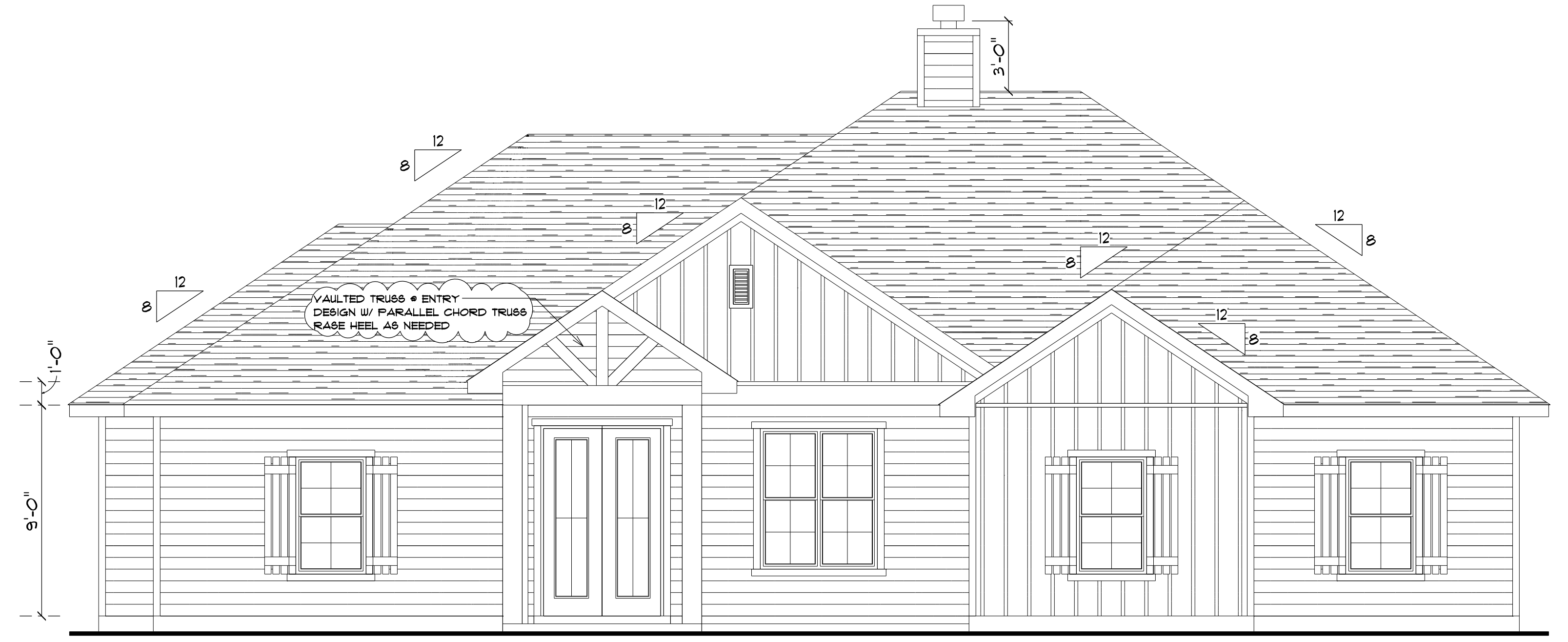


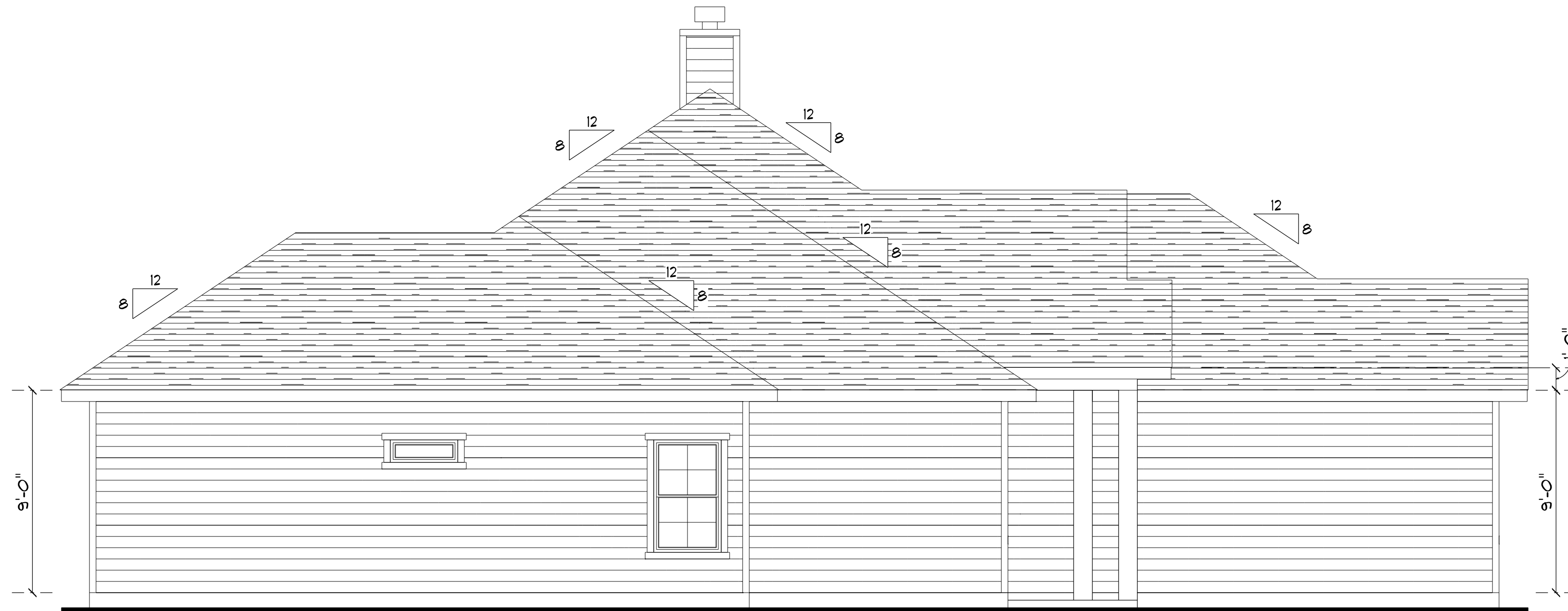
**REAR ELEVATION**

SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**

SCALE: 1/4" = 1'-0"



**LEFT ELEVATION**

SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**

SCALE: 1/4" = 1'-0"

**ROOF VENTILATION:**  
 R806.2 Minimum vent area.  
 The minimum net free ventilating area shall be 1/150 of the area of the vented space.  
 Exception: The minimum net free ventilating area shall be 1/300 of the vented space provided one or more of the following conditions are met:  
 1. In Climate Zones 6, 7 and 8, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.  
 2. At least 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located no more than 3 feet below the ridge or highest point of the space, measured vertically, with the balance of the required ventilation provided by eave or cornice vents. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet below the ridge or highest point of the space shall be permitted.

Reed McDaniel Construction, Inc

Stephan Res.

PROJECT ADDRESS:  
 234 SW Grassy Lane  
 Ft. White, FL 32058

**DIMENSIONS:**  
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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the Florida Building Code Residential (2020) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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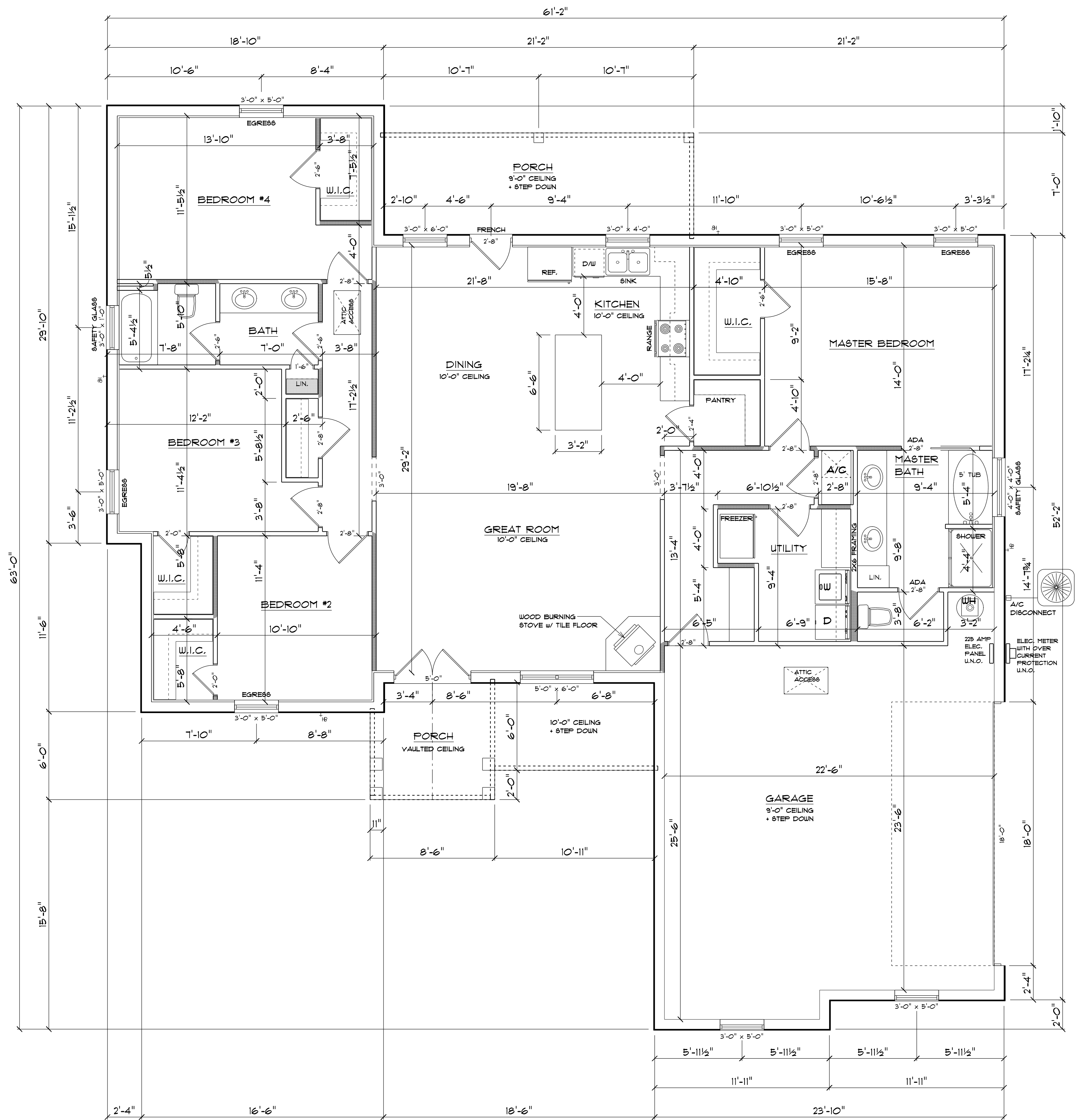


Monday, August 16, 2021

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 163 SW Midtown Place  
 Suite 103  
 Lake City, Florida 32025  
 386.754.5419  
 disoswaydesign@gmail.com

**JOB NUMBER:**  
 210654

1  
 OF 7 SHEETS



**FLOOR PLAN**  
 SCALE: 1/4" = 1'-0"  
 ALL CEILING HEIGHTS TO BE 9'-0" UNLESS NOTED OTHERWISE

**R302.5.1 Opening protection:**  
 Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches in thickness, solid or honeycomb-core steel doors not less than 1 3/8 inches thick, or 20-minute fire-rated doors, equipped with a self-closing device.

**TABLE R302.6 DWELLING/GARAGE SEPARATION:**

SEPARATION	MATERIAL
From the residence and attics	Not less than 1/2-inch gypsum board or equivalent applied to the garage side
From all habitable rooms above the garage	Not less than 5/8-inch Type X gypsum board or equivalent
Structure(s) supporting floor/ceiling assemblies used for separation required by this section	Not less than 1/2-inch gypsum board or equivalent
Garages located less than 3 feet from a dwelling unit on the same lot	Not less than 1/2-inch gypsum board or equivalent applied to the interior side of exterior walls that are within this area

**AREA SCHEDULE**

NAME	AREA
Living	1962.2 sq. ft.
Front Porch	131.7 sq. ft.
Rear Porch	148.2 sq. ft.
Garage	620.8 sq. ft.
Total	2862.9 sq. ft.

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 Ft. White, FL 33608

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 disowaydesign@gmail.com

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2  
 OF 7 SHEETS

**ELECTRICAL PLAN NOTES:**

E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.

E-3 ALL INSTALLATIONS SHALL BE PER NATL. ELECTRIC CODE.

E-4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NECLATEST EDITION.

E-6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E-7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E-8 ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DEN, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED AFCI FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.

E-9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.

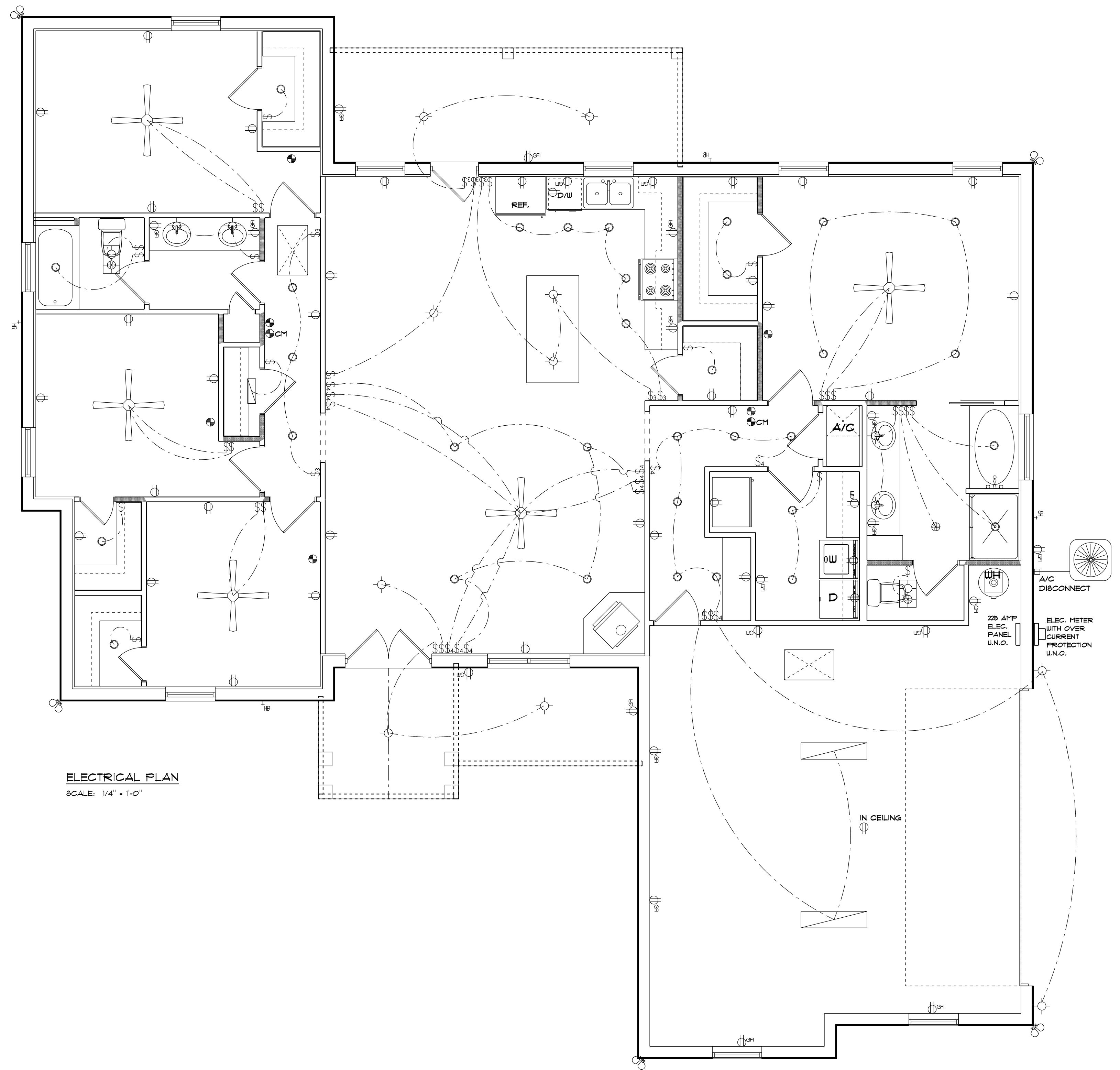
E-10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

E-11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

E-12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER RESISTANT PER NEC.

E-13 A MINIMUM OF 75% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC EC SEC. R404.1

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2x4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220V OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM



**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

Reed McDaniel Construction, Inc.

Stephan Res.

PROJECT ADDRESS:  
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FT. WILKINSON, FL 32088

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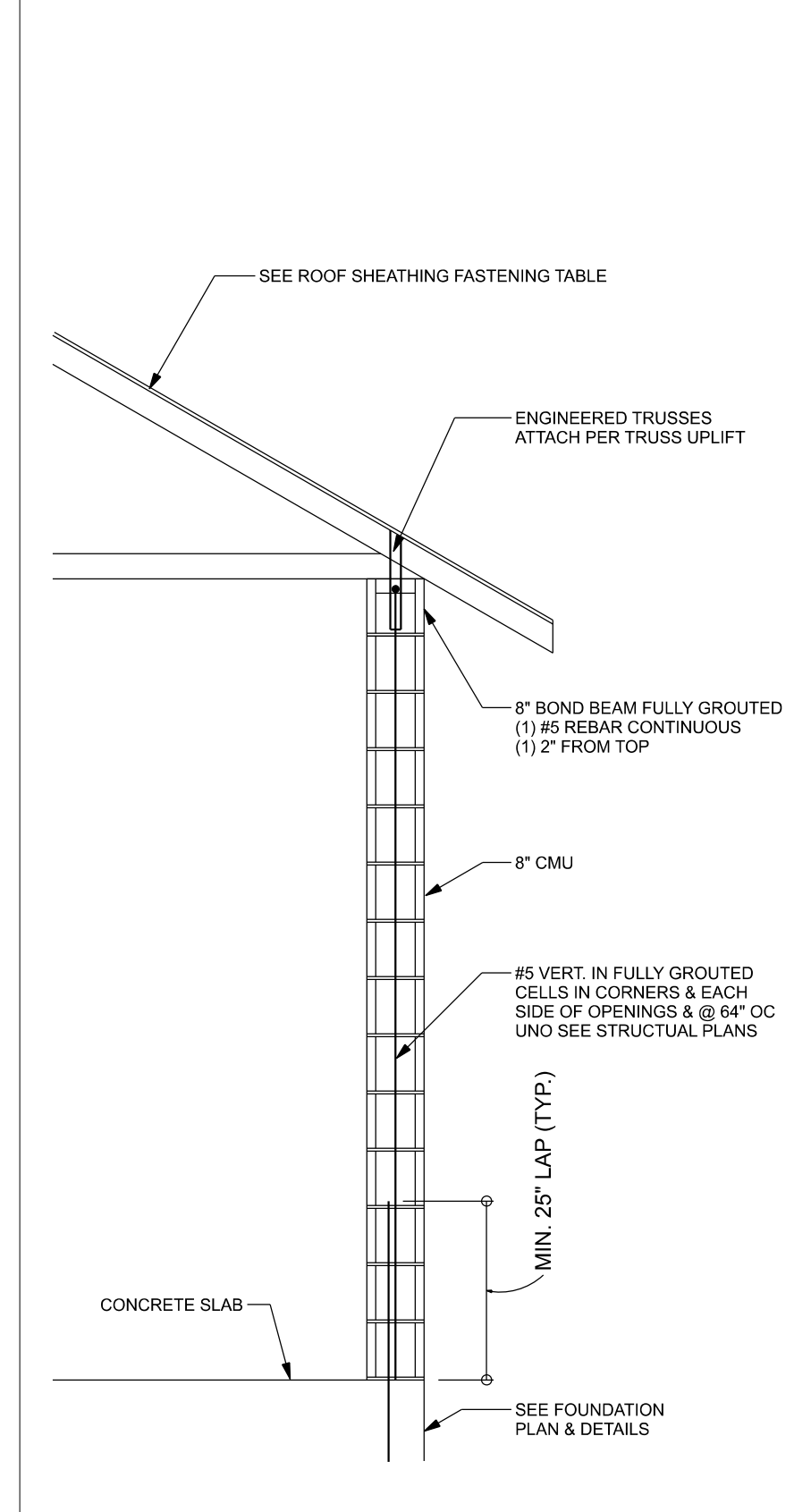


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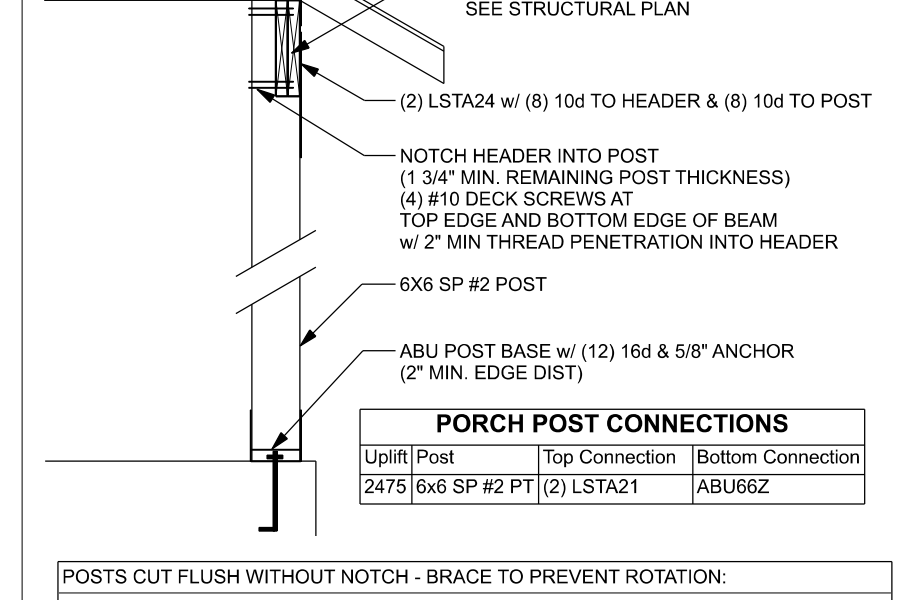
3  
OF 7 SHEETS



**(TYP.) EXTERIOR WALL**  
SCALE: 1/2" = 1'-0"

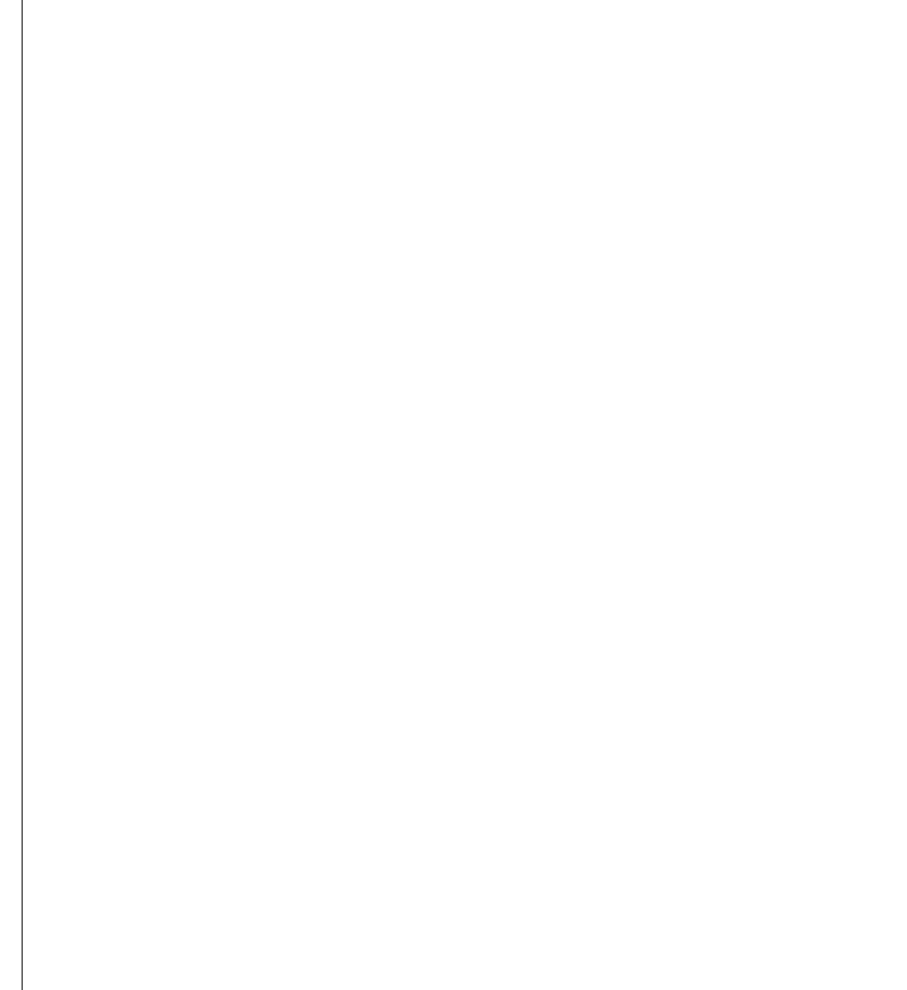


**(TYP.) PORCH POST**  
ONE STORY WOOD



**(TYP.) PORCH POST**  
ONE STORY WOOD

**INT. FRAME WALL TO CMU CONNECTION**

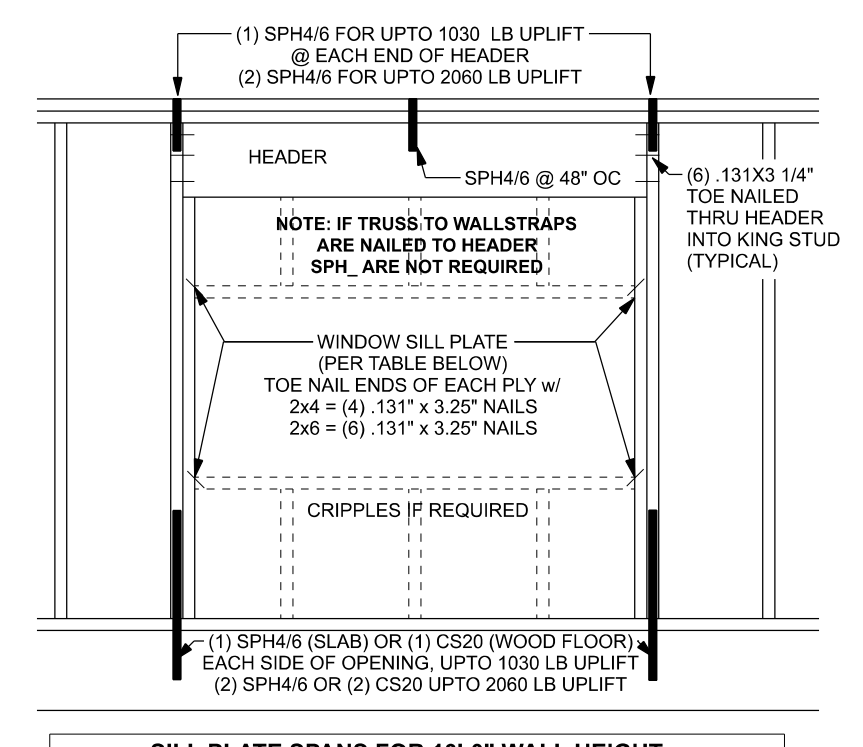


**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

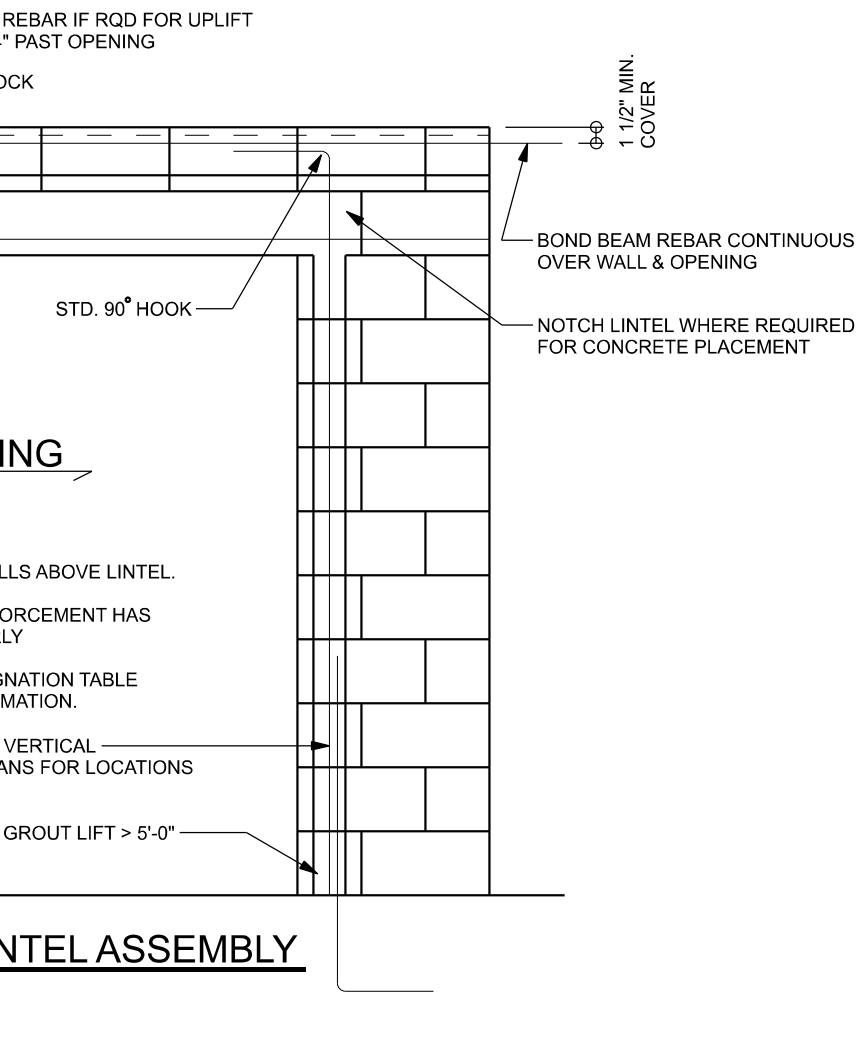
**ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)**

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

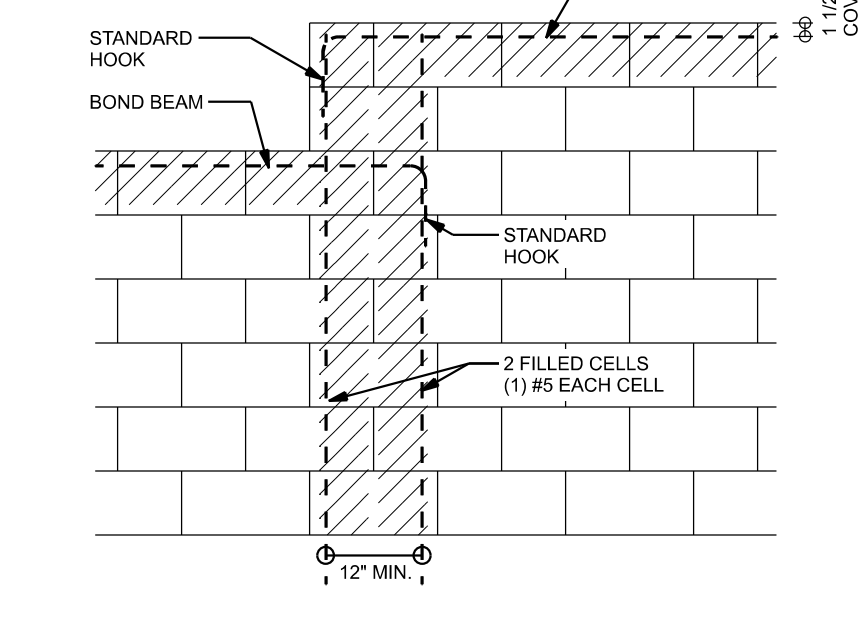
Note:  
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 8 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.  
Note:  
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



**TYP. FRAME OPENING DETAILS**  
STRAPS ON WD FLOOR OR SLAB N.T.S.

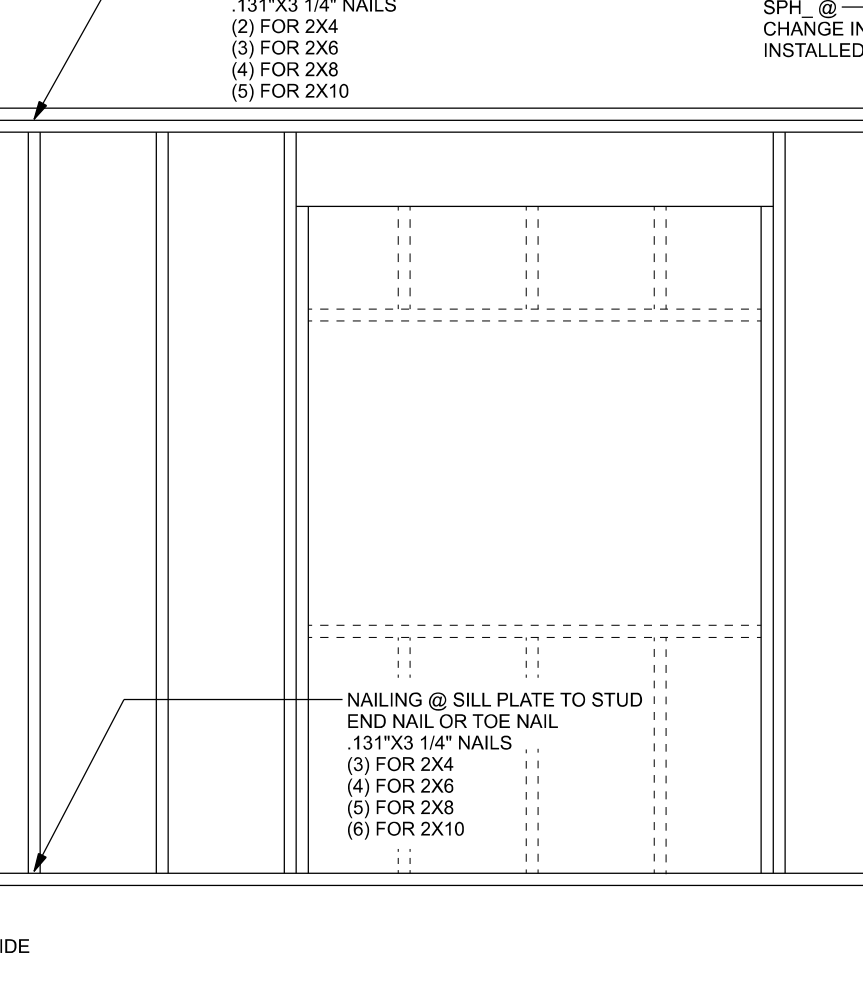


**TYPICAL FILLED LINTEL ASSEMBLY**  
CMU WALL SCALE: 1/2" = 1'-0"

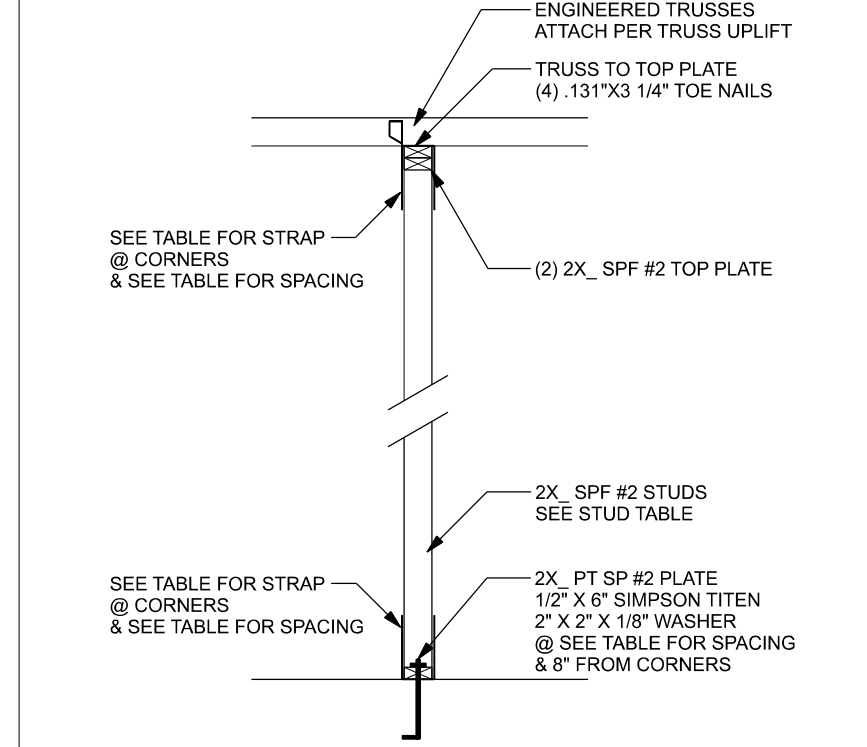


**CHANGES IN BOND BEAM HEIGHT**  
(BASED ON FBC FIG. R609.2.5)  
SCALE: 1/2" = 1'-0"

**KNEEWALL DETAIL**  
SCALE: 1/2" = 1'-0"

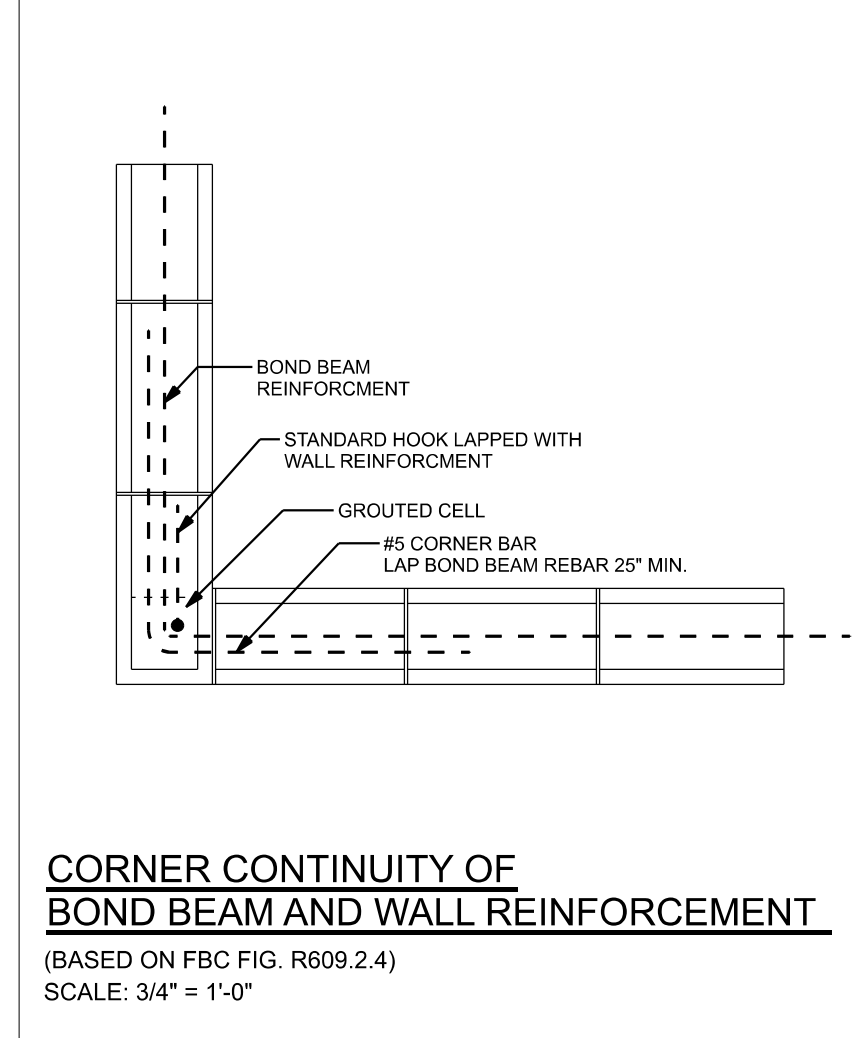


**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

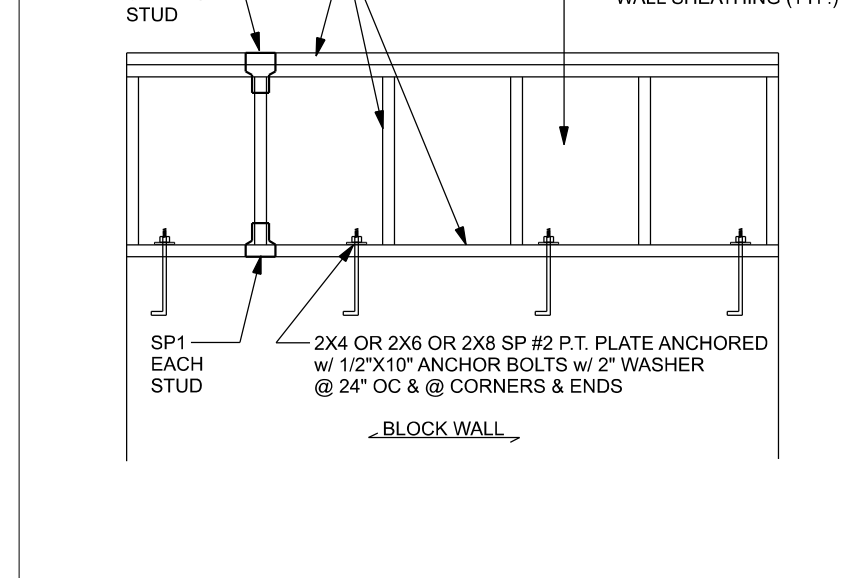


**INTERIOR BEARING WALL UPLIFT STRAP TABLE**

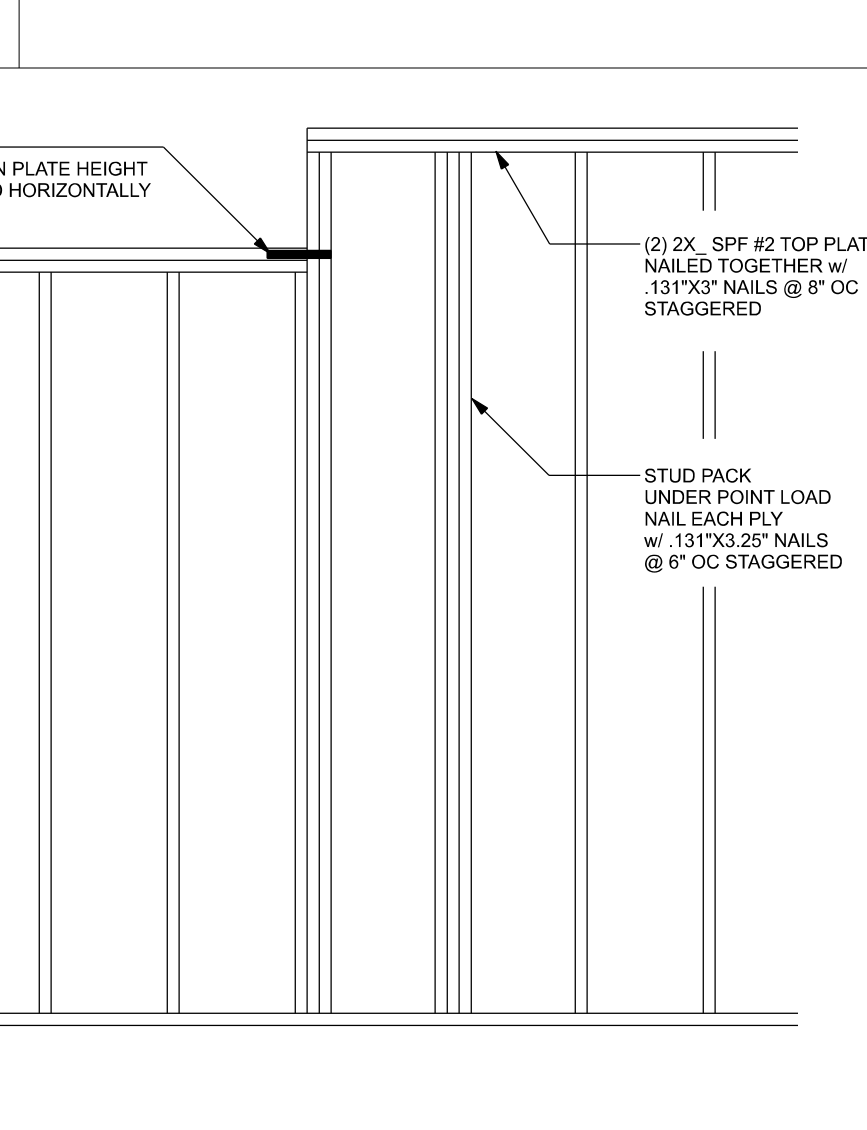
TAG	Uplift on wall	Top Connection	Bottom Connection	Anchor Spacing
IBW1	227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
IBW2	154 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
IBW3	309 pif	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
IBW4	465 pif	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC



**CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT**  
(BASED ON FBC FIG. R609.2.4)  
SCALE: 3/4" = 1'-0"



**(TYP.) GABLE BRACING DETAIL**  
CMU



**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

**CONNECTOR TABLE**

Uplift SP	Uplift SFF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8d@12"	4-8d@12"
615	280	H3	5-8d@12"	5-8d@12"
575	495	H2.5A	5-8d@12"	5-8d@12"
1340	1015	H10A	9-10d@12"	9-10d@12"
720	820	LTS12-30	6-10d@12"	6-10d@12"
1000	860	MTS12-30	7-10d@12"	7-10d@12"
1450	1345	HTS20-30	12-10d@12"	12-10d@12"
Uplift SP	Uplift SFF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SFF	Stud Plate Ties	To Stud	To Plate
585	585	SF2	6-10d	4-10d
1065	665	SF2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SFF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SFF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SFF	Post Bases @ Stenwall	To Post	Anchor
2250		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SFF	Post Bases @ Mono	To Post	Anchor
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy

**EXTERIOR WALL STUD TABLE FOR SP# 2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.0B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

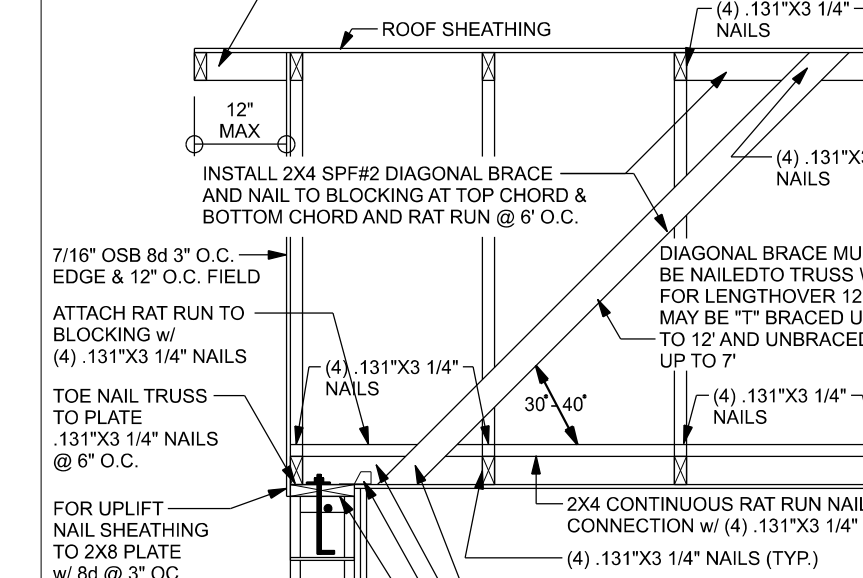
(1) 2x4 @ 16" OC	To 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	To 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	To 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	To 17'-3" STUD HEIGHT

**GRADE & SPECIES TABLE**

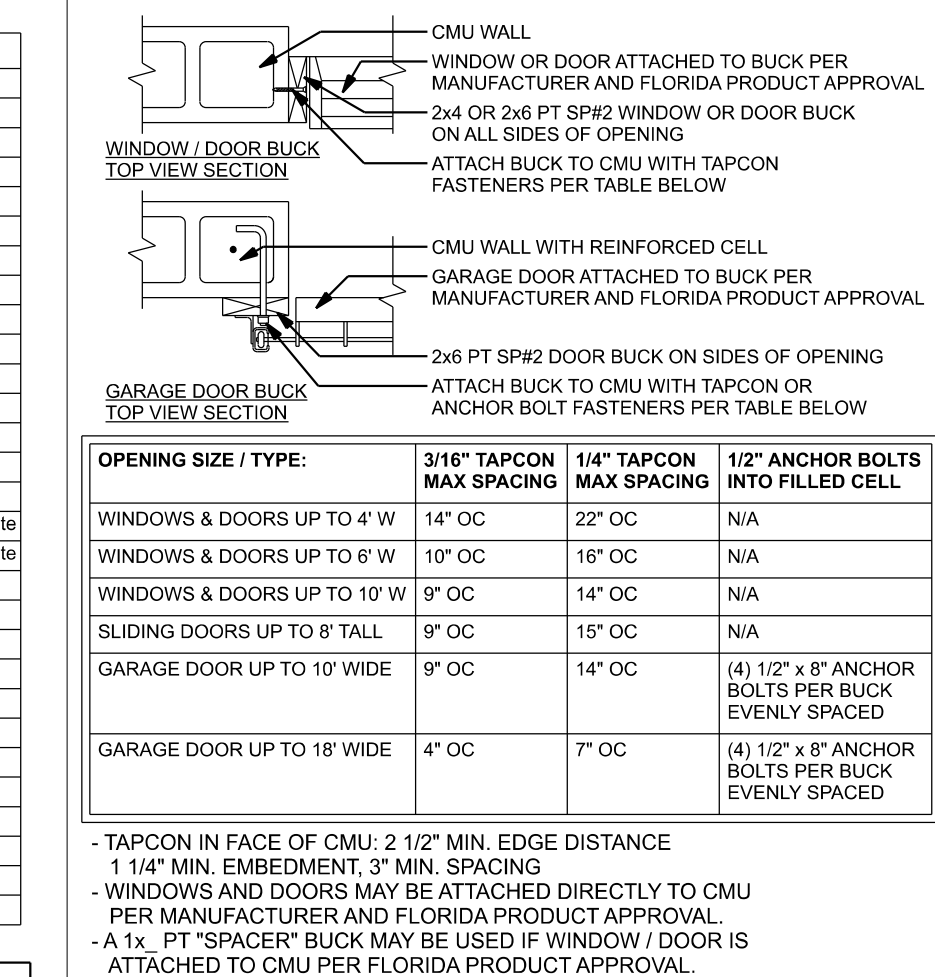
	Fb	E
2x8 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2600	1.9
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2950	2.0
PSL PARALAM	2900	2.0



**(TYP.) GABLE BRACING DETAIL**  
CMU

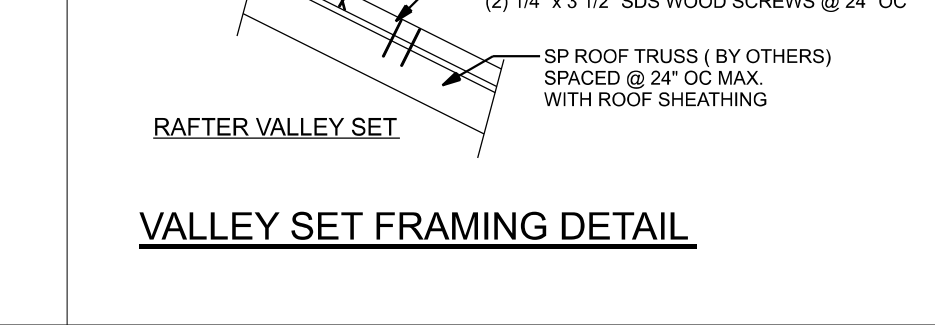
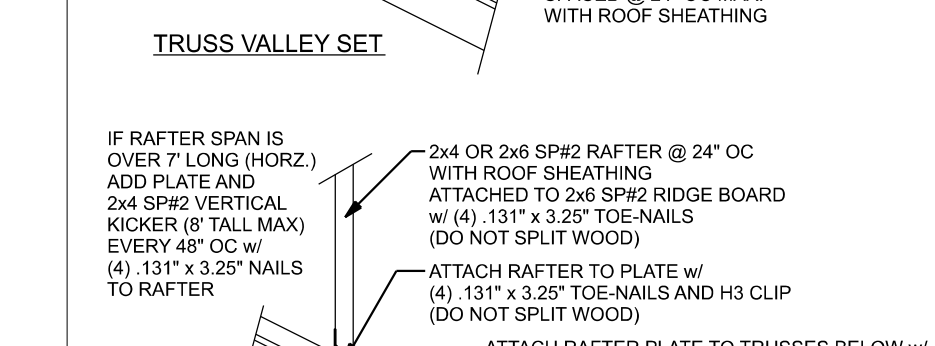


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

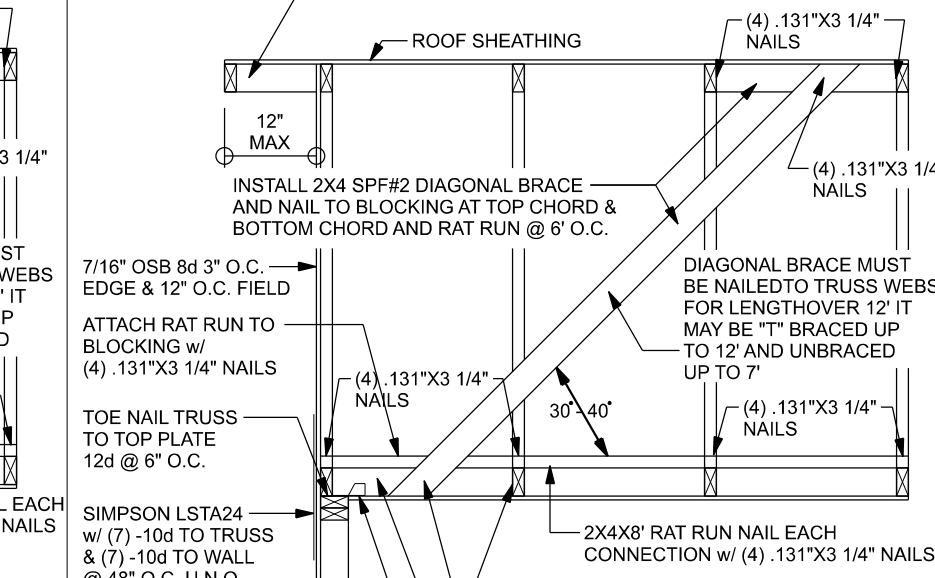


**DOOR & WINDOW BUCK ATTACHMENT**

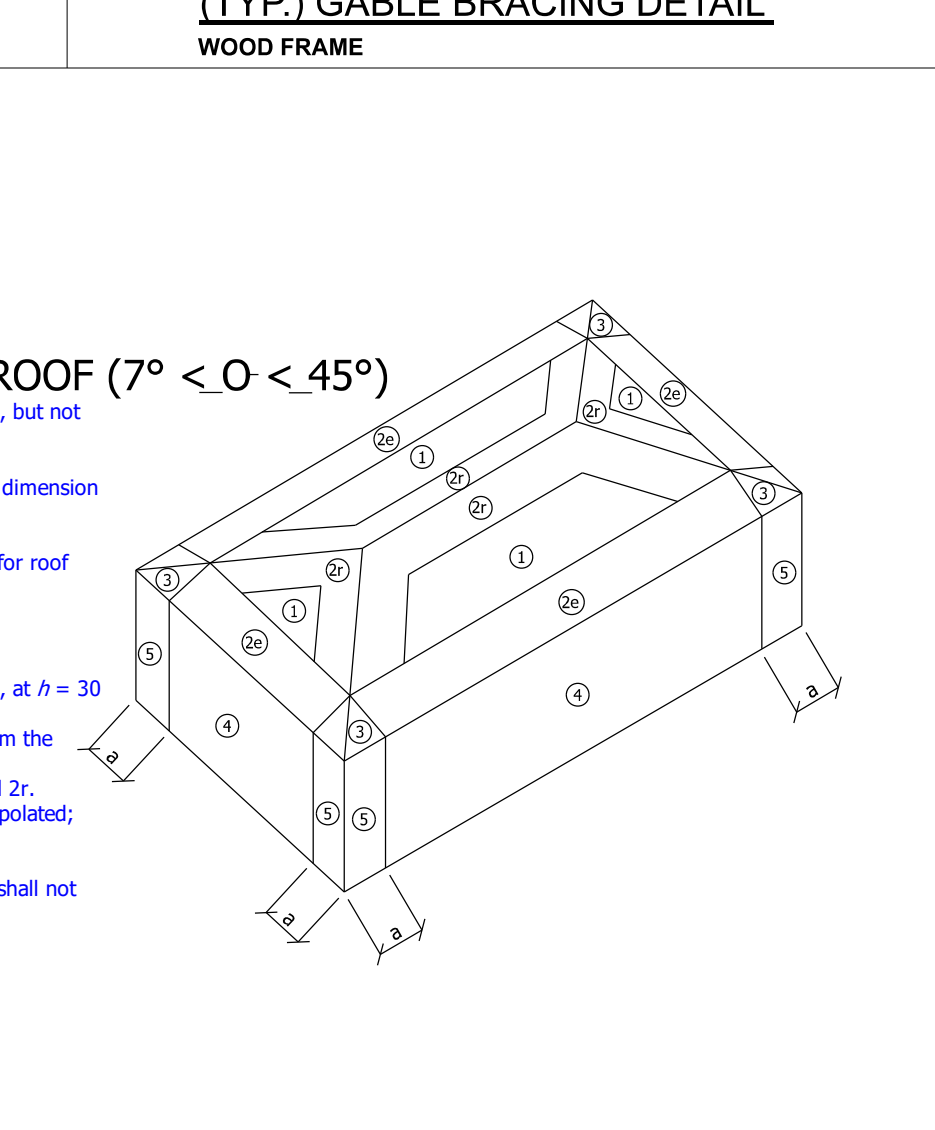
OPENING SIZE / TYPE:	3/16" TAPCON MAX SPACING	1/4" TAPCON MAX SPACING	1/2" ANCHOR BOLTS INTO FILLED CELL
WINDOWS & DOORS UP TO 6' W	14" OC	22" OC	N/A
WINDOWS & DOORS UP TO 6' H	10" OC	16" OC	N/A
WINDOWS & DOORS UP TO 10' W	9" OC	14" OC	N/A
SLIDING DOORS UP TO 8' TALL	9" OC	15" OC	N/A
GARAGE DOOR UP TO 10' WIDE	9" OC	14" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED
GARAGE DOOR UP TO 18' WIDE	4" OC	7" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED



**VALLEY SET FRAMING DETAIL**



**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

**GENERAL NOTES:**  
TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. TRUSS UPLIFT WITH MIN. UPLIFT CONNECTION 418LB EACH END; 2X8 RAFTERS 700 LB EACH END.  
SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.  
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).  
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.  
WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, F<sub>y</sub> = 85KSI, WELDED WIRE REINFORCEMENT FABRIC SHALL CONFORM TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.  
FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBER TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.  
CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WITH OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.  
REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP PLACES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DEVELOPED AND SPACED IN ACCORDANCE WITH ACI 318-16, L10.2.  
STRUCTURAL CONNECTORS: MANUFACTURER AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.  
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 5.1/ASCE 601) AND THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

**ACI 530.1-02 Section**

Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls F <sub>m</sub> = 1500 psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column bond
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM A615, Grade 40, F <sub>y</sub> = 40 ksi, Lap splices min. 40 bar dia. (25" for #4)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A503, Class C60, 0.60 oz/lb or 34SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 34SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

**BUILDER'S RESPONSIBILITY:**  
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION IF YOU BELIEVE THE WIND LOADS EXCEEDS THE LOAD PATH CONNECTION. CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.  
**ROOF SYSTEM DESIGN:**  
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC. BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN ENGINEER'S DESIGN FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**DESIGN CRITERIA & LOADS:**

BUILDING CODE	FTH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
SOIL BEARING CAPACITY	ASCE 7-16
WINDLOADS	130 MPH (ASCE 7-16, SS GUST)
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&D DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	ROOMS OTHER THAN SLEEPING ROOM: 40 PSF LIVE LOAD SLEEPING ROOMS: 30 PSF LIVE LOAD
ROOF LOADING	FLAT OR < 4:12: 20 PSF LIVE LOAD 4:12 TO < 12:12: 16 PSF LIVE LOAD 12:12 & GREATER: 12 PSF LIVE LOAD 1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

**COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP. C)**

EFFECTIVE WIND AREA (F <sub>T</sub> )	ZONE 4 INTERIOR	ZONE 5 EXTERIOR CORNER
0 - 20	+25.6/(Vasd) -27.8/(Vasd)	+25.6/(Vasd) -34.2/(Vasd)
17 - 20	+22.6/(Vasd) -24.8/(Vasd)	+22.6/(Vasd) -31.2/(Vasd)
9x7 GARAGE DOOR	+22.8/(Vasd) -25.5/(Vasd)	
16x7 GARAGE DOOR	+21.7/(Vasd) -24.1/(Vasd)	

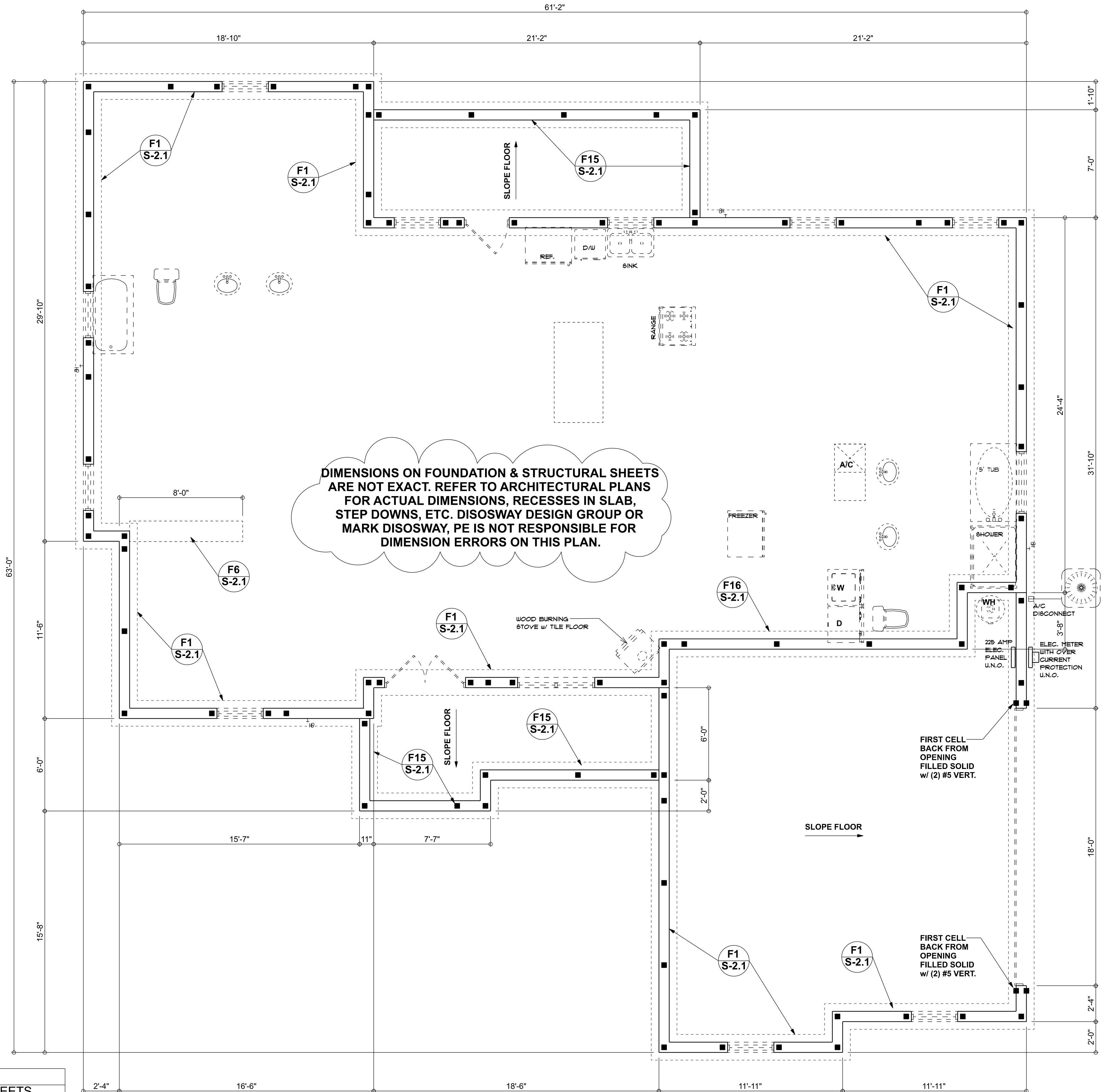
Reed McDaniel Construction, Inc  
Stephan Res.  
PROJECT ADDRESS: 234-SW Grassy Lane Ft. White, FL 32038

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163 SW Midway Place Suite 103 Lake City, Florida 32025 386.754.5419 disowaydesign@gmail.com

**JOB NUMBER: 210654**  
**S-1**  
OF 7 SHEETS

Monday, August 16, 2021



DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS  
FOR ACTUAL DIMENSIONS, RECESSES IN SLAB,  
STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR  
MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR  
DIMENSION ERRORS ON THIS PLAN.

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"

Reed McDaniel Construction, Inc.  
Stephan Res.  
PROJECT ADDRESS:  
163 SW Midtown Place  
Suite 103  
Lake City, FL 32025

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.


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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

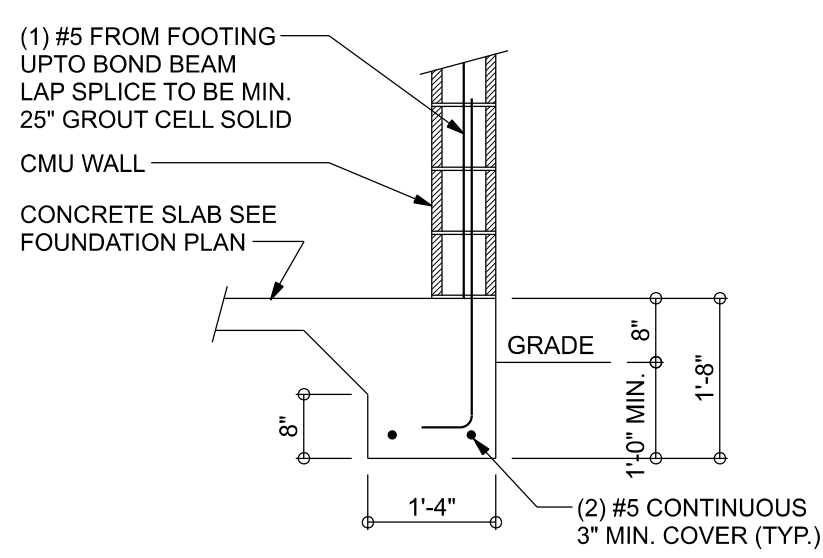
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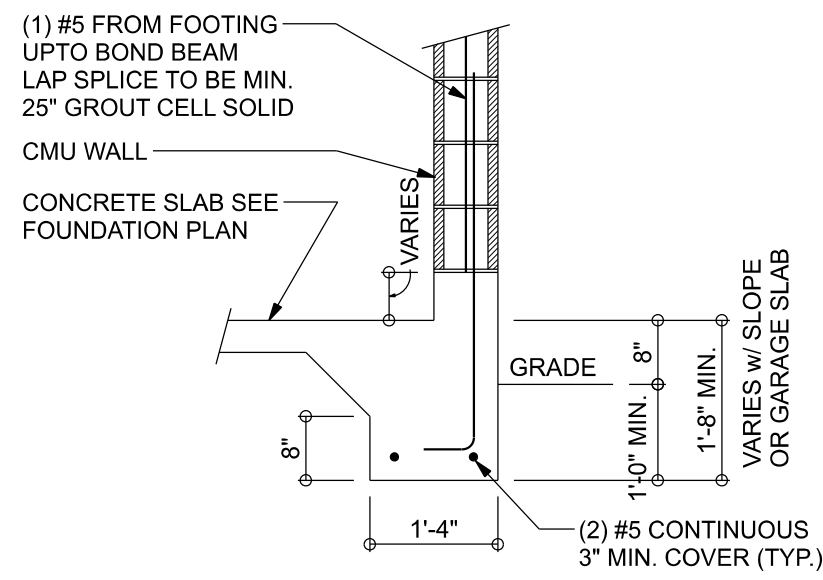
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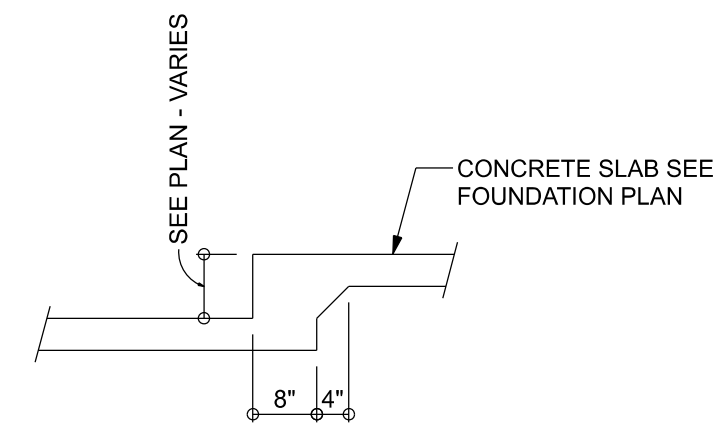
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210654  
**S-2**  
OF 7 SHEETS



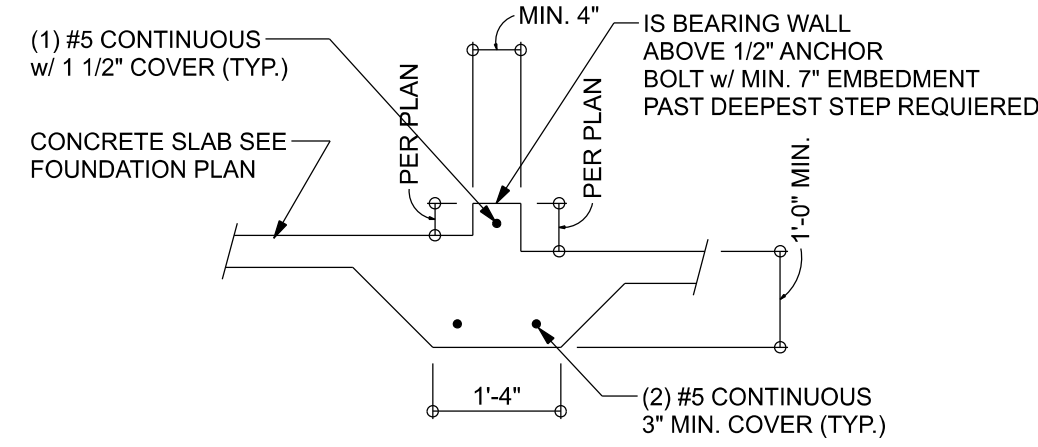
**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



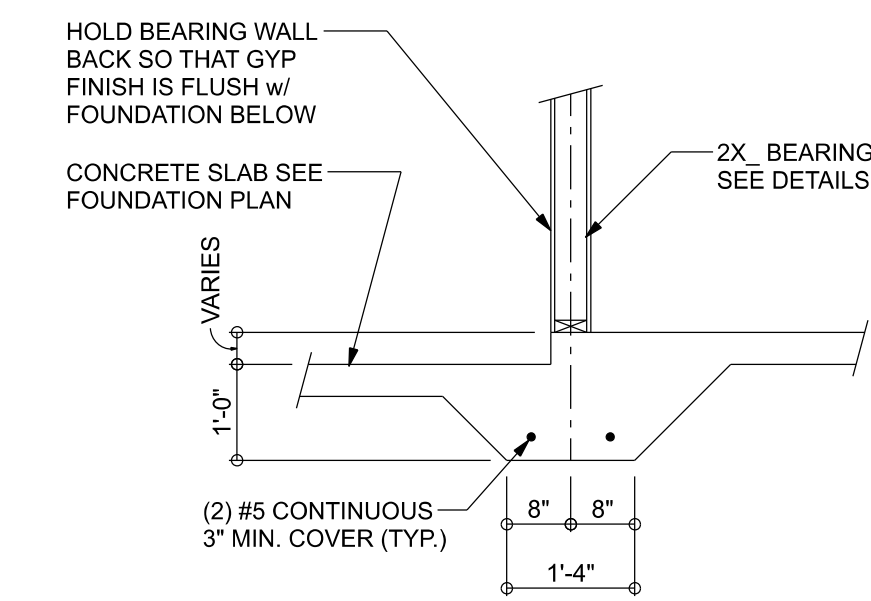
**F2 GARAGE CURB FOOTING**  
SCALE: 1/2" = 1'-0"



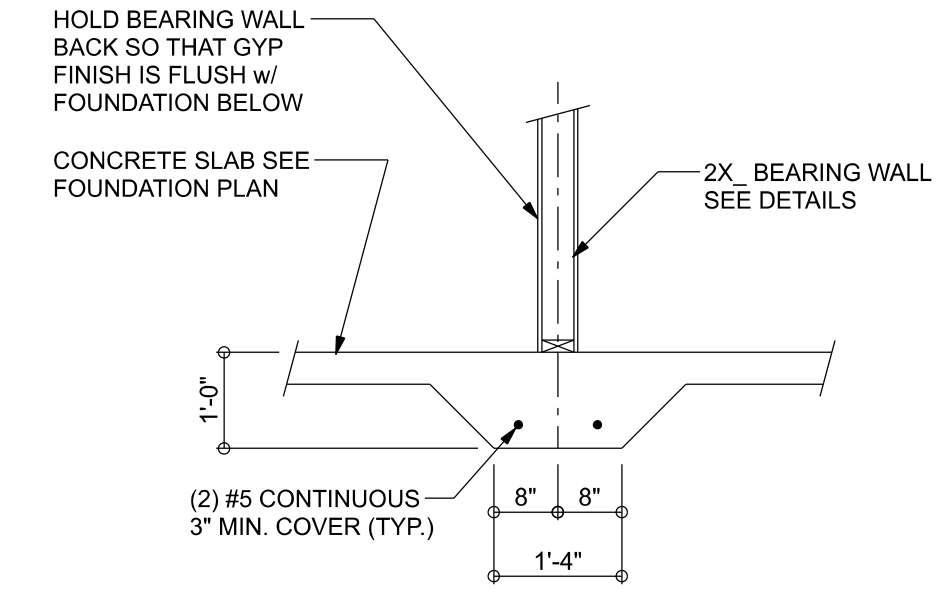
**F3 NON-BEARING STEP DOWN**  
SCALE: 1/2" = 1'-0"



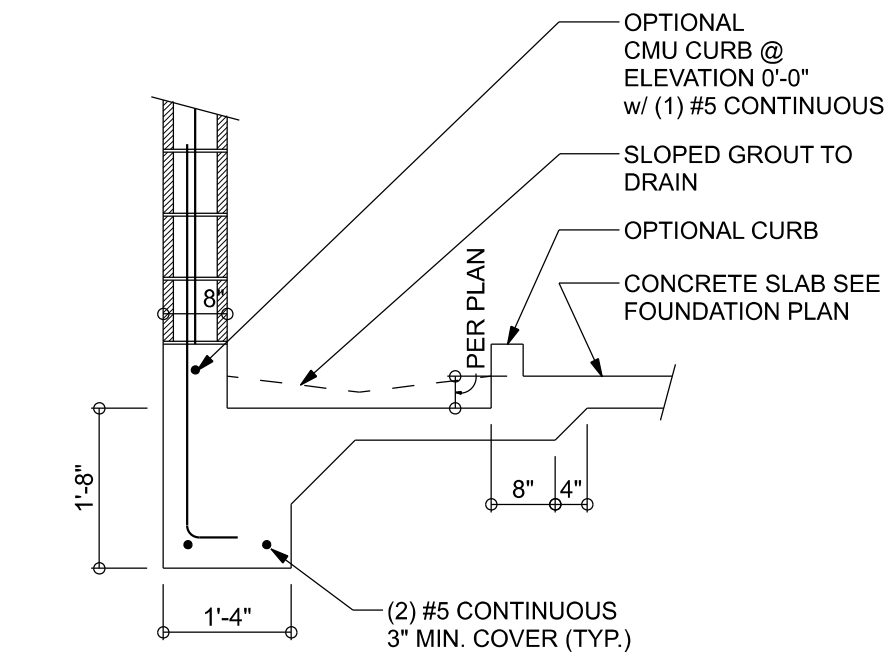
**F4 DOUBLE STEP FOOTING**  
SCALE: 1/2" = 1'-0"



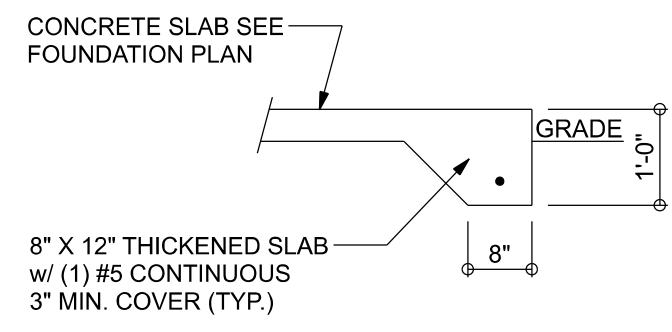
**F5 STEP FOOTING BEARING**  
SCALE: 1/2" = 1'-0"



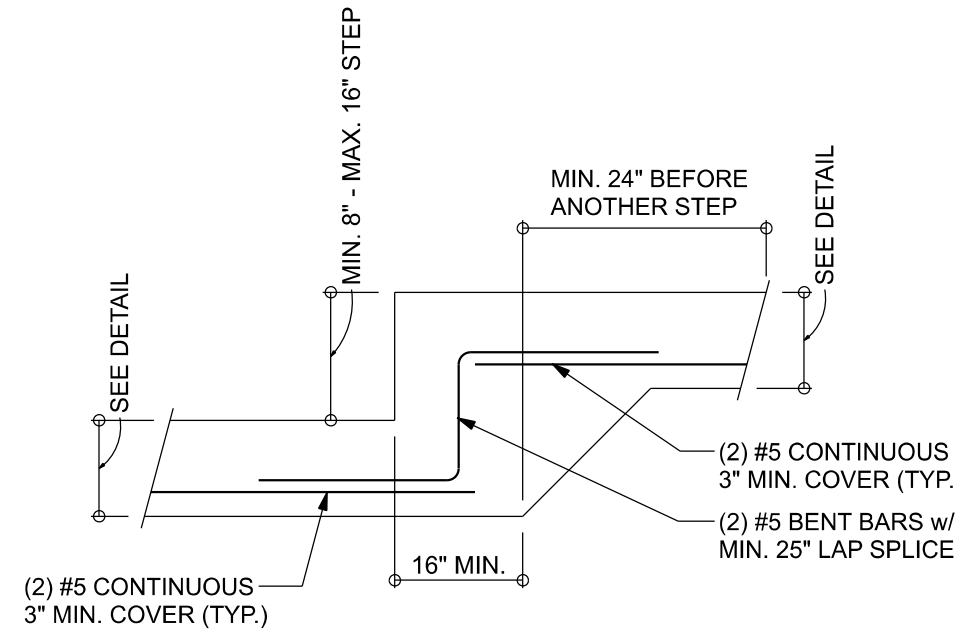
**F6 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



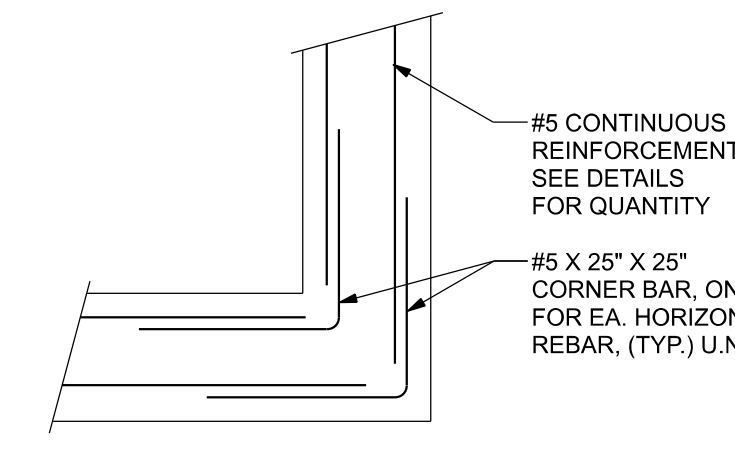
**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"



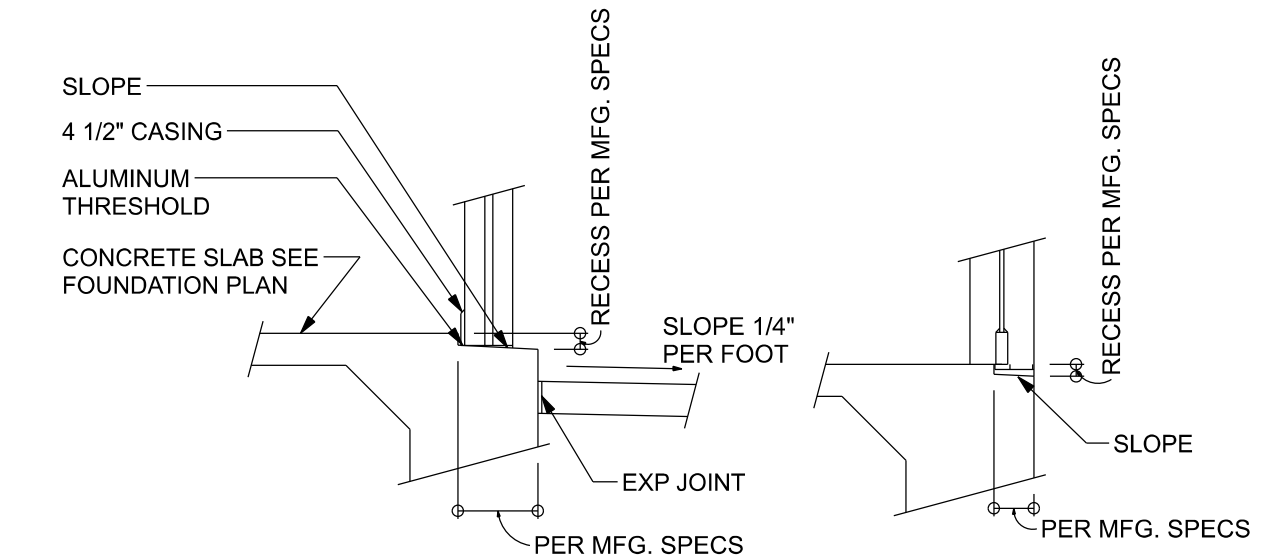
**F8 THICKENED EDGE**  
SCALE: 1/2" = 1'-0"



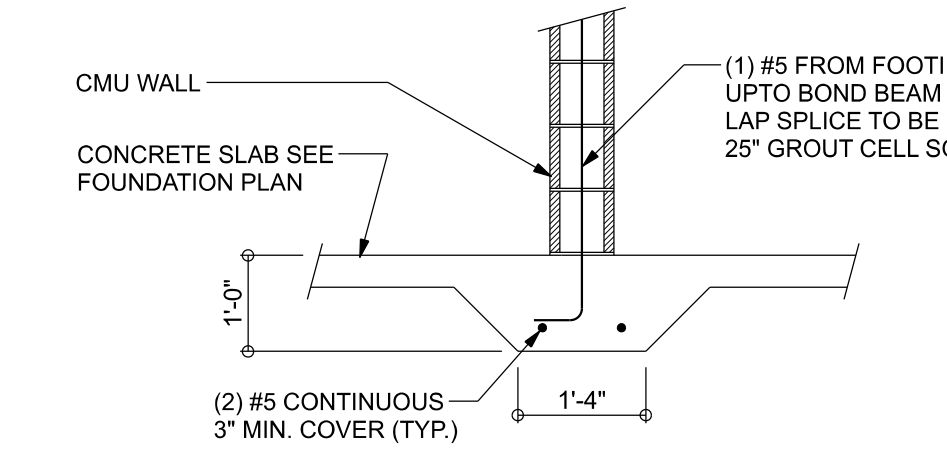
**F9 (TYP.) STEP FOOTING DETAIL**  
SCALE: 1/2" = 1'-0"



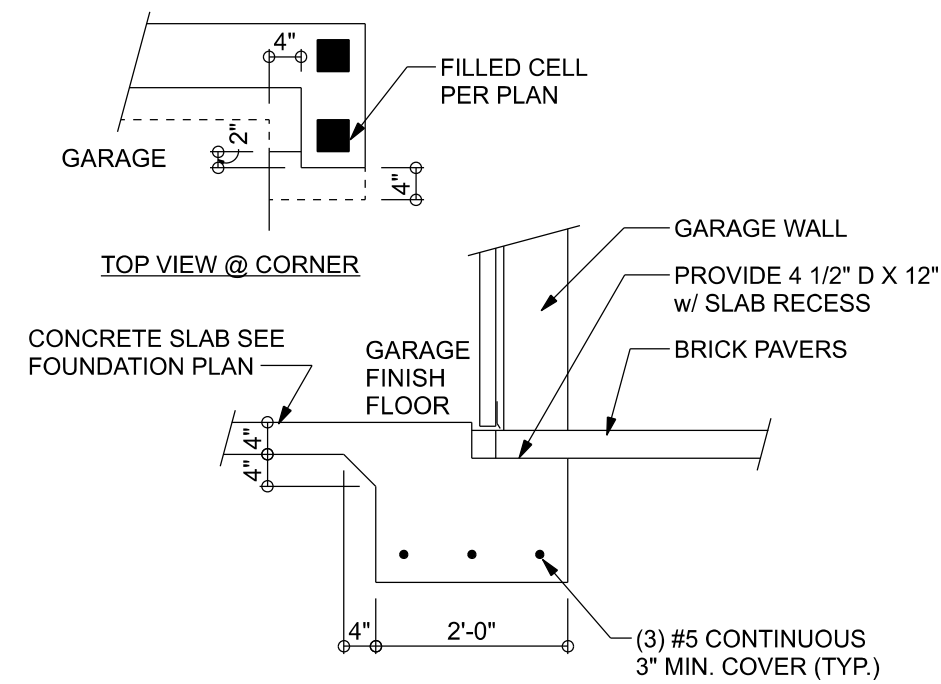
**F10 (TYP.) CORNER BAR DETAIL**  
SCALE: 1/2" = 1'-0"



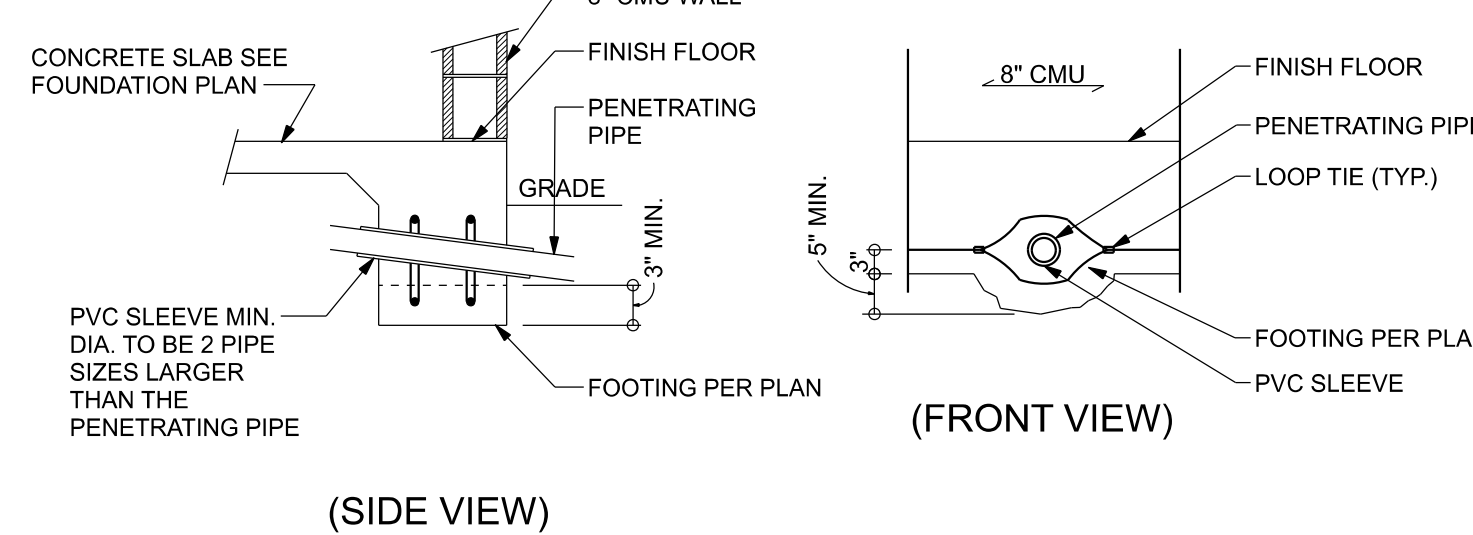
**F11 EXTERIOR DOOR POURED SILLS**  
SCALE: 1/2" = 1'-0"



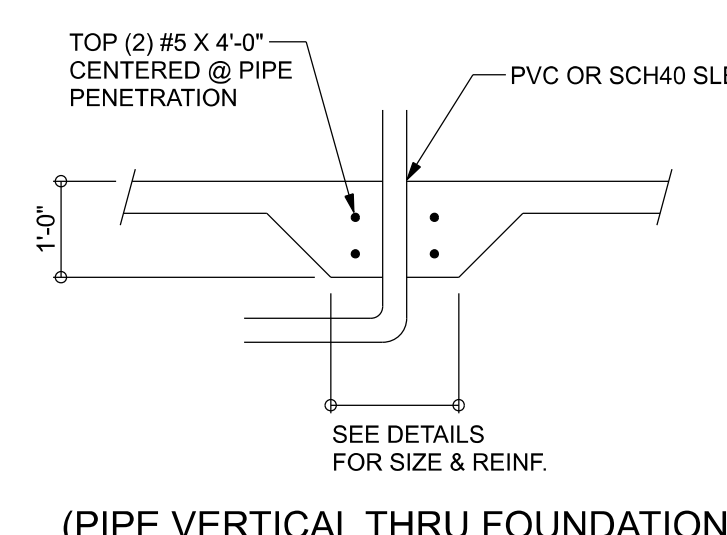
**F12 INTERIOR BEARING FOOTING w/ CMU WALL**  
SCALE: 1/2" = 1'-0"



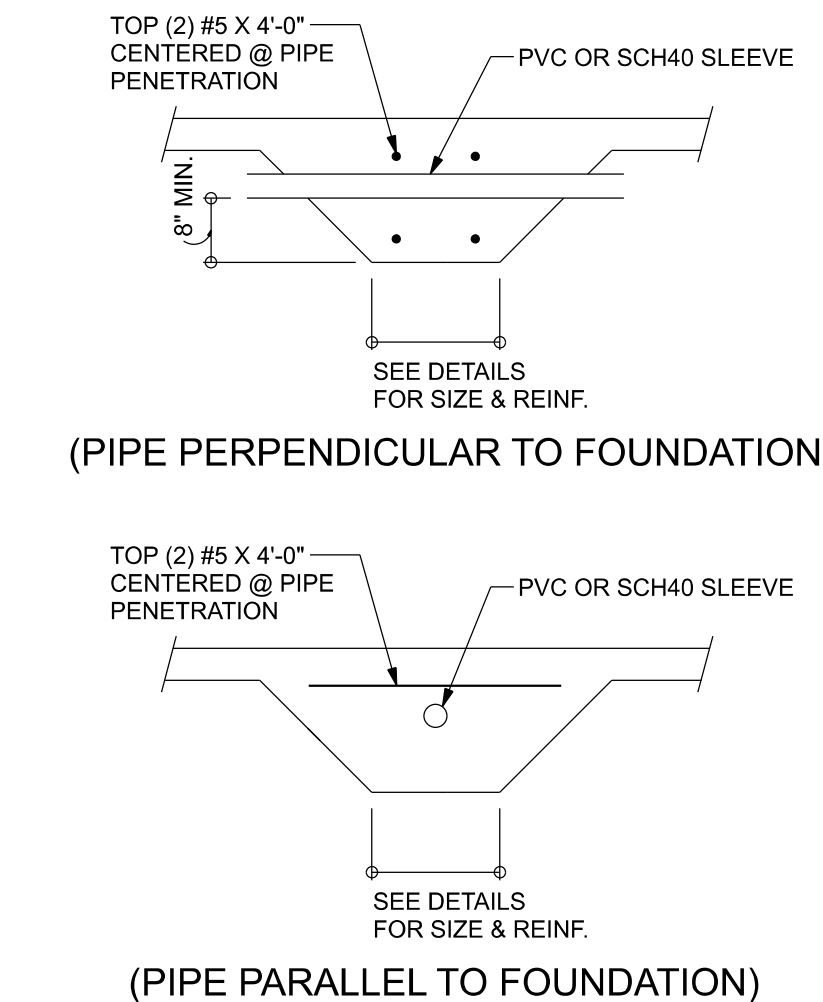
**F13 THICKENED EDGE @ GARAGE DOOR**  
SCALE: 1/2" = 1'-0"



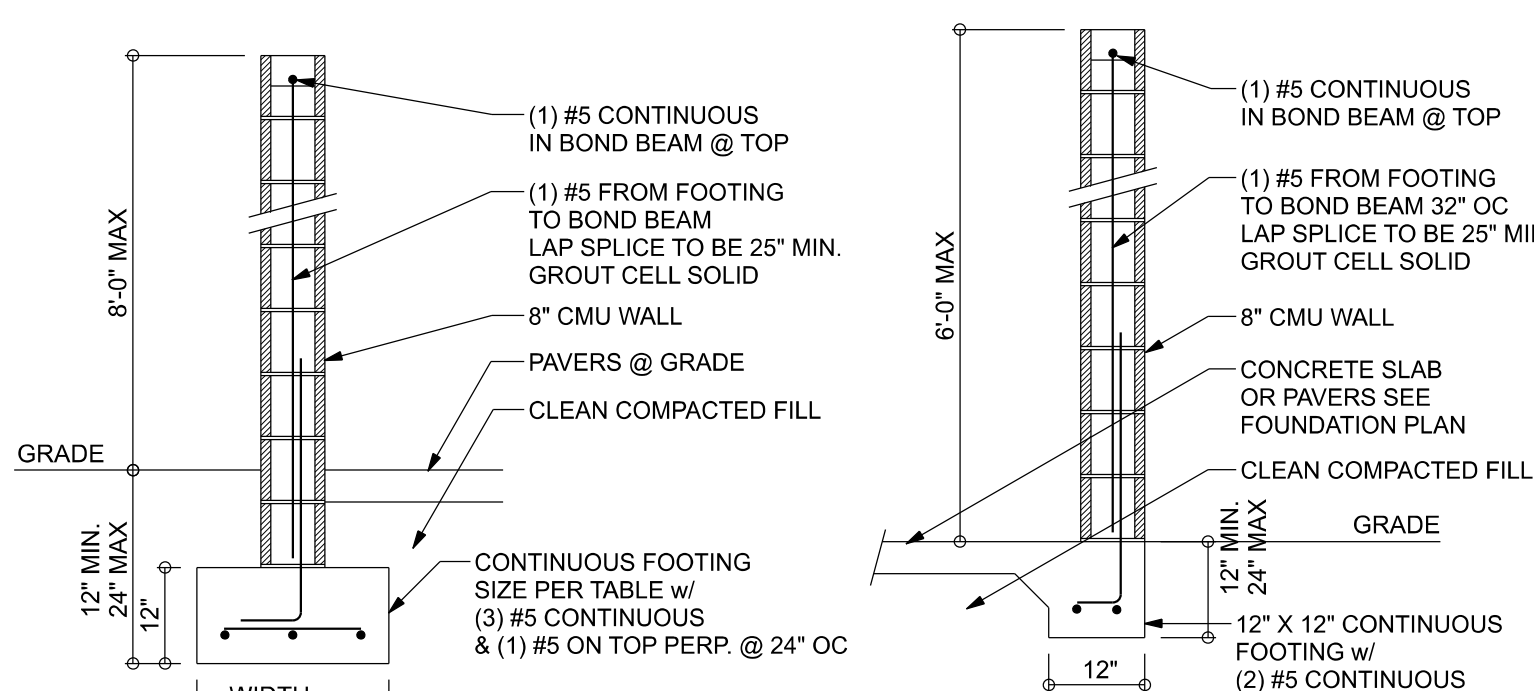
**F14 TYPICAL FOUNDATION PENETRATIONS**  
SCALE: 1/2" = 1'-0"



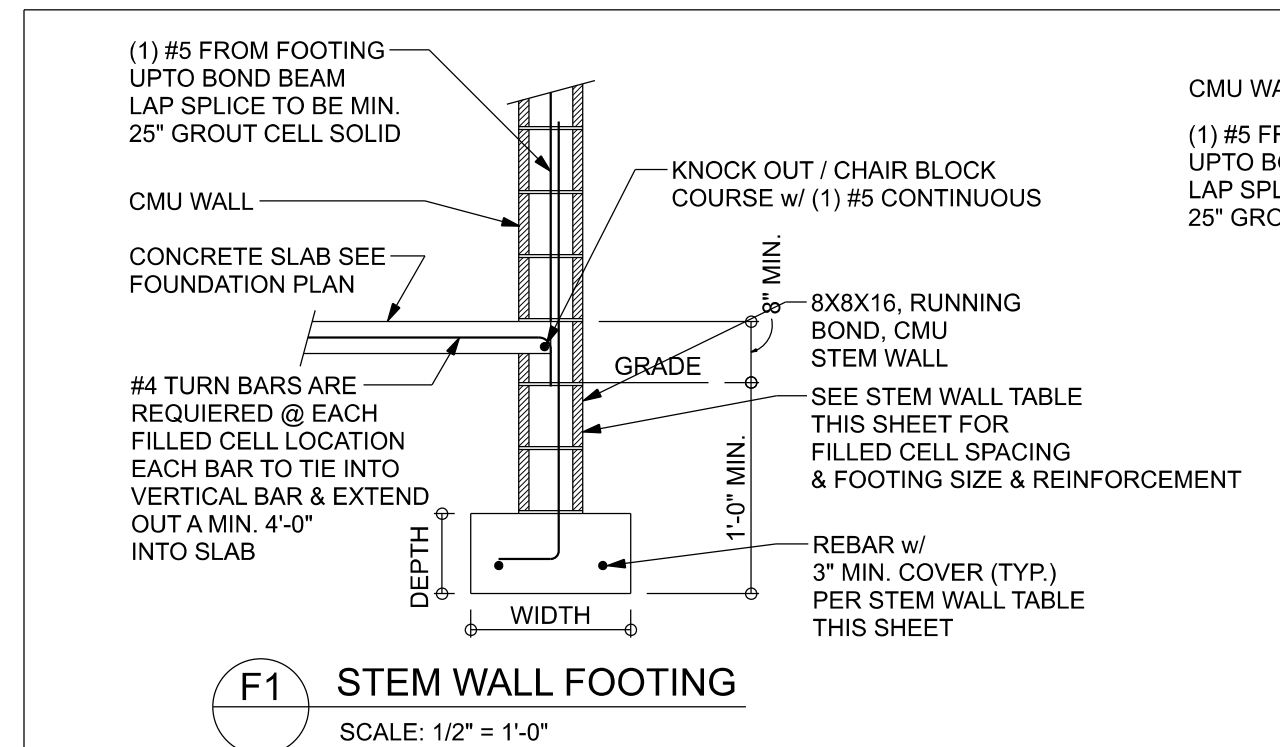
**F15 STEM WALL FOOTING @ PORCH**  
SCALE: 1/2" = 1'-0"



**F16 STEM WALL @ GARAGE STEP DOWN**  
SCALE: 1/2" = 1'-0"

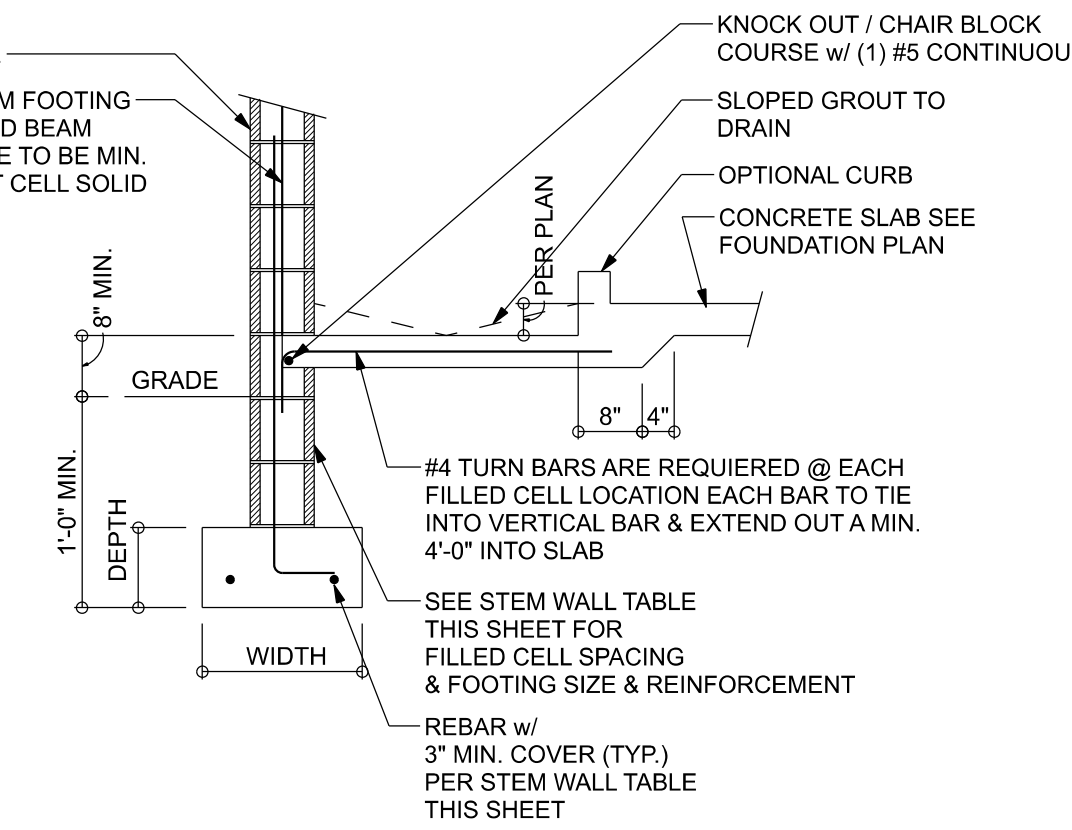


**F17 PRIVACY WALL FOUNDATION DETAILS**  
SCALE: 1/2" = 1'-0"



**F1 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"

STEM WALL HEIGHT	FOOTING DIMENSION		NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	DEPTH	WIDTH		
8" - 40"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"



**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"

**OPTIONAL STEM WALL FOUNDATION**

NOTE: ALL STEM WALL FOUNDATIONS OVER 3'-0" IN HEIGHT TO BE POURED SOLID

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FL White, FL 32088

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MARK DISOSWAY P.E. 53915

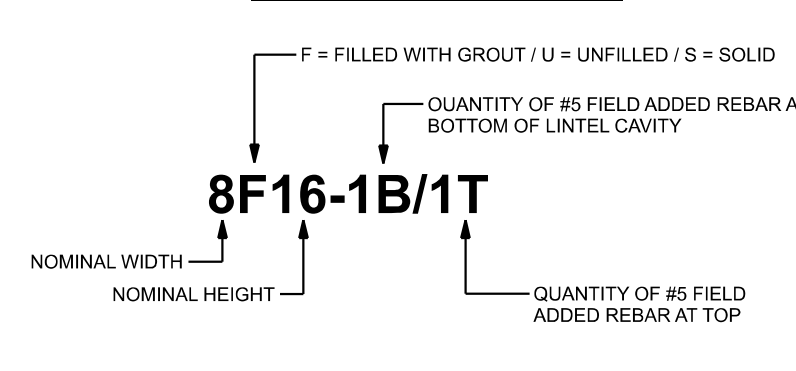
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Monday, August 16, 2021

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JOB NUMBER:  
210654  
**S-2.1**  
OF 7 SHEETS

**TYPE DESIGNATION**



**MATERIALS**

1. fc 8" precast lintel = 3500 psi
2. fc prestressed lintel = 6000 psi
3. Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

**GENERAL NOTES**

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:  

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right.

**SAFE LOAD TABLE NOTES**

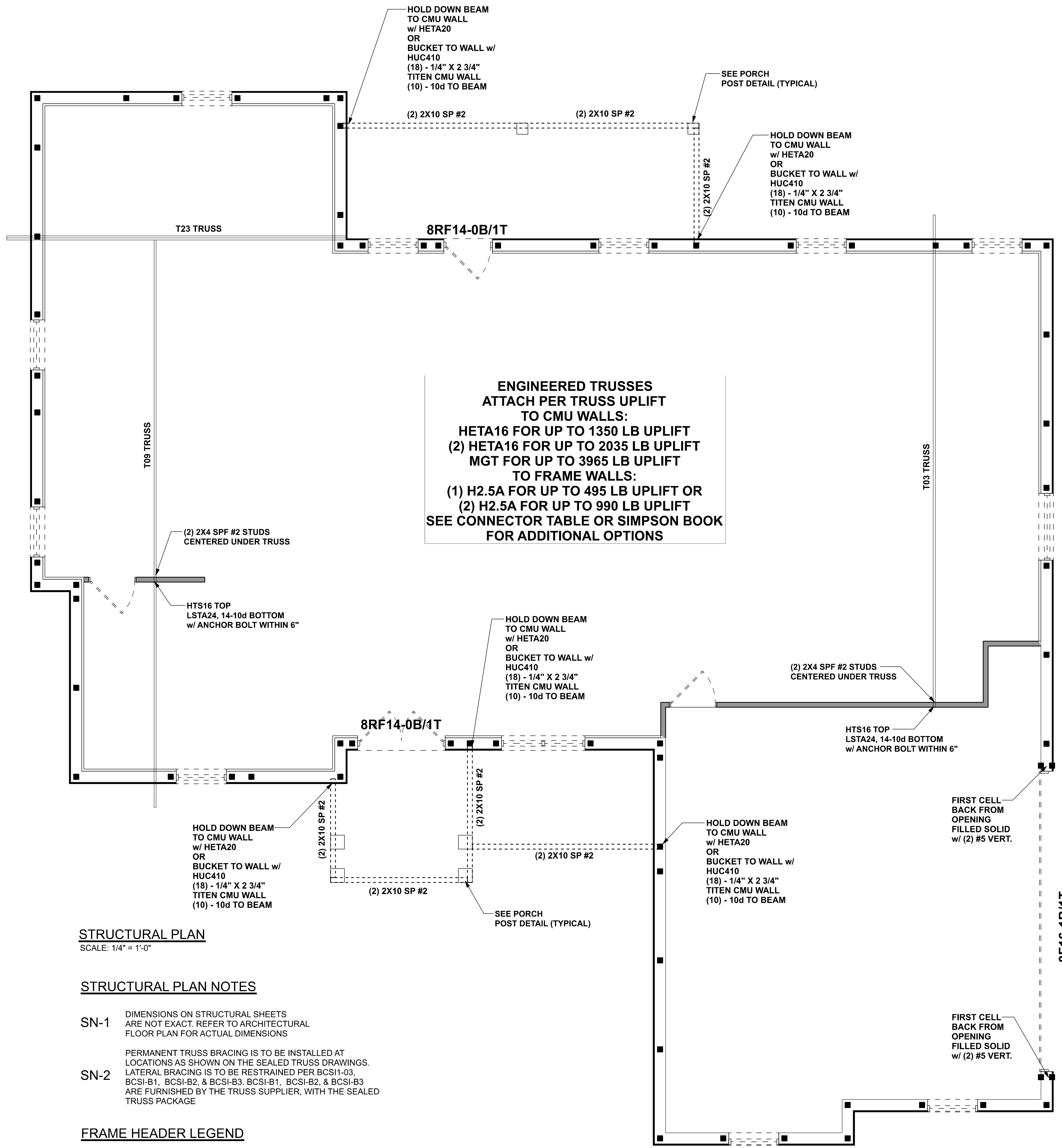
1. All values based on minimum 4 inch nominal bearing.
- Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.  
 Example 7'-6" lintel type 8F32-1B safe gravity load = 6472; H0.0469; (15); H0.0781; w/ 15% reduction 6472 \* (.85) = 5501 plf

**SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS**

LENGTH	TYPE	8U8	SAFE LOAD - POUNDS PER LINEAR FOOT															
			8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F8-1B	8F12-1B	8F16-1B	8F20-1B					
2'-10"	(34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809								
3'-6"	(42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809								
4'-0"	(48")	PRECAST	1966	2693	4025	5357	6689	8021	9353	10685								
4'-6"	(54")	PRECAST	1599	2189	3275	4361	5447	6533	7619	8705								
5'-4"	(64")	PRECAST	1217	1663	2500	3337	4174	5011	5848	6685								
5'-10"	(70")	PRECAST	1062	1451	2189	2927	3665	4403	5141	5879								
6'-6"	(78")	PRECAST	908	1238	1876	2514	3152	3790	4428	5066								
7'-6"	(90")	PRECAST	743	1011	1479	2047	2615	3183	3751	4319								
9'-4"	(112")	PRECAST	554	699	1160	1621	2082	2543	3004	3465								
10'-6"	(126")	PRECAST	475	752	1245	1843	2441	3039	3637	4235								
11'-4"	(136")	PRECAST	362	582	945	1308	1671	2034	2397	2760								
12'-0"	(144")	PRECAST	337	540	873	1254	1635	2016	2397	2778								
13'-4"	(160")	PRECAST	296	471	755	1075	1428	1838	2316	2863								
14'-0"	(168")	PRECAST	279	442	706	1002	1326	1697	2127	2630								
14'-8"	(176")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
15'-4"	(184")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
17'-4"	(208")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
19'-4"	(232")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
21'-4"	(256")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
22'-0"	(264")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								
24'-0"	(288")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR								

**SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS**

LENGTH	TYPE	8RU6	SAFE LOAD - POUNDS PER LINEAR FOOT															
			8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B					
4'-4"	(52")	PRECAST	1635	1749	3355	3280	4349	5421	6493	7567								
4'-6"	(54")	PRECAST	1494	1891	3699	3206	4639	6072	7505	8938								
5'-8"	(68")	PRECAST	866	1596	3063	2992	3969	4946	5924	6904								
5'-10"	(70")	PRECAST	810	920	1770	1716	2277	2839	3402	3966								
6'-8"	(80")	PRECAST	797	1167	2481	2457	3289	4117	4945	5773								
7'-6"	(90")	PRECAST	669	859	1653	1600	2124	2649	3174	3700								
9'-8"	(116")	PRECAST	411	1113	2342	2424	3289	4117	4945	5773								
				901	1825	3120	3048	3715	4482	5249								
				755	1490	2459	2376	2743	3110	3477								
				466	999	1568	1523	1839	2155	2471								
				526	999	1568	1523	1839	2155	2471								



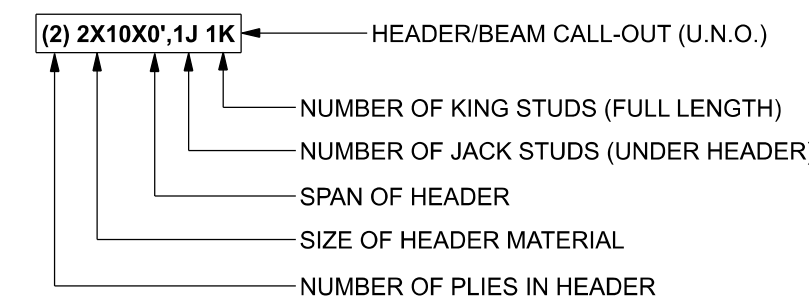
**STRUCTURAL PLAN**

SCALE: 1/4" = 1'-0"

**STRUCTURAL PLAN NOTES**

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSS1-03, BCSS1-B1, BCSS1-B2, & BCSS1-B3. BCSS1-B1, BCSS1-B2, & BCSS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**FRAME HEADER LEGEND**



**UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS**

1. USE HETA16 CMU TO TRUSS
2. USE H2.5A FRAME TO TRUSS
3. ALL LINTELS TO BE: 8F16-0B/1T
4. ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2
5. ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE
6. ALL INTERIOR BEARING WALLS TO BE: IBW1 SEE DETAIL ON SHEET S-1 1/2" X 6" TITEN @ 48" OC SP1 BOTTOM & SP2 TOP @ 32" OC

CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2857394

Read McDaniel Construction, Inc.  
 Stephan Res.  
 PROJECT ADDRESS: 163 SW Midtown Place, Suite 103, Lake City, Florida 32025, FL 32025

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915  
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Monday, August 16, 2021  
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JOB NUMBER:  
 210654  
**S-3**  
 OF 7 SHEETS