



COMPONENT DESIGN DRAWINGS & DETAILS

**Simpson Strong-Tie
Company, Inc.**

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(800) 999-5099
www.strongtie.com

Prepared for:	19 Lumber Inc
Project:	P-1038 - Martinez
Date:	6/1/2026 9:36 AM
Project Location:	Project location has not been provided.
SST Project (TID):	318742

Notes:

1. The component design drawings referenced below have been prepared based on design criteria and requirements set forth in the Construction Documents, as communicated by the Component Manufacturer.
2. The engineer's signature on these drawings indicates professional engineering responsibility solely for the individual components to be able to resist the design loads indicated, utilizing all the design parameter and materials indicated or referenced on each individual design.
3. It is the Building Designer's responsibility to review the component design drawings to insure compatibility with the Building design, Refer to all notes on the individual component design drawings.

47 Component Design Drawing(s):

1-G01: SID 4280704	13-PB02: SID 4280716	25-T10: SID 4280728	37-V13: SID 4280740
2-G02: SID 4280705	14-S01: SID 4280717	26-T18: SID 4280729	38-V14: SID 4280741
3-G03: SID 4280706	15-S02: SID 4280718	27-T19: SID 4280730	39-V15: SID 4280742
4-G04: SID 4280707	16-T01: SID 4280719	28-T20: SID 4280731	40-V16: SID 4280743
5-G05: SID 4280708	17-T02: SID 4280720	29-V05: SID 4280732	41-V17: SID 4280744
6-G06: SID 4280709	18-T03: SID 4280721	30-V06: SID 4280733	42-V18: SID 4280745
7-G07: SID 4280710	19-T04: SID 4280722	31-V07: SID 4280734	43-V19: SID 4280746
8-G08: SID 4280711	20-T05: SID 4280723	32-V08: SID 4280735	44-V20: SID 4280747
9-G10: SID 4280712	21-T06: SID 4280724	33-V09: SID 4280736	45-V21: SID 4280748
10-GR02: SID 4280713	22-T07: SID 4280725	34-V10: SID 4280737	46-V22: SID 4280749
11-M01: SID 4280714	23-T08: SID 4280726	35-V11: SID 4280738	47-V23: SID 4280750
12-PB01: SID 4280715	24-T09: SID 4280727	36-V12: SID 4280739	

General Notes

- Each Truss Design Drawing (TDD) provided with this sheet has been prepared in conformance with ANSI/TPI 1. Refer to ANSI/TPI 1 Chapter 2 for the responsibilities of all parties involved, which include but are not limited to the responsibilities listed on this sheet, and for the definitions of all capitalized terms referenced in this document.
- TDDs should not be assumed to be to scale.
- The Contractor and Building Designer shall review and approve the Truss Submittal Package.
- The suitability and use of the component depicted on the TDD for any particular building design is the responsibility of the Building Designer.
- The Building Designer is responsible for the anchorage of the truss at all bearing locations as required to resist uplift, gravity and lateral loads, and for all Truss-to-Structural Element connections except Truss-to-Truss connections.
- The Building Designer shall ensure that the supporting structure can accommodate the vertical and/or horizontal truss deflections.
- Unless specifically stated otherwise, each Design assumes trusses will be adequately protected from the environment and will not be used in corrosive environments unless protected using an approved method. This includes not being used in locations where the sustained temperature is greater than 150°F.
- Trusses are designed to carry loads within their plane. Any out-of-plane loads must be resisted by the Permanent Building Stability Bracing.
- Design dead loads must account for all materials, including self-weight. The TDD notes will indicate the min. pitch above which the dead loads are automatically increased for pitch effects.
- Trusses installed with roof slopes less than 0.25/12 may experience (but are not designed for) ponding. The Building Designer must ensure that adequate drainage is provided to prevent ponding.
- Camber is a non-structural consideration and is the responsibility of truss fabricator.

Handling, Installing, Restraint & Bracing

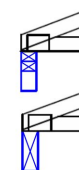
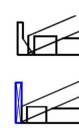
- The Contractor is responsible for the proper handling, erection, restraint and bracing of the Trusses. In lieu of job-specific details, refer to BCSI.
- ANSI/TPI 1 stipulates that for trusses spanning 60' or greater, the Owner shall contract with any Registered Design Professional for the design and inspection of the temporary and permanent truss restraint and bracing. Simpson Strong-Tie is not responsible for providing these services.
- Trusses require permanent lateral restraint to be applied to chords and certain web members (when indicated) at the locations or intervals indicated on the TDD. Web restraints are to be located at mid points, or third points of the member and chord purlins are not to exceed the spacing specified by the TDD. Chords shown without bracing indicated are assumed to be continuously braced by sheathing or drywall. Permanent lateral restraint shall be accomplished in accordance with: standard industry lateral restraint/bracing details in BCSI-B3 or BCSI-B7, supplemental bracing details referenced on the TDD, or as specified in a project-specific truss permanent bracing plan provided by the Building Designer.
- Additional building stability permanent bracing shall be installed as specified in the Construction Documents.
- Special end wall bracing design considerations may be required if a flat gable end frame is used with adjacent trusses that have sloped bottom chords (see BCSI-B3).
- Do not cut, drill, trim, or otherwise alter truss members or plates without prior written approval of an engineer, unless specifically noted on the TDD.
- Piggyback assemblies shall be braced as per BCSI-B3 unless otherwise specified in the Construction Documents.
- For floor trusses, when specified, Strongbacking shall be installed per BCSI-B7 unless otherwise specified in the Construction Documents.
- For IBC 2021 and newer, truss chords without a diaphragm require a project specific bracing design prepared by a registered design professional.

Referenced Standards

ANSI/TPI 1: National Design Standard for Metal Plate Connected Wood Truss Construction, a Truss Plate Institute publication (www.tpinst.org).

BCSI: Guide to Good Practice for Handling, Installing, Restraining & Bracing Metal Plate Connected Wood Trusses, a joint publication of the Truss Plate Institute (www.tpinst.org) and the Structural Building Components Association (www.sbcindustry.com).

Symbols and Nomenclature

- 5x7** Plate size; the first digit is the plate width (perp. to the slots) and the second digit is the plate length (parallel to the slots).
- 5x7-18** -18, -18S5, or -18S6 following the plate size indicates different 18 gauge plate types.
- || = > <** These symbols following the plate size indicate the direction of the plate length (and tooth slots) for square and nearly square plates.
- 10'-3-14** Dimensions are shown in feet-inches-sixteenths (for this example, the dimension is 10'-3 14/16").
-  Joints are numbered left to right, first along the top chord and then along the bottom chord. Mid-panel splice joint numbers are not shown on the drawing. Members are identified using their end joint numbers (e.g., TC 2-3).
-  When this symbol is shown, permanent lateral restraint is required. Lateral restraint may be applied to either edge of the member. See Note 3 under Handling, Installing, Restraint & Bracing for more information.
-  Bearing supports (wall, beam, etc.), locations at which the truss is required to have full bearing. Minimum required bearing width for the given reactions are reported on the TDD. Required bearing widths are based on the truss material and indicated PSI of the support material. The Building Designer is responsible for verifying that the capacity of the support material exceeds the indicated PSI, and for all other bearing design considerations.
-  Truss-to-Truss or Truss-to-Structural Element connection, which require a hanger or other structural connection (e.g., toe-nail) that has adequate capacity to resist the maximum reactions specified in the Reaction Summary. Structural connection type is not limited by type shown on TDD. Toe-nails may be used where hanger type shown where allowed by detail or other connection design information. Design of the Structural Element and the connection of the Truss to a Structural Element is by others.

Note: These symbols are for graphical interpretation only; they are not intended to give any indication of the geometry requirements of the actual item that is represented.

Materials and Fabrication

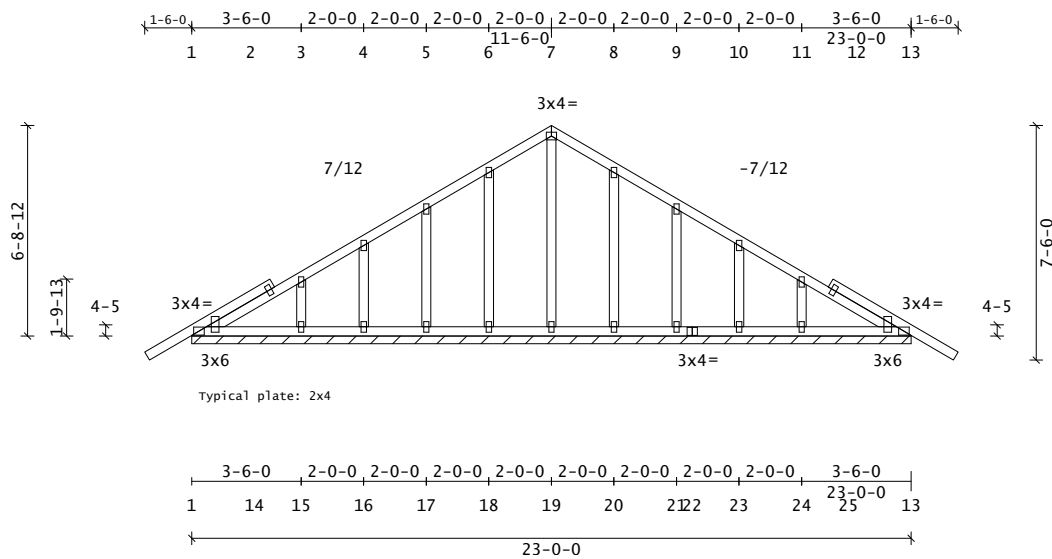
- Design assumes truss is manufactured in accordance with the TDD and the quality criteria in ANSI/TPI 1 Chapter 3, unless more restrictive criteria are part of the contract specifications.
- Unless specifically stated, lumber shall not exceed 19% moisture content at time of fabrication or in service.
- Design is not applicable for use with fire retardant, preservative treated or green lumber unless specifically stated on the TDD.
- Plate type, size, orientation and location indicated are based on the specified design parameters. Larger plate sizes may be substituted in accordance with ANSI/TPI, Section 3.6.3. Plates shall be embedded within ANSI/TPI 1 tolerances on both faces of the truss at each joint, unless noted otherwise.
- Truss plates shall be centered on the joint unless otherwise specified.

DSB-89 Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, a Truss Plate Institute publication (www.tpinst.org).

NDS: National Design Specification for Wood Construction published by American Forest & Paper Association and American Wood Council.

ESR-2762 Simpson Strong-Tie® AS Truss Plates are covered under ESR-2762 published by the International Code Council Evaluation Service (www.icc-es.org).

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	10.0	Lbr	1.25	N/A	1.60
Total	40.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph
Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks	
10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary			
UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Reaction Summary (lb, u.n.o.)			
	Maximum LDF	Minimum LDF	
Horizontal @ Jnt 19:	224	1.60	-224 1.60
Jnt 1 - Jnt 13 (pIf):	88	1.25	27 1.60
Reactions not shown:	< 400	> -150	

Deflection Summary			
Creep (Kcr) = 2.00			
TrussSpan Limit	Actual(in)	Location	
Vert LL L/240	L/999(0.00)	1-14	
Vert DL L/120	L/999(0.00)	1-14	
Vert CR L/180	L/999(0.00)	1-14	
Horz LL 0.75in	(0.00)	@Jt13	
Horz CR 1.25in	(0.00)	@Jt13	
Ohng CR 2L/180	2L/999(0.01)	1- 1	
Ohng CR 2L/180	2L/999(0.01)	13-13	

Member Forces Summary (lb)			
Max CSI in TC PANEL	1 - 1	0.21	
Max CSI in BC PANEL	1 - 14	0.11	
Max CSI in Web	19 - 7	0.07	

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 11-06-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	49	0	0.21
	1- 7	90	132	0.08
	7-13	90	132	0.08
	13-OH	49	0	0.21
BC	1-22	167	102	0.11
	13-22	167	102	0.11
Web	3-15	118	146	0.02
	4-16	88	116	0.02
	5-17	98	120	0.03
	6-18	90	127	0.06
	7-19	29	99	0.07
	8-20	90	127	0.06
	9-21	98	120	0.03
	10-23	88	116	0.02
	11-24	118	146	0.02

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

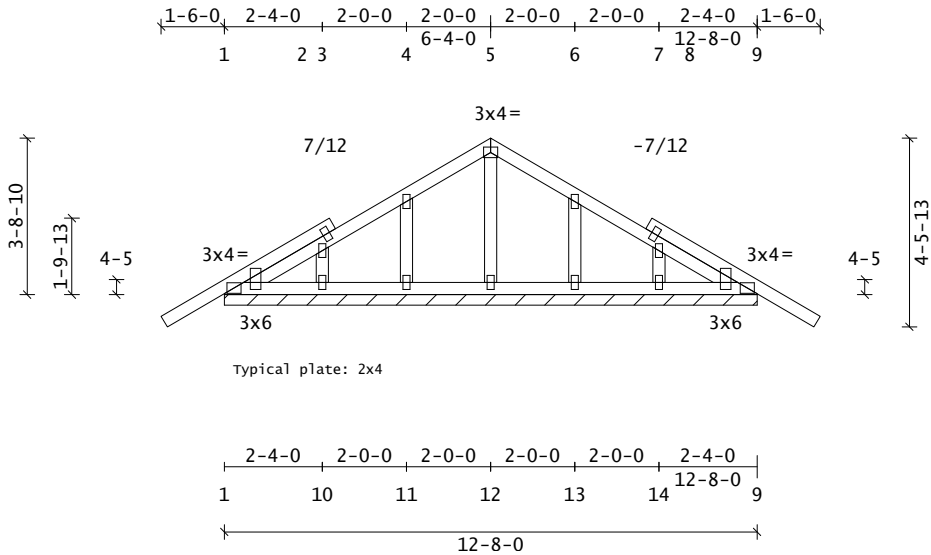
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 23-00-00; connection details to be provided by the Building Designer.



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the 'Important Information & General Notes' page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

Project: P-1038 - Martinez		Truss ID: G02	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280705
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 01 / 26
Project Note(s):		Weight: 71.2 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 1	0.29
Max CSI in BC PANEL	1 - 10	0.17
Max CSI in Web	11 - 4	0.03

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	49	0	0.29
	1- 5	50	63	0.09
	5- 9	50	63	0.09
	9-OH	49	0	0.29
BC	1- 9	99	72	0.17
Web	3-10	78	98	0.02
	4-11	102	137	0.03
	5-12	0	100	0.02
	6-13	102	137	0.03
	7-14	78	98	0.02

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 12:	122 1.60	-122 1.60
Jnt 1 - Jnt 9 (plf):	95 1.25	30 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-04-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

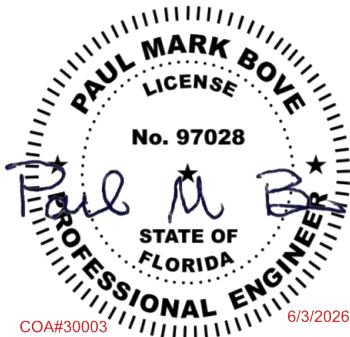
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 12-08-00; connection details to be provided by the Building Designer.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(0.00)	1-10
Vert DL	L/120	L/999(0.00)	1-10
Vert CR	L/180	L/999(0.00)	1-10
Horz LL	0.75in	(0.00)	@Jt 1
Horz CR	1.25in	(0.00)	@Jt 1
Ohng CR	2L/180	2L/999(0.02)	1- 1
Ohng CR	2L/180	2L/999(0.02)	9- 9

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



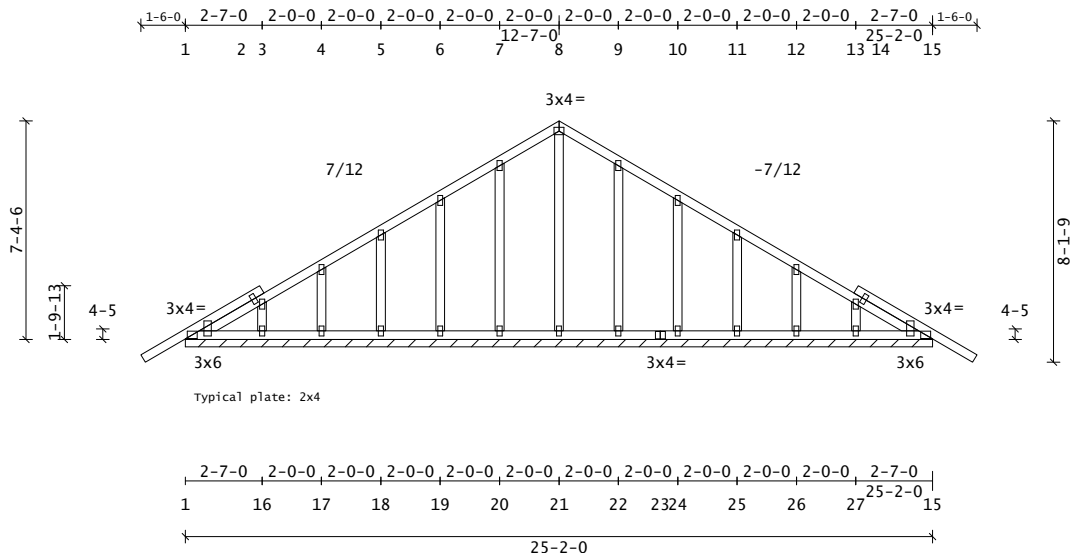
COA#30003

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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0 ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 1	0.21
Max CSI in BC PANEL	1 - 16	0.12
Max CSI in Web	21 - 8	0.10

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	49	0	0.21
	1- 8	91	116	0.11
	8-15	98	153	0.08
	15-OH	49	0	0.21
BC	1-23	179	108	0.12
	15-23	179	108	0.12
Web	3-16	77	109	0.02
	4-17	98	127	0.02
	5-18	93	119	0.02
	6-19	97	119	0.04
	7-20	89	127	0.07
	8-21	40	99	0.10
	9-22	89	127	0.07
	10-24	97	119	0.04
	11-25	93	119	0.02
	12-26	98	127	0.02
	13-27	77	109	0.02

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 21:	242 1.60	-242 1.60
Jnt 1 - Jnt 15 (plf):	87 1.25	26 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-07-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 25-02-00; connection details to be provided by the Building Designer.

Deflection Summary

Creep (Kcr) = 2.00	TrussSpan Limit	Actual(in)	Location
Vert LL	L/240	L/999(0.00)	27-15
Vert DL	L/120	L/999(0.00)	27-15
Vert CR	L/180	L/999(0.00)	27-15
Horz LL	0.75in	(0.00)	@Jt 1
Horz CR	1.25in	(0.00)	@Jt 1
Ohng CR	2L/180	2L/999(0.01)	1- 1
Ohng CR	2L/180	2L/999(0.01)	15-15

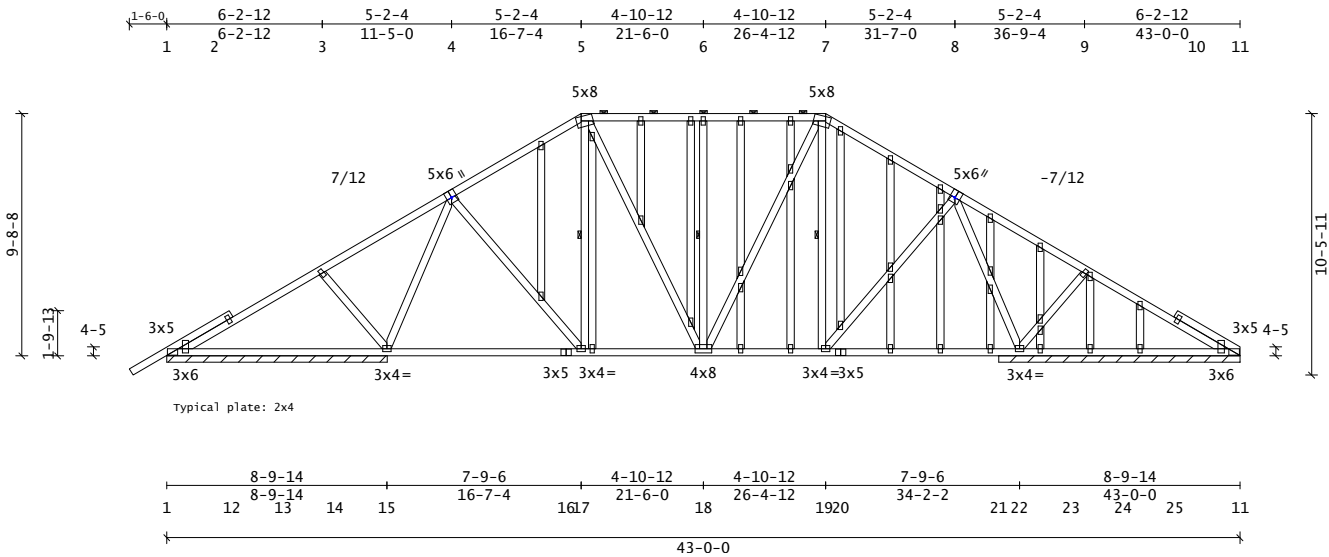
Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



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TC Plate offsets (X,Y): J4(-0-04,0-07), J8(0-04,0-07)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00	o.c.	Plies:	1	
Repetitive	Member	Increase:	Yes		
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	3 - 4	0.43
Max CSI in BC PANEL	15 - 16	0.57
Max CSI in Web	15 - 4	0.91

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	49	0	0.21
	1- 3	146	131	0.38
	3- 4	247	115	0.43
	4- 5	336	692	0.38
	5- 6	375	617	0.34
	6- 7	375	617	0.34
	7- 8	339	688	0.38
	8- 9	238	3	0.42
	9-11	62	140	0.37
BC	1-12	207	176	0.18
	11-25	61	28	0.26
	12-13	207	176	0.06
	13-14	207	176	0.09
	14-15	207	176	0.48
	15-16	265	118	0.57
	16-17	265	118	0.50
	17-18	522	56	0.52
	18-19	519	18	0.43
	19-20	265	0	0.38
	20-21	265	0	0.44
	21-22	265	0	0.44
	22-23	61	28	0.04
	23-24	61	28	0.04
	24-25	61	28	0.09
Web	3-15	223	284	0.09
	4-15	344	1101	0.91
	4-17	404	3	0.09
	5-17	72	161	0.06
	5-18	239	74	0.22
	6-18	174	312	0.13
	7-18	246	112	0.21
	7-19	100	169	0.07
	8-19	390	47	0.09
	8-22	344	1088	0.90
	9-22	226	286	0.09

Reaction Summary (lb, u.n.o.)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
15	8-09-14	Pin	1393	1.25	-419	1.60	8-10-00	02-14	Wall SPF 425~
22	34-02-02	HRoll	1168	1.25	-519	1.60	9-08-00	02-11	Wall SPF 425~
Horizontal @ Jnt	1:	349	1.60	-309	1.60				
Jnt 1 - Jnt 15 (plf):	47	1.25	13	1.60					
Jnt 21 - Jnt 11 (plf):	57	1.25	0						
Reactions not shown:	< 400		> -150						

~ Includes adjustment for Bearing Area Factor, Cb.

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 21-06-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

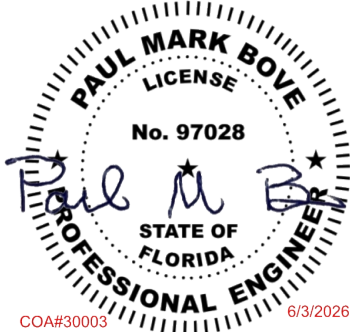
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 43-00-00; connection details to be provided by the Building Designer.
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.09) 15-17
Vert DL	L/120	L/999(-0.09) 15-17
Vert CR	L/180	L/999(-0.18) 15-17
Horz LL	0.75in	(0.01) @Jt11
Horz CR	1.25in	(0.02) @Jt11
Ohng CR	2L/180	2L/999(0.01) 1- 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Purlins:
Mbr Start End Spacing
TC 17-04-05 25-07-11 2-00-00
BC 03-08 0 8-06-00
Web(s): Continuous Lateral Restraint;
Sgl: 17-5, 18-6, 19-7



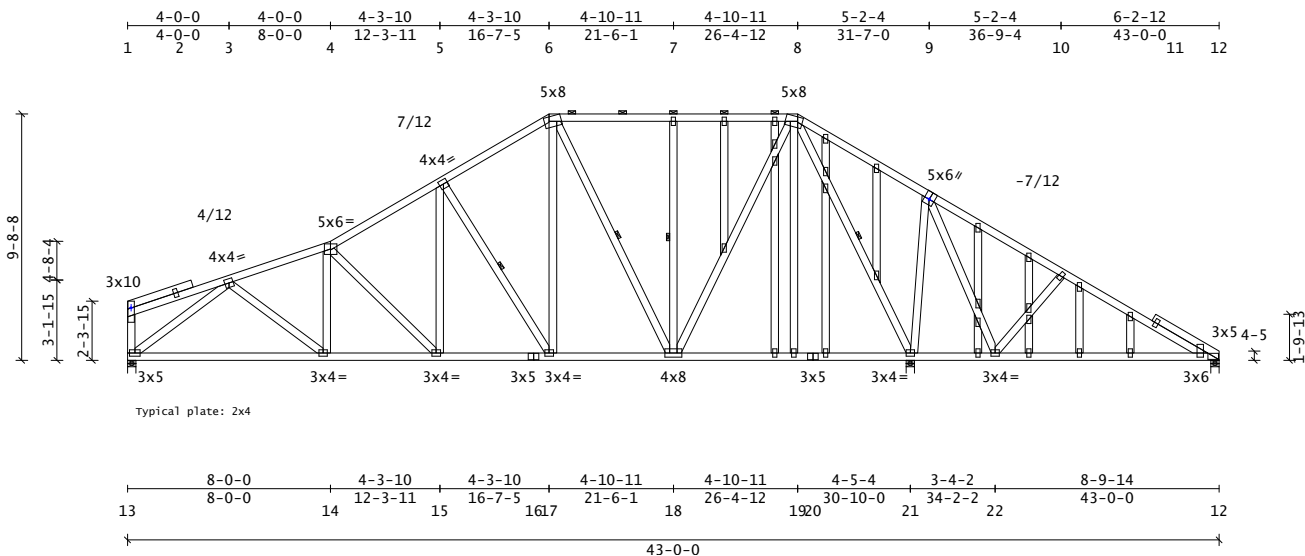
COA#30003

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): J1(0,-1-13), J9(0-04,0-07)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF Live	Dead	Load	Duration	Factors
TC 20.0	10.0	Live	Snow	Wind
BC 0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A 1.60
OH Soffit	2.0			
Spacing:	2-00-00 o.c.	Plies:	1	
Repetitive Member Increase:	Yes			
Green Lbr:	No	Temp:	T ≤ 100°	
Treated Lbr:	No	Incised Lbr:	No	
Seasoned Lbr:	Yes	Wet Service:	No	

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
13	01-12	Pin	1169	1.25	-315	1.60	04-00	01-08	Wall SPF 531~
21	30-10-00	HRo11	2084	1.25	-622	1.60	04-00	02-10	Wall SPF 531~
12	42-10-00	HRo11	272	1.25	-61	1.60	04-00	01-08	Wall SPF 531~
Horizontal @ Jnt 13:			302	1.60	-334	1.60			

~ Includes adjustment for Bearing Area Factor, Cb.

Deflection Summary

Creep (Kcr) =	2.00
TrussSpan Limit	Actual(in) Location
Vert LL	L/240 L/999(-0.14) 13-14
Vert DL	L/120 L/999(-0.16) 13-14
Vert CR	L/180 L/999(-0.31) 13-14
Horz LL	0.75in (0.03) @Jt21
Horz CR	1.25in (0.05) @Jt21

Member Forces Summary (lb)

Max CSI in TC PANEL	8 - 9	0.56
Max CSI in BC PANEL	13 - 14	0.71
Max CSI in Web	8 - 21	0.78

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 21-06-01 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

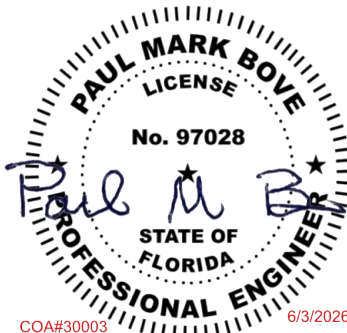
- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 43-00-00; connection details to be provided by the Building Designer.
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Purlins:
Mbr Start End Spacing
TC 17-04-05 25-07-11 2-00-00
Web(s): Continuous Lateral Restraint;
Sgl: 5-17, 6-18, 18-7, 8-21

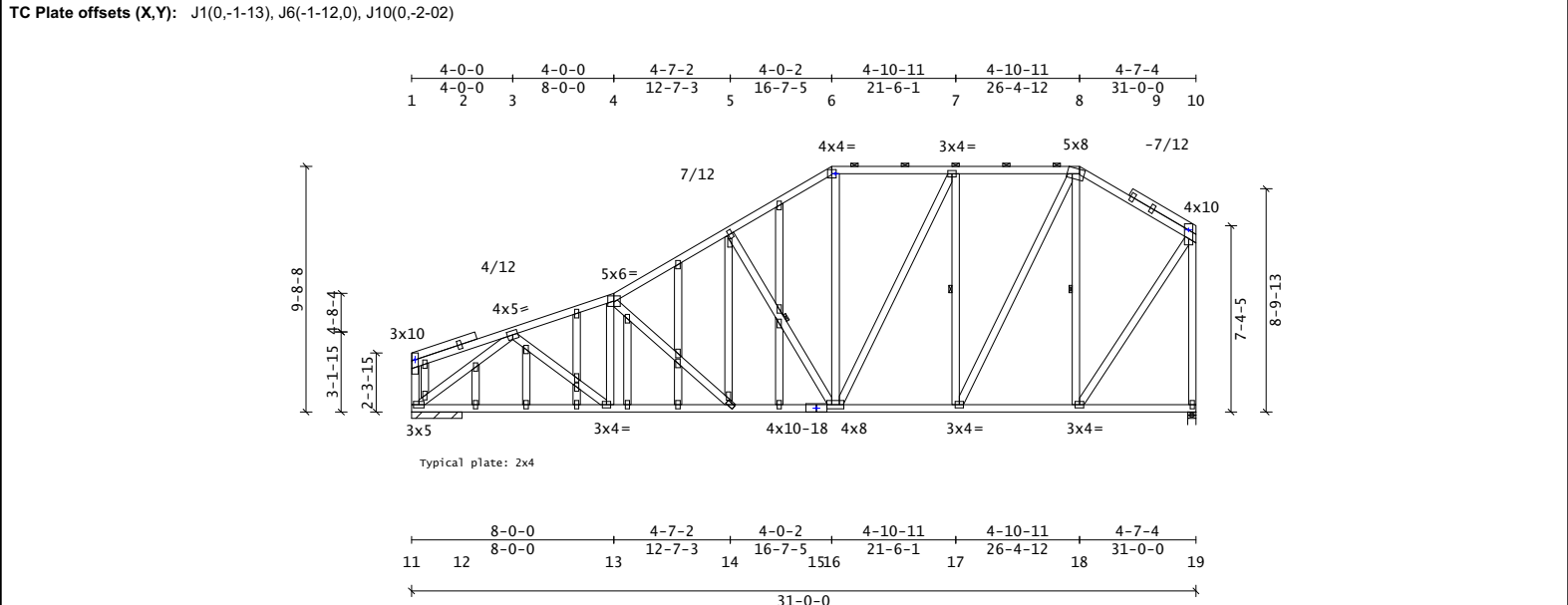


COA#30003

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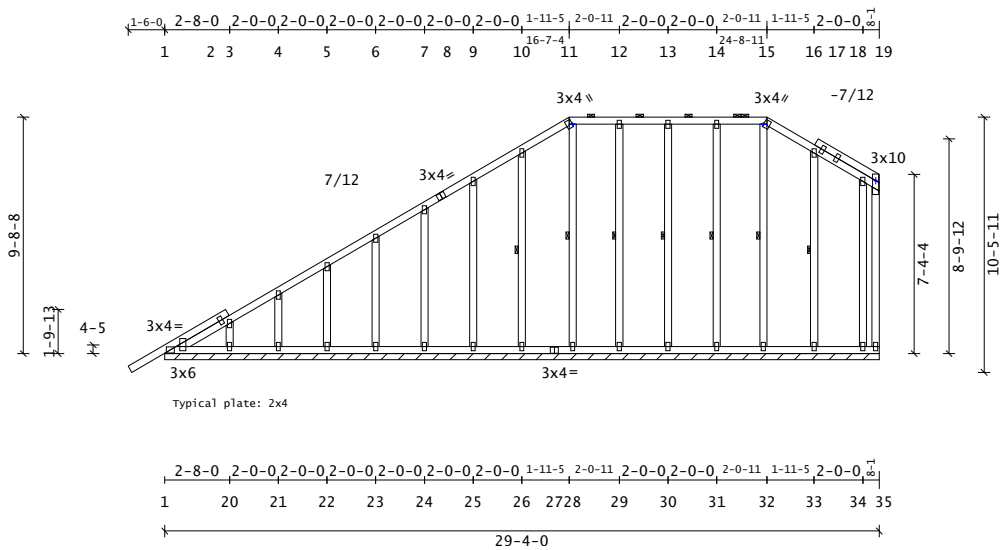
Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com



BC Plate offsets (X,Y): J15(0,0-04)	FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 10.0 Lbr 1.25 N/A 1.60 Total 40.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 5.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TRCLL Maintenance: No 300 lb TRCLL/BCLL Person Loading: No 2000 lb Moving TRCLL Office: No Partial Length TRCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary UTC 2x4 SYP #2 LTC 2x4 SYP #2 TC 2x4 SYP #2 BC 2x4 SYP DSS Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 19 30-10-04 HRoll 1228 1.25 -359 1.60 04-00 01-09 Wall SPF 531~ 11 01-12 Pin 1014 1.25 -356 1.60 2-00-00 02-00 Wall SPF 425~ Horizontal @ Jnt 11: 427 1.60 -293 1.60 Jnt 11 - Jnt 12 (plf): 121 1.25 0 Reactions not shown: < 400 > -150 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/908(-0.38) 13-14 Vert DL L/120 L/639(-0.54) 13-14 Vert CR L/180 L/375(-0.92) 13-14 Horz LL 0.75in (0.02) @Jt19 Horz CR 1.25in (0.03) @Jt19
Member Forces Summary (lb) Max CSI in TC PANEL 4 - 5 0.68 Max CSI in BC PANEL 13 - 14 0.70 Max CSI in Web 19 - 10 0.96	Loads Summary • User-defined unbalanced Top Chord Live Loads (TRCLL) occurring at 21-06-01 are based on 1.00 full and 0.00 reduced load factors. • See Loadcase Report for load combinations and additional details.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Purlins: Mbr Start End Spacing TC 17-04-05 25-07-11 2-00-00 Web(s): Continuous Lateral Restraint; Sgl: 5-16, 17-7, 18-8
Design Notes • Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. • Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.	Installation Notes • If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents. • The maximum rake overhang length is 12.0". • Gable requires 7/16" OSB sheathing on front from 0-00-00 to 31-00-00; connection details to be provided by the Building Designer. • Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBACE. Alternatively, see D-WEBREINFORCE.	
Member Forces Summary (lb) Mbr Jnt-Jnt Tens Comp CSI TC 1- 3 59 27 0.26 3- 4 557 1698 0.43 4- 5 512 1418 0.68 5- 6 528 1280 0.68 6- 7 491 1030 0.45 7- 8 434 837 0.43 8-10 326 658 0.35 BC OH- 1 3 0 0.00 OH-11 0 0 0.00 11-12 1289 536 0.35 12-13 1289 536 0.35 13-14 1566 516 0.70 14-15 1207 431 0.68 15-16 1207 431 0.66 16-17 837 311 0.64 17-18 516 207 0.15 18-19 113 73 0.12 19-OH 0 0 0.00 10-OH 5 0 0.00 Web 1-11 68 99 0.05 3-11 509 1649 0.70 3-13 432 13 0.09 4-13 125 87 0.03 4-14 211 482 0.35 5-16 225 344 0.11 6-16 416 139 0.22 7-16 502 149 0.29 7-17 281 705 0.29 8-17 707 229 0.44 8-18 244 613 0.25 10-18 917 237 0.27 10-19 380 1195 0.96		

TC Plate offsets (X,Y): J11(-1-12,0), J15(1-12,0), J19(0,-1-13)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCCL Office:	No
Partial Length TCCL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 26:	472	1.60
Jnt 1 - Jnt 35 (plf):	83	1.25
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 20-08-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

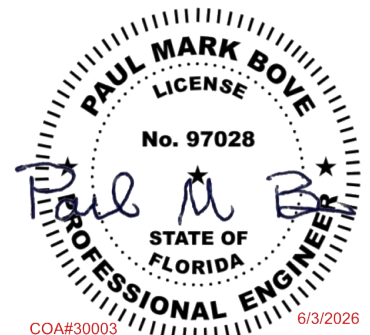
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 29-04-00; connection details to be provided by the Building Designer.
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00	
TrussSpan Limit	Actual(in) Location
Vert LL	L/240 L/999(0.00) 1-20
Vert DL	L/120 L/999(0.00) 1-20
Vert CR	L/180 L/999(0.00) 1-20
Horz LL	0.75in (0.01) @Jt 1
Horz CR	1.25in (0.01) @Jt 1
Ohng CR	2L/180 2L/999(0.01) 1- 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Purlins:
Mbr Start End Spacing
TC 17-04-05 23-11-11 2-00-00
Web(s): Continuous Lateral Restraint;
Sgl: 26-10, 28-11, 29-12, 30-13, 31-14, 32-15, 33-16



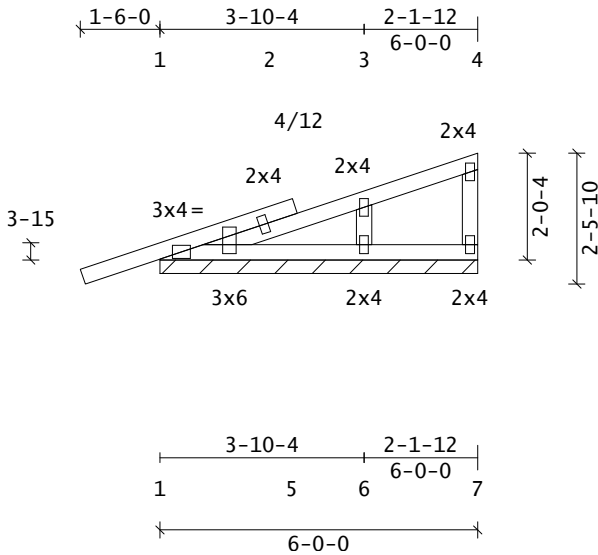
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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez		Truss ID: G08	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280711
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 2	Date: 06/01/26
Project Note(s):		Weight: 28.7 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
 Exposure Category: C
 Bldg Dims: L = 61.0 ft B = 59.7 ft
 MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
 Bldg Type: Enclosed
 Wind DL (psf): TC = 5.0 BC = 5.0
 End Vertical Exposed: L = Yes R = Yes
 C&C Location: End Zone | a = 5-11-10
 Design: MWFRS/C&C | Reactions: MWFRS
 Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
 Rotation: +/- 10 degrees, u.n.o.

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 1	0.36
Max CSI in BC PANEL	1 - 5	0.07
Max CSI in Web	7 - 4	0.07

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	32	0	0.36
	1- 4	43	56	0.12
BC	1- 7	86	73	0.07
Web	3- 6	94	147	0.05
	4- 7	26	55	0.07

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 5:	103 1.60	-15 1.60
Jnt 1 - Jnt 7 (plf):	95 1.25	30 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-00-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

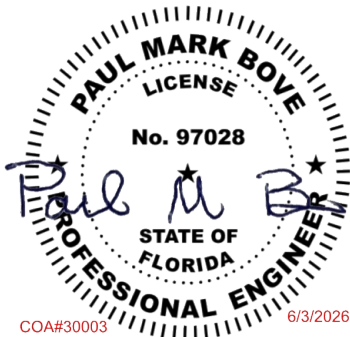
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 0-00-00 to 6-00-00; connection details to be provided by the Building Designer.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	6- 7
Vert DL	L/120	L/999(-0.00)	6- 7
Vert CR	L/180	L/999(-0.00)	6- 7
Horz LL	0.75in	(0.00)	@Jt 1
Horz CR	1.25in	(0.00)	@Jt 1
Ohng CR	2L/180	2L/999(0.03)	1- 1

Bracing Summary

Top Chord: Structural Wood Sheathing
 Bot Chord: Gypsum Board Sheathing
 Web(s): None;

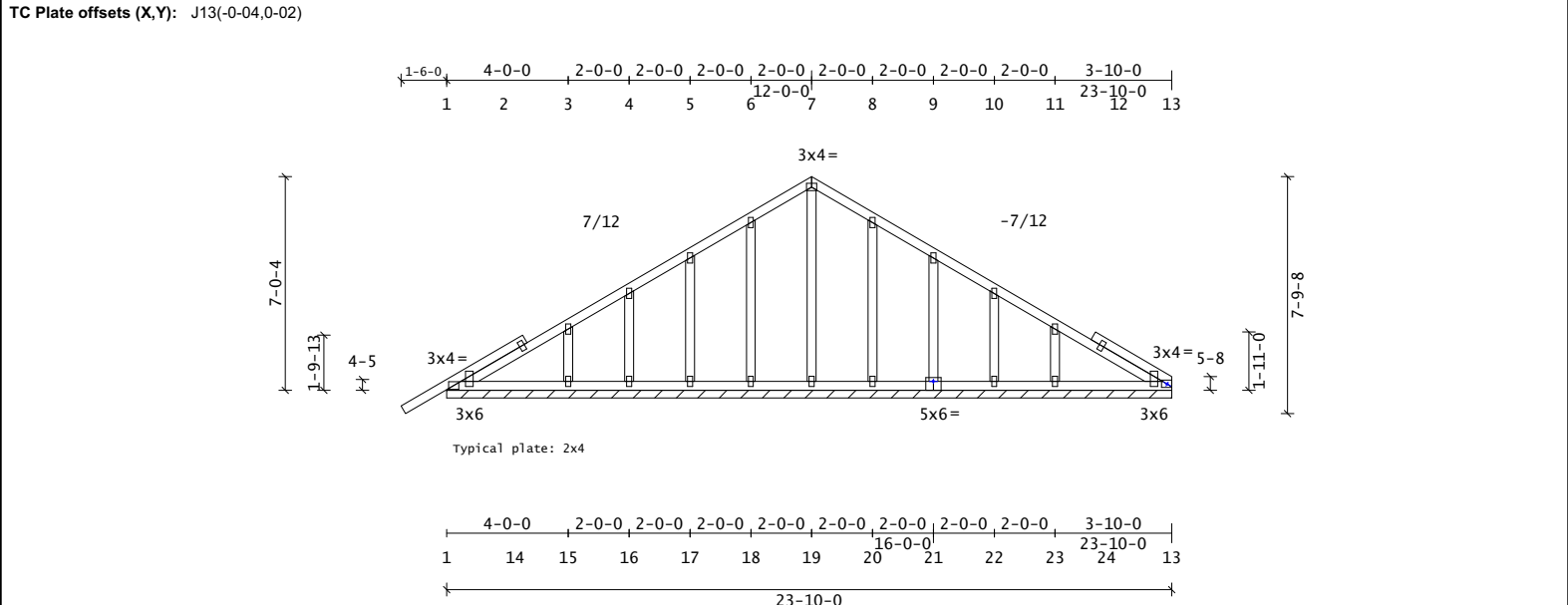


COA#30003

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



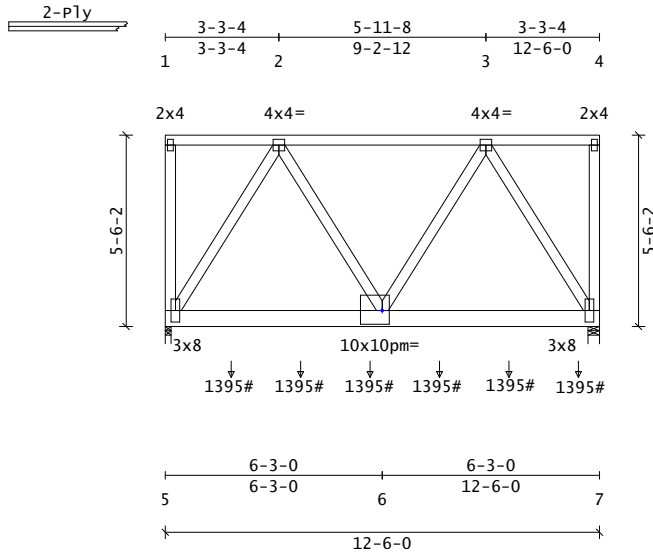
Component Solutions
 Truss Studio V
 2026.3.0.65
 Helpdesk: 1-866-252-8606
 CSHelp@strongtie.com



BC Plate offsets (X,Y): J21(0,-1-00)		Roof Snow Load (Pf) = N/A		Wind Speed (Vult) = 140 mph		Additional Live Load Checks	
FBC-2023 ASCE7-22 TPI 1-2014		Risk Category: II (I = 1.00)		Exposure Category: C		10 psf Non-Concurrent BCLL:	
PSF Live Dead Load Duration Factors		Bldg Dims: L = 61.0 ft B = 59.7 ft		MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00		20 psf BCLL Limited Storage:	
TC 20.0 10.0 Live Snow Wind		Bldg Type: Enclosed		Wind DL (psf): TC = 5.0 BC = 5.0		200 lb Moving BCLL Access:	
BC 0.0 10.0 Lbr 1.25 N/A 1.60		End Vertical Exposed: L = Yes R = Yes		C&C Location: End Zone a = 5-11-10		300 lb Moving TCLL Maintenance:	
Total 40.0 Plt 1.25 N/A 1.60		Design: MWFRS/C&C Reactions: MWFRS		Net Uplift: 0.0 psf		300 lb TCLL/BCLL Person Loading:	
OH Soffit 2.0						2000 lb Moving TCLL Office:	
Spacing: 2-00-00 o.c. Plies: 1						Partial Length TCLL:	
Repetitive Member Increase: Yes						Plate Tolerance Checks	
Green Lbr: No Temp: T ≤ 100°						Fabrication: 20% (Cq = 0.80), u.n.o.	
Treated Lbr: No Incised Lbr: No						Rotation: +/- 10 degrees, u.n.o.	
Seasoned Lbr: Yes Wet Service: No							

Material Summary		Reaction Summary (lb, u.n.o.)		Deflection Summary	
UTC 2x4 SYP #2		Maximum LDF Minimum LDF		Creep (Kcr) = 2.00	
LTC 2x4 SYP #2		Horizontal @ Jnt 19: 251 1.60 -211 1.60		TrussSpan Limit Actual(in) Location	
BC 2x4 SYP #2		Jnt 1 - Jnt 13 (plf): 84 1.25 25 1.60		Vert LL L/240 L/999(-0.00) 1-14	
Webs 2x4 SYP #2		Reactions not shown: < 400 > -150		Vert DL L/120 L/999(-0.00) 24-13	
Member Forces Summary (lb)		Loads Summary		Vert CR L/180 L/999(-0.00) 24-13	
Max CSI in TC PANEL 1 - 1 0.21		• User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-00-00 are based on 1.00 full and 0.00 reduced load factors.		Horz LL 0.75in (0.01) @Jt13	
Max CSI in BC PANEL 24 - 13 0.11		• See Loadcase Report for load combinations and additional details.		Horz CR 1.25in (0.01) @Jt13	
Max CSI in Web 19 - 7 0.08		Design Notes		Ohng CR 2L/180 2L/999(0.01) 1- 1	
Mbr Jnt-Jnt Tens Comp CSI		• Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.		Bracing Summary	
TC OH- 1 49 0 0.21		• Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.		Top Chord: Structural Wood Sheathing	
1- 7 93 138 0.10		Installation Notes		Bot Chord: Gypsum Board Sheathing	
7-13 91 138 0.11		• If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.		Web(s): None;	
BC 1-21 176 104 0.10		• The maximum rake overhang length is 12.0".			
13-21 152 75 0.11		• Gable requires 7/16" OSB sheathing on front from 0-00-00 to 23-10-00; connection details to be provided by the Building Designer.			
Web 3-15 135 169 0.03					
4-16 81 109 0.02					
5-17 99 122 0.04					
6-18 90 127 0.07					
7-19 36 93 0.08					
8-20 90 127 0.06					
9-21 99 122 0.04					
10-22 78 102 0.02					
11-23 145 174 0.03					

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): J6(-2-09,0-05)

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	2		
Repetitive Member Increase:	No				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x6	SYP	DSS
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	2 - 3	0.25
Max CSI in BC PANEL	5 - 6	0.66
Max CSI in Web	2 - 6	0.35

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	0	0	0.00
	1- 2	84	54	0.22
	2- 3	1000	3348	0.25
	3- 4	84	54	0.22
	4-OH	0	0	0.00
BC	OH- 5	0	0	0.00
	5- 6	1791	662	0.66
	6- 7	1791	603	0.66
	7-OH	0	0	0.00
Web	1- 5	31	68	0.21
	2- 5	1000	3446	0.32
	2- 6	3102	848	0.35
	3- 6	3102	848	0.35
	3- 7	1060	3446	0.32
	4- 7	31	67	0.21

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
5	01-12	Pin	4177	1.25	-1225	1.60	02-00	02-15*	Wall SPF 478~
7	12-04-04	HRoll	5061	1.25	-1469	1.60	04-00	03-08	Wall SPF 478~
Horizontal @ Jnt 5:	238	1.60	-238	1.60					
Horizontal @ Jnt 7:	238	1.60	-238	1.60					
* Req Width > Brg Width; enhancement required.									
~ Includes adjustment for Bearing Area Factor, Cb.									
Jnt	Bearing Area	Required Area	Required Truss Width	Bearing Enhancement					
5	6.00 in²	8.74 in²	4.37 in (3-ply)	Per Building Designer					

Loads Summary

· User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-03-00 are based on 1.00 full and 0.00 reduced load factors.
· See Loadcase Report for load combinations and additional details.

Concentrated Loads (lbs) based on Max and Min:

Mbr	Location	Max	Min	Dir	DL/TL	Angle	Connection	Description
BC	1-10-12	1395	-385	V	0.48	90°	Hanger	T01
BC	3-10-12	1395	-385	V	0.48	90°	Hanger	T01
BC	5-10-12	1395	-385	V	0.48	90°	Hanger	T01
BC	7-10-12	1395	-385	V	0.48	90°	Hanger	T01
BC	9-10-12	1395	-385	V	0.48	90°	Hanger	T01
BC	11-10-12	1395	-385	V	0.48	90°	Hanger	T01

2-PLY TRUSS Fastener Schedule

Fasten each ply to the adjacent ply as follows(rows staggered):
TC 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.
BC 2x6, 2-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 12.0" o.c.**
WB 2x4, 1-row(s) of 10d Nails (0.120" dia. x 2-7/8" min.) @ 9.0" o.c.
** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated below (except where approved hangers are used with fasteners that transfer the load to all plies):

BC: 1-10-12, 3 ; BC: 3-10-12, 3 ; BC: 5-10-12, 3 ; BC: 7-10-12, 3
BC: 9-10-12, 3 ; BC: 11-10-12, 3

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.

Installation Notes

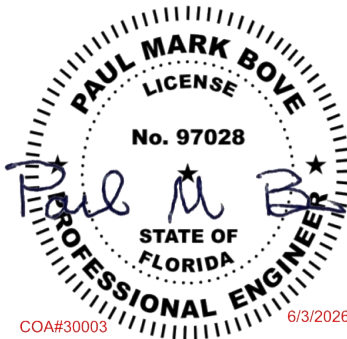
- Less than 0.25/12 pitch requires adequate drainage to prevent ponding.
- This truss is not symmetric - proper orientation is critical.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan Limit	Actual(in)	Location	
Vert LL	L/240	L/999(-0.11)	5- 6
Vert DL	L/120	L/999(-0.08)	5- 6
Vert CR	L/180	L/787(-0.19)	5- 6
Horz LL	0.75in	(0.00) @Jt 7	
Horz CR	1.25in	(0.01) @Jt 7	

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



COA#30003

6/3/2026

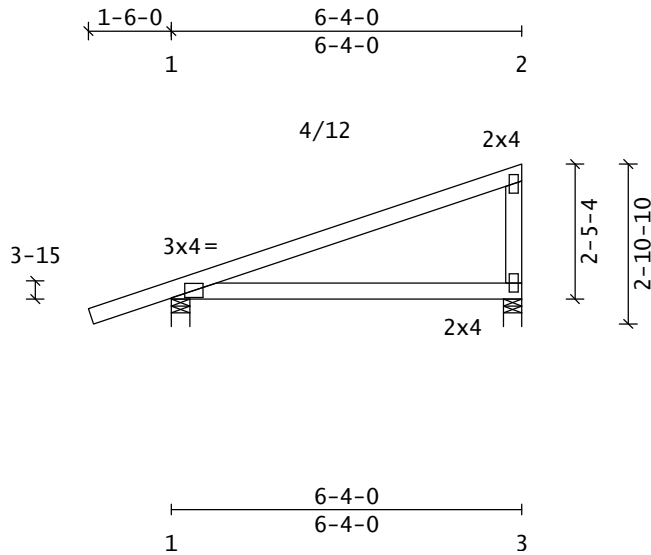
NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez		Truss ID: M01	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280714
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 17	Date: 06 / 03 / 26
Project Note(s):		Weight: 24.0 lb	Page: 1 of 1

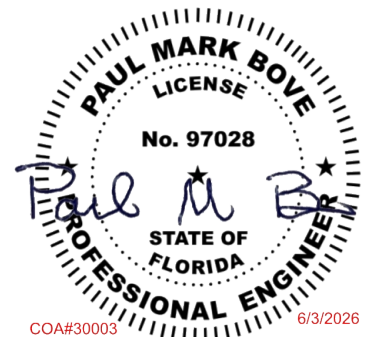
TC Plate offsets (X,Y): None



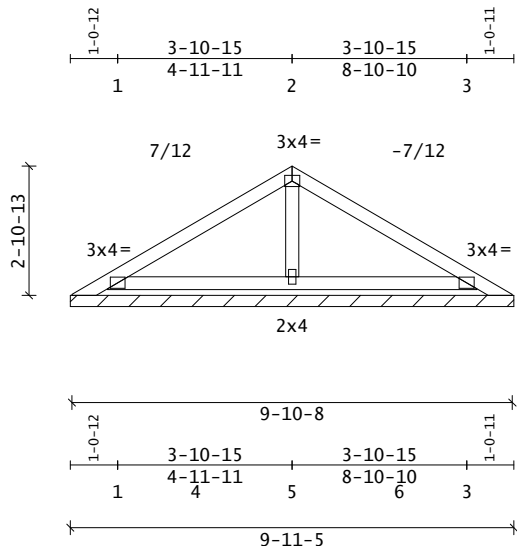
BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 10.0 Lbr 1.25 N/A 1.60 Total 40.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100' Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No		Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 5.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
--	--	----------------------------------	--	--

Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2 Member Forces Summary (lb) Max CSI in TC PANEL 1 - 2 0.56 Max CSI in BC PANEL 1 - 3 0.39 Max CSI in Web 3 - 2 0.13 Mbr Jnt-Jnt Tens Comp CSI TC OH- 1 32 0 0.31 1- 2 50 53 0.56 2-OH 0 2 0.00 BC 1- 3 0 20 0.39 3-OH 0 0 0.00 Web 2- 3 85 169 0.13		Reaction Summary (lb) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 1 02-00 Pin 364 1.25 -117 1.60 04-00 01-08 Wall SPF 531~ 3 6-02-04 HRoll 246 1.25 -69 1.60 04-00 01-08 Wall SPF 531~ Horizontal @ Jnt 1: 114 1.60 -26 1.60 ~ Includes adjustment for Bearing Area Factor, Cb. Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-04-00 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details. Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.06) 1- 3 Vert DL L/120 L/976(-0.07) 1- 3 Vert CR L/180 L/553(-0.13) 1- 3 Horz LL 0.75in (0.02) @Jt 1 Horz CR 1.25in (0.03) @Jt 1 Ohng CR 2L/180 2L/999(0.03) 1- 1 Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
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TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	1.0	Lbr	1.25	N/A	1.60
Total	31.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph
Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks	
10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

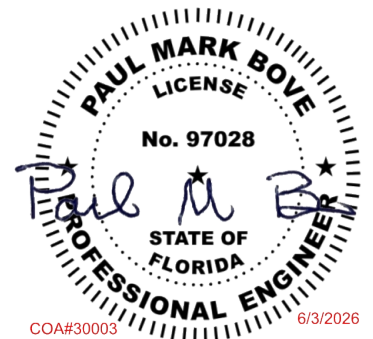
Material Summary				
TC	2x4	SYP	#2	
BC	2x4	SYP	#2	
Webs	2x4	SYP	#2	
Member Forces Summary (lb)				
Max CSI in TC PANEL	1 - 2	0.19		
Max CSI in BC PANEL	1 - 4	0.21		
Max CSI in Web	5 - 2	0.07		
Reaction Summary (lb, u.n.o.)				
TC	1 - 2	109	34	0.19
BC	1 - 4	74	93	0.21
Web	2 - 5	161	291	0.07

Reaction Summary (lb, u.n.o.)
Maximum LDF Minimum LDF
Horizontal @ Jnt 5: 73 1.60 -73 1.60
Jnt 1 - Jnt 3 (plf): 62 1.25 31 1.60
Reactions not shown: < 400 > -150

Loads Summary
· User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 4-11-11 are based on 1.00 full and 0.00 reduced load factors.
· See Loadcase Report for load combinations and additional details.

Design Notes
· Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
· Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary			
Creep (Kcr)	2.00		
TrussSpan Limit	Actual(in)	Location	
Vert LL	L/240	L/999(-0.01)	1- 4
Vert DL	L/120	L/999(-0.00)	1- 4
Vert CR	L/180	L/999(-0.01)	1- 4
Horz LL	0.75in	(0.01) @Jt 3	
Horz CR	1.25in	(0.01) @Jt 3	
Bracing Summary			
Top Chord:	Structural Wood Sheathing		
Bot Chord:	Gypsum Board Sheathing		
Web(s):	None;		



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SIMPSON

Strong-Tie

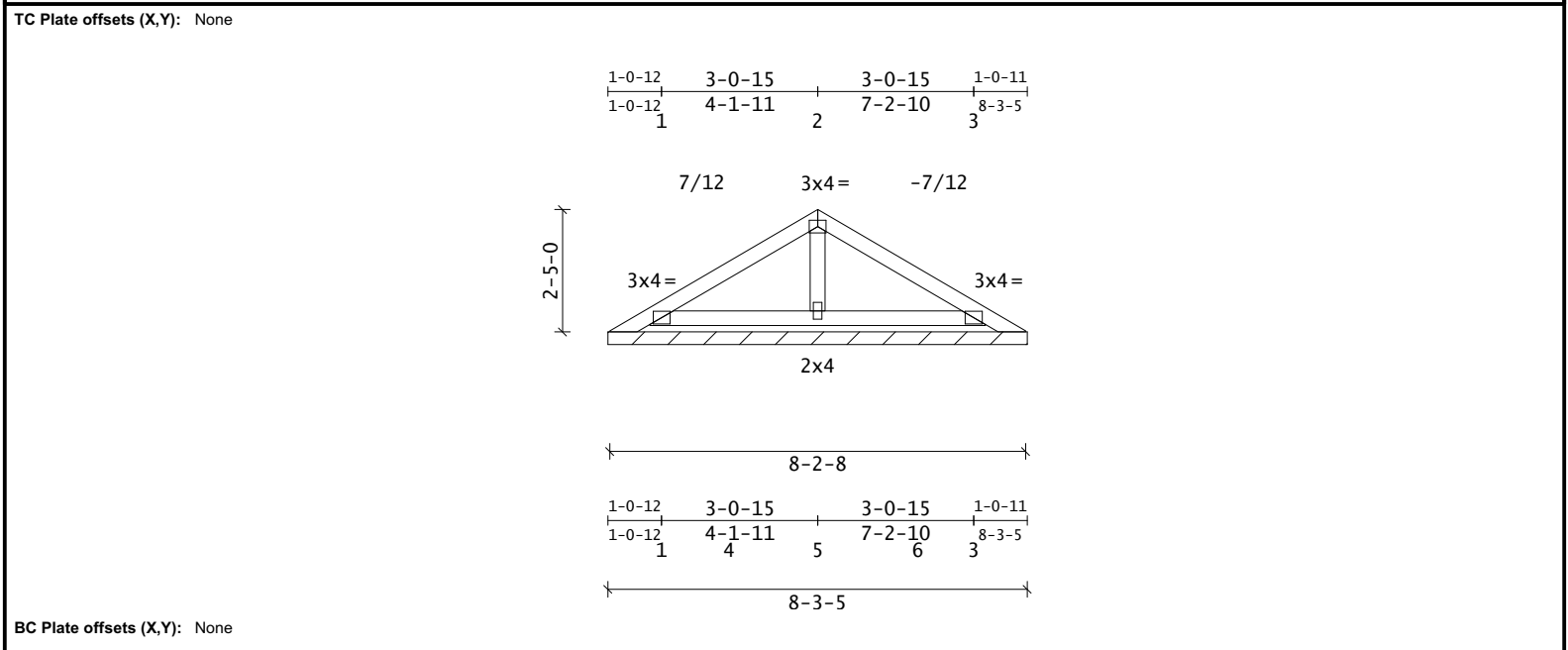
Component Solutions

Truss Studio V

2026.3.0.65

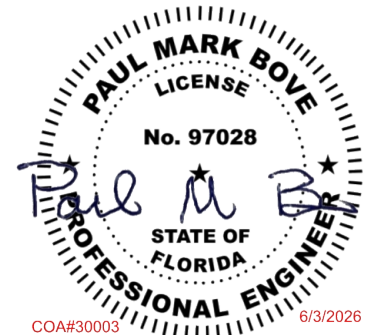
Helpdesk: 1-866-252-8606

CSHelp@strongtie.com



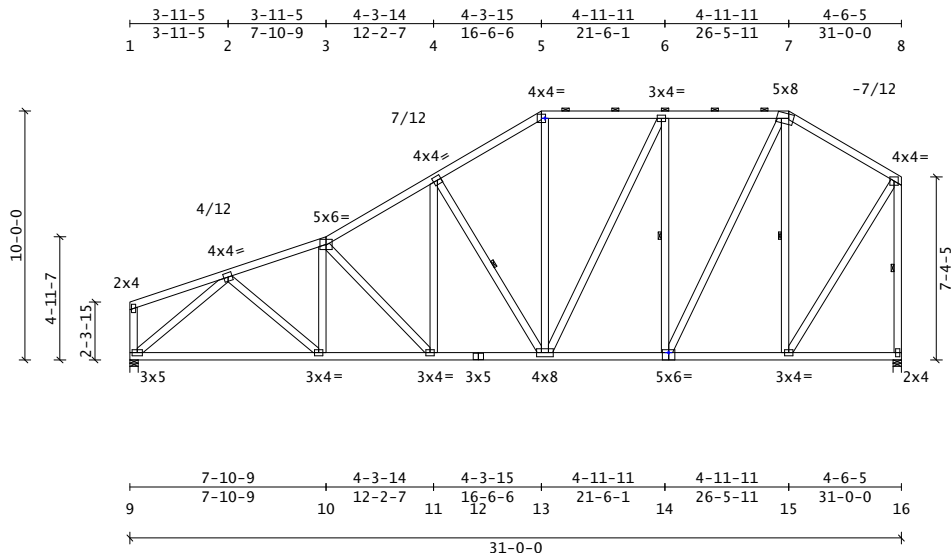
FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h) = 18.0 ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Maximum LDF Minimum LDF Horizontal @ Jnt 5: 57 1.60 -57 1.60 Jnt 1 - Jnt 3 (plf): 61 1.25 31 1.60 Reactions not shown: < 400 > -150	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 1- 4 Vert DL L/120 L/999(-0.00) 1- 4 Vert CR L/180 L/999(-0.01) 1- 4 Horz LL 0.75in (0.00) @Jt 3 Horz CR 1.25in (0.00) @Jt 3	
Member Forces Summary (lb) Max CSI in TC PANEL 1 - 2 0.13 Max CSI in BC PANEL 1 - 4 0.14 Max CSI in Web 5 - 2 0.05	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 4-01-11 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;	
Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.			

SID: 0004280719
TID: 318742
Date: 06 / 01 / 26
Page: 1 of 1



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): J5(-1-12,0)



BC Plate offsets (X,Y): J14(0,-1-00)

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00	o.c.	Plies:	1	
Repetitive	Member	Increase:	Yes		
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	5 - 6	0.45
Max CSI in BC PANEL	9 - 10	0.69
Max CSI in Web	16 - 8	0.79

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	2	0	0.00
	1- 2	62	29	0.22
	2- 3	515	1679	0.38
	3- 4	525	1521	0.38
	4- 5	495	1196	0.37
	5- 6	469	972	0.45
	6- 7	427	830	0.44
	7- 8	319	635	0.32
	8-OH	4	0	0.00
BC	OH- 9	0	0	0.00
	9-10	1206	512	0.69
	10-11	1562	497	0.63
	11-12	1273	439	0.32
	12-13	1273	439	0.39
	13-14	839	314	0.37
	14-15	497	207	0.32
	15-16	362	211	0.17
	16-OH	0	0	0.00
Web	1- 9	69	102	0.07
	2- 9	460	1613	0.72
	2-10	465	35	0.10
	3-10	101	154	0.05
	3-11	162	414	0.29
	4-11	376	85	0.08
	4-13	277	569	0.19
	5-13	340	106	0.17
	6-13	375	137	0.28
	6-14	259	590	0.26
	7-14	758	235	0.48
	7-15	254	676	0.29
	8-15	925	242	0.30
	8-16	378	1201	0.79

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
9	01-12	Pin	1239	1.25	-343	1.60	04-00	01-09	Wall SPF 531~
16	30-10-04	HRoll	1240	1.25	-358	1.60	04-00	01-09	Wall SPF 531~
Horizontal @ Jnt 9:	440	1.60	-305	1.60					
Horizontal @ Jnt 16:	440	1.60	-305	1.60					

~ Includes adjustment for Bearing Area Factor, Cb.

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 21-06-01 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

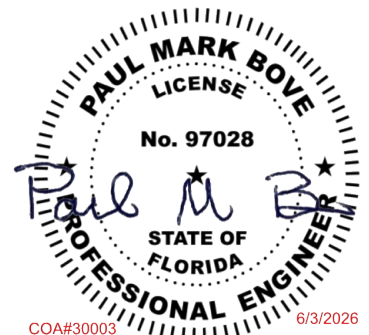
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan	Limit	Actual(in)
Vert LL	L/240	L/999(-0.14)
Vert DL	L/120	L/999(-0.15)
Vert CR	L/180	L/999(-0.29)
Horz LL	0.75in	(0.02) @Jt16
Horz CR	1.25in	(0.05) @Jt16

Bracing Summary

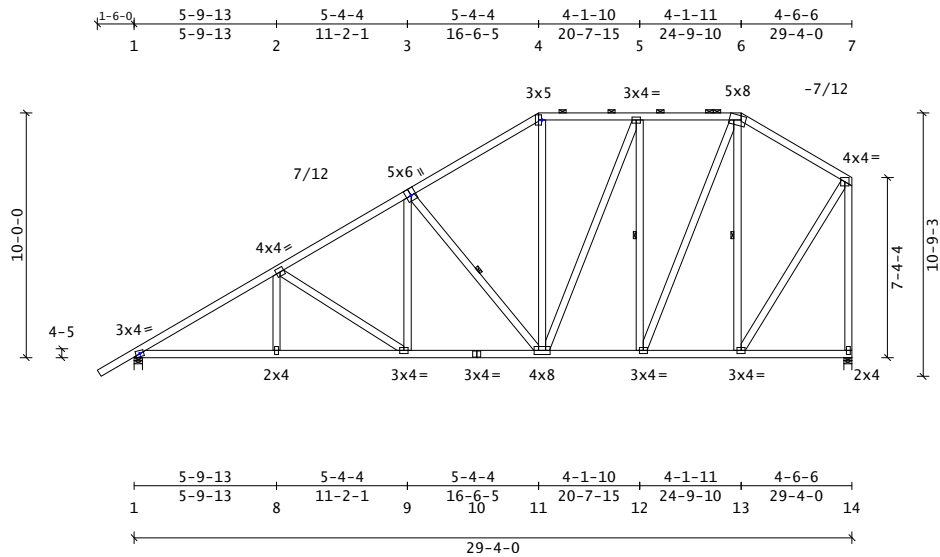
Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Purlins:
Mbr Start End Spacing
TC 17-04-05 25-07-11 2-00-00
Web(s): Continuous Lateral Restraint;
Sgl: 4-13, 14-6, 15-7, 16-8



COA#30003

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TC Plate offsets (X,Y): J1(0,0-01), J3(-0-04,0-07), J4(-1-12,0)



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	10.0	Lbr	1.25	N/A	1.60
Total	40.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph
Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks	
10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCCL Office:	No
Partial Length TCCL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary				
TC	2x4	SYP	#2	
BC	2x4	SYP	#2	
Webs	2x4	SYP	#2	

Member Forces Summary (lb)				
Max CSI in TC PANEL	2 - 3	0.44		
Max CSI in BC PANEL	1 - 8	0.50		
Max CSI in Web	14 - 7	1.00		

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	0H- 1	49	0	0.19
	1- 2	524	1903	0.41
	2- 3	492	1509	0.44
	3- 4	447	1080	0.42
	4- 5	437	857	0.31
	5- 6	398	728	0.30
	6- 7	309	599	0.32
	7-OH	4	0	0.00
BC	1- 8	1572	529	0.50
	8- 9	1572	529	0.50
	9-10	1231	429	0.45
	10-11	1231	429	0.42
	11-12	728	283	0.34
	12-13	466	199	0.28
	13-14	399	241	0.21
	14-OH	0	0	0.00
Web	2- 8	207	0	0.04
	2- 9	193	401	0.27
	3- 9	386	50	0.08
	3-11	280	589	0.20
	4-11	236	72	0.12
	5-11	399	144	0.28
	5-12	238	564	0.24
	6-12	684	219	0.42
	6-13	240	628	0.27
	7-13	865	227	0.28
	7-14	359	1132	1.00

Reaction Summary (lb)									
Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
1	02-00	Pin	1273	1.25	-369	1.60	04-00	01-10	Wall SPF 531~
14	29-02-04	HRoll	1171	1.25	-339	1.60	04-00	01-08	Wall SPF 531~
Horizontal @ Jnt 1:			476	1.60	-272	1.60			
Horizontal @ Jnt 14:			476	1.60	-272	1.60			
~ Includes adjustment for Bearing Area Factor, Cb.									

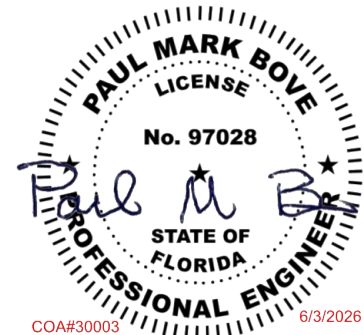
Loads Summary
· User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 20-08-00 are based on 1.00 full and 0.00 reduced load factors.
· See Loadcase Report for load combinations and additional details.

Design Notes
· Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
· Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes
· Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary			
Creep (Kcr) = 2.00			
TrussSpan Limit	Actual(in)	Location	
Vert LL	L/240	L/999(-0.07)	9-11
Vert DL	L/120	L/999(-0.08)	9-11
Vert CR	L/180	L/999(-0.15)	9-11
Horz LL	0.75in	(0.02) @Jt14	
Horz CR	1.25in	(0.05) @Jt14	
Ohng CR	2L/180	2L/999(0.01)	1- 1

Bracing Summary
Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Purlins:
Mbr Start End Spacing
TC 17-04-05 23-11-11 2-00-00
Web(s): Continuous Lateral Restraint;
Sgl: 3-11, 12-5, 13-6



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SIMPSON

Strong-Tie

Component Solutions

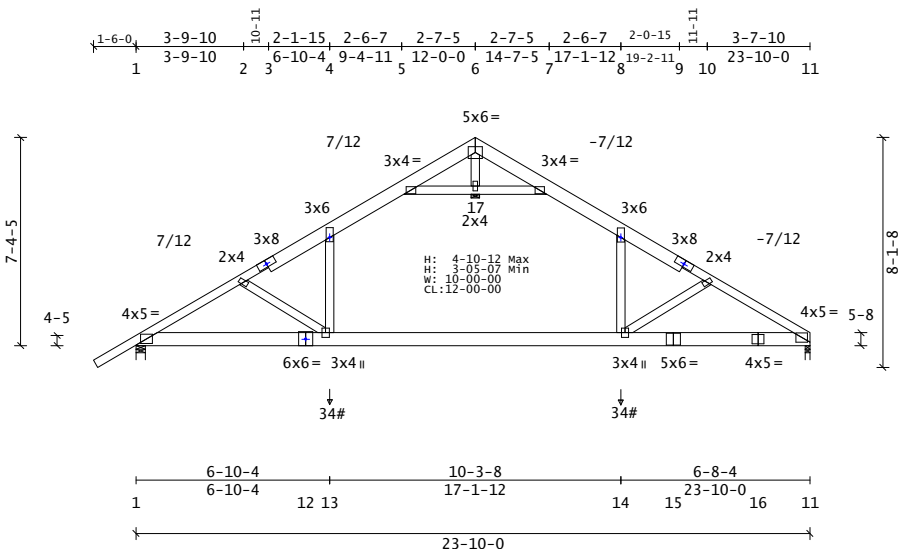
Truss Studio V

2026.3.0.65

Helpdesk: 1-866-252-8606

CSHelp@strongtie.com

TC Plate offsets (X,Y): J3(-0-02,0-03), J4(0,1-02), J8(0,1-01), J9(0-02,0-03)



BC Plate offsets (X,Y): J12(0,0-04)

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	10.0	Lbr	1.25	N/A	1.60
Total	40.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00	o.c.	Plies:	1		
Repetitive	Member	Increase:	Yes			
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph
Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks	
10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary				
TC	2x4	SYP	#2	
	2x6	SYP	DSS	3-6 6-9
BC	2x6	SYP	DSS	
	2x6	SYP	#2	15-16 16-11
Webs	2x4	SYP	#2	
TB	2x4	SYP	#2	
Member Forces Summary (lb)				
Max CSI in TC PANEL	1 - 2	0.63		
Max CSI in BC PANEL	15 - 16	0.73		
Max CSI in Web	5 - 17	0.30		
Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	51	0	0.19
	1- 2	228	2163	0.63
	2- 3	166	1949	0.62
	3- 4	171	1937	0.45
	4- 5	245	1404	0.44
	5- 6	494	0	0.47
	6- 7	490	0	0.48
	7- 8	244	1407	0.45
	8- 9	174	1929	0.43
	9-10	169	1940	0.58
	10-11	237	2141	0.61
BC	1-12	1883	148	0.34
	11-16	1857	158	0.51
	12-13	1883	148	0.39
	13-14	1537	23	0.71
	14-15	1857	158	0.38
	15-16	1857	158	0.73
Web	2-13	149	414	0.12
	4-13	1017	0	0.28
	5-17	234	2126	0.30
	6-17	59	0	0.01
	7-17	234	2126	0.30
	8-14	993	0	0.28
	10-14	161	486	0.12

Reaction Summary (lb)												
Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI			
1	02-00	Pin	1416	1.25	-205	1.60	04-00	01-12	Wall SPF 531~			
11	23-09-00	HRoll	> 1326	1.25	-154	1.60	02-00	01-11	Wall SPF 531~			
Horizontal @ Jnt 1:			241	1.60	-199	1.60						
> Critical @ Jnt 11:			1076	1.00								
~ Includes adjustment for Bearing Area Factor, Cb.												

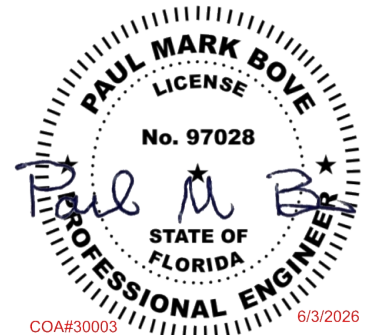
Loads Summary
• User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-00-00 are based on 1.00 full and 0.00 reduced load factors.
• Attic space centered at 12-00-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

Concentrated Loads (lbs) based on Max and Min:									
Mbr	Location	Max	Min	Dir	DL/TL	Angle	Connection	Description	
Web	6-10-04	34	17	V	1.00	90°	None	SidewallDL	
Web	17-01-12	34	17	V	1.00	90°	None	SidewallDL	

Design Notes
• Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
• Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary			
Creep (Kcr)	= 2.00		
TrussSpan Limit	Actual(in)	Location	
Vert LL	L/240	L/594(-0.48)	13-14
Vert DL	L/120	L/841(-0.34)	13-14
Vert CR	L/180	L/348(-0.81)	13-14
Horz LL	0.75in	(0.02) @Jt11	
Horz CR	1.25in	(0.04) @Jt11	
Ohng CR	2L/180	2L/999(0.01)	1- 1

Bracing Summary
Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Attic tie beam (TB) & walls; bracing indicated or rigid sheathing.
Purlins:
Mbr Start End Spacing
TB 9-05-11 14-06-05 2-07-00
Web(s): Continuous Lateral Restraint;



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SIMPSON

Strong-Tie

Component Solutions

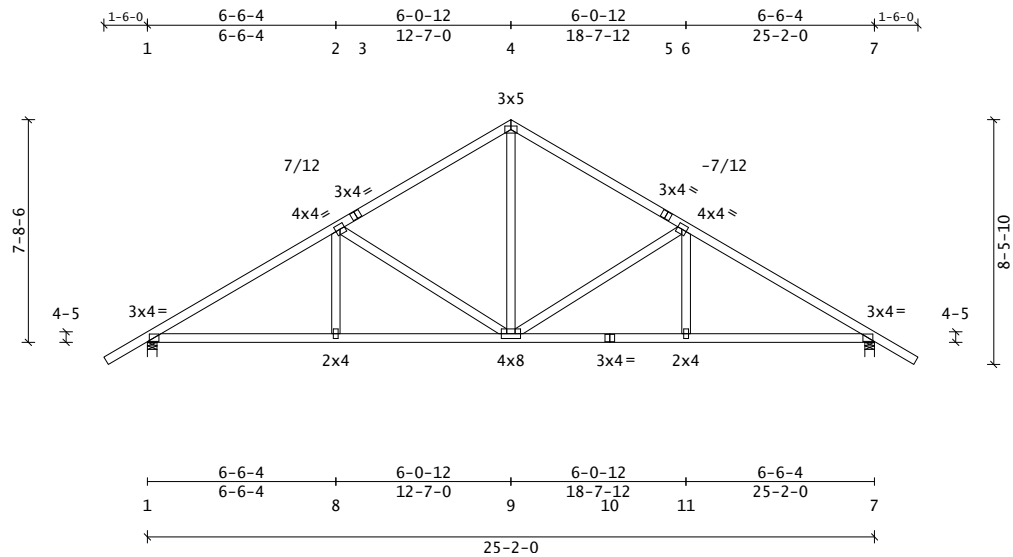
Truss Studio V

2026.3.0.65

Helpdesk: 1-866-252-8606

CSHelp@strongtie.com

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.52
Max CSI in BC PANEL	1 - 8	0.43
Max CSI in Web	2 - 9	0.45

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	49	0	0.19
	1- 2	448	1566	0.52
	2- 3	370	1087	0.50
	3- 4	396	1066	0.31
	4- 5	396	1066	0.31
	5- 6	370	1087	0.50
	6- 7	448	1566	0.52
	7-OH	49	0	0.19
BC	1- 8	1278	257	0.43
	7-11	1278	257	0.43
	8- 9	1278	257	0.43
	9-10	1278	257	0.42
	10-11	1278	257	0.43
Web	2- 8	249	0	0.05
	2- 9	243	506	0.45
	4- 9	662	201	0.19
	6- 9	243	506	0.45
	6-11	249	0	0.05

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
1	02-00	Pin	1102	1.25	-330	1.60	04-00	01-08	Wall SPF 531~
7	25-00-00	HRoll	1102	1.25	-330	1.60	04-00	01-08	Wall SPF 531~
Horizontal @ Jnt 1:			242	1.60	-242	1.60			

~ Includes adjustment for Bearing Area Factor, Cb.

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-07-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

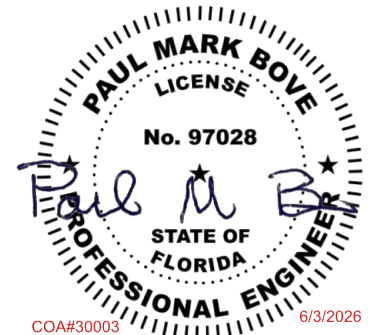
- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.06)	8- 9
Vert DL	L/120	L/999(-0.07)	8- 9
Vert CR	L/180	L/999(-0.13)	8- 9
Horz LL	0.75in	(0.03) @Jt 7	
Horz CR	1.25in	(0.05) @Jt 7	
Ohng CR	2L/180	2L/999(0.01)	1- 1
Ohng CR	2L/180	2L/999(0.01)	7- 7

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;

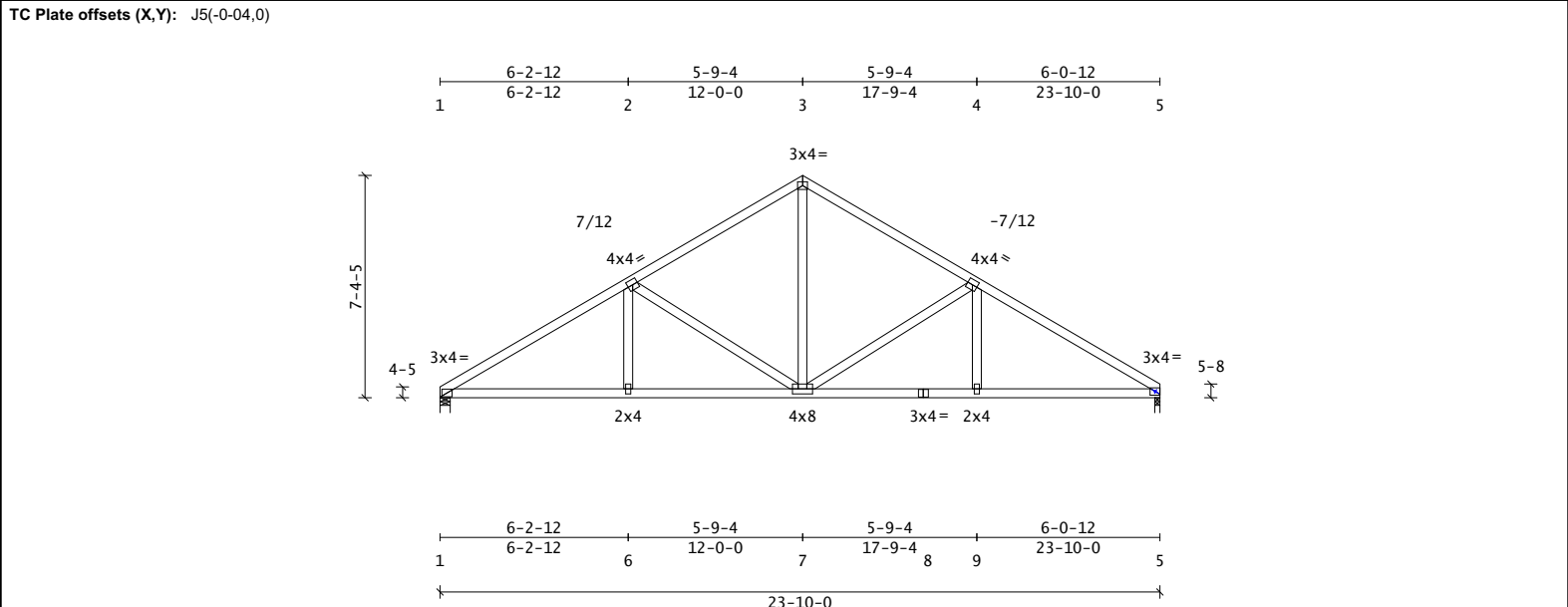


COA#30003

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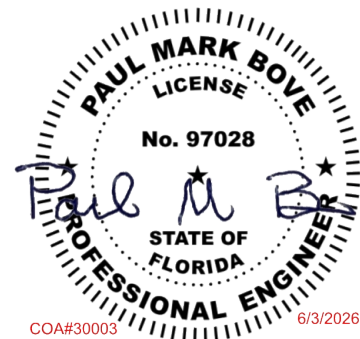


Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

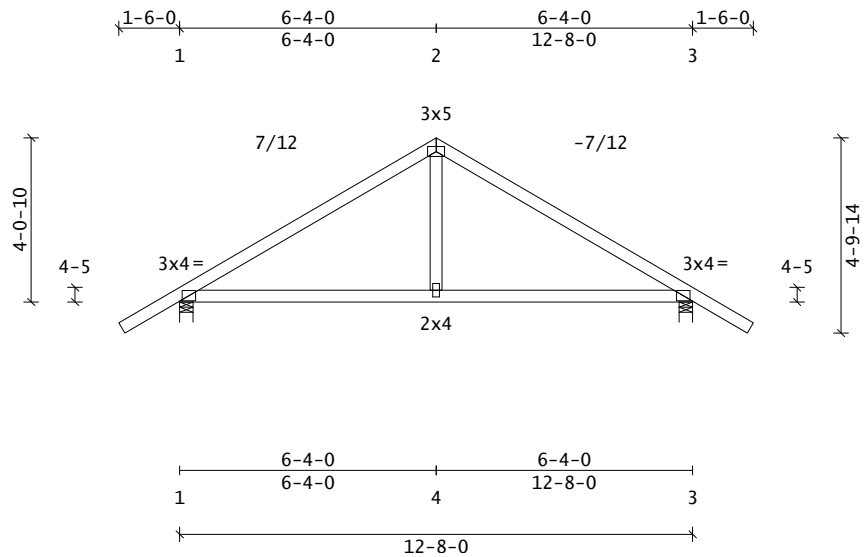


FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 10.0 Lbr 1.25 N/A 1.60 Total 40.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h) = 18.0 ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 5.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 1 02-00 Pin 956 1.25 -271 1.60 04-00 01-08 Wall SPF 531~ 5 23-09-00 HRoll 949 1.25 -268 1.60 02-00 01-08 Wall SPF 531~ Horizontal @ Jnt 1: 227 1.60 -225 1.60 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.05) 7- 9 Vert DL L/120 L/999(-0.07) 7- 9 Vert CR L/180 L/999(-0.12) 7- 9 Horz LL 0.75in (0.02) @Jt 5 Horz CR 1.25in (0.05) @Jt 5
Member Forces Summary (lb) Max CSI in TC PANEL 1 - 2 0.50 Max CSI in BC PANEL 1 - 6 0.41 Max CSI in Web 2 - 7 0.40	Loads Summary • User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-00-00 are based on 1.00 full and 0.00 reduced load factors. • See Loadcase Report for load combinations and additional details.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Member Forces Summary (lb) Mbr Jnt-Jnt Tens Comp CSI TC 1- 2 440 1492 0.50 2- 3 384 1034 0.48 3- 4 383 1033 0.47 4- 5 436 1478 0.49 BC 1- 6 1219 297 0.41 5- 9 1203 293 0.39 6- 7 1219 297 0.40 7- 8 1203 293 0.39 8- 9 1203 293 0.39 Web 2- 6 240 0 0.05 2- 7 242 499 0.40 3- 7 628 198 0.17 4- 7 237 483 0.39 4- 9 232 0 0.05	Design Notes • Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. • Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.	



TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	10.0	Lbr	1.25	N/A	1.60
Total	40.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

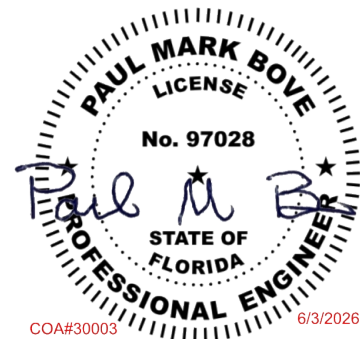
Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph
Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks	
10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes
Plate Tolerance Checks	
Fabrication: 20% (Cq = 0.80), u.n.o.	
Rotation: +/- 10 degrees, u.n.o.	

Material Summary				
TC	2x4	SYP	#2	
BC	2x4	SYP	#2	
Webs	2x4	SYP	#2	
Member Forces Summary (lb)				
Max CSI in TC PANEL	1 - 2	0.43		
Max CSI in BC PANEL	1 - 4	0.37		
Max CSI in Web	4 - 2	0.06		
Reaction Summary (lb)				
Jnt	XLocation	Model	Maximum LDF	Minimum LDF
1	02-00	Pin	602	1.25
3	12-06-00	HRoll	602	1.25
Horizontal @ Jnt 1:	118	1.60	-118	1.60
~ Includes adjustment for Bearing Area Factor, Cb.				
Loads Summary				
• User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-04-00 are based on 1.00 full and 0.00 reduced load factors.				
• See Loadcase Report for load combinations and additional details.				
Design Notes				
• Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.				
• Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.				

Deflection Summary			
Creep (Kcr)	2.00		
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.03)	4- 3
Vert DL	L/120	L/999(-0.04)	4- 3
Vert CR	L/180	L/999(-0.07)	4- 3
Horz LL	0.75in	(0.01) @Jt 1	
Horz CR	1.25in	(0.01) @Jt 1	
Ohng CR	2L/180	2L/999(0.02)	1- 1
Ohng CR	2L/180	2L/999(0.02)	3- 3
Bracing Summary			
Top Chord: Structural Wood Sheathing			
Bot Chord: Gypsum Board Sheathing			
Web(s): None;			



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SIMPSON

Strong-Tie

Component Solutions

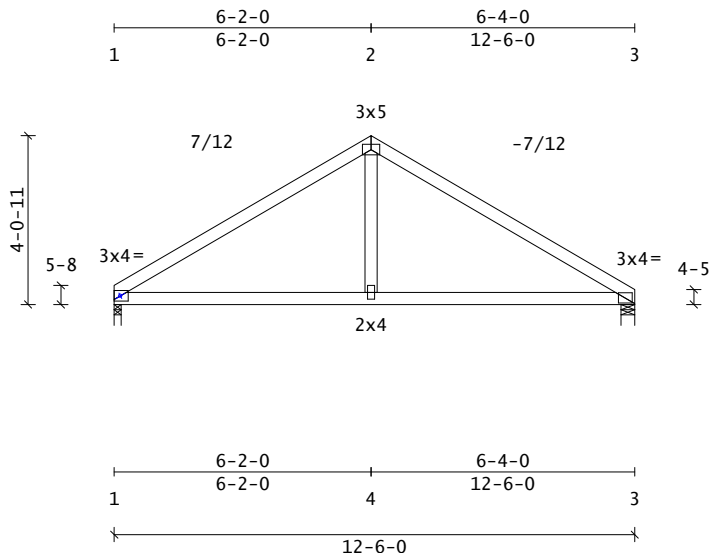
Truss Studio V

2026.3.0.65

Helpdesk: 1-866-252-8606

CSHelp@strongtie.com

TC Plate offsets (X,Y): J1(0-04,0)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0 ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCCL Maintenance:	No
300 lb TCCL/BCLL Person Loading:	No
2000 lb Moving TCCL Office:	No
Partial Length TCCL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	2 - 3	0.58
Max CSI in BC PANEL	1 - 4	0.38
Max CSI in Web	4 - 2	0.06

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	189	627	0.53
	2- 3	189	625	0.58
BC	1- 4	466	72	0.38
	3- 4	466	72	0.38
Web	2- 4	281	0	0.06

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
1	01-00	Pin	496	-140	1.60	02-00	01-08	Wall	SPF 531~
3	12-04-00	HRoll	503	-143	1.60	04-00	01-08	Wall	SPF 531~
Horizontal @ Jnt 1: 113 1.60 -115 1.60									

~ Includes adjustment for Bearing Area Factor, Cb.

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-02-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

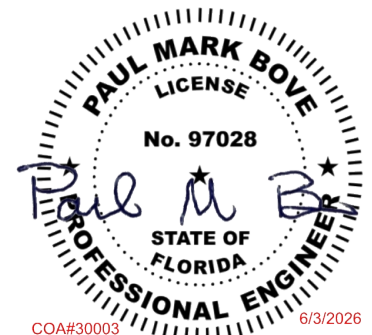
- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.04) 4- 3
Vert DL	L/120	L/999(-0.04) 4- 3
Vert CR	L/180	L/999(-0.08) 4- 3
Horz LL	0.75in	(0.01) @Jt 1
Horz CR	1.25in	(0.02) @Jt 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



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SIMPSON

Strong-Tie

Component Solutions

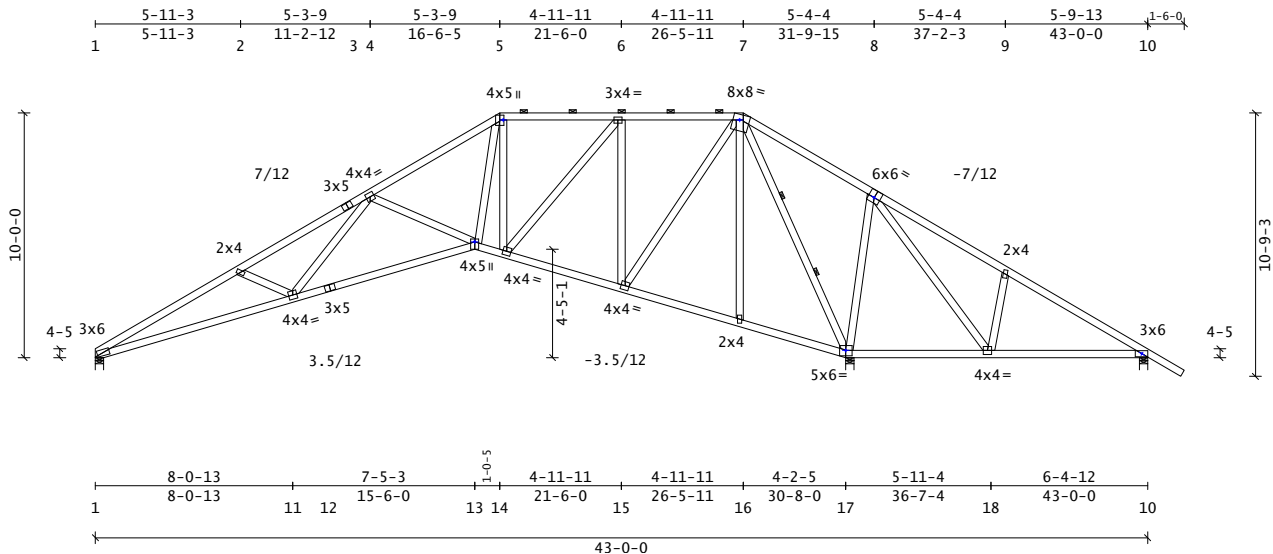
Truss Studio V

2026.3.0.65

Helpdesk: 1-866-252-8606

CSHelp@strongtie.com

TC Plate offsets (X,Y): J5(-1-12,-0-03), J7(0-07,-1-06), J8(0-04,0-07), J10(-0-04,0)



BC Plate offsets (X,Y): J13(0,-1-02), J17(0,0-02)

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00	o.c.	Plies:	1	
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCCL Office:	No
Partial Length TCCL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	7 - 8	0.81
Max CSI in BC PANEL	1 - 11	0.76
Max CSI in Web	7 - 17	0.82

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum	LDF	Minimum	LDF	Brg_Width	Req_Width	Type	Mat	PSI
1	02-00	Pin	970	1.25	-288	1.60	04-00	01-08	Wall	SPF	531~
17	30-09-12	HRoll	2694	1.25	-699	1.60	04-00	03-06	Wall	SPF	531~
10	42-10-00	HRoll	> 208	1.60	-398	1.25	04-00	01-08	Wall	SPF	531~
Horizontal @ Jnt 1:			306	1.60	-344	1.60					
> Critical @ Jnt 10:			176	1.25							
~ Includes adjustment for Bearing Area Factor, Cb.											

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 21-06-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

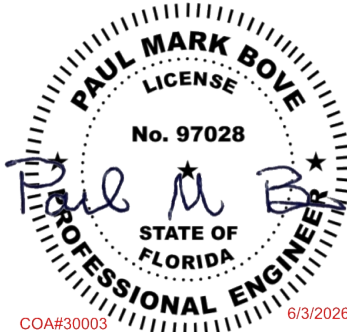
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.21) 11-13
Vert DL	L/120	L/999(-0.27) 11-13
Vert CR	L/180	L/766(-0.48) 11-13
Horz LL	0.75in	(0.14) @Jt17
Horz CR	1.25in	(0.26) @Jt17
Ohng CR	2L/180	2L/999(0.01) 10-10

Bracing Summary

Top Chord: Structural Wood Sheathing			
Bot Chord: Gypsum Board Sheathing			
Purlins:			
Mbr	Start	End	Spacing
TC	17-04-05	25-07-11	2-00-00

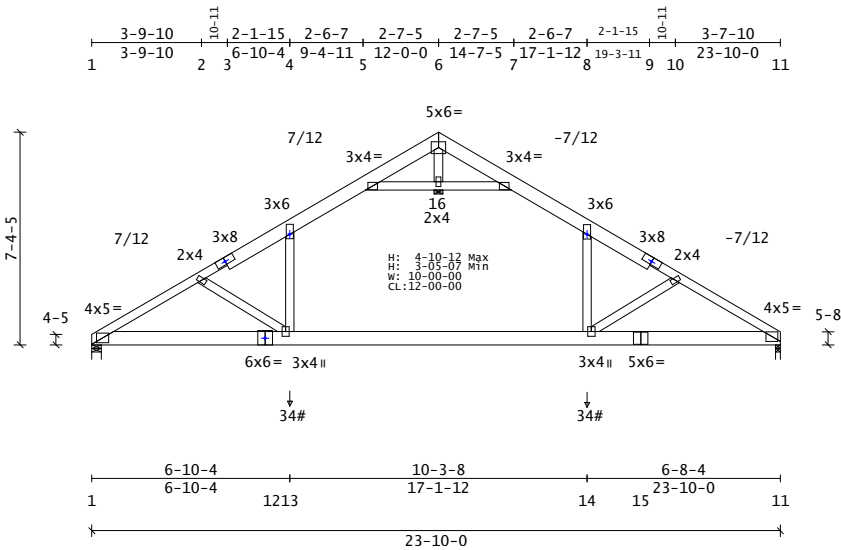


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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): J3(-0-02,0-03), J4(0,1-02), J8(0,1-01), J9(0-02,0-03)



BC Plate offsets (X,Y): J12(0,0-04)

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2		
	2x6	SYP	DSS	3-6	6-9
BC	2x6	SYP	DSS		
	2x6	SYP	#2	15-11	
Webs	2x4	SYP	#2		
TB	2x4	SYP	#2		

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
1	02-00	Pin	> 1332	1.25	-160	1.60	04-00	01-11	Wall SPF 531~
11	23-09-00	HRoll	> 1330	1.25	-156	1.60	02-00	01-11	Wall SPF 531~
Horizontal @ Jnt 1: 219 1.60 -219 1.60									
> Critical @ Jnt 1: 1076 1.00									
> Critical @ Jnt 11: 1077 1.00									
~ Includes adjustment for Bearing Area Factor, Cb.									

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan Limit	Actual(in)	Location
Vert LL	L/240	L/595(-0.48) 13-14
Vert DL	L/120	L/841(-0.34) 13-14
Vert CR	L/180	L/348(-0.81) 13-14
Horz LL	0.75in	(0.02) @Jt11
Horz CR	1.25in	(0.04) @Jt11

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.64
Max CSI in BC PANEL	15 - 11	0.73
Max CSI in Web	5 - 16	0.30

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 12-00-00 are based on 1.00 full and 0.00 reduced load factors.
- Attic space centered at 12-00-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

• See Loadcase Report for load combinations and additional details.

Concentrated Loads (lbs) based on Max and Min:

Mbr	Location	Max	Min	Dir	DL/TL	Angle	Connection	Description
Web	6-10-04	34	17	V	1.00	90°	None	SidewallDL
Web	17-01-12	34	17	V	1.00	90°	None	SidewallDL

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

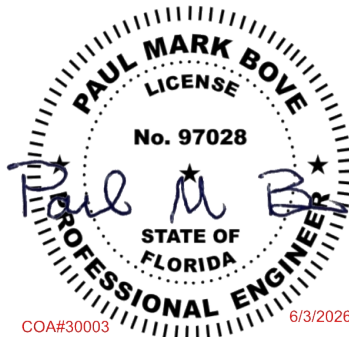
Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Attic tie beam (TB) & walls; bracing indicated or rigid sheathing.

Purlins:

Mbr	Start	End	Spacing
TB	9-05-11	14-06-05	2-07-00

Web(s): Continuous Lateral Restraint;

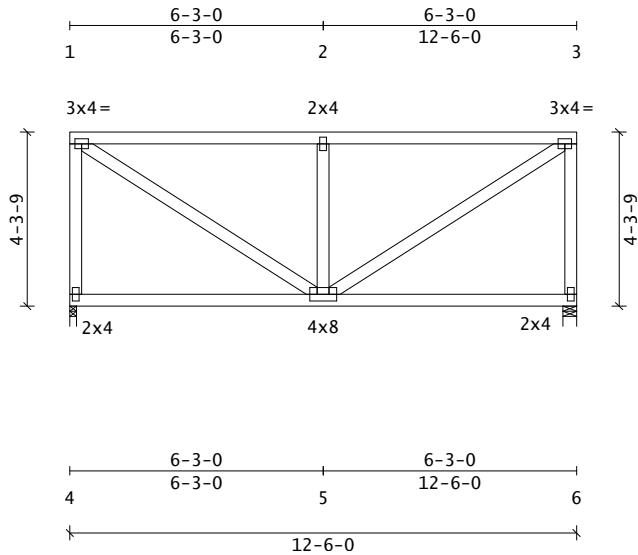


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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): None



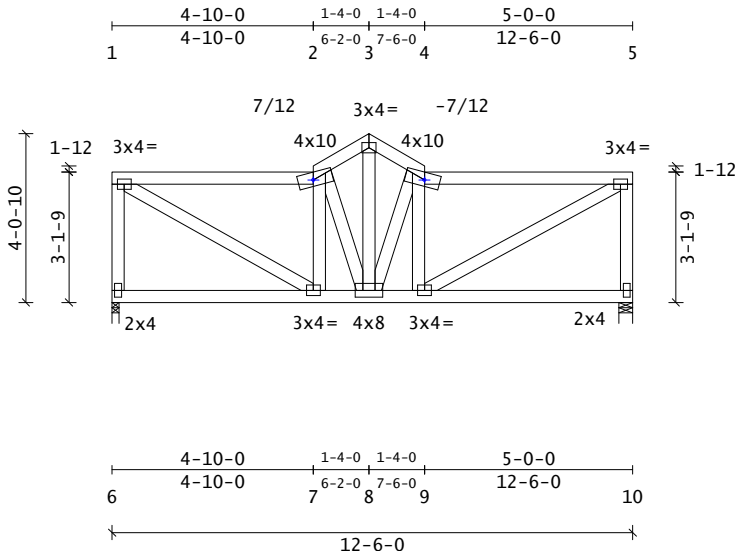
BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 10.0 Lbr 1.25 N/A 1.60 Total 40.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 5.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 4 01-12 Pin 499 1.25 -172 1.60 02-00 01-08 Wall SPF 531~ 6 12-04-04 HRoll 500 1.25 -172 1.60 04-00 01-08 Wall SPF 531~ Horizontal @ Jnt 4: 185 1.60 -185 1.60 Horizontal @ Jnt 6: 185 1.60 -185 1.60 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.04) 5- 6 Vert DL L/120 L/999(-0.04) 5- 6 Vert CR L/180 L/999(-0.09) 5- 6 Horz LL 0.75in (0.00) @Jt 4 Horz CR 1.25in (0.00) @Jt 4
Member Forces Summary (lb) Max CSI in TC PANEL 1 - 2 0.86 Max CSI in BC PANEL 4 - 5 0.35 Max CSI in Web 4 - 1 0.34	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-03-00 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Member Forces Summary (lb) Mbr Jnt-Jnt Tens Comp CSI TC OH- 1 0 0 0.00 1- 2 195 459 0.86 2- 3 195 459 0.86 3-OH 0 0 0.00 BC OH- 4 0 0 0.00 4- 5 143 134 0.35 5- 6 143 134 0.35 6-OH 0 0 0.00 Web 1- 4 165 449 0.34 1- 5 550 155 0.19 2- 5 247 454 0.13 3- 5 550 211 0.19 3- 6 196 449 0.34	Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. · Lumber and plating have been applied symmetrically.	
	Installation Notes · Less than 0.25/12 pitch requires adequate drainage to prevent ponding.	

Project: P-1038 - Martinez		Truss ID: T19	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280730
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 03 / 26
Project Note(s):		Weight: 90.1 lb	Page: 1 of 1

TC Plate offsets (X,Y): J2(0-08,0-06), J4(-0-08,0-06)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	4 - 5	0.69
Max CSI in BC PANEL	8 - 9	0.25
Max CSI in Web	6 - 1	0.22

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	0	0	0.00
	1- 2	173	511	0.66
	2- 3	200	503	0.38
	3- 4	201	505	0.39
	4- 5	174	520	0.69
BC	5-OH	0	0	0.00
	OH- 6	0	0	0.00
	6- 7	116	110	0.20
	7- 8	514	162	0.24
	8- 9	522	147	0.25
Web	9-10	0	28	0.21
	10-OH	0	0	0.00
	1- 6	157	455	0.22
	1- 7	593	148	0.13
	2- 7	110	154	0.05
	2- 8	118	357	0.07
	3- 8	549	213	0.12
	4- 8	122	379	0.08
	4- 9	111	144	0.05
	5- 9	598	149	0.13
	5-10	158	454	0.22

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
6	01-12	Pin	500	1.25	-137	1.60	02-00	01-08	Wall SPF 531~
10	12-04-04	HRoll	500	1.25	-137	1.60	04-00	01-08	Wall SPF 531~
Horizontal @ Jnt 6: 145 1.60 -145 1.60									
~ Includes adjustment for Bearing Area Factor, Cb.									

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-02-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

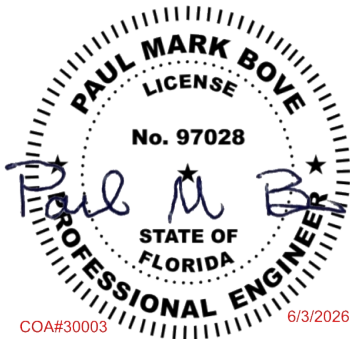
- Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.04)	7- 8
Vert DL	L/120	L/999(-0.03)	7- 8
Vert CR	L/180	L/999(-0.07)	7- 8
Horz LL	0.75in	(0.00)	@Jt10
Horz CR	1.25in	(0.00)	@Jt10

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



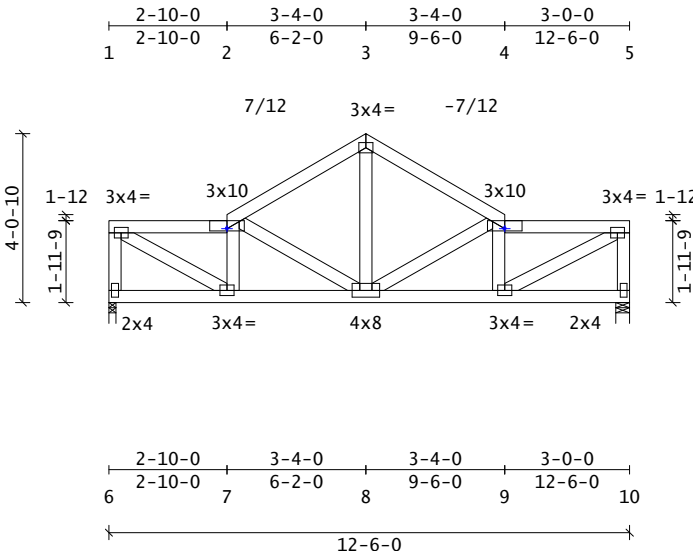
NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez	Truss ID: T20		
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280731
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 03 / 26
Project Note(s):		Weight: 78.0 lb	Page: 1 of 1

TC Plate offsets (X,Y): J2(0,0-13), J4(0,0-13)



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	10.0	Lbr	1.25	N/A 1.60
Total	40.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 5.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	4 - 5	0.32
Max CSI in BC PANEL	8 - 9	0.18
Max CSI in Web	1 - 7	0.16

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	OH- 1	0	0	0.00
	1- 2	185	609	0.30
	2- 3	181	524	0.21
	3- 4	182	524	0.22
	4- 5	192	636	0.32
	5-OH	0	0	0.00
BC	OH- 6	0	0	0.00
	6- 7	110	107	0.07
	7- 8	618	141	0.17
	8- 9	645	148	0.18
	9-10	0	15	0.08
	10-OH	0	0	0.00
Web	1- 6	147	469	0.12
	1- 7	708	187	0.16
	2- 7	130	299	0.06
	2- 8	99	247	0.06
	3- 8	313	67	0.07
	4- 8	107	274	0.06
	4- 9	130	292	0.06
	5- 9	728	193	0.16
	5-10	148	468	0.12

Reaction Summary (lb)

Jnt	XLocation	Model	Maximum LDF	Minimum LDF	Brg_Width	Req_Width	Type	Mat	PSI
6	01-12	Pin	499	1.25	-137	1.60	02-00	01-08	Wall SPF 531~
10	12-04-04	HRoll	500	1.25	-137	1.60	04-00	01-08	Wall SPF 531~
Horizontal @ Jnt 6: 126 1.60 -126 1.60									
~ Includes adjustment for Bearing Area Factor, Cb.									

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-02-00 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

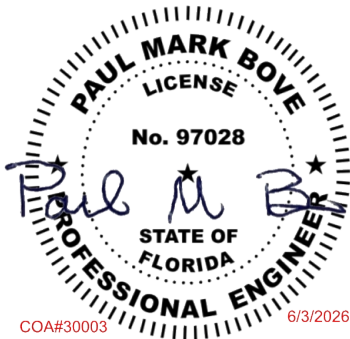
- Less than 0.25/12 pitch requires adequate drainage to prevent ponding.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.03)	8- 9
Vert DL	L/120	L/999(-0.03)	8- 9
Vert CR	L/180	L/999(-0.06)	8- 9
Horz LL	0.75in	(0.00)	@Jt10
Horz CR	1.25in	(0.01)	@Jt10

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



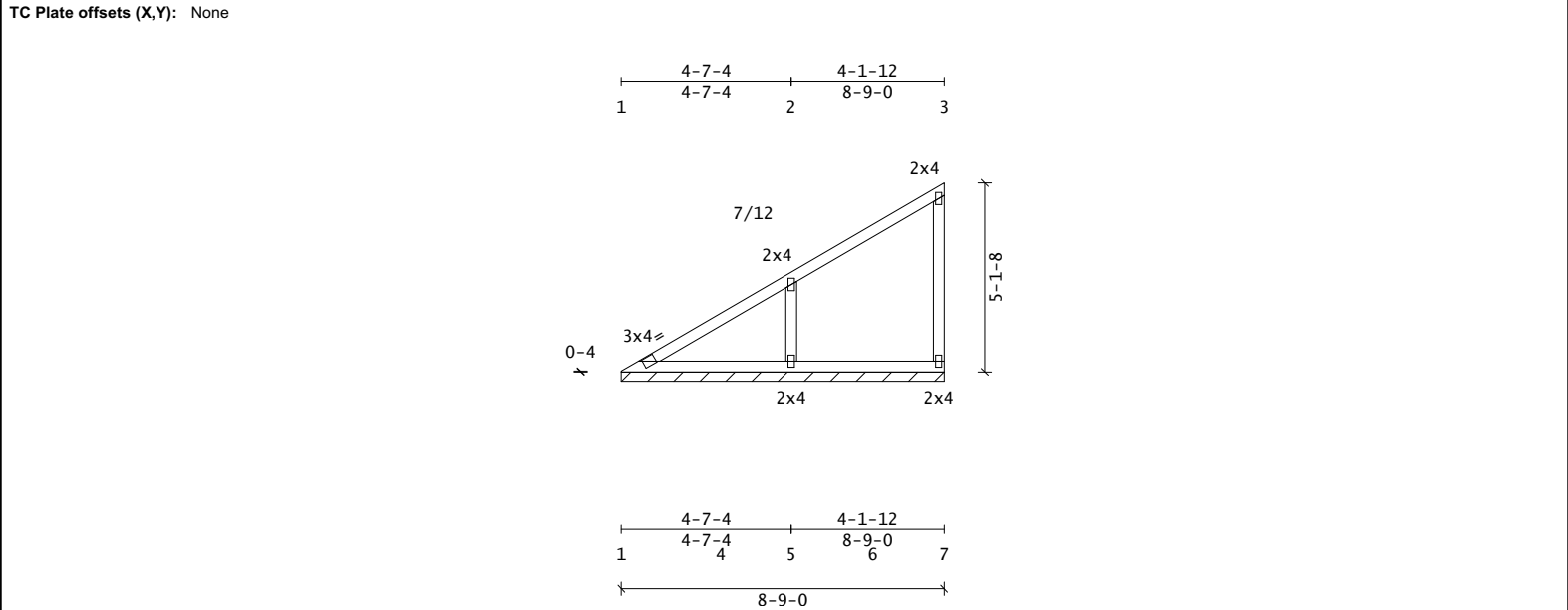
Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

SID: 0004280732
TID: 318742
Date: 06 / 01 / 26
Page: 1 of 1

The diagram shows a roof truss system with the following dimensions and specifications:

- Roof Slope:** 7/12
- Roofing:** 2x4
- Truss Members:** 2x4
- Supports:** 2x4
- Vertical Height:** 6'-3" (63 inches)
- Horizontal Span:** 10'-9" (109 inches)
- Material Specifications:** 2-7-4, 4-0-0, 4-1-12, 10-9-0, 3, 4, 5, 6, 7, 8, 9

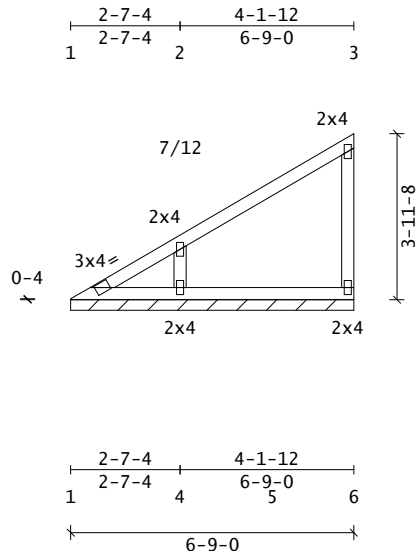
EngDwg: 2026 3 1 0 PM



FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 5 4-08-12 Pin 275 1.25 -208 1.60 8-09-00 01-08 Wall SPF 425~ Horizontal @ Jnt 5: 248 1.60 -136 1.60 Jnt 1 - Jnt 7 (plf): 30 1.25 6 1.60 Reactions not shown: < 400 > -150 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 1- 4 Vert DL L/120 L/999(-0.00) 1- 4 Vert CR L/180 L/999(-0.00) 1- 4 Horz LL 0.75in (0.01) @Jt 1 Horz CR 1.25in (0.01) @Jt 1
Member Forces Summary (lb) Max CSI in TC PANEL 2 - 3 0.39 Max CSI in BC PANEL 1 - 4 0.21 Max CSI in Web 7 - 3 0.41	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 8-10-08 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Member Forces Summary (lb) Mbr Jnt-Jnt Tens Comp CSI TC 1- 2 199 200 0.38 2- 3 82 107 0.39 3-OH 0 4 0.00 BC 1- 4 198 194 0.21 4- 5 198 194 0.12 5- 6 77 50 0.03 6- 7 77 50 0.02 7-OH 0 0 0.00 Web 2- 5 219 279 0.10 3- 7 84 102 0.41	Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.	
	Installation Notes · Valley Truss application only.	

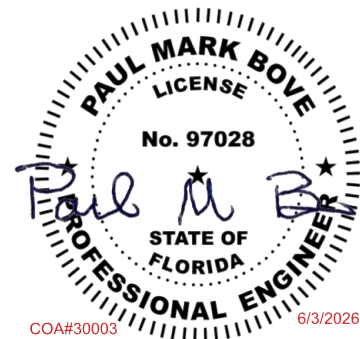
TC Plate offsets (X,Y): None

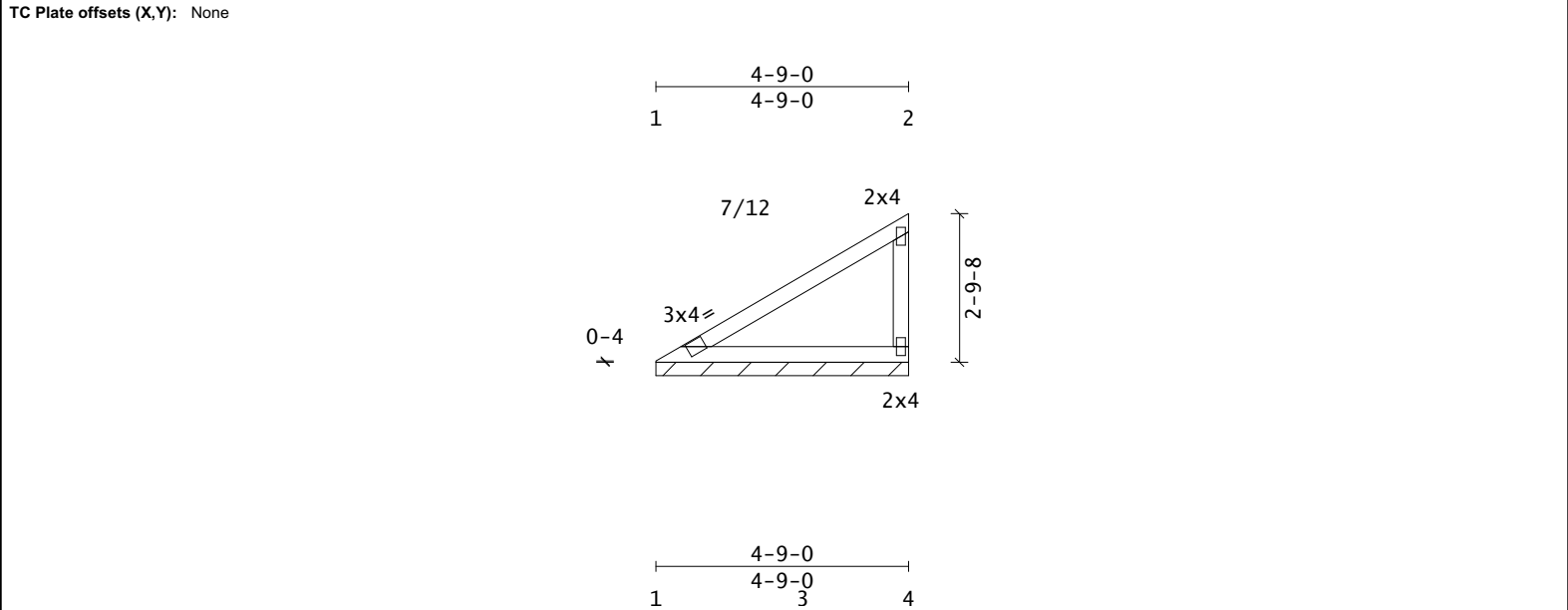


BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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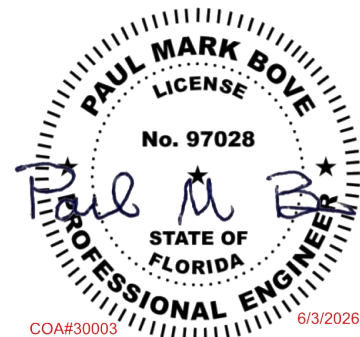
Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2 Member Forces Summary (lb) Max CSI in TC PANEL 2 - 3 0.34 Max CSI in BC PANEL 1 - 4 0.08 Max CSI in Web 6 - 3 0.26 Mbr Jnt-Jnt Tens Comp CSI TC 1- 2 196 167 0.31 2- 3 71 93 0.34 3-OH 0 4 0.00 BC 1- 4 148 160 0.08 4- 5 57 36 0.01 5- 6 57 36 0.01 6-OH 0 0 0.00 Web 2- 4 191 328 0.10 3- 6 79 144 0.26	Reaction Summary (lb, u.n.o.) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 4 2-08-12 Pin 244 1.25 -185 1.60 6-09-00 01-08 Wall SPF 425~ Horizontal @ Jnt 4: 186 1.60 -101 1.60 Jnt 1 - Jnt 6 (plf): 25 1.25 3 1.60 Reactions not shown: < 400 > -150 ~ Includes adjustment for Bearing Area Factor, Cb. Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-10-08 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details. Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Installation Notes · Valley Truss application only.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 5- 6 Vert DL L/120 L/999(-0.00) 5- 6 Vert CR L/180 L/999(-0.00) 5- 6 Horz LL 0.75in (0.00) @Jt 1 Horz CR 1.25in (0.00) @Jt 1 Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
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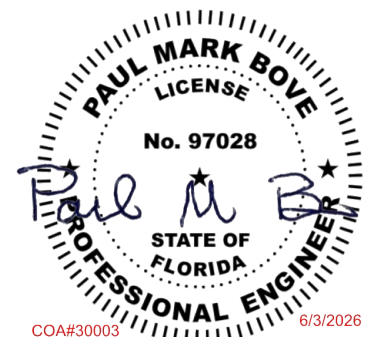
FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100' Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary					Reaction Summary (lb, u.n.o.)					Deflection Summary				
TC	2x4	SYP	#2		Maximum LDF Minimum LDF					Creep (Kcr) = 2.00				
BC	2x4	SYP	#2		Horizontal @ Jnt 3: 123 1.60 -65 1.60					TrussSpan Limit Actual(in) Location				
Webs	2x4	SYP	#2		Jnt 1 - Jnt 4 (plf): 61 1.25 30 1.60					Vert LL L/240 L/999(-0.00) 1- 3				
					Reactions not shown: < 400 > -150					Vert DL L/120 L/999(-0.00) 1- 3				
Member Forces Summary (lb)					Loads Summary					Bracing Summary				
Max CSI in TC PANEL 1 - 2 0.31					· User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 4-10-08 are based on 1.00 full and 0.00 reduced load factors.					Top Chord: Structural Wood Sheathing				
Max CSI in BC PANEL 1 - 3 0.30					· See Loadcase Report for load combinations and additional details.					Bot Chord: Gypsum Board Sheathing				
Max CSI in Web 4 - 2 0.15					Design Notes					Web(s): None;				
					· Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.									
					· Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.									
Installation Notes														
· Valley Truss application only.														
Mbr Jnt-Jnt Tens Comp CSI														
TC 1- 2 76 76 0.31														
2-OH 0 4 0.00														
BC 1- 3 99 116 0.30														
3- 4 36 23 0.10														
4-OH 0 0 0.00														
Web 2- 4 71 154 0.15														



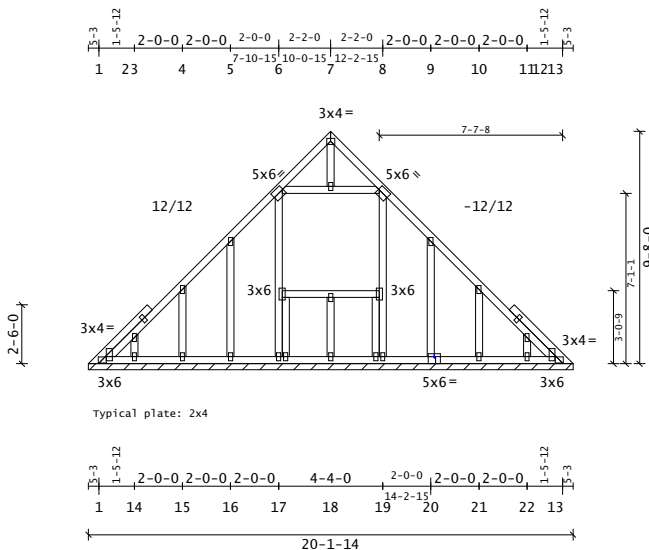
SID: 0004280736
TID: 318742
Date: 06 / 03 / 26
Page: 1 of 1

Diagram of a right-angled triangle. The horizontal base is labeled $2-9-0$ with dimension lines and points 1 and 2. The vertical height is labeled $7/12$ and 2×4 . The hypotenuse is labeled 3×4 . A right-angle symbol is shown at the vertex where the base and height meet. The triangle is supported by a hatched base. A vertical dimension line on the right indicates a height of $1-7-8$. A horizontal dimension line at the bottom indicates a width of $2-9-0$ with points 1 and 3.



Project: P-1038 - Martinez		Truss ID: V10
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.
Project Location: Project location has not been provided		Quantity: 1
Project Note(s):		Weight: 164.0 lb
		SID: 0004280737
		TID: 318742
		Date: 06 / 03 / 26
		Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): J20(0,-1-00)

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h) = 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

UTC	2x4	SYP	#2
LTC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 18:	368	1.60
Jnt 1 - Jnt 13 (plf):	70	1.25
Reactions not shown:	< 400	> -150

Deflection Summary

Creep (Kcr) =	2.00
TrussSpan Limit	Actual(in) Location
Vert LL	L/240 L/999(-0.00) 19-20
Vert DL	L/120 L/999(-0.00) 19-20
Vert CR	L/180 L/999(-0.00) 19-20
Horz LL	0.75in (0.01) @Jt13
Horz CR	1.25in (0.01) @Jt13

Member Forces Summary (lb)

Max CSI in TC PANEL	3 - 4	0.08
Max CSI in BC PANEL	1 - 14	0.08
Max CSI in Web	17 - 6	0.13

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 10-01-09 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

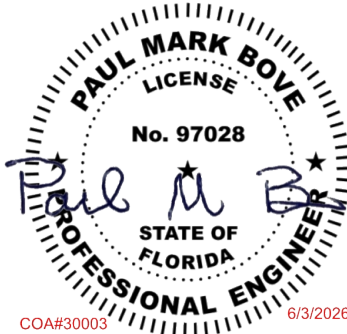
Installation Notes

- Valley Truss application only.
- If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
- The maximum rake overhang length is 12.0".
- Gable requires 7/16" OSB sheathing on front from 00-14 to 20-02-04; connection details to be provided by the Building Designer.

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

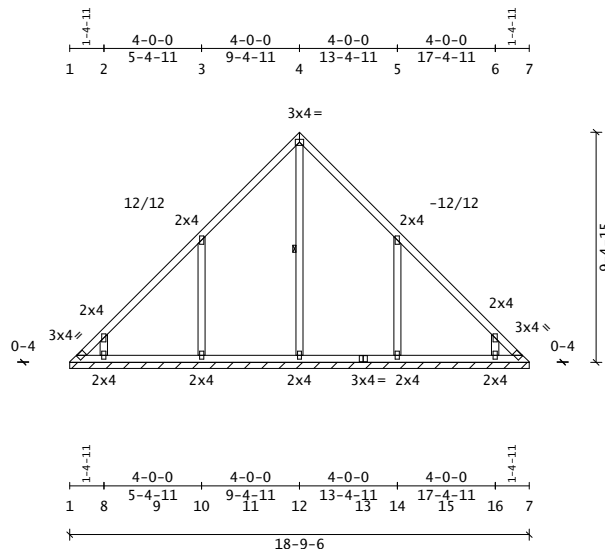


COA#30003



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014							
PSF	Live	Dead	Load	Duration	Factors		
TC	20.0	10.0	Live	Snow	Wind		
BC	0.0	1.0	Lbr	1.25	N/A	1.60	
Total	31.0	Plt	1.25	N/A	1.60		
OH Soffit	2.0						
Spacing:	2-00-00 o.c.	Plies:	1				
Repetitive Member Increase:	Yes						
Green Lbr:	No	Temp:	T ≤ 100°				
Treated Lbr:	No	Incised Lbr:	No				
Seasoned Lbr:	Yes	Wet Service:	No				

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	3 - 4	0.34
Max CSI in BC PANEL	1 - 8	0.05
Max CSI in Web	10 - 3	0.13

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	205	335	0.17
	2- 3	145	216	0.31
	3- 4	183	178	0.34
	4- 5	183	178	0.34
	5- 6	145	216	0.31
	6- 7	205	335	0.17
BC	1- 8	227	106	0.05
	7-16	227	106	0.05
	8- 9	227	106	0.04
	9-10	227	106	0.04
	10-11	227	106	0.04
	11-12	227	106	0.05
Web	12-13	227	106	0.05
	13-14	227	106	0.04
	14-15	227	106	0.04
	15-16	227	106	0.04
	2- 8	244	306	0.07
	3-10	305	301	0.13
	4-12	12	144	0.05
	5-14	305	301	0.13
	6-16	244	306	0.07

Reaction Summary (lb, u.n.o.)

Jnt	XLocation	Model	Maximum	LDF	Minimum	LDF	Brg_Width	Req_Width	Type	Mat	PSI
8	1-05-09	Pin	224	1.25	-232	1.60	18-09-06	02-02	Wall	SPF	425~
10	5-05-09	Pin	305	1.25	-301	1.60	18-09-06	02-02	Wall	SPF	425~
14	13-05-09	Pin	304	1.25	-302	1.60	18-09-06	02-02	Wall	SPF	425~
16	17-05-09	Pin	223	1.25	-232	1.60	18-09-06	02-02	Wall	SPF	425~
Horizontal @ Jnt 12:			334	1.60	-334	1.60					
Jnt 1 - Jnt 7 (plf):			15	1.25	0						
Reactions not shown:			< 400		> -150						
~ Includes adjustment for Bearing Area Factor, Cb.											

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 9-05-09 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

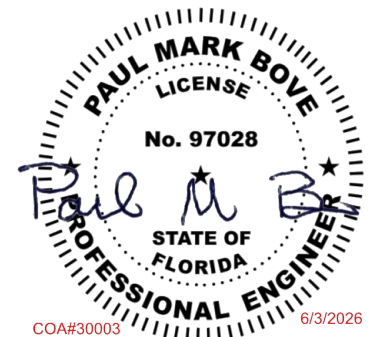
- Valley Truss application only.
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	12-14
Vert DL	L/120	L/999(-0.00)	12-14
Vert CR	L/180	L/999(-0.00)	12-14
Horz LL	0.75in	(0.00) @Jt 7	
Horz CR	1.25in	(0.00) @Jt 7	

Bracing Summary

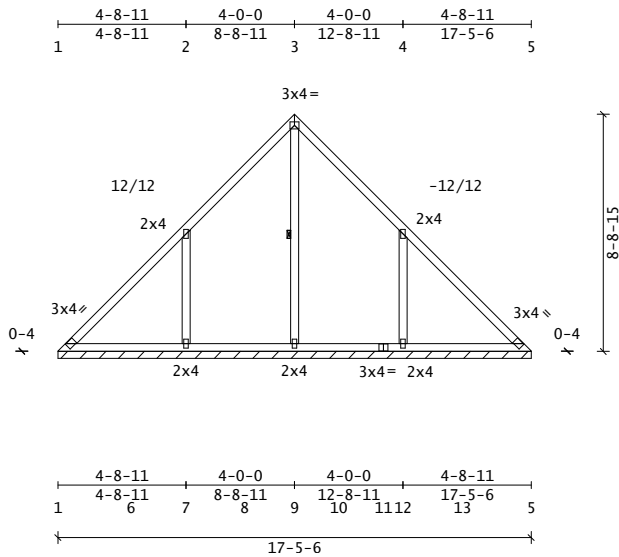
Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): Continuous Lateral Restraint;
Sgl: 12-4



COA#30003

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	1.0	Lbr	1.25	N/A	1.60
Total	31.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL: Yes
20 psf BCLL Limited Storage: Yes
200 lb Moving BCLL Access: No
300 lb Moving TCCL Maintenance: No
300 lb TCCL/BCLL Person Loading: No
2000 lb Moving TCCL Office: No
Partial Length TCCL: Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	2 - 3	0.41
Max CSI in BC PANEL	1 - 6	0.27
Max CSI in Web	7 - 2	0.10

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	143	227	0.38
	2- 3	168	180	0.41
	3- 4	168	180	0.41
	4- 5	143	227	0.38
BC	1- 6	210	98	0.27
	5-13	210	98	0.27
	6- 7	210	98	0.10
	7- 8	210	98	0.05
	8- 9	210	98	0.04
	9-10	210	98	0.04
	10-11	210	98	0.04
	11-12	210	98	0.05
	12-13	210	98	0.10
Web	2- 7	323	408	0.10
	3- 9	4	138	0.04
	4-12	323	408	0.10

Reaction Summary (lb, u.n.o.)

Jnt	XLocation	Model	Maximum	LDF	Minimum	LDF	Brg_Width	Req_Width	Type	Mat	PSI
7	4-09-09	Pin	310	1.25	-306	1.60	17-05-06	02-00	Wall	SPF	425~
12	12-09-09	Pin	305	1.25	-303	1.60	17-05-06	02-00	Wall	SPF	425~
Horizontal @ Jnt 9:			309	1.60	-309	1.60					
Jnt 1 - Jnt 5 (plf):			37	1.25	0						
Reactions not shown:			< 400	> -150							
~ Includes adjustment for Bearing Area Factor, Cb.											

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 8-09-09 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Installation Notes

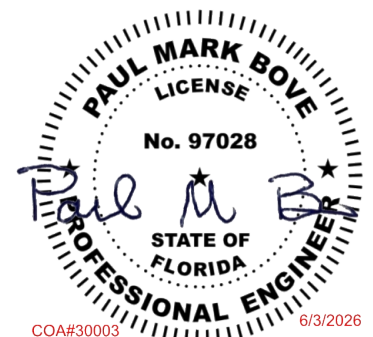
- Valley Truss application only.
- Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	1- 6
Vert DL	L/120	L/999(-0.00)	1- 6
Vert CR	L/180	L/999(-0.01)	1- 6
Horz LL	0.75in	(0.01)	@Jt 1
Horz CR	1.25in	(0.01)	@Jt 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): Continuous Lateral Restraint;
Sgl: 9-3



COA#30003

6/3/2026

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



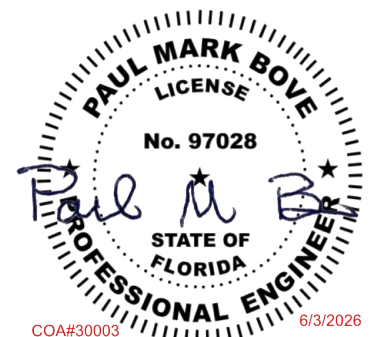
Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Plies:	1	SID:	0004280740
Spacing:	2-00-00 o.c.	TID:	318742
Quantity:	1	Date:	06 / 03 / 26
Weight:	77.0 lb	Page:	1 of 1

The diagram shows a roof truss structure with the following dimensions and specifications:

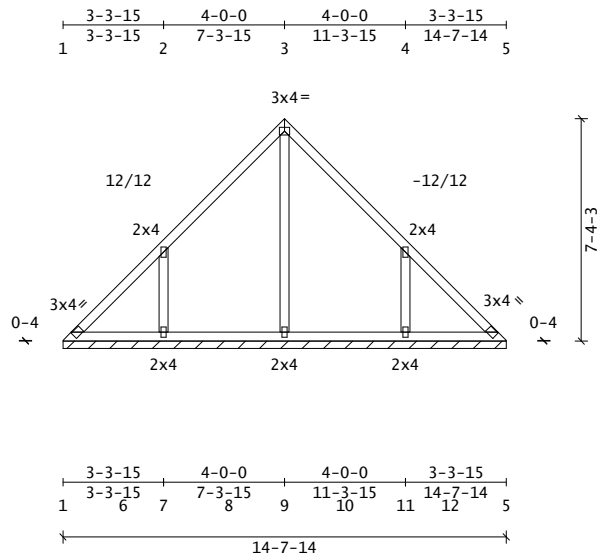
- Roof Pitch:** 12/12 (rise over run).
- Truss Members:**
 - Top Chords: 3x4
 - Bottom Chords: 2x4
 - Vertical Posts: 2x4
 - Diagonal Bracing: 3x4
- Dimensions:**
 - Span: 16'-0" (16'-0" - 0" - 11")
 - Height: 8'-0" (8'-0" - 0" - 11")
 - Overhang: 4'-0" (4'-0" - 0" - 11")
- Material Specifications:**
 - 3x4: 3x4
 - 2x4: 2x4
 - 3x4: 3x4

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

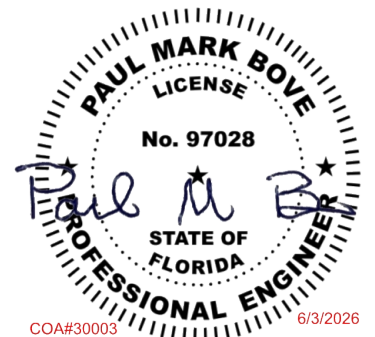
TC Plate offsets (X,Y): None



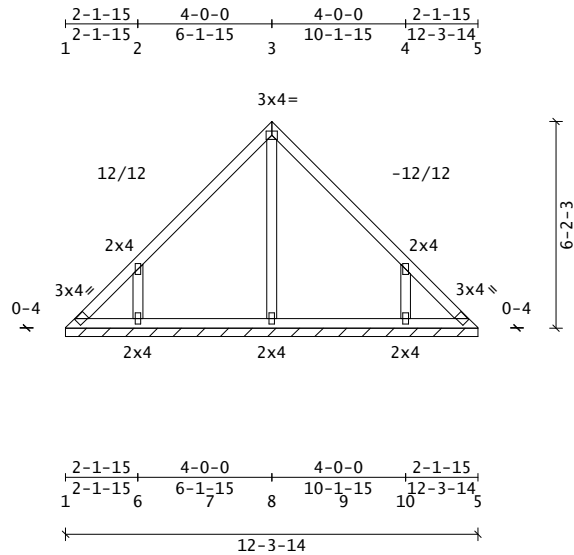
BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 7 3-04-13 Pin 273 1.25 -267 1.60 14-07-14 01-11 Wall SPF 425~ 11 11-04-13 Pin 273 1.25 -267 1.60 14-07-14 01-11 Wall SPF 425~ Horizontal @ Jnt 9: 255 1.60 -255 1.60 Jnt 1 - Jnt 5 (plf): 35 1.25 0 Reactions not shown: < 400 > -150 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 8- 9 Vert DL L/120 L/999(-0.00) 8- 9 Vert CR L/180 L/999(-0.00) 8- 9 Horz LL 0.75in (0.00) @Jt 5 Horz CR 1.25in (0.01) @Jt 5
Member Forces Summary (lb) Max CSI in TC PANEL 2 - 3 0.34 Max CSI in BC PANEL 1 - 6 0.10 Max CSI in Web 9 - 3 0.12	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 7-04-13 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details. · Dead loads have been slope adjusted for any pitch 12.00/12 or greater.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. · Lumber and plating have been applied symmetrically.	Installation Notes · Valley Truss application only.	



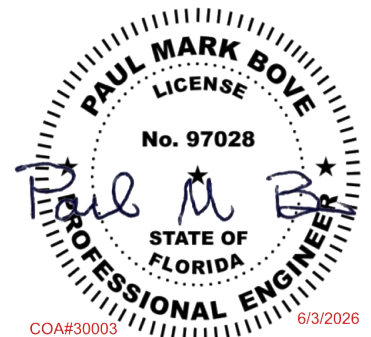
TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

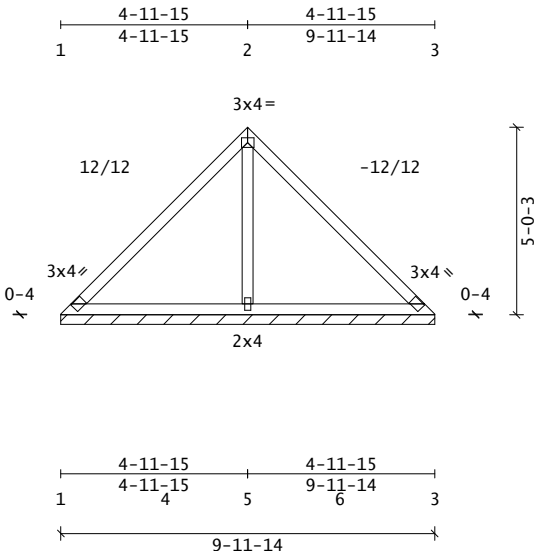
FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Jnt XLocation Model Maximum LDF Minimum LDF Brg_Width Req_Width Type Mat PSI 6 2-02-13 Pin 256 1.25 -250 1.60 12-03-14 01-08 Wall SPF 425~ 10 10-02-13 Pin 256 1.25 -250 1.60 12-03-14 01-08 Wall SPF 425~ Horizontal @ Jnt 8: 211 1.60 -211 1.60 Jnt 1 - Jnt 5 (plf): 28 1.25 0 Reactions not shown: < 400 > -150 ~ Includes adjustment for Bearing Area Factor, Cb.	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 6- 7 Vert DL L/120 L/999(-0.00) 6- 7 Vert CR L/180 L/999(-0.00) 6- 7 Horz LL 0.75in (0.00) @Jt 5 Horz CR 1.25in (0.00) @Jt 5
Member Forces Summary (lb) Max CSI in TC PANEL 2 - 3 0.34 Max CSI in BC PANEL 1 - 6 0.03 Max CSI in Web 6 - 2 0.10	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 6-02-13 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details. · Dead loads have been slope adjusted for any pitch 12.00/12 or greater.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. · Lumber and plating have been applied symmetrically.	Installation Notes · Valley Truss application only.	



Project: P-1038 - Martinez		Truss ID: V16	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280743
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 03 / 26
Project Note(s):		Weight: 40.0 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A 1.60
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.43
Max CSI in BC PANEL	1 - 4	0.34
Max CSI in Web	5 - 2	0.05

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	91	210	0.43
	2- 3	91	210	0.43
BC	1- 4	108	57	0.34
	3- 6	108	57	0.34
	4- 5	108	57	0.12
	5- 6	108	57	0.12
Web	2- 5	30	155	0.05

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 5:	166 1.60	-166 1.60
Jnt 1 - Jnt 3 (plf):	70 1.25	26 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 5-00-13 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

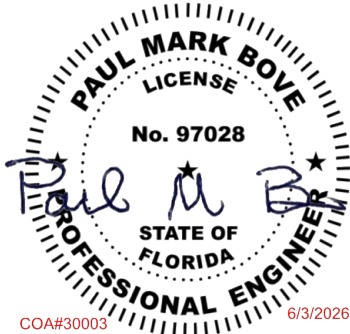
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.01)	1- 4
Vert DL	L/120	L/999(-0.00)	1- 4
Vert CR	L/180	L/999(-0.01)	1- 4
Horz LL	0.75in	(0.01)	@Jt 3
Horz CR	1.25in	(0.02)	@Jt 3

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



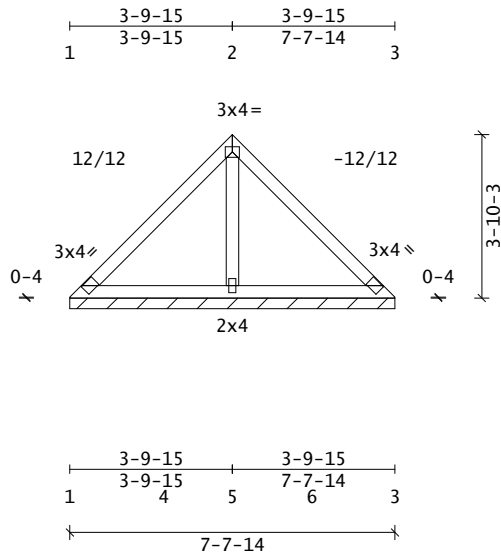
COA#30003

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 ASCE7-22 TPI 1-2014						
PSF	Live	Dead	Load	Duration	Factors	
TC	20.0	10.0	Live	Snow	Wind	
BC	0.0	1.0	Lbr	1.25	N/A	1.60
Total	31.0	Plt	1.25	N/A	1.60	
OH Soffit	2.0					
Spacing:	2-00-00 o.c.	Plies:	1			
Repetitive Member Increase:	Yes					
Green Lbr:	No	Temp:	T ≤ 100°			
Treated Lbr:	No	Incised Lbr:	No			
Seasoned Lbr:	Yes	Wet Service:	No			

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.24
Max CSI in BC PANEL	1 - 4	0.21
Max CSI in Web	5 - 2	0.02

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	63	154	0.24
	2- 3	63	154	0.24
BC	1- 4	80	40	0.21
	3- 6	80	40	0.21
	4- 5	80	40	0.06
	5- 6	80	40	0.06
Web	2- 5	28	116	0.02

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 5:	121 1.60	-121 1.60
Jnt 1 - Jnt 3 (plf):	70 1.25	26 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 3-10-13 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

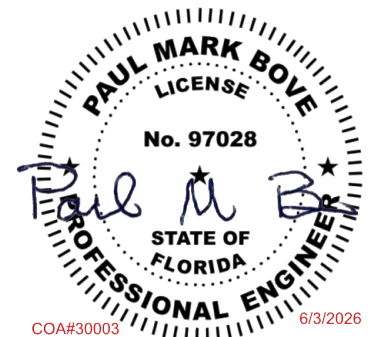
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	1- 4
Vert DL	L/120	L/999(-0.00)	1- 4
Vert CR	L/180	L/999(-0.00)	1- 4
Horz LL	0.75in	(0.01)	@Jt 1
Horz CR	1.25in	(0.01)	@Jt 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;

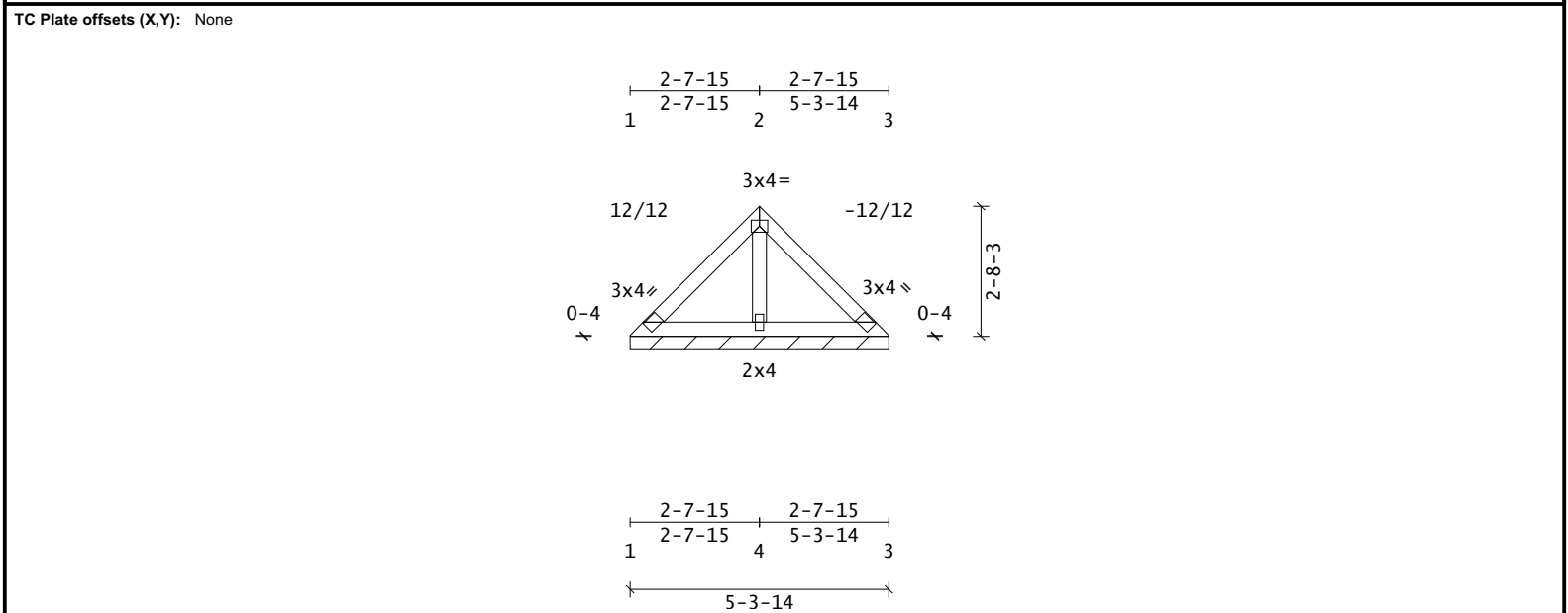


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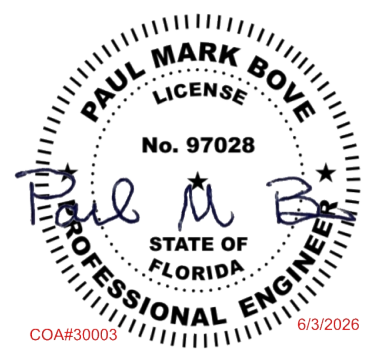
Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez		Truss ID: V18	
Truss Mfr Customer: Turnsole Builders		Plies: 1	SID: 0004280745
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 03 / 26
Project Note(s):		Weight: 21.5 lb	Page: 1 of 1



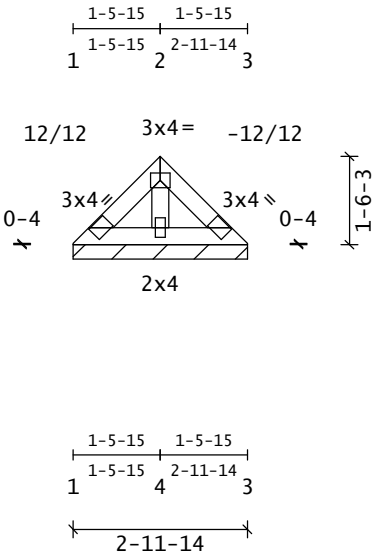
FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary TC 2x4 SYP #2 BC 2x4 SYP #2 Webs 2x4 SYP #2	Reaction Summary (lb, u.n.o.) Maximum LDF Minimum LDF Horizontal @ Jnt 4: 76 1.60 -76 1.60 Jnt 1 - Jnt 3 (plf): 70 1.25 27 1.60 Reactions not shown: < 400 > -150	Deflection Summary Creep (Kcr) = 2.00 TrussSpan Limit Actual(in) Location Vert LL L/240 L/999(-0.00) 1- 4 Vert DL L/120 L/999(-0.00) 1- 4 Vert CR L/180 L/999(-0.00) 1- 4 Horz LL 0.75in (0.00) @Jt 1 Horz CR 1.25in (0.00) @Jt 1
Member Forces Summary (lb) Max CSI in TC PANEL 1 - 2 0.13 Max CSI in BC PANEL 1 - 4 0.10 Max CSI in Web 4 - 2 0.01	Loads Summary · User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 2-08-13 are based on 1.00 full and 0.00 reduced load factors. · See Loadcase Report for load combinations and additional details. · Dead loads have been slope adjusted for any pitch 12.00/12 or greater.	Bracing Summary Top Chord: Structural Wood Sheathing Bot Chord: Gypsum Board Sheathing Web(s): None;
Member Forces Summary (lb) Mbr Jnt-Jnt Tens Comp CSI TC 1- 2 38 99 0.13 2- 3 38 99 0.13 BC 1- 4 51 24 0.10 3- 4 51 24 0.10 Web 2- 4 25 76 0.01	Design Notes · Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. · Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. · Lumber and plating have been applied symmetrically.	
	Installation Notes · Valley Truss application only.	



Project: P-1038 - Martinez		Truss ID: V19	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280746
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 01 / 26
Project Note(s):		Weight: 12.2 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A 1.60
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone a = 5-11-10
Design: MWFRS/C&C Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.02
Max CSI in BC PANEL	1 - 4	0.02
Max CSI in Web	4 - 2	0.01

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	15	43	0.02
	2- 3	15	43	0.02
BC	1- 4	19	8	0.02
	3- 4	19	8	0.02
Web	2- 4	15	35	0.01

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 4:	32 1.60	-32 1.60
Jnt 1 - Jnt 3 (plf):	70 1.25	27 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 1-06-13 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.
- Dead loads have been slope adjusted for any pitch 12.00/12 or greater.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

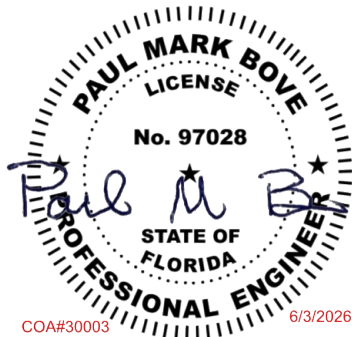
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	1- 4
Vert DL	L/120	L/999(-0.00)	1- 4
Vert CR	L/180	L/999(-0.00)	1- 4
Horz LL	0.75in	(0.00)	@Jt 1
Horz CR	1.25in	(0.00)	@Jt 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



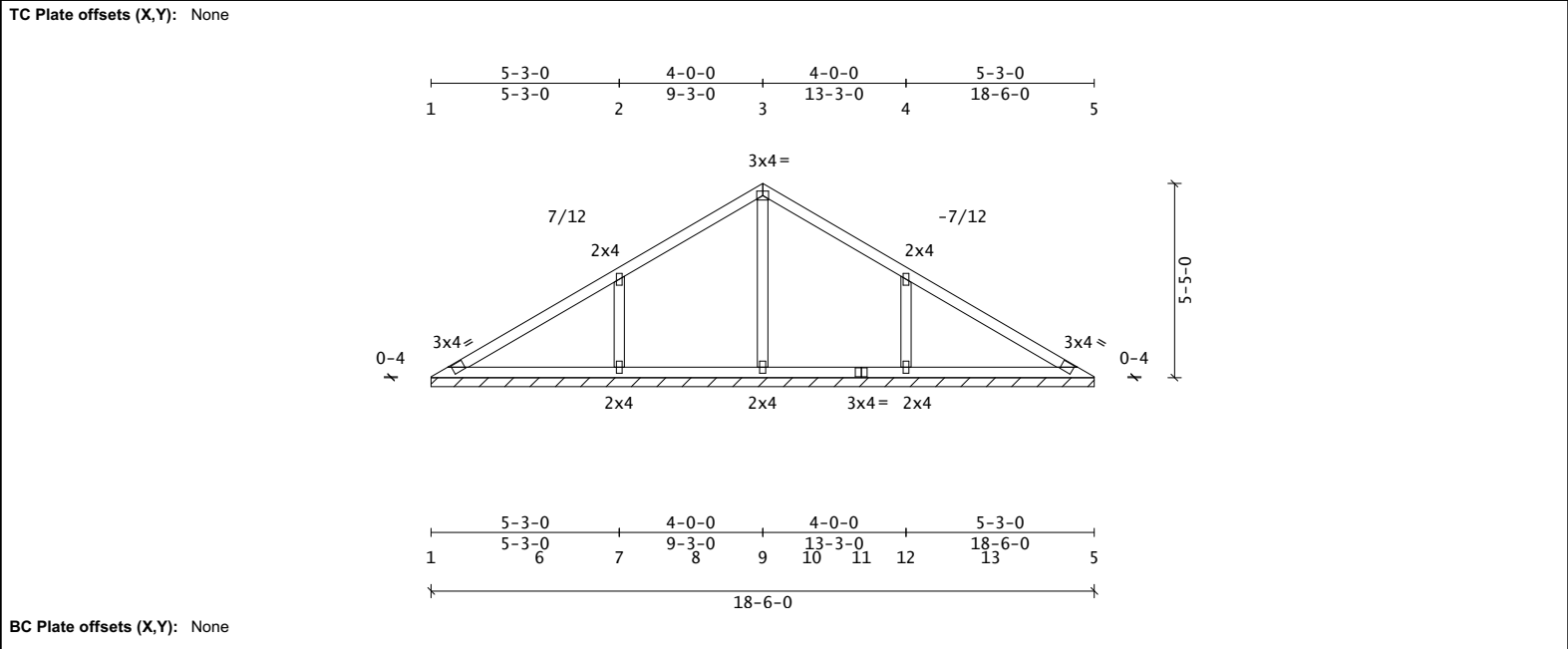
COA#30003

6/3/2026

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

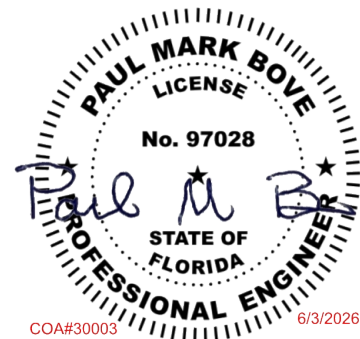


Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com



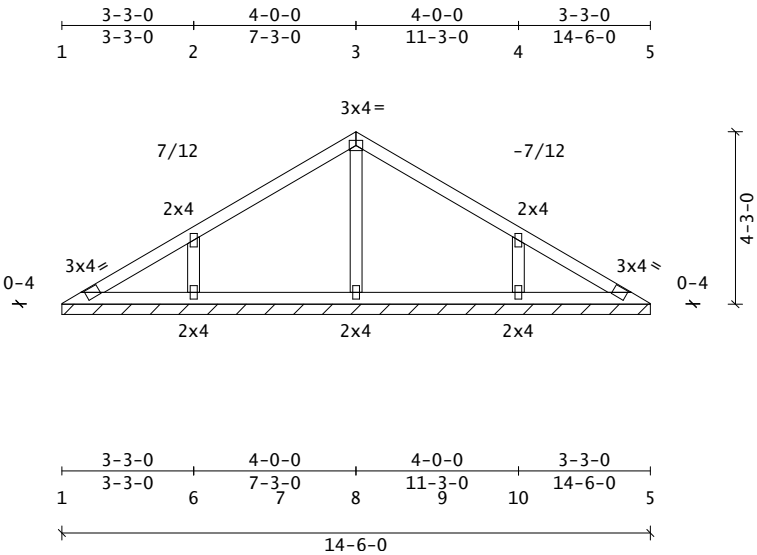
FBC-2023 ASCE7-22 TPI 1-2014 PSF Live Dead Load Duration Factors TC 20.0 10.0 Live Snow Wind BC 0.0 1.0 Lbr 1.25 N/A 1.60 Total 31.0 Plt 1.25 N/A 1.60 OH Soffit 2.0 Spacing: 2-00-00 o.c. Plies: 1 Repetitive Member Increase: Yes Green Lbr: No Temp: T ≤ 100° Treated Lbr: No Incised Lbr: No Seasoned Lbr: Yes Wet Service: No	Roof Snow Load (Pf) = N/A	Wind Speed (Vult) = 140 mph Risk Category: II (I = 1.00) Exposure Category: C Bldg Dims: L = 61.0 ft B = 59.7 ft MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00 Bldg Type: Enclosed Wind DL (psf): TC = 5.0 BC = 1.0 End Vertical Exposed: L = Yes R = Yes C&C Location: End Zone a = 5-11-10 Design: MWFRS/C&C Reactions: MWFRS Net Uplift: 0.0 psf	Additional Live Load Checks 10 psf Non-Concurrent BCLL: Yes 20 psf BCLL Limited Storage: Yes 200 lb Moving BCLL Access: No 300 lb Moving TCLL Maintenance: No 300 lb TCLL/BCLL Person Loading: No 2000 lb Moving TCLL Office: No Partial Length TCLL: Yes Plate Tolerance Checks Fabrication: 20% (Cq = 0.80), u.n.o. Rotation: +/- 10 degrees, u.n.o.
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Material Summary					Reaction Summary (lb, u.n.o.)												Deflection Summary								
TC	2x4	SYP	#2		Jnt	X	Location	Model	Maximum	LDF	Minimum	LDF	Brg_Width	Req_Width	Type	Mat	PSI	Creep (Kcr) = 2.00							
BC	2x4	SYP	#2		7		5-04-08	Pin	281	1.25	-208	1.60	18-06-00	01-13	Wall	SPF	425~	TrussSpan	Limit	Actual(in)	Location				
Webs	2x4	SYP	#2		12		13-04-08	Pin	279	1.25	-207	1.60	18-06-00	01-13	Wall	SPF	425~	Vert LL	L/240	L/999(-0.00)	1- 6				
					Horizontal @ Jnt 9: 163 1.60 -163 1.60																				
					Jnt 1 - Jnt 5 (plf):	31	1.25		8	1.60															
					Reactions not shown: < 400 > -150																				
					~ Includes adjustment for Bearing Area Factor, Cb.																				
Member Forces Summary (lb)					Loads Summary												Bracing Summary								
Max CSI in TC PANEL	2 - 3	0.27																		Top Chord: Structural Wood Sheathing					
Max CSI in BC PANEL	1 - 6	0.13																		Bot Chord: Gypsum Board Sheathing					
Max CSI in Web	9 - 3	0.07																		Web(s): None;					
					Design Notes																				
					· User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 9-04-08 are based on 1.00 full and 0.00 reduced load factors.																				
					· See Loadcase Report for load combinations and additional details.																				
					Installation Notes																				
					· Valley Truss application only.																				
					Member Forces Summary (lb)																				
Mbr	Jnt-Jnt	Tens	Comp	CSI																					
TC	1- 2	71	98	0.25																					
	2- 3	125	112	0.27																					
	3- 4	125	112	0.27																					
	4- 5	71	98	0.25																					
BC	1- 6	108	54	0.13																					
	5-13	108	54	0.13																					
	6- 7	108	54	0.05																					
	7- 8	108	54	0.02																					
	8- 9	108	54	0.03																					
	9-10	108	54	0.03																					
	10-11	108	54	0.01																					
	11-12	108	54	0.02																					
	12-13	108	54	0.05																					
Web	2- 7	223	290	0.06																					
	3- 9	0	159	0.07																					
	4-12	223	290	0.06																					



Project: P-1038 - Martinez		Truss ID: V21	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280748
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 01 / 26
Project Note(s):		Weight: 54.2 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A 1.60
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	3 - 4	0.21
Max CSI in BC PANEL	1 - 6	0.03
Max CSI in Web	6 - 2	0.06

Reaction Summary (lb, u.n.o.)

Jnt	XLocation	Model	Maximum	LDF	Minimum	LDF	Brg_Width	Req_Width	Type	Mat	PSI
6	3-04-08	Pin	247	1.25	-180	1.60	14-06-00	01-08	Wall	SPF	425~
10	11-04-08	Pin	247	1.25	-180	1.60	14-06-00	01-08	Wall	SPF	425~
Horizontal @ Jnt 8:			123	1.60	-123	1.60					
Jnt 1 - Jnt 5 (plf):			27	1.25	6	1.60					
Reactions not shown:			< 400		> -150						
~ Includes adjustment for Bearing Area Factor, Cb.											

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 7-04-08 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

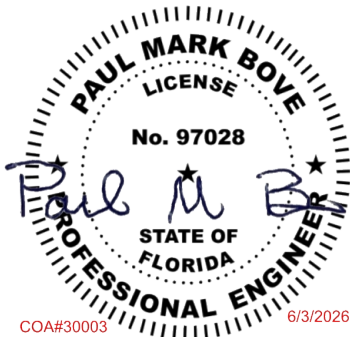
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	10- 5
Vert DL	L/120	L/999(-0.00)	10- 5
Vert CR	L/180	L/999(-0.00)	10- 5
Horz LL	0.75in	(0.00)	@Jt 1
Horz CR	1.25in	(0.00)	@Jt 1

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



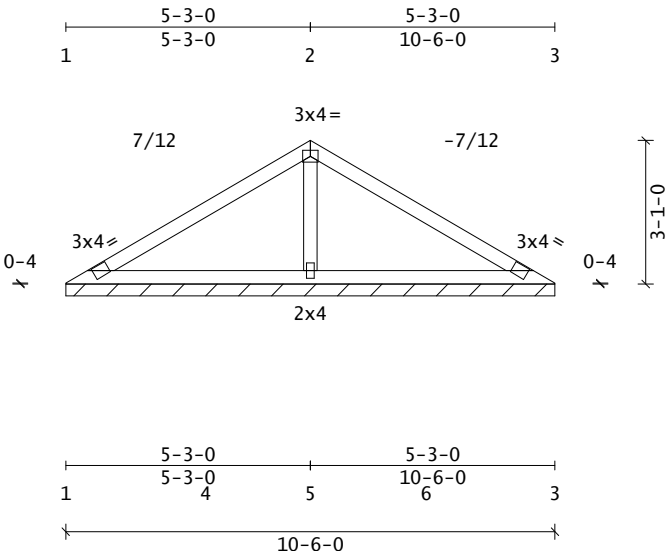
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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez		Truss ID: V22	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280749
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 01 / 26
Project Note(s):		Weight: 34.7 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A 1.60
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
Exposure Category: C
Bldg Dims: L = 61.0 ft B = 59.7 ft
MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
Bldg Type: Enclosed
Wind DL (psf): TC = 5.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
C&C Location: End Zone | a = 5-11-10
Design: MWFRS/C&C | Reactions: MWFRS
Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.26
Max CSI in BC PANEL	1 - 4	0.21
Max CSI in Web	5 - 2	0.04

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	70	136	0.26
	2- 3	70	136	0.26
BC	1- 4	65	32	0.21
	3- 6	65	32	0.21
	4- 5	65	32	0.08
	5- 6	65	32	0.08
Web	2- 5	92	193	0.04

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 5:	84 1.60	-84 1.60
Jnt 1 - Jnt 3 (plf):	62 1.25	31 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 5-04-08 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

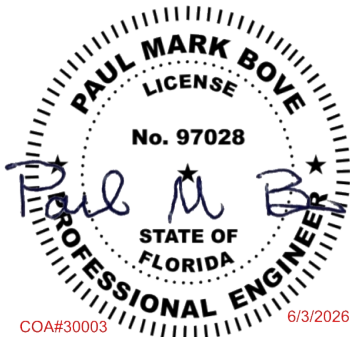
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00		
TrussSpan Limit	Actual(in)	Location
Vert LL L/240	L/999(-0.01)	1- 4
Vert DL L/120	L/999(-0.00)	1- 4
Vert CR L/180	L/999(-0.01)	1- 4
Horz LL 0.75in	(0.01) @Jt 3	
Horz CR 1.25in	(0.01) @Jt 3	

Bracing Summary

Top Chord: Structural Wood Sheathing
Bot Chord: Gypsum Board Sheathing
Web(s): None;



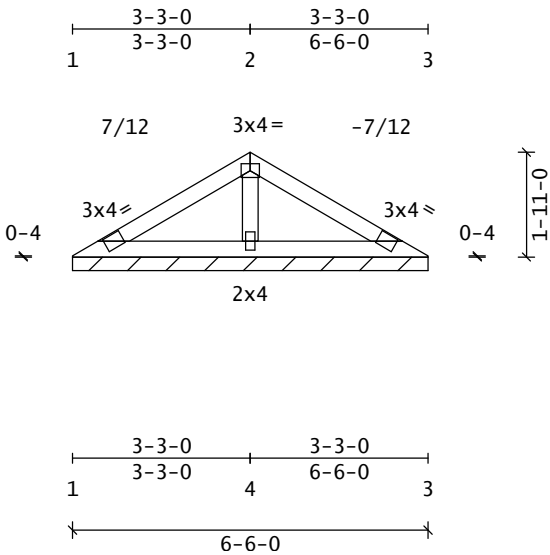
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Component Solutions
Truss Studio V
2026.3.0.65
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

Project: P-1038 - Martinez		Truss ID: V23	
Truss Mfr Customer: Turnsole Builders	19 Lumber Inc	Plies: 1	SID: 0004280750
Truss Mfr Contact: Monica Register		Spacing: 2-00-00 o.c.	TID: 318742
Project Location: Project location has not been provided		Quantity: 1	Date: 06 / 01 / 26
Project Note(s):		Weight: 21.4 lb	Page: 1 of 1

TC Plate offsets (X,Y): None



BC Plate offsets (X,Y): None

FBC-2023 | ASCE7-22 | TPI 1-2014

PSF	Live	Dead	Load	Duration	Factors
TC	20.0	10.0	Live	Snow	Wind
BC	0.0	1.0	Lbr	1.25	N/A 1.60
Total	31.0	Plt	1.25	N/A	1.60
OH Soffit	2.0				
Spacing:	2-00-00 o.c.	Plies:	1		
Repetitive Member Increase:	Yes				
Green Lbr:	No	Temp:	T ≤ 100°		
Treated Lbr:	No	Incised Lbr:	No		
Seasoned Lbr:	Yes	Wet Service:	No		

Roof Snow Load (Pf) = N/A

Wind Speed (Vult) = 140 mph

Risk Category: II (I = 1.00)
 Exposure Category: C
 Bldg Dims: L = 61.0 ft B = 59.7 ft
 MRH(h)= 18.0ft Kzt = 1.00 Ke = 1.00
 Bldg Type: Enclosed
 Wind DL (psf): TC = 5.0 BC = 1.0
 End Vertical Exposed: L = Yes R = Yes
 C&C Location: End Zone | a = 5-11-10
 Design: MWFRS/C&C | Reactions: MWFRS
 Net Uplift: 0.0 psf

Additional Live Load Checks

10 psf Non-Concurrent BCLL:	Yes
20 psf BCLL Limited Storage:	Yes
200 lb Moving BCLL Access:	No
300 lb Moving TCLL Maintenance:	No
300 lb TCLL/BCLL Person Loading:	No
2000 lb Moving TCLL Office:	No
Partial Length TCLL:	Yes

Plate Tolerance Checks

Fabrication: 20% (Cq = 0.80), u.n.o.
 Rotation: +/- 10 degrees, u.n.o.

Material Summary

TC	2x4	SYP	#2
BC	2x4	SYP	#2
Webs	2x4	SYP	#2

Member Forces Summary (lb)

Max CSI in TC PANEL	1 - 2	0.11
Max CSI in BC PANEL	1 - 4	0.07
Max CSI in Web	4 - 2	0.03

Mbr	Jnt-Jnt	Tens	Comp	CSI
TC	1- 2	38	75	0.11
	2- 3	38	75	0.11
BC	1- 4	39	16	0.07
	3- 4	39	16	0.07
Web	2- 4	58	108	0.03

Reaction Summary (lb, u.n.o.)

	Maximum LDF	Minimum LDF
Horizontal @ Jnt 4:	44 1.60	-44 1.60
Jnt 1 - Jnt 3 (plf):	62 1.25	31 1.60
Reactions not shown:	< 400	> -150

Loads Summary

- User-defined unbalanced Top Chord Live Loads (TCLL) occurring at 3-04-08 are based on 1.00 full and 0.00 reduced load factors.
- See Loadcase Report for load combinations and additional details.

Design Notes

- Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.
- Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
- Lumber and plating have been applied symmetrically.

Installation Notes

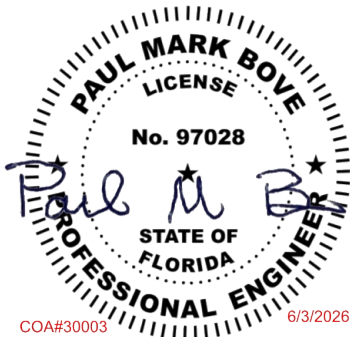
- Valley Truss application only.

Deflection Summary

Creep (Kcr) = 2.00			
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	4- 3
Vert DL	L/120	L/999(-0.00)	4- 3
Vert CR	L/180	L/999(-0.00)	4- 3
Horz LL	0.75in	(0.00)	@Jt 3
Horz CR	1.25in	(0.00)	@Jt 3

Bracing Summary

Top Chord: Structural Wood Sheathing
 Bot Chord: Gypsum Board Sheathing
 Web(s): None;



COA#30003

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



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 CSHelp@strongtie.com

This detail provides minimum connection requirements between a cap truss and a base truss of a piggyback assembly for assemblies that meet the following conditions and design requirements:

- the cap truss has continuous bearing or bearings at 2' o.c. max.;
- the cap truss contains vertical web members at 4' o.c. max.;
- the cap trusses are spaced no greater than 24" o.c.;
- the pitch does not exceed 12/12;
- the cap truss span does not exceed 100'; for spans > 36', see Table 2 Footnote 5
- the cap truss supports no point loads or drag loads (see detail TD-CPS-0002 for drag load connection requirements)

Design Requirements:

Max. Wind Speed: 140 mph (nominal) Load Duration Factor: 1.6
Max. Mean Roof Height: 30' Lumber: SPF or Better
Exp. Category: B or C (Min. SG = 0.42)

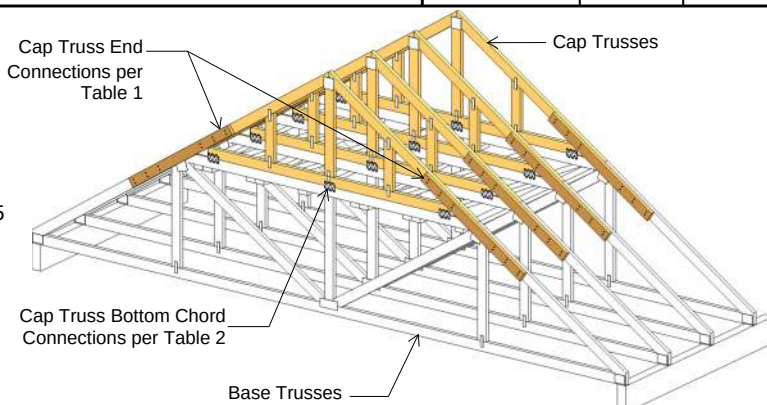


TABLE 1 - Connection Requirements At Each End of Cap Truss (One Face Only)

Cap Truss Span	Max. Wind Speed (mph)		End Connection Options	Fasteners ¹
	Nominal	Ultimate		
Up to 18'	100	125	Sheathing - See Note 2	---
	140	180	4' 2x Scab	16-10d nails
			LSTA18 or CS20 Strap	14-10dx1.5 nails
18' < Span <= 100'	120	150	4' 2x Scab	16-10d nails
	130	160	LSTA18 or CS20 Strap	14-10dx1.5 nails
	140	180	6' 2x Scab	24-10d nails
			LSTA30 or CS18 Strap	10-SDS 1/4x3
				20-10dx1.5

See footnotes below

TABLE 2 - Connection Requirements Along Cap Truss Bottom Chord

Condition	Max. Wind Speed (mph)		Connection Options Along BC	Fasteners
	Nominal	Ultimate		
With 1.5" Gap (See Fig 1)	140	180	Nails into purlins installed on base truss at 24" o.c. (max)	Purlin-to-Base: 2-10d nails (ea purlin) Cap-to-purlin: 2-10d toe-nails (ea purlin)
			(3) 7"x7"x7/16" Plywood/OSB Gussets (each face)	6-6d nails per Gusset (see note 1 & 5)
			(3) LTP5 Framing Angles (one face)	8-8dx1.5 nails per LTP5 (see note 1 & 5)
No Gap (See Fig 2)	130	160	(2) LTP4 Framing Angles (one face)	12-8dx1.5 nails per LTP4 (see note 1 & 5)
	140	180	Toe-Nails @ 24" o.c.	2-10d toe-nails along BC @ 24" o.c.
			(3) 7"x7"x7/16" Plywood/OSB Gussets (each face)	6-6d nails per Gusset (see note 1 & 5)
			(3) LTP4 Framing Angles (one face)	12-8dx1.5 nails per LTP4 (see note 1 & 5)

Figure 1 - Typical Piggyback Assembly with 1 1/2" Gap Between Cap and Base

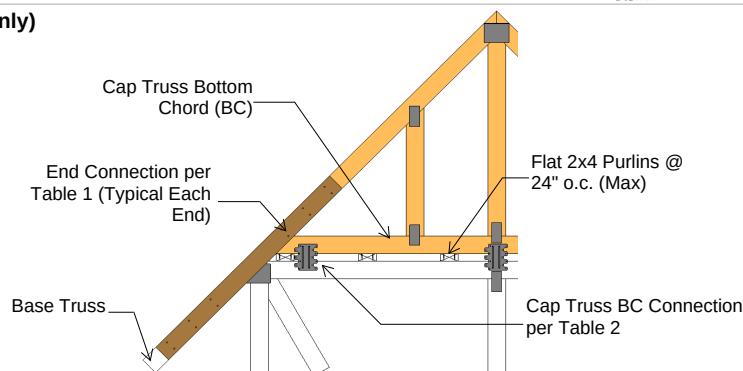
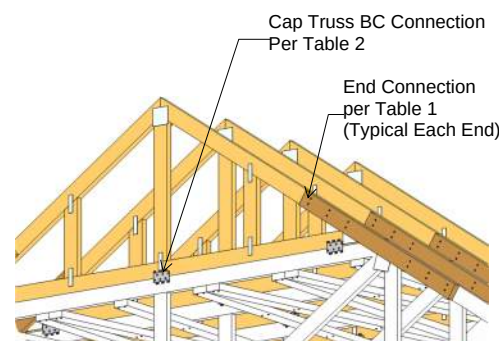


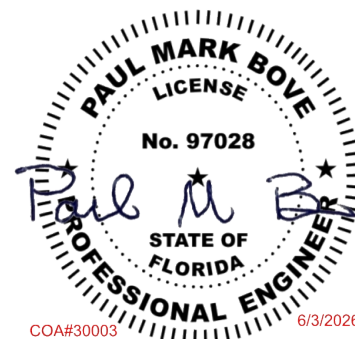
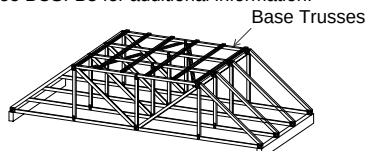
Figure 2 - Typical Piggyback Assembly with No Gap Between Cap and Base



1. Where noted, install half of the specified fasteners in each member being connected.
2. An additional end connection is not required for wind speeds up to 100 mph (nominal) and cap truss spans up to 18' if the sheathing is continuous and extends at least 12" beyond the intersection.
3. When using (2) connectors along the cap truss BC, install one at each end of the bottom chord; when using (3) connectors, install one on each end and one in the center of the bottom chord.
4. NAILS: 10d=0.148" dia.x3" long, 10dx1.5=0.148" dia.x1½" long, 8dx1.5=0.131" dia.x1½" long, 6d=0.113" dia.x2" long
5. For piggyback truss spans greater than 36' and less than or equal to 100', where there is a 1.5" gap between BC of cap truss and TC of base truss, attach BC of cap truss to the TC of base truss with 7"x7"x7/16" Plywood/OSB Gussets (each face) with 6-6d nails per gusset or LTP5 Framing Angles (one face) with 8-8dx1.5" nails per LTP5 under each vertical web at 4' on center. Where there is no gap between BC of cap truss and TC of base truss, attach BC of cap truss to the TC of base truss with 7"x7"x7/16" Plywood/OSB Gussets (each face) with 6-6d nails per gusset or LTP4 Framing Angles (one face) with 12-8dx1.5" nails per LTP4 under each vertical web at 4' on center. Note: spacing for vertical webs must not exceed 4' on center.

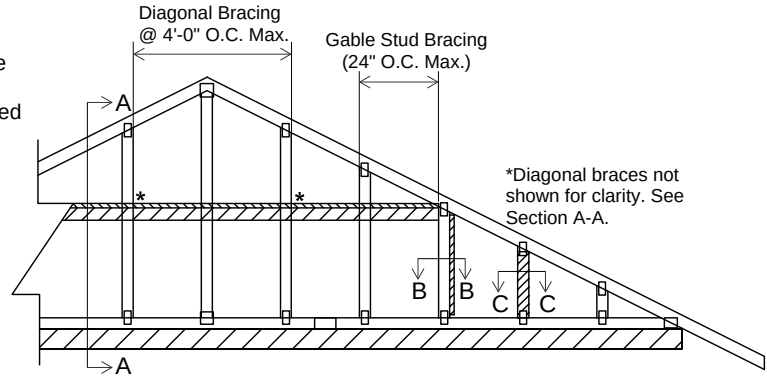
Bracing for the Base Truss Flat Top Chord in a Piggyback Assembly

The flat top chords of the supporting base trusses must be adequately braced to prevent them from buckling out from under the cap trusses. One option for accomplishing this is with flat 2x4 purlins in combination with diagonal bracing that gets repeated at max. 10' intervals. Other methods may be required as specified in the Construction Documents. See BCSI-B3 for additional information.



NOTES:

1. This detail provides bracing/reinforcement options for the gable studs to resist the out-of-plane wind loading. Refer to the individual truss design drawing for bracing/reinforcement requirements for resisting the vertical (in-plane) loads assumed in the design of the gable end frame. Additional bracing/reinforcement at the end of the building and/or at the gable end wall may be required. Refer to the Building Designer/Construction Documents for all gable end frame and roof system bracing requirements. For additional information, see BCSI-B3.
2. This detail does not apply to structural gables.
3. Connection requirements between the gable end frame and the wall to be specified by the Building Designer.
4. The gable end frame must match the profile of the adjacent trusses. Do not use a gable end frame with a flat bottom chord next to trusses with sloped bottom chords, such as scissor or vaulted trusses.



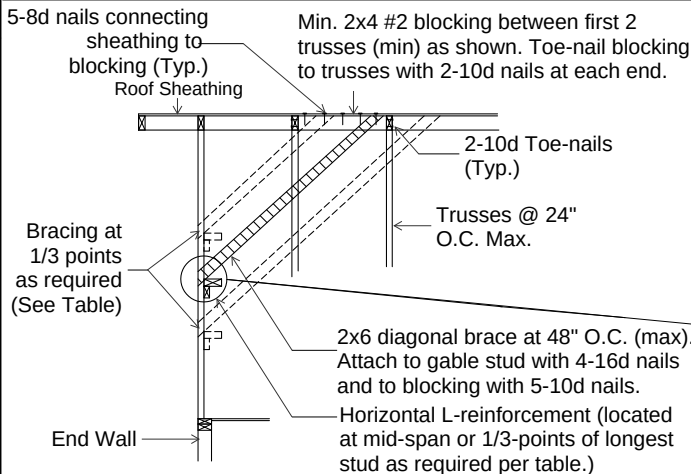
GABLE END WITH STUD BRACING/REINFORCEMENT

MINIMUM GABLE STUD SIZE, SPECIES & GRADE	MAX. GABLE STUD SPACING	WITHOUT BRACE	L- REINFORCEMENT ¹	SCAB REINFORCEMENT ¹	DIAGONAL BRACING @ MID-SPAN ²	DIAGONAL BRACING @ 1/3 POINTS ²
		MAXIMUM STUD LENGTH ³				
2X4 SPF STUD or STANDARD	12" O.C.	4-6-0	7-11-4	9-0-4	9-0-4	13-6-8
	16" O.C.	4-1-0	7-0-4	8-2-4	8-2-4	12-3-8
	24" O.C.	3-5-8	5-8-12	6-11-0	6-11-0	10-4-8

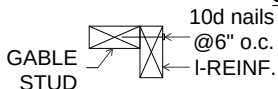
DETAIL LIMITATIONS:

Max. Mean Roof Height: 30'
Category: II
Exposure: B or C
Load Duration Factor: 1.6
Wind Speed: 110 mph Nominal
(140 mph Ultimate)

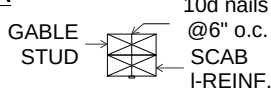
1. L- and Scab Reinforcements shall be minimum 2x4 stud grade and must be a minimum of 90% of the gable stud length. Fasten the reinforcement member to the gable stud with 10d nails @ 6" o.c.
2. Attach horizontal reinforcing member at mid-span (or 1/3 points as required) of the longest stud and install diagonal bracing @ 4' o.c. (max) as shown in Section A-A.
3. Tabulated maximum stud lengths are based on components and cladding wind pressures using the wind design parameters listed in the detail limitations. Gable stud deflection criteria is L/240.



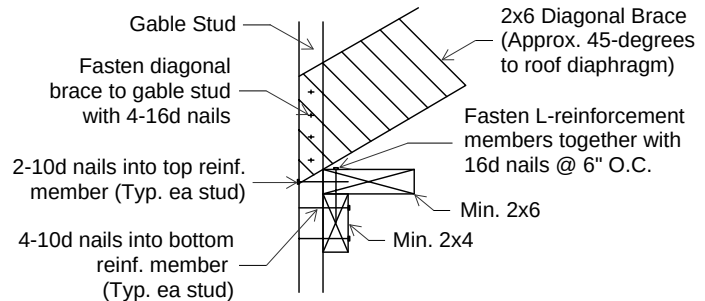
Section A-A



Section B-B



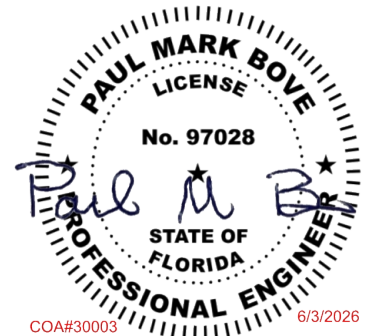
Section C-C



NOTE: Diagonal braces over 6'-3" require a 2x4 T-brace attached to one narrow edge. Diagonal braces over 12'-6" require 2x4s attached on both narrow edges. The braces must cover 90% of the diagonal brace and shall be fastened to the narrow edge with 10d nails at 6" o.c. (min. 3" end distance). When attached on both narrow edges, stagger the nails on each side by 3".

Nail Dimension

16d = 3.5" x 0.162"
10d = 3" x 0.148"
8d = 2.5" x 0.131"

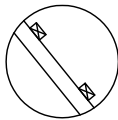
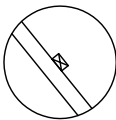


COA#30003

6/3/2026

NOTES:

1. This detail provides web reinforcement options that may be used as an alternative to continuous lateral restraint (CLR) when installing CLR in combination with diagonal bracing is not practical or desired.
2. Refer to the truss design drawing for web lateral restraint requirements. A  on the truss design drawing indicates that continuous lateral restraint is required at the locations shown (either at the midpoint or 1/3-points of the web member). Refer to the tables below for acceptable web reinforcement options that may be used in place of one or two rows of CLR.
3. T-, L-, I- and scab web reinforcements must be the same or better species and grade of the web member as indicated on the truss design drawing.
4. All reinforcements must extend to within 6" of each end of the web member.
5. This detail does not apply to single-ply webs that exceed 14' in length.



1 Row of CLR @ Web Mid-point 2 Rows of CLR @ Web 1/3 points

WEB REINFORCEMENT OPTIONS FOR SINGLE-PLY TRUSSES ¹

Specified Web Member Lateral Restraint (CLRs)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	2x4	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	2x6	---	
	2x8	2x8	2x8	2x8	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6				2-2x6	
	2x8				2-2x8	

WEB REINFORCEMENT OPTIONS FOR 2-PLY TRUSSES ²

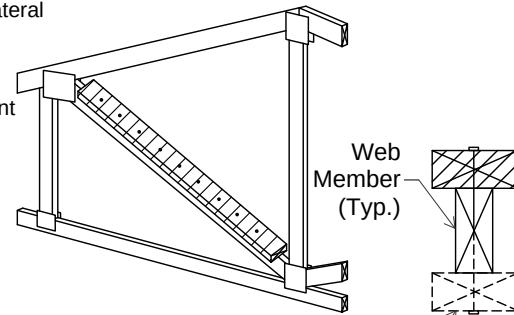
Specified Web Member Lateral Restraint (CLRs)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	---	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	---	---	
	2x8	2x8	2x8	---	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6				2-2x6	
	2x8				2-2x8	

1. The maximum allowable web length for single-ply trusses is 14'.
2. For 2-ply trusses, the reinforcement must be nailed to both plies of the web with the nailing pattern specified in the table.
3. For the scab reinforcement, 2 rows of 10d gun nails @ 6" o.c may be used in place of 16d gun nails for attaching the reinforcement to the web.
4. For I-reinforcement, attach each 2x_member to opposite edges of the web using the nailing pattern specified in the table.

Nail Dimension

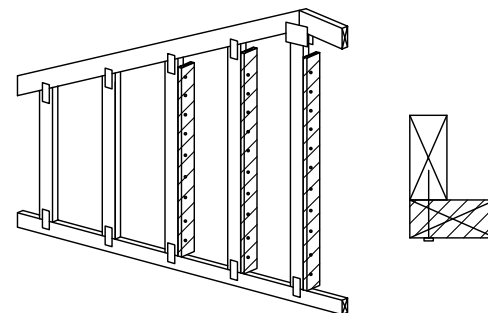
16d = 3.5" x 0.131"

10d = 3" x 0.120"

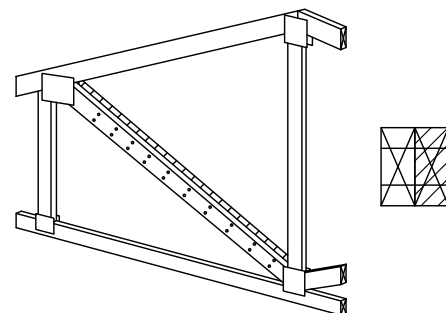


Add member to both edges for I-Reinforcement

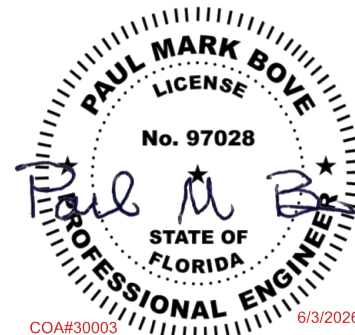
T- Reinforcement
(I-Reinforcement similar)



L- Reinforcement



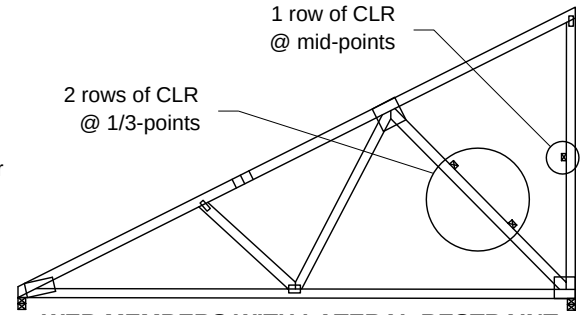
Scab Reinforcement



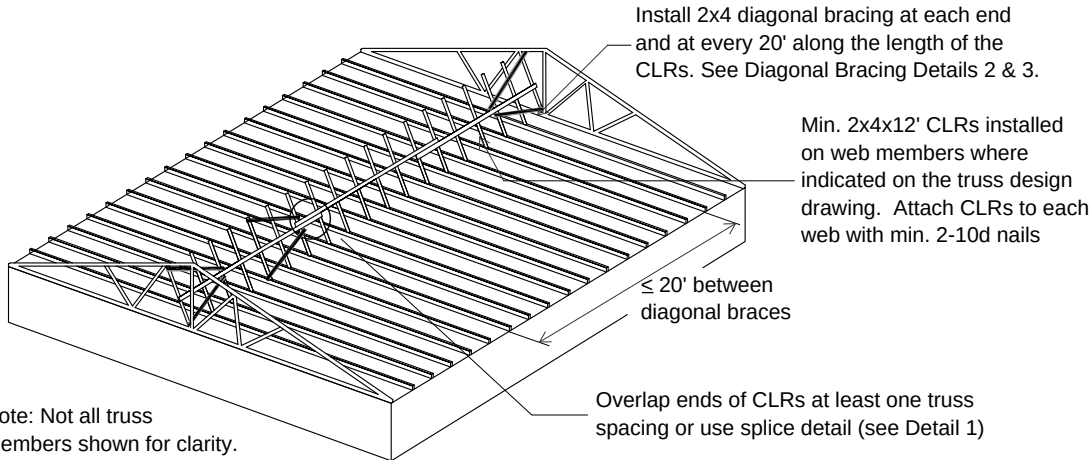
COA#30003

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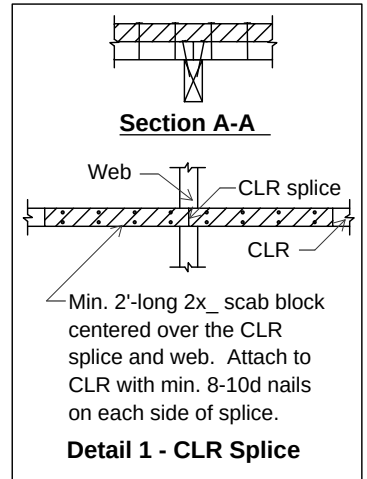
This detail provides information for laterally restraining and bracing web members to prevent lateral buckling using continuous lateral restraints (CLRs) in combination with diagonal bracing. In addition to the CLRs indicated on the truss design drawing, diagonal bracing must be installed as indicated in this detail and BCSI-B3. See WEBREINFORCE for web reinforcement options that may be used as an alternative to this detail when installing CLRs and diagonal bracing is not practical or desired. Properly attached full-length sheathing satisfies (may replace) any bracing requirements specified for end vertical webs. Refer to the Construction Documents for additional bracing requirements. For trusses with spacing greater than 2' o.c. refer to BCSI-B10



WEB MEMBERS WITH LATERAL RESTRAINT

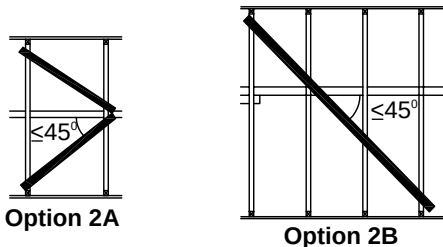


Note: Not all truss members shown for clarity.



Detail 1 - CLR Splice

For webs with one row of CLRs, diagonal bracing shall be installed using Option 2A or 2B. Attach diagonal braces to each truss with min. 2-10d nails.



Detail 2 - Diagonal Bracing for 1 Row of CLRs

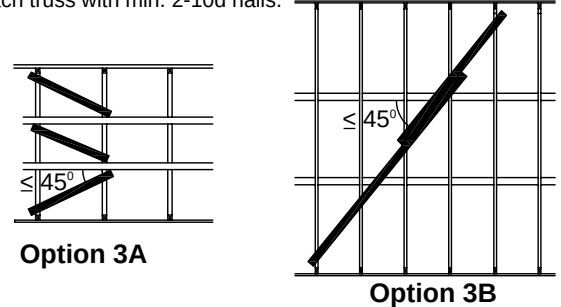
DETAIL LIMITATIONS:

1. Restraint and Bracing Material min. 2x4 stress graded lumber.
2. This detail does not address permanent building stability bracing to resist lateral forces acting on the building.
3. This detail shall not supersede any project-specific truss member permanent bracing design for the roof framing structural system.
4. This detail is not applicable for trusses with spacing greater than 2' o.c.

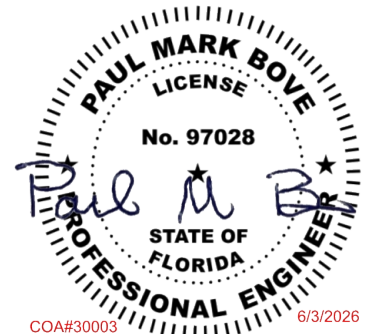
Nail Dimensions:

10d = 3" x 0.128"

For webs with 2 rows of CLRs, diagonal bracing shall be installed using Option 3A or 3B. Attach diagonal braces to each truss with min. 2-10d nails.



Detail 3 - Diagonal Bracing for 2 Rows of CLRs



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