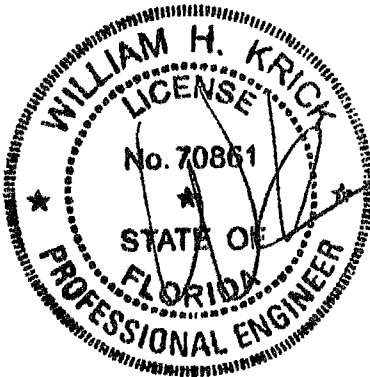




Alpine, an ITW Company  
 155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025  
 Phone: (800)755-6001  
 www.alpineitw.com



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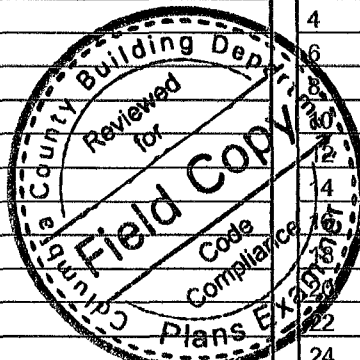
03/03/2025

|  |                            |
|--|----------------------------|
| <b>Site Information:</b>                                       | <b>Page 1:</b>             |
| <b>Customer:</b> W. B. Howland Company, Inc.                   | <b>Job Number:</b> 25-2306 |
| <b>Job Description:</b> MOSS                                   |                            |
| <b>Address:</b> Lot 29<br>cobblestone S/D, Lake City, FL 32055 |                            |

|  |                                      |
|--|--------------------------------------|
| <b>Job Engineering Criteria:</b>                             |                                      |
| <b>Design Code:</b> FBC 8th Ed. 2023 Res.                    | <b>IntelliVIEW Version:</b> 23.02.04 |
|  | <b>JRef #:</b> 1Y7W2150003           |
| <b>Wind Standard:</b> ASCE 7-22 <b>Wind Speed (mph):</b> 130 | <b>Design Loading (psf):</b> 40.00   |
| <b>Building Type:</b> Closed                                 |                                      |

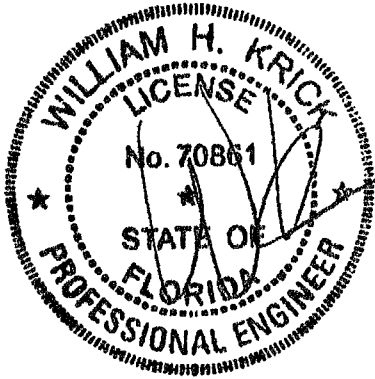
This package contains general notes pages, 64 truss drawing(s) and 4 detail(s).

| Item | Drawing Number    | Truss | Item | Drawing Number    | Truss |
|------|-------------------|-------|------|-------------------|-------|
| 1    | 062.25.0537.33483 | A01   | 2    | 062.25.0537.35603 | B01   |
| 3    | 062.25.0537.36953 | B02   | 4    | 062.25.0537.38430 | C01   |
| 5    | 062.25.0537.42987 | D01   | 6    | 062.25.0537.44920 | D02   |
| 7    | 062.25.0537.47620 | D03   | 8    | 062.25.0537.50690 | D04   |
| 9    | 062.25.0537.52153 | D05   | 10   | 062.25.0537.55290 | D06   |
| 11   | 062.25.0537.56780 | D07   | 12   | 062.25.0537.58773 | D08   |
| 13   | 062.25.0538.00887 | D09   | 14   | 062.25.0538.02260 | D10   |
| 15   | 062.25.0538.13660 | G01   | 16   | 062.25.0538.16637 | G02   |
| 17   | 062.25.0538.19950 | G03   | 18   | 062.25.0538.22553 | G04   |
| 19   | 062.25.0538.25033 | G05   | 20   | 062.25.0538.27467 | G06   |
| 21   | 062.25.0538.29130 | G07   | 22   | 062.25.0538.34000 | G08   |
| 23   | 062.25.0538.37640 | G09   | 24   | 062.25.0538.40797 | G10   |
| 25   | 062.25.0538.43843 | G11   | 26   | 062.25.0538.52993 | G12   |
| 27   | 062.25.0539.10740 | G13   | 28   | 062.25.0539.15353 | G14   |
| 29   | 062.25.0539.23567 | G15   | 30   | 062.25.0539.25293 | G16   |
| 31   | 062.25.0539.31520 | G17   | 32   | 062.25.0539.32987 | G18   |
| 33   | 062.25.0539.34650 | G19   | 34   | 062.25.0539.36340 | G20   |
| 35   | 062.25.0539.38520 | G21   | 36   | 062.25.0539.39867 | G22   |
| 37   | 062.25.0539.41487 | G23   | 38   | 062.25.0539.42973 | G24   |
| 39   | 062.25.0541.06840 | G25   | 40   | 062.25.0541.09887 | G26   |
| 41   | 062.25.0541.11140 | G27   | 42   | 062.25.0541.19283 | G28   |
| 43   | 062.25.0541.27847 | H01   | 44   | 062.25.0541.29170 | H02   |
| 45   | 062.25.0541.59533 | H03   | 46   | 062.25.0542.18760 | HJ01  |
| 47   | 062.25.0542.20400 | HJ02  | 48   | 062.25.0542.21760 | J01   |





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03/03/2025

|  |                            |
|--|----------------------------|
| <b>Site Information:</b>                                       | <b>Page 2:</b>             |
| <b>Customer:</b> W. B. Howland Company, Inc.                   | <b>Job Number:</b> 25-2306 |
| <b>Job Description:</b> MOSS                                   |                            |
| <b>Address:</b> Lot 29<br>cobblestone S/D, Lake City, FL 32055 |                            |

| Item | Drawing Number    | Truss |
|------|-------------------|-------|
| 49   | 062.25.0542.23027 | J02   |
| 51   | 059.25.1513.19464 | J04   |
| 53   | 062.25.0542.33153 | J06   |
| 55   | 062.25.0542.35757 | J08   |
| 57   | 062.25.0542.48640 | J10   |
| 59   | 062.25.0542.50557 | PB02  |
| 61   | 062.25.0542.53210 | PB05  |
| 63   | 062.25.0542.55673 | PB07  |
| 65   | BRCLBSUB0119      |       |
| 67   | VAL180220723      |       |

| Item | Drawing Number    | Truss |
|------|-------------------|-------|
| 50   | 062.25.0542.24457 | J03   |
| 52   | 062.25.0542.31477 | J05   |
| 54   | 062.25.0542.34520 | J07   |
| 56   | 062.25.0542.40297 | J09   |
| 58   | 062.25.0542.49377 | PB01  |
| 60   | 062.25.0542.51567 | PB03  |
| 62   | 062.25.0542.54597 | PB06  |
| 64   | 062.25.0542.58653 | PB08  |
| 66   | PB160220723       |       |
| 68   | VALTN220723       |       |

## **General Notes**

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Bearing Information:**

The bearing area factor,  $C_b$ , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

## General Notes (continued)

### **Coated Lumber:**

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Borafire Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

## **General Notes** (continued)

### **Key to Terms** (continued):

R<sub>w</sub> = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

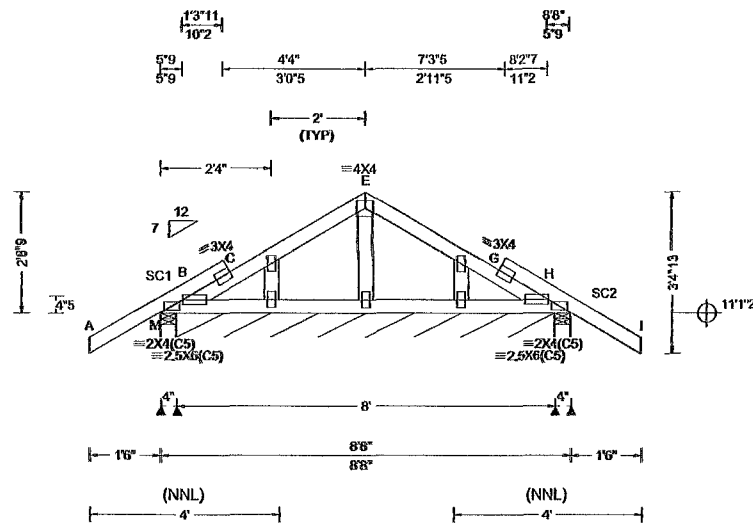
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

### **References:**

1. AWC: American Wood Council; 222 Catocfin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcacomponents.com](http://www.sbcacomponents.com)



|   |  |  |  |  |
|---|--|--|--|--|
| <b>Loading Criteria (psf)</b>   | <b>Wind Criteria</b>   | <b>Snow Criteria (Pg, Pf in PSF)</b>   | <b>Defl/CSI Criteria</b>   | <b>Maximum Reactions (lbs), or *=PLF</b>   |
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld: 40.00<br>NCBCLL 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: 0 to h/2<br>C&C Dist: 3.00 ft<br>Loc. from endwall Any<br>GCpt: 0.18<br>Wind Duration: 1.60 | Pg: NA Ct: NA CAT NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): -0.001 C 999 240<br>VERT(CL): -0.002 C 999 180<br>HORZ(LL): 0.000 G - -<br>HORZ(TL): 0.001 G - -<br><br>Creep Factor: 2.0<br>Max TC CSI: 0.239<br>Max BC CSI: 0.031<br>Max Web CSI: 0.070<br><br>VIEW Ver: 23.02.04.0123.14 | Gravity Non-Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>M 261 /- /- /182 /53 /106<br>B* 50 /- /- /32 /6 /-<br>H 260 /- /- /194 /53 /-<br><br>Wind reactions based on MWFRS<br>M Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>B Brg Wid = 96.0 Min Req = -<br>H Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings M, B, & H are a rigid surface<br>Members not listed have forces less than 375# |

**Lumber**  
Top chord 2x4 SP #2,  
Bot chord 2x4 SP #2,  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Plating Notes**  
All plates are 2X4 except as noted

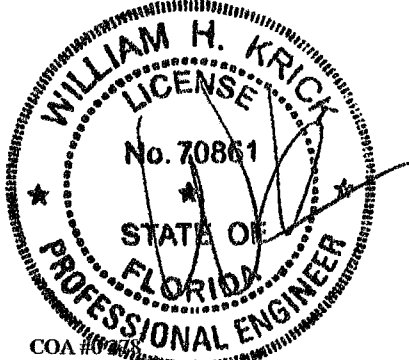
**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.  
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

**Additional Notes**  
Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSITPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 2-6-9.



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**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

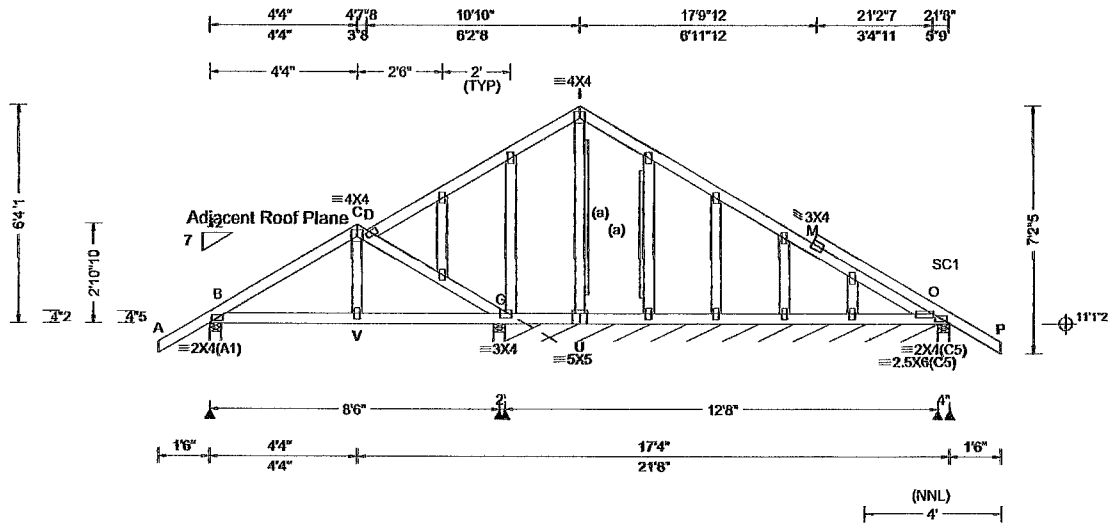
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSITPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSITPI 1 Sec.2.

For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI [tpinst.org](http://tpinst.org); SBCA: [sbccomponents.com](http://sbccomponents.com); ICC: [iccsafe.org](http://iccsafe.org); AWC: [awc.org](http://awc.org)



155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



| Loading Criteria (psf)  | Wind Criteria  | Snow Criteria (Pg, Pf in PSF)   | Defl/CSI Criteria  | ▲ Maximum Reactions (lbs), or * = PLF   |
|---|--|---|--|---|
| TCLL: 20.00<br>TCDL: 10.00<br>BCLL: 0.00<br>BCDL: 10.00<br>Des Ld: 40 00<br>NCBCLL: 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0 " | Wind Std. ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL: 5.0 psf<br>BCDL: 5.0 psf<br>MWFRS Parallel Dist. 0 to h/2<br>C&C Dist: 3.00 ft<br>Loc. from endwall: Any<br>GCpi: 0.18<br>Wind Duration: 1 60 | Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.010 E 999 240<br>VERT(CL): 0.022 E 999 180<br>HORZ(LL): 0.005 E - -<br>HORZ(TL): 0.010 E - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.280<br>Max BC CSI: 0.160<br>Max Web CSI: 0.837<br><br>VIEW Ver: 23.02.04.0123.14 | Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>Non-Gravity<br>B 429 /- /- /269 /73 /207<br>G 183 /- /- /100 /- /-<br>G* 96 /- /- /49 /22 /-<br>O 281 /- /- /175 /27 /-<br><br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>G Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>O Brg Wid = 152 Min Req = -<br>Bearings B, G, & O are a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Bot Chord Forces Per Ply (lbs)<br>Chords Tens.Comp.<br>V - G 488 - 189 |

**Lumber**  
Top chord 2x4 SP #2;  
Bot chord 2x4 SP #2,  
Webs: 2x4 SP #3,  
Stack Chord: SC1 2x4 SP #2;

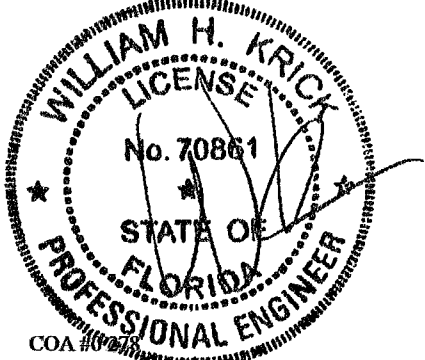
**Plating Notes**  
All plates are 2X4 except as noted

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
Wind loading based on both gable and hip roof types.  
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/237.

**Gable Reinforcement**  
(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

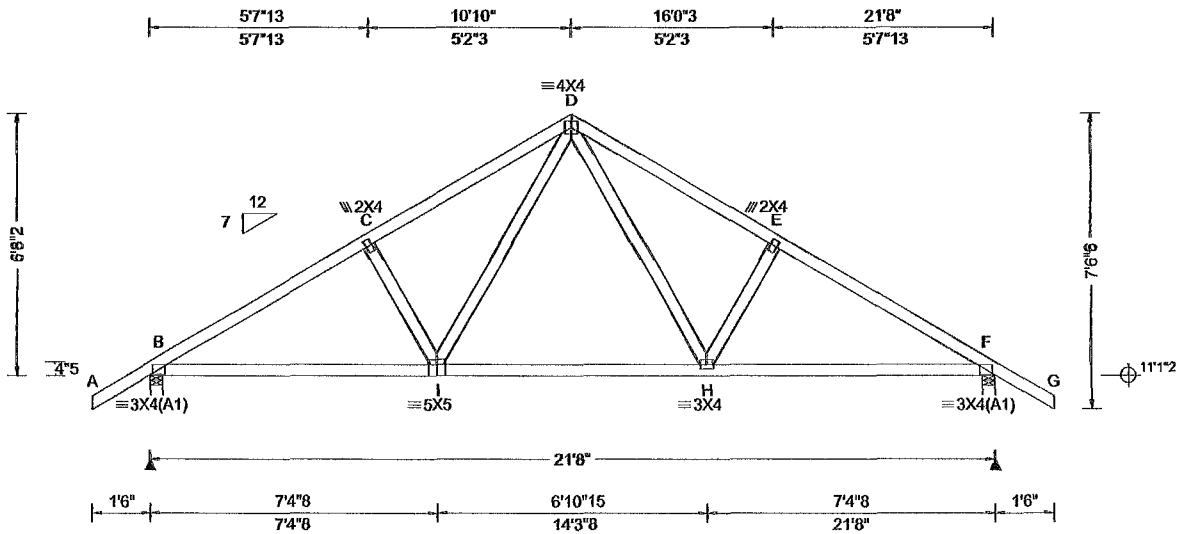
**Additional Notes**  
Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
The overall height of this truss excluding overhang is 6-4-1.



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| Loading Criteria (psf)   | Wind Criteria   | Snow Criteria (Pg,Pf in PSF)  | Defl/CSI Criteria   | ▲ Maximum Reactions (lbs)   |
|--|---|---|---|---|
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld. 40.00<br>NCBCLL 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist. 0 to h/2<br>C&C Dist a 3.00 ft<br>Loc. from endwall Any<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg: NA Ct: NA CAT NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT 20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.045 H 999 240<br>VERT(CL): 0.088 H 999 180<br>HORZ(LL): 0.019 F - -<br>HORZ(TL): 0.036 F - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.279<br>Max BC CSI: 0.526<br>Max Web CSI: 0.196<br>VIEW Ver 23.02.04.0123.14 | Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>B 1054 /- /- /605 /174 /208<br>F 1054 /- /- /605 /174 /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>F Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings B & F are a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens Comp<br>B - C 435 -1464 D - E 472 -1308<br>C - D 472 -1307 E - F 434 -1465 |

**Lumber**

Top chord 2x4 SP #2;  
Bot chord 2x4 SP #2;  
Webs: 2x4 SP #3;

**Loading**

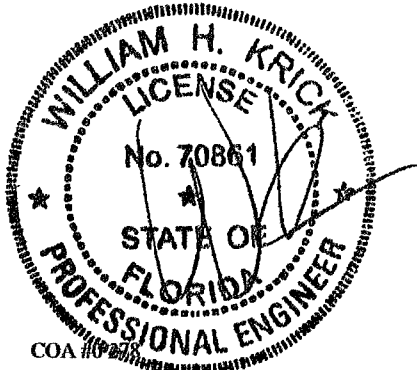
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

The overall height of this truss excluding overhang is 6-8-2.

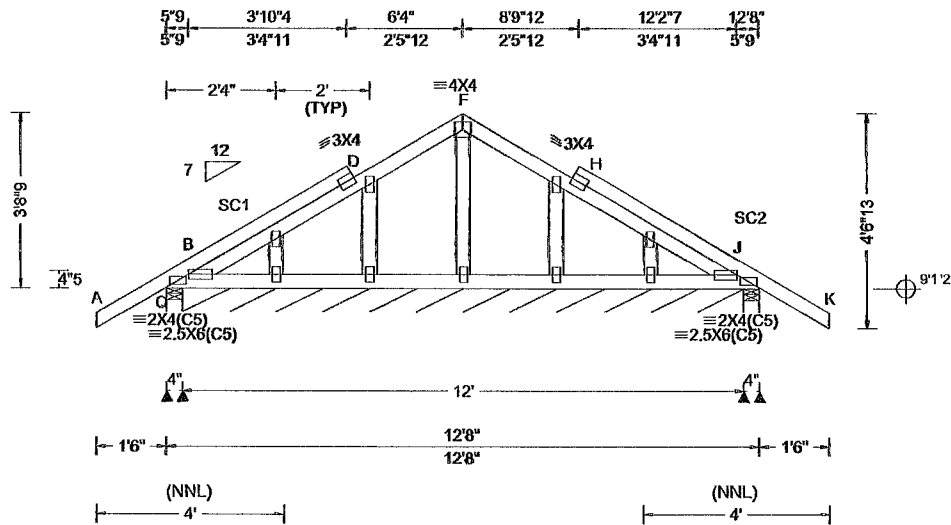


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|                           |                |                |   |   |
|---------------------------|----------------|----------------|---|---|
| SEQN: 801965<br>FROM: CDM | GABL<br>Qty: 1 | Ply 1<br>Qty 1 | Job Number: 25-2306<br>MOSS<br>Truss Label: C01 | Cust: R 215 JRef: 1Y7W2150003 T11<br>DrvNo: 062.25.0537.38430<br>KD / DF 03/03/2025 |
|---------------------------|----------------|----------------|---|---|



| Loading Criteria (psf) | Wind Criteria                 | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria               | ▲ Maximum Reactions (lbs), or * = PLF         |             |     |     |      |     |     |
|------------------------|-------------------------------|------------------------------|---------------------------------|---|-------------|-----|-----|------|-----|-----|
|                        |                               |                              |                                 | Gravity                                       | Non-Gravity |     |     |      |     |     |
|                        |                               |                              |                                 | Loc   | R+          | /R- | /Rh | /Rw  | /U  | /RL |
| TCLL 20.00             | Wind Std ASCE 7-22            | Pg: NA Ct: NA CAT NA         | PP Deflection in loc L/defl L/# | Q   | 292         | -   | -   | 184  | 167 | 136 |
| TCDL 10.00             | Speed 130 mph                 | Pf NA Ce NA                  | VERT(LL): 0.001 D 999 240       | B*  | 56          | -   | -   | 133  | 16  | -   |
| BCLL 0.00              | Enclosure: Closed             | Lu NA Cs NA                  | VERT(CL): 0.001 D 999 180       | J   | 292         | -   | -   | 1206 | 167 | -   |
| BCDL 10.00             | Risk Category: II             | Snow Duration: NA            | HORZ(LL): 0.000 I - -           | Wind reactions based on MWFRS                 |             |     |     |      |     |     |
| Des Ld: 40.00          | EXP: C Kzt: NA                | Building Code:               | HORZ(TL): 0.001 H - -           | Q Brg Wid = 4.0 Min Req = 1.5 (Truss)         |             |     |     |      |     |     |
| NCBCLL 10.00           | Mean Height: 15.00 ft         | FBC 8th Ed 2023 Res.         | Creep Factor: 2.0               | B Brg Wid = 144 Min Req = -                   |             |     |     |      |     |     |
| Soffit: 2.00           | TCDL: 5.0 psf                 | TPI Std: 2014                | Max TC CSI: 0.293               | J Brg Wid = 4.0 Min Req = 1.5 (Truss)         |             |     |     |      |     |     |
| Load Duration: 1.25    | BCDL: 5.0 psf                 | Rep Fac: Yes                 | Max BC CSI: 0.027               | Bearings Q, B, & J are a rigid surface.       |             |     |     |      |     |     |
| Spacing: 24.0"         | MWFRS Parallel Dist. 0 to h/2 | FT/RT: 20(0)/10(0)           | Max Web CSI: 0.392              | Members not listed have forces less than 375# |             |     |     |      |     |     |
|                        | C&C Dist a: 3.00 ft           | Plate Type(s):               | VIEW Ver: 23.02.04.0123.14      |   |             |     |     |      |     |     |
|                        | Loc. from endwall Any         | WAVE                         |                                 |   |             |     |     |      |     |     |
|                        | GCpi: 0.18                    |                              |                                 |   |             |     |     |      |     |     |
|                        | Wind Duration: 1.60           |                              |                                 |   |             |     |     |      |     |     |

**Lumber**  
Top chord: 2x4 SP #2,  
Bot chord: 2x4 SP #2,  
Webs: 2x4 SP #3,  
Stack Chord SC1: 2x4 SP #2,  
Stack Chord SC2: 2x4 SP #2;

**Plating Notes**  
All plates are 2X4 except as noted.

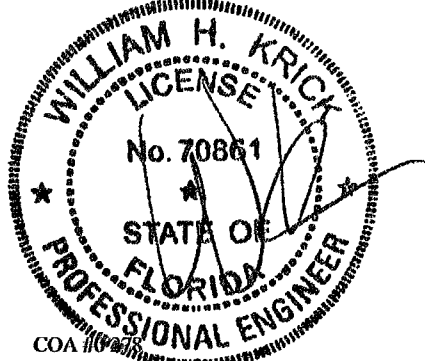
**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.  
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/753.

**Additional Notes**  
Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

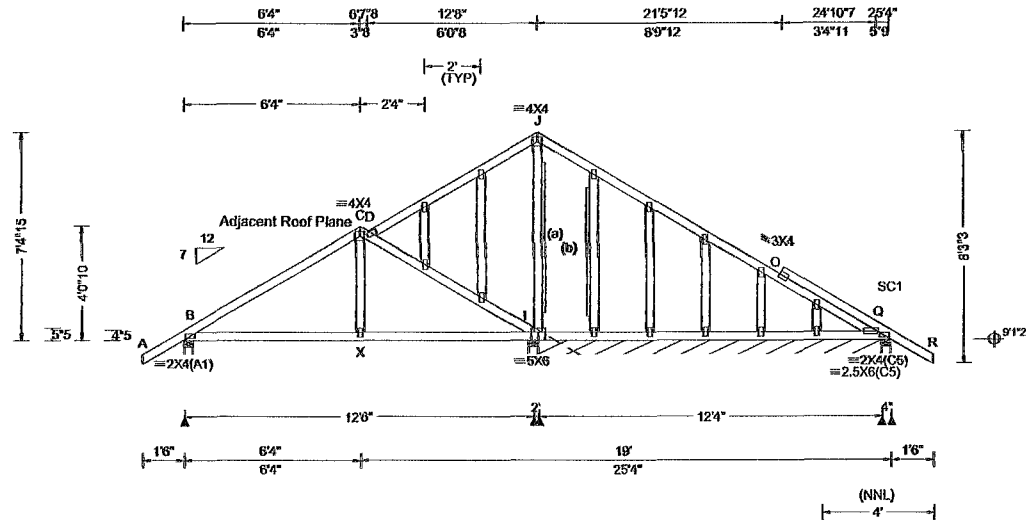
The overall height of this truss excluding overhang is 3-8-9.



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| Loading Criteria (psf) | Wind Criteria                 | Snow Criteria (Pg, Pf in PSF) | Defl/CSI Criteria               | ▲ Maximum Reactions (lbs), or * = PLF         |                                     |       |             |      |       |     |
|------------------------|-------------------------------|-------------------------------|---------------------------------|---|-------------------------------------|-------|-------------|------|-------|-----|
|                        |                               |                               |                                 | Gravity                                       |                                     |       | Non-Gravity |      |       |     |
| Loc                    | R+                            | /R-                           | /Rh                             | /Rw   | /U                                  | /RL   |             |      |       |     |
| TCLL 20.00             | Wind Std ASCE 7-22            | Pg. NA Ct: NA CAT NA          | PP Deflection in loc L/defl L/# | B   | 569                                 | -     | -           | 361  | 102   | 235 |
| TCDL 10.00             | Speed 130 mph                 | Pf NA Ce NA                   | VERT(LL) 0.039 G 999 240        | I   | 741                                 | -     | -           | 522  | 138   | -   |
| BCLL 0.00              | Enclosure: Closed             | Lr NA Cs: NA                  | VERT(CL) 0.080 G 999 180        | †   | 82                                  | -     | -           | 144  | 120   | -   |
| BCDL 10.00             | Risk Category: II             | Snow Duration: NA             | HORZ(LL) 0.021 G - -            | Q   | 270                                 | -     | -           | 166  | 118   | -   |
| Des Ld: 40.00          | EXP: C Kzt: NA                | Building Code:                | HORZ(TL) 0.044 G - -            | Wind reactions based on MWFRS                 |                                     |       |             |      |       |     |
| NCBCLL 10.00           | Mean Height: 15.00 ft         | FBC 8th Ed 2023 Res.          | Creep Factor: 2.0               | B   | Brg Wid = 4.0 Min Req = 1.5 (Truss) |       |             |      |       |     |
| Soffit: 2.00           | TCDL 5.0 psf                  | TPI Std: 2014                 | Max TC CSI 0.362                | I   | Brg Wid = 4.0 Min Req = 1.5 (Truss) |       |             |      |       |     |
| Load Duration: 1.25    | BCDL 5.0 psf                  | Rep Fac: Yes                  | Max BC CSI 0.359                | †   | Brg Wid = 147 Min Req = -           |       |             |      |       |     |
| Spacing: 24.0"         | MWFRS Parallel Dist. 0 to h/2 | FT/RT 20(0)/10(0)             | Max Web CSI 0.992               | Q   | Brg Wid = 4.0 Min Req = 1.5 (Truss) |       |             |      |       |     |
|                        | C&C Dist = 3.00 ft            | Plate Type(s):                | VIEW Ver: 23.02.04.0123.14      | Bearings B, I, I, & Q are a rigid surface     |                                     |       |             |      |       |     |
|                        | Loc. from endwall: Any        | WAVE                          |                                 | Members not listed have forces less than 375# |                                     |       |             |      |       |     |
|                        | Gcpi: 0.18                    |                               |                                 | Maximum Top Chord Forces Per Ply (lbs)        |                                     |       |             |      |       |     |
|                        | Wind Duration: 1.60           |                               |                                 | Chords  | Tens                                | Comp. | Chords      | Tens | Comp. |     |
|                        |                               |                               |                                 | B - C   | 152                                 | -519  | D - I       | 278  | -606  |     |
|                        |                               |                               |                                 | C - D   | 161                                 | -397  |             |      |       |     |

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3,  
 Slack Chord: SC1 2x4 SP #2;

**Plating Notes**  
 All plates are 2X4 except as noted

**Loading**  
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

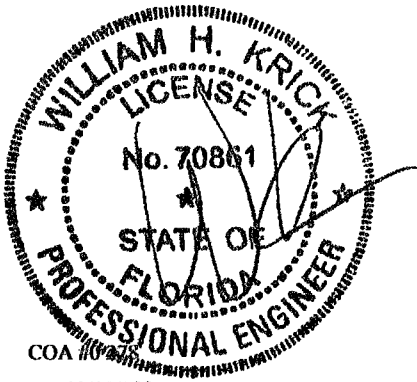
**Wind**  
 Wind loads based on MWFRS with additional C&C member design  
 Wind loading based on both gable and hip roof types.  
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/216.

**Gable Reinforcement**  
 (a) 1x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.  
 (b) 1x4 "L" reinforcement. Any species and grade. 80% length of web member Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder

**Additional Notes**  
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

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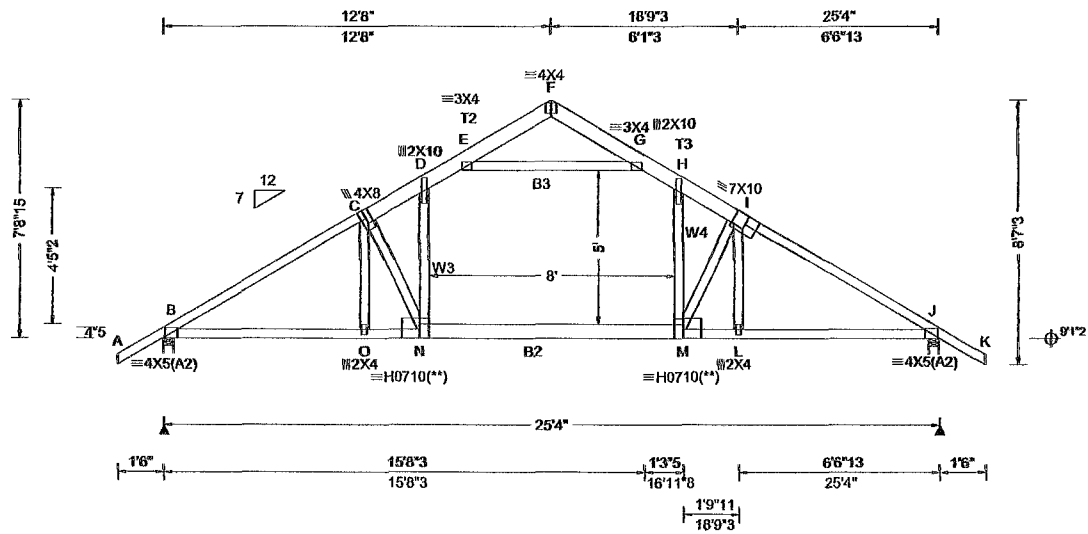
The overall height of this truss excluding overhang is 7-4-15.



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| Loading Criteria (psf)   | Wind Criteria   | Snow Criteria (Pg, Pf in PSF)   | Defl/CSI Criteria   | ▲ Maximum Reactions (lbs)  |
|--|---|---|---|--|
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40 00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP- C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: 0 to h/2<br>C&C Dist at 3.00 ft<br>Loc. from endwall. Any<br>GCpi. 0 18<br>Wind Duration: 1 60 | Pg: NA Ct: NA CAT NA<br>Pf: NA Ce NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE, HS | PP Deflection in loc L/defl L/#<br>VERT(LL) 0.134 M 999 240<br>VERT(CL) 0.319 M 941 180<br>HORZ(LL) 0.058 D - -<br>HORZ(TL) 0.139 D - -<br>Creep Factor 2 0<br>Max TC CSI 0.758<br>Max BC CSI 0.540<br>Max Web CSI 0.284<br>VIEW Ver 23.02.04.0123.14 | Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>B 1727 /- /- /692 /198 /235<br>J 1727 /- /- /692 /198 /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>J Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings B & J are a rigid surface<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens. Comp<br>B - C 447 -2716 G - H 419 -1972<br>C - D 491 -2644 H - I 477 -2616<br>D - E 419 -1971 I - J 450 -2723 |

**Lumber**  
Top chord: 2x4 SP #2; T2, T3 2x6 SP #2;  
Bot chord 2x4 SP M-31, B2 2x6 SP 2400f-2 0E,  
B3 2x4 SP #2,  
Webs: 2x4 SP #3; W3, W4 2x4 SP #2;

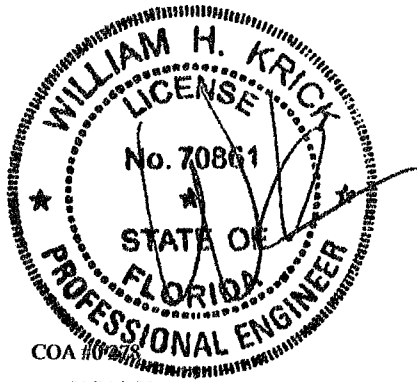
**Plating Notes**  
(\*\*) 2 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40  
PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls:  
10 PSF

**Purlins**  
Collar-tie braced with continuous lateral bracing at 24"  
oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C  
member design  
Wind loading based on both gable and hip roof types

**Additional Notes**  
The overall height of this truss excluding overhang is  
7-8-15.



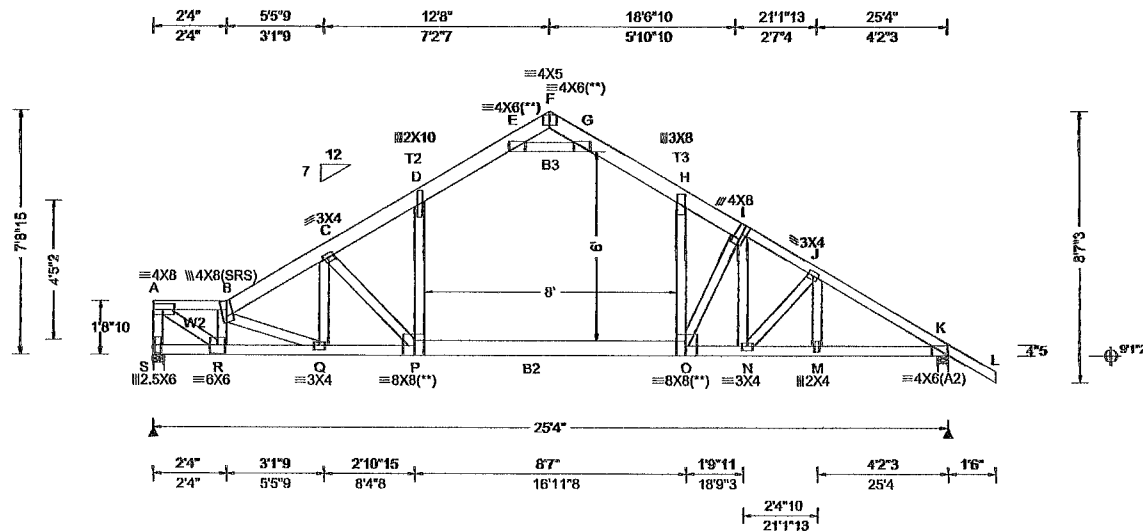
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|                           |                          |   |   |
|---------------------------|--------------------------|---|---|
| SEQN: 802002<br>FROM: CDM | ATIC<br>Ply: 1<br>Qty: 1 | Job Number: 25-2306<br>MOSS<br>Truss Label: D04 | Cust: R 215 JRef: 1Y7W2150003 T19<br>DrwNo: 062.25.0537.50690<br>KD / DF 03/03/2025 |
|---------------------------|--------------------------|---|---|



|  |   |   |  |   |
|--|---|---|--|---|
| <b>Loading Criteria (psf)</b><br>TCLL: 20 00<br>TCDL: 10.00<br>BCLL: 0.00<br>BCDL: 10.00<br>Des Ld: 40 00<br>NCBCLL: 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0 " | <b>Wind Criteria</b><br>Wind Std: ASCE 7-22<br>Speed: 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL: 5.0 psf<br>BCDL: 5.0 psf<br>MWFRS Parallel Dist: h/2 to h<br>C&C Dist a: 3.00 ft<br>Loc. from endwall: not in 9.00 ft<br>GCpi: 0.18<br>Wind Duration: 1 60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed, 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.225 P 999 240<br>VERT(CL): 0.486 P 622 180<br>HORZ(LL): 0.107 D - -<br>HORZ(TL): 0.239 D - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.561<br>Max BC CSI: 0.568<br>Max Web CSI: 0.604<br><br>VIEW Ver: 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>S 1624 /- /- /568 /175 /206<br>K 1752 /- /- /696 /198 /-<br>Wind reactions based on MWFRS<br>S Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>K Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings S & K are a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp<br>A - B 343 -2207 F - G 1299 -151<br>B - C 408 -2873 G - H 324 -1880<br>C - D 358 -2623 H - I 348 -2873<br>D - E 327 -1879 I - J 350 -2638<br>E - F 1301 -148 J - K 324 -2774 |
|--|---|---|--|---|

**Lumber**  
Top chord: 2x4 SP #2, T2, T3 2x6 SP 2400F-2 OE,  
Bot chord: 2x4 SP M-31, B2 2x6 SP 2400F-2 OE;  
B3 2x4 SP #2;  
Webs: 2x4 SP #3; W2 2x4 SP #2;

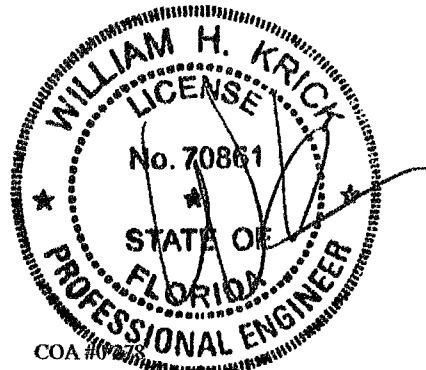
**Plating Notes**  
(\*\*) 4 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40  
PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls:  
10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat  
TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24"  
oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C  
member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is  
7-8-15.

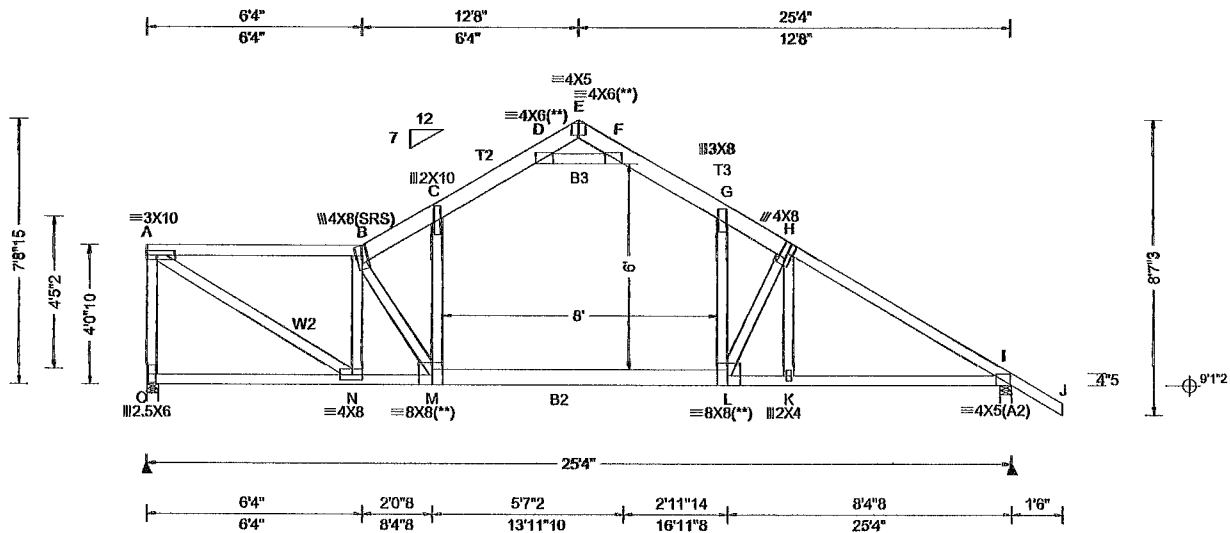


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|  |  |   |   |   |
|--|--|---|---|---|
| <b>Loading Criteria (psf)</b>  | <b>Wind Criteria</b>   | <b>Snow Criteria (Pg,Pf in PSF)</b>   | <b>Defl/CSI Criteria</b>  | <b>▲ Maximum Reactions (lbs)</b>  |
| TCLL. 20.00<br>TCDL. 10.00<br>BCLL. 0.00<br>BCDL. 10.00<br>Des Ld. 40.00<br>NCBCLL. 10.00<br>Soffit. 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL. 5.0 psf<br>BCDL. 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist a: 3.00 ft<br>Loc. from endwall: not in 9.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg NA Ct: NA CAT NA<br>Pf NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT-20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/def L/#<br>VERT(LL). 0.225 M 999 240<br>VERT(CL). 0.483 M 625 180<br>HORZ(LL). 0.090 C - -<br>HORZ(TL). 0.206 C - -<br>Creep Factor: 2.0<br>Max TC CSI 0.718<br>Max BC CSI: 0.565<br>Max Web CSI: 0.602<br><br>VIEW Ver 23.02 04.0123.14 | Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>O 1624 /- /- /546 /82 /206<br>I 1752 /- /- /706 /25 /-<br>Wind reactions based on MWFRS<br>O Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>I Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings O & I are a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens. Comp<br>A - B 477 -2281 E - F 1264 -174<br>B - C 493 -2684 F - G 375 -1878<br>C - D 381 -1873 G - H 424 -2700<br>D - E 1269 -188 H - I 375 -2764<br><br><b>Maximum Bot Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens. Comp.<br>N - M 2397 -339 L - K 2311 -207<br>M - L 1959 -126 K - I 2307 -207<br><br><b>Maximum Web Forces Per Ply (lbs)</b><br>Webs Tens.Comp. Webs Tens. Comp.<br>A - O 457 -1561 M - C 1496 -303<br>A - N 2666 -549 D - F 636 -3425<br>N - B 327 -1475 G - L 1534 -140<br>B - M 432 -963 L - H 199 -891 |

**Lumber**  
Top chord: 2x4 SP #2; T2, T3 2x6 SP 2400F-2.0E;  
Bot chord: 2x4 SP M-31, B2 2x6 SP 2400F-2.0E,  
B3 2x4 SP #2,  
Webs: 2x4 SP #3, W2 2x4 SP #2;

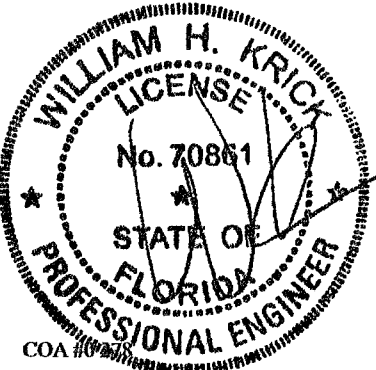
**Plating Notes**  
(\*\*) 4 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Attic room loading from 8-8-0 to 16-8-0: Live Load. 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 7-8-15.

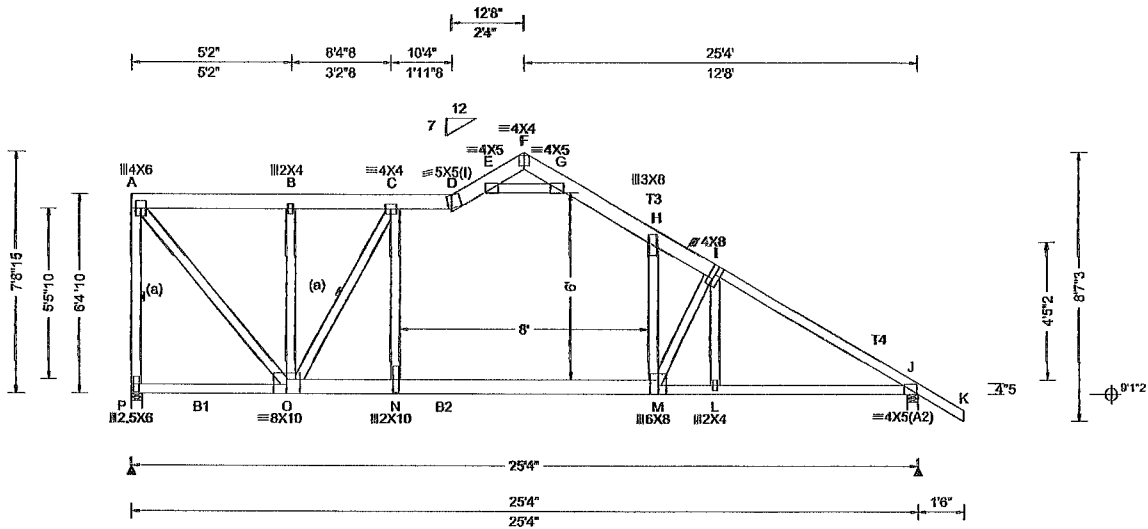


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| Loading Criteria (psf)  | Wind Criteria  | Snow Criteria (Pg, Pf in PSF)   | Defl/CSI Criteria  | ▲ Maximum Reactions (lbs)  |
|---|--|---|--|--|
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 3.00 ft<br>Loc. from endwall: not in 9.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg NA Ct: NA CAT NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.206 M 999 240<br>VERT(CL): 0.452 M 669 180<br>HORZ(LL): -0.076 H - -<br>HORZ(TL) 0.179 H - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.847<br>Max BC CSI: 0.808<br>Max Web CSI: 0.701<br><br>VIEW Ver: 23.02.04.0123.14 | Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>P 1594 /- /- /568 /152 /207<br>J 1731 /- /- /726 /50 /-<br>Wind reactions based on MWFRS<br>P Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>J Brg Wid = 4.0 Min Req = 2.0 (Truss)<br>Bearings P & J are a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens Comp<br>A - B 353 - 1185 F - G 817 - 130<br>B - C 361 - 1194 G - H 436 - 1868<br>C - D 429 - 1894 H - I 484 - 2595<br>D - E 394 - 1685 I - J 438 - 2731<br>E - F 933 - 177 |

**Lumber**  
Top chord: 2x6 SP #2; T3 2x6 SP 2400F-2.0E,  
T4 2x4 SP #2;  
Bot chord: 2x4 SP #2; B1 2x4 SP M-31,  
B2 2x6 SP 2400F-2.0E;  
Webs: 2x4 SP #3,

**Additional Notes**  
The overall height of this truss excluding overhang is 7-8-15

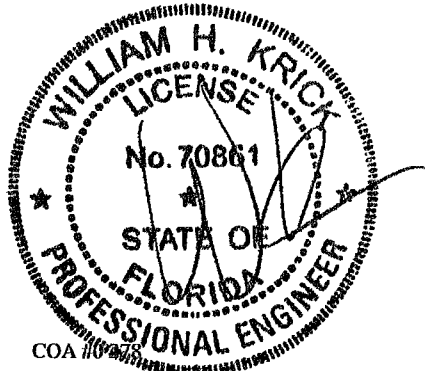
**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

**Loading**  
Attic room loading from 8-8-0 to 16-8-0: Live Load. 40 PSF, Dead Load 10 PSF Ceiling, 10 PSF, Knee walls: 10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

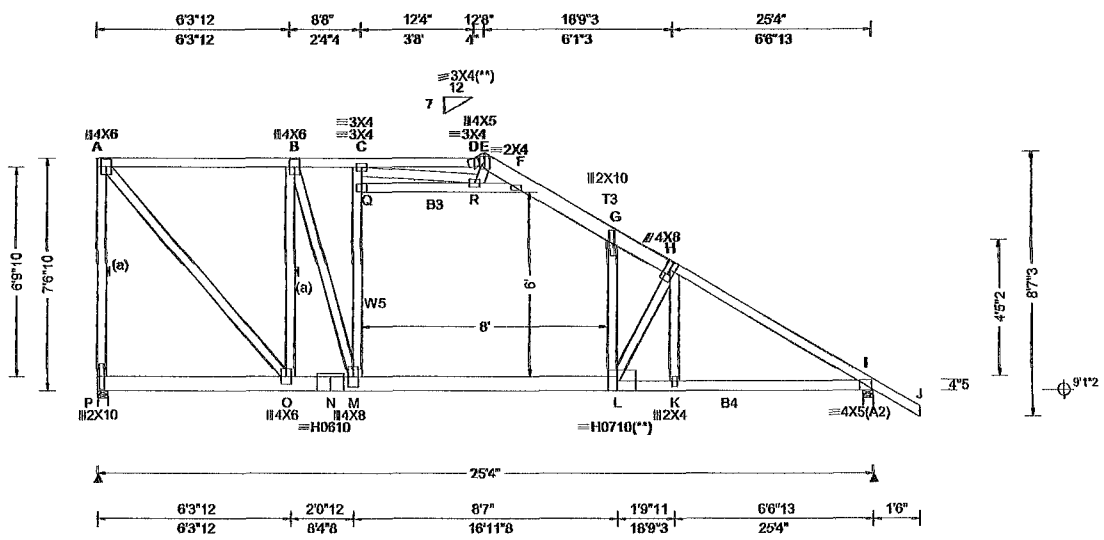
**Wind**  
Wind loads based on MWFRS with additional C&C member design  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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|  |  |  |   |  |
|--|--|--|---|--|
| <b>Loading Criteria (psf)</b>  | <b>Wind Criteria</b>   | <b>Snow Criteria (Pg,Pf in PSF)</b>  | <b>Defl/CSI Criteria</b>  | <b>▲ Maximum Reactions (lbs)</b>   |
| TCLL. 20.00<br>TCDL. 10.00<br>BCLL. 0.00<br>BCDL. 10.00<br>Des Ld. 40 00<br>NCBCLL. 10.00<br>Soffit. 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | Wind Std. ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category II<br>EXP: C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL. 5.0 psf<br>BCDL. 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 3.00 ft<br>Loc. from endwall not in 9.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg NA Ct: NA CAT NA<br>Pf NA Ce. NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE, HS | PP Deflection in loc L/defl L/#<br>VERT(LL) 0.370 L 817 240<br>VERT(CL) 0.752 L 402 180<br>HORZ(LL) -0.175 G - -<br>HORZ(TL) 0.367 G - -<br>Creep Factor: 2.0<br>Max TC CSI 0.879<br>Max BC CSI: 0.811<br>Max Web CSI: 0.958<br><br>VIEW Ver 23.02.04.0123.14 | Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>P 1635 /- /- /575 /182 /207<br>I 1757 /- /- /739 /69 /-<br>Wind reactions based on MWFRS<br>P Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>I Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings P & I are a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp<br>A - B 376 -1210 E - F 333 -752<br>B - C 440 -1748 F - G 441 -1865<br>C - D 325 -709 G - H 484 -2583<br>D - E 275 -646 H - I 457 -2784 |

**Lumber**  
Top chord 2x4 SP #2, T3 2x6 SP 2400F-2 0E,  
Bot chord 2x6 SP 2400F-2.0E; B3 2x4 SP #2,  
B4 2x4 SP M-31,  
Webs: 2x4 SP #3; W5 2x4 SP M-31.

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

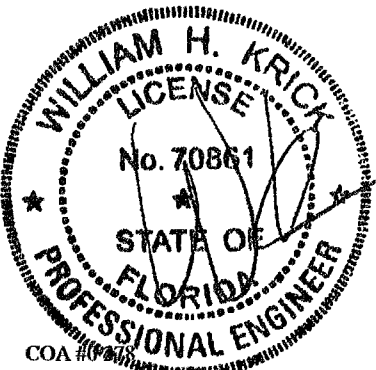
**Plating Notes**  
(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40  
PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls:  
10 PSF

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 7-8-15



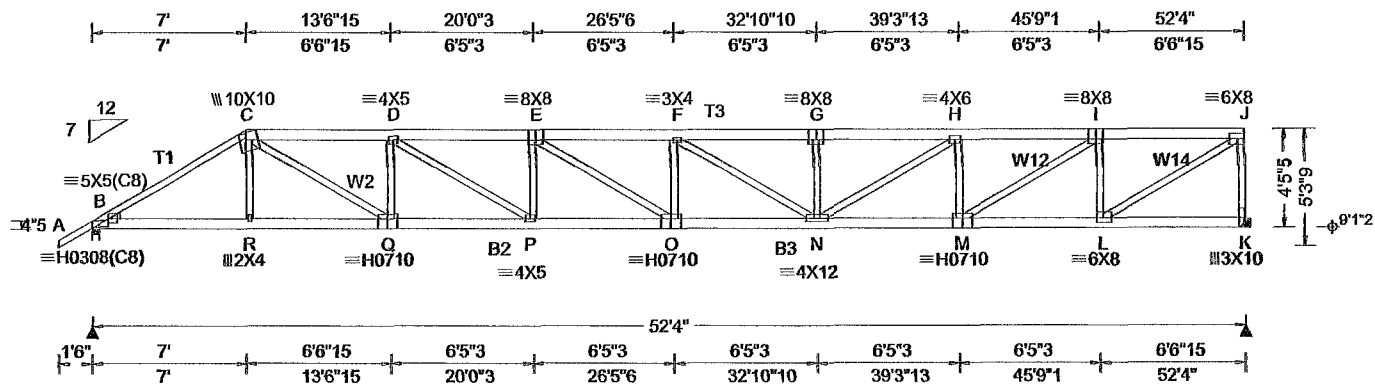
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For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI: [tpinst.org](http://tpinst.org); SBCA: [sbcacomponents.com](http://sbcacomponents.com); ICC: [iccsafe.org](http://iccsafe.org); AWC: [awc.org](http://awc.org)





2 Complete Trusses Required



|   |   |   |   |  |  |  |  |
|---|---|---|---|--|--|--|--|
| <b>Loading Criteria (psf)</b><br>TCDL 20.00<br>TCDD 10.00<br>BCCL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 0.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDD 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist. 0 to h/2<br>C&C Dist: 5.23 ft<br>Loc from endwall: not in 6.50 ft<br>GCpi 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA    Ct: NA    CAT: NA<br>Pf: NA    Ce: NA<br>Lu: NA    Cs: NA<br>Snow Duration: NA<br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: No<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>HS, WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.665 O 941 240<br>VERT(CL): 1.339 O 467 180<br>HORZ(LL): 0.134 C - -<br>HORZ(TL): 0.270 C - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.899<br>Max BC CSI: 0.840<br>Max Web CSI: 0.713<br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity      Non-Gravity<br>Loc R+ / R- / Rh      / Rw / U / RL  |  |  |  |
|   |   |   |   | B - C 5287 - / - / - / 1243 -<br>K 5255 - / - / - / 1302 -<br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0    Min Req = 3.1 (Truss)<br>K Brg Wid = -    Min Req = -<br>Bearing B is a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords    Tens.Comp.      Chords    Tens. Comp |  |  |  |

**Lumber**  
 Top chord: 2x6 SP #2, T1 2x4 SP #2;  
 T3 2x6 SP 2400F-2.0E,  
 Bot chord: 2x6 SP #2; B2,B3 2x6 SP 2400F-2.0E,  
 Webs: 2x4 SP #3; W2,W12 2x4 SP #2;  
 W14 2x4 SP M-31,  
 Lt Wedge 2x4 SP #3,

**Nailnote**  
 Nail Schedule: 0.131"x3", min. nails  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs: 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 63 plf at -1.50 to 63 plf at 7.00  
 TC: From 32 plf at 7.00 to 32 plf at 52.33  
 BC: From 5 plf at -1.50 to 5 plf at 0.00  
 BC: From 20 plf at 0.00 to 20 plf at 7.03  
 BC: From 10 plf at 7.03 to 10 plf at 52.33  
 TC: 546 lb Conc. Load at 7.03  
 TC: 190 lb Conc. Load at 9.06,11.06,13.06,15.06  
 17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06  
 33.06,35.06,37.06,39.06,41.06,43.06,45.06,47.06  
 49.06,51.06  
 BC: 401 lb Conc. Load at 7.03  
 BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06  
 17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06  
 33.06,35.06,37.06,39.06,41.06,43.06,45.06,47.06  
 49.06,51.06

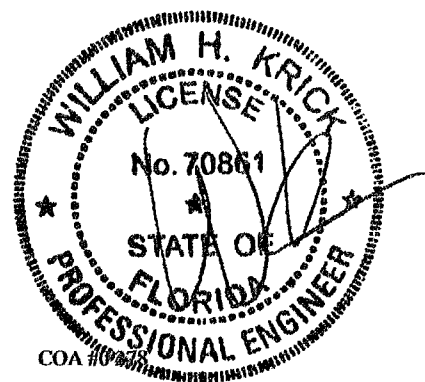
**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.  
**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - R  | 4182 -985  | O - N  | 8785 -2167  |
| R - Q  | 4193 -984  | N - M  | 6608 -1638  |
| Q - P  | 6905 -1684 | M - L  | 3982 -991   |
| P - O  | 8344 -2047 |        |             |

**Maximum Web Forces Per Ply (lbs)**

| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| C - Q | 3077 -785  | N - H | 1872 -451   |
| Q - D | 455 -1375  | H - M | 440 -1401   |
| D - P | 1657 -409  | M - I | 3008 -733   |
| P - E | 270 -679   | I - L | 634 -2104   |
| E - O | 522 -133   | L - J | 4493 -1108  |
| F - N | 175 -704   | J - K | 665 -2532   |
| G - N | 196 -389   |       |             |



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**Hangers / Ties**

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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

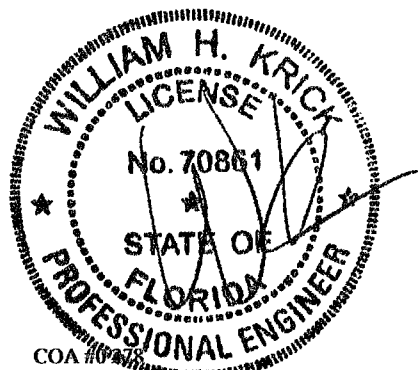
Bearing at location x=52"1" uses the following support conditions: 52"1"

Bearing K (52"1", 9"1"2) HGUS26-2  
 Supporting Member (2)2x6 SP 2400F-2.0E  
 (20) 0 162"x3.5" nails into supporting member,  
 (8) 0 162"x3.5" nails into supported member

**Additional Notes**

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

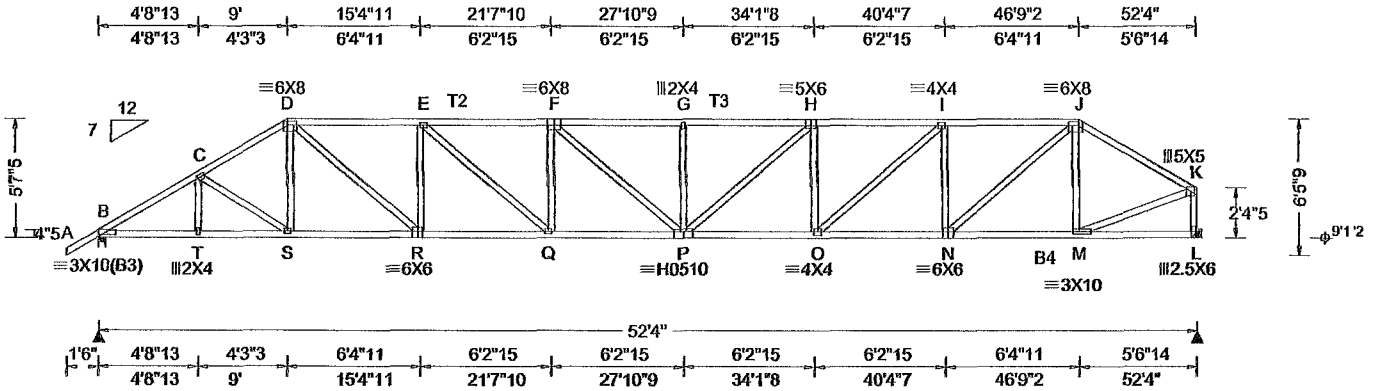
The overall height of this truss excluding overhang is 4-5-5.



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| Loading Criteria (psf)  | Wind Criteria   | Snow Criteria (Pg,Pf in PSF)   | Def/CSI Criteria   | Maximum Reactions (lbs)  |
|---|---|--|--|--|
| TCLL: 20.00<br>TCDL: 10.00<br>BCLL: 0.00<br>BCDL: 10.00<br>Des Ld: 40 00<br>NCBCLL: 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0 " | Wind Std: ASCE 7-22<br>Speed: 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL: 5.0 psf<br>BCDL: 5.0 psf<br>MWFRS Parallel Dist. h/2 to h<br>C&C Dist: 5.23 ft<br>Loc. from endwall: not in 6.50 ft<br>GCpt: 0.18<br>Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code: FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT 20(0)/10(0)<br>Plate Type(s): WAVE, HS | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.447 G 999 240<br>VERT(CL): 0.924 G 677 180<br>HORZ(LL): 0.108 D - -<br>HORZ(TL): 0.223 D - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.931<br>Max BC CSI: 0.595<br>Max Web CSI: 0.926<br>VIEW Ver: 23.02.04.0123 14 | Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>Non-Gravity<br>B 2285 /- /- /1304 /416 /160<br>L 2168 /- /- /1150 /393 /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4 0 Min Req = 1 9 (Truss)<br>L Brg Wid = - Min Req = -<br>Bearing B is a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens Comp |
|   |   |  |  | B - C 1185 - 3776 G - H 1781 - 5338<br>C - D 1222 - 3568 H - I 1667 - 4846<br>D - E 1568 - 4406 I - J 1381 - 3766<br>E - F 1760 - 5170 J - K 873 - 2487<br>F - G 1781 - 5338   |

**Lumber**  
Top chord: 2x4 SP #2, T2,T3 2x4 SP M-31,  
Bot chord: 2x4 SP M-31, B4 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 3X4 except as noted

**Hangers / Ties**  
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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.  
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.  
Bearing at location x=52'1" uses the following support conditions: 52'1"  
Bearing L (52'1", 9'1"2) HUS26  
Supporting Member (2)2x6 SP 2400f-2.0E  
(14) 0.148"x3" nails into supporting member,  
(6) 0.148"x3" nails into supported member

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc

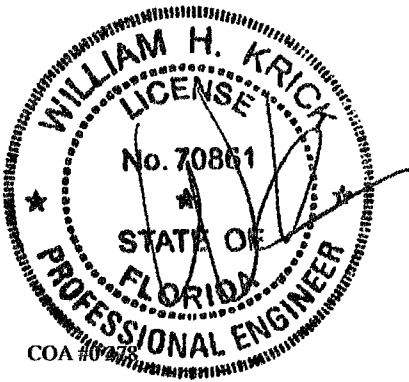
**Wind**  
Wind loads based on MWFRS with additional C&C member design  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp.  | Chords | Tens. Comp  |
|--------|-------------|--------|-------------|
| B - T  | 3181 - 1008 | Q - P  | 5197 - 1637 |
| T - S  | 3182 - 1010 | P - O  | 4888 - 1542 |
| S - R  | 3034 - 969  | O - N  | 3837 - 1269 |
| R - Q  | 4463 - 1462 | N - M  | 2082 - 680  |

**Maximum Web Forces Per Ply (lbs)**

| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| D - R | 1800 - 619 | O - I | 1352 - 432  |
| R - E | 468 - 1046 | I - N | 539 - 1309  |
| E - Q | 947 - 310  | N - J | 2208 - 737  |
| Q - F | 270 - 504  | J - M | 311 - 626   |
| G - P | 211 - 388  | M - K | 2195 - 714  |
| P - H | 597 - 219  | K - L | 749 - 2122  |
| H - O | 353 - 776  |       |             |

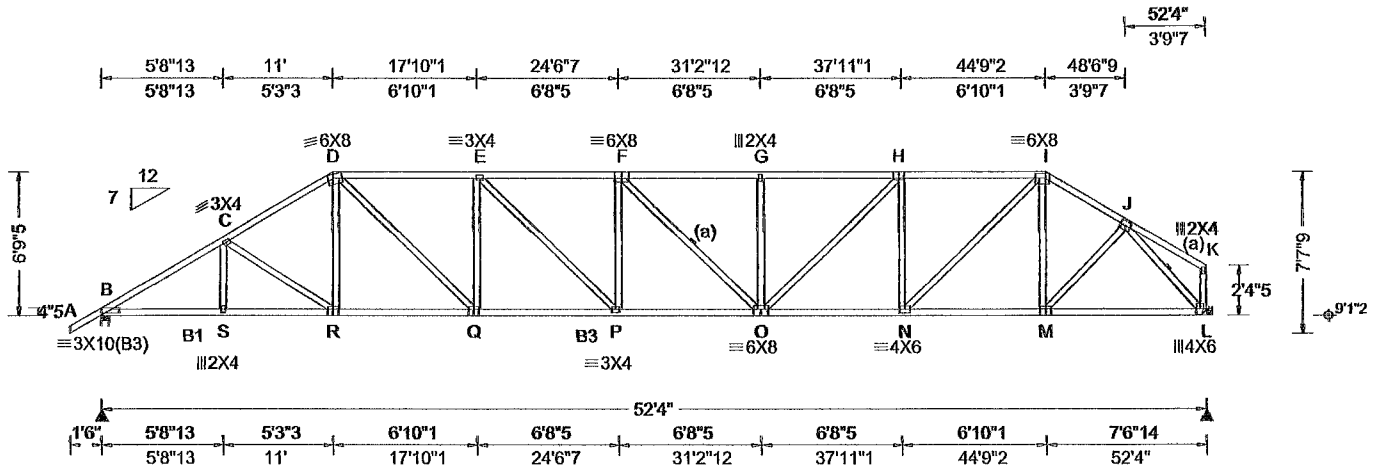


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| Loading Criteria (psf)   | Wind Criteria   | Snow Criteria (Pg,Pf in PSF)  | Defl/CSI Criteria   | Maximum Reactions (lbs)   |
|--|---|---|---|---|
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h/2 to h<br>C&C Dist a 5.23 ft<br>Loc. from endwall not in 6 50 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code: FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s): WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.371 F 999 240<br>VERT(CL): 0.768 F 815 180<br>HORZ(LL): 0.110 L - -<br>HORZ(TL): 0.227 L - -<br>Creep Factor 2.0<br>Max TC CSI 0.999<br>Max BC CSI 0.874<br>Max Web CSI 0.992<br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ /R- /Rh /Rw /U /RL<br>B 2285 /- /- /1327 /413 /193<br>L 2168 /- /- /1175 /390 /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4 0 Min Req = 1.9 (Truss)<br>L Brg Wid = - Min Req = -<br>Bearing B is a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp<br>B - C 1155 -3782 F - G 1478 -4243<br>C - D 1175 -3446 G - H 1478 -4243<br>D - E 1414 -3928 H - I 1296 -3503<br>E - F 1510 -4370 I - J 946 -2607 |

**Lumber**  
 Top chord 2x4 SP #2,  
 Bot chord 2x4 SP #2; B1,B3 2x4 SP M-31,  
 Webs: 2x4 SP #3,

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
 All plates are 5X6 except as noted.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

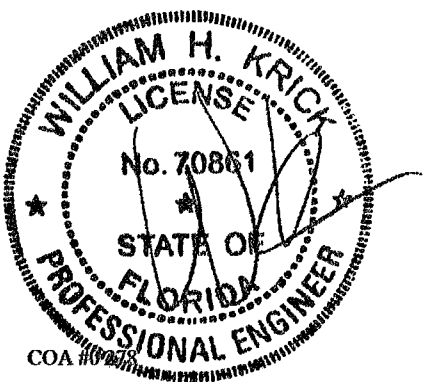
**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - S  | 3180 -972  | P - O  | 4382 -1338  |
| S - R  | 3179 -973  | O - N  | 3553 -1135  |
| R - Q  | 2911 -892  | N - M  | 2209 -676   |
| Q - P  | 3966 -1257 | M - L  | 1767 -581   |

**Maximum Web Forces Per Ply (lbs)**

| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| D - R | 394 -21    | O - H | 970 -327    |
| D - Q | 1414 -490  | H - N | 487 -1123   |
| Q - E | 411 -852   | N - I | 1800 -608   |
| E - P | 574 -209   | M - J | 653 -161    |
| G - O | 223 -407   | J - L | 877 -2640   |



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|              |      |       |                     |                                   |
|--------------|------|-------|---------------------|-----------------------------------|
| SEQN: 802208 | HIPS | Ply 1 | Job Number: 25-2306 | Cust: R 215 JRef: 1Y7W2150003 T67 |
| FROM: CDM    |      | Qty 1 | MOSS                | DrwNo: 062.25.0538.19950          |
| Page 2 of 2  |      |       | Truss Label: G03    | KD / DF 03/03/2025                |

**Hangers / Ties**

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Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

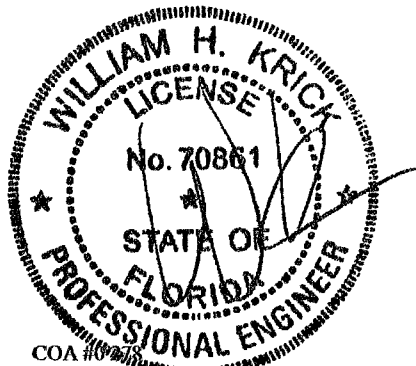
Bearing at location x=52'1" uses the following support conditions. 52'1"

Bearing L (52'1", 9'1"2) HUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(14) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member



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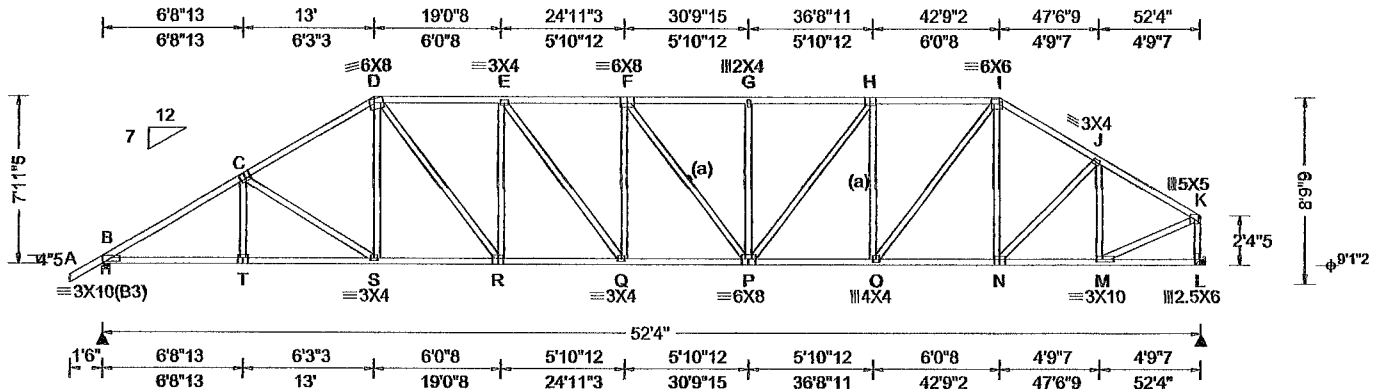
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|  |   |  |   |   |
|--|---|--|---|---|
| <b>Loading Criteria (psf)</b><br>TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h/2 to h<br>C&C Dist: 5.23 ft<br>Loc. from endwall not in 13.00 ft<br>GCpi 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT NA<br>Pf: NA Ce NA<br>Lr: NA Cs NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed. 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.315 F 999 240<br>VERT(CL): 0.651 F 961 180<br>HORZ(LL): 0.112 L - -<br>HORZ(TL): 0.232 L - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.672<br>Max BC CSI: 0.942<br>Max Web CSI: 0.846<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>B 2285 /- /- /1347 /410 /226<br>L 2168 /- /- /1196 /387 /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 2.7 (Truss)<br>L Brg Wid = - Min Req = -<br>Bearing B is a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens. Comp<br>B - C 1120 -3774 G - H 1296 -3608<br>C - D 1125 -3314 H - I 1172 -3108<br>D - E 1256 -3423 I - J 972 -2673<br>E - F 1317 -3704 J - K 760 -2329<br>F - G 1296 -3608 |
|--|---|--|---|---|

**Lumber**  
Top chord: 2x4 SP #2,  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3,

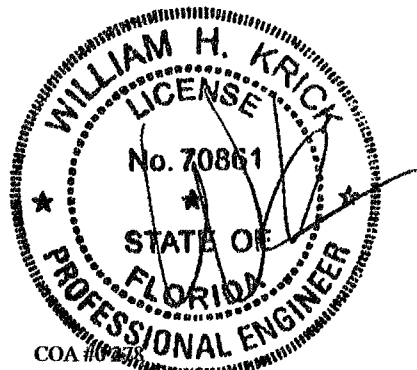
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
All plates are 5X6 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
The overall height of this truss excluding overhang is 7-11-5.



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**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - T  | 3164 -932  | Q - P  | 3713 -1101  |
| T - S  | 3163 -933  | P - O  | 3146 -963   |
| S - R  | 2777 -808  | O - N  | 2243 -653   |
| R - Q  | 3451 -1051 | N - M  | 1988 -603   |

**Maximum Web Forces Per Ply (lbs)**

| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| C - S | 176 -460   | H - O | 443 -1000   |
| D - S | 446 -39    | O - I | 1416 -481   |
| D - R | 1058 -384  | J - M | 311 -759    |
| R - E | 363 -726   | M - K | 2117 -635   |
| E - Q | 429 -169   | K - L | 697 -2125   |
| P - H | 770 -270   |       |             |

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|              |      |        |                     |                          |                   |            |
|--------------|------|--------|---------------------|--------------------------|-------------------|------------|
| SEQN: 802211 | HIPS | Ply: 1 | Job Number: 25-2306 | Cust: R 215              | JRef: 1Y7W2150003 | T66        |
| FROM: CDM    |      | Qty: 1 | MOSS                | DrwNo: 062.25.0538.22553 |                   |            |
| Page 2 of 2  |      |        | Truss Label: G04    | KD / DF                  |                   | 03/03/2025 |

**Hangers / Ties**

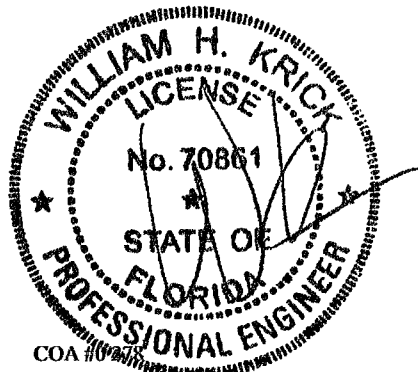
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=52'1" uses the following support conditions: 52'1"

- Bearing L (52'1", 9'1"2) HUS26
- Supporting Member: (2)2x6 SP 2400F-2.0E
- (14) 0.148"x3" nails into supporting member,
- (6) 0.148"x3" nails into supported member



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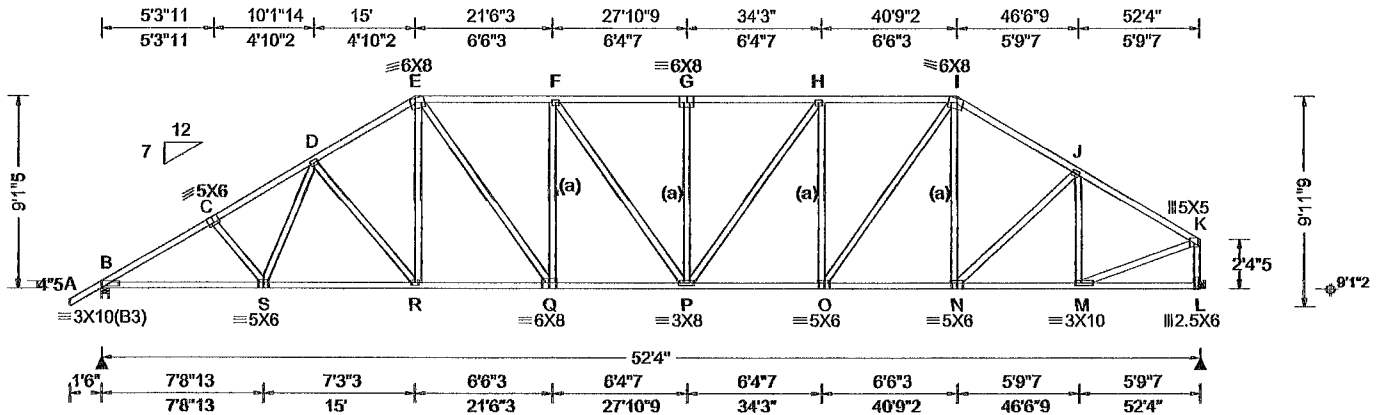
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| <b>Loading Criteria (psf)</b><br>TCLL 20.00<br>TCDD 10.00<br>BCCL 0.00<br>BCDL 10.00<br>Des Ld: 40.00<br>NCBCLL 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std: ASCE 7-22<br>Speed: 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.00 ft<br>TCDD: 5.0 psf<br>BCDL: 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 5.23 ft<br>Loc. from endwall: not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg. Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code: FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s): WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.266 G 999 240<br>VERT(CL): 0.551 G 999 180<br>HORZ(LL): 0.102 L - -<br>HORZ(TL): 0.211 L - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.695<br>Max BC CSI: 0.997<br>Max Web CSI: 0.886<br>VIEW Ver: 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b>   |          |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
|--|---|---|--|--|----------|-----|---------|--|-------------|--|--|----|-----|-----|-----|--------|---|------|---|---|-------|----------|---|------|---|---|-------|--------|--------|-------|-------|--------|-------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|------|-------|-----|-----|-------|-----|------|-------|-----|-----|-------|-----|------|
|  |   |   |  | <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U /RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>2285</td> <td>-</td> <td>-</td> <td>11364</td> <td>145 /259</td> </tr> <tr> <td>L</td> <td>2168</td> <td>-</td> <td>-</td> <td>11214</td> <td>1108 -</td> </tr> </tbody> </table> Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 2.7 (Truss)<br>L Brg Wid = - Min Req = -<br>Bearing B is a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>B-C</td> <td>1099</td> <td>-3796</td> <td>G-H</td> <td>1171</td> <td>-3220</td> </tr> <tr> <td>C-D</td> <td>1105</td> <td>-3609</td> <td>H-I</td> <td>1113</td> <td>-2921</td> </tr> <tr> <td>D-E</td> <td>1085</td> <td>-3133</td> <td>I-J</td> <td>989</td> <td>-2693</td> </tr> <tr> <td>E-F</td> <td>1161</td> <td>-3119</td> <td>J-K</td> <td>784</td> <td>-2490</td> </tr> <tr> <td>F-G</td> <td>1171</td> <td>-3220</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> |          | Loc | Gravity |  | Non-Gravity |  |  | R+ | /R- | /Rh | /Rw | /U /RL | B | 2285 | - | - | 11364 | 145 /259 | L | 2168 | - | - | 11214 | 1108 - | Chords | Tens. | Comp. | Chords | Tens. | Comp. | B-C | 1099 | -3796 | G-H | 1171 | -3220 | C-D | 1105 | -3609 | H-I | 1113 | -2921 | D-E | 1085 | -3133 | I-J | 989 | -2693 | E-F | 1161 | -3119 | J-K | 784 | -2490 | F-G | 1171 |
| Loc  | Gravity   |   | Non-Gravity  |  |          |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
|  | R+  | /R-   | /Rh  | /Rw  | /U /RL   |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| B  | 2285  | -   | -  | 11364  | 145 /259 |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| L  | 2168  | -   | -  | 11214  | 1108 -   |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| Chords   | Tens.   | Comp.   | Chords   | Tens.  | Comp.    |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| B-C  | 1099  | -3796   | G-H  | 1171   | -3220    |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| C-D  | 1105  | -3609   | H-I  | 1113   | -2921    |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| D-E  | 1085  | -3133   | I-J  | 989  | -2693    |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| E-F  | 1161  | -3119   | J-K  | 784  | -2490    |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |
| F-G  | 1171  | -3220   |  |  |          |     |         |  |             |  |  |    |     |     |     |        |   |      |   |   |       |          |   |      |   |   |       |        |        |       |       |        |       |       |     |      |       |     |      |       |     |      |       |     |      |       |     |      |       |     |     |       |     |      |       |     |     |       |     |      |

**Lumber**  
 Top chord 2x4 SP #2;  
 Bot chord 2x4 SP #2;  
 Webs: 2x4 SP #3;

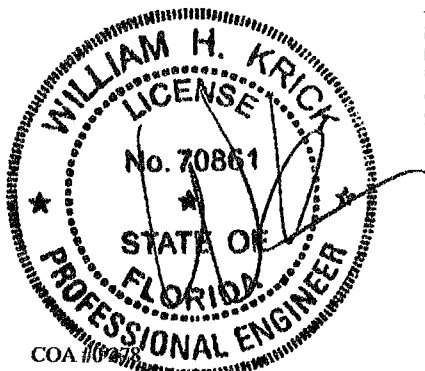
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
 All plates are 3x4 except as noted.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below  
 The overall height of this truss excluding overhang is 9'-1.5".



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|              |      |        |                     |                          |                   |     |
|--------------|------|--------|---------------------|--------------------------|-------------------|-----|
| SEQN: 802214 | HIPS | Ply: 1 | Job Number: 25-2306 | Cust: R 215              | JRef: 1Y7W2150003 | T65 |
| FROM: CDM    |      | Qty: 1 | MOSS                | DrwNo: 062.25.0538.25033 |                   |     |
| Page 2 of 2  |      |        | Truss Label: G05    | KD / DF                  | 03/03/2025        |     |

**Hangers / Ties**

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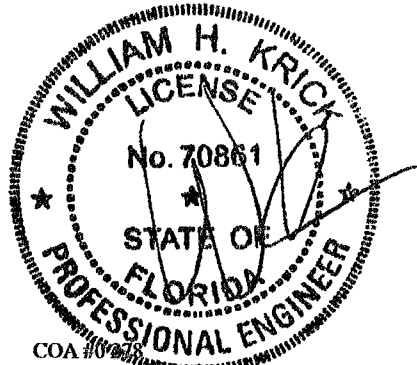
Bearing at location x=52"1" uses the following support conditions. 52"1"

Bearing L (52"1", 9"1"2) HUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(14) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member.



Florida State of Product Approval #FL 1999

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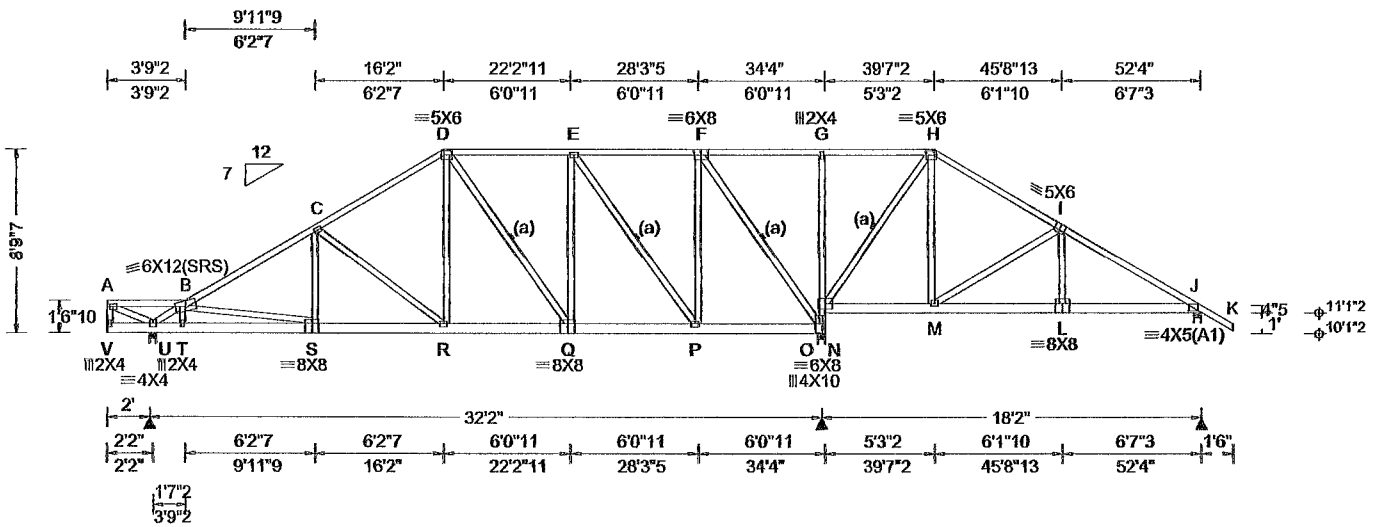
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| Loading Criteria (psf) | Wind Criteria                      | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria               | Maximum Reactions (lbs)                       |                             |
|------------------------|------------------------------------|------------------------------|---------------------------------|---|-----------------------------|
|                        |                                    |                              |                                 | Gravily                                       | Non-Gravity                 |
| TCLL 20.00             | Wind Std ASCE 7-22                 | Pg: NA Ct: NA CAT NA         | PP Deflection in loc L/defl L/# | Loc R+  | /R- /Rh /Rw /U /RL          |
| TCDL 10.00             | Speed 130 mph                      | Pf: NA Ce: NA                | VERT(LL) 0.059 R 999 240        | U 1484 /-                                     | /- /893 /85 /238            |
| BCLL 0.00              | Enclosure: Closed                  | Lr: NA Cs: NA                | VERT(CL) 0.123 R 999 180        | O 2167 /-                                     | /- /1144 /150 /-            |
| BCDL 10.00             | Risk Category: II                  | Snow Duration: NA            | HORZ(LL) 0.020 O - -            | J 830 /-                                      | /- /622 /86 /-              |
| Des Ld 40.00           | EXP-C Kzt: NA                      | Building Code:               | HORZ(TL) 0.042 O - -            | Wind reactions based on MWFRS                 |                             |
| NCBCLL 10.00           | Mean Height: 15.00 ft              | FBC 8th Ed 2023 Res.         | Creep Factor: 2.0               | U Brg Wid = 4.0 Min Req = 1.5 (Truss)         |                             |
| Soffit: 2.00           | TCDL 5.0 psf                       | TPI Std: 2014                | Max TC CSI 0.467                | O Brg Wid = 4.0 Min Req = 2.6 (Truss)         |                             |
| Load Duration: 1.25    | BCDL 5.0 psf                       | Rep Fac: Yes                 | Max BC CSI 0.365                | J Brg Wid = 4.0 Min Req = 1.5 (Truss)         |                             |
| Spacing: 24.0"         | MWFRS Parallel Dist: h to 2h       | FT/RT:20(0)/10(0)            | Max Web CSI 0.730               | Bearings U, O, & J are a rigid surface        |                             |
|                        | C&C Dist a: 5.23 ft                | Plate Type(s):               | VIEW Ver 23.02.04.0123.14       | Members not listed have forces less than 375# |                             |
|                        | Loc. from endwall: not in 13.00 ft | WAVE                         |                                 | <b>Maximum Top Chord Forces Per Ply (lbs)</b> |                             |
|                        | GCpi: 0.18                         |                              |                                 | Chords  | Tens.Comp. Chords Tens Comp |
|                        | Wind Duration: 1 60                |                              |                                 | B - C   | 853 -1936 E - F 496 -692    |
|                        |                                    |                              |                                 | C - D   | 648 -1499 H - I 332 -498    |
|                        |                                    |                              |                                 | D - E   | 635 -1127 I - J 354 -1019   |

**Lumber**  
 Top chord 2x4 SP #2,  
 Bot chord 2x6 SP #2  
 Webs: 2x4 SP #3,

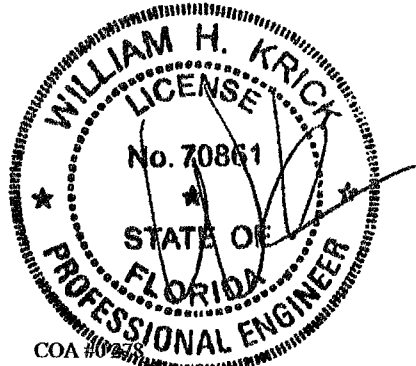
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
 All plates are 3X4 except as noted.

**Purlins**  
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**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Left end vertical not exposed to wind pressure.  
 Left cantilever is exposed to wind  
 Wind loading based on both gable and hip roof types.

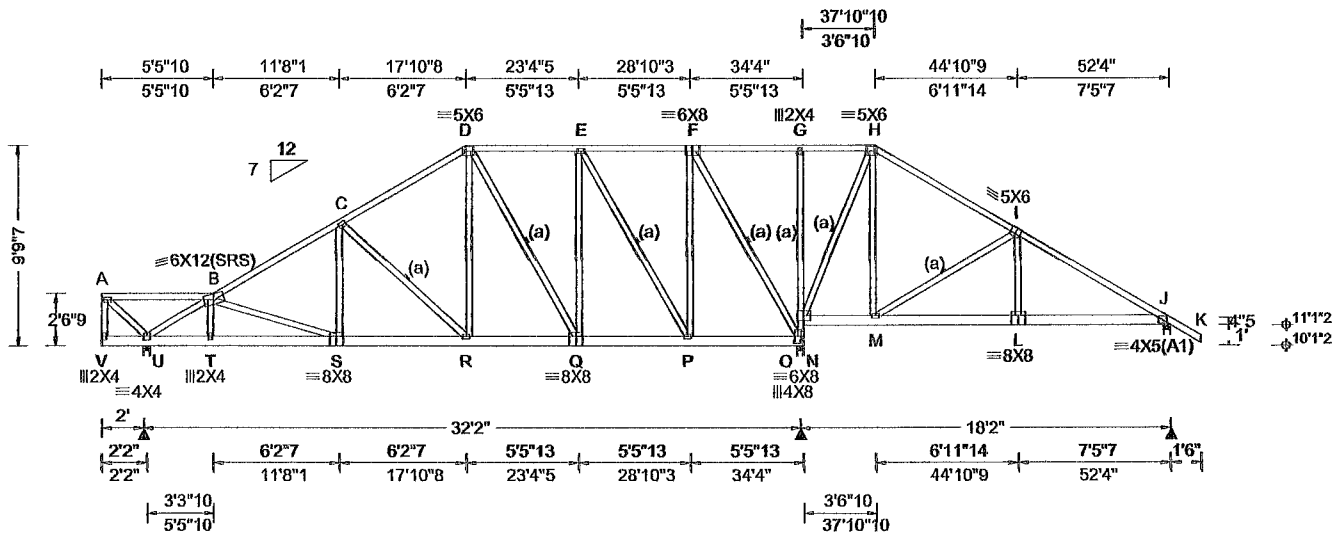
**Additional Notes**  
**WARNING:** Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
 The overall height of this truss excluding overhang is 8-9-7



Florida Certificate of Product Approval #FL 1999  
 03/03/2025

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 For more information see these web sites: Alpine. alpineitw.com; TPI: tpinst.org; SBCA: sbacomponents.com; ICC: iccsafe.org, AWC awc.org





| <b>Loading Criteria (psf)</b><br>TCDL 20.00<br>TCCL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std: ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 15.23 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 5.23 ft<br>Loc from endwall not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code:<br>FBC 8th Ed. 2023 Rec.<br>TPI Std. 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.062 S 999 240<br>VERT(CL): 0.130 S 999 180<br>HORZ(LL): 0.020 O - -<br>HORZ(TL): 0.041 O - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.558<br>Max BC CSI: 0.376<br>Max Web CSI: 0.779<br>VIEW Ver: 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br><table border="1"> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> <tr> <td>U</td> <td>1483</td> <td>-</td> <td>-</td> <td>871</td> <td>104</td> <td>1254</td> </tr> <tr> <td>O</td> <td>2169</td> <td>-</td> <td>-</td> <td>1159</td> <td>112</td> <td>-</td> </tr> <tr> <td>J</td> <td>830</td> <td>-</td> <td>-</td> <td>1636</td> <td>199</td> <td>-</td> </tr> </table> <p>Wind reactions based on MWFRS<br/>         U Brg Wid = 4.0 Min Req = 1.5 (Truss)<br/>         O Brg Wid = 4.0 Min Req = 2.6 (Truss)<br/>         J Brg Wid = 4.0 Min Req = 1.5 (Truss)</p> <p>Bearings U, O, &amp; J are a rigid surface<br/>         Members not listed have forces less than 375#<br/> <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>B - C</td> <td>659 - 1845</td> <td>E - F</td> <td>489 - 550</td> </tr> <tr> <td>C - D</td> <td>640 - 1362</td> <td>I - J</td> <td>364 - 998</td> </tr> <tr> <td>D - E</td> <td>607 - 953</td> <td></td> <td></td> </tr> </table> | Loc | Gravity |  |  | Non-Gravity |  |  | R+ | /R- | /Rh | /Rw | /U | /RL | U | 1483 | - | - | 871 | 104 | 1254 | O | 2169 | - | - | 1159 | 112 | - | J | 830 | - | - | 1636 | 199 | - | Chords | Tens.Comp. | Chords | Tens. Comp. | B - C | 659 - 1845 | E - F | 489 - 550 | C - D | 640 - 1362 | I - J | 364 - 998 | D - E | 607 - 953 |  |  |
|--|--|---|--|--|-----|---------|--|--|-------------|--|--|----|-----|-----|-----|----|-----|---|------|---|---|-----|-----|------|---|------|---|---|------|-----|---|---|-----|---|---|------|-----|---|--------|------------|--------|-------------|-------|------------|-------|-----------|-------|------------|-------|-----------|-------|-----------|--|--|
| Loc  | Gravity  |   |  | Non-Gravity  |     |         |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
|  | R+   | /R-   | /Rh  | /Rw  | /U  | /RL     |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| U  | 1483   | -   | -  | 871  | 104 | 1254    |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| O  | 2169   | -   | -  | 1159   | 112 | -       |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| J  | 830  | -   | -  | 1636   | 199 | -       |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| Chords   | Tens.Comp.   | Chords  | Tens. Comp.  |  |     |         |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| B - C  | 659 - 1845   | E - F   | 489 - 550  |  |     |         |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| C - D  | 640 - 1362   | I - J   | 364 - 998  |  |     |         |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |
| D - E  | 607 - 953  |   |  |  |     |         |  |  |             |  |  |    |     |     |     |    |     |   |      |   |   |     |     |      |   |      |   |   |      |     |   |   |     |   |   |      |     |   |        |            |        |             |       |            |       |           |       |            |       |           |       |           |  |  |

**Lumber**

Top chord 2x4 SP #2;  
 Bot chord 2x6 SP #2,  
 Webs: 2x4 SP #3,

**Bracing**

(a) Continuous lateral restraint equally spaced on member

**Plating Notes**

All plates are 3X4 except as noted.

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Left end vertical not exposed to wind pressure.  
 Left cantilever is exposed to wind  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

**WARNING:** Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
 The overall height of this truss excluding overhang is 9-9.7



**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| U - T  | 1761 - 522 | Q - P  | 942 - 253   |
| T - S  | 1760 - 528 | P - O  | 534 - 128   |
| S - R  | 1504 - 405 | M - L  | 765 - 169   |
| R - Q  | 1087 - 242 | L - J  | 768 - 168   |

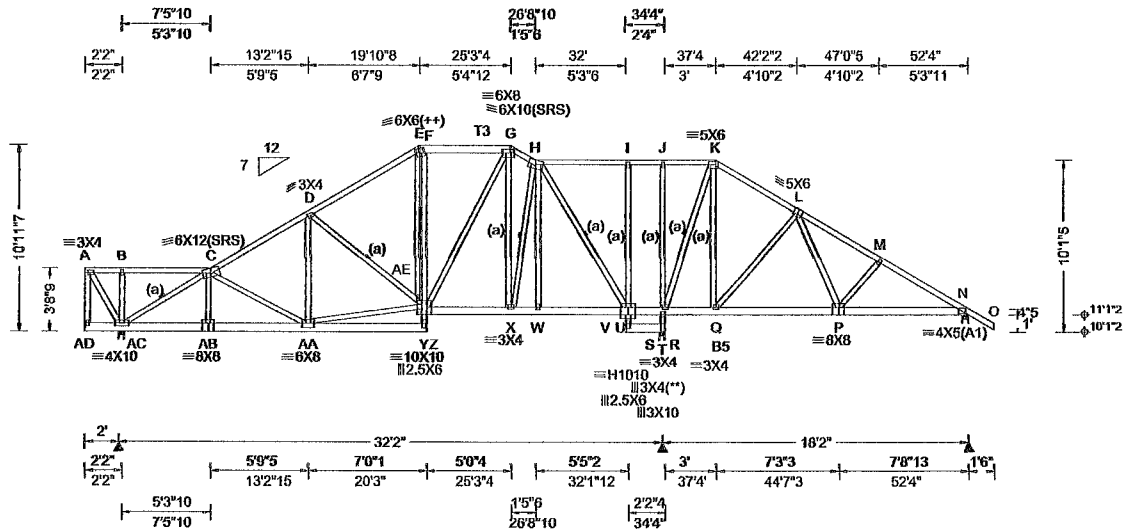
**Maximum Web Forces Per Ply (lbs)**

| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| U - B | 823 - 2205 | F - O | 406 - 1324  |
| C - R | 223 - 585  | O - N | 279 - 960   |
| D - R | 530 - 94   | N - H | 94 - 703    |
| Q - E | 391 - 25   | H - M | 466 - 49    |
| E - P | 248 - 804  | M - I | 231 - 648   |
| P - F | 825 - 157  |       |             |

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.  
 Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see these web sites. Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org  
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|                           |                          |   |   |
|---------------------------|--------------------------|---|---|
| SEQN: 802253<br>FROM: CDM | SPEC<br>Ply: 1<br>Qty: 1 | Job Number: 25-2306<br>MOSS<br>Truss Label: G08 | Cust: R215 JRef:1Y7W2150003 T52<br>DrwNo: 062.25.0538.34000<br>KD / DF 03/03/2025 |
|---------------------------|--------------------------|---|---|



| <b>Loading Criteria (psf)</b><br>TCDL 20 00<br>TCCL 10.00<br>BCCL 0.00<br>BCDL 10.00<br>Des Ld 40 00<br>NCBCLL 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 15 81 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist a: 5 23 ft<br>Loc. from endwall not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE, HS | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.151 W 999 240<br>VERT(CL): 0.310 W 999 180<br>HORZ(LL): 0.157 S - -<br>HORZ(TL): 0.326 S - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.856<br>Max BC CSI: 0.938<br>Max Web CSI: 0.722<br>VIEW Ver 23.02 04.0123.14 | <b>Maximum Reactions (lbs)</b><br><table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U /RL</th> </tr> </thead> <tbody> <tr> <td>AC 1571</td> <td>-</td> <td>-</td> <td>/910</td> <td>/110</td> <td>/283</td> </tr> <tr> <td>S 1985</td> <td>-</td> <td>-</td> <td>/1057</td> <td>/90</td> <td>-</td> </tr> <tr> <td>N 917</td> <td>-</td> <td>-</td> <td>/622</td> <td>/38</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS<br/>         AC Brg Wid = 4.0 Min Req = 1.5 (Truss)<br/>         S Brg Wid = 4.0 Min Req = 2.3 (Truss)<br/>         N Brg Wid = 4.0 Min Req = 1.5 (Truss)<br/>         Bearings AC, S, &amp; N are a rigid surface<br/>         Members not listed have forces less than 375#<br/> <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>624 - 1882</td> <td>G - H</td> <td>630 - 1242</td> </tr> <tr> <td>D - E</td> <td>617 - 1542</td> <td>K - L</td> <td>270 - 437</td> </tr> <tr> <td>E - F</td> <td>572 - 1174</td> <td>L - M</td> <td>322 - 1023</td> </tr> <tr> <td>F - G</td> <td>600 - 1246</td> <td>M - N</td> <td>320 - 1211</td> </tr> </tbody> </table> <p><b>Maximum Bot Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens Comp.</th> </tr> </thead> <tbody> <tr> <td>AC-AB</td> <td>1831 - 475</td> <td>W - U</td> <td>1069 - 174</td> </tr> <tr> <td>AB-AA</td> <td>1828 - 478</td> <td>Q - P</td> <td>660 - 44</td> </tr> <tr> <td>Y - X</td> <td>1041 - 142</td> <td>P - N</td> <td>988 - 160</td> </tr> <tr> <td>X - W</td> <td>1062 - 172</td> <td></td> <td></td> </tr> </tbody> </table> <p><b>Maximum Web Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens Comp.</th> </tr> </thead> <tbody> <tr> <td>AC - C</td> <td>711 - 2144</td> <td>H - U</td> <td>466 - 1369</td> </tr> <tr> <td>AA - Y</td> <td>1546 - 332</td> <td>J - R</td> <td>223 - 486</td> </tr> <tr> <td>D - AE</td> <td>200 - 409</td> <td>T - S</td> <td>707 - 2015</td> </tr> <tr> <td>AE - Y</td> <td>187 - 378</td> <td>Q - L</td> <td>227 - 558</td> </tr> <tr> <td>Y - G</td> <td>453 - 100</td> <td>L - P</td> <td>462 - 58</td> </tr> <tr> <td>H - W</td> <td>393 - 95</td> <td></td> <td></td> </tr> </tbody> </table> | Loc    | Gravity |  | Non-Gravity |  |  | R+ | /R- | /Rh | /Rw | /U /RL | AC 1571 | - | - | /910 | /110 | /283 | S 1985 | - | - | /1057 | /90 | - | N 917 | - | - | /622 | /38 | - | Chords | Tens.Comp. | Chords | Tens Comp. | C - D | 624 - 1882 | G - H | 630 - 1242 | D - E | 617 - 1542 | K - L | 270 - 437 | E - F | 572 - 1174 | L - M | 322 - 1023 | F - G | 600 - 1246 | M - N | 320 - 1211 | Chords | Tens.Comp. | Chords | Tens Comp. | AC-AB | 1831 - 475 | W - U | 1069 - 174 | AB-AA | 1828 - 478 | Q - P | 660 - 44 | Y - X | 1041 - 142 | P - N | 988 - 160 | X - W | 1062 - 172 |  |  | Webs | Tens.Comp. | Webs | Tens Comp. | AC - C | 711 - 2144 | H - U | 466 - 1369 | AA - Y | 1546 - 332 | J - R | 223 - 486 | D - AE | 200 - 409 | T - S | 707 - 2015 | AE - Y | 187 - 378 | Q - L | 227 - 558 | Y - G | 453 - 100 | L - P | 462 - 58 | H - W | 393 - 95 |  |  |
|---|---|---|---|---|--------|---------|--|-------------|--|--|----|-----|-----|-----|--------|---------|---|---|------|------|------|--------|---|---|-------|-----|---|-------|---|---|------|-----|---|--------|------------|--------|------------|-------|------------|-------|------------|-------|------------|-------|-----------|-------|------------|-------|------------|-------|------------|-------|------------|--------|------------|--------|------------|-------|------------|-------|------------|-------|------------|-------|----------|-------|------------|-------|-----------|-------|------------|--|--|------|------------|------|------------|--------|------------|-------|------------|--------|------------|-------|-----------|--------|-----------|-------|------------|--------|-----------|-------|-----------|-------|-----------|-------|----------|-------|----------|--|--|
| Loc   | Gravity   |   | Non-Gravity   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
|   | R+  | /R-   | /Rh   | /Rw   | /U /RL |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AC 1571   | -   | -   | /910  | /110  | /283   |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| S 1985  | -   | -   | /1057   | /90   | -      |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| N 917   | -   | -   | /622  | /38   | -      |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| Chords  | Tens.Comp.  | Chords  | Tens Comp.  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| C - D   | 624 - 1882  | G - H   | 630 - 1242  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| D - E   | 617 - 1542  | K - L   | 270 - 437   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| E - F   | 572 - 1174  | L - M   | 322 - 1023  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| F - G   | 600 - 1246  | M - N   | 320 - 1211  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| Chords  | Tens.Comp.  | Chords  | Tens Comp.  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AC-AB   | 1831 - 475  | W - U   | 1069 - 174  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AB-AA   | 1828 - 478  | Q - P   | 660 - 44  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| Y - X   | 1041 - 142  | P - N   | 988 - 160   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| X - W   | 1062 - 172  |   |   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| Webs  | Tens.Comp.  | Webs  | Tens Comp.  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AC - C  | 711 - 2144  | H - U   | 466 - 1369  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AA - Y  | 1546 - 332  | J - R   | 223 - 486   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| D - AE  | 200 - 409   | T - S   | 707 - 2015  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| AE - Y  | 187 - 378   | Q - L   | 227 - 558   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| Y - G   | 453 - 100   | L - P   | 462 - 58  |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |
| H - W   | 393 - 95  |   |   |   |        |         |  |             |  |  |    |     |     |     |        |         |   |   |      |      |      |        |   |   |       |     |   |       |   |   |      |     |   |        |            |        |            |       |            |       |            |       |            |       |           |       |            |       |            |       |            |       |            |        |            |        |            |       |            |       |            |       |            |       |          |       |            |       |           |       |            |  |  |      |            |      |            |        |            |       |            |        |            |       |           |        |           |       |            |        |           |       |           |       |           |       |          |       |          |  |  |

**Lumber**  
 Top chord 2x4 SP #2, T3 2x6 SP #2,  
 Bot chord 2x6 SP #2, B5 2x6 SP 2400f-2 0E,  
 Webs: 2x4 SP #3,

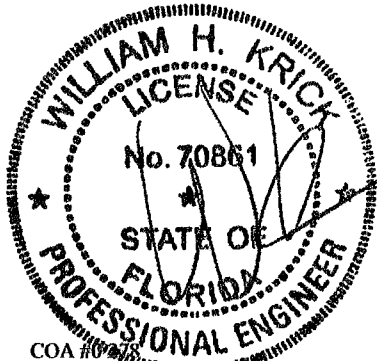
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
 All plates are 2x4 except as noted.  
 (++) - This plate works for both joints covered.  
 (\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Left end vertical not exposed to wind pressure.  
 Left cantilever is exposed to wind  
 Wind loading based on both gable and hip roof types.  
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).

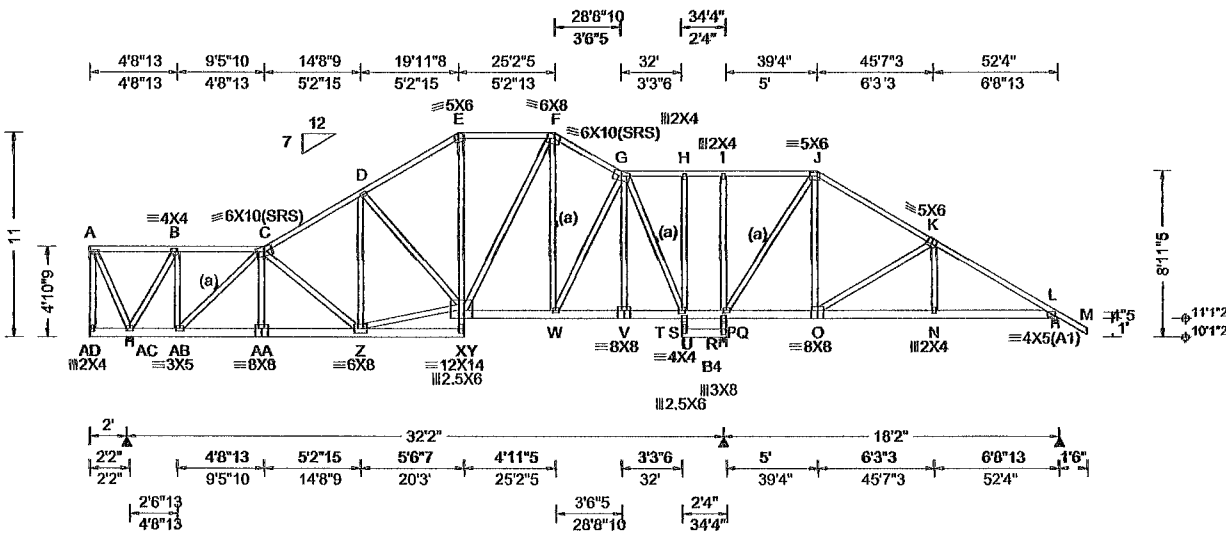
**Additional Notes**  
 WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
 The overall height of this truss excluding overhang is 10-11-7



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**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see these web sites: Alpine: alpinetw.com; TPI: tpinst.org; SBCEA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





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| <b>Loading Criteria (psf)</b><br>TCLL 20 00<br>TCDL 10 00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld: 40 00<br>NCBCLL 10 00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std: ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 16 39 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist a: 5.23 ft<br>Loc. from endwall not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1 60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br>Building Code: FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s): WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.136 U 999 240<br>VERT(CL): 0.281 V 999 180<br>HORZ(LL): 0.153 Q - -<br>HORZ(TL): 0.318 Q - -<br>Creep Factor: 2.0<br>Max TC CSI: 0.702<br>Max BC CSI: 0.477<br>Max Web CSI: 0.781<br>VIEW Ver: 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>AC 1561 /- /- /874 /105 /283<br>Q 2003 /- /- /1061 /39 /-<br>L 904 /- /- /618 /31 /-<br>Non-Gravity<br>Wind reactions based on MWFRS<br>AC Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Q Brg Wid = 4.0 Min Req = 2.4 (Truss)<br>L Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings AC, Q, & L are a rigid surface<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens. Comp. |
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord 2x6 SP #2; B4 2x6 SP 2400F-2.0E,  
Webs: 2x4 SP #3,  
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).

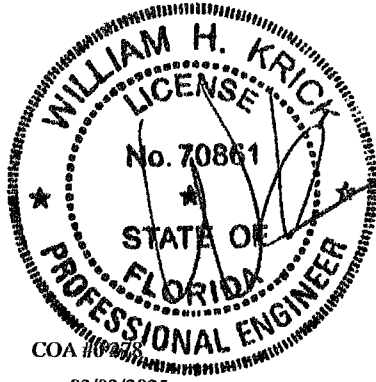
**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
All plates are 3X4 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Left end vertical not exposed to wind pressure.  
Left cantilever is exposed to wind  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
**WARNING:** Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
The overall height of this truss excluding overhang is 11-0-0.



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**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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For more information see these web sites: Alpine alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org







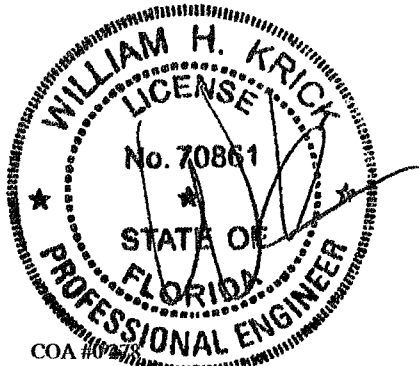


|              |      |        |                     |                          |                   |            |
|--------------|------|--------|---------------------|--------------------------|-------------------|------------|
| SEQN: 802310 | SPEC | Ply 1  | Job Number: 25-2306 | Cust: R 215              | JRef: 1Y7W2150003 | T25        |
| FROM: CDM    |      | Qty: 1 | MOSS                | DrwNo: 062.25.0538.52993 |                   |            |
| Page 2 of 2  |      |        | Truss Label: G12    | KD / DF                  |                   | 03/03/2025 |

**Additional Notes**

**WARNING:** Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below

The overall height of this truss excluding overhang is 11-0-0



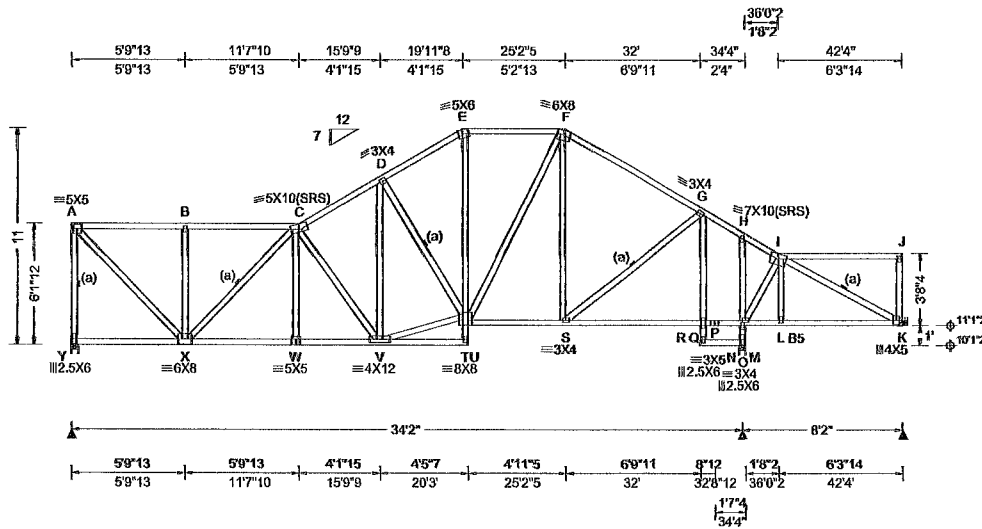
Florida Certificate of Product Approval #FL 1999

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 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.

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For more information see these web sites: Alpine: [alpineitw.com](http://alpineitw.com); TPI: [tpinst.org](http://tpinst.org); SBCA: [sbccomponents.com](http://sbccomponents.com); ICC: [tccsafe.org](http://tccsafe.org); AWC: [awc.org](http://awc.org)





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| <b>Loading Criteria (psf)</b><br>TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing 24.0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category II<br>EXP-C Kzt: NA<br>Mean Height: 18.70 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 4.23 ft<br>Loc. from endwall, not in 13 00 ft<br>GCpt: 0.18<br>Wind Duration: 1 60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed. 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT: 20(0)/10(0)<br>Plate Type(s):<br>WAVE | <b>Def/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL) 0.130 V 999 240<br>VERT(CL) 0.269 V 999 180<br>HORZ(LL) 0.114 N - -<br>HORZ(TL) 0.237 N - -<br>Creep Factor: 2.0<br>Max TC CSI 0.730<br>Max BC CSI 0.662<br>Max Web CSI 0.758<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>Non-Gravity<br>Y 1602 - / - / 1823 / 112 / 173<br>N 831 - / - / 1451 / 3 -<br>K 1085 - / - / 1578 / 8 -<br>Wind reactions based on MWFRS<br>Y Brg Wid = 4.0 Min Req = 1.9 (Truss)<br>N Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>K Brg Wid = - Min Req = -<br>Bearings Y & N are a rigid surface<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens Comp. |
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| <b>Lumber</b><br>Top chord: 2x4 SP #2;<br>Bot chord 2x4 SP #2; B5 2x4 SP M-31,<br>Webs: 2x4 SP #3, | <b>Maximum Reactions (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp.<br>A - B 419 - 1403 E - F 426 - 1646<br>B - C 420 - 1403 F - G 373 - 1811<br>C - D 503 - 2174 G - H 318 - 1827<br>D - E 462 - 1961 H - I 284 - 1704 |
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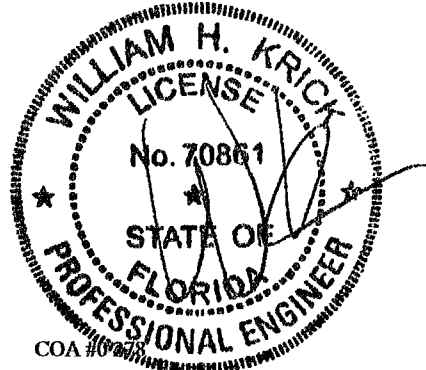
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| <b>Bracing</b><br>(a) Continuous lateral restraint equally spaced on member | <b>Maximum Bot Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp.<br>X - W 2248 - 503 P - O 1480 - 216<br>W - V 2246 - 504 O - M 1518 - 232<br>T - S 1481 - 137 M - L 1546 - 261<br>S - Q 1519 - 233 L - K 1542 - 257<br>Q - P 1480 - 216 |
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| <b>Plating Notes</b><br>All plates are 2X4 except as noted. | <b>Maximum Web Forces Per Ply (lbs)</b><br>Webs Tens.Comp. Webs Tens. Comp.<br>A - Y 540 - 1555 V - T 1881 - 327<br>A - X 1990 - 595 E - T 693 - 242<br>B - X 357 - 429 O - N 261 - 827<br>X - C 191 - 1198 I - K 276 - 1725<br>C - V 322 - 731 |
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| <b>Purlins</b><br>In lieu of structural panels use purlins to brace all flat TC @ 24" oc. | <b>Maximum Web Forces Per Ply (lbs)</b><br>Webs Tens.Comp. Webs Tens. Comp.<br>A - Y 540 - 1555 V - T 1881 - 327<br>A - X 1990 - 595 E - T 693 - 242<br>B - X 357 - 429 O - N 261 - 827<br>X - C 191 - 1198 I - K 276 - 1725<br>C - V 322 - 731 |
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|  |   |
|--|---|
| <b>Wind</b><br>Wind loads based on MWFRS with additional C&C member design.<br>End verticals not exposed to wind pressure.<br>Wind loading based on both gable and hip roof types. | <b>Maximum Web Forces Per Ply (lbs)</b><br>Webs Tens.Comp. Webs Tens. Comp.<br>A - Y 540 - 1555 V - T 1881 - 327<br>A - X 1990 - 595 E - T 693 - 242<br>B - X 357 - 429 O - N 261 - 827<br>X - C 191 - 1198 I - K 276 - 1725<br>C - V 322 - 731 |
|--|---|

**Additional Notes**  
The overall height of this truss excluding overhang is 11-0-0.  
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).



Florida Certificate of Product Approval #FL 1999  
03/03/2025

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**Hangers / Ties**

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Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

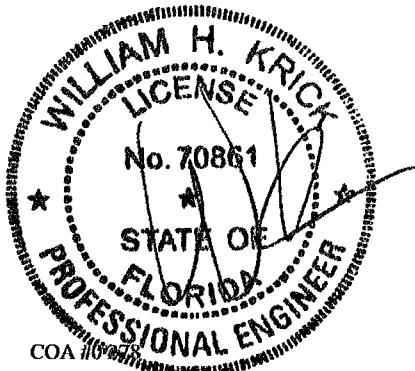
Bearing at location x=42'1" uses the following support conditions: 42'1"

Bearing K (42'1", 11'1"2) LUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(4) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member



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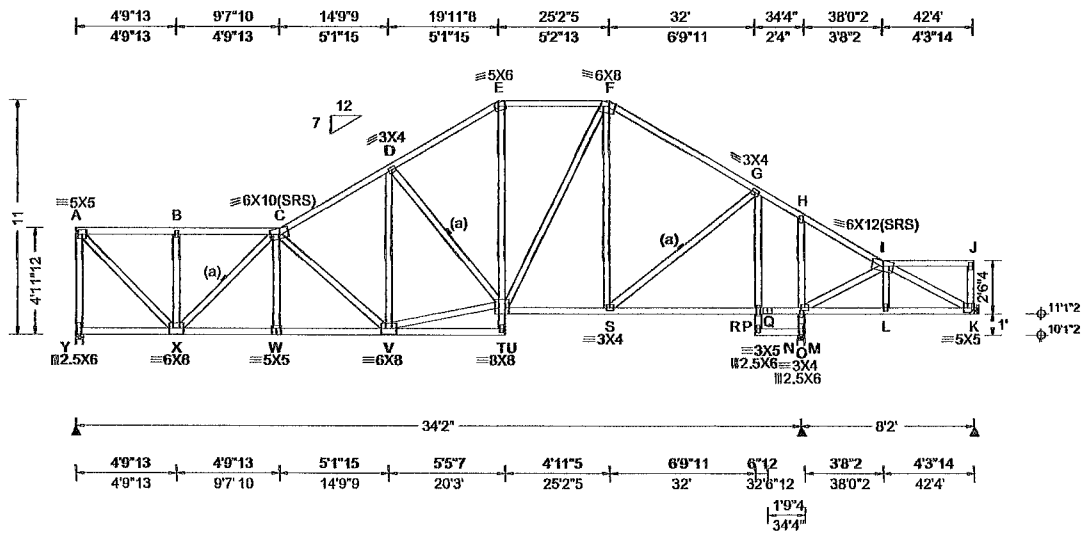
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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



|   |  |  |  |  |
|---|--|--|--|--|
| <b>Loading Criteria (psf)</b><br>TCLL 20 00<br>TCDL 10 00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40 00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing 24 0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C    Kzt: NA<br>Mean Height: 18.12 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist. h to 2h<br>C&C Dist a 4.23 ft<br>Loc. from endwall, not in 13 00 ft<br>GCpi: 0.18<br>Wind Duration: 1 60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg. NA    Ct: NA    CAT NA<br>Pf NA    Ce. NA<br>Lur NA    Cs. NA<br>Snow Duration: NA<br><br>Building Code<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT-20(0)/10(0)<br>Plate Type(s).<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL): 0.143 T 999 240<br>VERT(CL): 0.296 V 999 180<br>HORZ(LL): 0.123 N - -<br>HORZ(TL): 0.255 N - -<br>Creep Factor: 2.0<br>Max TC CSI 0.783<br>Max BC CSI 0.973<br>Max Web CSI 0.825<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity                      Non-Gravity<br>Loc R+ / R- / Rh            / Rw / U / RL<br>Y 1612 /- /- /850 /70 /205<br>N 801 /- /- /466 /11 /-<br>K 1109 /- /- /587 /- /-<br>Wind reactions based on MWFRS<br>Y Brg Wid = 4.0    Min Req = 1.9 (Truss)<br>N Brg Wid = 4.0    Min Req = 1.5 (Truss)<br>K Brg Wid = -      Min Req = -<br>Bearings Y & N are a rigid surface<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords    Tens.Comp.    Chords    Tens Comp. |
|---|--|--|--|--|

**Lumber**  
Top chord 2x4 SP #2;  
Bot chord 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
All plates are 2X4 except as noted

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 11-0-0.  
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).

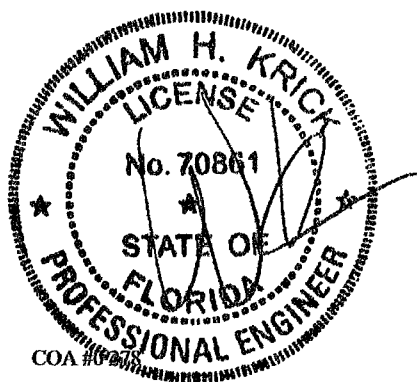
|       |            |       |            |
|-------|------------|-------|------------|
| A - B | 408 - 1491 | E - F | 402 - 1667 |
| B - C | 408 - 1491 | F - G | 352 - 1840 |
| C - D | 486 - 2329 | G - H | 308 - 1882 |
| D - E | 425 - 2006 | H - I | 247 - 1804 |

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords    Tens.Comp.    Chords    Tens Comp

|       |            |       |            |
|-------|------------|-------|------------|
| X - W | 2520 - 530 | P - O | 1534 - 158 |
| W - V | 2517 - 531 | O - M | 1568 - 173 |
| T - S | 1507 - 76  | M - L | 1792 - 273 |
| S - Q | 1570 - 174 | L - K | 1791 - 289 |
| Q - P | 1534 - 158 |       |            |

**Maximum Web Forces Per Ply (lbs)**  
Webs    Tens. Comp.    Webs    Tens. Comp.

|       |            |       |            |
|-------|------------|-------|------------|
| A - Y | 495 - 1572 | D - T | 299 - 461  |
| A - X | 2080 - 569 | E - T | 680 - 207  |
| X - C | 241 - 1434 | O - N | 246 - 796  |
| C - V | 324 - 769  | I - K | 294 - 1996 |
| V - T | 1975 - 296 |       |            |



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|              |      |        |                     |                          |                   |     |
|--------------|------|--------|---------------------|--------------------------|-------------------|-----|
| SEQN: 802283 | SPEC | Ply: 1 | Job Number: 25-2306 | Cust: R 215              | JRef: 1Y7W2150003 | T44 |
| FROM: CDM    |      | Qty: 1 | MOSS                | DrwNo: 062.25.0539.16353 |                   |     |
| Page 2 of 2  |      |        | Truss Label: G14    | KD / DF                  | 03/03/2025        |     |

**Hangers / Ties**

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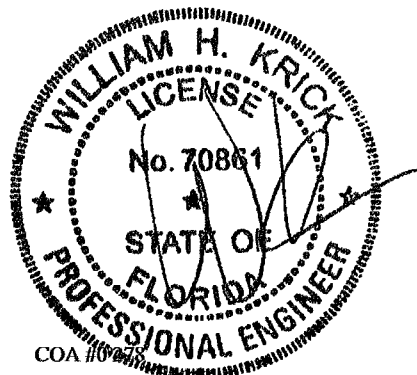
Bearing at location  $x=42'1"$  uses the following support conditions:  $42'1"$

Bearing K ( $42'1"$ ,  $11'1"2$ ) LUS26

Supporting Member: (2)  $2x6$  SP 2400F-2.0E

(4)  $0.148"x3"$  nails into supporting member,

(4)  $0.148"x3"$  nails into supported member.



COA # 08/03/2025

Florida Certificate of Product Approval #FL 1999

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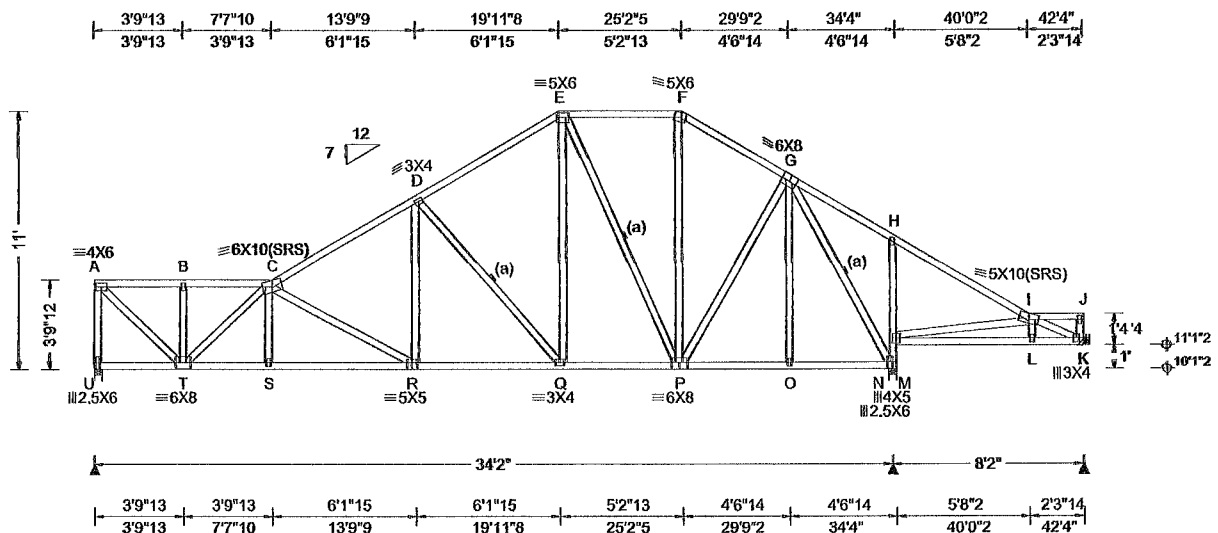
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155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025



| Loading Criteria (psf) | Wind Criteria                     | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria               | Maximum Reactions (lbs)                       |                                    |
|------------------------|-----------------------------------|------------------------------|---------------------------------|---|------------------------------------|
|                        |                                   |                              |                                 | Gravity                                       | Non-Gravity                        |
| TCLL. 20 00            | Wind Std: ASCE 7-22               | Pg NA    Ct: NA    CAT NA    | PP Deflection in loc L/defl L/# | Loc R+ / R-                                   | / Rh / Rl / U / RL                 |
| TCDL. 10 00            | Speed 130 mph                     | Pf NA    Cs: NA    Ce NA     | VERT(LL). 0.102 R 999 240       | U 1398 /- /-                                  | /759 /45 /237                      |
| BCLL. 0.00             | Enclosure: Closed                 | Lu: NA    Snow Duration: NA  | VERT(CL). 0.214 R 999 180       | N 1874 /- /-                                  | /1035 /- /-                        |
| BCDL. 10.00            | Risk Category: II                 | Building Code:               | HORZ(LL). 0.037 A - -           | K 274 /- /-                                   | /158 /22 /-                        |
| Des Ld: 40 00          | EXP- C    Kzt: NA                 | FBC 8th Ed 2023 Res.         | HORZ(TL) 0.076 A - -            | Wind reactions based on MWFRS                 |                                    |
| NCBCLL. 10 00          | Mean Height: 17.53 ft             | TPI Std. 2014                | Creep Factor: 2.0               | U Brg Wid = 4.0    Min Req = 1.6 (Truss)      |                                    |
| Soffit: 2.00           | TCDL 5.0 psf                      | Rep Fac: Yes                 | Max TC CSI 0.486                | N Brg Wid = 4.0    Min Req = 2.2 (Truss)      |                                    |
| Load Duration: 1.25    | BCDL 5.0 psf                      | FT/RT:20(0)/10(0)            | Max BC CSI 0.641                | K Brg Wid = -    Min Req = -                  |                                    |
| Spacing: 24.0"         | MWFRS Parallel Dist: h to 2h      | Plate Type(s).               | Max Web CSI: 0.828              | Bearings U & N are a rigid surface            |                                    |
|                        | C&C Dist a: 4.23 ft               | WAVE                         | VIEW Ver 23.02.04.0123.14       | Members not listed have forces less than 375# |                                    |
|                        | Loc. from endwall not in 13.00 ft |                              |                                 | Maximum Top Chord Forces Per Ply (lbs)        |                                    |
|                        | GCpi: 0.18                        |                              |                                 | Chords  | Tens.Comp.    Chords    Tens Comp. |
|                        | Wind Duration: 1.80               |                              |                                 | A-B   | 371 -1368    D-E    336 -1353      |

| Lumber   | Maximum Bot Chord Forces Per Ply (lbs)       |
|--|--|
| Top chord 2x4 SP #2,<br>Bot chord 2x4 SP #2,<br>Webs: 2x4 SP #3, | Chords    Tens Comp.    Chords    Tens Comp. |
|  | T-S    2389 -513    P-O    606    0          |
|  | S-R    2385 -515    O-N    605    0          |
|  | R-Q    1636 -218    M-L    426    -130       |
|  | Q-P    1078    0    L-K    437    -120       |

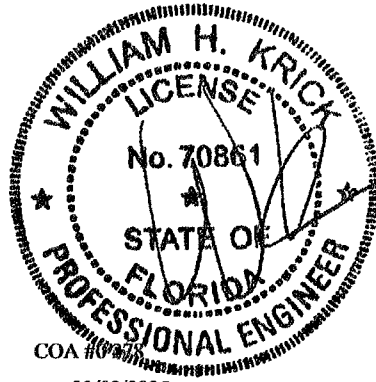
| Bracing   | Maximum Web Forces Per Ply (lbs)          |
|---|---|
| (a) Continuous lateral restraint equally spaced on member | Webs    Tens.Comp.    Webs    Tens. Comp. |
|   | A-U    423 -1361    E-P    110    -543    |
|   | A-T    1853 -501    P-G    485    0       |
|   | T-C    262 -1387    G-N    80    -1489    |
|   | C-R    341 -848    N-M    260    -537     |
|   | R-D    587 -107    M-H    254    -391     |
|   | D-Q    347 -859    M-I    170    -506     |
|   | E-Q    767 -205    I-K    127    -475     |

**Plating Notes**  
All plates are 2X4 except as noted.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 11-0-0.



COA #09315  
03/03/2025  
Florida Certificate of Product Approval #FL 1999

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|              |      |        |                     |                                  |
|--------------|------|--------|---------------------|----------------------------------|
| SEQN: 802290 | SPEC | Ply: 1 | Job Number: 25-2306 | Cust: R 215 JRef 1Y7W2150003 T29 |
| FROM: CDM    |      | Qty: 1 | MOSS                | DrwNo: 062.25.0539.23567         |
| Page 2 of 2  |      |        | Truss Label: G15    | KD / DF 03/03/2025               |

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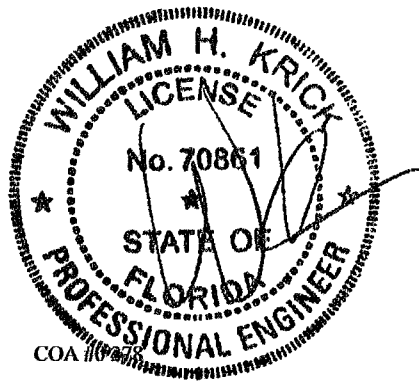
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Bearing K (42'1", 11'1"2) LUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(4) 0.148"x3" nails into supporting member,

(3) 0.148"x3" nails into supported member



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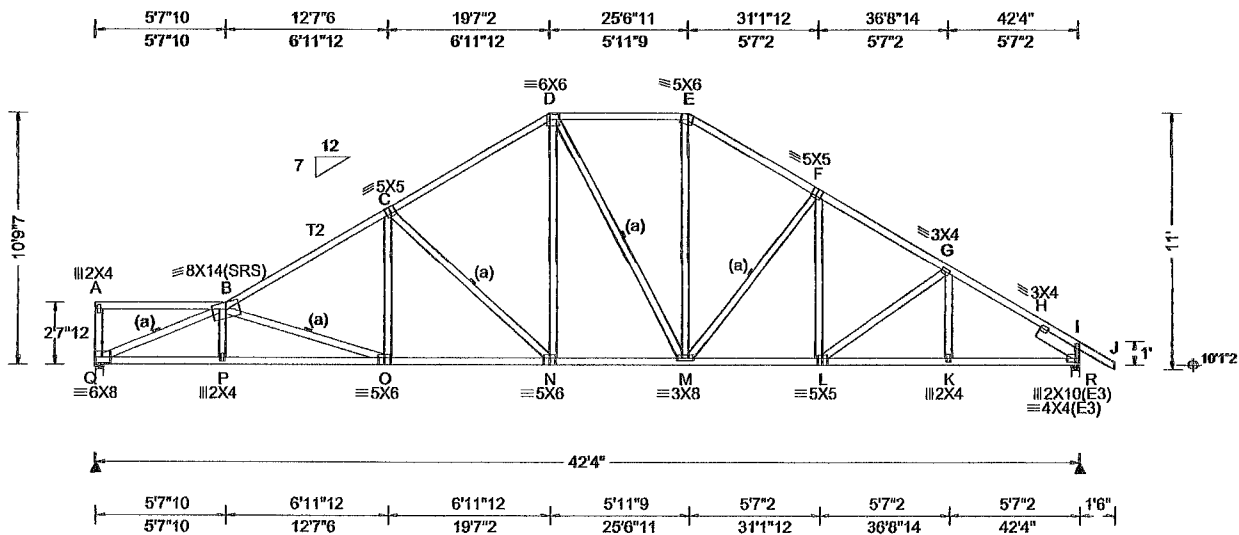
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155 Harlem Ave  
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| Loading Criteria (psf)   | Wind Criteria  | Snow Criteria (Pg,Pf in PSF)  | Defl/CSI Criteria   | Maximum Reactions (lbs)   |
|--|--|---|---|---|
| TCLL 20.00<br>TCDL 10.00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40.00<br>NCBCLL 10.00<br>Soffit 2.00<br>Load Duration: 1.25<br>Spacing 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP-C Kzt: NA<br>Mean Height: 16.42 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: > 2h<br>C&C Dist: 4.23 ft<br>Loc. from endwall: not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg NA Ct: NA CAT NA<br>Pf NA Ce NA<br>Lr NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT-20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL): 0.222 O 999 240<br>VERT(CL): 0.458 O 999 180<br>HORZ(LL): 0.130 H - -<br>HORZ(TL): 0.269 H - -<br>Creep Factor 2.0<br>Max TC CSI 0.949<br>Max BC CSI 0.955<br>Max Web CSI 0.946<br><br>VIEW Ver 23.02.04.0123.14 | Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>Q 1757 - / - / - / 971 - / 1284<br>R 1862 - / - / - / 1113 - / -<br>Non-Gravity<br>Q Brg Wid = 4.0 Min Req = 2.1 (Truss)<br>R Brg Wid = 4.0 Min Req = 2.2 (Truss)<br>Bearings Q & R are a rigid surface.<br>Members not listed have forces less than 375#<br>Maximum Top Chord Forces Per Ply (lbs)<br>Chords Tens.Comp. Chords Tens Comp<br>B - C 475 - 2982 F - G 383 - 2426<br>C - D 392 - 2188 G - H 364 - 2616<br>D - E 373 - 1715 H - I 387 - 2895<br>E - F 375 - 2070 |

**Lumber**  
Top chord 2x4 SP #2, T2 2x4 SP M-31,  
Bot chord 2x4 SP #2,  
Webs: 2x4 SP #3;  
Rt Slider: 2x6 SP #2, block length = 2.093'

**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
Left end vertical not exposed to wind pressure  
Wind loading based on both gable and hip roof types.

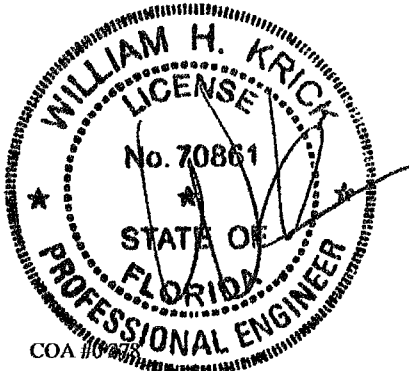
**Additional Notes**  
The overall height of this truss excluding overhang is 10-9-7

**Maximum Bot Chord Forces Per Ply (lbs)**

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| Q - P  | 3672 - 560 | M - L  | 2029 - 107  |
| P - O  | 3665 - 565 | L - K  | 2138 - 205  |
| O - N  | 2481 - 228 | K - I  | 2141 - 205  |
| N - M  | 1786 0     |        |             |

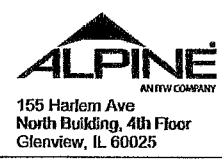
**Maximum Web Forces Per Ply (lbs)**

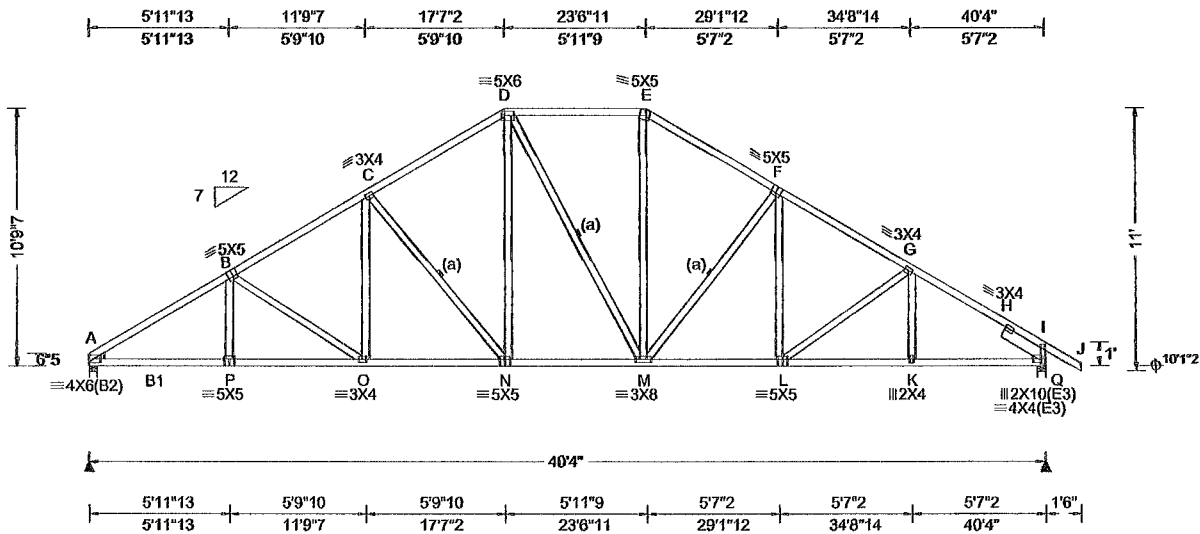
| Webs  | Tens.Comp. | Webs  | Tens. Comp. |
|-------|------------|-------|-------------|
| Q - B | 688 - 3945 | D - N | 775 - 171   |
| B - O | 355 - 1232 | M - E | 654 - 162   |
| O - C | 614 - 45   | M - F | 232 - 523   |
| C - N | 346 - 951  |       |             |



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|   |   |  |  |   |
|---|---|--|--|---|
| <b>Loading Criteria (psf)</b><br>TCLL 20 00<br>TCDL 10 00<br>BCLL 0.00<br>BCDL 10.00<br>Des Ld 40 00<br>NCBCLL 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category II<br>EXP-C Kzt: NA<br>Mean Height: 16.42 ft<br>TCDL 5.0 psf<br>BCDL 5.0 psf<br>MWFRS Parallel Dist: > 2h<br>C&C Dist a 4.03 ft<br>Loc. from endwall: not in 13.00 ft<br>GCp: 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg, Pf in PSF)</b><br>Pg: NA Ct: NA CAT NA<br>Pf: NA Ce: NA<br>Lur: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT 20(0)/10(0)<br>Plate Type(s):<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL) 0.155 N 999 240<br>VERT(CL) 0.319 N 999 180<br>HORZ(LL) 0.107 H - -<br>HORZ(TL) 0.221 H - -<br>Creep Factor: 2.0<br>Max TC CSI 0.882<br>Max BC CSI 0.791<br>Max Web CSI 0.320<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>A 1679 /- /- /1024 /- /309<br>Q 1777 /- /- /1119 /- /-<br>Wind reactions based on MWFRS<br>A Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Q Brg Wid = 4.0 Min Req = 2.1 (Truss)<br>Bearings A & Q are a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp<br>A - B 347 -2765 E - F 289 -1911<br>B - C 331 -2397 F - G 290 -2274<br>C - D 305 -1942 G - H 282 -2475<br>D - E 291 -1578 H - I 301 -2552 |
|---|---|--|--|---|

**Lumber**  
Top chord 2x4 SP #2,  
Bot chord 2x4 SP #2, B1 2x4 SP M-31,  
Webs 2x4 SP #3;  
Rt Slider 2x6 SP #2; block length = 2.093'  
Lt Wedge: 2x4 SP #3,

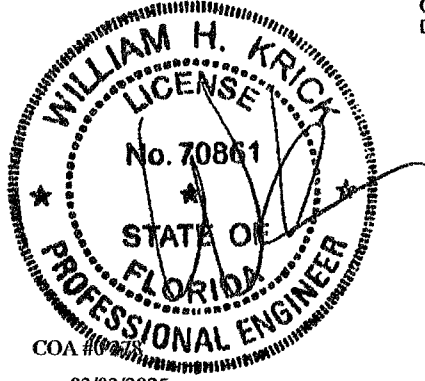
**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 10-9-7.

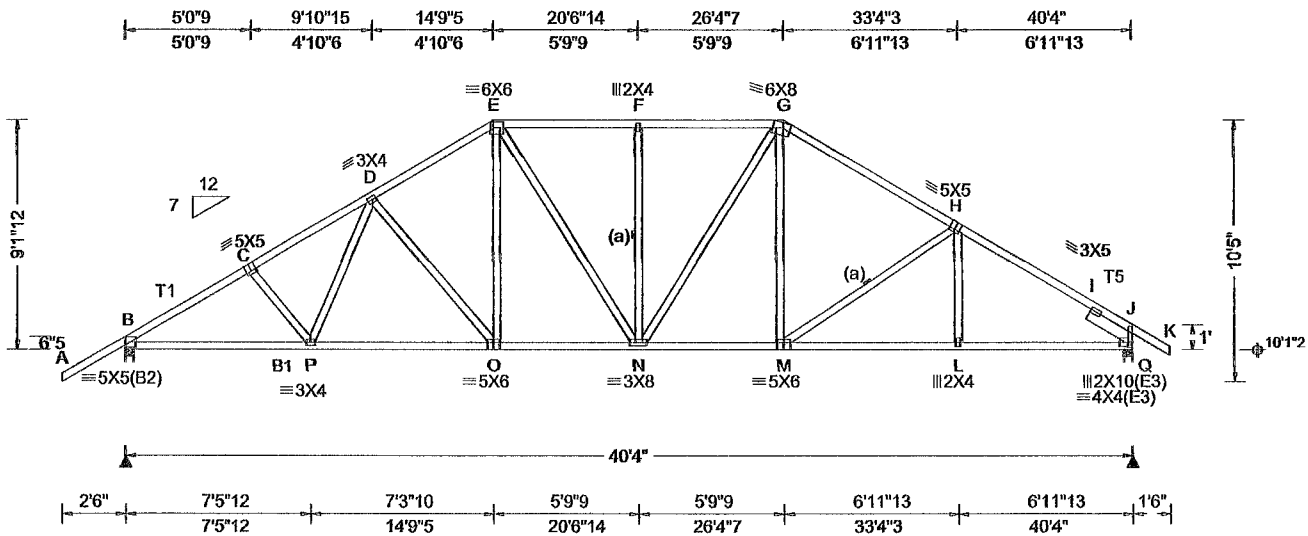
|   |            |        |             |
|---|------------|--------|-------------|
| <b>Maximum Bot Chord Forces Per Ply (lbs)</b> |            |        |             |
| Chords  | Tens.Comp. | Chords | Tens. Comp. |
| A - P   | 2292 -199  | M - L  | 1895 -25    |
| P - O   | 2291 -200  | L - K  | 2021 -134   |
| O - N   | 1985 -48   | K - I  | 2024 -134   |
| N - M   | 1595 0     |        |             |
| <b>Maximum Web Forces Per Ply (lbs)</b>       |            |        |             |
| Webs  | Tens.Comp. | Webs   | Tens. Comp. |
| O - C   | 395 -44    | M - E  | 572 -130    |
| C - N   | 247 -623   | M - F  | 231 -528    |
| D - N   | 598 -127   |        |             |



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| Loading Criteria (psf)  | Wind Criteria   | Snow Criteria (Pg,Pf in PSF)   | Defl/CSI Criteria  | Maximum Reactions (lbs)   |
|---|---|--|--|---|
| TCLL. 20.00<br>TCDL. 10.00<br>BCLL. 0.00<br>BCDL. 10.00<br>Des Ld 40.00<br>NCBCLL. 10.00<br>Soffit. 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | Wind Std ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP- C Kzt: NA<br>Mean Height: 15.60 ft<br>TCDL. 5.0 psf<br>BCDL. 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist a 4.03 ft<br>Loc. from endwall, not in 13.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | Pg. NA Ct: NA CAT. NA<br>Pf NA Ce NA<br>Lu: NA Cs. NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std: 2014<br>Rep Fac: Yes<br>FT/RT-20(0)/10(0)<br>Plate Type(s):<br>WAVE | PP Deflection in loc L/defl L/#<br>VERT(LL). 0.188 F 999 240<br>VERT(CL). 0.353 F 999 180<br>HORZ(LL). 0.127 I - -<br>HORZ(TL). 0.239 I - -<br>Creep Factor: 2.0<br>Max TC CSI 0.783<br>Max BC CSI 0.771<br>Max Web CSI 0.560<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>B 2044 /- /- /1142 /- /295<br>Q 1917 /- /- /1066 /- /-<br>Wind reactions based on MWFRS<br>B Brg Wid = 4.0 Min Req = 17 (Truss)<br>Q Brg Wid = 4.0 Min Req = 23 (Truss)<br>Bearings B & Q are a rigid surface.<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens.Comp. Chords Tens Comp<br>B - C 152 -3076 F - G 102 -2186<br>C - D 160 -2901 G - H 118 -2396<br>D - E 136 -2452 H - I 135 -2743<br>E - F 102 -2186 I - J 147 -2823 |

**Lumber**  
Top chord 2x4 SP #2; T1, T5 2x4 SP M-31,  
Bot chord 2x4 SP #2; B1 2x4 SP M-31,  
Webs. 2x4 SP #3,  
Rt Slider: 2x6 SP #2, block length = 2 093'

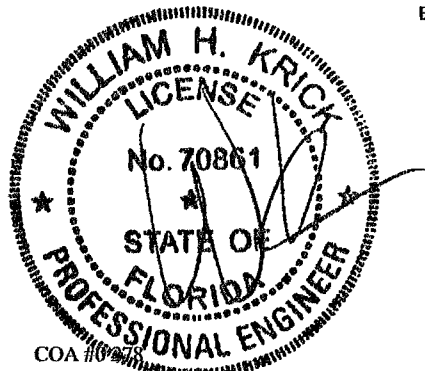
**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 9'-1.12.



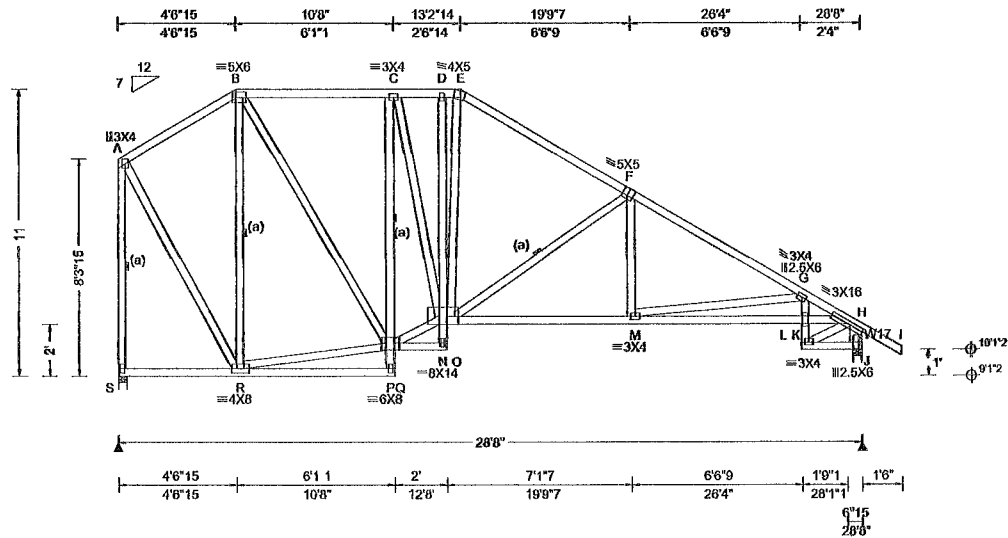
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|  |  |   |  |   |
|--|--|---|--|---|
| <b>Loading Criteria (psf)</b><br>TCLL. 20.00<br>TCDL. 10.00<br>BCLL. 0.00<br>BCDL. 10.00<br>Des Ld 40.00<br>NCBCLL. 10.00<br>Soffit: 2.00<br>Load Duration: 1.25<br>Spacing: 24.0" | <b>Wind Criteria</b><br>Wind Std. ASCE 7-22<br>Speed 130 mph<br>Enclosure: Closed<br>Risk Category: II<br>EXP: C Kzt: NA<br>Mean Height: 15.96 ft<br>TCDL. 5.0 psf<br>BCDL. 5.0 psf<br>MWFRS Parallel Dist: h to 2h<br>C&C Dist: 3.00 ft<br>Loc. from endwall, not in 9.00 ft<br>GCpi: 0.18<br>Wind Duration: 1.60 | <b>Snow Criteria (Pg,Pf in PSF)</b><br>Pg: NA Ct: NA CAT: NA<br>Pf: NA Ce: NA<br>Lu: NA Cs: NA<br>Snow Duration: NA<br><br>Building Code:<br>FBC 8th Ed 2023 Res.<br>TPI Std. 2014<br>Rep Fac: Yes<br>FT/RT.20(0)/10(0)<br>Plate Type(s).<br>WAVE | <b>Defl/CSI Criteria</b><br>PP Deflection in loc L/defl L/#<br>VERT(LL) 0.095 M 999 240<br>VERT(CL) 0.195 M 999 180<br>HORZ(LL) 0.078 L - -<br>HORZ(TL) 0.161 L - -<br>Creep Factor: 2.0<br>Max TC CSI 0.561<br>Max BC CSI 0.832<br>Max Web CSI 0.767<br><br>VIEW Ver 23.02.04.0123.14 | <b>Maximum Reactions (lbs)</b><br>Gravity Non-Gravity<br>Loc R+ / R- / Rh / Rw / U / RL<br>S 1189 /- /- /651 /- /252<br>J 1296 /- /- /809 /- /-<br>Wind reactions based on MWFRS<br>S Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>J Brg Wid = 4.0 Min Req = 1.5 (Truss)<br>Bearings S & J are a rigid surface<br>Members not listed have forces less than 375#<br><b>Maximum Top Chord Forces Per Ply (lbs)</b><br>Chords Tens Comp. Chords Tens Comp.<br>A - B 71 -586 E - F 64 -1235<br>B - C 97 -841 F - G 99 -1916<br>C - D 101 -977 G - H 195 -2880<br>D - E 102 -983 |
|--|--|---|--|---|

**Lumber**  
Top chord 2x4 SP #2,  
Bot chord 2x4 SP #2,  
Webs. 2x4 SP #3; W17 2x6 SP #2,

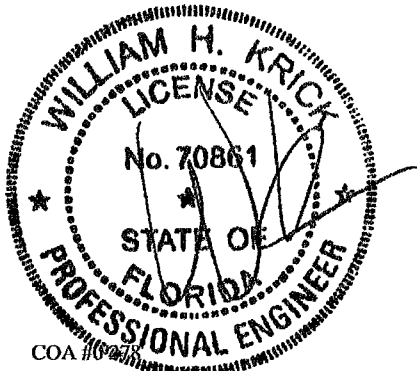
**Bracing**  
(a) Continuous lateral restraint equally spaced on member

**Plating Notes**  
All plates are 2X4 except as noted

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design  
End verticals not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
The overall height of this truss excluding overhang is 11-0-0.  
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).



COA #032085  
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