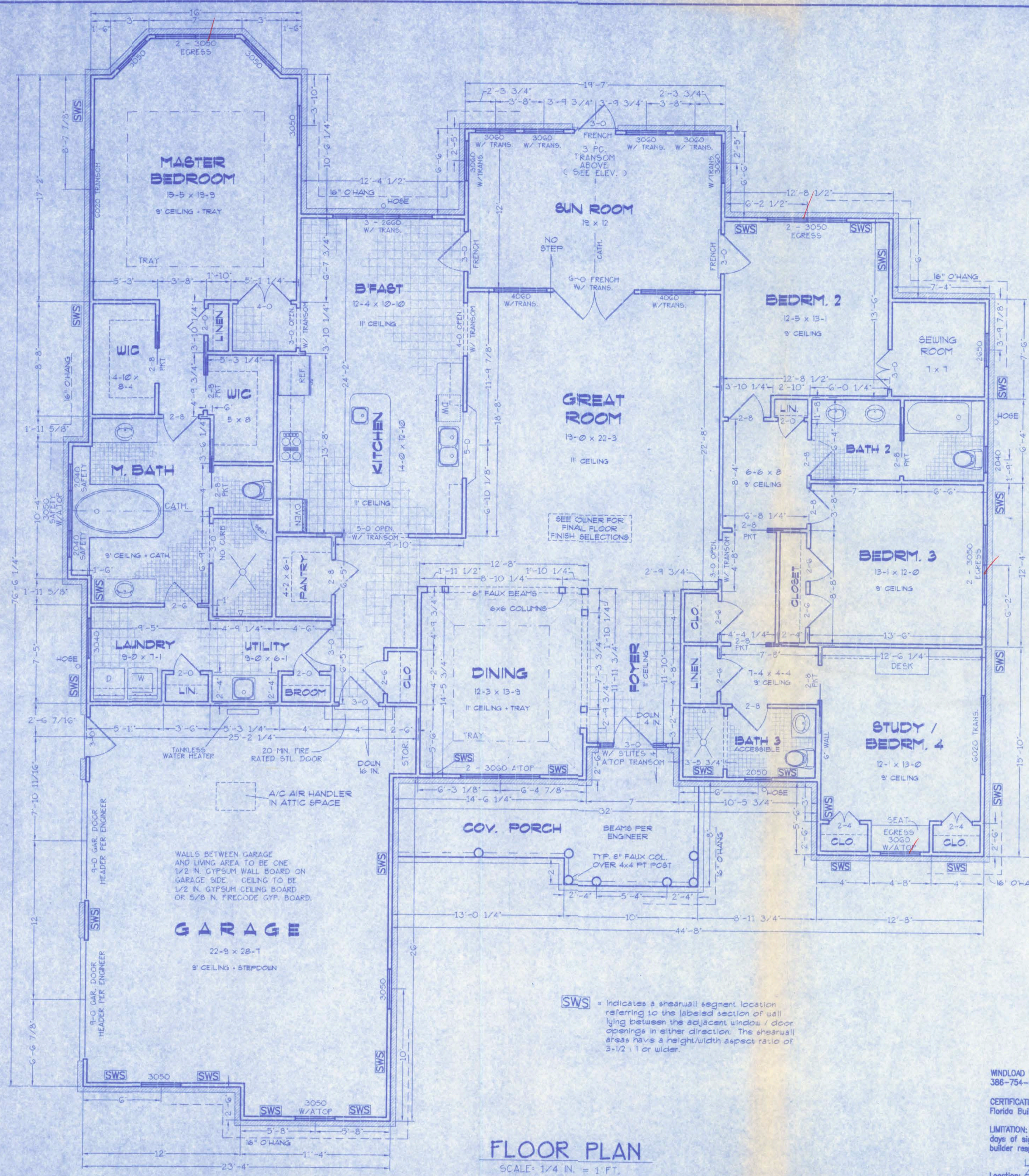
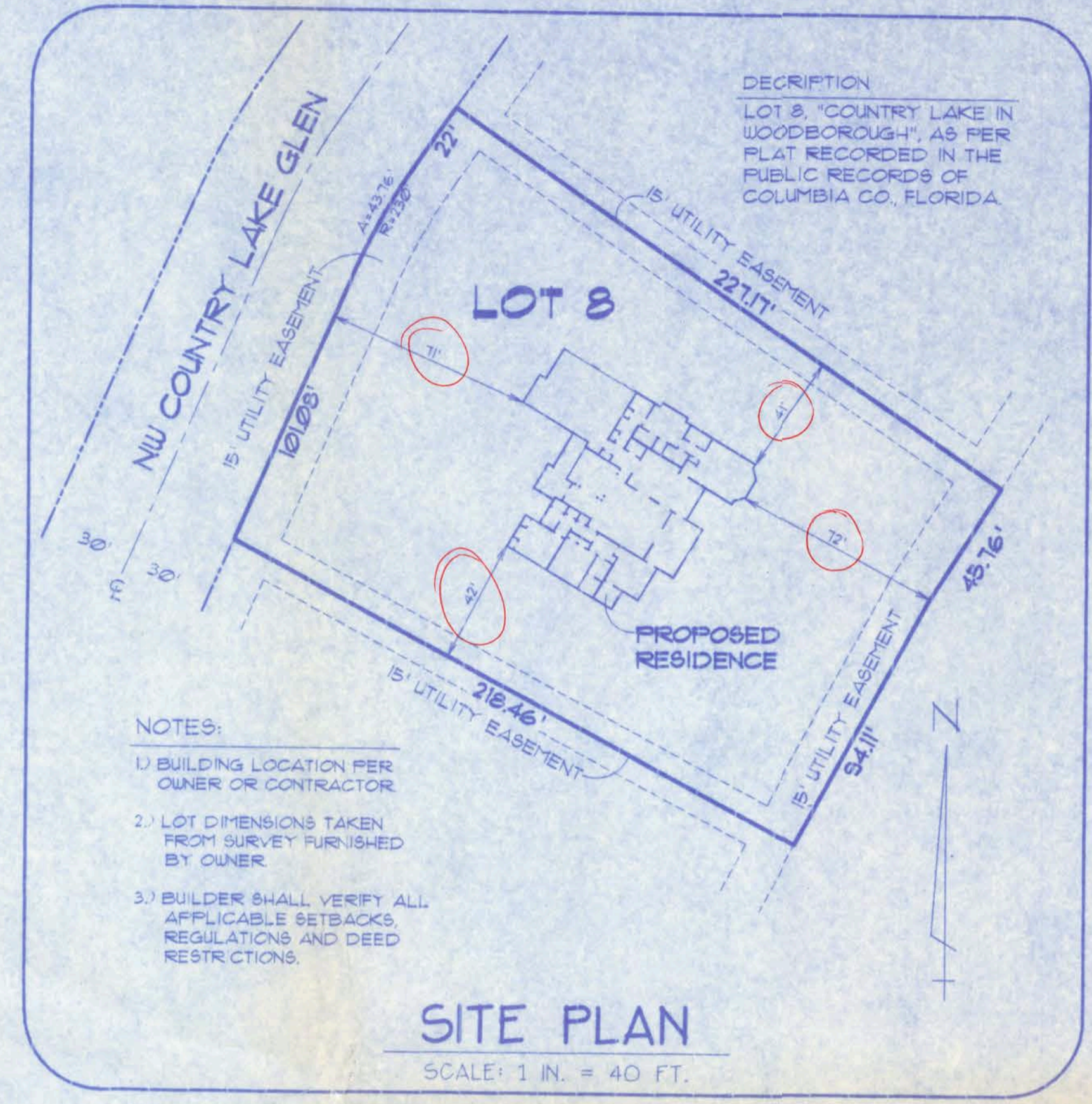


McNeill Residence



FLOOR PLAN
SCALE: 1/4 IN. = 1 FT.



- NOTES:
1. BUILDING LOCATION PER OWNER OR CONTRACTOR.
 2. LOT DIMENSIONS TAKEN FROM SURVEY FURNISHED BY OWNER.
 3. BUILDER SHALL VERIFY ALL APPLICABLE SETBACKS, REGULATIONS AND DEED RESTRICTIONS.

SITE PLAN
SCALE: 1 IN. = 40 FT.

AREA SUMMARY

CONDITIONED	2874 SF
GARAGE	710 SF
PORCHES	418 SF
TOTAL ROOF	4002 SF

Index to Sheets

SHEET A-1	SITE PLAN + FLOOR PLAN
SHEET A-2	ELEVATIONS + GEN. NOTES
SHEET A-3	ELEVATIONS
SHEET A-4	FOUNDATION + SECTIONS
SHEET A-5	ELECTRICAL
SHEET S-1	WIND ENGINEERING

A-1

WINDLOAD ENGINEER: Mark Diesoway, PE No.53915, POB 888, Lake City, FL 32056, 386-754-5419

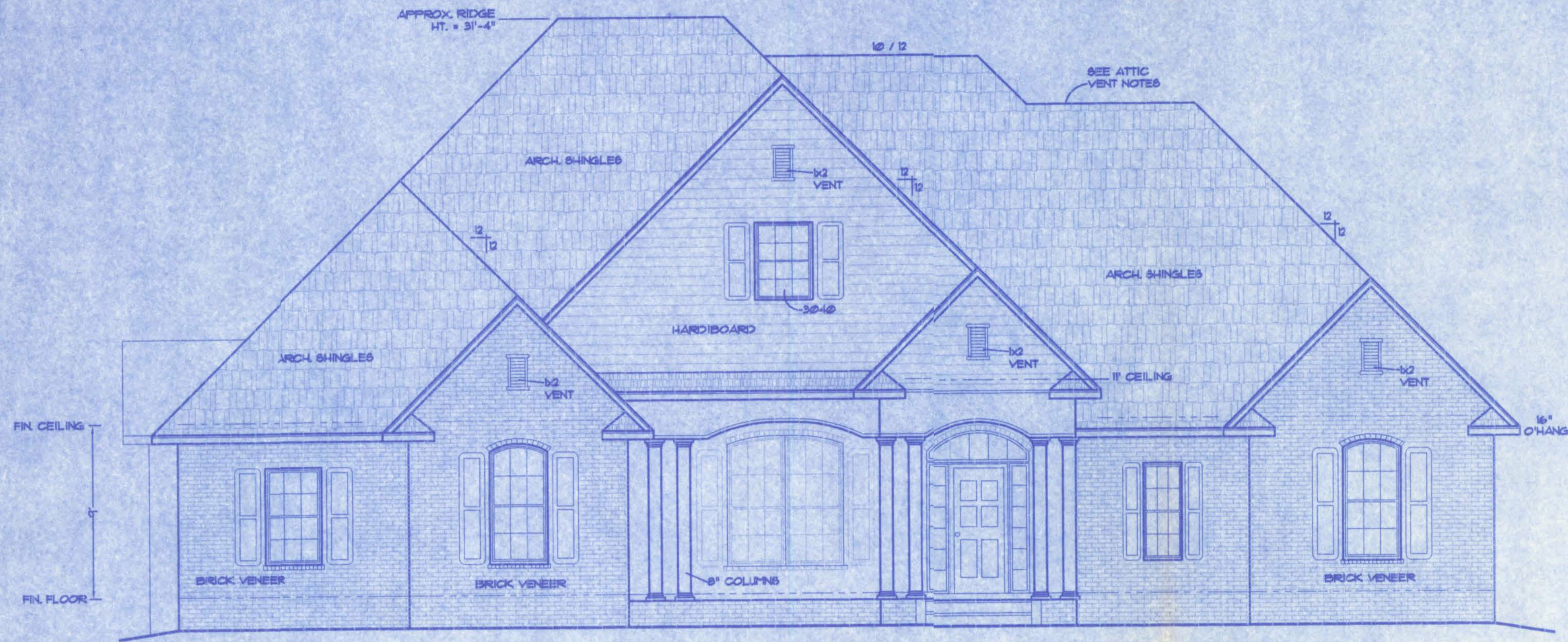
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 8 - COUNTRY LAKES
Location: AT WOODBOROUGH Job No: 709201

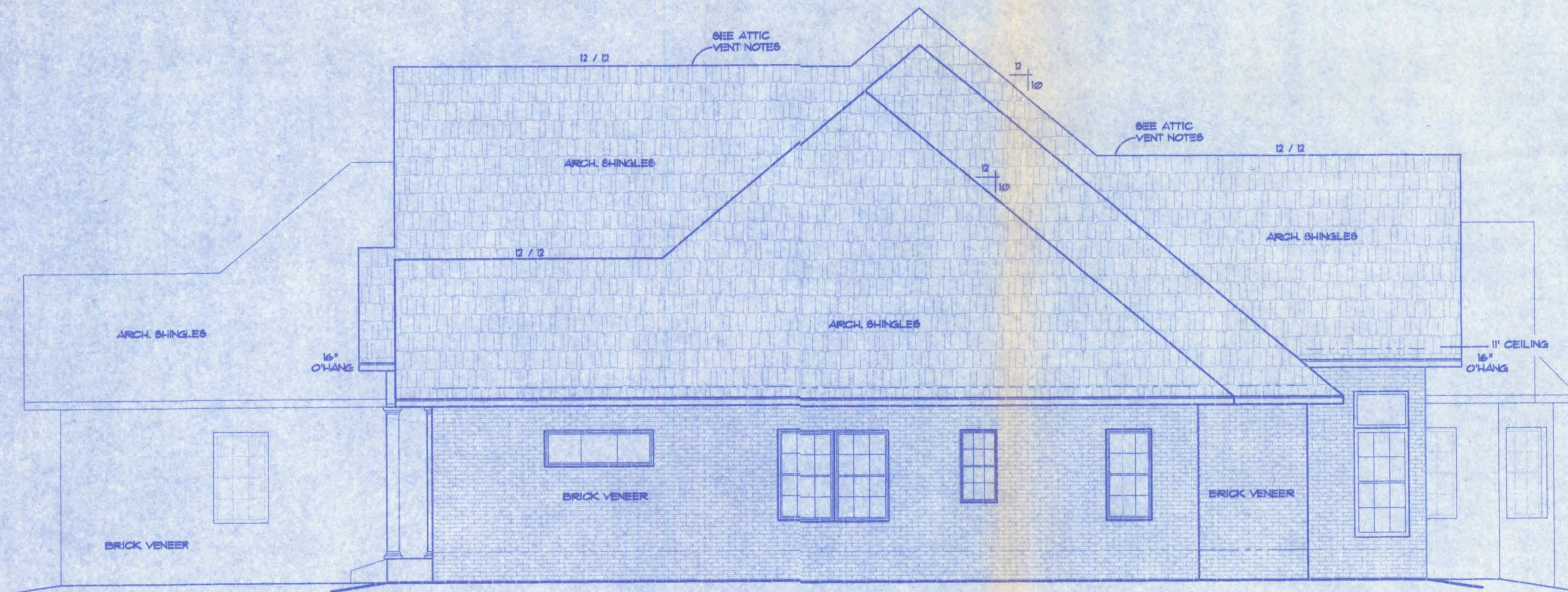
Mark Diesoway
27sep07

FILE: 07-014	MCNEILL RESIDENCE	SHEET: 1 of 5
DATE: 8-28-07		CAD FILE: 07014
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sugarwood Cir., Lake City, FL 32024 Phone: (386) 755-9391	REV:



FRONT ELEVATION

SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.

GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

A-2

WINDLOAD ENGINEER: Mark Diosoay, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

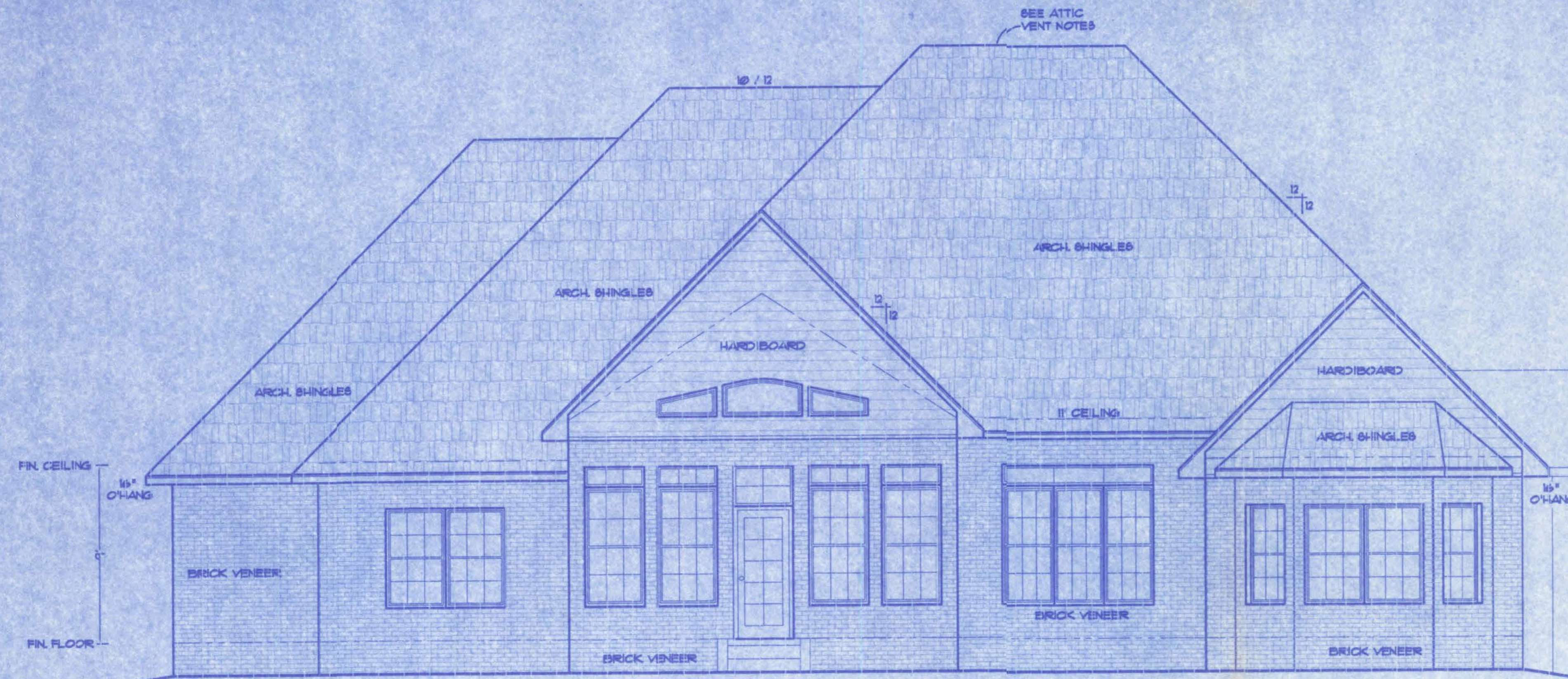
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 8 - COUNTRY LAKES
Location: AT WOODBOROUGH Job No. 709207

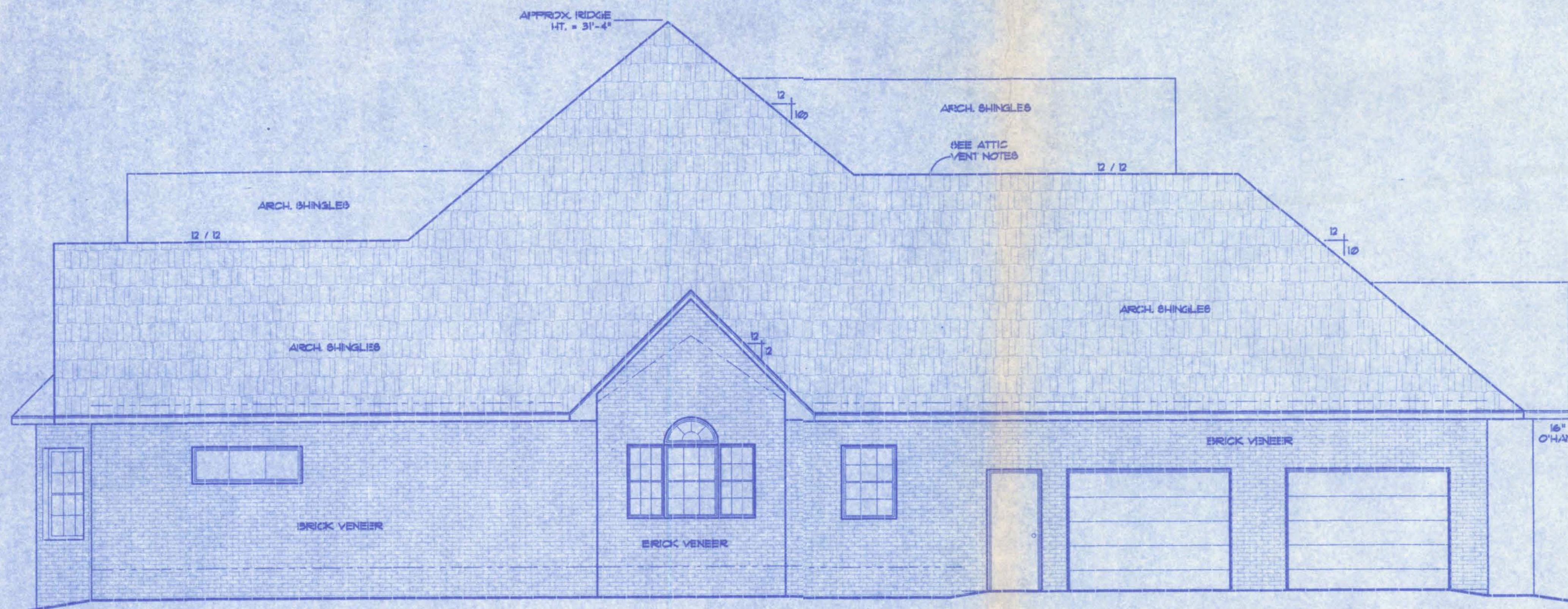
Handwritten signature and date:
27 Sep 07

FILE: 07-014	McNEILL RESIDENCE	SHEET: 2 of 5
DATE: 8-28-07		CAD FILE: 07D14
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Cir., Lake City, FL 32024 Phone (336) 758-5891	REV:



REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



LEFT ELEVATION

SCALE: 1/4 IN. = 1 FT.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation or such separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 3/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

A-3

WINDLOAD ENGINEER: Mark Dinsway, PE No.53915, POB 808, Lake City, FL 32056, 386-754-5419

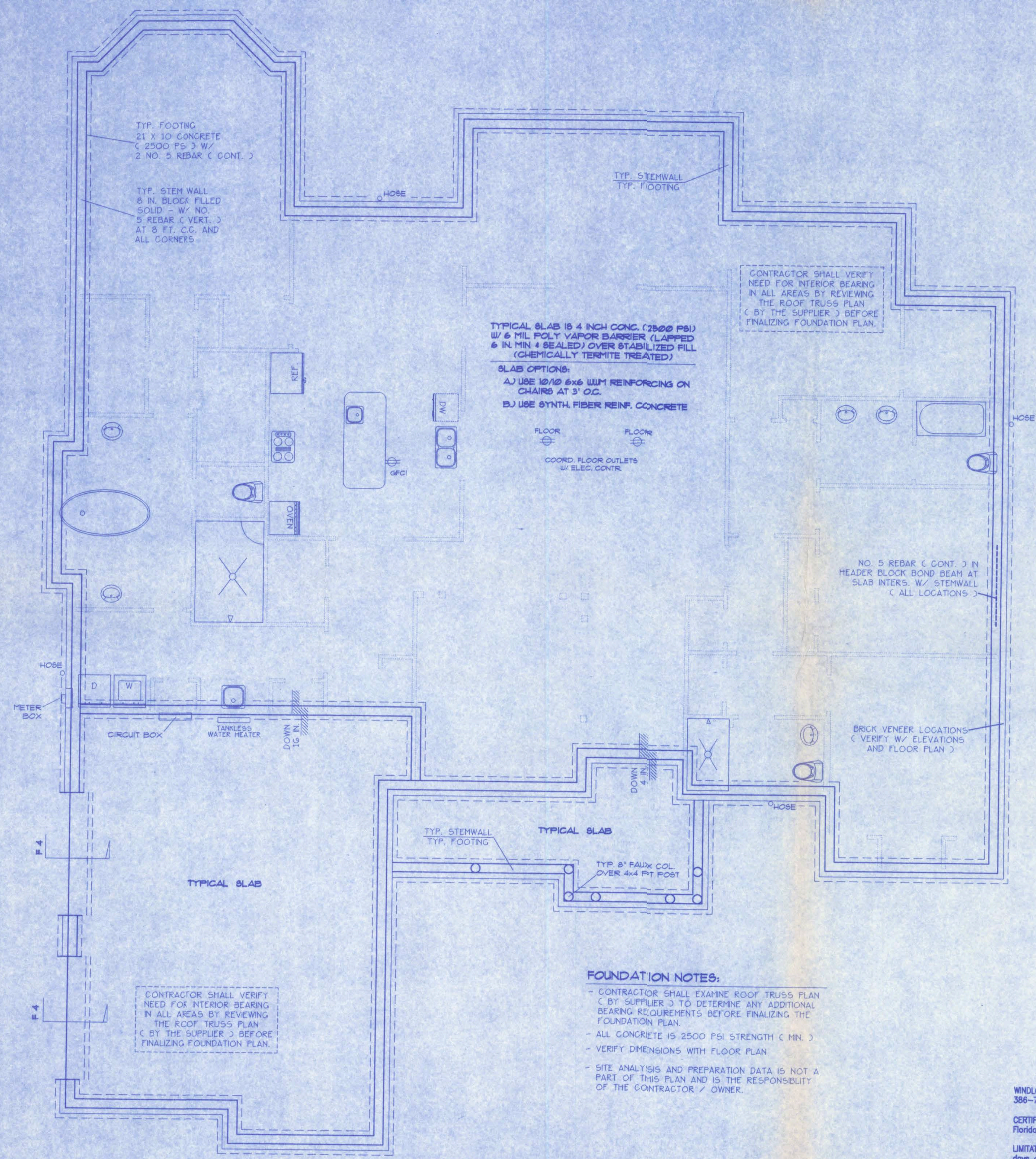
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 8 - COUNTRY LAKES
Location: AT WOODBOROUGH Job No.: 709201

FILE: 07-014	McNEILL RESIDENCE	SHEET: 3 of 5
DATE: 8-28-07		CAD FILE: 07014
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Englewood Cir, Lake City, FL 32024 Phone (352) 755-5891	REV:

Handwritten signature and date:
27sep07



TYP. FOOTING
21" X 10" CONCRETE
(2500 PSI) W/
2 NO. 5 REBAR (CONT.)

TYP. STEM WALL
8 IN. BLOCK FILLED
SOLID - W/ NO.
5 REBAR (VERT.)
AT 8 FT. C.C. AND
ALL CORNERS

TYP. STEM WALL
TYP. FOOTING

TYPICAL SLAB IS 4 INCH CONC. (2500 PSI)
W/ 6 MIL POLY VAPOR BARRIER (LAPPED
6 IN. MIN. & SEALED) OVER STABILIZED FILL
(CHEMICALLY TERMITE TREATED)

SLAB OPTIONS:
A) USE 10/10 6x6 W/M REINFORCING ON
CHAIRS AT 3' O.C.
B) USE SYNTH. FIBER REINF. CONCRETE

FLOOR FLOOR
COORD. FLOOR OUTLETS
W/ ELEC. CONTR.

CONTRACTOR SHALL VERIFY
NEED FOR INTERIOR BEARING
IN ALL AREAS BY REVIEWING
THE ROOF TRUSS PLAN
(BY THE SUPPLIER) BEFORE
FINALIZING FOUNDATION PLAN.

NO. 5 REBAR (CONT.) IN
HEADER BLOCK BOND BEAM AT
SLAB INTERS. W/ STEMWALL
(ALL LOCATIONS)

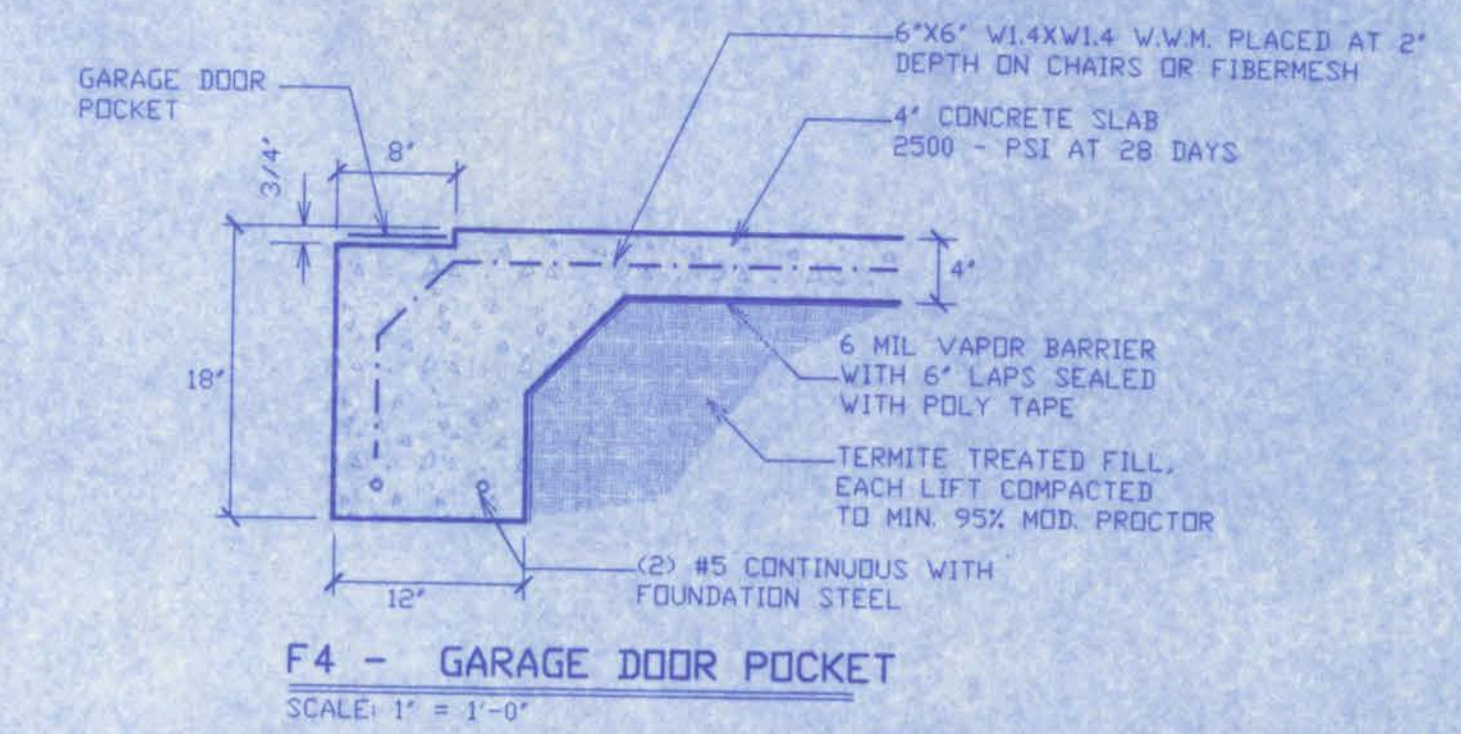
BRICK VENEER LOCATIONS
(VERIFY W/ ELEVATIONS
AND FLOOR PLAN)

CONTRACTOR SHALL VERIFY
NEED FOR INTERIOR BEARING
IN ALL AREAS BY REVIEWING
THE ROOF TRUSS PLAN
(BY THE SUPPLIER) BEFORE
FINALIZING FOUNDATION PLAN.

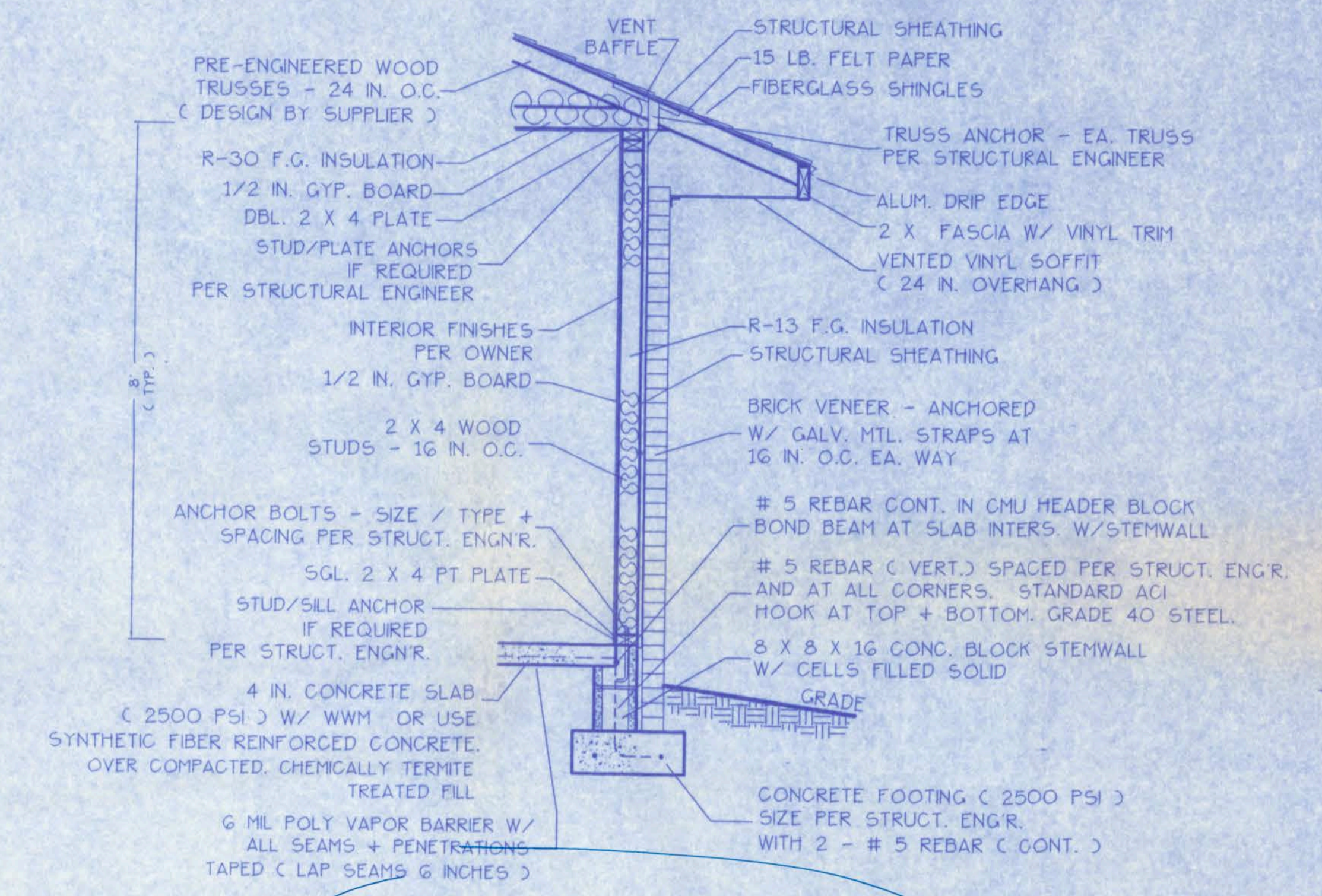
FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

FOUNDATION PLAN
SCALE: 1/4 IN. = 1 FT.



F4 - GARAGE DOOR POCKET
SCALE: 1" = 1'-0"



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

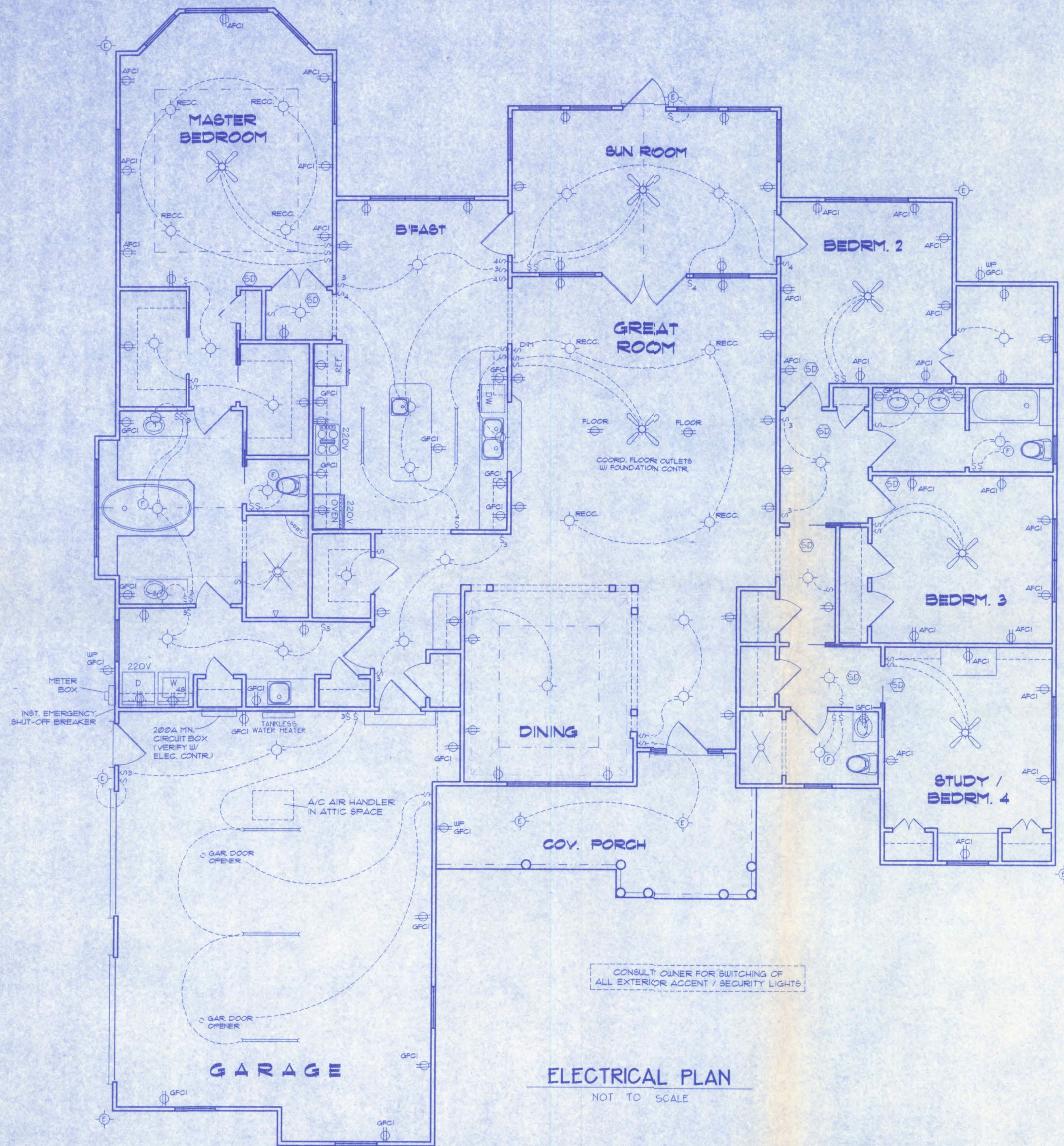
DESIGN WALL SECTION
NON-STRUCTURAL DATA
SCALE: 1/2 IN. = 1 FT.

A-4

WINDLOAD ENGINEER: Mark Doseway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419
 CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.
 LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.
 LOT 8 - COUNTRY LAKES
 Location: AT WOODBOROUGH Job No: 709206

FILE: 07-014	McNEILL RESIDENCE	SHEET: 4 of 5
DATE: 8-28-07		CAD FILE: 07014
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW. Sapwood Gln. Lake City, FL 32024 Phone (386) 755-5891	REV:

709206
Mark Doseway
275e002



ELECTRICAL PLAN
NOT TO SCALE

ELECTRICAL SYMBOL LEGEND	
	= FLOURESCENT LIGHTING FIXTURE.
	= CEILING LIGHT FIXTURE.
	= EXTERIOR LIGHTING FIXTURE.
	= LIGHT SWITCH.
	= THREE-WAY SWITCH.
	= 110 V. DUPLEX OUTLET.
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET.
	= GROUND FAULT CIRC. OUTLET.
	= AFCI FAULT CIRC. OUTLET.
	= 110 V. SINGLE RECEPTACLE OUTLET.
	= 220 VOLT OUTLET (4 WIRE)
	= FAN LOCATION (CEILING)
	= FAN LOCATION (EXHAUST)
	= SMOKE DETECTOR.

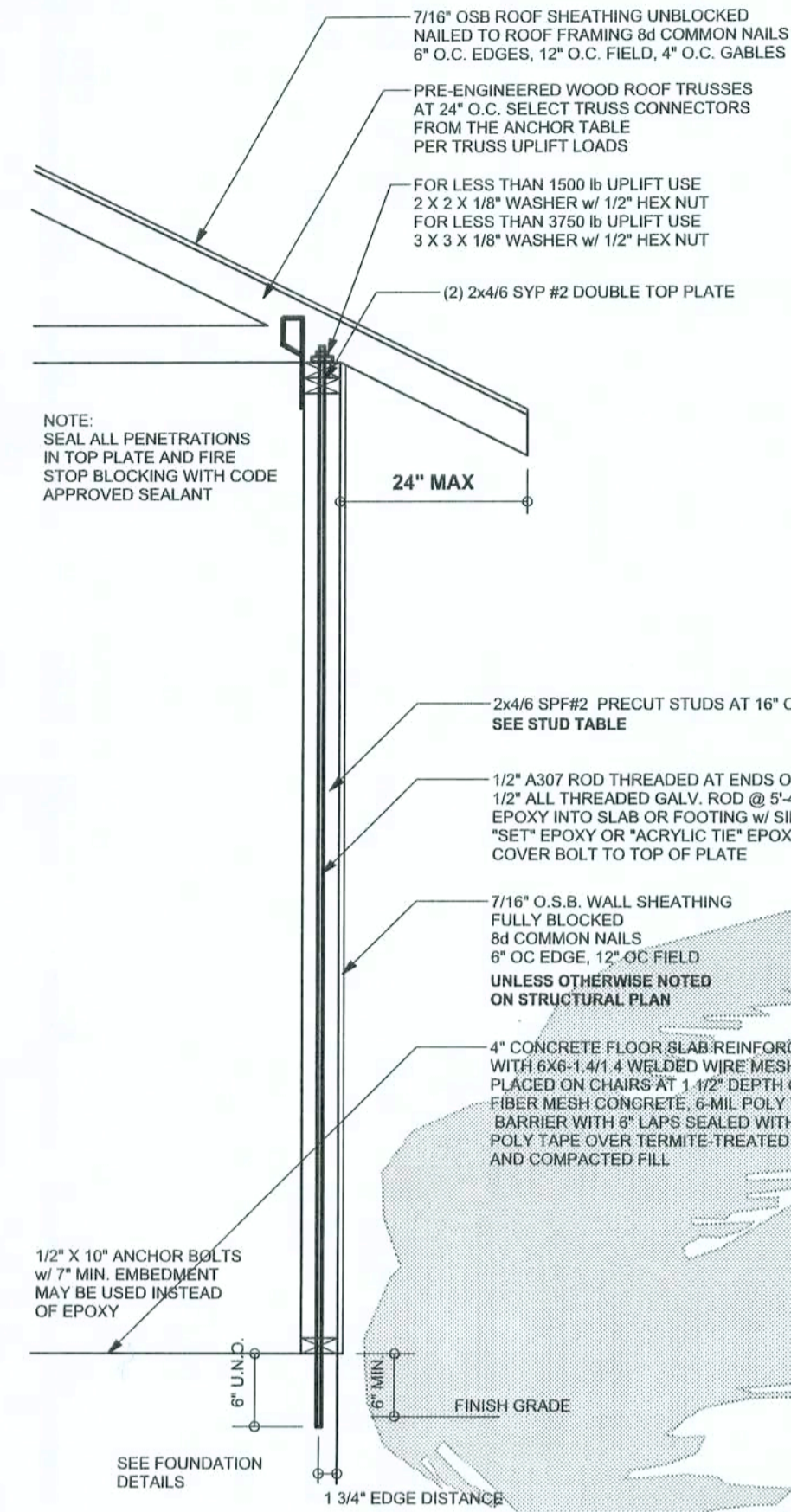
ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR' SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

A-5

LOT 8 - COUNTRY LAKES AT WOODBOROUGH

FILE: 07-D14	McNEILL RESIDENCE	SHEET: 5 of 5
DATE: 8-28-07		CAD FILE: 07014
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Cir. Lake City, FL 32024 Phone: (386) 755-5891	REV:

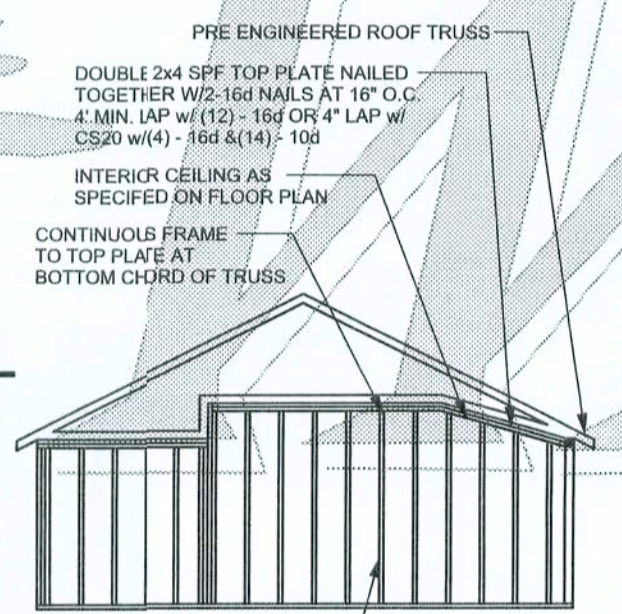


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

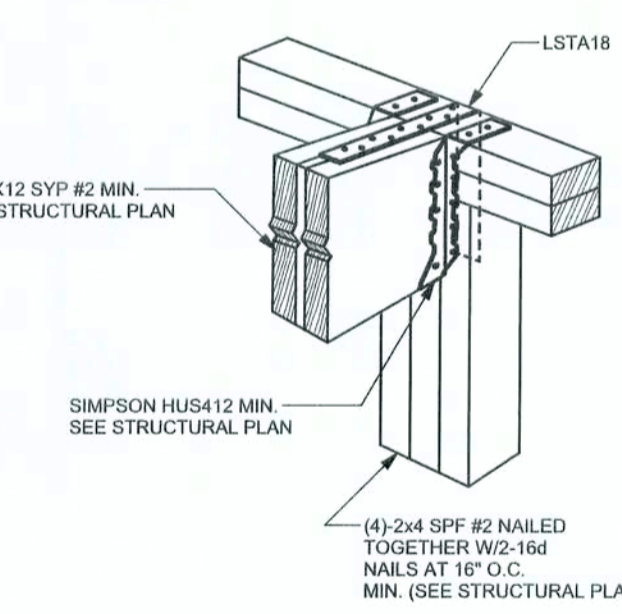
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

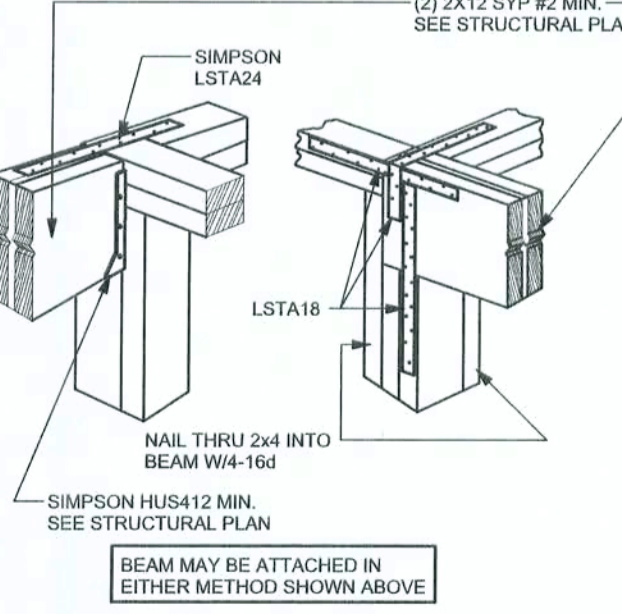
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: 1/2" = 1'-0"



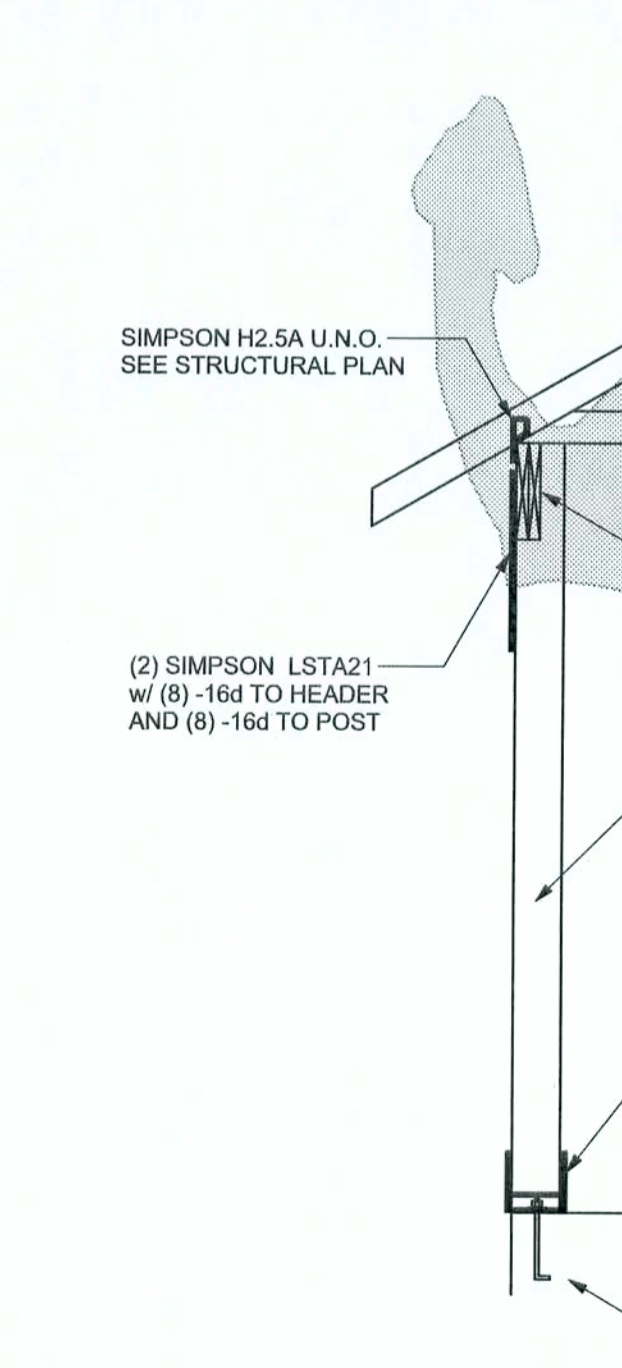
BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.



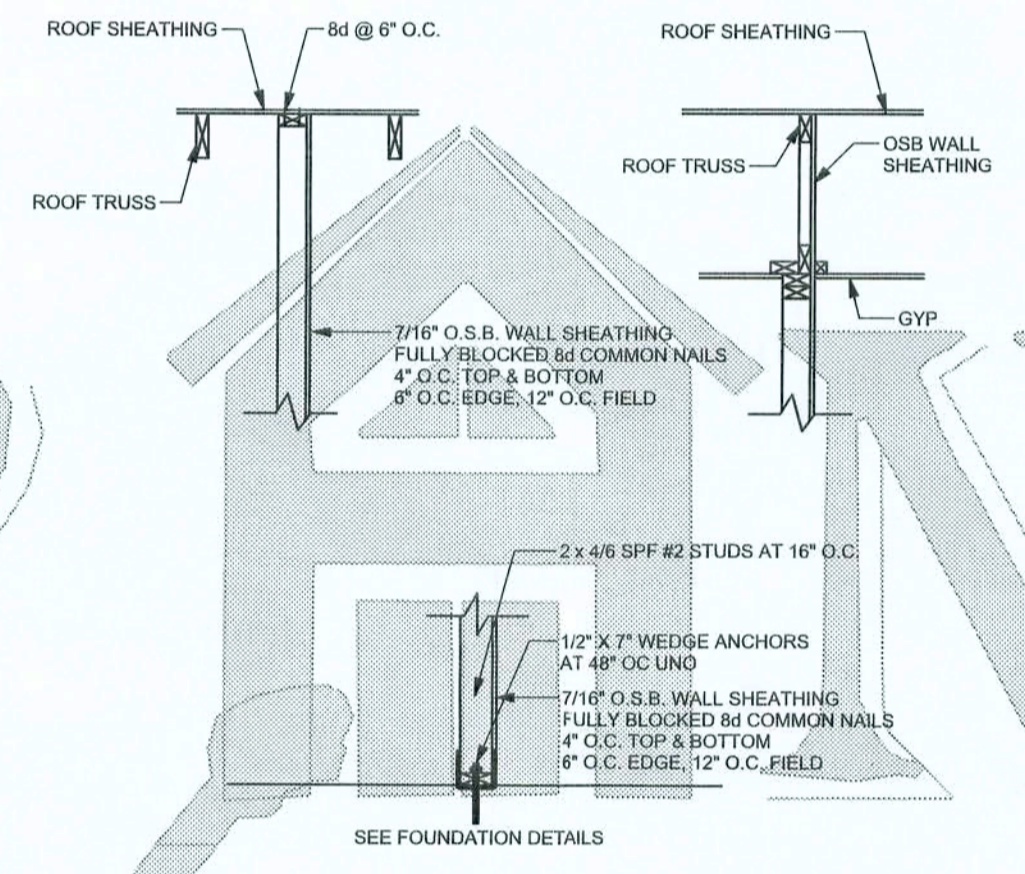
SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.



TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"

INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

GABLE BRACING DETAIL
SCALE: 1/2" = 1'-0"



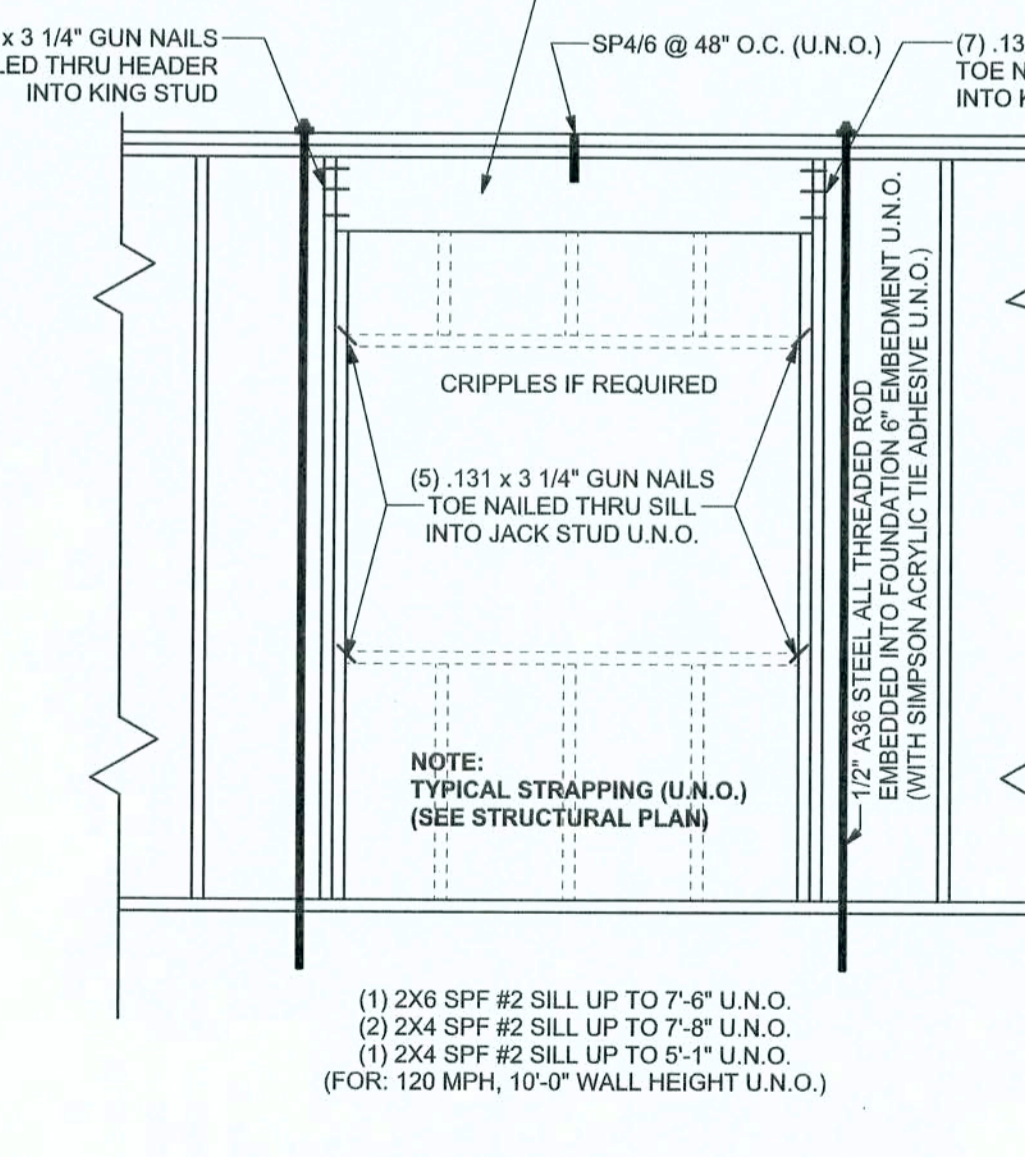
INTERIOR SHEAR WALL DETAIL
SCALE: 1/2" = 1'-0"

NOTE: IF TRUSSES TO WALL STRAPS ARE NAILED TO THE HEADER THE SP4/6 @ 48" O.C. ARE NOT REQUIRED

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8" WASHER FOR LESS THAN 3750 lb UPLIFT USE 3 X 3 X 1/8" WASHER

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8" WASHER FOR LESS THAN 3750 lb UPLIFT USE 3 X 3 X 1/8" WASHER

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8" WASHER FOR LESS THAN 3750 lb UPLIFT USE 3 X 3 X 1/8" WASHER



TYPICAL 1 STORY HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	13-8d	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 900	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG72	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3865	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 8035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 800	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18		14-10d	
< 1235	< 1235	LSTA21		16-10d	
< 1030	< 1030	CS20		18-8d	
< 1705	< 1705	CS16		28-8d	
STUD ANCHORS*					
< 1350	< 1305	LTT19		8-16d	1/2" AB
< 2310	< 2310	LTT131		18-10d, 1 1/2"	1/2" AB
< 2775	< 2570	HD2A		2-5/8" BOLTS	5/8" AB
< 4175	< 3665	HTR76		18-16d	5/8" AB
< 1400	< 1400	PAHD42		16-16d	
< 3335	< 3335	HPAH222		16-16d	
< 2200	< 2200	ABU44			1/2" AB
< 2300	< 2300	ABU66			1/2" AB
< 2320	< 2320	ABU88			2-5/8" AB

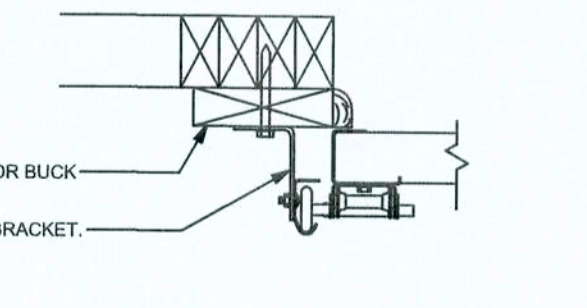
GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200 1.6
2x10	SYP #2	1050 1.6
2x12	SYP #2	975 1.6
GLB	24F-V3 SP	2400 1.8
LSL	TIMBERSTRAND	1700 1.7
LVL	MICROLAM	2900 2.0
PSL	PARALAM	2900 2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" X 4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 1 3/4" x 3 1/4" ON PER TABLE BELOW.

DOOR WIDTH	38" x 4" LAG	16d STAGGER	(2) ROWS OF 1 3/4" x 3 1/4" ON
8' - 10'	24" O.C.	6" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, TEMPORARY AND PERMANENT BRACING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS DESIGN AND PROVIDE TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR INTERIOR BEARING WALLS. TRUSS ENGINEERING SHALL BE PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB + 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAW CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10 FT. DO NOT CUT W/OUT REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM GLB, 24F-V3SP, F_b = 2.4ksi, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. UNO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 15" IN GROUDED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 7" x 2" x 3/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 9/64", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMFITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL, FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	15	20
1	19.9	21.8	18.1	-18.1
2	19.9	25.5	18.1	-21.8
3	19.9	40.6		-40.6
3	19.9	-25.5	18.1	-21.8
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows		21.8	-29.1	
Worst Case (Zone 5, 10 ft ²)				
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0	

DESIGN LOADS

FLOOR: 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3-12)

ROOF: 20 PSF (FLAT OR <4-12)

16 PSF (4-12 TO <12-12)

12 PSF (12-12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS



WINDLOAD ENGINEER: Mark Discoway, P.E. No. 53815, PCB 868, Lake City, FL 32066, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
P.E. 53815

26 SEP 07

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PRINTED DATE:
September 26, 2007

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Ben Sparks

FINALS DATE:
26Sep07

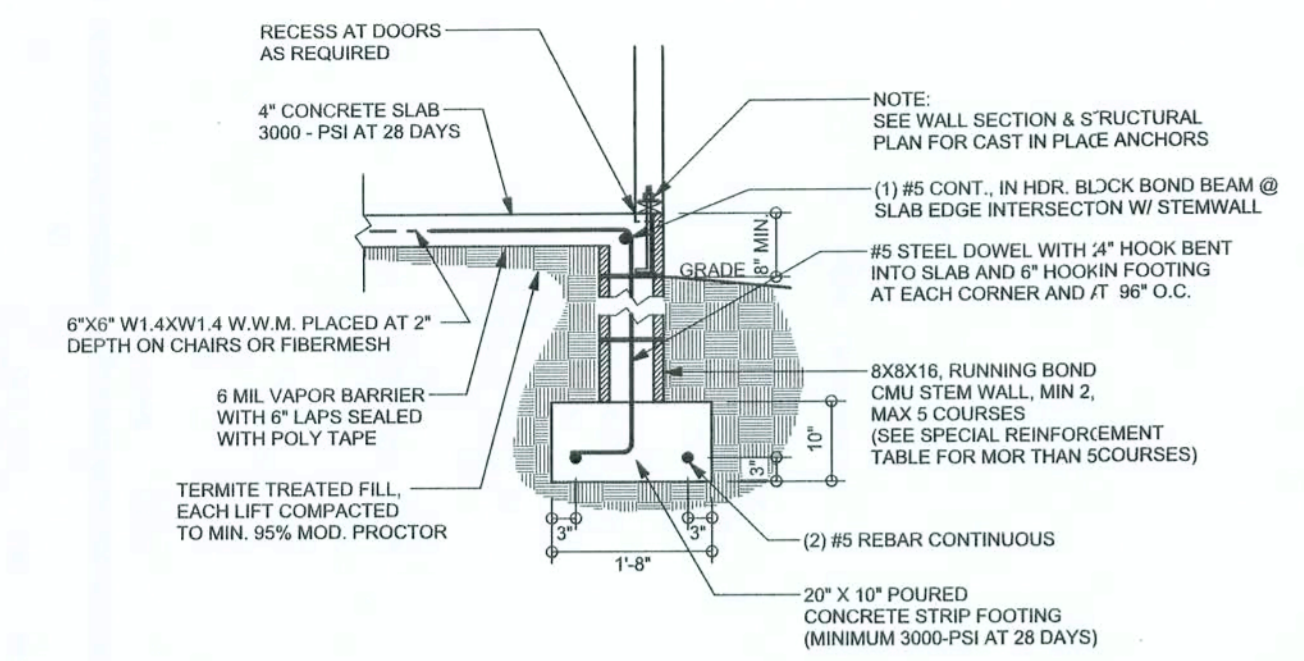
JOB NUMBER:
709201

DRAWING NUMBER
S-1

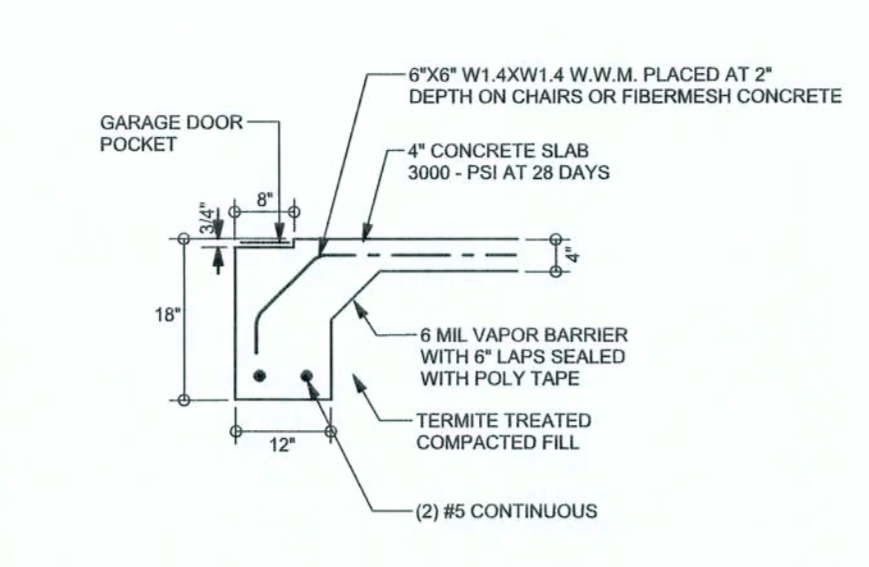
OF 3 SHEETS

REVISIONS	

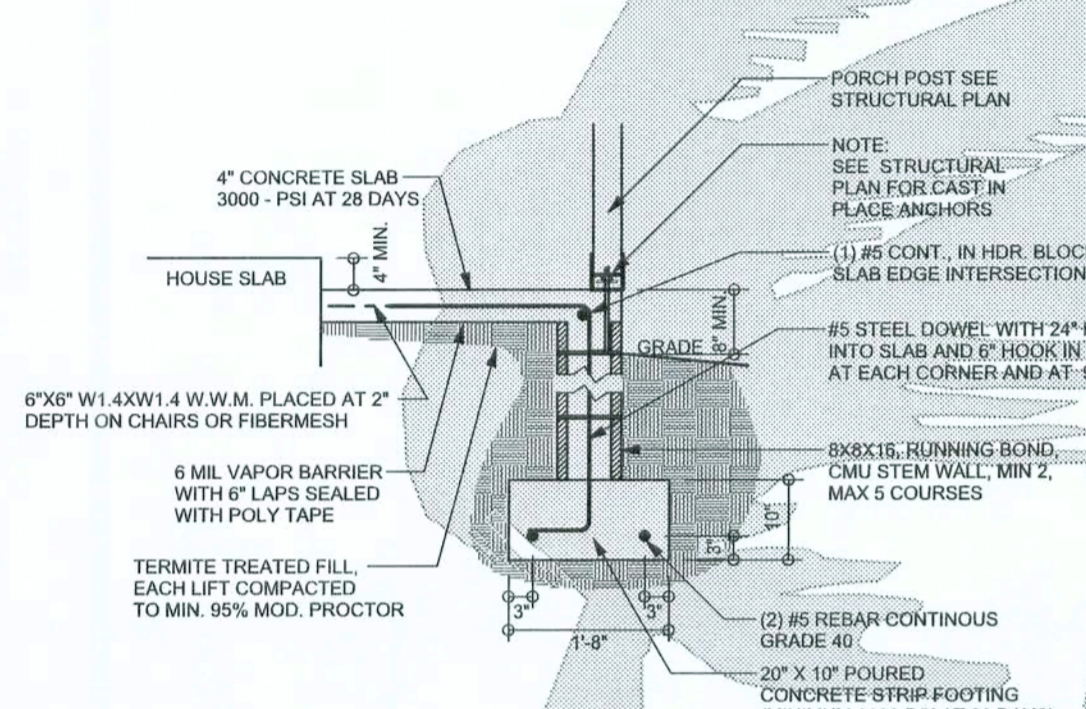
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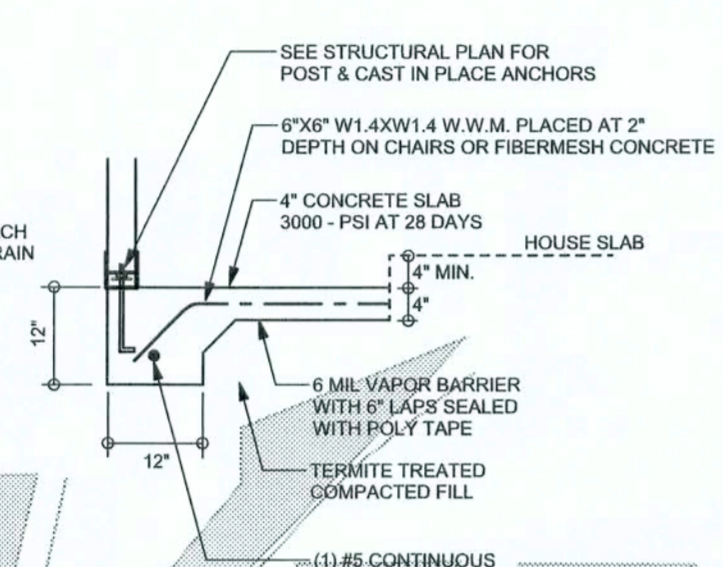
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



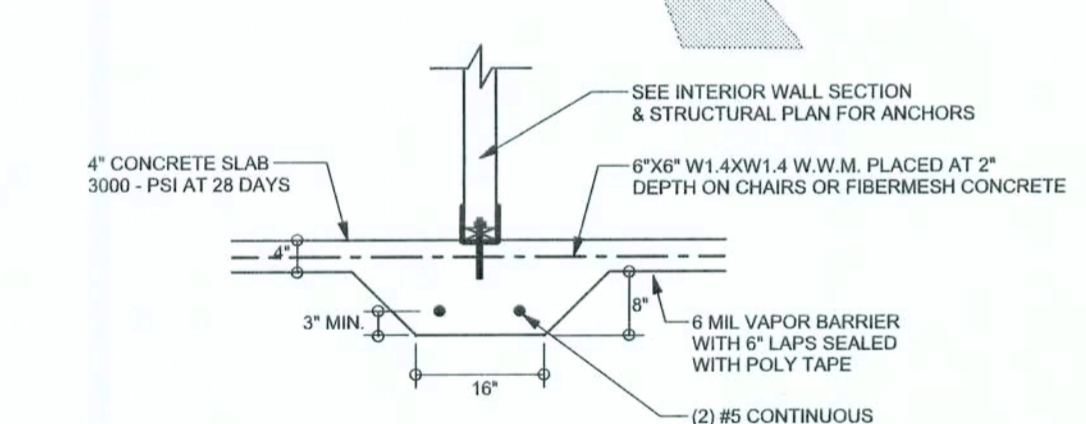
F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

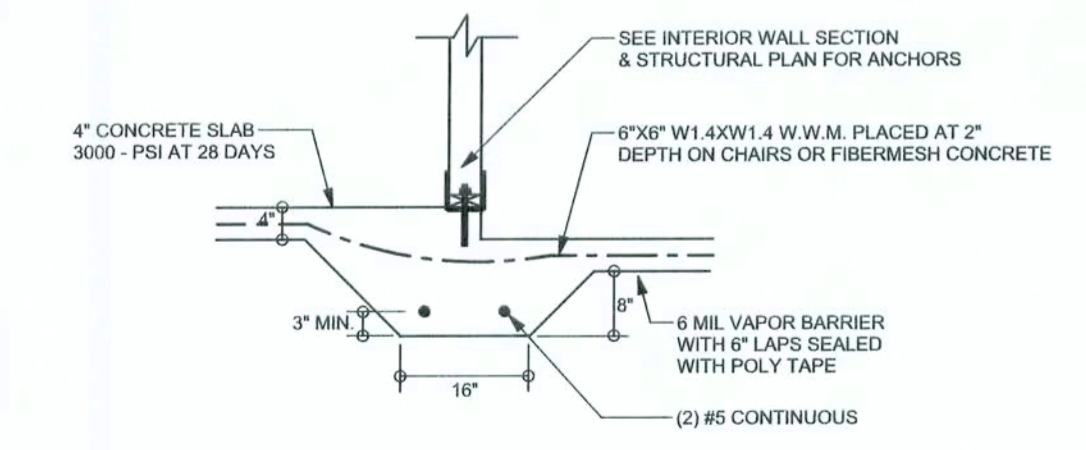


F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

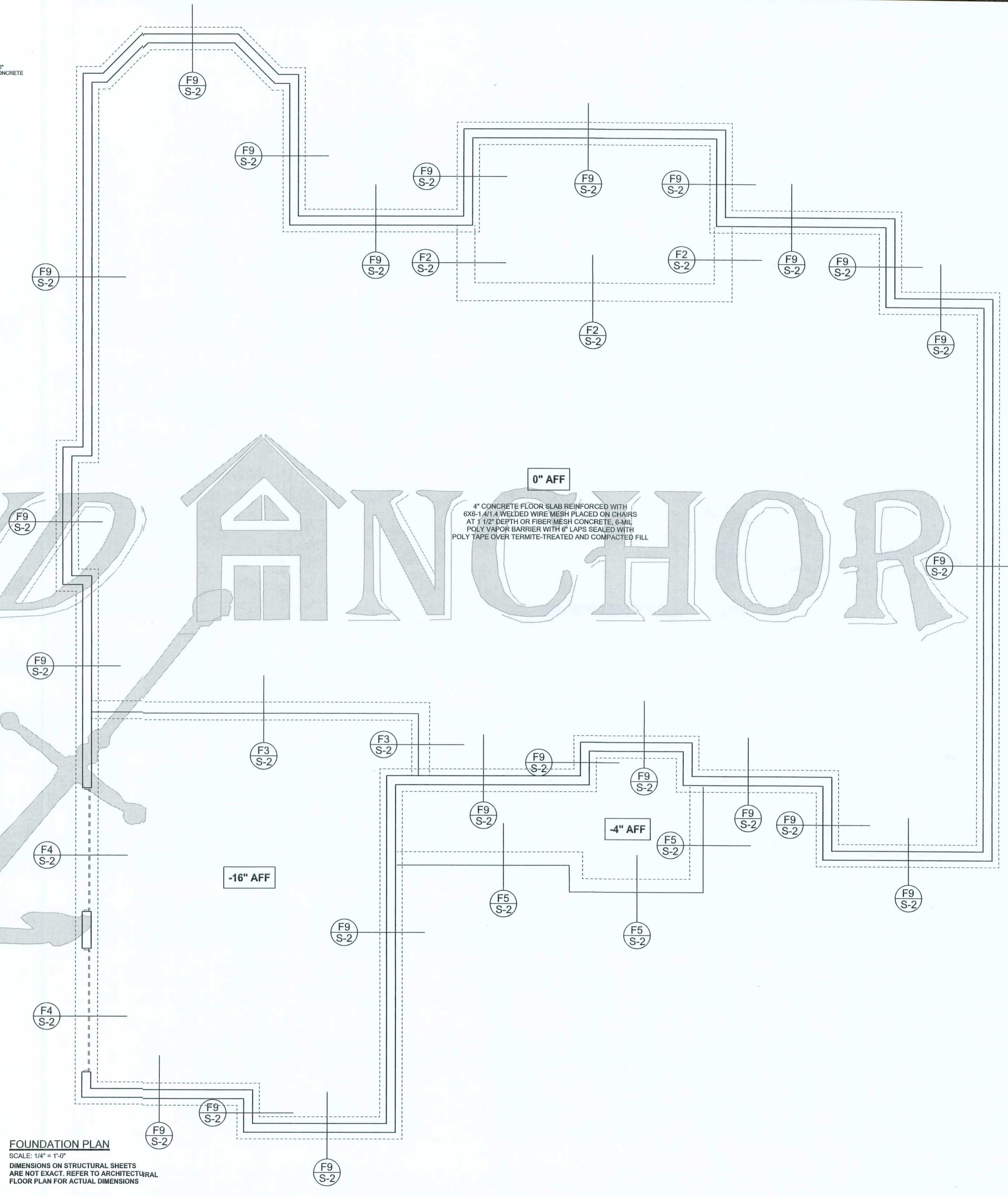
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section FDOT 2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

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FINALS DATE:
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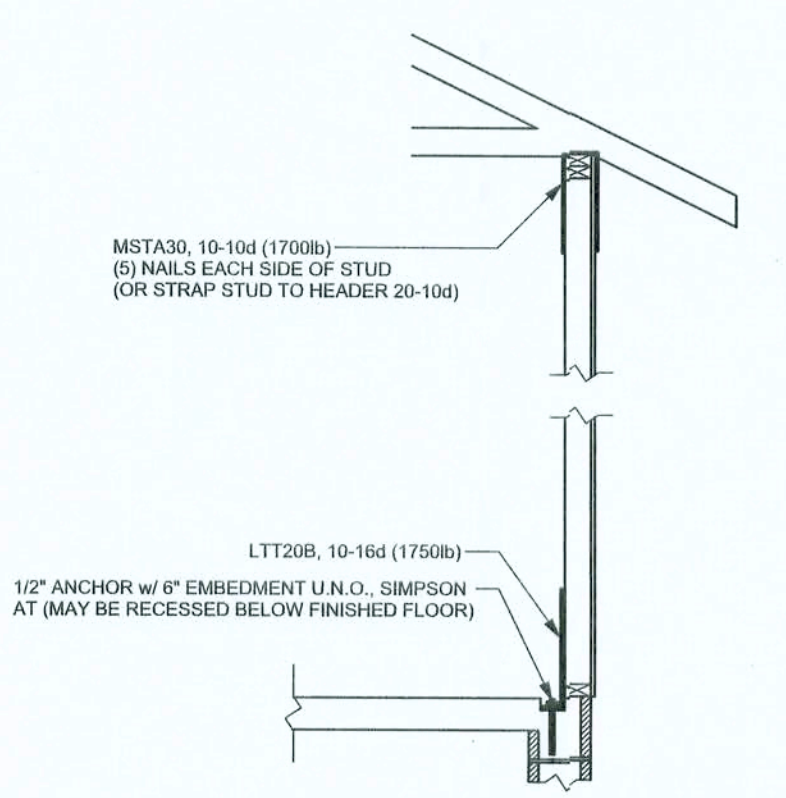
DRAWING NUMBER
S-2
OF 3 SHEETS

REVISIONS	



STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	67.0'	99.5'
LONGITUDINAL	44.0'	67.5'

ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL
SCALE: 1/2" = 1'-0"

THREADED ROD LEGEND

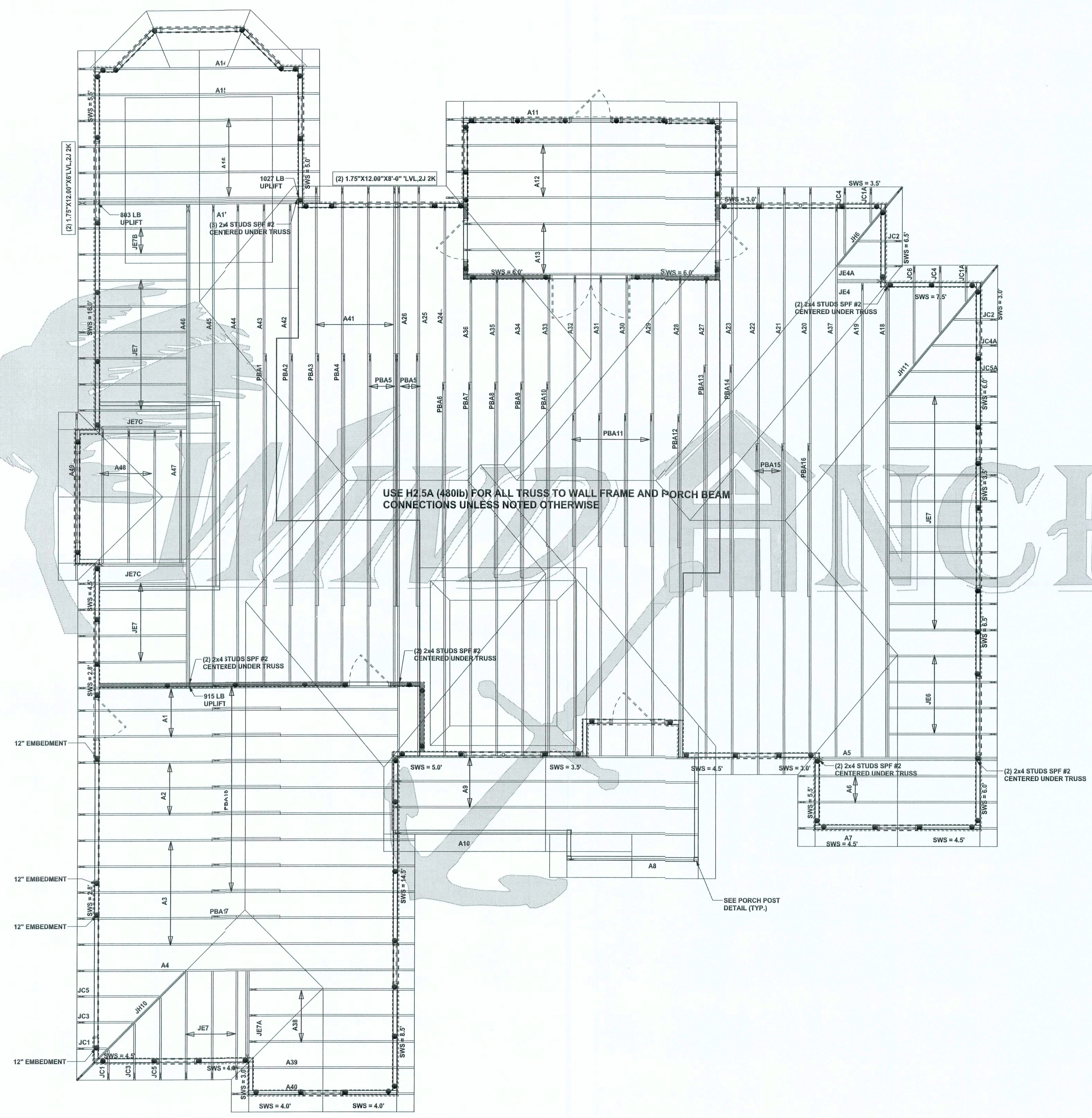
- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

- (2) 2X12X0',1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)
- ↑ NUMBER OF KING STUDS (FULL LENGTH)
- ↑ NUMBER OF JACK STUDS (UNDER HEADER)
- ↑ SPAN OF HEADER
- ↑ SIZE OF HEADER MATERIAL
- ↑ NUMBER OF PLYS IN HEADER

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER: W.B. HOWLAND
JOB #4883

WINDLOAD ENGINEER: Mark Disosway
PE No. 53915, POB 866, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section FC07.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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Mark Disosway
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JOB NUMBER:
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DRAWING NUMBER
S-3

OF 3 SHEETS

REVISIONS
1/14/2008

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	HS	4-8d	4-8d	
< 350	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	HB	8-8d	8-8d	
< 745	< 565	HB	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	13-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 960	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2800	< 2490	2-HTS24			
< 2050	< 1785	LG72	14-16d	14-16d	

		TO FOUNDATION	
< 3965	< 3330	MG7	22-10d, 1-5/8" THREADED ROD 12" EMBEDMENT
< 1080	< 6485	HGT-2	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4	2-5/8" THREADED ROD 12" EMBEDMENT

		TO STUDS		TO FOUNDATION	
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d	4-10d	
< 455	< 420	SSP SINGLE SILL PLATE	1-10d	4-10d	
< 825	< 825	DSP DOUBLE SILL PLATE	6-10d	8-10d	
< 825	< 600	DSP SINGLE SILL PLATE	2-10d	8-10d	
< 885	< 760	SP4	8-10d, 1 1/2"		
< 1240	< 1065	SPH4	10-10d, 1 1/2"		
< 885	< 760	SP6	8-10d, 1 1/2"		
< 1240	< 1065	SPH6	10-10d, 1 1/2"		
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	16-8d		
< 1705	< 1705	CS16	28-8d		

		TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d	1/2" AB	
< 2310	< 2310	LTT31	16-10d, 1 1/2"	1/2" AB	
< 2775	< 2570	H2A	2-5/8" BOLTS	5/8" AB	
< 4175	< 3695	HTT16	18-10d	5/8" AB	
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d	1/2" AB	
< 2300	< 2300	ABU86	12-16d	1/2" AB	
< 2320	< 2320	ABU88	18-16d	2-5/8" AB	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED WITH THE TRUSS MANUFACTURER TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415L EACH END; 2X8 RAFTERS 700 LB EACH END.

FOUNDATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION & TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185 LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, FB = 24KSI, E = 1800KSI, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH #6 COMMON NAILS (1 1/2") @ 6" O.C. EDGES, 12" O.C. INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTIONS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 10" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

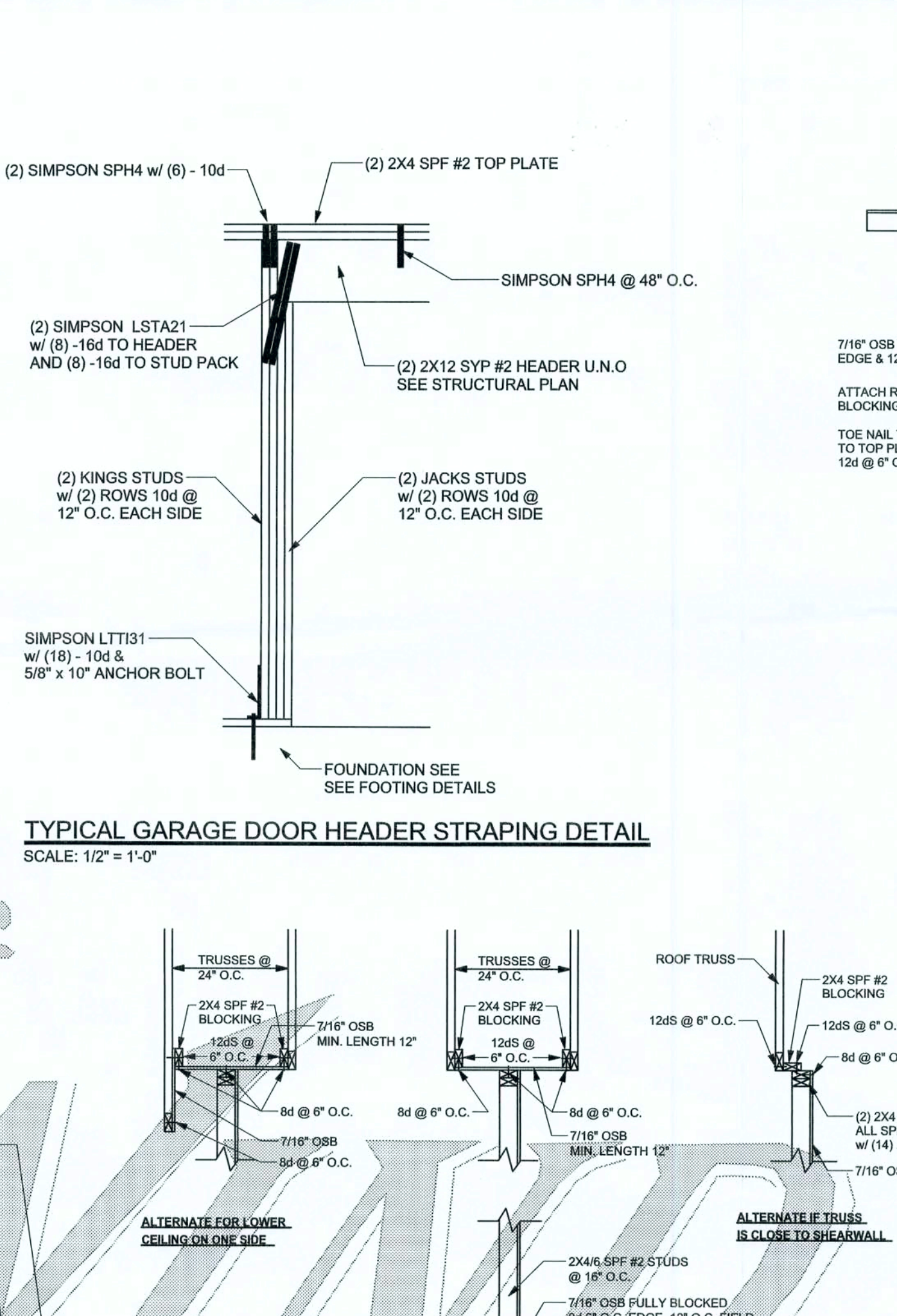
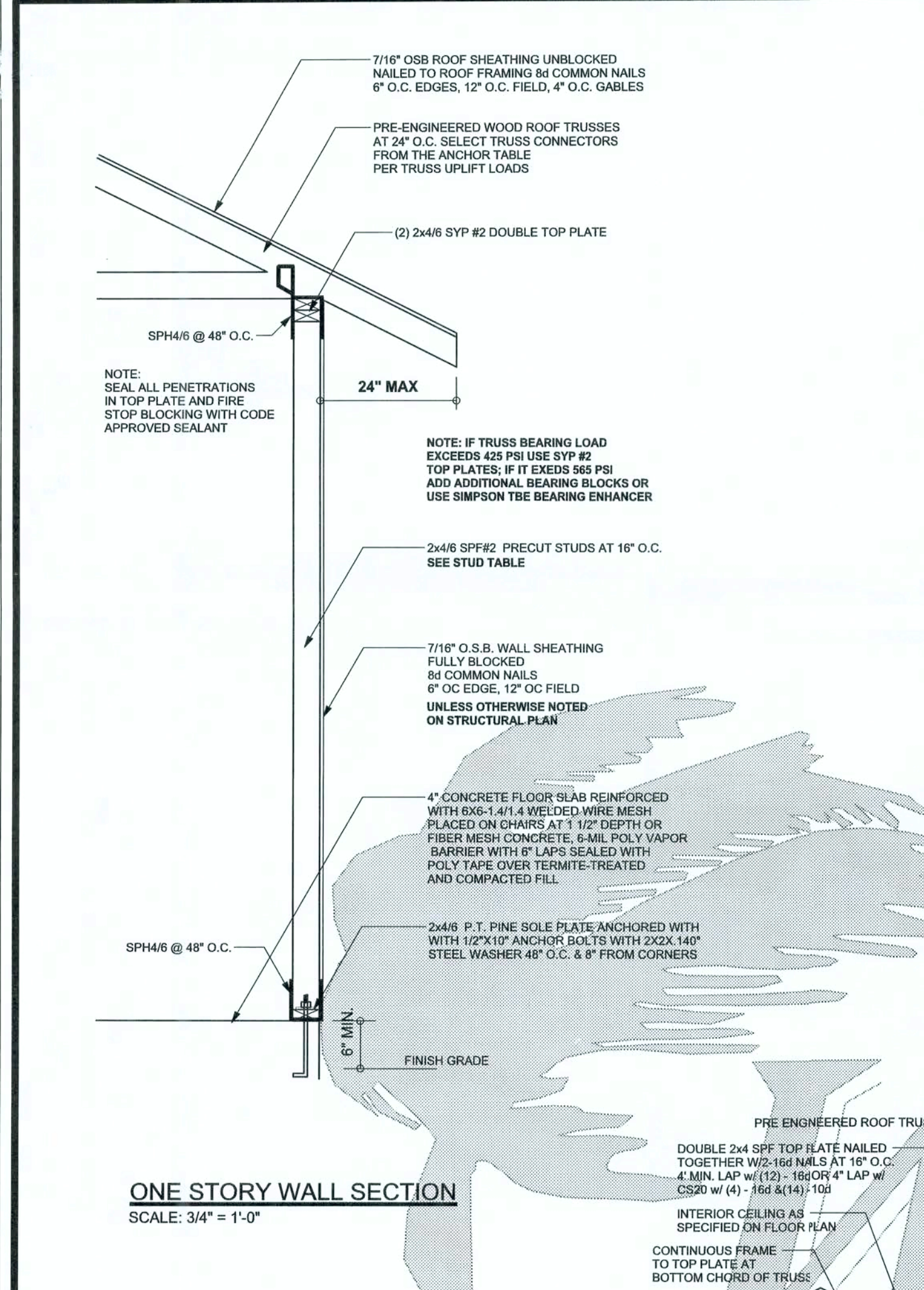
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMFITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. THE BUILDER IS RESPONSIBLE FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



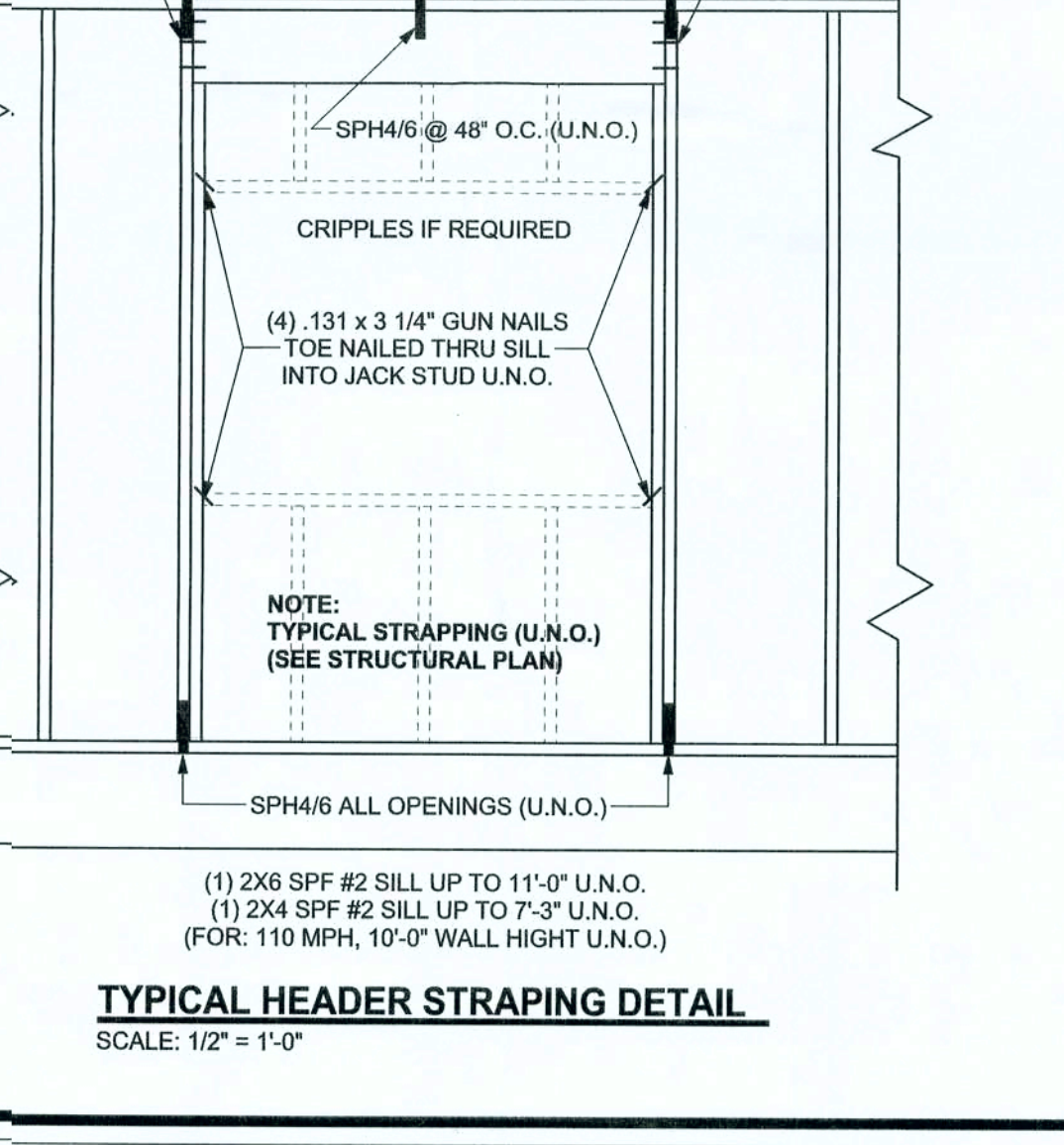
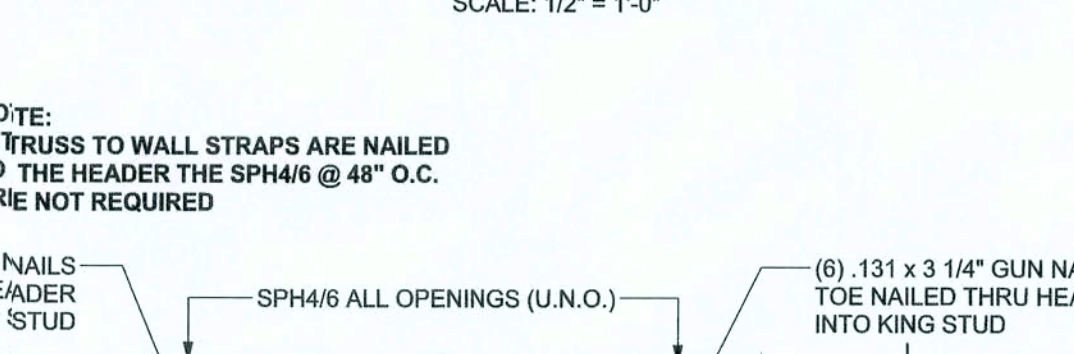
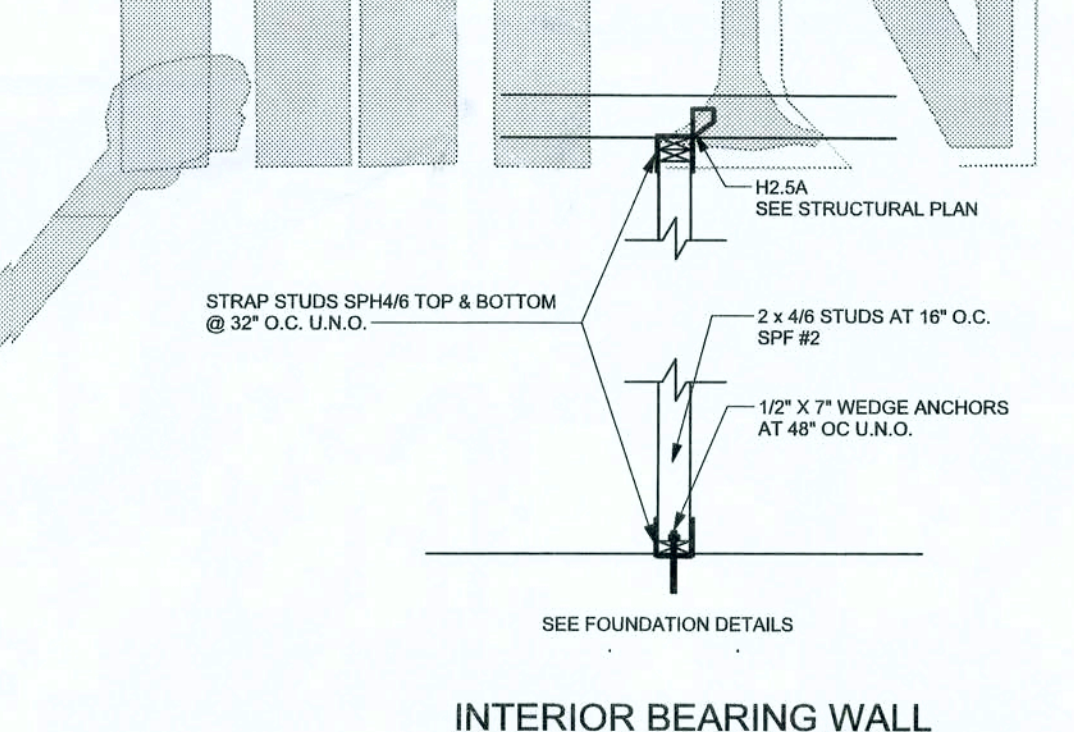
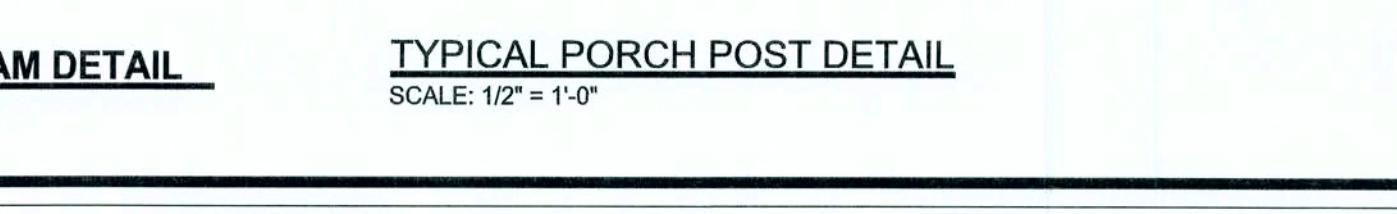
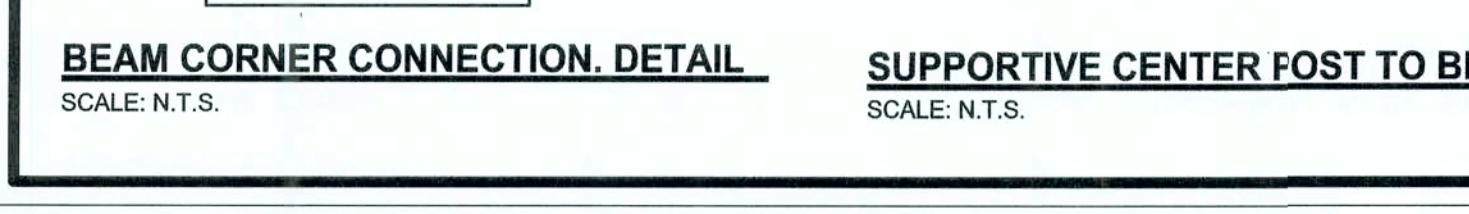
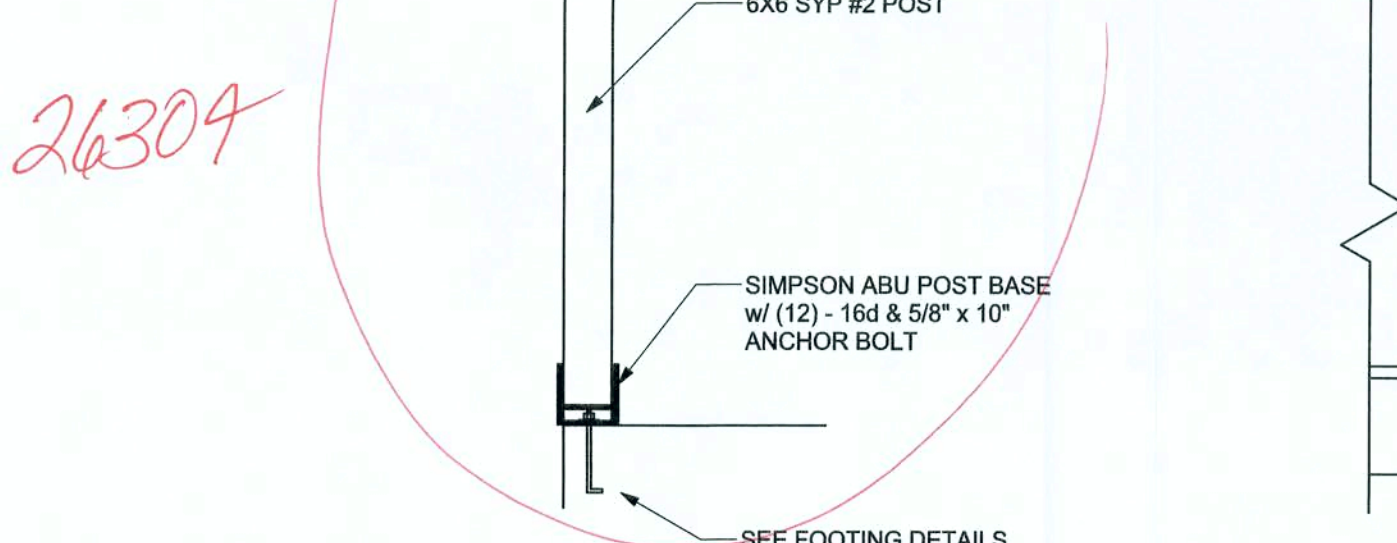
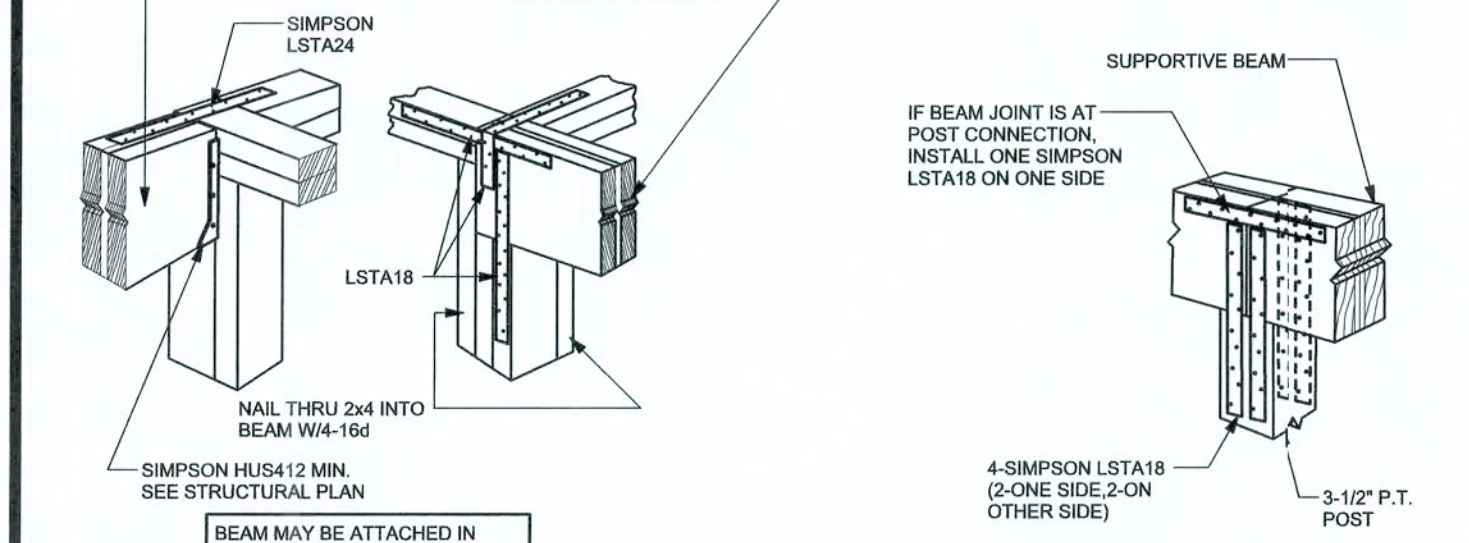
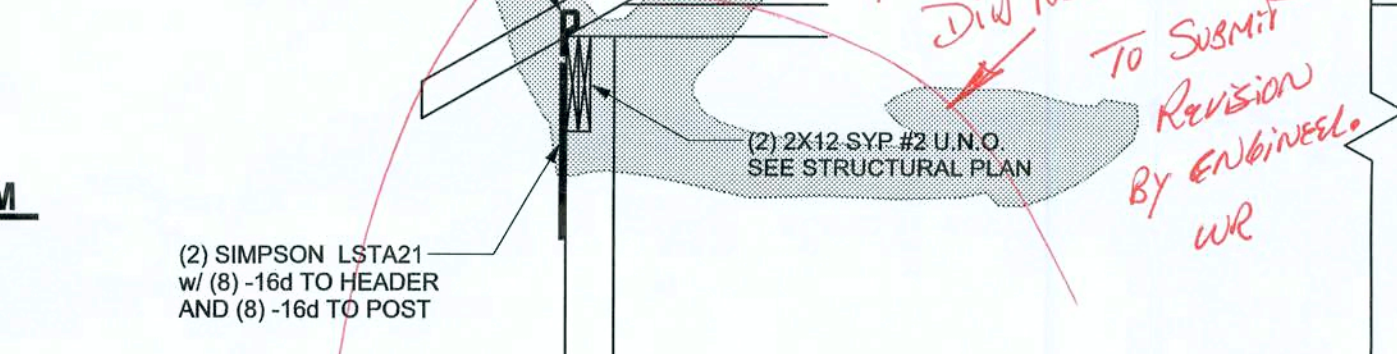
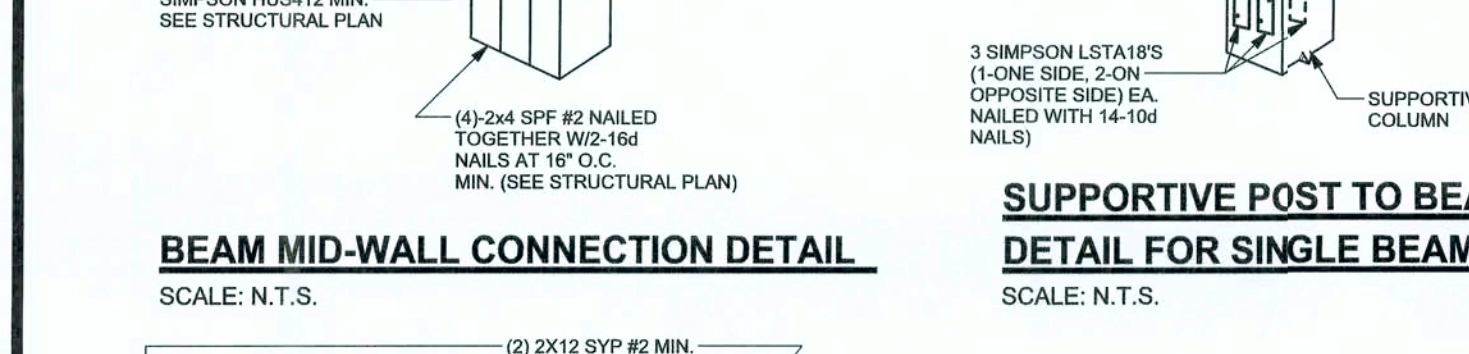
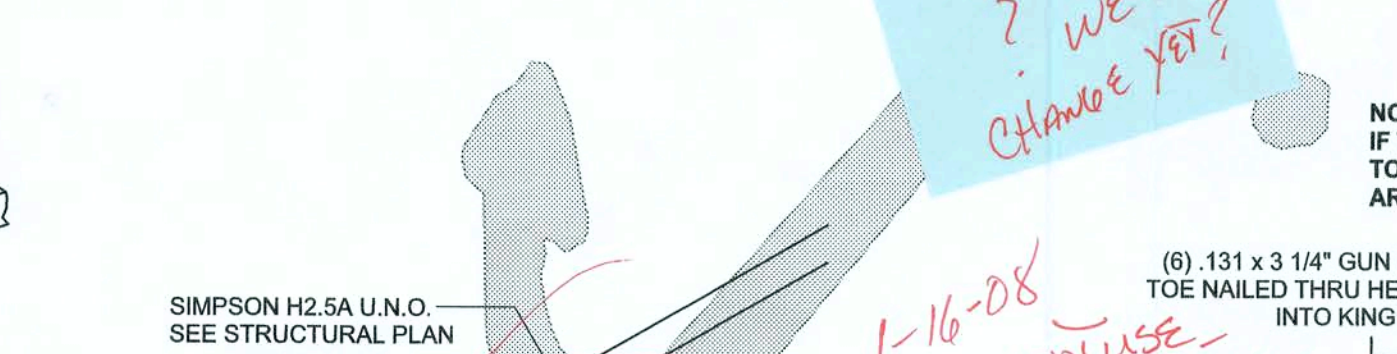
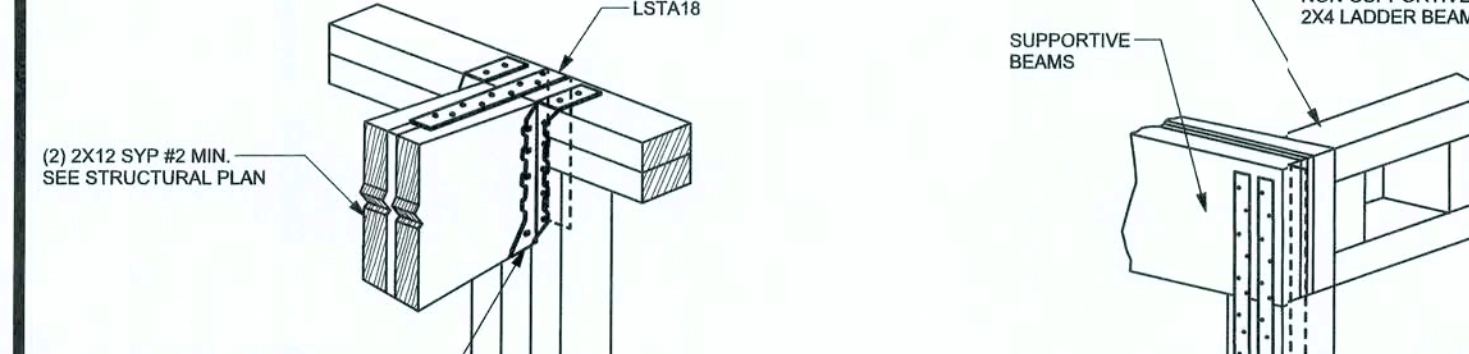
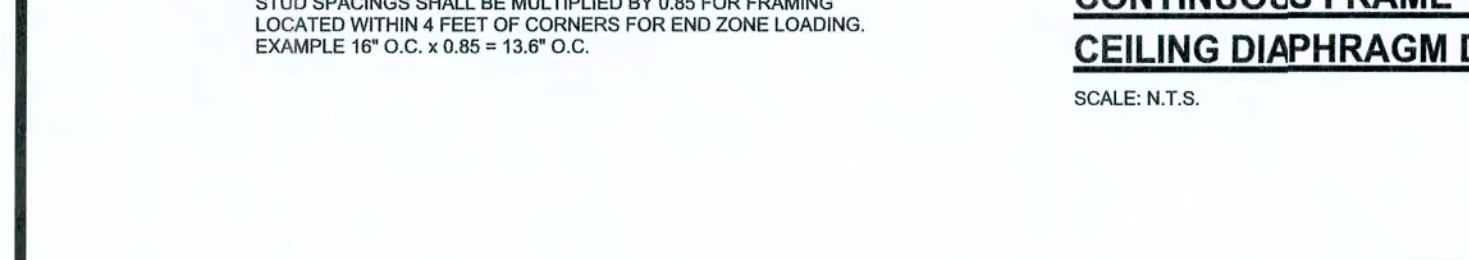
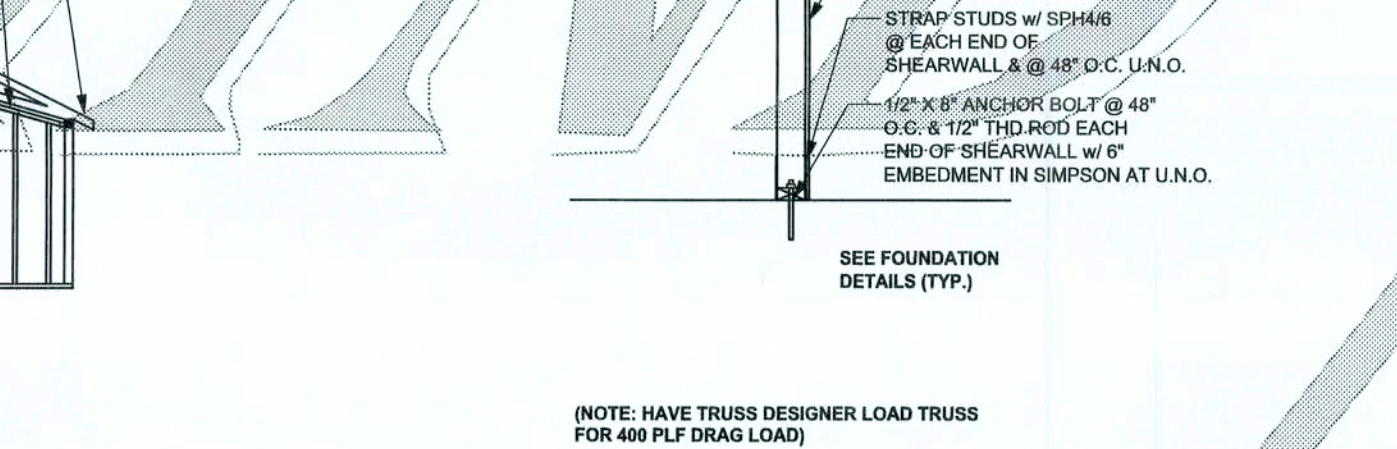
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

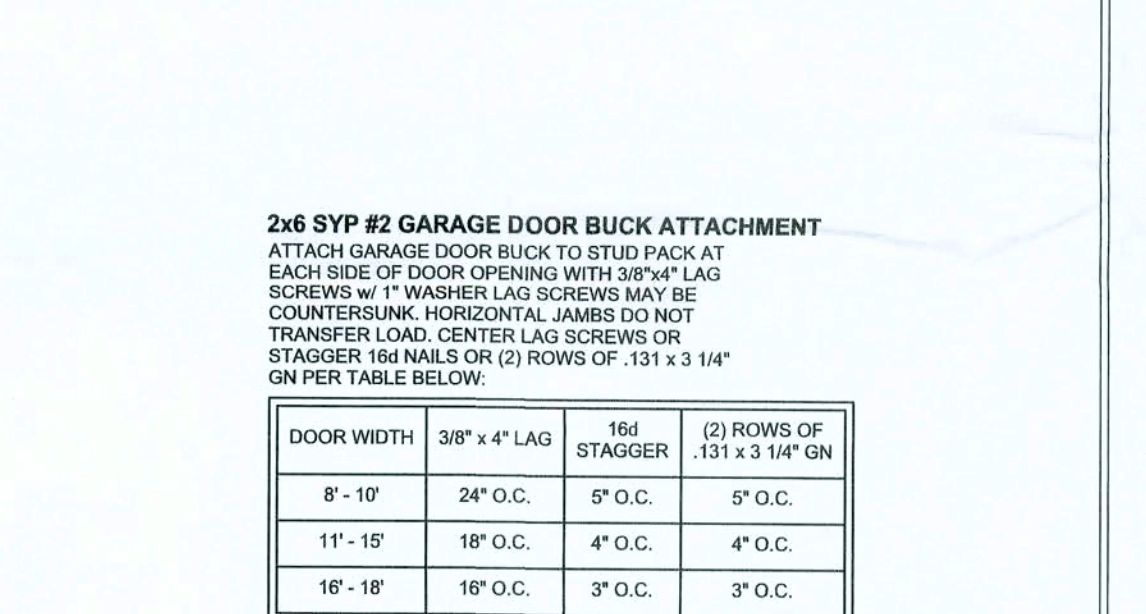
ALL STUDS TO BE 2x4 SPF NAILED TO TOP AND BOTTOM PLATES WITH 2-16d NAILS

SCALE: N.T.S.



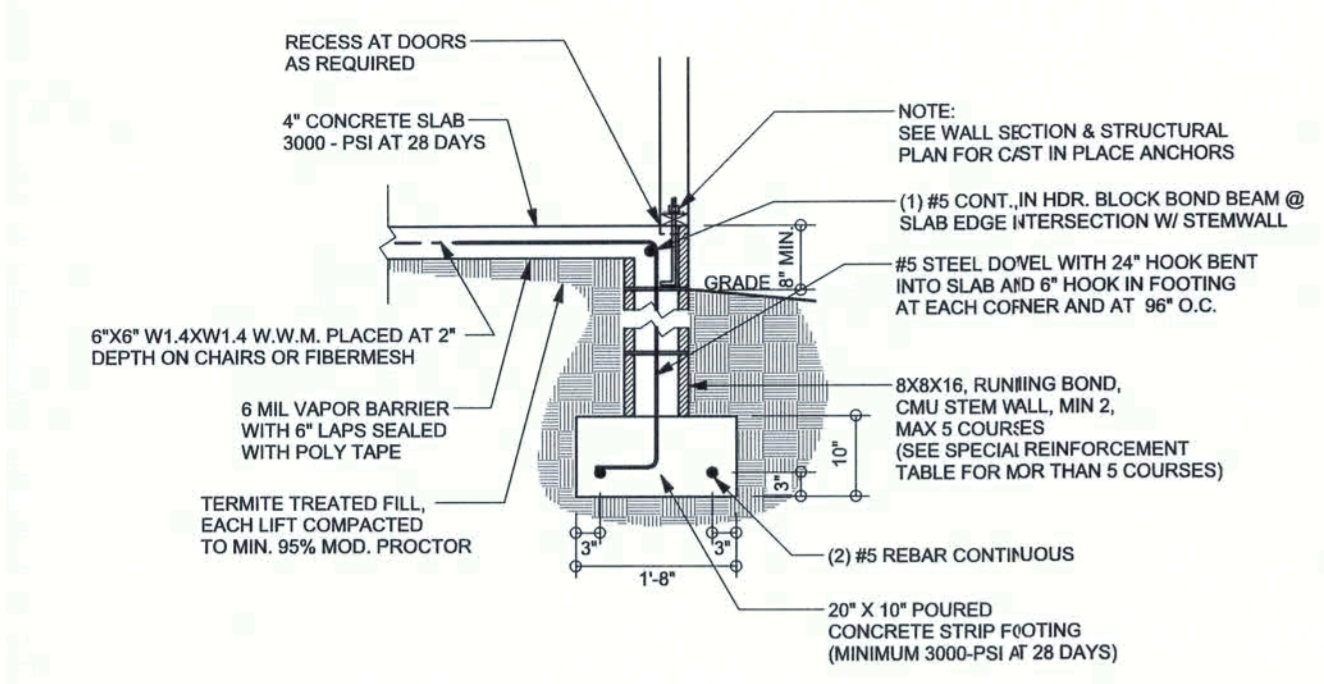
GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8 SYP #2	1200	1.6
2x10 SYP #2	1050	1.6
2x12 SYP #2	975	1.6
GLB 24F-V3 SP	2400	1.8
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2900	2.0
PSL PARALAM	2900	2.0



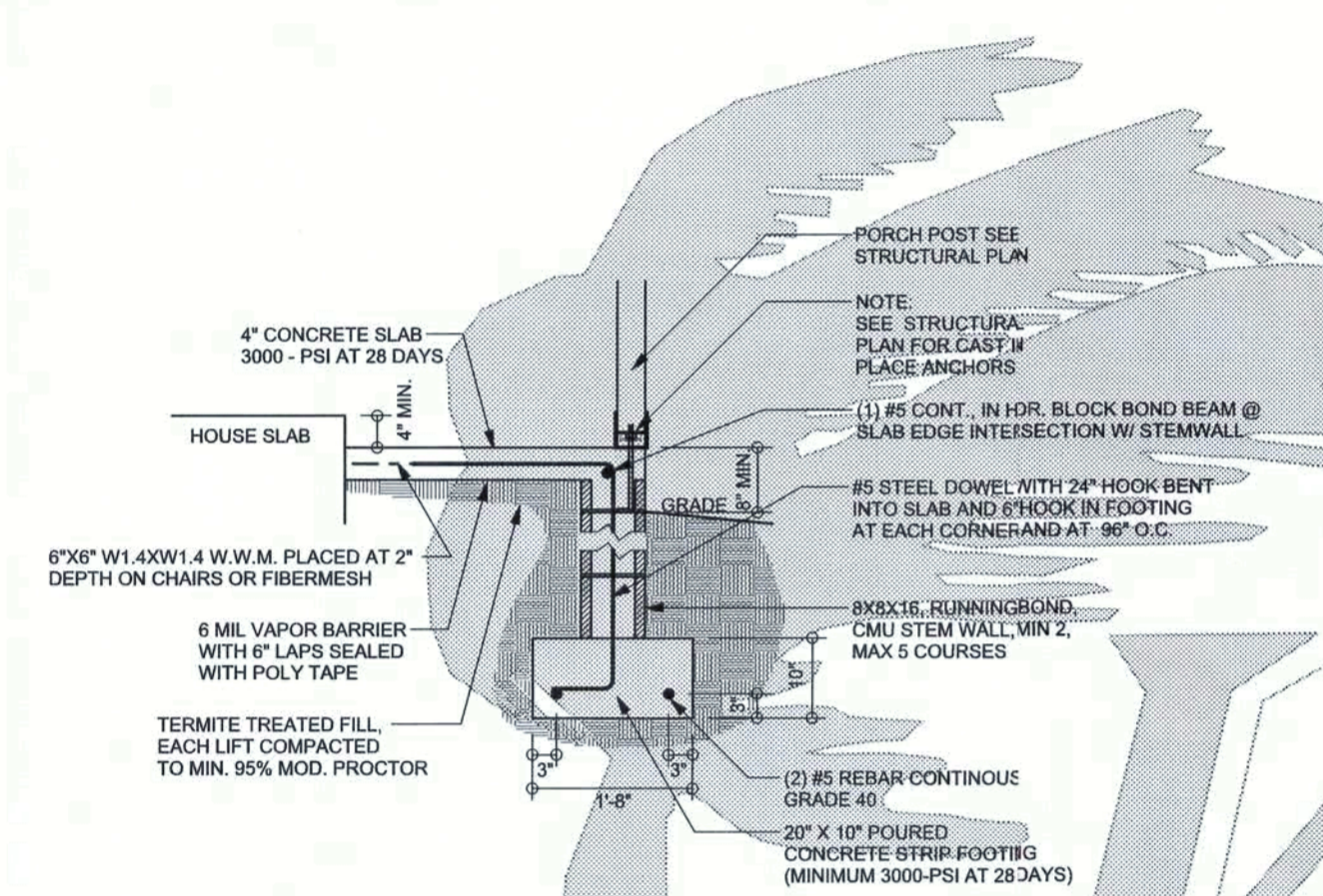
REVISIONS	
1/14/2008	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



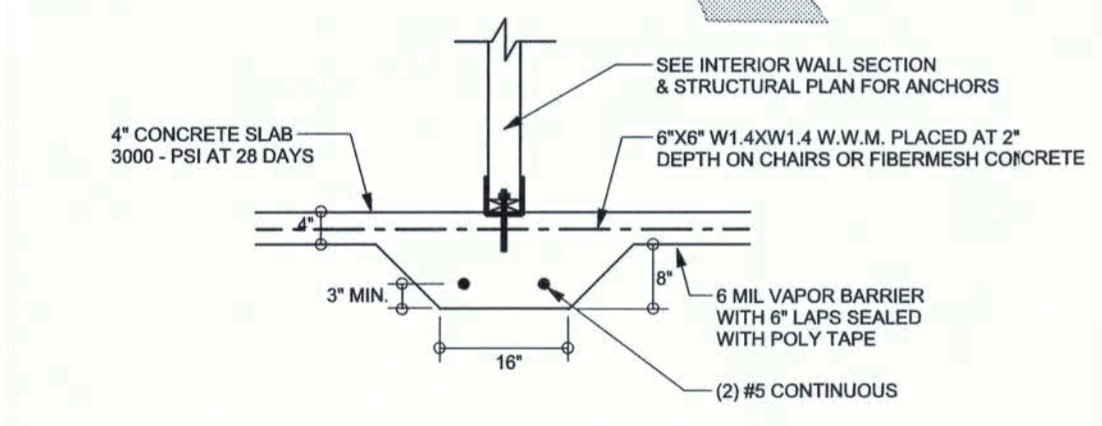
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

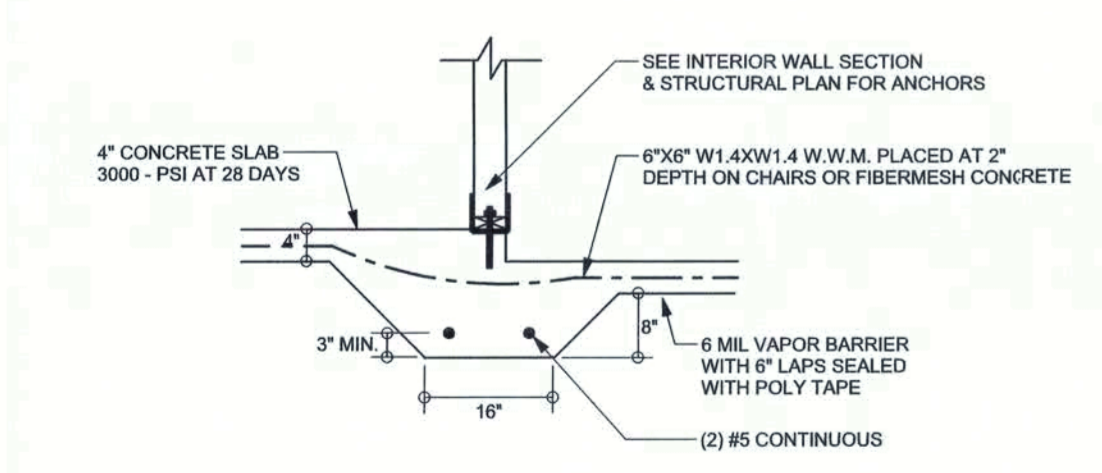


F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

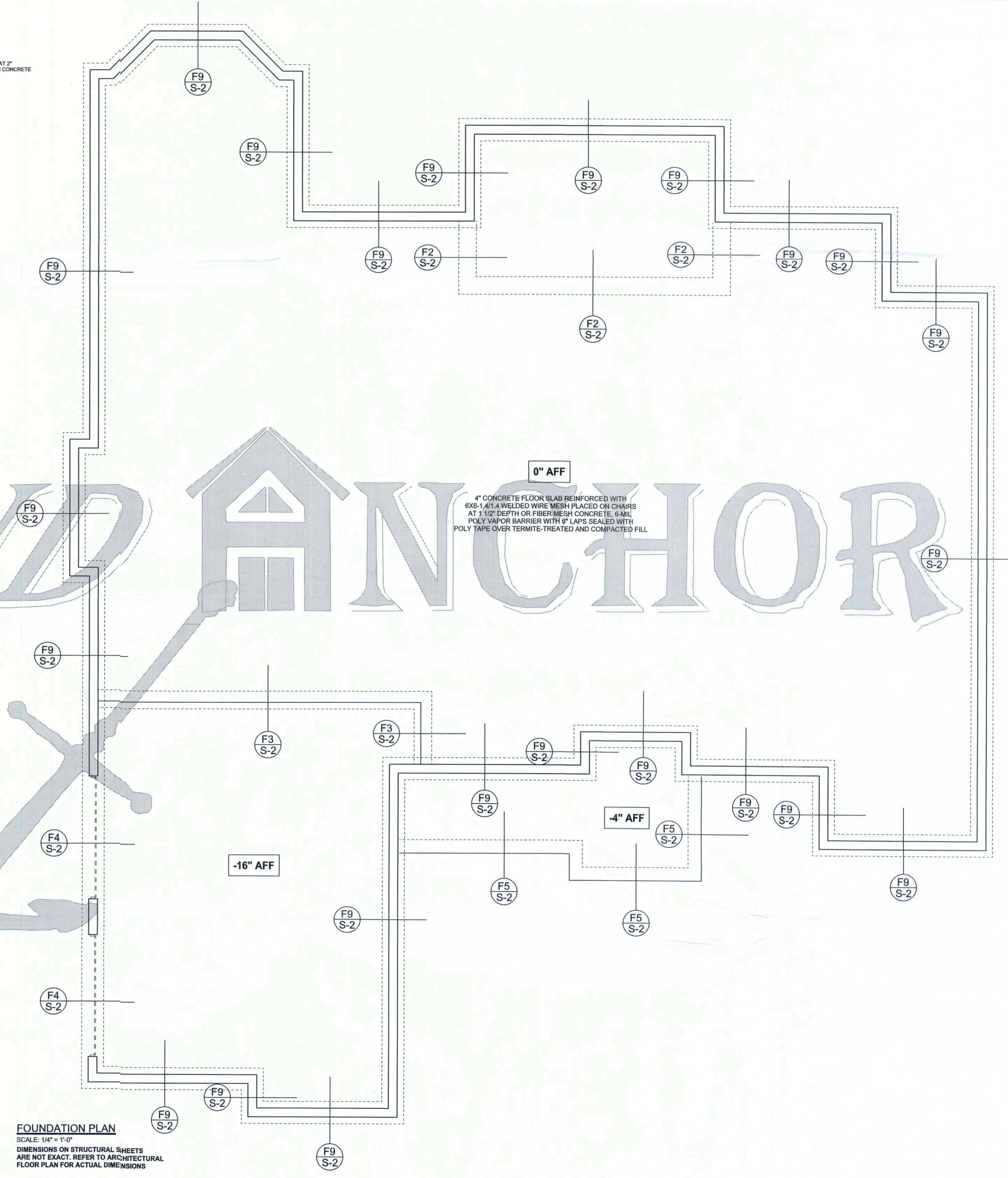


F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 16" continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 888, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 903.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
14 Jan 08
SEAL

Blake Construction

McNeill Residence

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Phone: (386) 754 - 5419
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PRINTED DATE:
January 14, 2008

DRAWN BY: Ben Sparks
STRUCTURAL BY: Ben Sparks

FINALS DATE:
26Sep07

JOB NUMBER:
709201

DRAWING NUMBER
S-2
OF 3 SHEETS

REVISIONS	
1/14/2008	



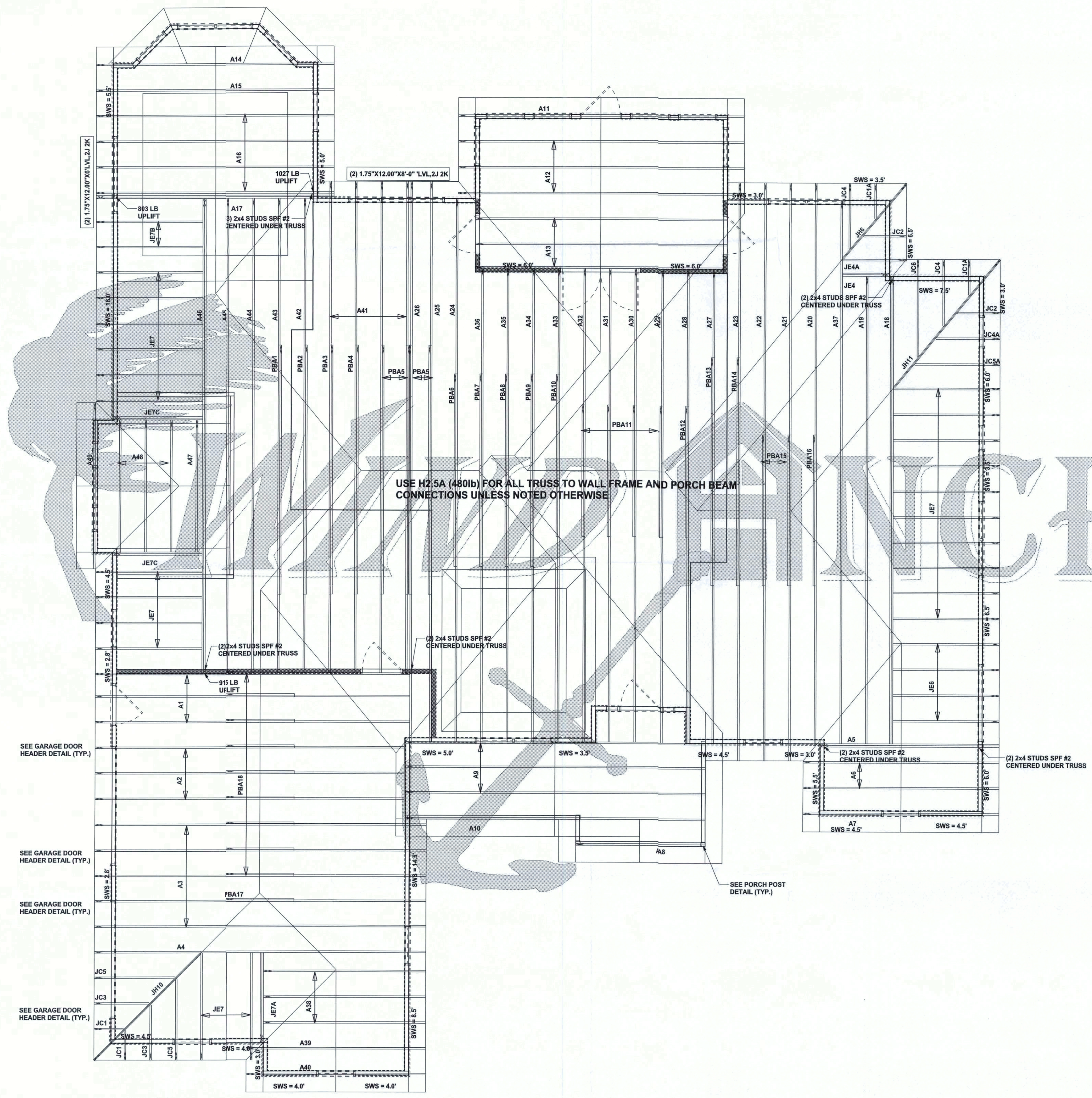
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

TOTAL SHEAR WALL SEGMENTS

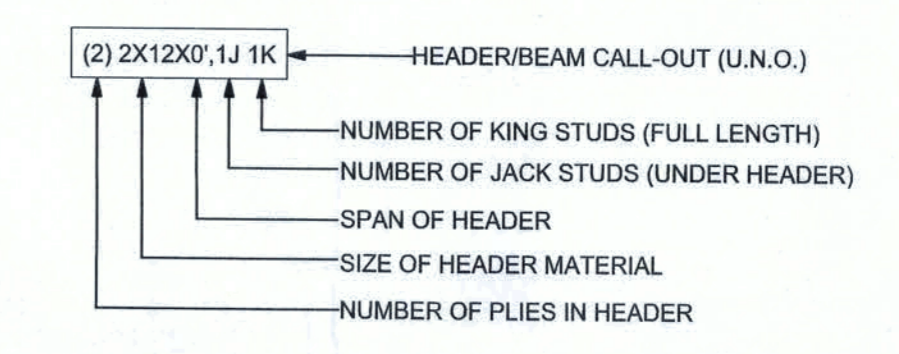
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	67.0'	99.5'
LONGITUDINAL	44.0'	67.5'



USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

HEADER LEGEND



WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND, JOB #4883

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
Mark Disoway
14.10.08
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PRINTED DATE:
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DRAWN BY: STRUCTURAL BY:
Ben Sparks

FINALS DATE:
26Sep07

JOB NUMBER:
709201

DRAWING NUMBER
S-3

OF 3 SHEETS