

STRUCTURAL NOTES

GENERAL NOTES

- The governing Code for this Project is the Florida Building Code, 8th Edition (2023). This Code prescribes which edition of each referenced standard applies to this Project.
- To the best of our knowledge, the Structural Drawings and comply with the applicable requirements of the governing Building Code.
- Construction is to comply with the requirements of the governing Building Code and all other applicable Federal, State, and Local Codes, Standards, Regulations and Laws.
- The Structural Documents are to be used in conjunction with the Architectural Documents. If a conflict exists, the more stringent governs.
- Details labeled "typical" apply to all situations that are the same or similar to those specifically referenced, whether or not they are keyed in at each location. Questions regarding the applicability of typical details shall be resolved by the Architect.
- Contractors who discover discrepancies, omissions or variations in the Contract Documents during bidding shall immediately notify the Architect. The Architect will resolve the condition and issue a written clarification.
- The General Contractor shall coordinate all Contract Documents with field conditions and dimensions and Project Shop Drawings prior to construction. Do not scale drawings; use only printed dimensions. Report any discrepancies in writing to the Architect prior to proceeding with work. Do not change size or location of structural members without written instructions from the Structural Engineer of Record.
- The Contractor shall protect adjacent property, his own work and the public from harm. The Contractor is solely responsible for construction means and methods, and jobsite safety including all OSHA requirements.
- The Structure is designed to be structurally sound when completed. Prior to completion, the Contractor is responsible for stability and temporary bracing, including, but not limited to, masonry walls. Wherever the Contractor is unsure of these requirements, the Contractor shall retain a Florida Licensed Engineer to design and inspect the temporary bracing and stability of the structure.
- Design Superimposed Loads:

OCCUPANCY	LIVE LOAD	DEAD LOAD
Roof	20 psf	15 psf *
Storage / Public Areas	125 psf **	20 psf *

* Does not Include Wood Joists, Beams, Trusses or Decking.
** Roof trusses will **NOT** be designed for Storage Loading.

11. Design Wind Loads

Governing Code	Florida Building Code, 8 th Edition / ASCE 7-22
Basic Wind Speed	V _{ult} = 120 mph V _{asd} = 93 mph
Building Risk Category	II
Wind Exposure	C
Internal Pressure Coefficient	GCpi = +/- 0.18
Design Wind Pressure for Components & Cladding	See Drawing S002
Directionality Factor	K _d = 0.85
Mean Roof Height	13'-0"

12. Design Rain Loads

Governing Code	Florida Building Code, 8 th Edition / ASCE 7-22
Rain Intensity (100 - Year, 1-Hour Rainfall)	4 1/2" / hr
Rain Load	20 psf

SHOP DRAWINGS AND OTHER SUBMITTALS

- Incomplete submittals will be returned without review.
- Submit specific components, such as columns, footings, etc., in a single package. Submit similar floors together.
- On first submittal, clearly flag and cloud all differences from the Contract Documents. On resubmittals, flag and cloud all changes and additions to previous submittal; only clouded items will be reviewed.
- Submittals for special structural, load-carrying items that are required by Codes or Standards to resist forces must be prepared by, or under the direct supervision of, a Delegated Engineer. Examples include Prefabricated Wood Components such as Wood Trusses and Exterior Enclosure Systems.
- A Delegated Engineer is defined as a Florida Licensed Engineer who specializes in and undertakes the design of Structural Components or Structural Systems Included in a specific submittal prepared for this Project and is an employee or officer of, or consultant to, the Contractor or Fabricator responsible for the submittal. The Delegated Engineer shall sign, seal and date the submittal, including calculations and drawings. See Specifications for more specific criteria.
- The Trade Contractor is responsible for confirming and correlating dimensions at the job sites, for tolerances, clearances, quantities, fabrication processes and techniques of construction, coordination of the work with other trades and full compliance with the Contract Documents.
- The General Contractor/Construction Manager shall review and approve submittals and shall sign and date each drawing prior to submitting to the Architect. This approval is to confirm that the Submittal is complete, complies with the Submittal Requirements and is coordinated with field dimensions, other trades, erection sequencing and constructability.
- The Structural Engineer reviews submittals to confirm that the submittal is in general conformance with the design concept presented in the Contract Documents. Quantities and dimensions are not checked. Notations on submittals do not authorize changes to the contract sum. Checking of the submittal by the Structural Engineer shall not relieve the Contractor of responsibility for deviations from the Contract Documents and from errors or omissions in the submittal.
- In addition to the above, the Structural Engineer's review of Delegated Engineer submittals is limited to verifying that the specified structural submittal has been furnished, signed and sealed by the Delegated Engineer and that the Delegated Engineer has understood the design intent and used the specified Structural Criteria. No detailed check of calculations will be made. The Delegated Engineer is solely responsible for his/her design, including but not limited to the accuracy of his/her calculations and compliance with the applicable codes and standards.

EXCAVATION, BACKFILL AND DEWATERING

- The Contractor is solely responsible for all excavation procedures including lagging, shoring, and protection of adjacent property, structures, streets and utilities in accordance with the requirements of the Local Building Department and OSHA regulations. Do not excavate within one foot of the angle of repose of any soil bearing foundation unless the foundation is properly protected against settlement.
- Do not backfill against walls until 7 days after the walls are braced by the structure or are temporarily braced. Do not backfill until after completion and inspection of any waterproofing.
- The Contractor is responsible for the disposal of all accumulated water in a manner that does not inconvenience or damage the work.

SHALLOW FOUNDATIONS

- Footing sizes and reinforcing are based on an assumed allowable soil bearing capacity of 2,000 psf. All footings shall bear on compacted fill, natural soil or rock prepared per the Geotechnical Report.
- Subgrade preparation shall be field controlled and tested by a Licensed Soils Engineer in accordance with the Geotechnical Report. At completion, that Engineer shall prepare and submit to the Owner, Architect, Contractor and Structural Engineer a signed and sealed letter indicating that the recommendations of the Geotechnical Report have been followed.
- Center all footings under their respective columns or walls, u.o.n.

REINFORCED CONCRETE

- Comply with ACI 301 and 318.
- Provide structural concrete with a minimum ultimate compressive design strength in 28 days as follows:

Element	Strength
Footings	3,000 psi
Thicken Slabs / Slabs on Grade	3,000 psi
- Contractor shall provide one set of concrete test cylinders for each placement of concrete to verify the concrete strength. The four (4) test cylinders should be provided and tested as follows:
 - Test Cylinder break at 7 days
 - Test Cylinders break at 28 days
 - Test Cylinder break at 54 days or hold
- Use normal weight concrete for all structural members. u.o.n.
- Provide ASTM A-615 Grade 60 reinforcing steel. Reinforcing shall be accurately placed, rigidly supported and firmly tied in place, with appropriate bar supports and spacers. Lap continuous reinforcing 48 bar dia. Lap bottom steel over supports and top steel at midspan (u.o.n.). Hook discontinuous ends of all top bars and all bars in walls, u.o.n. provide cover over reinforcing as follows:

Element	Bottom	Top	Sides
Footings	3"	2"	3"
Thicken Slabs / Slabs on Grade	2"	1"	2"
- Where specified, provide plain, cold-drawn electrically-welded wire reinforcement conforming to ASTM A-185. Supply in flat sheets only. Lap splice one cross wire spacing plus two inches.
- Utilities shall not penetrate beams or columns but may pass through slabs and walls individually, uon. For openings 24" long or less, cut reinforcing and replace alongside opening with splice bars of equivalent area with 48 bar dia. lap. Prepare and submit shop drawings for openings longer than 24". For rectangular openings 12" long or longer, add 1#5 x 6" mid depth diagonal at all 4 corners.
- Where reinforcing steel congestion permits, conduit and pipes up to 1" diameter may be embedded in concrete per ACI 318, Section 6.3. Space at 3 diameters o.c. Place between outer layers of reinforcing if conduits are significantly congested, additional reinforcing perpendicular to piping may be required. Requests to embed larger pipes should be accompanied by a detailed description and be submitted to the architect for evaluation.
- Provide 3/4" chamfer for all exposed corners.
- Provide reinforcing steel placer with a set of Structural Drawings for field reference. Inspect reinforcing steel placing from structural drawings.

CONCRETE ANCHORS

- Use Simpson Strong-Tie Titen HD anchors or accepted equivalent for anchors to concrete. Follow manufacturer's specifications for use and installation. See details for quantities, sizes and lengths.
- Confirm the absence of reinforcing steel by drilling a 1/4" diameter pilot hole for each anchor. Do not cut reinforcing steel without approval of the Structural Engineer.
- Provide anchor embedment, spacing and edge distance as shown on the Drawings.

SLABS ON GRADE

- Refer to Geotechnical Report for subgrade preparation more than 12" below bottom of slab.
- Above subgrade, use fill containing not more than 10% passing #200 sieve and maximum 1 inch diameter. Compact to 95% of maximum dry density as determined by modified proctor ASTM D-1557. Each layer of fill shall not exceed 6" loose thickness. Compact prior to placement of the next layer.
- Fill placement and compaction shall be monitored and accepted by the testing agency. Take a min. of one field density test (ASTM D-1556 or D-2922) for each 2,500 square feet of each layer. The testing agency shall randomly select test locations.
- For interior slabs place 10 mil polyethylene sheeting between soil and bottom of slab. Do not use any sheeting below exterior concrete slabs.
- Use 4" thick slabs on grade reinforced with 6 x 6 - W2.0 x W2.0 welded wire reinforcement supplied in flat sheets only. Use chairs to support wire fabric in the center of slab.
- Place concrete in long-strip construction method. Provide crack control joints at 10'-0" feet maximum to limit areas between joints to 100 sq. ft. in all floating slabs on grade. Locate to conform to bay spacing whenever possible, add crack control joints at re-entrant corners which tend to invite cracks.
- In sidewalks and walkways, locate isolation joints at 20 ft. o.c. maximum score and tool between isolation joints in equal bays of 5 ft. or less.
- See the Architectural Drawings for slab on grade depressions and other requirements.

WOOD CONSTRUCTION

- All wood construction and connections shall conform to AITC "American Institute of Timber Construction" Manual, and the "National Design Specifications" for Wood Construction and Florida Building Code, Chapter 23.
- All member sizes are to be as shown on drawing and provide the following minimum properties:

Member	Species	Fb (PSI)	E (PSI)
Joists	So. Pine No. 2	1200	1,600,000
Studs	So. Pine No. 2	1250	1,600,000
Columns	So. Pine No. 1	1350	1,500,000
- All wood in contact with concrete, masonry or exposed to weather shall be pressure treated.
- All bolts and bolted connections shall conform to ASTM A307 and shall be galvanized. Use washers between wood and all bolt heads and nuts.
- All metal wood connectors shall be galvanized and shall be manufactured by Simpson Strong Tie Co., or approved equal. All fasteners exposed to weather shall be stainless steel or designed by the manufacturer to resist corrosion.
- All joists shall be laterally supported at ends by solid blocking.
- Stud walls shall be 2 X 6 at 16" o.c..
- Unless noted otherwise on plan, provide double studs at all jambs of openings up to 6'-0". Use triple studs for openings greater than 6'-0" and less than 10'-0".
- Where beams or columns are formed of two (2) or more members, they shall be full length and fastened together per Chapter 23 of the Florida Building Code or as shown in the structural details.
- Exterior walls shall have 5/8" CDX or better plywood nailed with 10d ring shank nails at 6" o.c. maximum at all panel edges. Space nails at 12" o.c. maximum along intermediate framing. See wood shear walls information on plans for nailing requirements.
- Plywood flooring shall be 3/4" CDX tongue and groove with 10d x 2 1/2" ring shank nails at 6" o.c. maximum at all supported edges. Space nails 12" o.c. along intermediate framing members. Apply glue to all edges and intermediate framing members.

PRE-ENGINEERED WOOD TRUSSES

- Design and fabricate all timber in conformance with the "National Design Specification For Stress-Grade Lumber And Its Fastenings" and Florida Building Code, 8th Edition (2023), and the National Design Standard for metal plate connected wood truss construction.
- In accordance with Rule 61G15-31.003 of the Florida Administrative Code, the Truss System Engineer, a Delegated Engineer, shall design the Truss System. The Truss System Engineer shall submit shop drawings and calculations for review to Architect/Engineer for the assembly of prefabricated, engineered wood trusses and truss girders, together with all bracing, connections and other structural elements and all spacing and location criteria (truss placement plan), that, in combination, function to support the dead, live and wind loads applicable to the roof Truss System. The Truss System does not include walls, or any other structural support systems. These shop drawings and calculations shall be signed and sealed by the Truss System Engineer.
- In accordance with Rule 61G15-31.003 of the Florida Administrative Code, the Truss Design Engineer, a Delegated Engineer, shall design the individual trusses of the Truss System, but does not design the Truss System. The Truss Design Engineer shall submit shop (piece) drawings and calculations for each different truss of the Truss System such that each truss will function to support the dead, live and wind loads applicable to each truss and truss girder that together comprise the Truss System. These shop drawings and calculations shall be signed and sealed by the Truss Design Engineer.
- The Truss System Engineer and the Truss Design Engineer shall each be responsible for their own work. However, they may be the same individual providing two separate services.
- The loads, layouts and connections provided on the structural construction documents are the minimums to be followed by the Truss System Engineer and the Truss Design Engineer. The Truss Design Engineer is responsible for the truss to truss connections.
- Use stress-rated timber for all wood structural members. Do not use wood structural members with a bending stress less than 1200 psi or a modulus of elasticity less than 1,400,000 psi when used at 19% maximum moisture content.
- Pressure treatment of all structural lumber shall be in accordance with AWPA Standards C1 and C2, latest Editions with a waterborne preservative in accordance with Standard P5, latest Edition. All lumber to be kiln-dried after treatment to a moisture content not to exceed 19% oven-dry basis, per Standard C2. All lumber 4 x 4 in. (nominal size) and larger in dimension to be treated to the soil and fresh water retention and penetration requirements of Standard C2. All lumber less than 4 x 4 (nominal size) to be treated to the above ground requirements of Standard C2.
- Minimum design loads for roof trusses:

20 psf LL top chord
15 psf DL top chord
15 psf DL bottom chord

Wind pressure based on ASCE-7. See General Notes and Drawing S002.
- The design and erection of wood trusses, including bracing, shall conform to the commentary and recommendations of the truss plate institute. In addition to continuous lateral bracing of top and bottom chords (designed by the Truss System Engineer, but spaced not more than 10' o.c.) provide diagonal bracing (minimum 2" thick nominal lumber) as follows:
 - In the plane of the top chord: locate between lateral bracing, set at 45 degree angles, see drawings for intervals.
 - In the plane of the web members (perpendicular to the trusses): at each web member requiring continuous lateral bracing but not more than 16' intervals, spacing between sets of diagonals shall not exceed 20' or twice the horizontal run of the diagonal.
 - In the plane of the bottom chord: place between continuous lateral bracing at 45 degree angle at each end of building.

SHEET NUMBERING SYSTEM

S001 Series:	Notes
S002 Series:	Wind Loading
S100 Series:	Plans
S200 Series:	Foundation & Concrete Details
S300 Series:	Wood Details



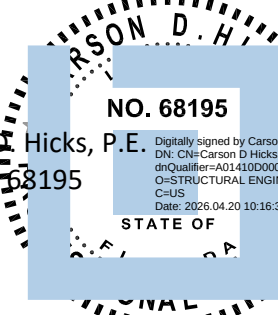
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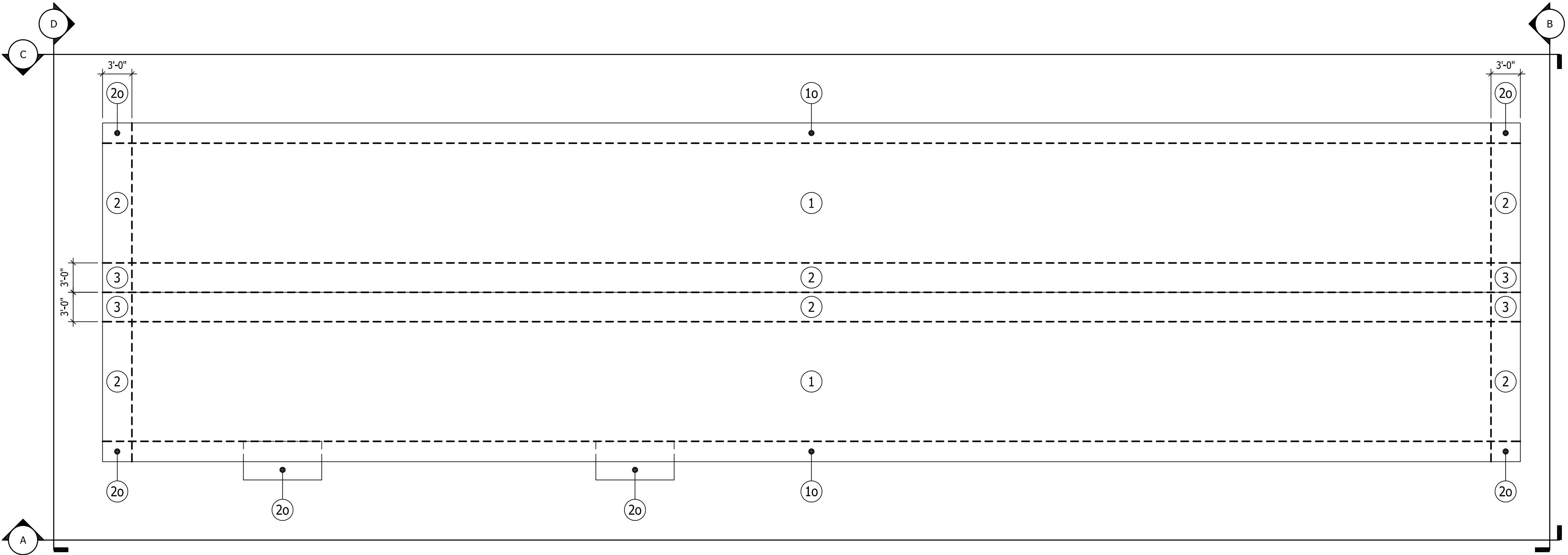
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APPROVED CDH
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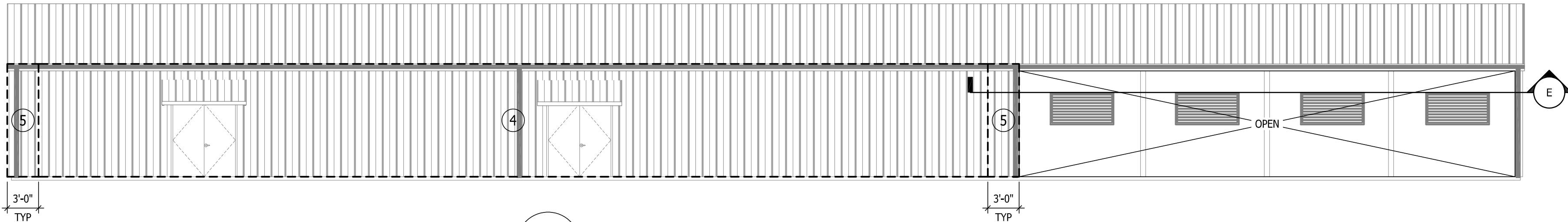
STRUCTURAL
NOTES

S001

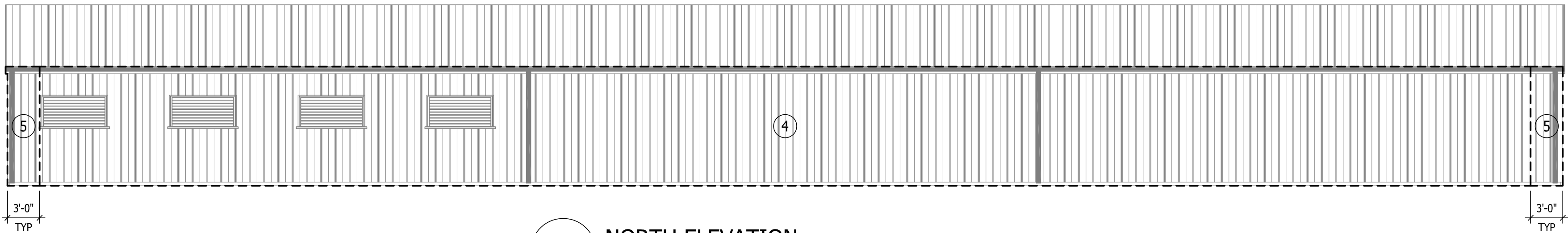
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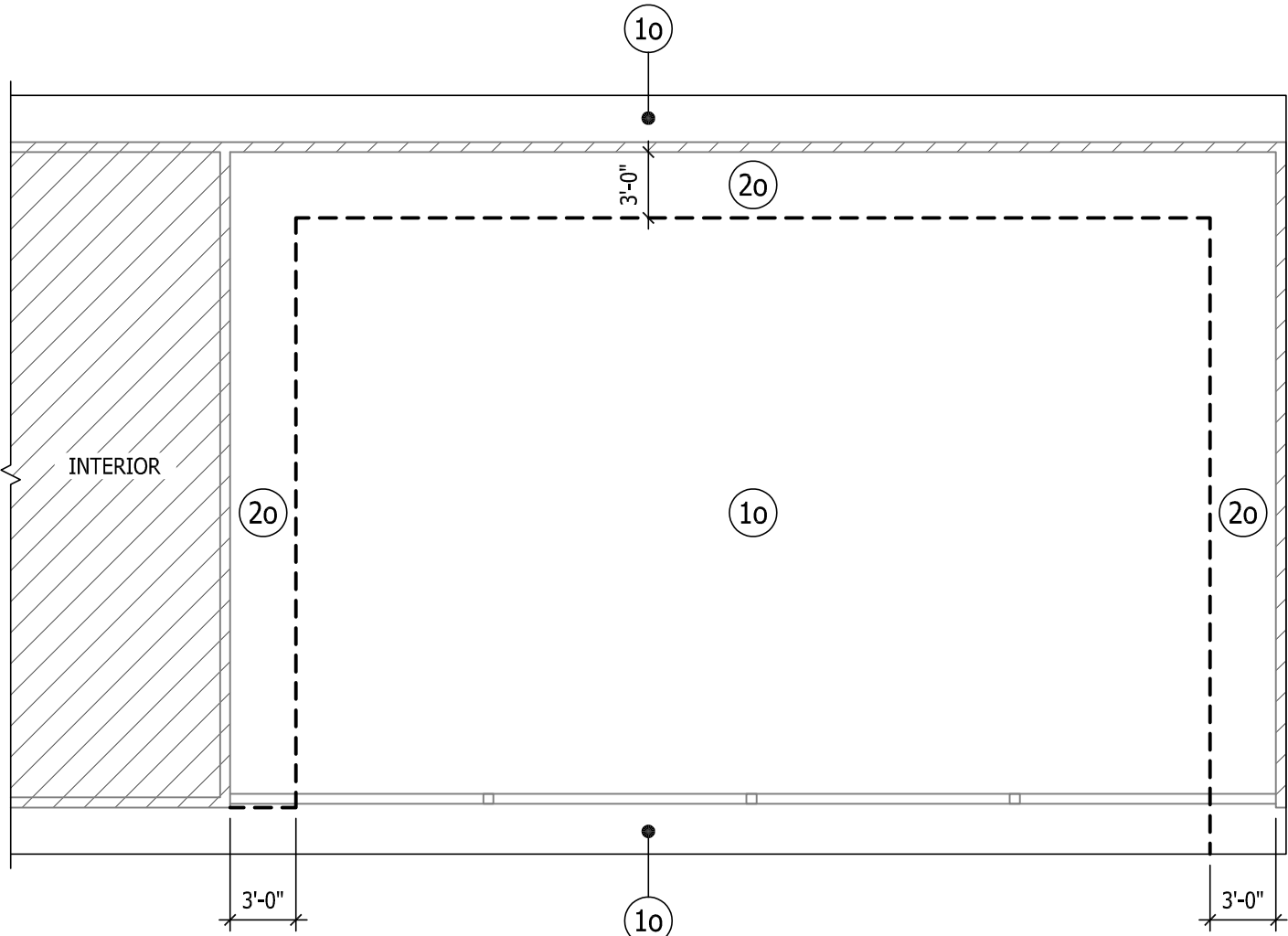
BUILDING ROOF PLAN
GABLE ROOD 7° < ANGLE < 27°
SCALE: 1/8"=1'-0"



A SOUTH ELEVATION



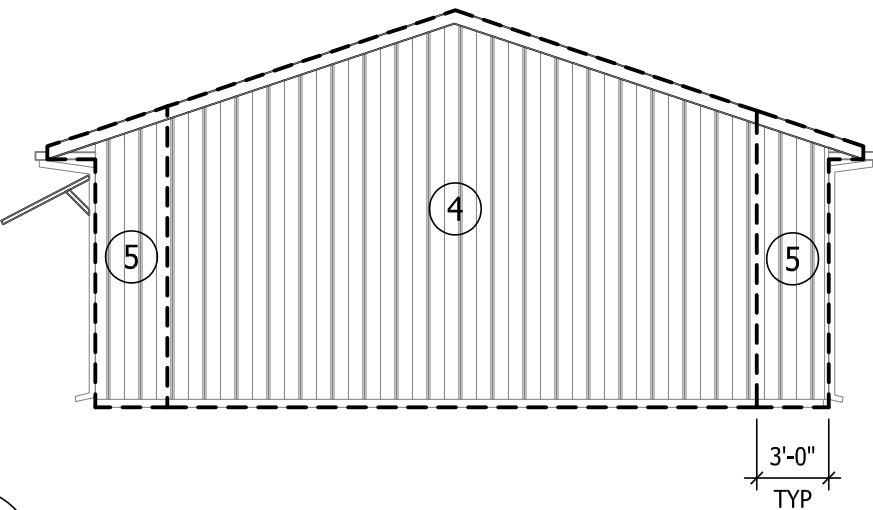
C NORTH ELEVATION



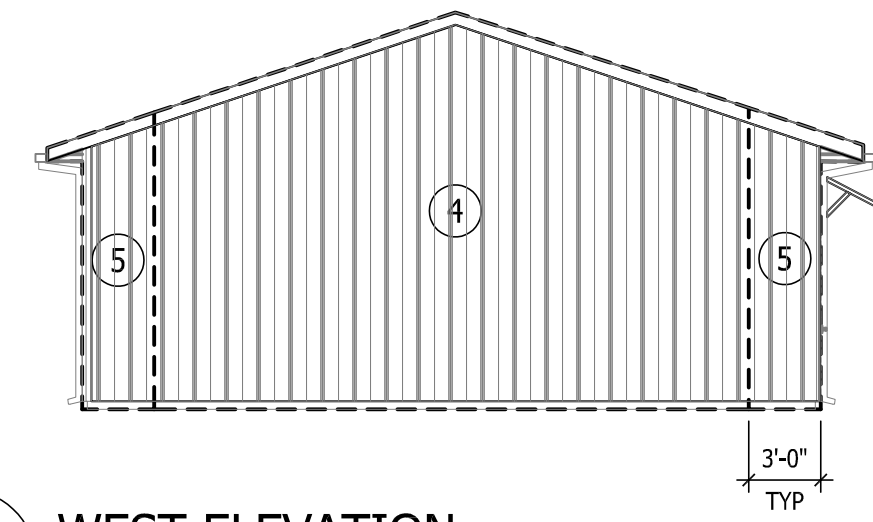
E BUILDING SOFFIT

COMPONENTS AND CLADDING GABLE ROOF & WALL DESIGN WIND PRESSURES					
ZONES	TRIBUTARY AREA (SF)				
	10	20	50	100	200
①	-60/25	-60/20	-40/20	-35/20	-25/20
⑩	-105	-90	-75	-65	-50
②	-80/25	-70/20	-55/20	-45/20	-35/20
②0	-110	-100	-80	-70	-55
③	-105/25	-90/20	-70/20	-55/20	-55/20
④	+35	+35	-35/30	+30	+30
⑤	-45/35	-40/35	-40/30	-35/30	+30

- WIND PRESSURE NOTES
- The design pressures listed above are Ultimate Design Pressures as determined by ASCE 7-22. Numbers on this sheet are the components and cladding gross factored load pressures perpendicular to the surface (in psf) based on tributary area. To obtain service level pressures, use the Allowable Stress Design basic load combinations in ASCE 7-22.
 - Pressures are derived from ASCE 7-22.
 - Negative pressures act away from surface, positive pressures act toward surface.
- e = Edge of Roof
n = Edge of Gable Roof
r = Roof Ridge
o = Roof overhangs
p = Parapets
h = 13'-0"



B EAST ELEVATION



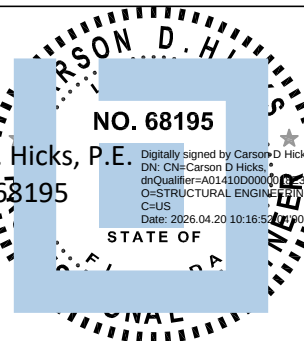
D WEST ELEVATION

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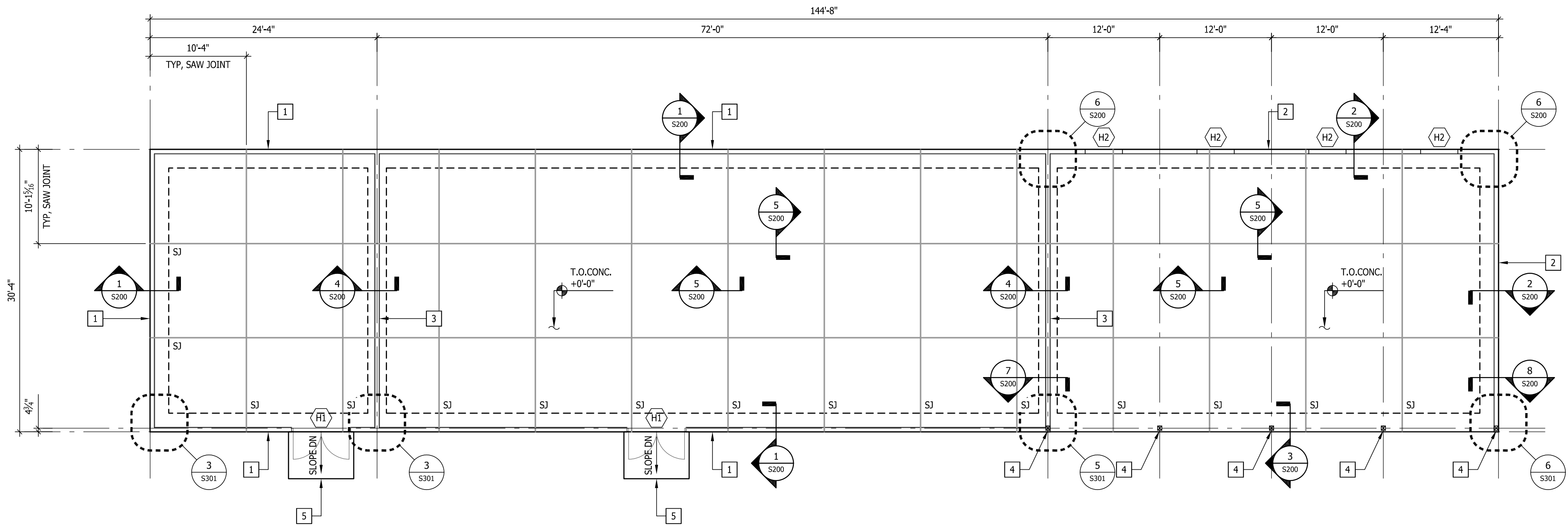
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WIND LOADING

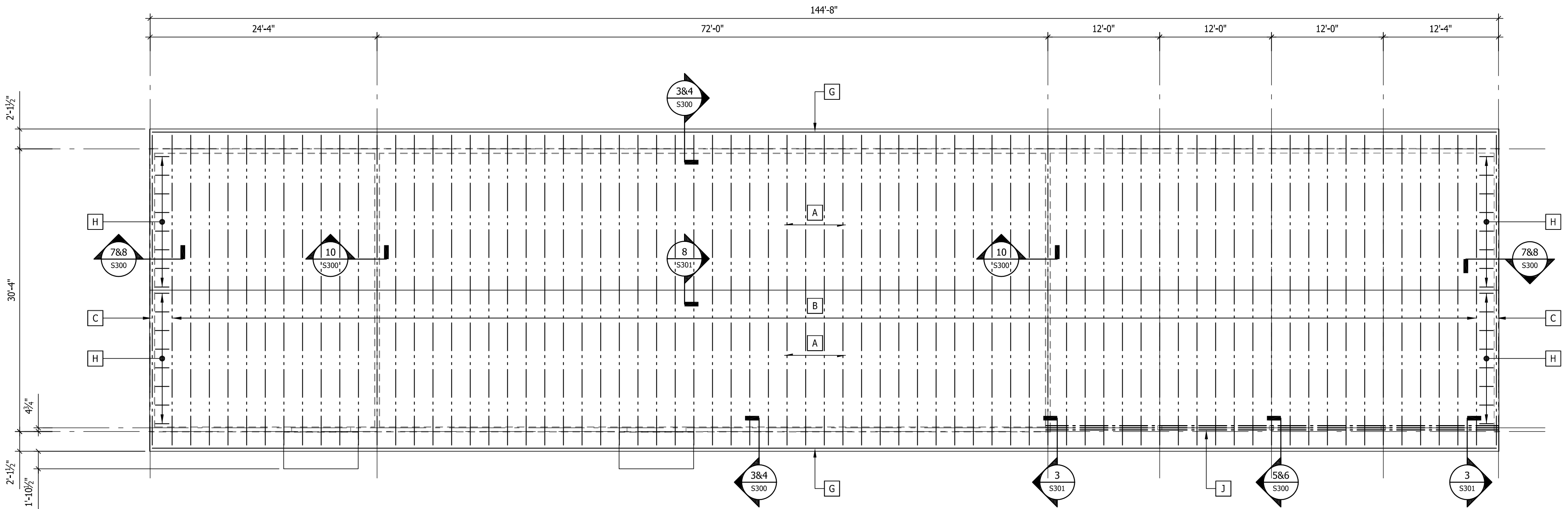
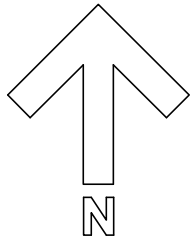
S002

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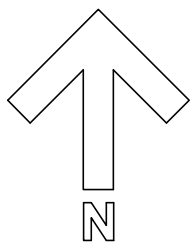
FOUNDATION / SLAB ON GRADE PLAN

SCALE: 1/8"=1/8"



ROOF PLAN

SCALE: 1/8"=1'-0"



FOUNDATION / SLAB ON GRADE PLAN NOTES

1. 2x6 AT 16"o.c. WALL WITH 3/4" CDX PLYWOOD ON BOTH SIDE WITH 10d NAILS AT 6"o.c. AT EDGES AND 12"o.c. IN FIELD AND 2x6 PT SILL PLATE WITH 1/2"x12" ANCHOR BOLT AT 32"o.c. AND WITHIN 8" AT THE CORNER, DOOR AND WINDOW JAMBS (ARCHITECTURAL WALL TYPE A). DETAIL 1/S200, EXTERIOR SHEAR WALL.
2. 2x6 AT 16"o.c. WALL WITH 3/4" CDX PLYWOOD ON BOTH SIDE ONLY WITH 10d NAILS AT 4"o.c. AT EDGES AND 8"o.c. IN FIELD AND 2x6 PT SILL PLATE WITH 1/2"x12" ANCHOR BOLT AT 24"o.c. AND WITHIN 8" AT THE CORNER, DOOR AND WINDOW JAMBS (ARCHITECTURAL WALL TYPE A). DETAIL 2/S200, EXTERIOR SHEAR WALL.
3. 2x6 AT 16"o.c. WALL (INTERIOR SHEAR WALL) WITH 3/4" CDX PLYWOOD BOTH SIDE WITH 10d NAILS AT 6"o.c. AT EDGES AND 12"o.c. IN FIELD AND 2x6 PT SILL PLATE WITH 1/2"x12" ANCHOR BOLT AT 32"o.c. AND WITHIN 8" AT THE CORNER, DOOR AND WINDOW JAMBS (ARCHITECTURAL WALL TYPE C). DETAIL 4/S200.
4. 6x6 PT WOOD COLUMN, SEE DETAIL 3/S200.
5. 4" CONCRETE SLAB ON GRADE AT DOORS WITH 6x6 - W2.0xW2.0 WWR.
6. SEE DETAIL 6/S200 FOR TYPICAL FOUNDATION CORNERS AND INTERSECTIONS.
7. SEE DETAIL 2/S301 FOR TYPICAL CORNERS AND INTERSECTIONS FOR WOOD FRAMING.
8. SEE DETAIL 1/S301 FOR HEADER AND JAMB (H#) INFORMATION FOR DOORS AND WINDOWS OPENINGS.

ROOF FRAMING PLAN NOTES

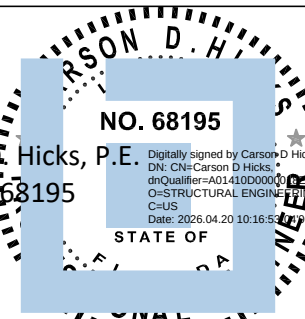
- A. ROOFING (SEE ARCH. DRAWINGS) OVER 3/4" PLYWOOD ROOF DECK OVER PRE-ENGINEERED WOOD TRUSSES (BY SPECIALTY ENGINEER). SEE STRUCTURAL NOTES FOR FASTENING REQUIREMENTS AND DETAIL 1/S300.
 - B. PRE-ENGINEERED WOOD TRUSSES AT 2'-0"o.c. BY SPECIALTY ENGINEER.
 - C. GABLE WALL WOOD TRUSS BY TRUSS SUPPLIER'S SPECIALTY ENGINEER. SPECIALTY ENGINEER TO DESIGN THE BRACING FOR THE GABLE WALL WOOD TRUSS. SEE DETAIL 2/S300.
 - D. PRE-ENGINEERED WOOD TRUSS SUPPLIER'S SPECIALTY ENGINEER TO PROVIDE WOOD FRAMING AT ALL ROOF PEAKS FOR PLYWOOD DECK ATTACHMENT. SEE DETAILS 8/S301.
 - E. ALL CONNECTIONS FOR THE PRE-ENGINEERED WOOD TRUSSES AND GIRDER TRUSS TO EACH OTHER ARE BY TRUSS SUPPLIER'S SPECIALTY ENGINEER. SPECIALTY ENGINEER TO PROVIDE SIMPSON STRONG TIE CONNECTIONS TO WOOD BEAM.
 - F. PRE-ENGINEERED WOOD TRUSS BRACING AND BRIDGING IS TO BE DESIGNED BY THE TRUSS SUPPLIER'S SPECIALTY ENGINEER. SEE STRUCTURAL NOTES AND DETAIL 2/S300. PROVIDE TRUSS BRACING AT THE TOP OF THE WOOD SHEAR WALLS.
 - G. 2x FASCIA BOARD, SEE ARCHITECTURAL DRAWINGS.
 - H. 2x WOOD BLOCKING BY TRUSS SUPPLIER'S SPECIALTY ENGINEER.
 - J. (3) 1 1/2"x1 1/4" 2.0E GP LAM LVL BEAM WITH 2X6 HORIZONTAL TOP AND BOTTOM OF LVL BEAM, STAGGER LVL BEAM SPLICES.
- ALL LVL BEAMS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES:
1. BENDING STRESS, Fb = 2,900 psi
 2. MODULUS OF ELASTICITY, E = 2,000 ksi
 3. SHEAR STRESS, Fv = 285 psi
 4. COMPRESSION PERPENDICULAR TO GRAIN, Fcp = 750 psi
- K. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MECHANICAL EQUIPMENT LOADING, SIZES AND LOCATIONS ON THE ROOF OR IN THE ATTIC SPACE.
 - L. SEE DETAIL 1/S301 FOR DOOR AND WINDOWS HEADERS AND JAMBS.

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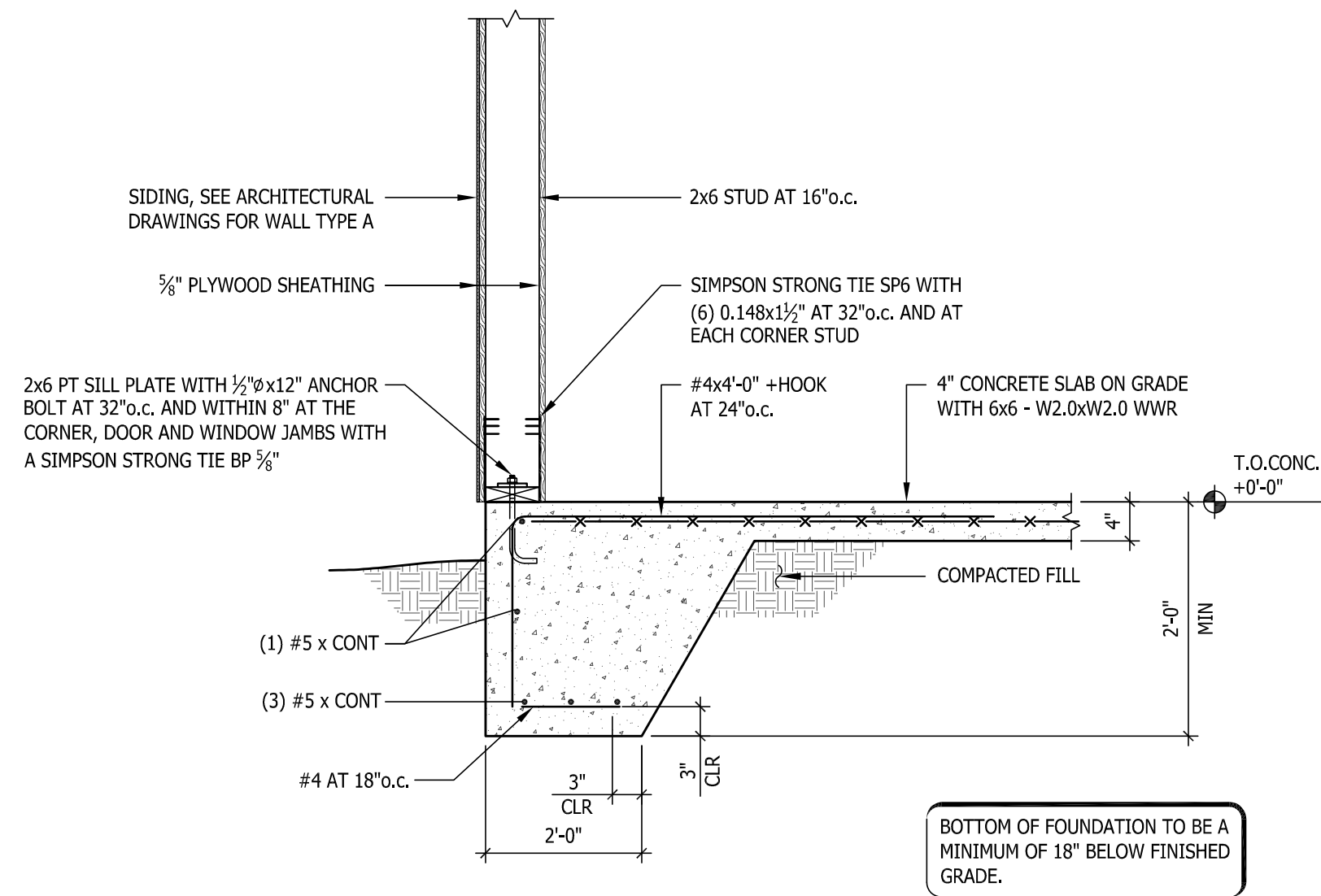
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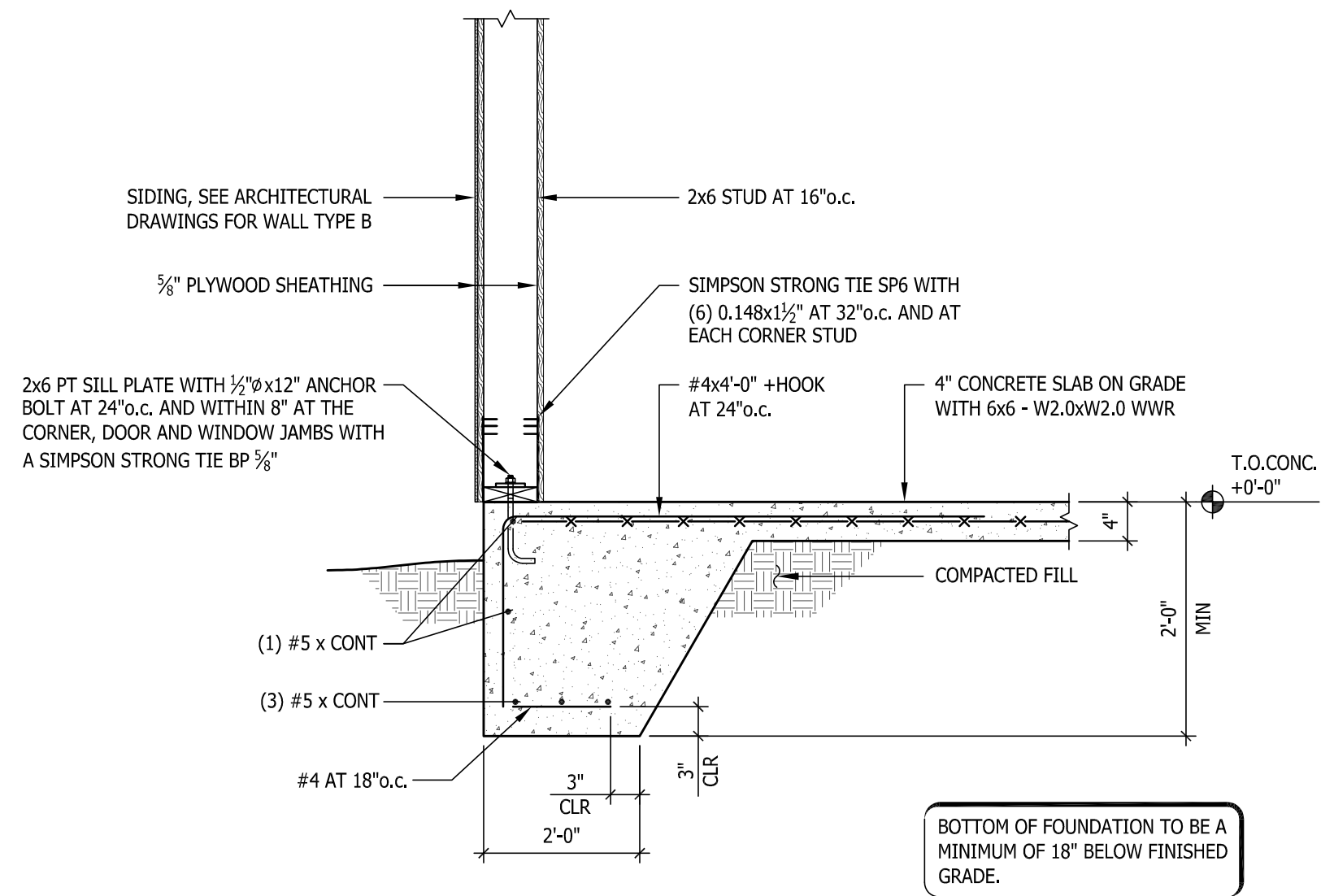
PLANS

S102

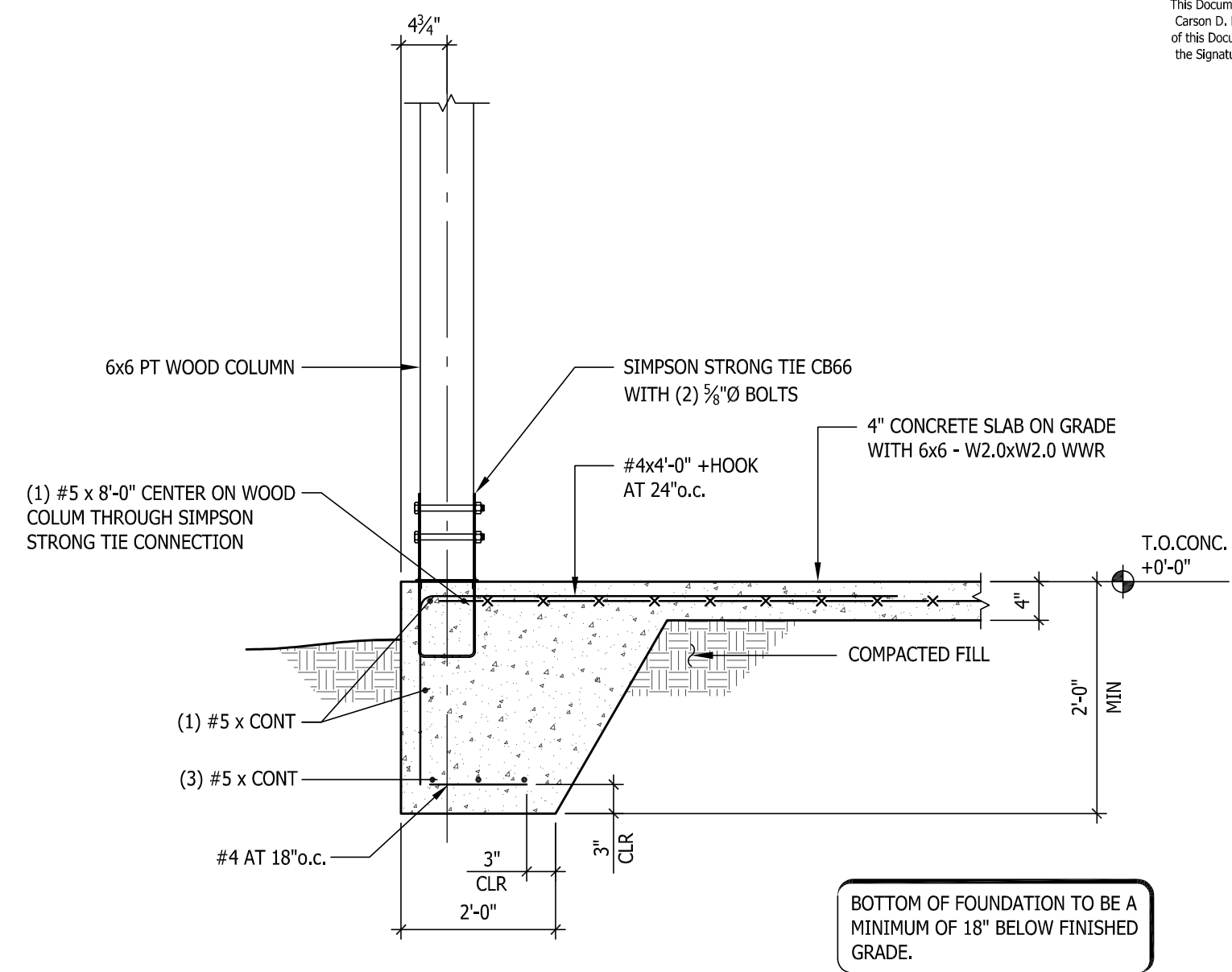
To the best of my knowledge, the Plans & Specifications comply with applicable minimum Building Codes.
This Document has been electronically Signed & Sealed by Carson D. Hicks using a Digital Signature. Printed Copies of this Document are not Considered Signed & Sealed and the Signatures must be verified on any Electronic copies.



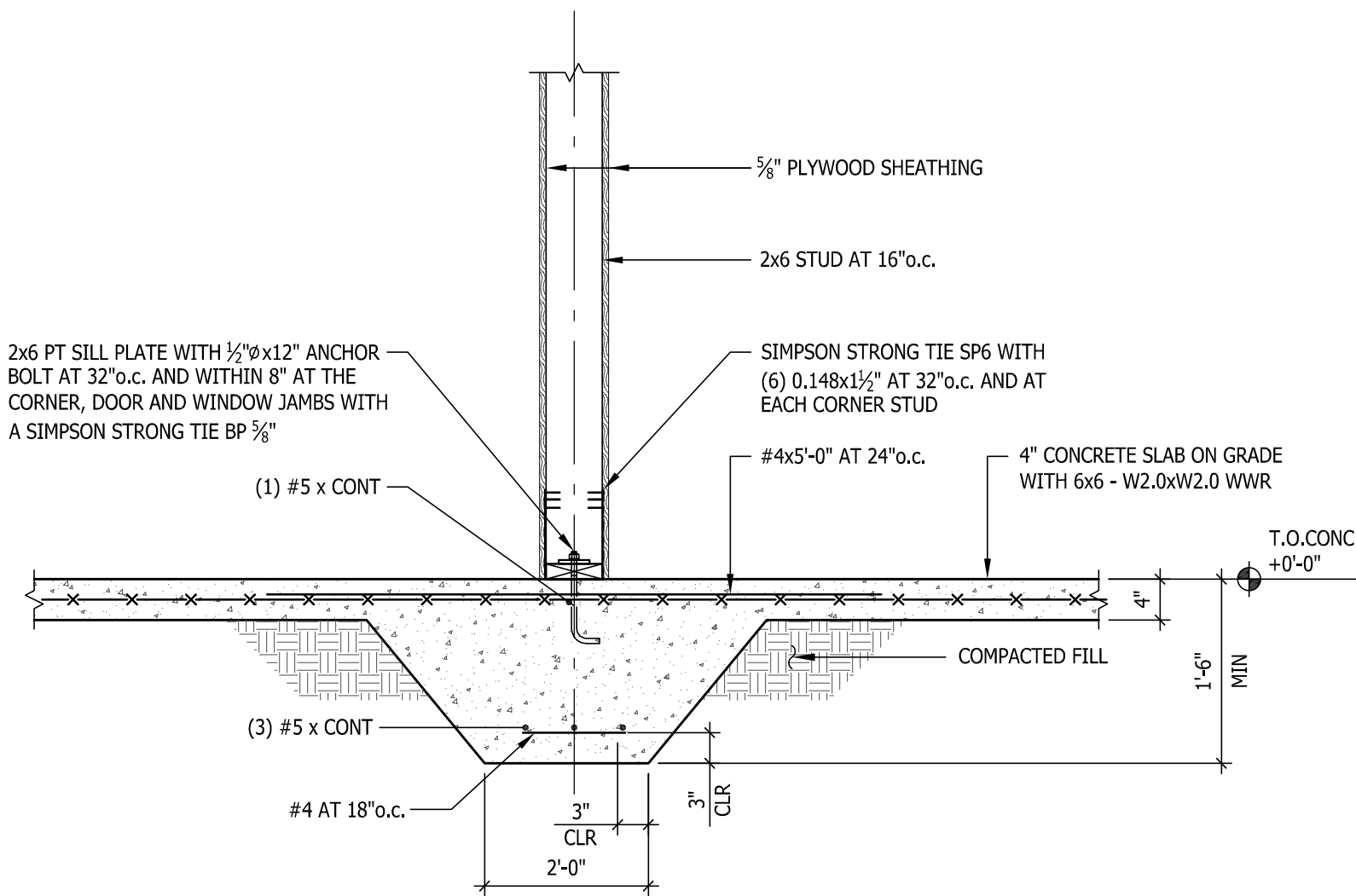
1 CONCRETE FOUNDATION DETAIL
AT EXTERIOR WALL



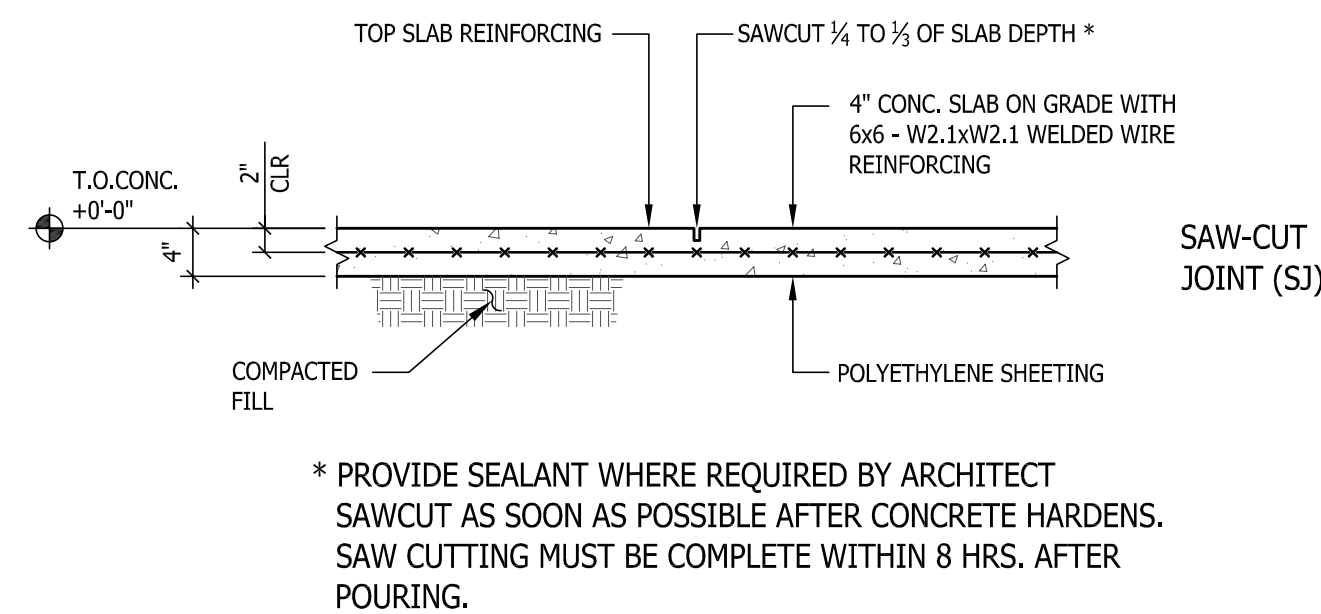
2 CONCRETE FOUNDATION DETAIL
AT EXTERIOR WALL



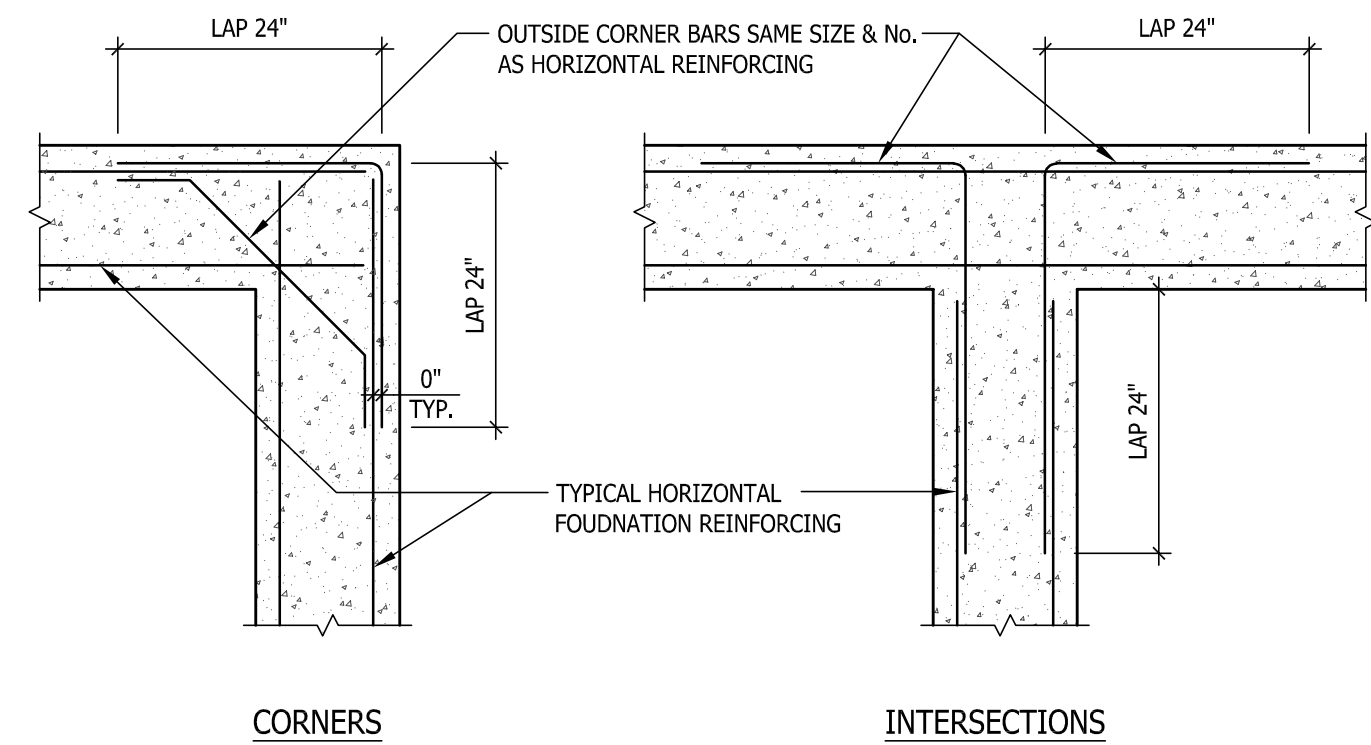
3 CONCRETE FOUNDATION DETAIL
AT WOOD COLUMN



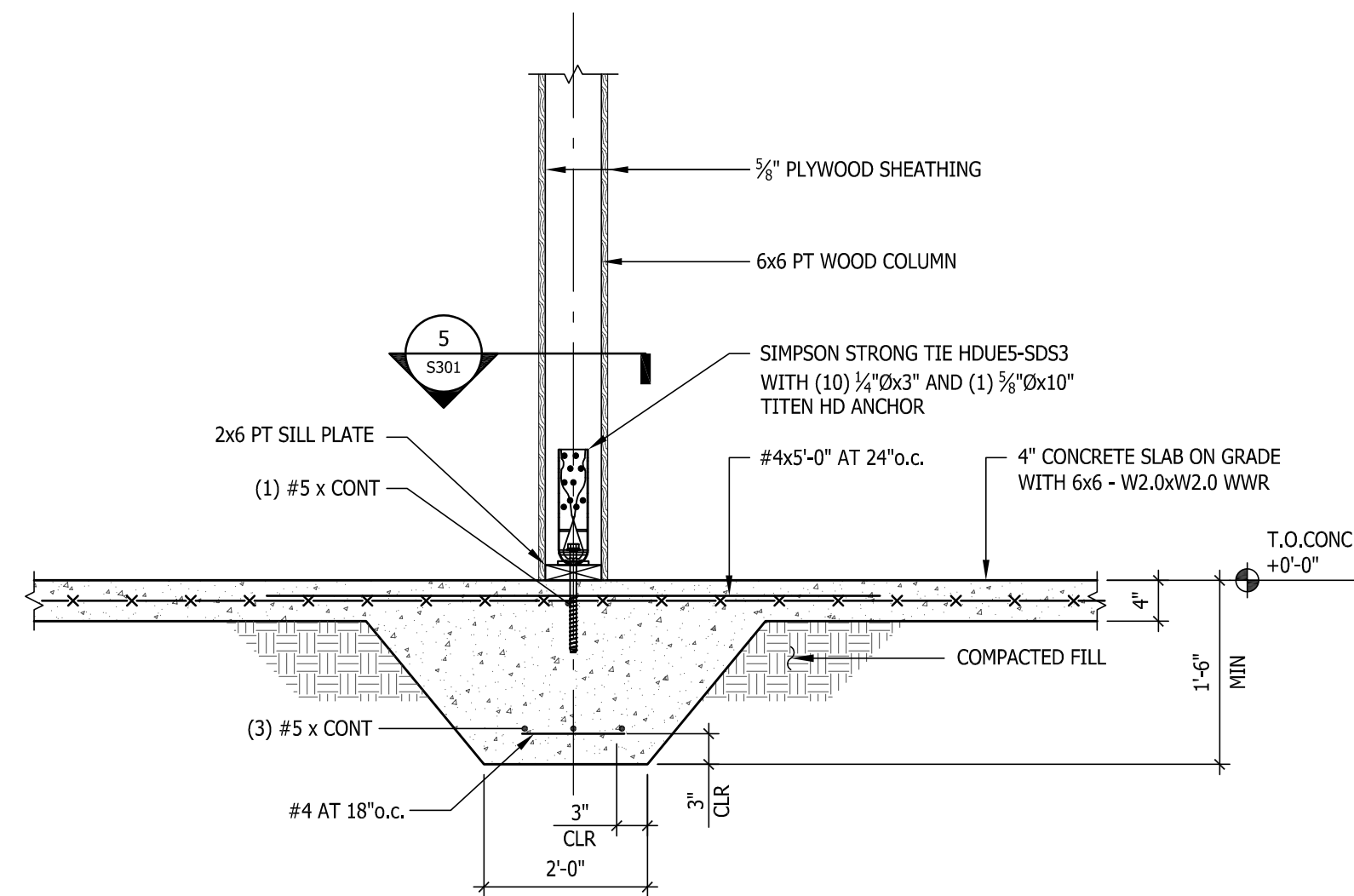
4 CONCRETE FOUNDATION DETAIL
AT INTERIOR WALL



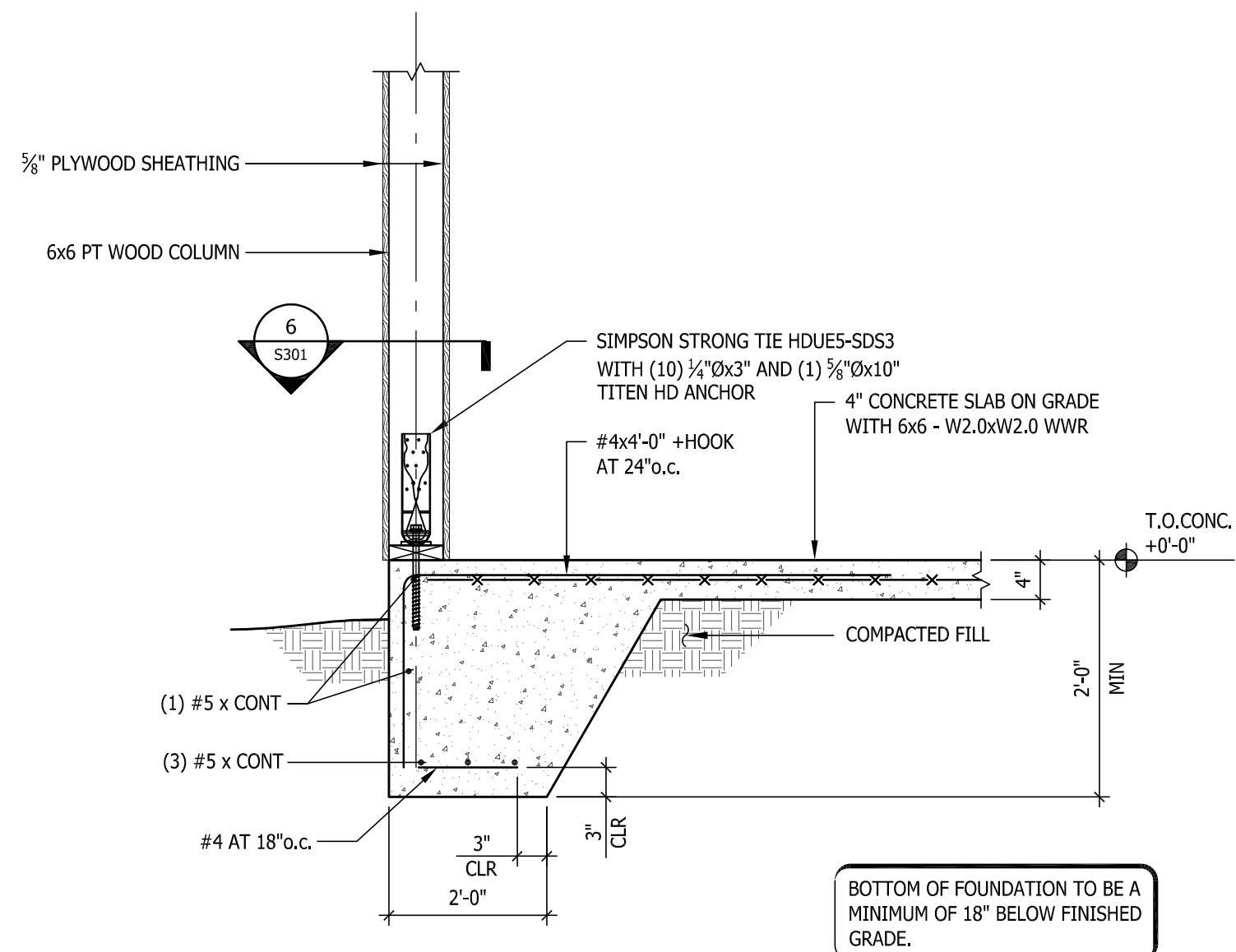
5 SLAB CONTROL JOINTS
SLAB ON GRADE



6 TYPICAL HORIZONTAL REINFORCING
CONCRETE FOUNDATION CORNERS AND INTERSECTIONS



7 CONCRETE FOUNDATION DETAIL
AT WOOD COLUMN AT WALL



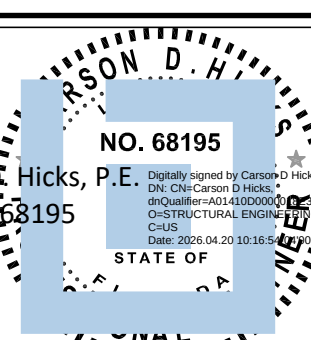
8 CONCRETE FOUNDATION DETAIL
AT WOOD COLUMN

CONSULTANTS



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FWC STORAGE BUILDING - FWC-02329180
FLORIDA FISH AND WILDLIFE CONSERVATION
COMMISSION
LAKE CITY, FLORIDA

**CONSTRUCTION
DOCUMENTS**

PROJ. NO. 172224
DATE 04/20/2026
DRAWN CDH
CHECKED CDH
APPROVED CDH
REVISION
REVISION DATE

**CONCRETE
DETAILS**

S200

DRAWING SCALE: 3/4" = 1'-0"

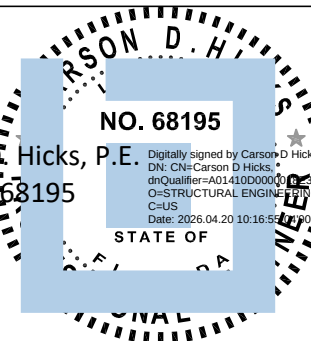
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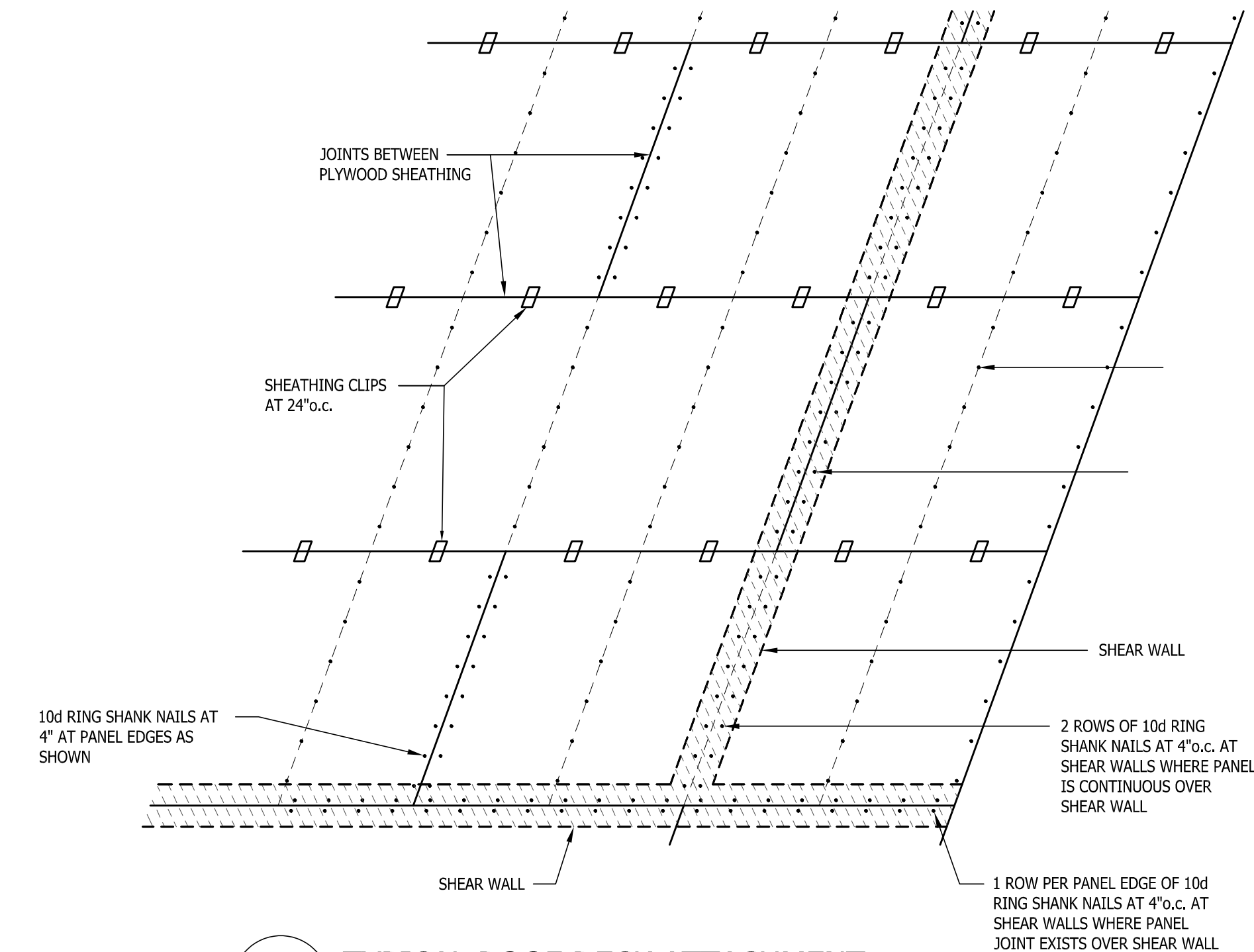
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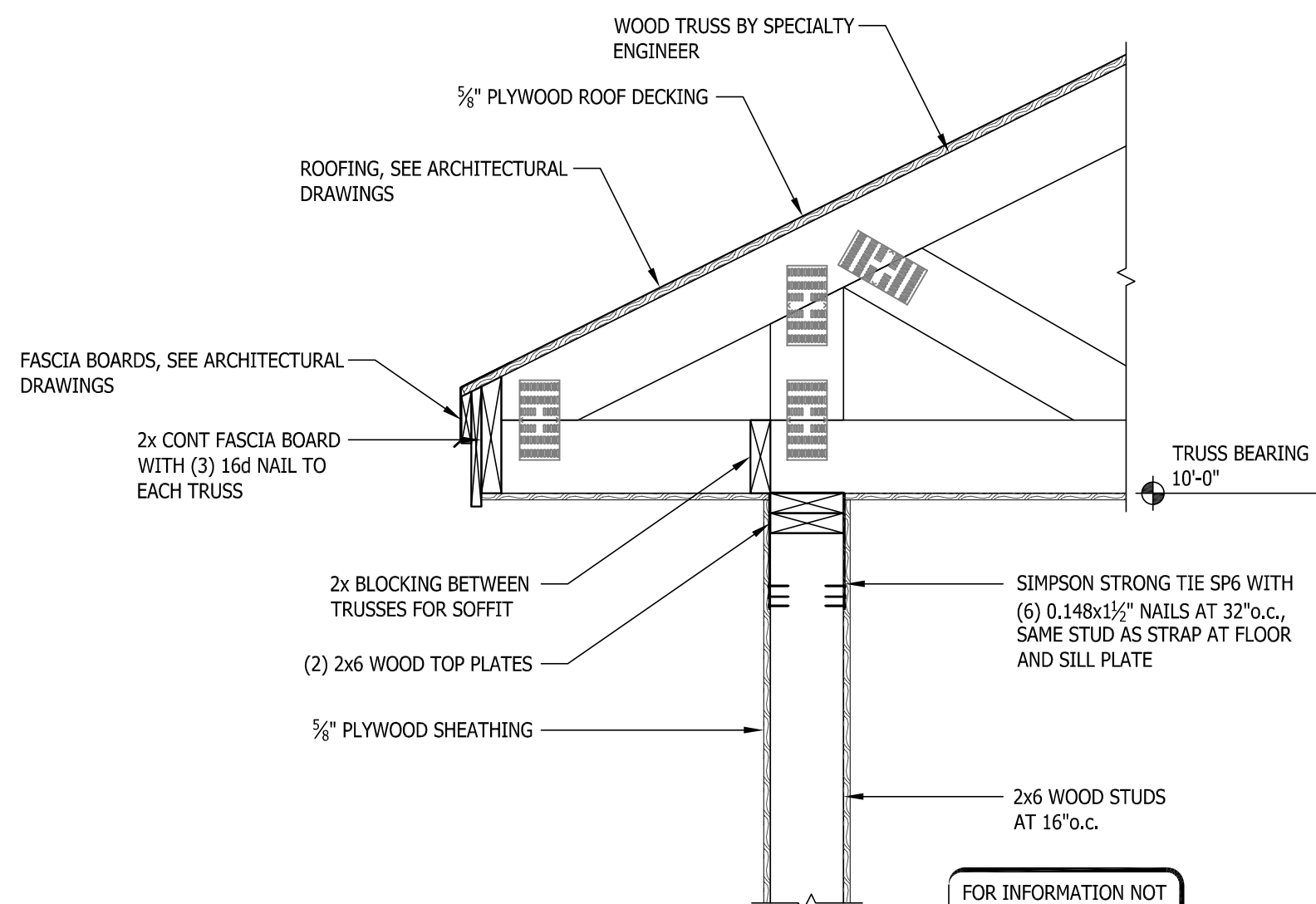
WOOD DETAILS

S300

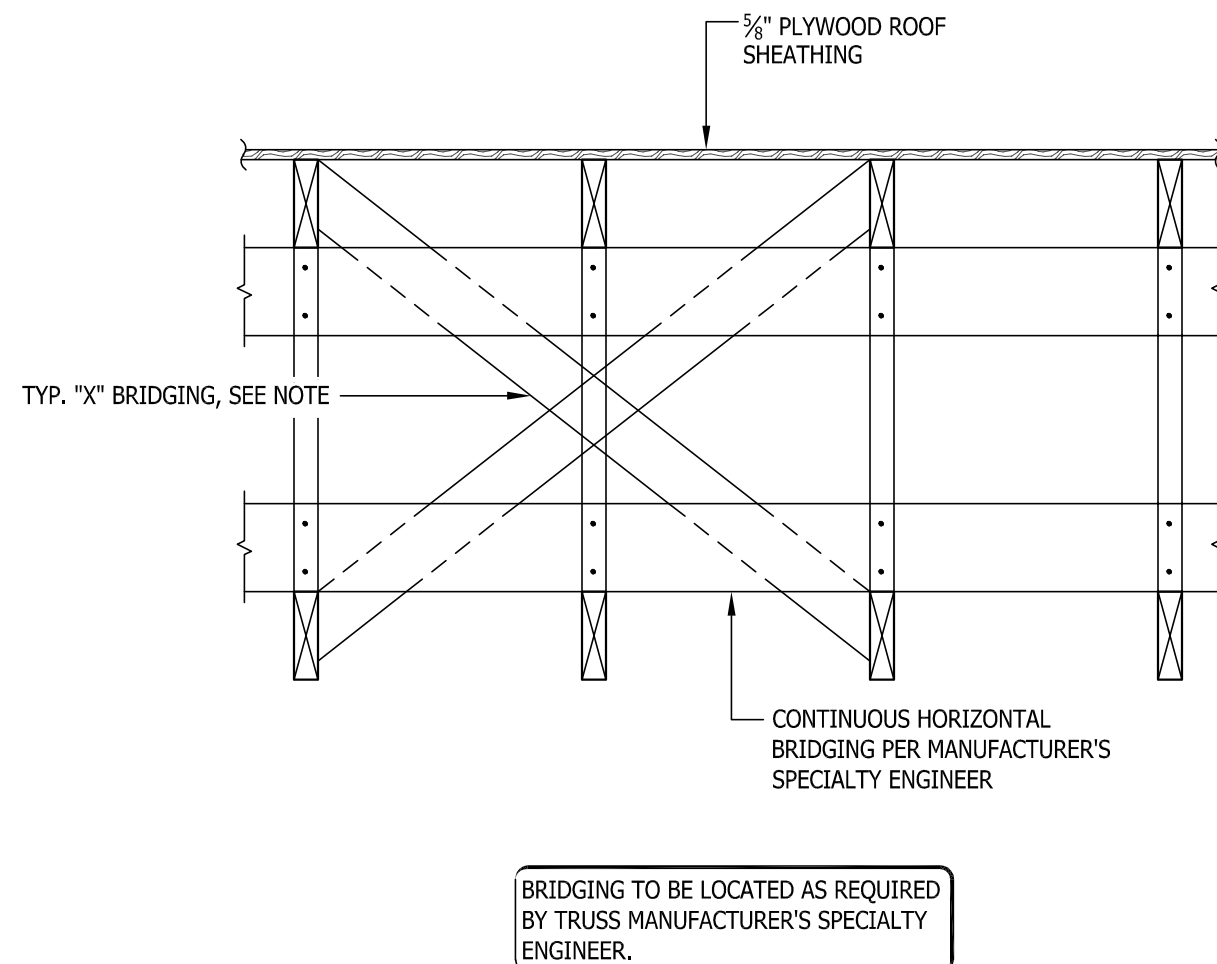
DRAWING SCALE: 1" = 1'-0"



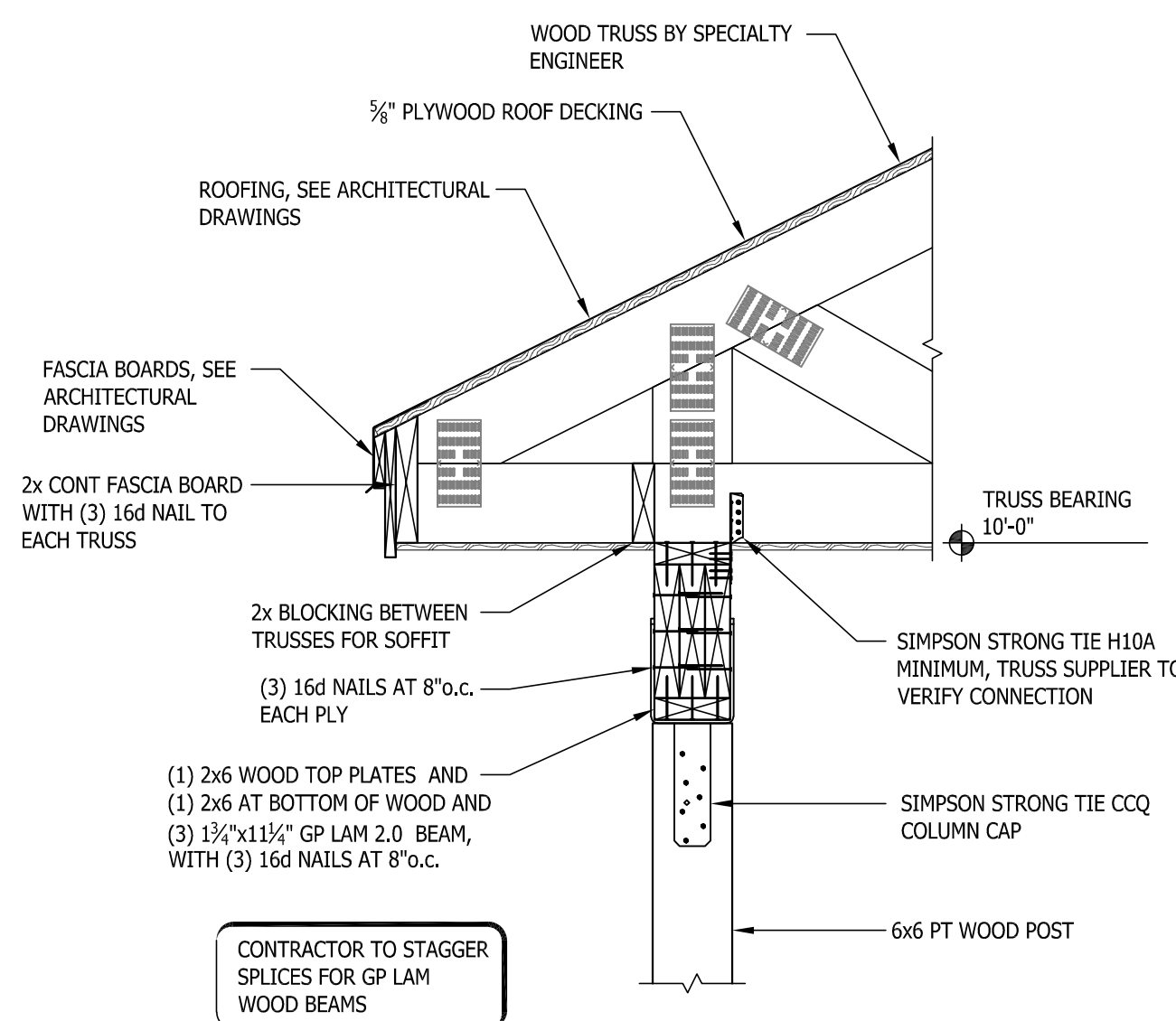
**1 TYPICAL ROOF DECK ATTACHMENT
UNBLOCKED DIAPHRAGM**



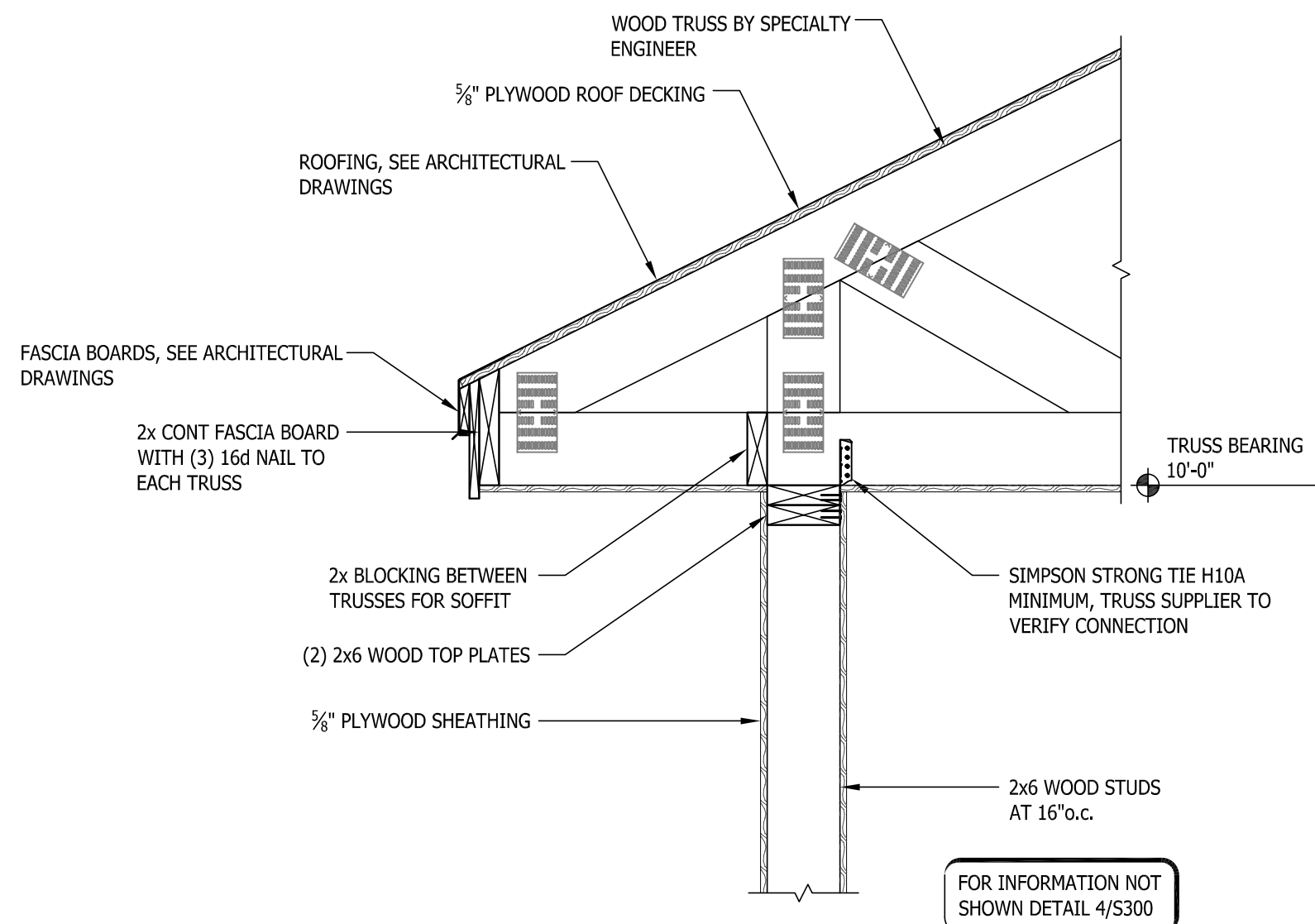
**4 TYPICAL TRUSS TO WALL
AT EXTERIOR WALLS**



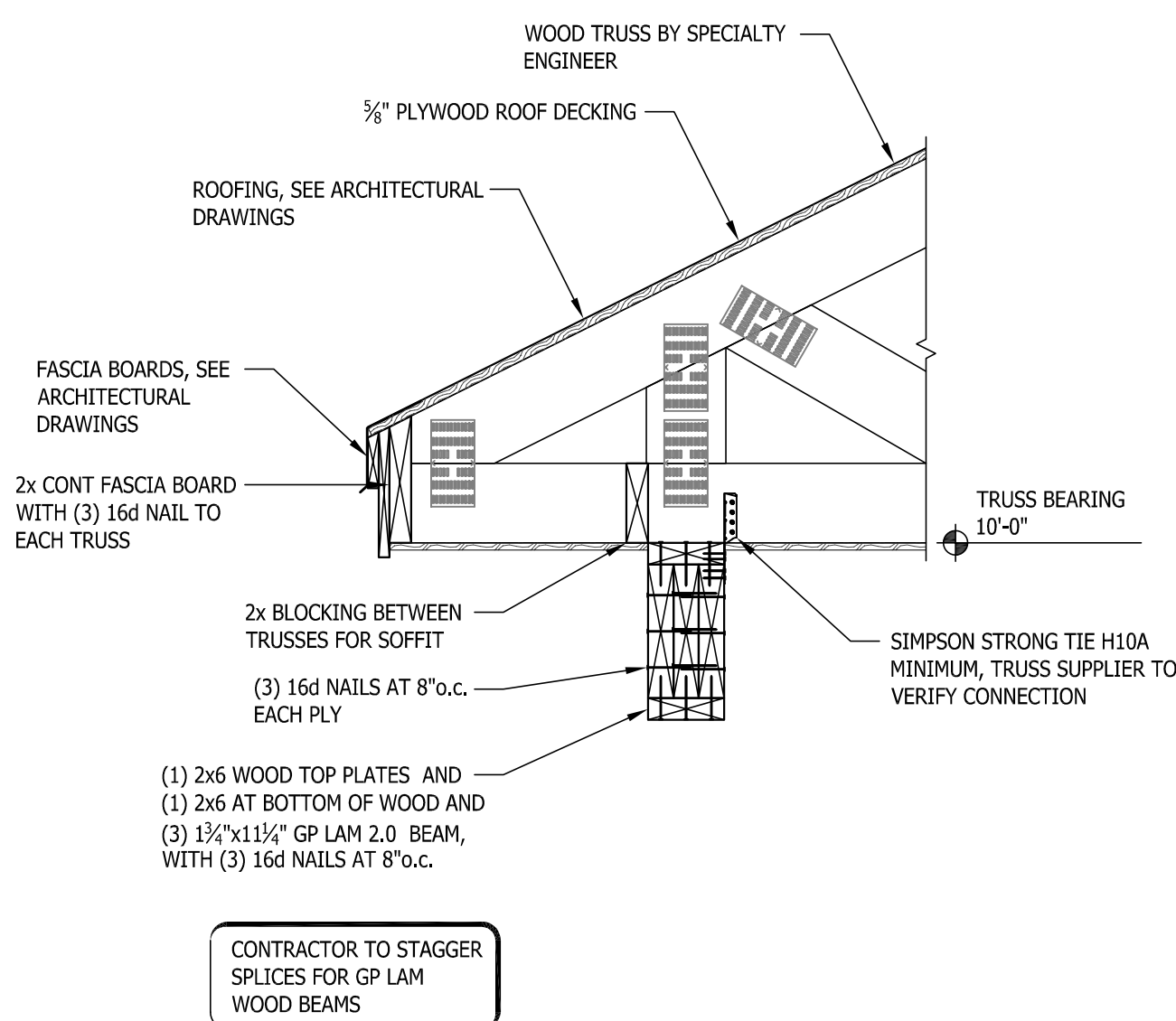
2 TYPICAL TRUSS BRIDGING DETAIL



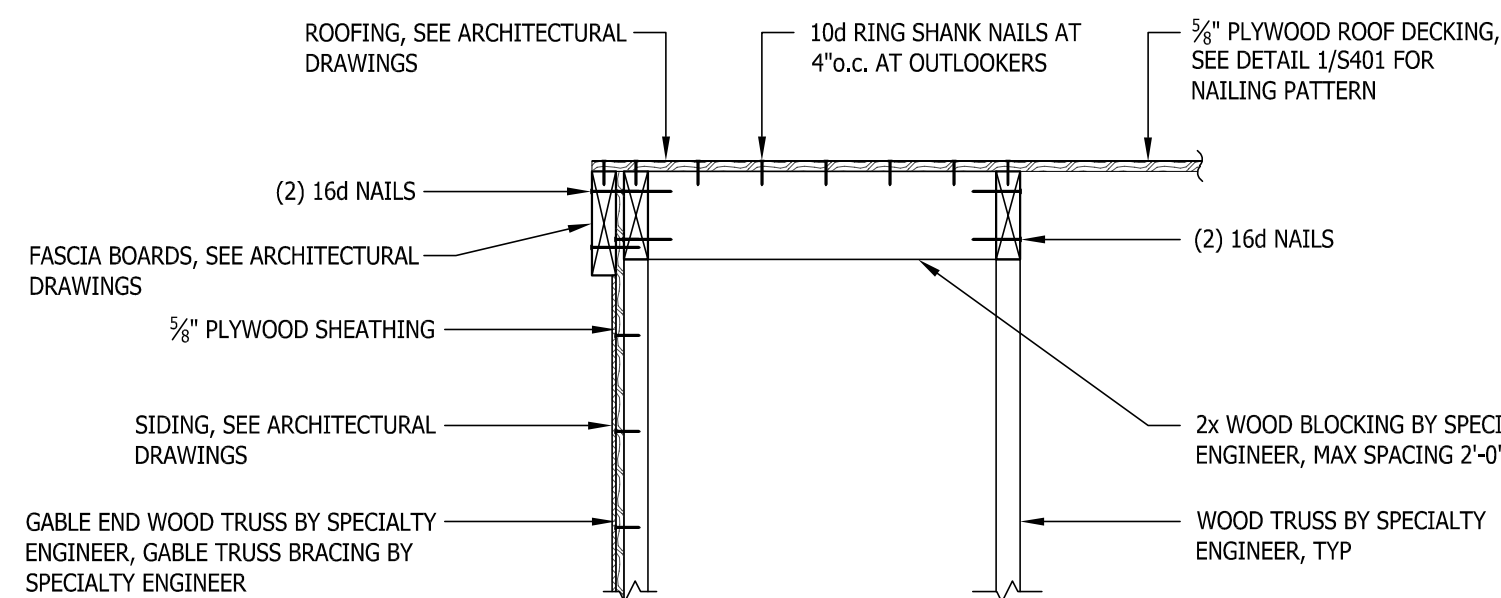
5 TYPICAL CONNECTION AT COLUMN



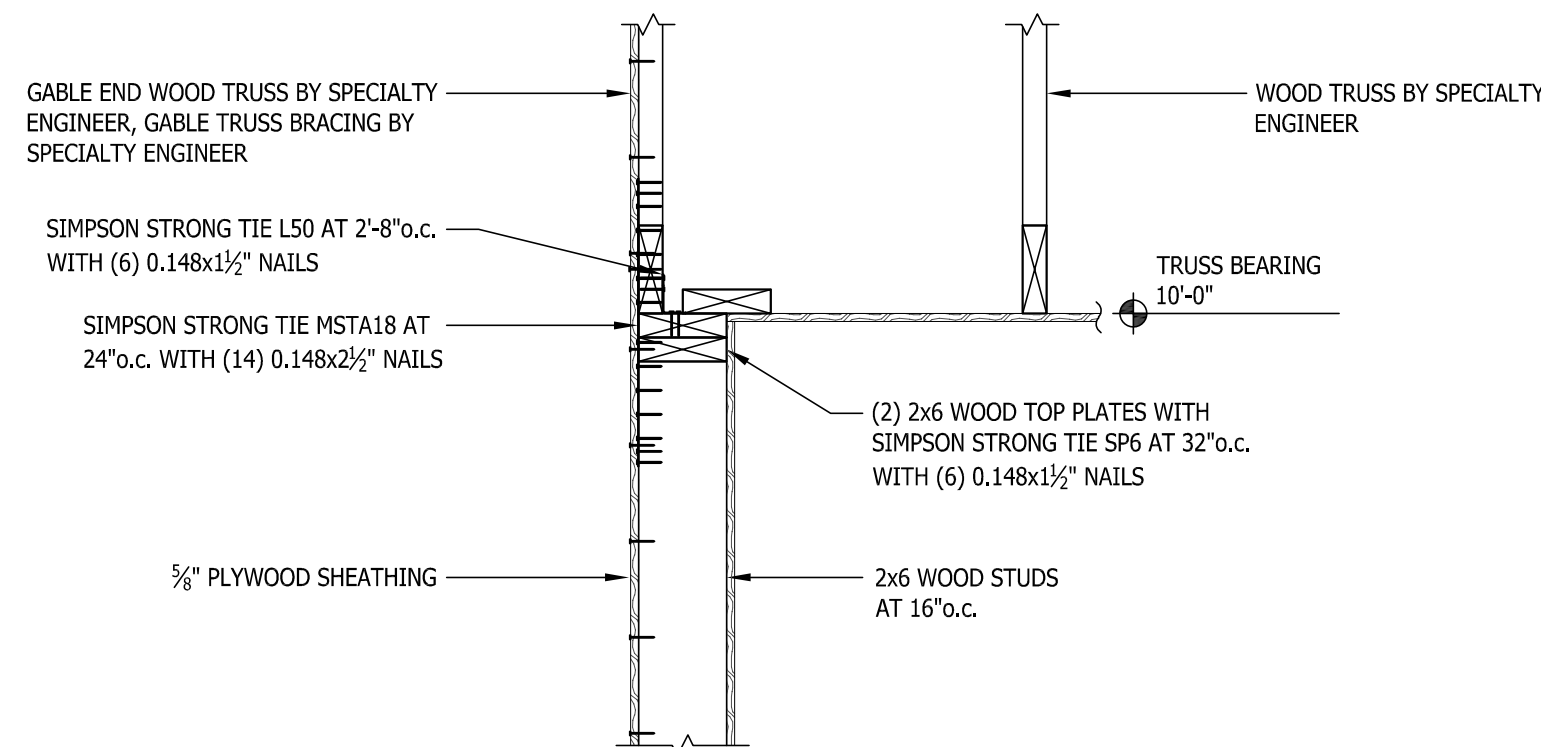
**3 TYPICAL TRUSS TO WALL
AT EXTERIOR WALLS**



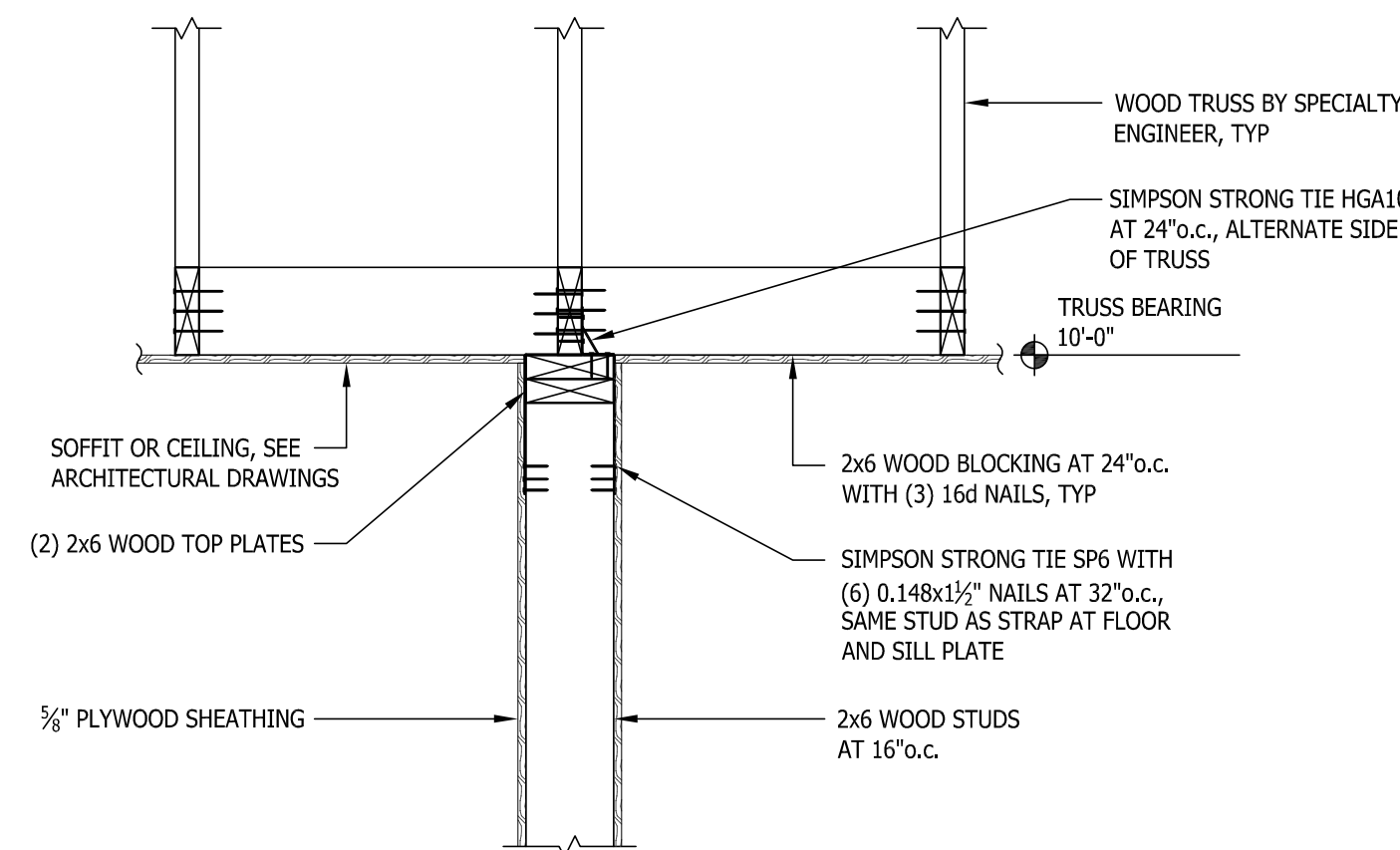
6 TYPICAL CONNECTION AT BEAM



**8 TYPICAL OUTLOOKER DETAIL
AT ROOF GABLES**



**9 TYPICAL TRUSS TO WOOD FRAMING
AT ROOF GABLES**



**10 TYPICAL TRUSS CONNECTION TO WALL
AT INTERIOR SHEAR WALL**

