



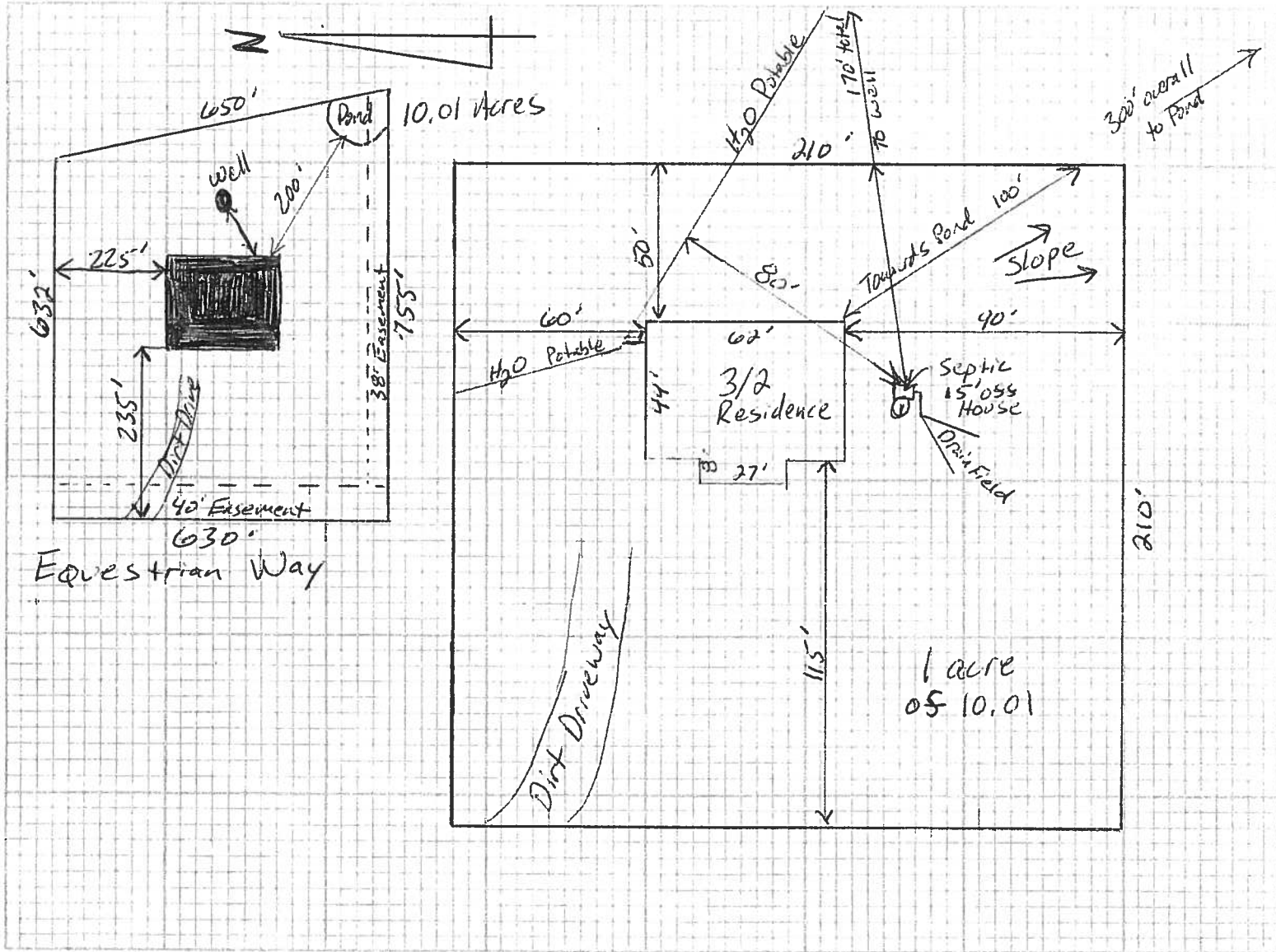
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0957

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: Residence overall Dimensions - 62' x 50' 3 Bedroom 2 Bath

① Septic tank is 15' south of house. Closest Property Line to Septic tank is 200'

Building Dept Permit # 0712-29

Site Plan submitted by: David [Signature] Signature

Plan Approved: **APPROVED** Not Approved

By: [Signature] Date 12/14/7

Columbia CHD County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

- Single Family Dwelling
- Two-Family Residence
- Farm Outbuilding
- Other _____
- Addition, Alteration, Modification or other Improvement

I Michelle Stagg, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Michelle Stagg _____ 12-10-07
Owner Builder Signature Date

FLORIDA NOTARY

The above signer is personally known to me or produced identification FL DL
Notary Signature L. Locken Date 12-10-07



FOR BUILDING DEPARTMENT USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date 12-10-07 Building Official/Representative L. Locken



26518

RE: TOPFLIGHT-STAGG -

Site Information:

Customer Info: TOP FLIGHT Model: STAGG

City: COLUMBIA COUNTY

State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: Robbins Online Plus 22.0.019

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 40.0 psf

This package includes 3 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T3138374	A3	9/5/08
2	T3138375	A4	9/5/08
3	T3138376	A5	9/5/08

The truss drawing(s) referenced above have been prepared by Robbins Engineering, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Schmidt, Lyndon

My license renewal date for the state of Florida is February 28, 2009.

Lyndon F. Schmidt

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Lyndon F. Schmidt, FL Lic #43409
Robbins Engineering
6904 Parke East Blvd
Tampa, FL, 33610
FL Cert.#5555

6904 Parke East Boulevard

Tampa, FL 33610-4115

Phone: 813-972-1135 • Fax: 813-971-6117

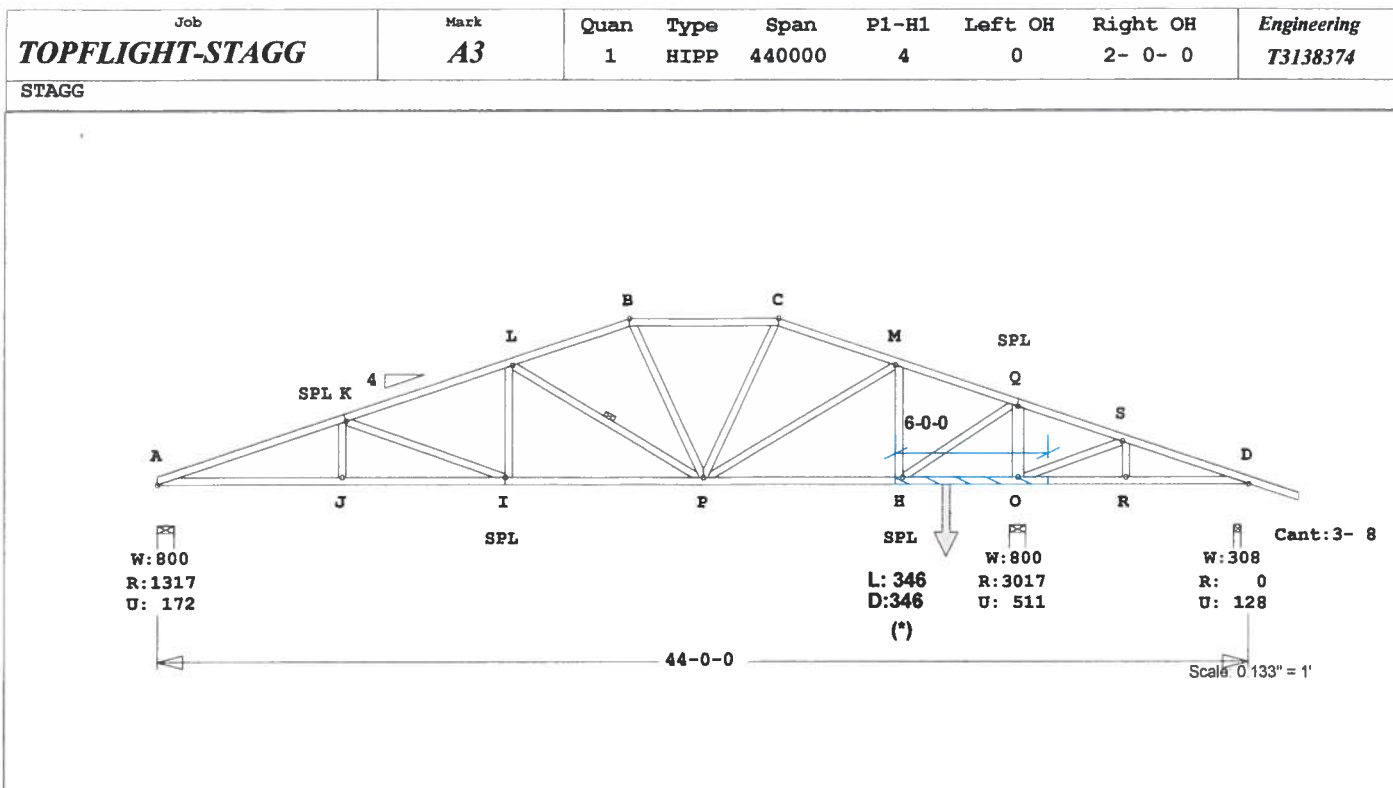
www.robbseng.com

September 5, 2008

DALLAS

TAMPA

Schmidt, Lyndon



For all lumber, plates, etc. not shown, refer to page 2 of this drawing.

Repair is based on information received from truss fabricator.

Truss must be in original, undistorted position prior to carrying out the repair.

Provide temporary support to the truss prior to carrying out the repair.

REPAIR PROBLEM:

Need to add a point load to the bottom chord where shown (*).

Point load is from header to adjacent truss. Design of header and connections by others.

REPAIR SOLUTION:

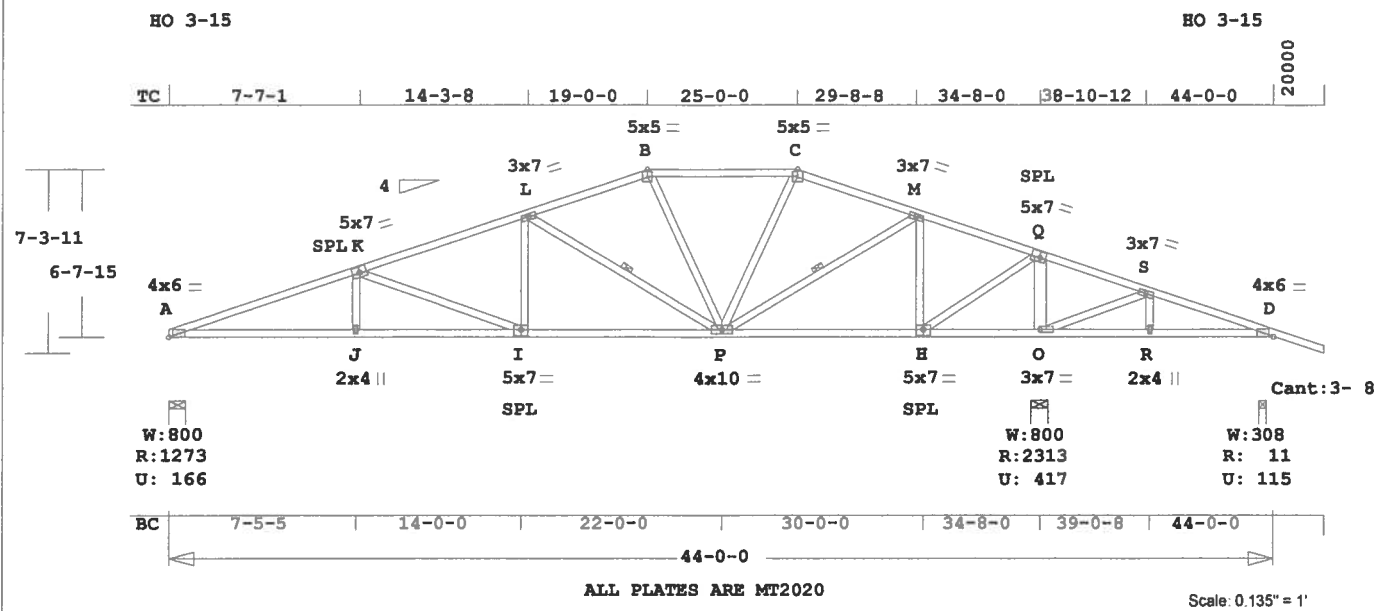
Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.

(A) Attach 2x4 SP-#2 scab (shaded) to (ONE) face, using (2 rows) of 10d common nails at 6" on center each row staggered.

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 Robbins Engineering
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 Tampa, FL, 33610
 FL Cert.#5555

Job TOPFLIGHT-STAGG	Mark A3	Quan 1	Type HIPP	Span 440000	P1-H1 4	Left OH 0	Right OH 2- 0- 0	Engineering T3138374
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STAGG



ALL PLATES ARE MT2020

Scale: 0.135" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 296.3 LBS

Online Plus -- Version 22.0.019
RUN DATE: 05-SEP-08

CSI	Size	Lumber
TC	0.60 2x 4	SP-#2
BC	0.68 2x 4	SP-#2
WB	0.67 2x 4	SP-#2
--	0.21 2x 6	SP-#2
O	-Q	

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 44- 0- 0
BC Cont. 0- 0- 0 44- 0- 0
One Continuous Lateral Brace
L -P P -M
Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (lbs)			
Jt	Down	Uplift	Horiz-
A	1274	166 U	102 R
O	2314	418 U	
D	11	115 U	101 R

Jt	Brg Size	Required
A	8.0"	1.5"
O	8.0"	2.6"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

Top Chords			
A	K	L	B
-K	0.60	3091	C 0.16 0.44
-L	0.45	2241	C 0.03 0.42
-B	0.31	1347	C 0.09 0.22
-C	0.32	1323	C 0.01 0.31
-M	0.26	1348	C 0.09 0.17
-Q	0.21	528	C 0.00 0.21
-S	0.45	1319	T 0.21 0.24
-D	0.36	696	T 0.12 0.24

-----Bottom Chords-----

A	-J	0.68	2938	T	0.49	0.19
J	-I	0.63	2938	T	0.49	0.14
I	-P	0.57	2123	T	0.22	0.35
P	-H	0.40	501	T	0.05	0.35
H	-O	0.23	1252	C	0.00	0.00
O	-R	0.15	648	C	0.00	0.00
R	-D	0.17	648	C	0.00	0.00

-----Webs-----

J	-K	0.04	274	T	
K	-I	0.67	870	C	
I	-L	0.08	513	T	
L	-P	0.33	992	C	1 Br
P	-B	0.03	219	T	
B	-C	0.03	217	T	
C	-M	0.16	907	T	1 Br
M	-H	0.35	1016	C	
H	-Q	0.40	2093	T	
Q	-O	0.21	2002	C	
O	-S	0.20	646	C	
S	-R	0.03	211	T	

TL Defl -0.33" in I -P L/999
LL Defl -0.15" in J -I L/999
Shear // Grain in A -K 0.27

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.72
K	MT20	5.0x 7.0	-0.2	0.5	0.38
L	MT20	3.0x 7.0	Ctr	Ctr	0.38
B	MT20	5.0x 5.0	Ctr	Ctr	0.51
C	MT20	5.0x 5.0	Ctr	Ctr	0.51
M	MT20	3.0x 7.0	Ctr	Ctr	0.35
Q	MT20	5.0x 7.0	-0.3	0.7	0.96
S	MT20	3.0x 7.0	Ctr	Ctr	0.21
D	MT20	4.0x 6.0	Ctr	0.1	0.40
J	MT20	2.0x 4.0	Ctr	Ctr	0.29
I	MT20	5.0x 7.0	Ctr	-0.5	0.58
P	MT20	4.0x10.0	Ctr	Ctr	0.31
H	MT20	5.0x 7.0	Ctr	-0.5	0.54
O	MT20	3.0x 7.0	Ctr	Ctr	0.40
R	MT20	2.0x 4.0	Ctr	Ctr	0.29

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

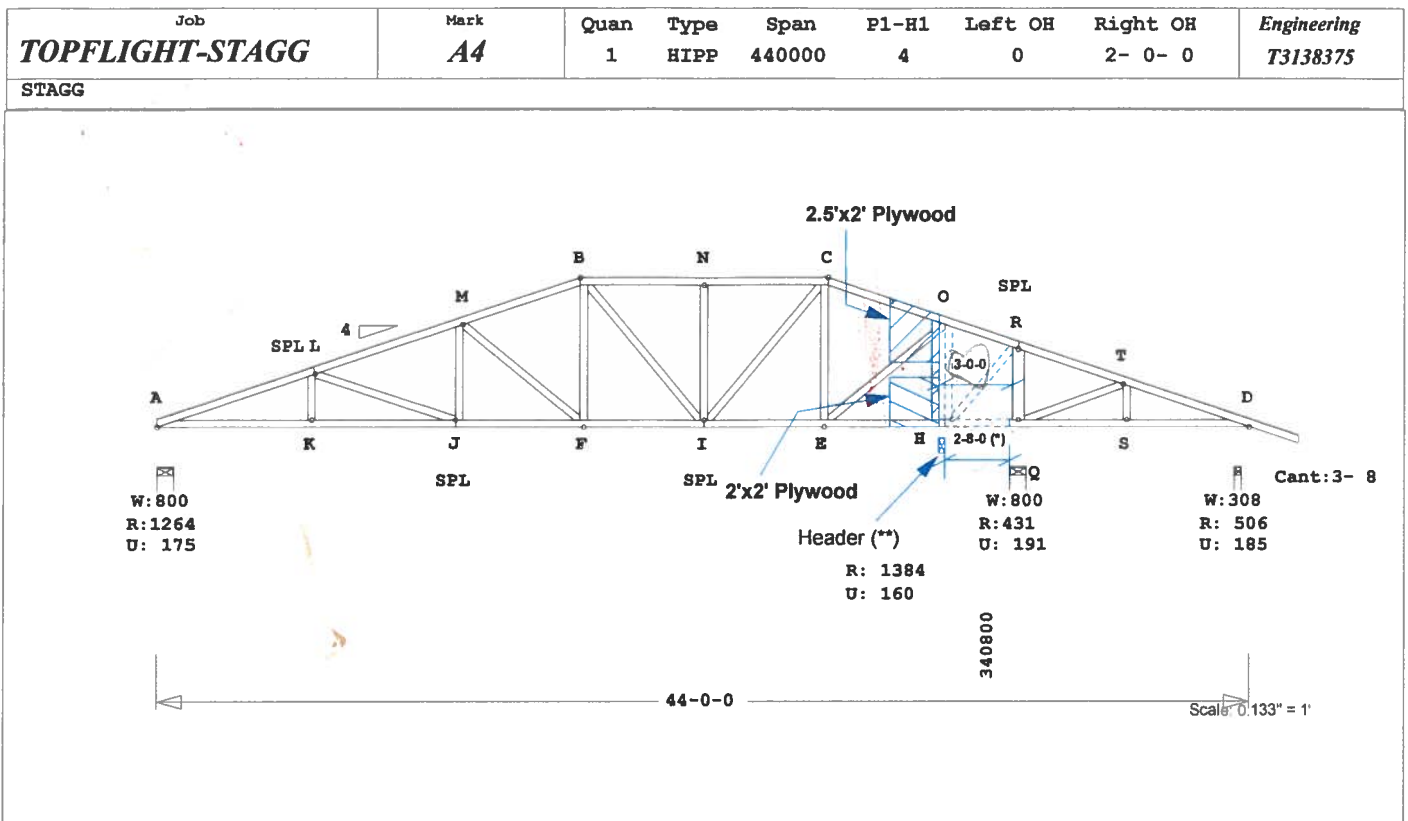
REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss Design
and
for
location.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
User-defined wind-exposed BC
regions --From-- --To--
34- 8- 0 43- 8- 8
Max comp. force 3091 Lbs
Max tens. force 2938 Lbs
Quality Control Factor 1.25

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Robbins Engineering
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FL Cert. #5555

Sept. 5, 2008



For all lumber, plates, etc. not shown, refer to page 2 of this drawing.
 Repair is based on information received from truss fabricator.
 Truss must be in original, undeflected position prior to carrying out the repair.
 Provide temporary support to the truss prior to carrying out the repair.

REPAIR PROBLEM:

Need to cut a portion of the bottom chord where shown dashed (*).
 Additional supports (**) provided from header to adjacent trusses.
 Design and connection of header is by others.

REPAIR SOLUTION:

Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.

- (A) Use a carbide tipped blade to cut and remove portion of truss shown dashed.
 All remaining plates must be intact and fully embedded.
- (B) Install 2x4 SP-#2 member (+) using 1/2" CDX plywood or structurally equivalent material applied to (EACH) face of truss using 8d common nails as per the {Plywood Nailing Schedule}.
 Installed members must have complete wood to wood contact with original members.

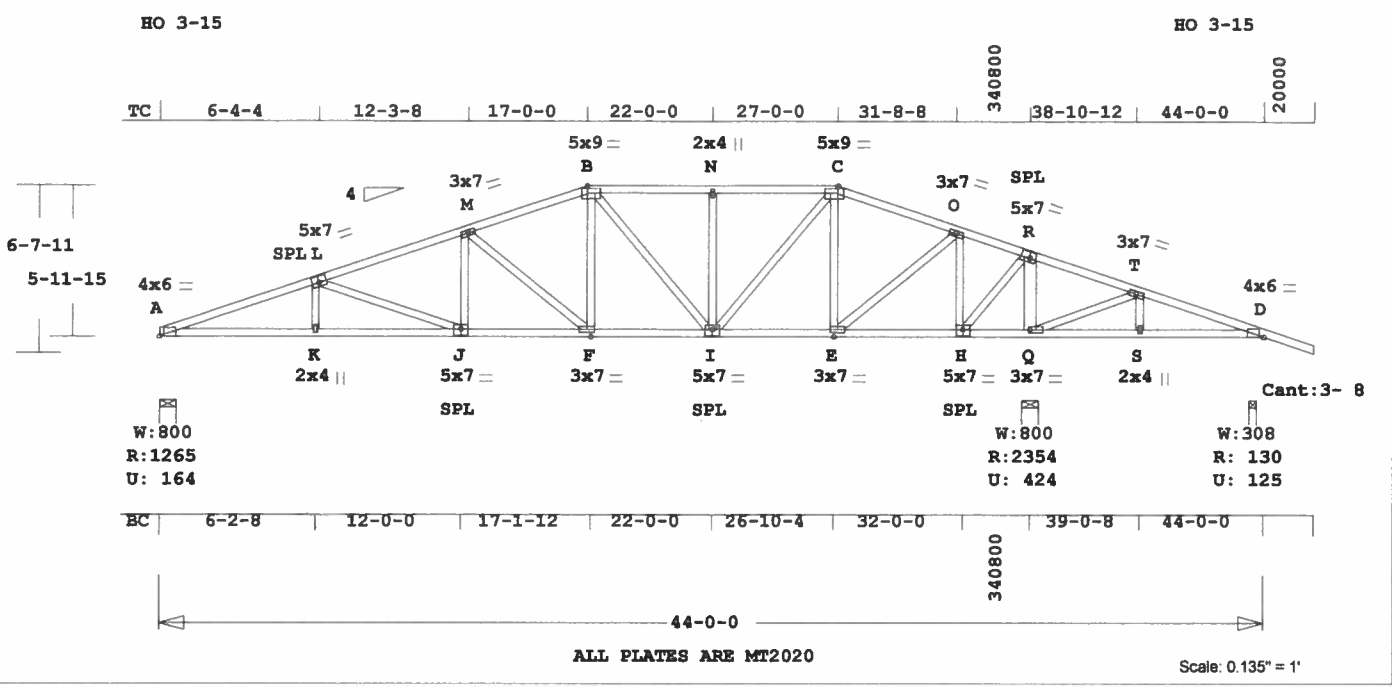
{Plywood Nailing Schedule}

- 2x4 ----- (1-row) @ 2" o.c.
- 2x6 ----- (2-rows) @ 2" o.c.

Stagger each row when two or more rows are required.

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 Tampa, FL, 33610
 FL Cert.#5555

STAGG



ALL PLATES ARE MT2020

Scale: 0.135" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 309.7 LBS

Online Plus -- Version 22.0.019
 RUN DATE: 05-SEP-08

CSI	Size	Lumber
TC	0.49 2x 4	SP-#2
BC	0.61 2x 4	SP-#2
WB	0.47 2x 4	SP-#2
--	0.22 2x 6	SP-#2
Q	-R	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	44- 0- 0
BC Cont.	0- 0- 0	44- 0- 0

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (lbs)

Jt	Down	Uplift	Horiz
A	1265	165 U	91 R
Q	2354	424 U	
D	130	125 U	90 R

Jt	Brg Size	Required
A	8.0"	1.5"
Q	8.0"	2.7"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)
 Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -L	0.37	3156 C	0.17	0.20
L -M	0.34	2449 C	0.07	0.27
M -B	0.31	1825 C	0.11	0.20
B -N	0.24	1483 C	0.01	0.23
N -C	0.24	1483 C	0.01	0.23
C -O	0.24	932 C	0.06	0.18
O -R	0.20	89 T	0.01	0.19
R -T	0.49	1430 T	0.23	0.26
T -D	0.39	782 T	0.13	0.26
-----Bottom Chords-----				
A -K	0.61	2997 T	0.50	0.11
K -J	0.60	2997 T	0.50	0.10
J -F	0.48	2323 T	0.39	0.09
F -I	0.34	1719 T	0.28	0.06

I -E	0.22	891 T	0.14	0.08
E -B	0.13	196 T	0.00	0.13
H -Q	0.12	1361 C	0.09	0.03
Q -S	0.15	728 C	0.00	0.15
S -D	0.17	728 C	0.00	0.17
-----Webs-----				
K -L	0.03	230 T		
L -J	0.42	714 C		
J -M	0.06	407 T		
M -F	0.47	765 C		
F -B	0.10	577 T		
B -I	0.30	363 C		
I -N	0.15	316 C		
I -C	0.32	907 T		
E -C	0.33	679 C		
E -O	0.28	1239 T		
H -O	0.35	1359 C		
H -R	0.35	1918 T		
Q -R	0.22	2043 C		
Q -T	0.21	678 C		
S -T	0.03	215 T		
TL Defl	-0.33"	in K -J	L/999	
LL Defl	-0.16"	in K -J	L/999	
Shear // Grain		in L -M	0.22	

Plates for each ply each face.

Plate	MT20	20 Ga	Gross Area	
Plate	MT2H	20 Ga	Gross Area	
Jt Type	Plt Size	X	Y	JSI
A	MT20	4.0x	6.0	Ctr 0.1 0.74
L	MT20	5.0x	7.0	0-0.2 0.5 0.38
M	MT20	3.0x	7.0	Ctr Ctr 0.33
B	MT20	5.0x	9.0	Ctr Ctr 0.46
N	MT20	2.0x	4.0	Ctr Ctr 0.29
C	MT20	5.0x	9.0	Ctr Ctr 0.46
O	MT20	3.0x	7.0	Ctr Ctr 0.56
R	MT20	5.0x	7.0	0-0.3 0.7 0.92
T	MT20	3.0x	7.0	Ctr Ctr 0.22
D	MT20	4.0x	6.0	Ctr 0.1 0.40
K	MT20	2.0x	4.0	Ctr Ctr 0.29
J	MT20	5.0x	7.0	Ctr-0.5 0.59
F	MT20	3.0x	7.0	Ctr Ctr 0.23
I	MT20	5.0x	7.0	Ctr-0.5 0.48
E	MT20	3.0x	7.0	Ctr Ctr 0.45
H	MT20	5.0x	7.0	Ctr-0.5 0.59
Q	MT20	3.0x	7.0	Ctr Ctr 0.41
S	MT20	2.0x	4.0	Ctr Ctr 0.29

REVIEWED BY:
 Robbins Engineering, Inc.
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 Tampa, FL 33610

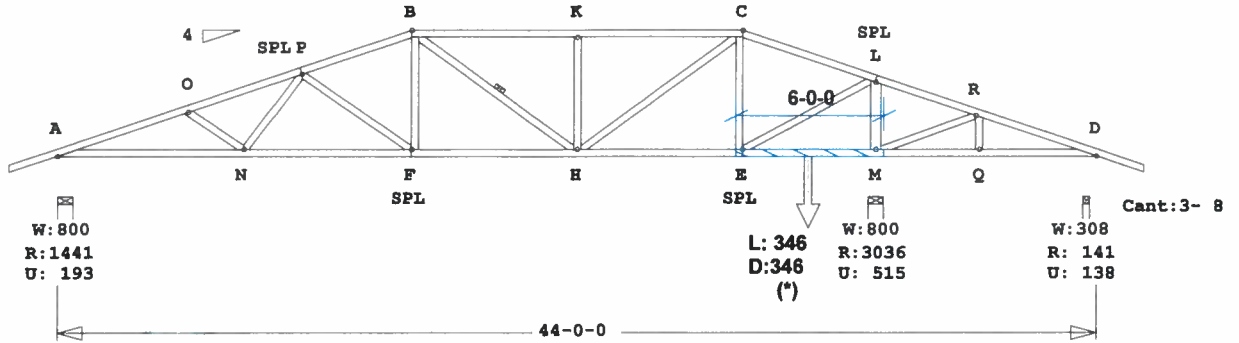
REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:
 Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 cy. current LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor: 1.00
 Building Type: Enclosed
 TC Dead Load: 5.0 psf
 BC Dead Load: 5.0 psf
 User-defined wind-exposed BC
 regions --From-- ---To---
 34- 8- 0 43- 8- 8
 Max comp. force 3156 Lbs
 Max tens. force 2997 Lbs
 Quality Control Factor 1.25

Job TOPFLIGHT-STAGG	Mark A5	Quan 1	Type HIPP	Span 440000	P1-H1 4	Left OH 2- 0- 0	Right OH 2- 0- 0	Engineering T3138376
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STAGG



For all lumber, plates, etc. not shown, refer to page 2 of this drawing.
 Repair is based on information received from truss fabricator.
 Truss must be in original, undeflected position prior to carrying out the repair.
 Provide temporary support to the truss prior to carrying out the repair.

REPAIR PROBLEM:

Need to add a point loads to the bottom chord where shown (*).
 Point load is from header to adjacent truss. Design of header and connections by others.

REPAIR SOLUTION:

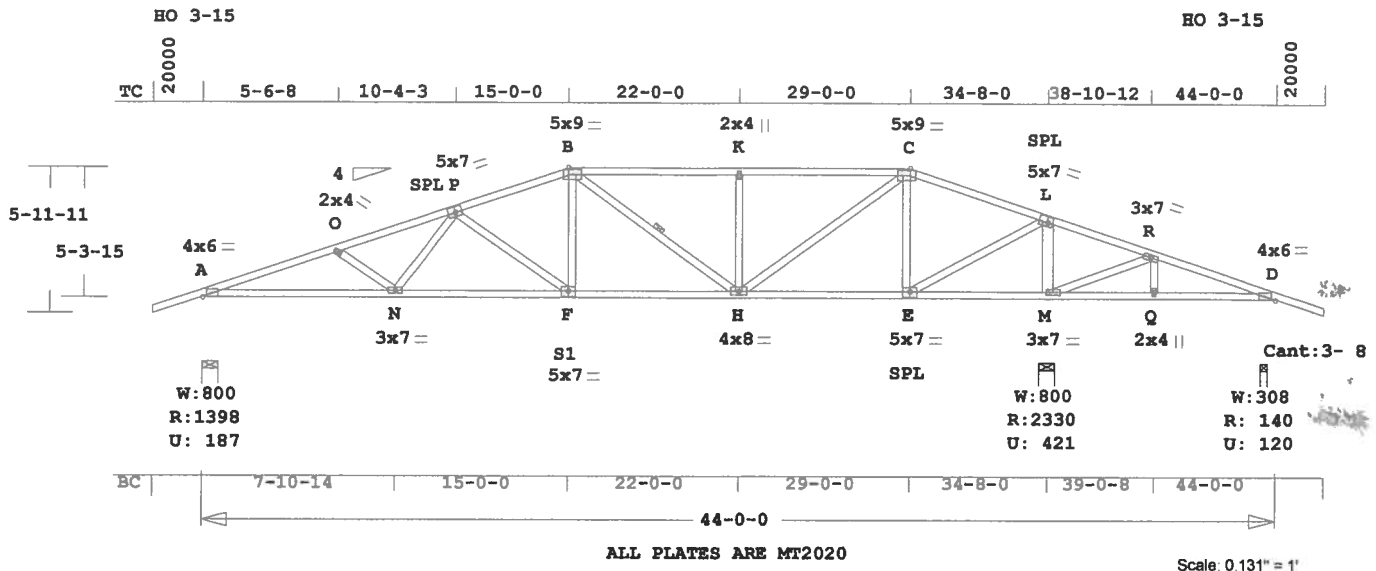
Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
 (A) Attach 2x4 SP-#2 scab (shaded) to (ONE) face, using (2 rows) of 10d common nails at 6" on center each row staggered.

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September 5,2008

Job TOPFLIGHT-STAGG	Mark A5	Quan 1	Type HIPP	Span 440000	P1-H1 4	Left OH 2- 0- 0	Right OH 2- 0- 0	Engineering T3138376
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STAGG



ALL PLATES ARE MT2020

Scale: 0.131" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 289.6 LBS

Online Plus -- Version 22.5.068
RUN DATE: 05-SEP-08

CSI	Size	Lumber
TC	0.56 2x 4	SP-#2
BC	0.64 2x 4	SP-#2
WB	0.60 2x 4	SP-#2
--	0.22 2x 6	SP-#2
M	-L	

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 44- 0- 0
BC Cont. 0- 0- 0 44- 0- 0
One Continuous Lateral Brace
B -H
Attach CLB with (2)-10d nails
at each web.

psf-Ld	Dead	Live
TC	10.0	20.0
BC	10.0	0.0
TC+BC	20.0	20.0
Total	40.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC	Fb=1.15	Fc=1.10 Ft=1.10
BC	Fb=1.10	Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)			
Jt	Down	Uplift	Horiz-
A	1398	188 U	80 R
M	2331	421 U	
D	140	121 U	80 R

Jt	Brg Size	Required
A	8.0"	1.6"
M	8.0"	2.6"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A	-O	0.30	3189 C	0.17 0.13
O	-P	0.31	2894 C	0.16 0.15
P	-B	0.32	2104 C	0.02 0.30
B	-K	0.50	1736 C	0.01 0.49
K	-C	0.50	1736 C	0.01 0.49
C	-L	0.33	691 C	0.00 0.33
L	-R	0.56	1338 T	0.23 0.33
R	-D	0.34	744 T	0.13 0.21
-----Bottom Chords-----				

A	-N	0.64	3027 T	0.50	0.14
N	-S1	0.56	2503 T	0.42	0.14
S1	-H	0.47	1991 T	0.33	0.14
H	-E	0.30	666 T	0.06	0.24
E	-M	0.22	1265 C	0.00	0.22
M	-Q	0.15	695 C	0.00	0.15
Q	-D	0.18	695 C	0.00	0.18
-----Webs-----					
O	-N	0.06	347 C		
N	-P	0.07	461 T		
P	-S1	0.33	620 C		
S1	-B	0.09	525 T		
B	-H	0.09	313 C	1 Br	
H	-K	0.17	450 C		
K	-C	0.60	1319 T		
C	-E	0.35	905 C		
E	-L	0.51	2173 T		
L	-M	0.22	2014 C		
M	-R	0.19	617 T		
R	-Q	0.03	201 T		

TL Defl -0.37" in N -S1 L/999
LL Defl -0.17" in N -S1 L/999
Shear // Grain in B -K 0.31

Plates for each ply each face.						
Plate	Size	Gross Area	Jt Type	Plt Size	X	Y JSI
A	MT20	4.0x 6.0	Ctr	0.1	0.75	
O	MT20	2.0x 4.0	Ctr	0.22		
P	MT20	5.0x 7.0	0.2	0.5	0.38	
B	MT20	5.0x 9.0	Ctr	Ctr	0.46	
K	MT20	2.0x 4.0	Ctr	Ctr	0.29	
C	MT20	5.0x 9.0	Ctr	Ctr	0.46	
L	MT20	5.0x 7.0	0.3	0.7	0.98	
R	MT20	3.0x 7.0	Ctr	Ctr	0.19	
D	MT20	4.0x 6.0	Ctr	0.1	0.40	
N	MT20	3.0x 7.0	0.1	Ctr	0.21	
S1	MT20	5.0x 7.0	Ctr	0.5	0.49	
H	MT20	4.0x 8.0	Ctr	Ctr	0.53	
E	MT20	5.0x 7.0	Ctr	0.5	0.55	
M	MT20	3.0x 7.0	Ctr	Ctr	0.40	
Q	MT20	2.0x 4.0	Ctr	Ctr	0.29	

REVIEWED BY:
Robbins Engineering, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings* for Exterior zone location.

Wind Speed: 110 mph

Main Roof Height: 15-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

User-defined wind-exposed BC regions --From-- --To--

34- 8- 0 43- 8- 8

Max comp. force 3189 Lbs

Max tens. force 3027 Lbs

Quality Control Factor 1.25

Lyndon F. Schmidt, FL Lic #43409
Robbins Engineering
6904 Parke East Blvd
Tampa, FL 33610
FL Cert. #5555

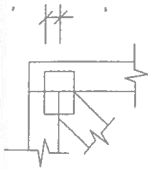
Sept. 5, 2008

ROBBINS ENG. GENERAL NOTES & SYMBOLS

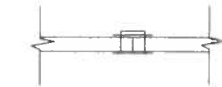
108

PLATE LOCATION

Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)



FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

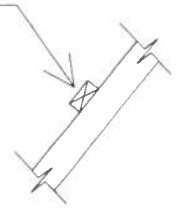


PLATE SIZE AND ORIENTATION

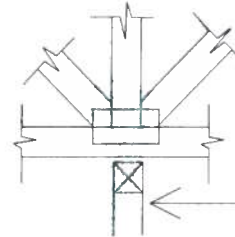
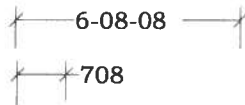
3x5 ||



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6'-8.5" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring **must be** in place before trusses are installed. If necessary, shim bearings to assure **solid** contact with truss.

W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

Metal connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on Truss Design Drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to "Building Component Safety Information" (BCSI 1) as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records. When truss hangers are specified on the Truss Design Drawing, they must be installed per manufacturer's details and specifications.






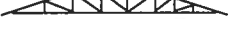
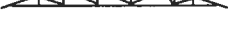






FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS MANUFACTURER.



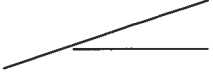
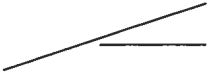
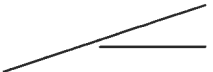


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Qty	Plys	Truss Type	Mark	R e a c t i o n s			Sketch
				Horizontal	Vertical	Uplift	
2	1	TR	TOPFLIGHT-STAGG/ A1	118	1274	166	
					2310	417	
				117	12	113	
1	1	HIPP	TOPFLIGHT-STAGG/ A2	113	1276	166	
					2303	415	
				112	14	112	
1	1	HIPP	TOPFLIGHT-STAGG/ A3	102	1273	166	
					2313	417	
				101	11	115	
1	1	HIPP	TOPFLIGHT-STAGG/ A4	91	1265	164	
					2354	424	
				90	130	125	
1	1	HIPP	TOPFLIGHT-STAGG/ A5	80	1398	187	
					2330	421	
				79	140	120	
1	1	HIPP	TOPFLIGHT-STAGG/ A6	69	1375	184	
					2441	437	
				68	149	168	
2	1	HIPP	TOPFLIGHT-STAGG/ A7	58	1360	181	
					2513	450	
				57	155	225	
2	1	HIPP	TOPFLIGHT-STAGG/ A8	47	1357	183	
					2556	445	
				46	157	265	
2	2	HIPP	TOPFLIGHT-STAGG/ A9GIR	35	2578	347	
					6517	978	
				34	480	1335	
1	1	SP	TOPFLIGHT-STAGG/ A10	119	1277	169	
					2184	389	
				118	185	185	
3	1	SP	TOPFLIGHT-STAGG/ A11	118	1333	199	
					2047	365	
				118	266	168	
3	1	SP	TOPFLIGHT-STAGG/ A12	119	1284	171	
					2153	379	
				118	210	185	
1	1	SP	TOPFLIGHT-STAGG/ A13	114	1286	172	
					2143	377	
				113	217	183	

Qty	Plys	Truss Type	Mark	Reactions			Sketch
				Horizontal	Vertical	Uplift	
1	1	SP	TOPFLIGHT-STAGG/ A14	103	1283	171	
					2158	380	
				102	205	187	
1	1	SP	TOPFLIGHT-STAGG/ A15	92	1410	197	
					2160	363	
				91	204	197	
1	1	SP	TOPFLIGHT-STAGG/ A16	81	1377	190	
					2312	394	
				80	161	221	
1	1	SP	TOPFLIGHT-STAGG/ A17	71	1336	184	
					2500	425	
				69	170	254	
1	1	TR	TOPFLIGHT-STAGG/ B1	59	1221	449	
				59	1221	449	
1	1	TR	TOPFLIGHT-STAGG/ B2	59	1093	426	
				59	1093	426	
1	2	DDHI	TOPFLIGHT-STAGG/ B3GAB	21	2040	553	
				21	2040	553	
2	1	MONO	TOPFLIGHT-STAGG/ CJ1	62	458	135	
					349	14	
				86	233	94	
2	1	MONO	TOPFLIGHT-STAGG/ CJ2	66	466	239	
					342	113	
				86	235	95	
2	1	MONO	TOPFLIGHT-STAGG/ CJ3	42	329	198	
					186	81	
				53	131	52	
28	1	JCA2	TOPFLIGHT-STAGG/ J1	186	408	63	
					130		
				47	188	67	
2	1	JCA2	TOPFLIGHT-STAGG/ J1B	186	601	67	
				47	86	35	
2	1	MONO	TOPFLIGHT-STAGG/ J1C	191	409	136	
					128	44	
				47	188	77	
4	1	JCA2	TOPFLIGHT-STAGG/ J2	154	323	59	
					140	50	
				33	92		

Qty	Plys	Truss Type	Mark	R e a c t i o n s			Sketch
				Horizontal	Vertical	Uplift	
8	1	JCA2	TOPFLIGHT-STAGG/ J2A	159 33	329 137 89	112 56 30	
4	1	JCA2	TOPFLIGHT-STAGG/ J3	108 19	243 86 54	56 30	
8	1	JCA2	TOPFLIGHT-STAGG/ J3A	113 19	250 83 51	89 34 17	
4	1	JCA2	TOPFLIGHT-STAGG/ J4	32 5	157 41 9	54 5 3	
8	1	JCA2	TOPFLIGHT-STAGG/ J4A	34 5	157 41 9	58 6 3	
10	1	JCA2	TOPFLIGHT-STAGG/ J5	165 35	346 96 138	117 33 57	