	in U	-O	0.23	

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	-RHS	20 Ga,	Gross Area
Jt Type	Plt Size	X	Y JSI
A	LOCK	4.0x 6.0	Ctr Ctr 0.1 0.82
S	LOCK	3.0x 7.0	Ctr Ctr 0.40
B	LOCK	5.0x 9.0	0.5 0.2 0.94
L	LOCK	3.0x 7.0	Ctr Ctr 0.52
C	LOCK	5.0x 7.0	0.6-3.7 0.87
M	LOCK	2.0x 4.0	Ctr Ctr 0.47
D	LOCK	5.0x 7.0	0.6-3.7 0.87
U	LOCK	3.0x 7.0	Ctr Ctr 0.52
O	LOCK	5.0x 5.0	Ctr Ctr 0.78
R	LOCK	2.0x 4.0	Ctr Ctr 0.47
G#	LOCK	5.0x10.0	-1.7 3.3 0.73
Q	LOCK	5.0x 7.0	0.2-0.5 0.77
P	LOCK	3.0x 7.0	Ctr Ctr 0.43
F#	LOCK	6.0x 8.0	Ctr-0.6 0.68
T	LOCK	3.0x 7.0	Ctr Ctr 0.43
H	LOCK	5.0x 7.0	0.2-0.5 0.77
E	LOCK	2.0x 4.0	Ctr Ctr 0.54

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Provide connection to bearing
for 331 Lbs Horiz Reaction
Design checked for 10 psf non-
concurrent LL on BC.

NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate
monitor.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- --To---
0- 0- 0 8- 3- 8
Max comp. force 1516 Lbs
Quality Control Factor 1.25

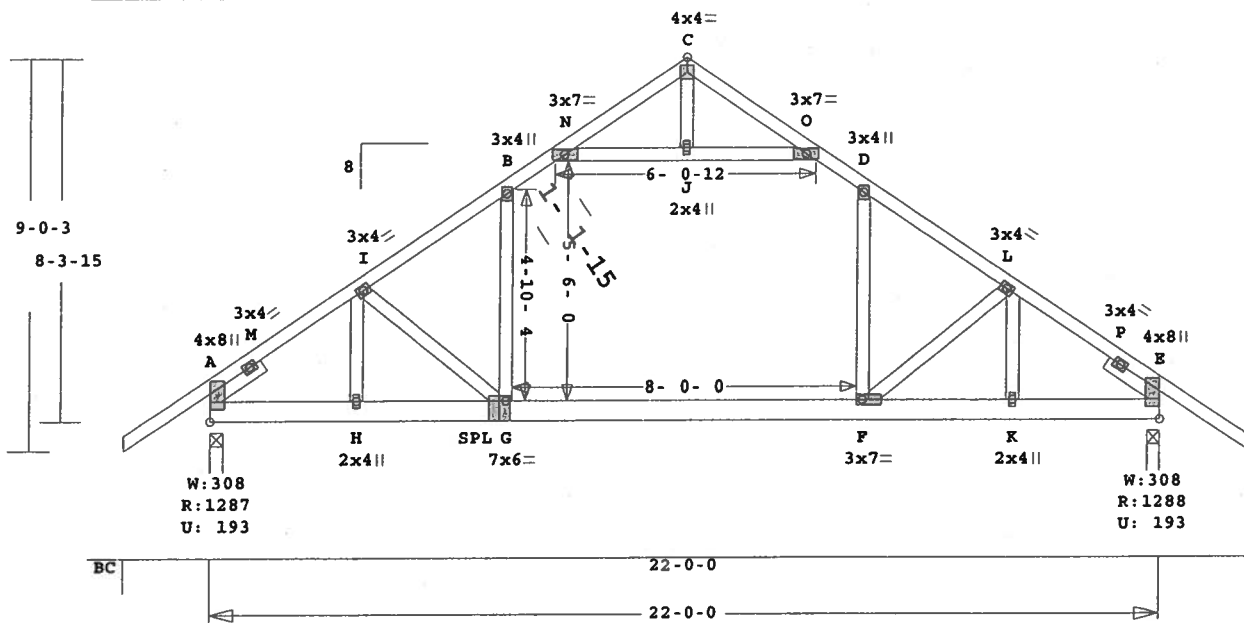
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark B1	Quan 6	Type TR	Span 220000	P1-H1 8	Left OH 2-0-0	Right OH 2-0-0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY

DG 13-9-9 HO 11-15	13-2-10 11-0-0	13-2-10 22-0-0	DG 13-9-9 HO 11-15
SL TC 2-0-0			2-0-0



ALL PLATES ARE LOCK20

Scale: 0.225" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 185.8 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

TC	CSI	-Size-	---	Lumber	---
TC	0.79	2x 4	SP-#2		
BC	0.84	2x 6	SP-#2		
WB	0.12	2x 4	SP-#2		
ACT	0.12	2x 4	SP-#2		
AWT	0.00	2x 4	SP-#2		
SL	0.06	2x 4	SP-#2		

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0
BC Cont.	0- 0- 0	22- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00		
Plate Duration Factor	1.00		
plf - Live	Dead	From	To
TC V	40	20	0.0' 22.0'
BC V	0	20	0.0' 22.0'
TC V	0	0	7.0' 8.0'
TC V	0	0	14.0' 15.0'
BC V	60	10	7.0' 15.0'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1288	193	3- 8	1- 8
			Hz =	-187
E	1288	193	3- 8	1- 8
			Hz =	188

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
A -M	0.30	922	C	0.00	0.30
M -I	0.31	1368	C	0.01	0.30
I -B	0.65	1463	C	0.02	0.63
B -N	0.79	1096	C	0.00	0.79
N -C	0.79	96	T	0.00	0.79

---Bottom Chords---					
A -H	0.71	1126	T	0.17	0.54
H -G	0.82	1126	T	0.12	0.70
G -F	0.84	1173	T	0.14	0.70
F -K	0.82	1126	T	0.12	0.70
K -E	0.71	1126	T	0.17	0.54
---Webs---					
H -I	0.05	360	C		
I -G	0.05	219	T		
G -B	0.12	641	T		
F -D	0.12	641	T		
F -L	0.05	219	T		
K -L	0.05	360	C		
---Attic Chords (Top)---					
N -J	0.12	1217	C	0.12	0.00
J -O	0.12	1217	C	0.12	0.00
---Attic Webs (Top)---					
J -C	0.00	7	T		
---Sliders---					
A -M	0.06	551	C		
P -E	0.06	551	C		

TL Defl	-0.30"	in G -F	L/857
LL Defl	-0.19"	in G -F	L/999
Shear // Grain		in B -N	0.81

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	- LOCK 20 Ga,	Gross Area
Plate	- RHS 20 Ga,	Gross Area
Jt Type	Plt Size	X Y JSI
A	LOCK 4.0x 8.0	2.0 0.7 0.91
M	LOCK 3.0x 4.0	Ctr Ctr 0.50
I	LOCK 3.0x 4.0	Ctr Ctr 0.64
B	LOCK 3.0x 4.0	Ctr Ctr 0.25
N	LOCK 3.0x 7.0	Ctr Ctr 0.26
C	LOCK 4.0x 4.0	Ctr Ctr 0.64
O	LOCK 3.0x 7.0	Ctr Ctr 0.26
D	LOCK 3.0x 4.0	Ctr Ctr 0.25
L	LOCK 3.0x 4.0	Ctr Ctr 0.64
P	LOCK 3.0x 4.0	Ctr Ctr 0.50
E	LOCK 4.0x 8.0	-2.0 0.7 0.91
H	LOCK 2.0x 4.0	Ctr Ctr 0.40
G	LOCK 7.0x 6.0	Ctr Ctr 0.8 0.40
F	LOCK 3.0x 7.0	Ctr Ctr 0.37
K	LOCK 2.0x 4.0	Ctr Ctr 0.40
J	LOCK 2.0x 4.0	Ctr Ctr 0.40

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 1463 Lbs
Quality Control Factor 1.25

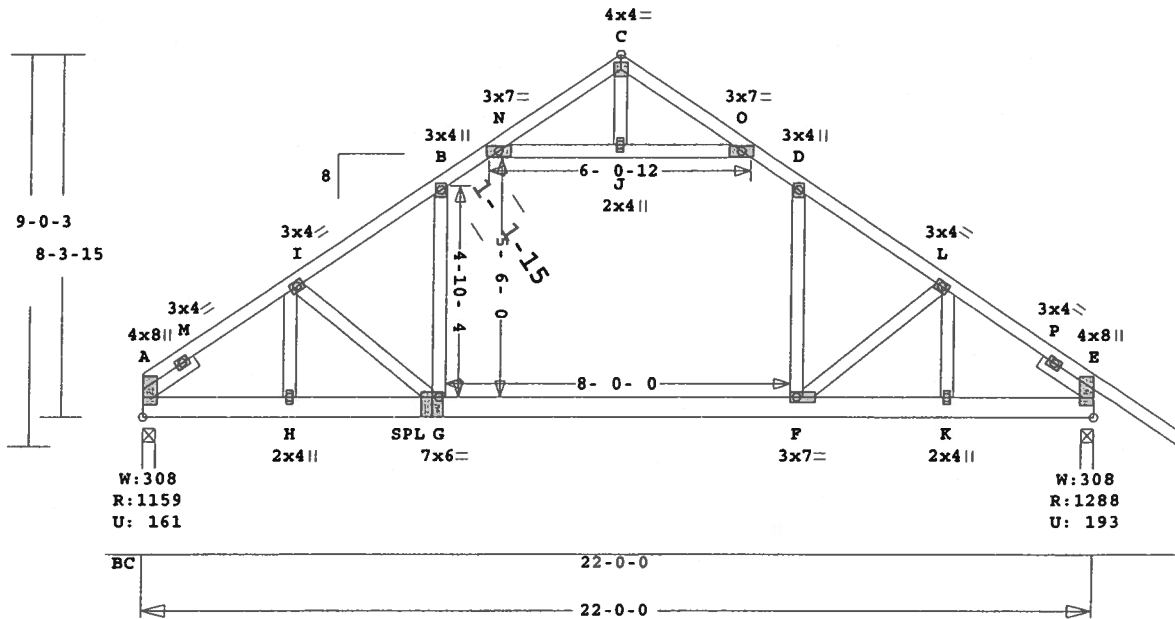
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark B2	Quan 5	Type TR	Span 220000	P1-H1 8	Left OH 0	Right OH 2-0-0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY

DG 13-9-9 HO 11-15	13-2-10 11-0-0	13-2-10 22-0-0	DG 13-9-9 HO 11-15
SL TC			2-0-0



ALL PLATES ARE LOCK20

Scale: 0.225" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 181.2 LBS

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	-Size-	-----Lumber-----
TC	0.79	2x 4 SP-#2
BC	0.84	2x 6 SP-#2
WB	0.12	2x 4 SP-#2
ACT	0.12	2x 4 SP-#2
AWT	0.00	2x 4 SP-#2
SL	0.06	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0-0-0	22-0-0
BC Cont.	0-0-0	22-0-0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00		
Plate Duration Factor	1.00		
plf - Live	Dead	From	To
TC V	40	20	0.0' 22.0'
BC V	0	20	0.0' 22.0'
TC V	0	0	7.0' 8.0'
TC V	0	0	14.0' 15.0'
BC V	60	10	7.0' 15.0'

Plus 9 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1160	161	3-8	1-8
			Hz = -187	
E	1288	193	3-8	1-8
			Hz = 188	

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A	-M	0.30	922	C	0.00 0.30
M	-I	0.31	1368	C	0.01 0.30
I	-B	0.65	1463	C	0.02 0.63
B	-N	0.79	1096	C	0.00 0.79
N	-C	0.79	96	T	0.00 0.79

-----Bottom Chords-----					
A	-H	0.71	1126	T	0.17 0.54
H	-G	0.82	1126	T	0.12 0.70
G	-F	0.84	1173	T	0.14 0.70
F	-K	0.82	1126	T	0.12 0.70
K	-E	0.71	1126	T	0.17 0.54
-----Webs-----					
H	-I	0.05	360	C	
I	-G	0.05	219	T	
G	-B	0.12	641	T	
F	-D	0.12	641	T	
F	-L	0.05	219	T	
K	-L	0.05	360	C	
-----Attic Chords (Top)-----					
N	-J	0.12	1217	C	0.12 0.00
J	-O	0.12	1217	C	0.12 0.00
-----Attic Webs (Top)-----					
J	-C	0.00	7	T	
-----Sliders-----					
A	-M	0.06	551	C	
P	-E	0.06	551	C	

TL Defl -0.30" in G -F L/857
LL Defl -0.19" in G -F L/999
Shear // Grain in B -N 0.81

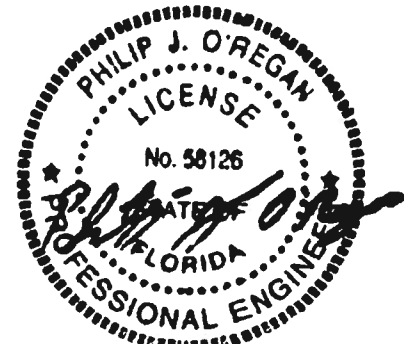
Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	- LOCK 20 Ga, Gross Area			
Plate	- RHS 20 Ga, Gross Area			
Jt Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	2.0	0.7 0.91
M	LOCK	3.0x 4.0	Ctr	0.50
I	LOCK	3.0x 4.0	Ctr	0.64
B	LOCK	3.0x 4.0	Ctr	0.25
N	LOCK	3.0x 7.0	Ctr	0.26
C	LOCK	4.0x 4.0	Ctr	0.64
O	LOCK	3.0x 7.0	Ctr	0.26
D	LOCK	3.0x 4.0	Ctr	0.25
L	LOCK	3.0x 4.0	Ctr	0.64
P	LOCK	3.0x 4.0	Ctr	0.50
E	LOCK	4.0x 8.0	-2.0	0.7 0.91
H	LOCK	2.0x 4.0	Ctr	0.40
G	LOCK	7.0x 6.0	Ctr	0.8 0.40
F	LOCK	3.0x 7.0	Ctr	0.37
K	LOCK	2.0x 4.0	Ctr	0.40
J	LOCK	2.0x 4.0	Ctr	0.40

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 1463 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark B3	Quan 1	Type TR	Span 220000	P1-H1 8	Left OH 0	Right OH 0	Engineering T06103079
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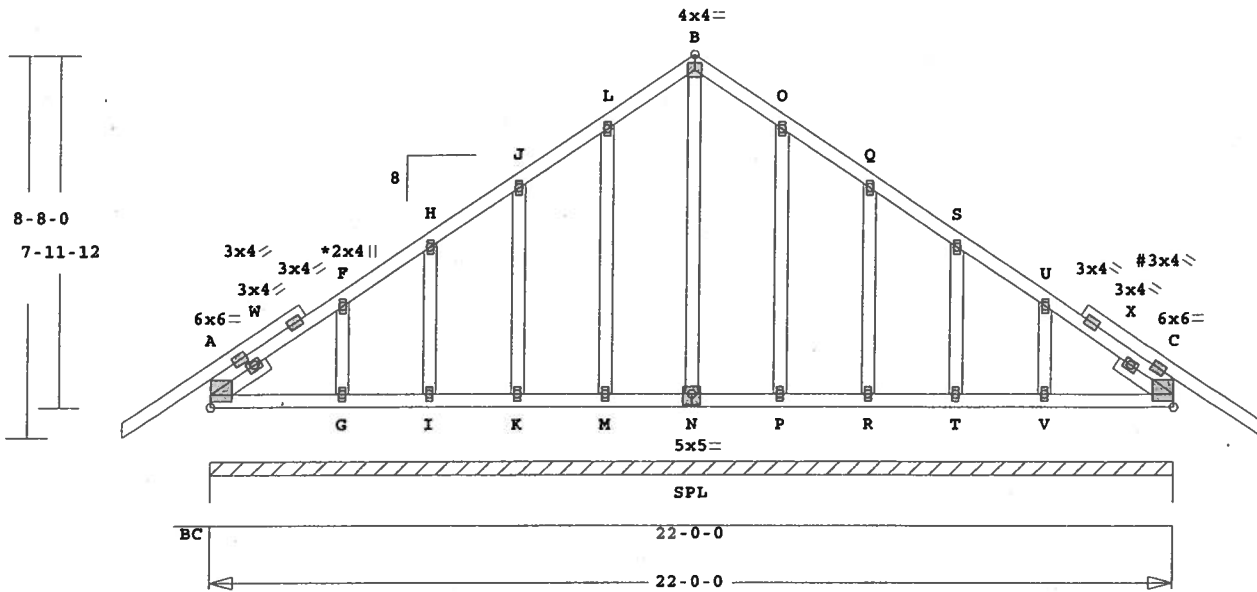
U# J#JONES-BUSSEY BUSSEY

DG 13-7-1
HO 7-12
SL
TC

13-2-10
11-0-0

13-2-10
22-0-0

DG 13-7-1
HO 7-12



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See Joint F For Typical Gable Plate Size and Placement

Scale: 0.228" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 192.6 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	-Size-	---	Lumber	----
TC	0.06	2x 4	SP-#2	
BC	0.07	2x 4	SP-#2	
GW	0.14	2x 4	SP-#2	
SL	0.00	2x 4	SP-#2	

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0	0- 0- 0
BC Cont.	0- 0- 0	22- 0- 0	0- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0	22- 0- 0	0- 0- 0
	1760	363	Hz =	188

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -W	0.04		88 C	0.00	0.04	
W -F	0.06		116 C	0.00	0.06	
F -H	0.06		91 C	0.00	0.06	
H -J	0.04		116 C	0.01	0.03	
J -L	0.05		188 C	0.02	0.03	
L -B	0.05		264 T	0.03	0.02	
B -O	0.05		264 T	0.03	0.02	
O -Q	0.05		188 C	0.02	0.03	
Q -S	0.04		116 C	0.01	0.03	
S -U	0.06		91 C	0.00	0.06	
U -X	0.06		116 C	0.00	0.06	
X -C	0.04		88 C	0.00	0.04	
-----Bottom Chords-----						
A -G	0.07		8 T	0.00	0.07	
G -I	0.03		0 T	0.00	0.03	
I -K	0.02		0 T	0.00	0.02	
K -M	0.02		0 T	0.00	0.02	
M -N	0.02		0 T	0.00	0.02	

N -P	0.02	0 T	0.00	0.02
P -R	0.02	0 T	0.00	0.02
R -T	0.02	0 T	0.00	0.02
T -V	0.03	0 T	0.00	0.03
V -C	0.07	8 T	0.00	0.07
-----Gable Webs-----				
G -F	0.02	197 C		
I -H	0.02	128 C		
K -J	0.04	151 C		
M -L	0.06	137 C		
N -B	0.14	204 C		
P -O	0.06	137 C		
R -Q	0.04	151 C		
T -S	0.02	128 C		
V -U	0.02	197 C		
-----Sliders-----				
A -W	0.00	55 C		
X -C	0.00	55 C		

TL Defl 0.00" in V -C L/999
LL Defl 0.00" in V -C L/999
Shear // Grain in W -F 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	6.0x	6.0	3.0	1.1	0.93
W	LOCK	3.0x	4.0	Ctr	Ctr	0.50
F	LOCK	2.0x	4.0	Ctr	Ctr	0.00
H	LOCK	2.0x	4.0	Ctr	Ctr	0.00
J	LOCK	2.0x	4.0	Ctr	Ctr	0.00
L	LOCK	2.0x	4.0	Ctr	Ctr	0.00
B	LOCK	4.0x	4.0	Ctr	Ctr	0.64
O	LOCK	2.0x	4.0	Ctr	Ctr	0.00
Q	LOCK	2.0x	4.0	Ctr	Ctr	0.00
S	LOCK	2.0x	4.0	Ctr	Ctr	0.00
U	LOCK	2.0x	4.0	Ctr	Ctr	0.00
X	LOCK	3.0x	4.0	Ctr	Ctr	0.50
C	LOCK	6.0x	6.0-3.0	1.1	0.93	
G	LOCK	2.0x	4.0	Ctr	Ctr	0.00
I	LOCK	2.0x	4.0	Ctr	Ctr	0.00
K	LOCK	2.0x	4.0	Ctr	Ctr	0.00
M	LOCK	2.0x	4.0	Ctr	Ctr	0.00
N	LOCK	5.0x	5.0	Ctr	-0.5	0.59
P	LOCK	2.0x	4.0	Ctr	Ctr	0.00
R	LOCK	2.0x	4.0	Ctr	Ctr	0.00
T	LOCK	2.0x	4.0	Ctr	Ctr	0.00
V	LOCK	2.0x	4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

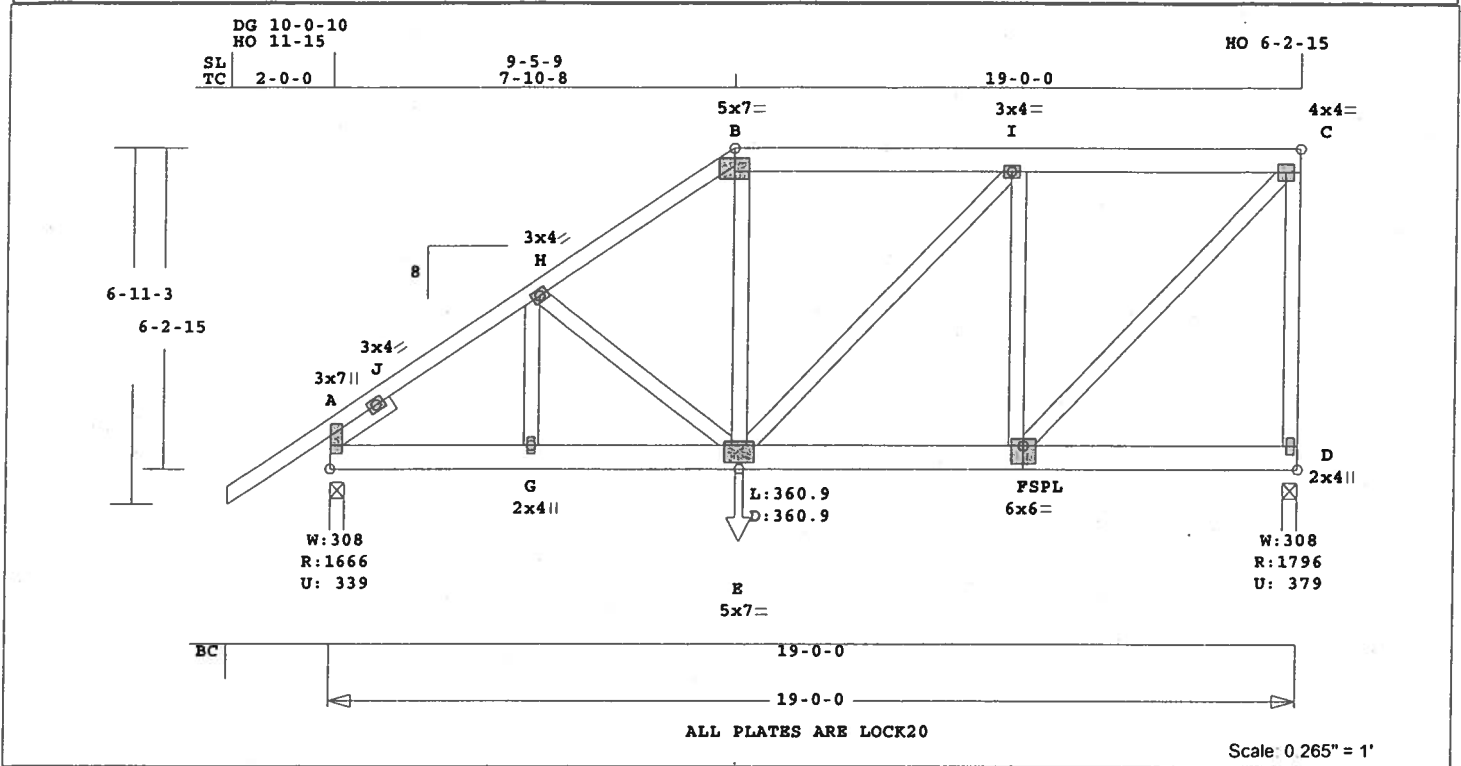
Design checked for 10 psf non-
concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 254 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark C1	Quan 1	Type HHIP	Span 190000	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 183.6 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI -Size-	Lumber----
TC 0.47 2x 4 SP-#2	
EX B -C 2x 6 SP-#2	
BC 0.82 2x 6 SP-#2	
WB 0.66 2x 4 SP-#2	
SL 0.08 2x 4 SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 0- 0
BC Cont.	0- 0- 0	19- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading

plf - Live	Dead	From	To
TC V 40	20	0.0'	19.0'
BC V 0	20	0.0'	19.0'
BC V 59	59	8.0'	18.0'
TC V -40	-20	18.0'	19.0'
BC V 0	-20	18.0'	19.0'
BC V 361	361	8.0'	CL-LB

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd
	Lbs	Lbs	In-Sx In-Sx
A	1666	340	3- 8 1-15
			H _z = -118
D	1796	379	3- 8 2- 2
			H _z = 234

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -J	0.45	1191 C	0.01 0.44

J -H	0.47	1921 C	0.03	0.44	
H -B	0.32	1995 C	0.02	0.30	
B -I	0.20	1683 C	0.09	0.11	
I -C	0.18	1423 C	0.07	0.11	
-----Bottom Chords-----					
A -G	0.82	1558 T	0.20	0.62	
G -E	0.43	1558 T	0.20	0.23	
E -F	0.58	1423 T	0.19	0.39	
F -D	0.39	177 T	0.00	0.39	
-----Webs-----					
G -H	0.03	249 C			
H -E	0.02	159 T			
E -B	0.15	821 T			
E -I	0.11	381 T			
F -I	0.26	674 C			
F -C	0.66	2082 T			
D -C	0.63	1603 C	WindLd		
-----Sliders-----					
A -J	0.08	874 C			

TL Defl -0.11" in E -F L/999
LL Defl -0.05" in E -F L/999
Splat // Grain in F -D 0.36

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 7.0 1.5 0.5 1.00
J LOCK 3.0x 4.0 Ctr Ctr 0.50
H LOCK 3.0x 4.0 Ctr Ctr 0.60
B LOCK 5.0x 7.0 Ctr-0.6 0.60
I LOCK 3.0x 4.0 Ctr Ctr 0.50
C LOCK 4.0x 4.0 Ctr Ctr 0.99
G LOCK 2.0x 4.0 Ctr Ctr 0.40
E LOCK 5.0x 7.0 Ctr-1.5 0.54
F LOCK 6.0x 6.0 Ctr-1.2 0.78
D LOCK 2.0x 4.0 Ctr Ctr 0.78

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:

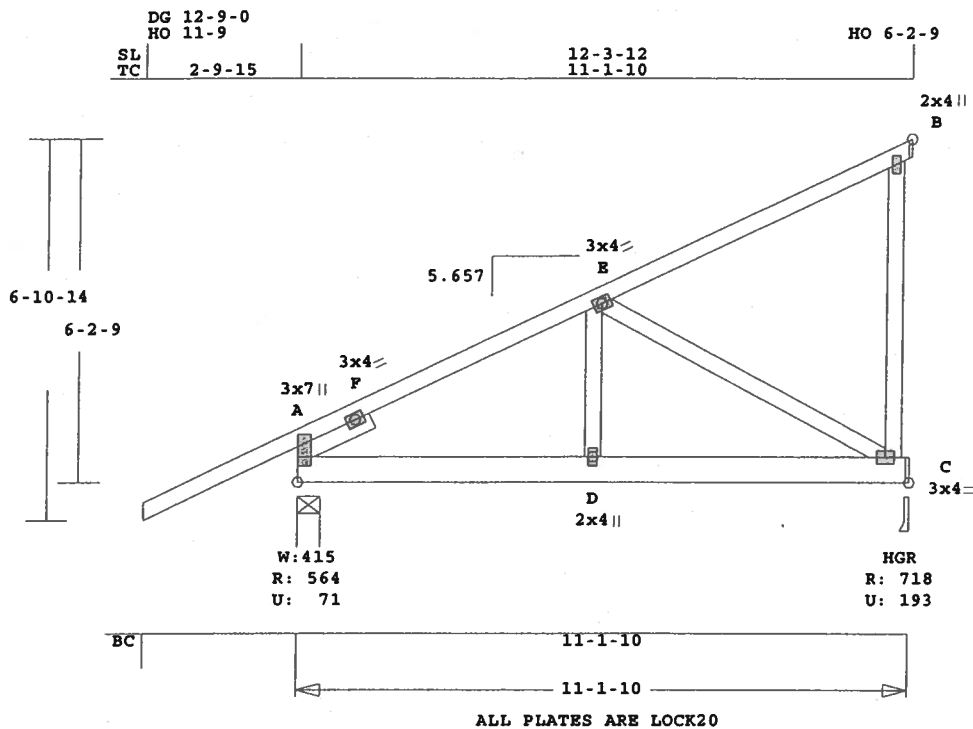
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Closed Face
Setback 7-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1995 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark CJ1	Quan 1	Type MONO.DD	Span 110110	P1-H1 5.657	Left OH 2- 9-15	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 90.3 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

	CSI -Size-	Lumber
TC	0.78 2x 4	SP-#2
BC	0.30 2x 6	SP-#2
WB	0.27 2x 4	SP-#2
SL	0.01 2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	11- 1-10	
BC Cont.	0- 0- 0	11- 1-10	

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading	Lumber Duration Factor	1.25		
Plate Duration Factor	1.25			
plf - Live Dead From To				
TC V	40	20	0.0'	11.1'
BC V	0	20	0.0'	11.1'
TC V	-40	-20	0.0'	11.1'
BC V	0	-20	0.0'	11.1'
	57	29		
	0	29		

Plus 7 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd
	Lbs	Lbs	In-Sx In-Sx
A	564	71	4-15 1- 8
			Hx = -88
C	719	194	3- 8 1- 8
			Hx = 254

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A - F	0.10		422 C	0.00	0.10	
F - E	0.62		533 C	0.02	0.60	
E - B	0.78		118 C	0.00	0.78	
-----Bottom Chords-----						
A - D	0.30		507 T	0.06	0.24	
D - C	0.23		507 T	0.04	0.19	
-----Webs-----						
D - E	0.03		253 T			
E - C	0.27		582 C			
C - B	0.19		430 T	WindLd		
-----Sliders-----						
A - F	0.01		171 C			

TL Defl -0.04" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.43

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	LOCK	20 Ga, Gross Area
Plate - LOCK 20 Ga, Gross Area		
Jt Type	Plt Size	X Y JSI
A LOCK	3.0x 7.0	1.5 0.3 0.87
F LOCK	3.0x 4.0	Ctr Ctr 0.50
E LOCK	3.0x 4.0	Ctr Ctr 0.46
B LOCK	2.0x 4.0	Ctr Ctr 0.38
D LOCK	2.0x 4.0	Ctr Ctr 0.38
C LOCK	3.0x 4.0	Ctr Ctr 0.71

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:

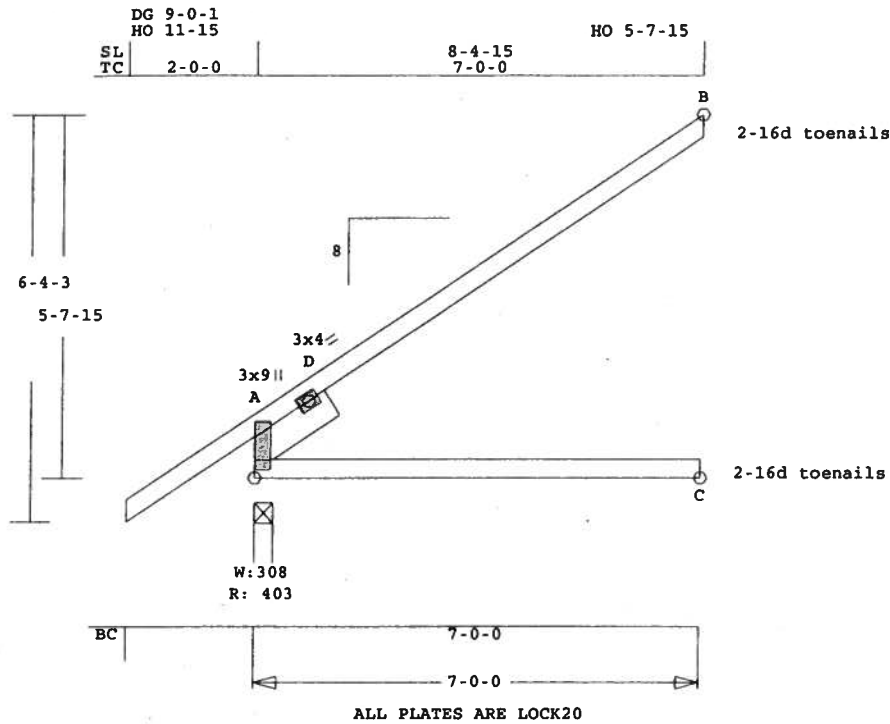
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7-10- 8
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Use properly rated hangers for
loads framing into girder
truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 582 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark J1	Quan 2	Type JCA2	Span 70000	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Scale: 0.331" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 38.3 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	-Size-	---	Lumber----
TC	0.51	2x 4	SP-#2
BC	0.52	2x 4	SP-#2
SL	0.03	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	7- 0- 0
BC Cont.		0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

	D -B	0.51	153 C	0.01	0.50
	B -B	0.00	4 C		
-----Bottom Chords-----					
	A -C	0.52	115 T	0.00	0.52
-----Sliders-----					
	A -D	0.03	325 T		
TL Defl		-0.18"	in A -C	L/416	
LL Defl		-0.07"	in A -C	L/999	
Shear // Grain	in D -B				0.19

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 9.0 1.5 0.5 0.66
D LOCK 3.0x 4.0 Ctr Ctr 0.50

Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 452 Lbs
Quality Control Factor 1.25

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	404	0	3- 8	1- 8
			Hz =	170
C	133	0	3- 8	1- 8
			Hz =	116
B	195	139	3- 8	1- 8

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -D	0.12		452 C	0.00	0.12	

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

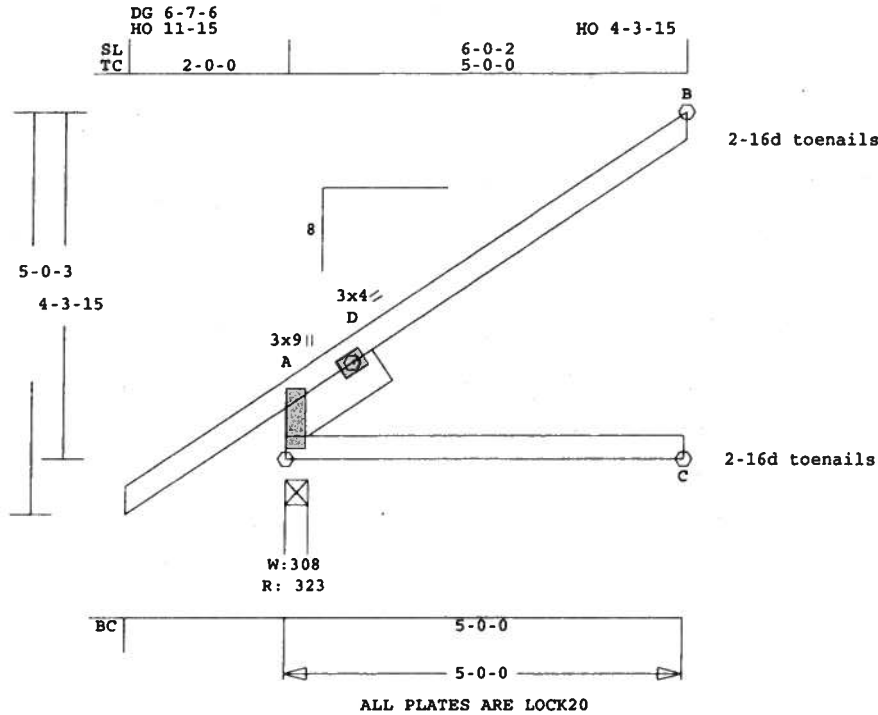
NOTES:
Trusses Manufactured by:

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark J2	Quan 2	Type JCA2	Span 50000	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Scale: 0.414" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.0 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

TC	BC	SL	CSI -Size-	Lumber	SP-#2
0.26	0.29	0.01	2x 4	2x 4	2x 6

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0
BC Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

D -B	B -B	A -C	A -D	TL Defl	LL Defl	Shear // Grain
0.26	0.00	0.29	0.01	-0.04" in A -C	-0.02" in A -C	0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 9.0 1.5 0.5 0.62
D LOCK 3.0x 4.0 Ctr Ctr 0.50

Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 221 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	324	0	3- 8	1- 8
			Hz =	122
C	95	1	3- 8	1- 8
			Hz =	83
B	141	104	3- 8	1- 8

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

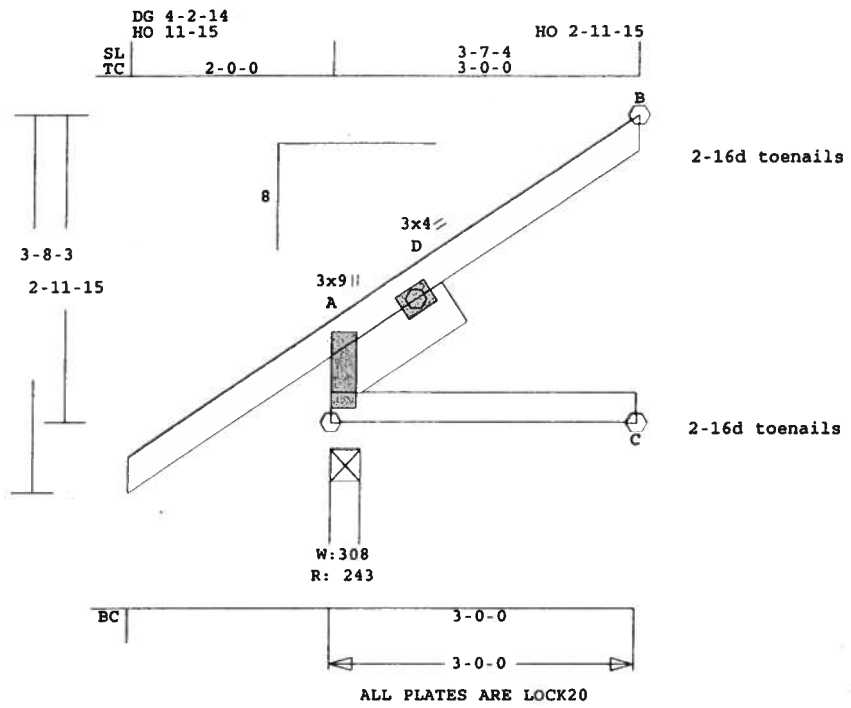
Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -D	0.11		221 C	0.02	0.09	

NOTES:
Trusses Manufactured by:



Job JONES-BUSSEY	Mark J3	Quan 2	Type JCA2	Span 30000	P1-H1 8	Left OH 2- 0- 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



Scale: 0.530" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.6 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI	Size	Lumber
TC	0.09	2x 4 SP-#2
BC	0.11	2x 4 SP-#2
SL	0.00	2x 6 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

	D - B	0.09	77 C	0.00	0.09
	B - B	0.00	4 C		
	A - C	0.11	49 T	0.00	0.11
	A - D	0.00	37 C		
TL Defl		0.00"	in A - C	L/999	
LL Defl		0.00"	in A - C	L/999	
Shear // Grain			in D - D	0.08	

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 3.0x 9.0 1.5 0.5 0.61
D LOCK 3.0x 4.0 Ctr Ctr 0.50

Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 77 Lbs
Quality Control Factor 1.25

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	243	0	3- 8	1- 8
			Hz =	73
C	56	3	3- 8	1- 8
			Hz =	50
B	88	71	3- 8	1- 8

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

Membr CSI P Lbs Axl-CSI-Bnd

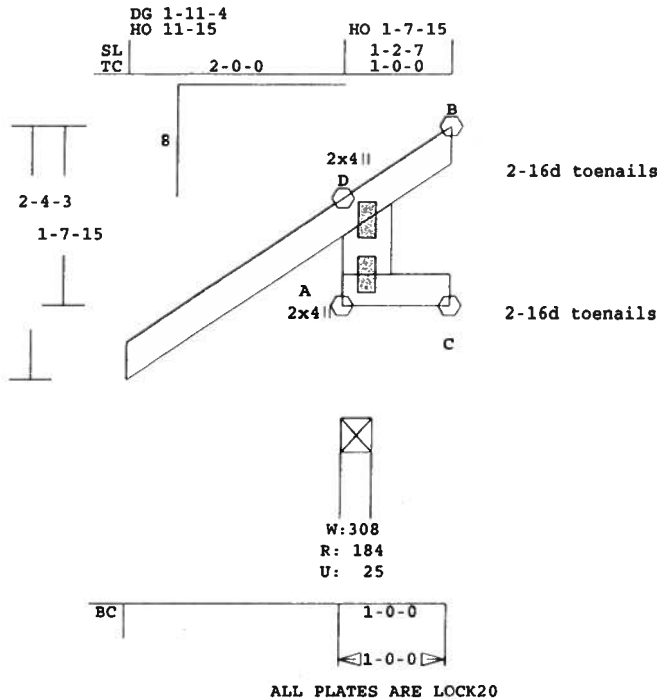
-----Top Chords-----

A - D	0.08	60 C	0.00	0.08
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NOTES:
Trusses Manufactured by:



U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 10.9 LBS

Online Plus -- Version 20.0.003	Run Date: 27-OCT-06	CSL -Size- ---Lumber----	TC 0.06 2x 4 SP-#2	BC 0.01 2x 4 SP-#2	WB 0.03 2x 6 SP-#2
Brace truss as follows:					
O.C.	From	To			
TC Cont.	0- 0- 0	1- 0- 0			
BC Cont.	0- 0- 0	1- 0- 0			
Loading	Live	Dead	(psf)		
TC	20.0	10.0			
BC	0.0	10.0			
Total	20.0	20.0	40.0		
Spacing			24.0"		
Lumber Duration Factor			1.25		
Plate Duration Factor			1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10			
BC Fb=1.10	Fc=1.10	Ft=1.10			

Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as Components and Claddings* for Exterior zone location.
 Wind Speed: 120 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 5.0 psf
 BC Dead Load: 5.0 psf
 Max comp. force 87 Lbs
 Quality Control Factor 1.25

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORTS: SBCCI 9761
 ROBBINS ENGINEERING, INC.
 BASED ON SP LUMBER
 USING GROSS AREA TEST.
 Plate - LOCK 20 Ga, Gross Area
 Plate - RHS 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 D LOCK 2.0x 4.0 Ctr Ctr 0.38
 A LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan
 License # 58126
 Address: P.O. Box 280055, Tampa, FL 33682



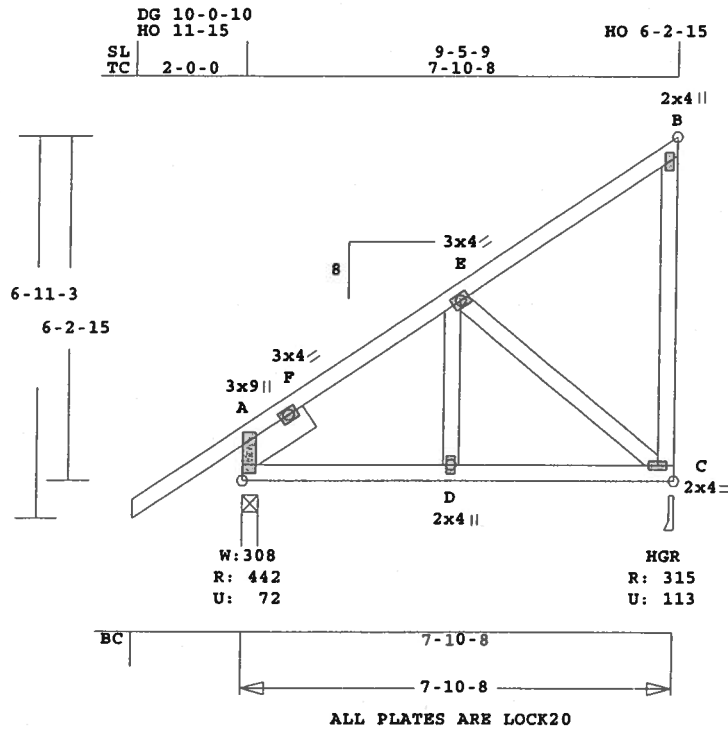
Plus 8 Wind Load Case(s)	Plus 1 UBC LL Load Case(s)
Jt React Uplft Size Req'd	
Lbs Lbs In-Sx In-Sx	
A 184 26 3- 8 1- 8	
	Hz = 43
A 56 17 3- 8 1- 8	
	Hz = 31
B 50 73 3- 8 1- 8	

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.
 For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

Membr CSI P Lbs Axl-CSI-Bnd	
-----Top Chords-----	
D -B 0.06 79 C 0.00 0.06	

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.

U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.0 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

	CSI	-Size-	---	Lumber	----
TC	0.18	2x 4	SP-#2		
BC	0.11	2x 4	SP-#2		
WB	0.21	2x 4	SP-#2		
SL	0.01	2x 6	SP-#2		

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	7-10- 8
BC Cont.		0- 0- 0	7-10- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	443	72	3- 8	1- 8
			Hz =	-115
C	315	114	3- 8	1- 8
			Hz =	232

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -F	0.07		106 C	0.00	0.07
F -E	0.18		243 C	0.02	0.16
E -B	0.17		130 T	0.01	0.16
-----Bottom Chords-----					
A -D	0.11		208 T	0.03	0.08
D -C	0.10		208 T	0.02	0.08

-----Webs-----
D -E 0.02 160 T
E -C 0.09 315 C
C -B 0.21 107 C WindLd
-----Sliders-----
A -F 0.01 232 C

TL Defl -0.02" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in E -B 0.15

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 9.0	1.5	0.5	0.67
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
E	LOCK	3.0x 4.0	Ctr	Ctr	0.46
B	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.76

concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 315 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

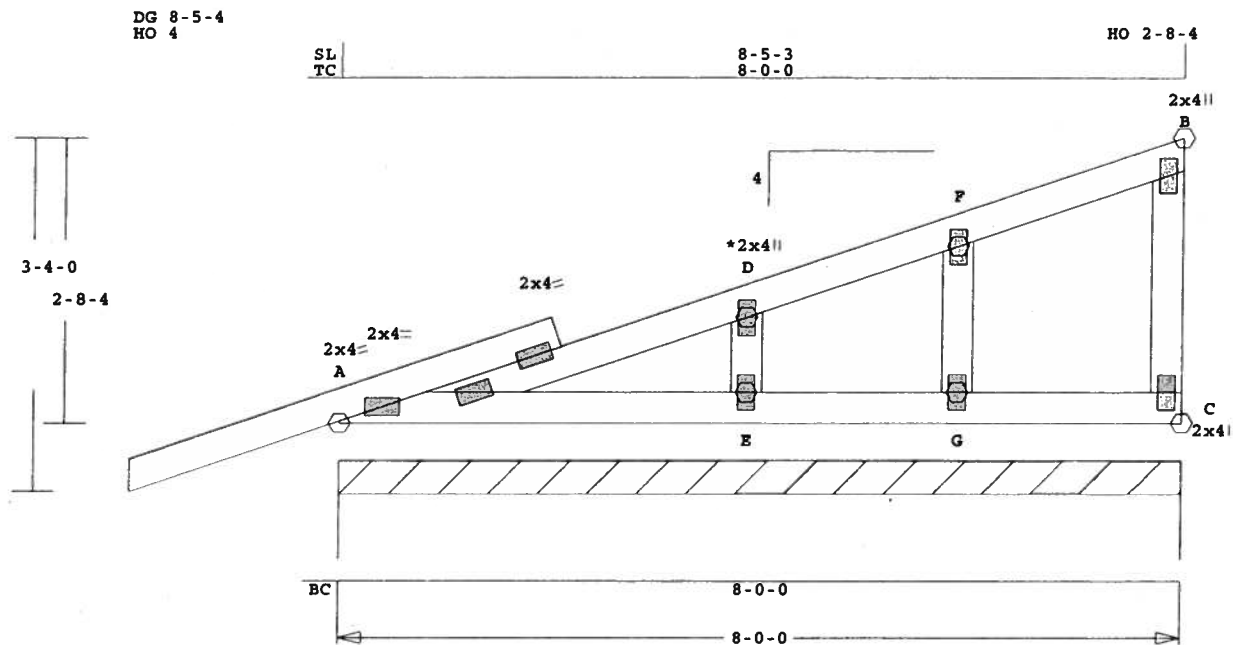
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

Truss Design Engineer: Philip J. O'Regan
License # 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job JONES-BUSSEY	Mark MGE	Quan 1	Type MONO	Span 80000	Pl-H1 4	Left OH 0	Right OH 0	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.547" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.6 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

C - B	0.07	122 T	0.01	0.06
E - D	0.05	160 T	0.02	0.03
G - F	0.03	231 T	0.03	0.00

Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 150 Lbs
Quality Control Factor 1.25

CSI	-Size-	---Lumber---	TL Defl	LL Defl	Shear // Grain in A - D
TC	0.08	2x 4 SP-#2	0.00" in A - E	0.00" in A - E	L/999
BC	0.06	2x 4 SP-#2			0.12
WB	0.07	2x 4 SP-#2			
GW	0.05	2x 4 SP-#2			

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 8- 0- 0
BC Cont. 0- 0- 0 8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.80
D LOCK 2.0x 4.0 Ctr Ctr 0.00
F LOCK 2.0x 4.0 Ctr Ctr 0.00
B LOCK 2.0x 4.0 Ctr Ctr 0.38
E LOCK 2.0x 4.0 Ctr Ctr 0.00
G LOCK 2.0x 4.0 Ctr Ctr 0.00
C LOCK 2.0x 4.0 Ctr Ctr 0.38

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to 8- 0- 0		
	640	132	Hz =	97

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A - D	0.08		54 C	0.00	0.08	
D - F	0.05		42 C	0.00	0.05	
F - B	0.06		46 C	0.00	0.06	
-----Bottom Chords-----						
A - E	0.06		14 T	0.00	0.06	
E - G	0.02		0 T	0.00	0.02	
G - C	0.02		0 T	0.00	0.02	
-----Webs-----						

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

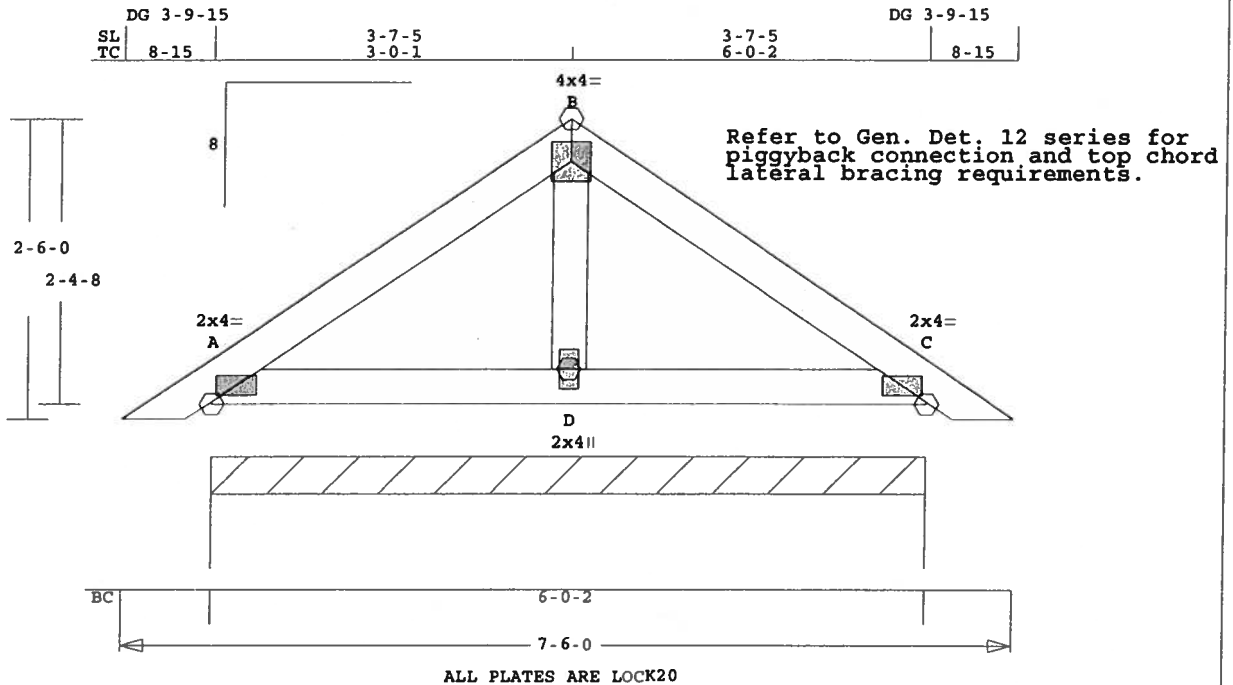
NOTES:
Trusses Manufactured by: Mayo Truss Co. Inc.
Analysis Conforms To: FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.

Truss Design Engineer: Philip J. O'Regan
License # 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
JONES-BUSSEY	P1	24	TR	70600	8	8-15	8-15	T06103079

U# J#JONES-BUSSEY BUSSEY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.6 LBS

D -C 0.05 1 T 0.00 0.05

concurrent LL on BC.

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

-----Webs-----

Refer to Gen Det 3 series for web bracing and plating.

CSI	-Size-	-----Lumber-----
TC	0.06	2x 4 SP-#2
BC	0.05	2x 4 SP-#2
WB	0.00	2x 4 SP-#2

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.08

Wind Loads - ANSI / ASCE 7-02
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 120 mph

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 6- 0
BC Cont.	0- 0- 0	7- 6- 0

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 146 Lbs
Quality Control Factor 1.25

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.62
B LOCK 4.0x 4.0 Ctr Ctr 0.43
C LOCK 2.0x 4.0 Ctr Ctr 0.62
D LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to	6- 0- 2	
	576	123	Hz =	46

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

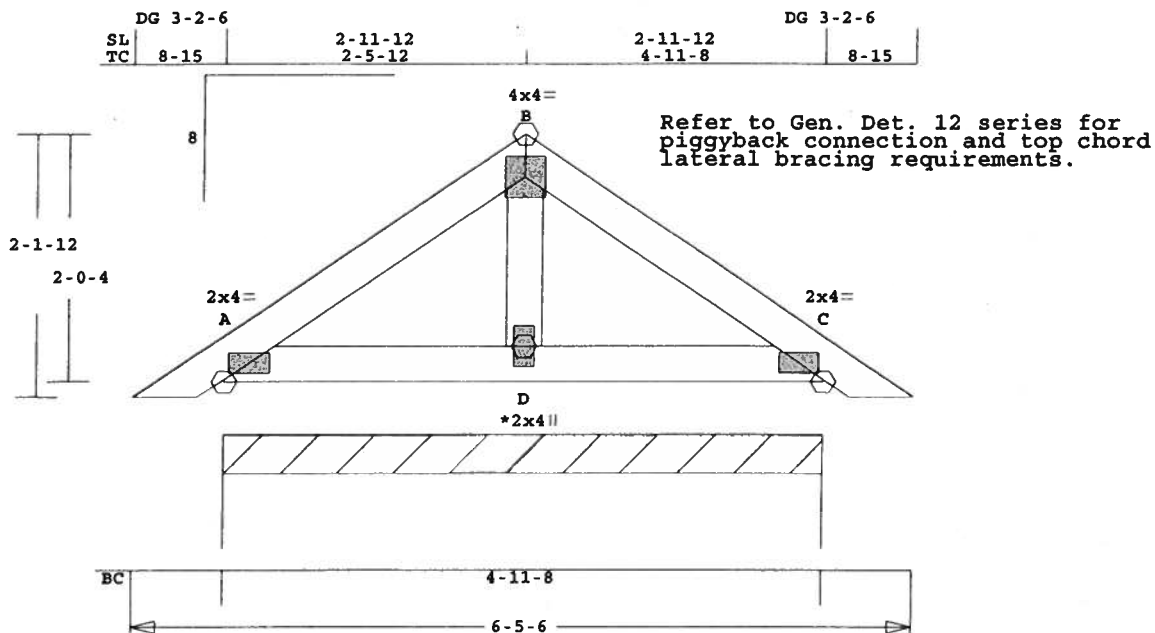
NOTES:
Trusses Manufactured by: Mayo Truss Co. Inc.
Analysis Conforms To: FBC2004
OH Loading Soffit psf 2.0
Design checked for 10 psf non-

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -B	0.06		146	C	0.01	0.05
B -C	0.06		146	C	0.01	0.05
-----Bottom Chords-----						
A -D	0.05		1	T	0.00	0.05



Job JONES-BUSSEY	Mark P2	Quan 1	Type TR	Span 60506	P1-H1 8	Left OH 8-15	Right OH 8-15	Engineering T06103079
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U# J#JONES-BUSSEY BUSSEY



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.632" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.6 LBS

Online Plus -- Version 20.0.003
RUN DATE: 27-OCT-06

CSI -Size- ----Lumber----
TC 0.04 2x 4 SP-#2
BC 0.03 2x 4 SP-#2
GW 0.00 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 5- 6
BC Cont.	0- 0- 0	6- 5- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

D -C 0.03 1 T 0.00 0.03
-----Gable Webs-----
D -B 0.00 33 C

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.06

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: SBCCI 9761
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.60
B LOCK 4.0x 4.0 Ctr Ctr 0.42
C LOCK 2.0x 4.0 Ctr Ctr 0.60
D LOCK 2.0x 4.0 Ctr Ctr 0.00

concurrent LL on BC.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 120 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 106 Lbs
Quality Control Factor 1.25

REVIEWED BY:

Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	4-11- 8	
	492	106	Hz =	38

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

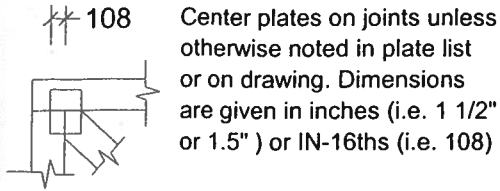
Design checked for 10 psf non-

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -B	0.04	106	C	0.01	0.03	
B -C	0.04	106	C	0.01	0.03	
-----Bottom Chords-----						
A -D	0.03	1	T	0.00	0.03	



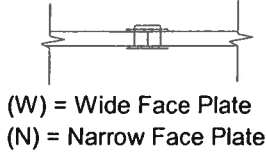
ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



FLOOR TRUSS SPLICE

(3X2, 4X2, 6X2)



LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

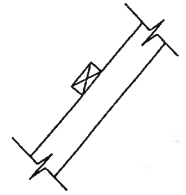
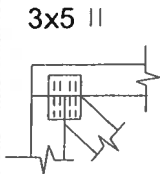


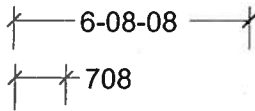
PLATE SIZE AND ORIENTATION

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.



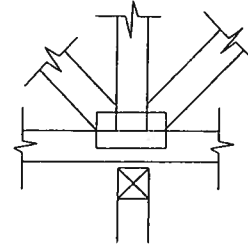
DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.



W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.

The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

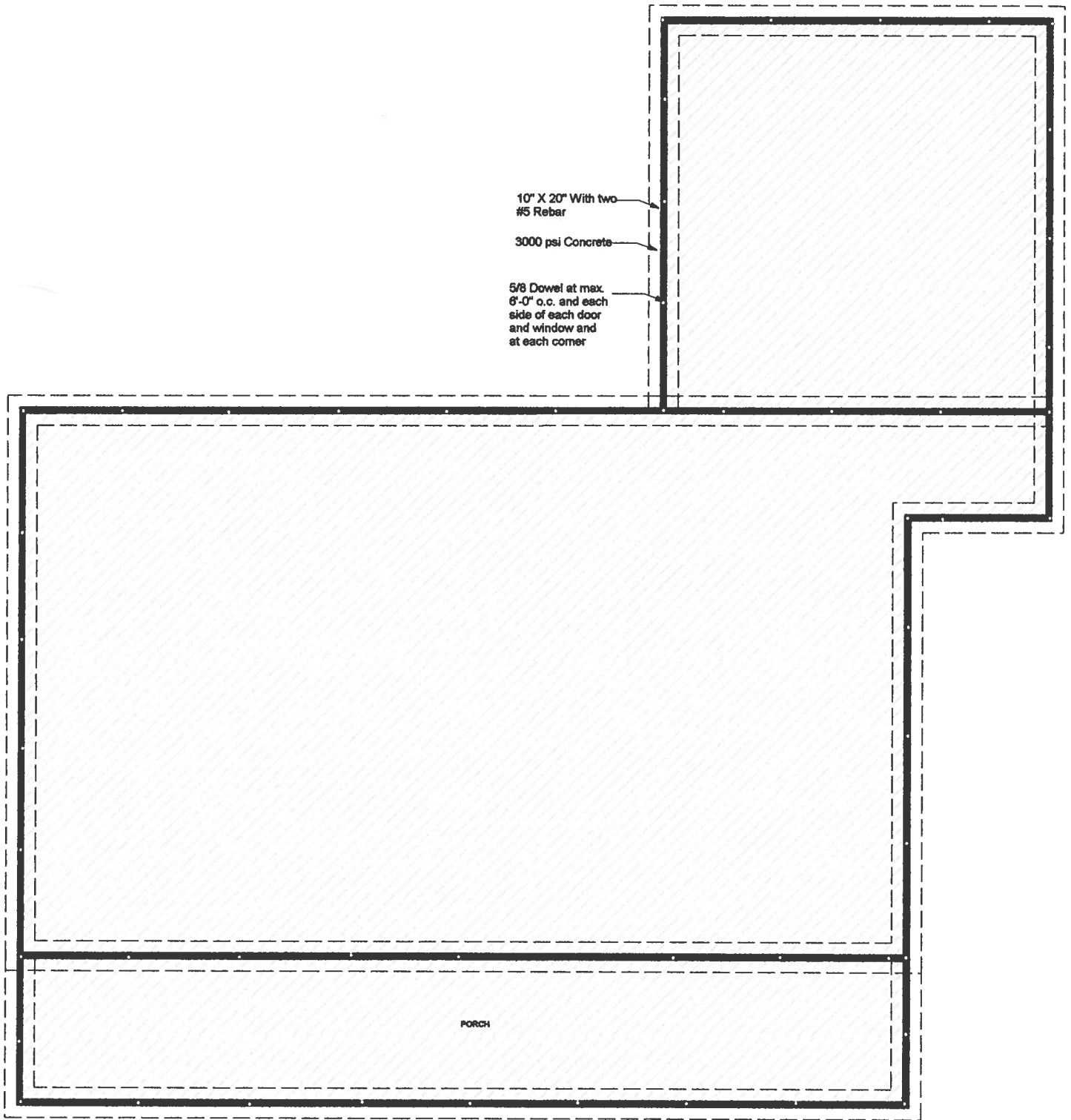
Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd.
Tampa, FL 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbseng.com



FOUNDATION PLAN

25256

This instrument was prepared by:
Beverly K. Smith, employee of
Vision Title of Alachua County, LLC
4881-1 NW 8th Ave., Gainesville, FL 32605

Please Return to:

Return to:
Vision Title of Alachua County, LLC
4881 NW 8th Avenue, Suite 1
Gainesville, FL 32605

Permit No. _____

NOTICE OF COMMENCEMENT

Loan No. 0059047191

Inst: 2006024289 Date: 10/12/2006 Time: 11:23
DC, P. DeWitt Cason, Columbia County B: 1098 P: 2000

State of Florida
County of Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: See attached Schedule A

(legal description of the property, and street address if available)

BUSSEY GLN
FORT WHITE Florida 32038

2. General description of improvement: Construction of Single Family Residence

3. Owner information:

a. Name and address: CARLTON BUSSEY

BUSSEY GLN
FORT WHITE Florida 32038

b. Interest in property: fee simple

c. Name and address of fee simple titleholder (if other than Owner):

4. a. Contractor's name and address:

LEON JONES CONSTRUCTION
7717 NW 19TH STREET
ALACHUA, FL 32615

b. Contractor's phone number: 386/418-4690

c. Fax number (optional, if service by fax is acceptable):

5. Surety

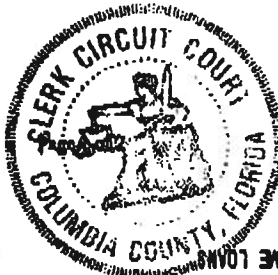
a. Name and address: N/A

b. Phone number:

c. Amount of bond: \$

d. Fax number (optional, if service by fax is acceptable):

Notice of Commencement 0605



STATE OF FLORIDA COUNTY OF COLUMBIA
I HEREBY CERTIFY that the above and foregoing
is a true copy of the original filed in this office
P. DeWITT CASON, CLERK OF COURTS

By: [Signature]
Deputy Clerk #16022K

Date: 10-12-06

3pgs

0059047191

- 6. a. Lender's name and address: First Horizon Home Loans
200 E Robinson Ste 400
Orlando, FL 32801
- b. Lender's phone number: (866) 684-8672
- c. Fax number (optional, if service by fax is acceptable): _____

- 7. a. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: _____

(name and address)

- b. Phone numbers of designated persons: _____
- c. Fax number (optional, if service by fax is acceptable): _____

- 8. a. In addition to himself or herself, Owner designates First Horizon Home Loans (address above) & Vision Title of Alachua County, * to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes. 4881-1 NW 8th Ave., Gainesville, FL 32605
- b. Phone number of person or entity designated by owner: _____
- c. Fax number (optional, if service by fax is acceptable): _____

- 9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

Carlton Bussey
 (Signature of Owner) **CARLTON BUSSEY**

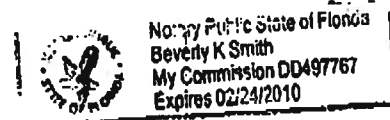
Sworn to (or affirmed) and subscribed before me this 10 day of Oct, 2006 by
Carlton Bussey
 (name of person making statement)

Beverly K. Smith
 (Signature of Notary Public - State of Florida)
Beverly K. Smith

(Print, Type, or Stamp Commissioned Name of Notary Public)

Personally Known _____ OR Produced Identification

Type of Identification Produced drivers license



Inst:2006024289 Date:10/12/2006 Time:11:23
 _____ DC,P.Dewitt Cason,Columbia County B:1098 P:2081

Schedule "A"

File Number: GV06-470

A tract of land situated in Section 18, Township 7 South, Range 17 East, Columbia county, Florida, said tract of land being more particularly described as follows:

Commence at a concrete monument at the Southeast corner of the aforementioned Section 18, Township 7 South, Range 17 East, for the Point of Reference and run South 88° 00' 29" West, along the South line of said Section 18, a distance 2641.60 feet to a concrete monument at the Southeast corner of the Southeast ¼ of the Southwest ¼ of said Section 18; thence run North 01° 30' 02" West, along the East line of said Southeast ¼ of the Southwest ¼, a distance of 210.00 feet to a concrete monument at the Northeast corner of the South 210 feet of said Southeast ¼ of the Southwest ¼; thence run South 88° 00' 29" West, along the North line of said South 210 feet, a distance of 166.33 feet; thence run North 03° 40' 54" West, a distance of 1441.93 feet to a concrete monument; thence run North 01° 28' 59" West, a distance of 464.97 feet to a steel rod and cap on the true Point of Beginning; thence continue North 01° 28' 59" West, a distance of 521.97 feet to a concrete monument on the North line of the Southwest ¼ of Section 18; thence run South 88° 01' 29" West, along said North line, a distance of 838.75 feet to a steel rod and cap; thence run South 02° 01' 55" East, a distance of 521.95 feet to a steel rod and cap; thence run North 88° 01' 29" East, a distance of 833.75 feet to the true Point of Beginning.

Subject to the maintained right of way for the County Road (Bussey Road) along the northerly boundary thereof.

Subject to and together with an easement for ingress, egress and public utilities over, under and across a 40 foot wide strip of land being more particularly described as follows:

A Tract of land situated in Section 18, Township 7 South, Range 17 East, Columbia County, Florida, said tract of land being more particularly described as follows

Commence at a concrete monument at the Southeast corner of the aforementioned Section 18, Township 7 South, Range 17 East for the Point of Reference and run South 88° 00' 29" West, along the South line of said Section 18, a distance of 2641.60 feet to a concrete monument at the Southeast corner of the Southeast ¼ of the Southwest ¼ of said Section 18; thence run North 01° 30' 02" West, along the East line of said Southeast ¼ of the Southwest ¼, a distance of 210.00 feet to a concrete monument at the Northeast corner of the South 210.00 feet of said Southeast ¼ of the Southwest ¼; thence run South 88° 00' 29" West, along the North line of said South 210.00 feet, a distance of 166.33 feet; thence run North 03° 40' 54" West, a distance of 879.75 feet to a steel rod and cap and the true Point of Beginning; thence continue North 03° 40' 54" West, a distance of 562.18 feet to a concrete monument; thence run North 01° 28' 59" West, a distance of 986.94 feet to a concrete monument on the North line of the Southwest ¼ of Section 18; thence run South 88° 01' 29" West, along said North line, a distance of 450.02 feet; thence run South 01° 28' 59" East, a distance of 40.00 feet; thence run North 88° 01' 29" East, a distance of 410.02 feet to a steel rod and cap; thence run South 01° 28' 59" East, a distance of 947.36 feet to a steel rod and cap; thence run South 03° 40' 54" East, a distance of 562.94 feet to a steel rod and cap; thence run North 86° 19', 06" East a distance of 40.00 feet to the true Point of Beginning.

Inst: 2006024289 Date: 10/12/2006 Time: 11:23

File Number: GV06-470

DC, P. DeWitt Cason, Columbia County B: 1098 P: 2082

Legal Description with Non Homestead
Closers Choice

*C.H.B.,
M.G.S.*

Joe,

They are not sure what kind of gas fireplace they are going with at this time.

Let them know what - if any additional info they will need.

NOTICE OF TREATMENT

Applicator Name McCall Service
Address 415 NW 250 St Suite 1
City Newberry
Time 8:25 Date 12-29-06

SITE LOCATION

Lot # 12 Block # 0000 Permit # 25256
Subdivision Bussey Glen Unra
Address 578 SW Bussey Glen

Name of Chemical Applied Bifenthrin 404.16oz Used .06 %

Area Treated 2826 Sqft 230 LNft

Gallons Used 4 21 gals

Remarks supported slab

Applicator - White

Permit File - Canary

Permit Holder - Pink

GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

FIELD DENSITY WORKSHEET

CLIENT LEON JONES CONST. DATE 29 DEC 06

PROJECT NAME BUSSEY RES. FORT WHITE PROJECT NO. _____

EARTH CONTRACTOR 578 SW BUSSEY GLEN PERMIT NO. #000025256

COMPACTION REQUIREMENT (%) 95% Standard Proctor Modified Proctor TESTED BY JWC

TOTAL ON-SITE TIME _____ FIELD CONTACT _____ MILES FROM OFFICE _____

Limerock Subgrade Pipe Backfill Building Pad Building Footing Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
CTR. OF PAD	105.0	11.0	FB	12"	3.4	108.3	104.8	99.8
CTR. OF E. SIDE	2	2	2	2	4.2	108.9	104.5	99.5
CTR. OF W. SIDE	2	2	2	2	3.7	108.4	104.6	99.6

REMARKS _____

* Density failed to meet minimum project requirement
 ** Retest indicates minimum density requirement was obtained.
 () Client is aware of unsatisfactory test results.

GERBANYNICO AVENUE DIN

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 18-7S-17-10021-010 Building permit No. 000025256

Use Classification SFD, UTILITY Fire: 134.42

Permit Holder LEON JONES Waste: 184.25

Owner of Building CARLTON & MARISA BUSSEY Total: 318.67

Location: 578 SW BUSSEY GLEN, FT. WHITE, FL

Date: 11/02/2007

Kenny Dieke
Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

25256

RETAIN THIS CERTIFICATE FOR A MINIMUM OF 3 YEARS
PRETREATMENT CERTIFICATE OF COMPLETION
And PROPERTY OWNER'S TRANSFER GUARANTEE



Call McCall... "We Do It All!"

Builder Leon Jones Construction Date 10/22/07

Address of Treatment 578 SW Bussy Glenn

City Fl. White State Fl. Zip _____

The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services.

Treatment Date 12-29-06 Square Footage of Property Treated 2826 By [Signature]
Authorized Agent

Type Material Used B-fenchrin % active .06 Total Gallons Applied 451 gal. Final 92 gal

Other means _____

NOTICE TO PROPERTY OWNER

Your builder selected our company to provide the subterranean termite protection on your property. This treatment provides coverage for you for one year from the date of treatment. As a new property owner, Florida Law, Chapter 482.227 FS, and 5E-14.105, requires that the property owner, "at the time of each renewal, if a previous renewal was purchased, shall have the option of extending the guarantee annually after the first year, for no less than four additional years."

I acknowledge my understanding of the options available to me under the Florida Statutes as outlined above, and by my signature below, authorize MCCALL SERVICE, INC. to transfer the subterranean Termite Protection currently registered in my builders name to:

Name _____ Signature _____

Address _____

City _____ State _____ Zip _____

Mailing Address if Different from Treated Property _____

Home Telephone () _____ Business Telephone () _____

Financing (check one) Conventional FHA/VA Closing Date _____

NOTE: As the new owner, you will receive a Subterranean Termite Protection Guarantee in your name. If you have not received your guarantee within thirty (30) days of closing, please contact our Termite Department at (904) 389-5561