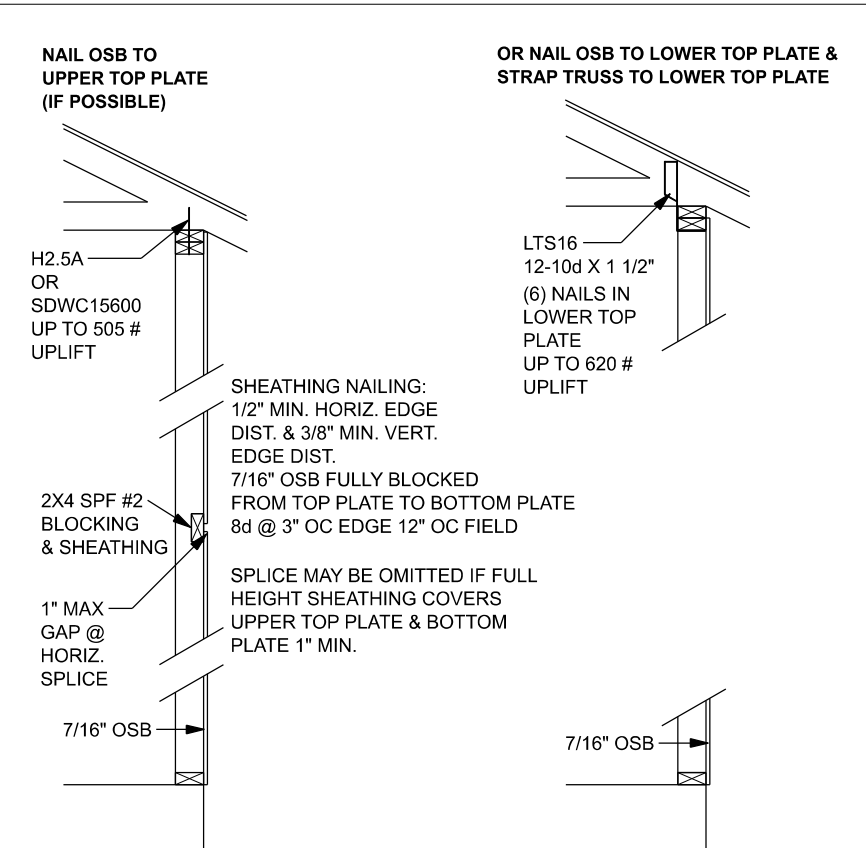


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

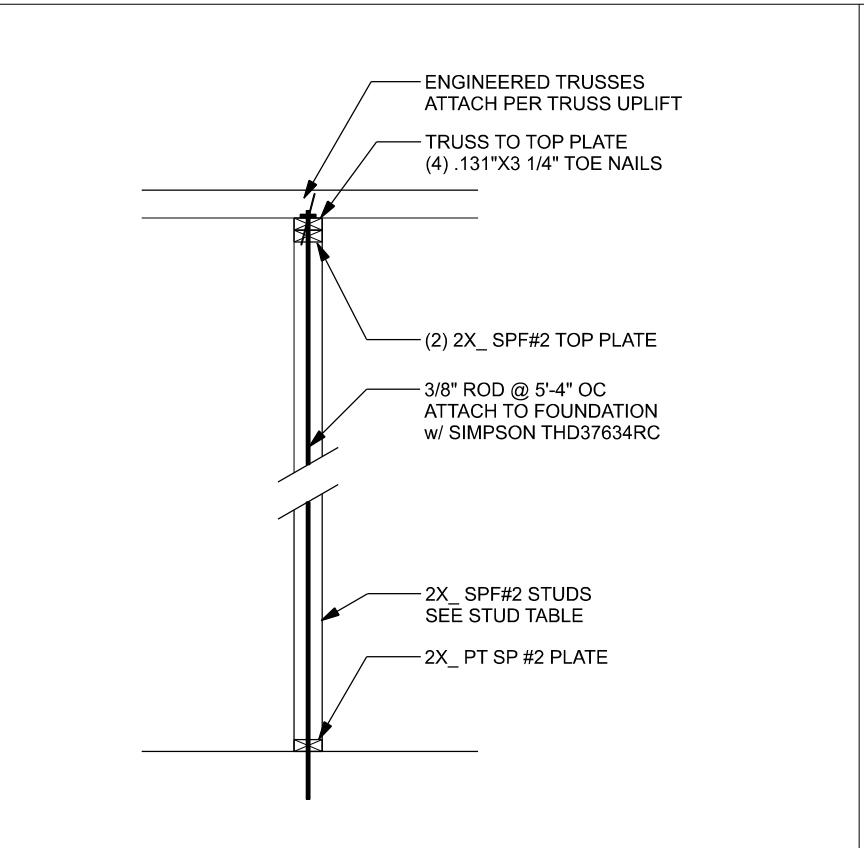
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 9 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ RODS

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600		
400	290	H3	4-131'x3 1/4" 12"	4-131'x3 1/4" 12"
625	540	H2.5A	5-131'x1 1/2" 12"	5-131'x1 1/2" 12"
1040	1015	H10A	9-148'x1 1/2" 12"	9-148'x1 1/2" 12"
645	515	LTS12-20	6-148'x1 1/2" 12"	6-148'x1 1/2" 12"
990	850	MTS12-30	7-148'x1 1/2" 12"	7-148'x1 1/2" 12"
1415	1215	HTS16-30	8-148'x1 1/2" 12"	8-148'x1 1/2" 12"
Uplift SP Uplift SPF	Strap Ties			
1235	1235	LSTA21	8-148'x1 1/2" 12"	8-148'x1 1/2" 12"
1640	1460	MSTA24	9-148'x1 1/2" 12"	9-148'x1 1/2" 12"
1030	1030	CS20	7-148'x1 1/2" 12"	7-148'x1 1/2" 12"
Uplift SP Uplift SPF	Stud Plate Ties	To Stud	To Plate	
555	535	SP1	4-148'x3"	4-148'x3"
1010	805	SP2	6-148'x3"	6-148'x3"
1280	1100	SPH46	12-148'x1 1/2" 12"	wrap under or over plate
771	771	LSTA24	10-148'x1 1/2" 12"	wrap under or over plate
1235	1235	LSTA24	14-148'x1 1/2" 12"	wrap under or over plate
Uplift SP Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor	
2145	1835	DTT22	8-SDS 1/4"x1 1/2" 12"x12" Titan HD	
4235	3640	HTT4	18-162'x2 1/2" 12"x12" Titan HD	
2145	1835	DTT22	8-SDS 1/4"x1 1/2" 12"x12" Titan HD	
4235	3640	HTT4	18-162'x2 1/2" 12"x12" Titan HD	
Uplift SP Uplift SPF	Post Bases @ Stenwall	To Post	Anchor	
1900	1900	ABU44Z	12-162'x3 1/2" 5/8"x12" Drill & Epoxy	
2475	2475	ABU66Z	12-162'x3 1/2" 5/8"x12" Drill & Epoxy	
Uplift SP Uplift SPF	Post Bases @ Mono	To Post	Anchor	
1900	1900	ABU44Z	12-162'x3 1/2" 5/8"x7" Drill & Epoxy	
2475	2475	ABU66Z	12-162'x3 1/2" 5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS OR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Stud Spacing	TO 10'-1" STUD HEIGHT	TO 11'-2" STUD HEIGHT	TO 15'-7" STUD HEIGHT	TO 17'-3" STUD HEIGHT
(1) 2x4 @ 16" OC				
(1) 2x4 @ 12" OC				
(1) 2x6 @ 16" OC				
(1) 2x6 @ 12" OC				

GRADE & SPECIES TABLE

Grade	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND 1700	17.0	2.0
LVL	MICROLAM	2950	1.7
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, BRACING PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIES ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REACTION TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 41LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.F.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.175 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 1 FT. DO NOT CUT W/ OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS). UNO: ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH THE REBAR SCHEDULE.

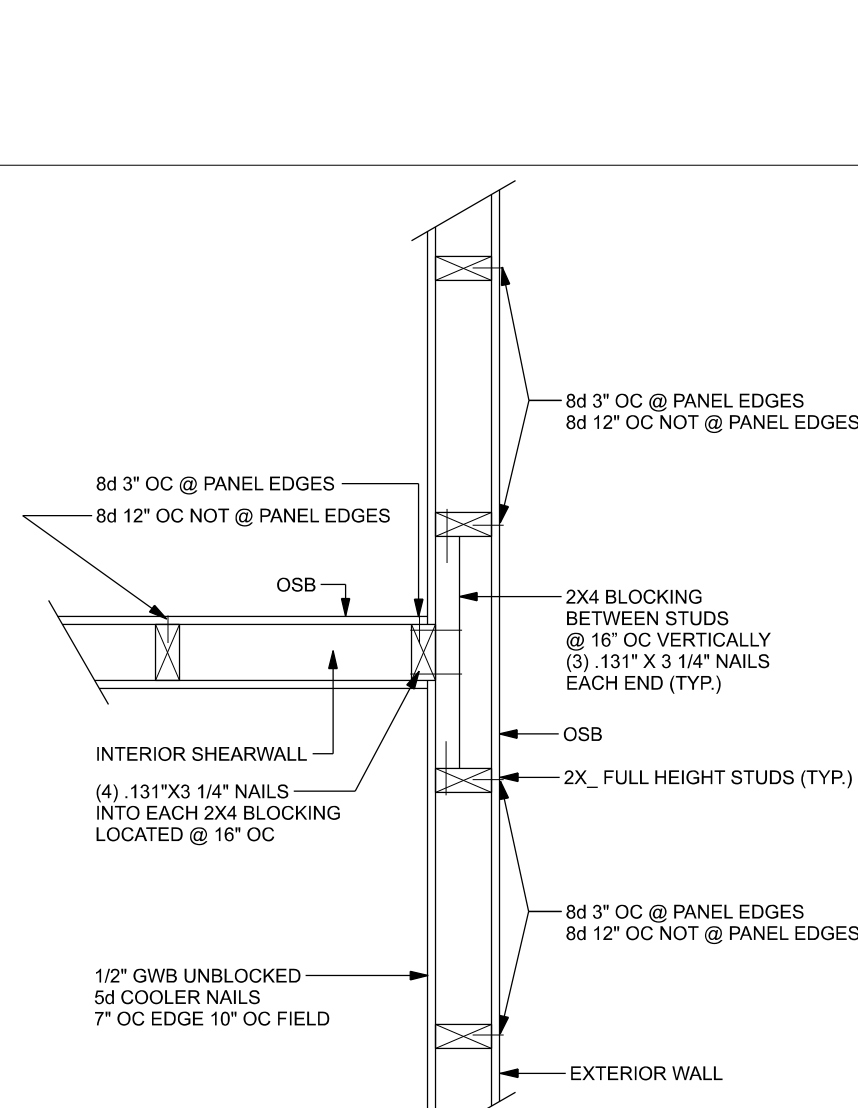
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

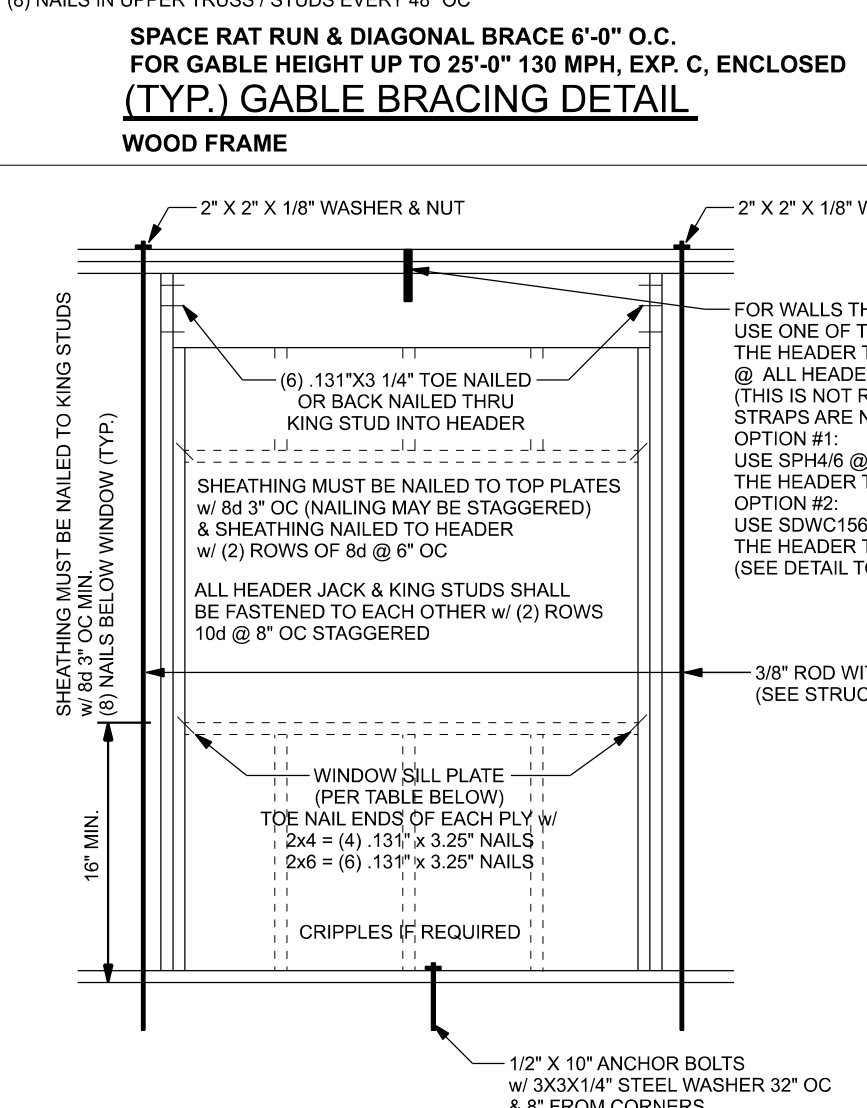
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUVED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMBITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, BRACING PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

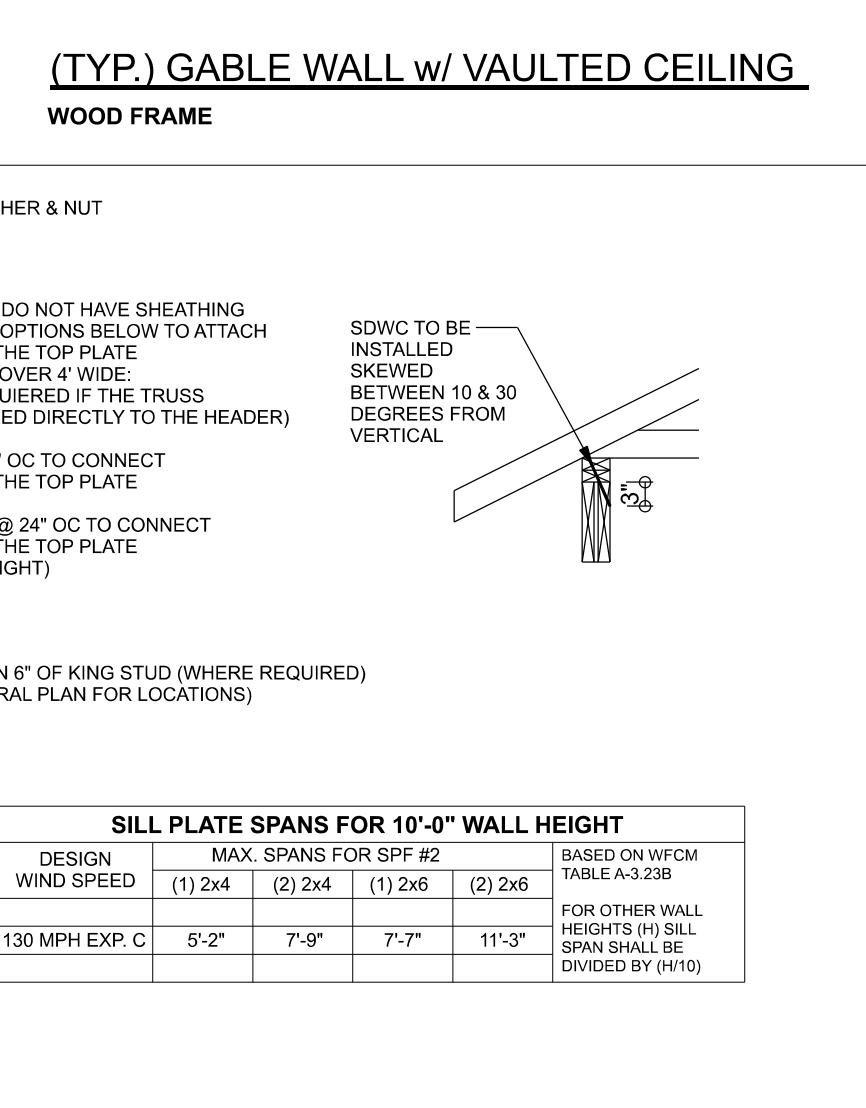
ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



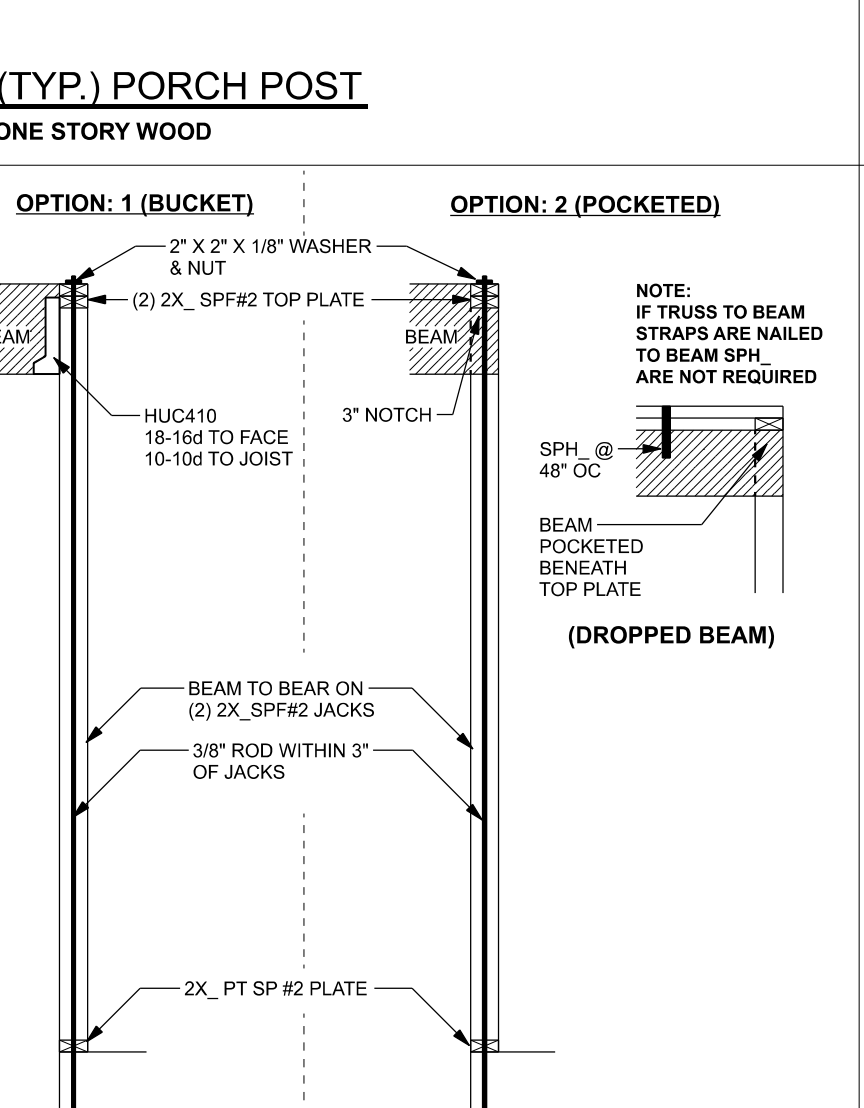
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



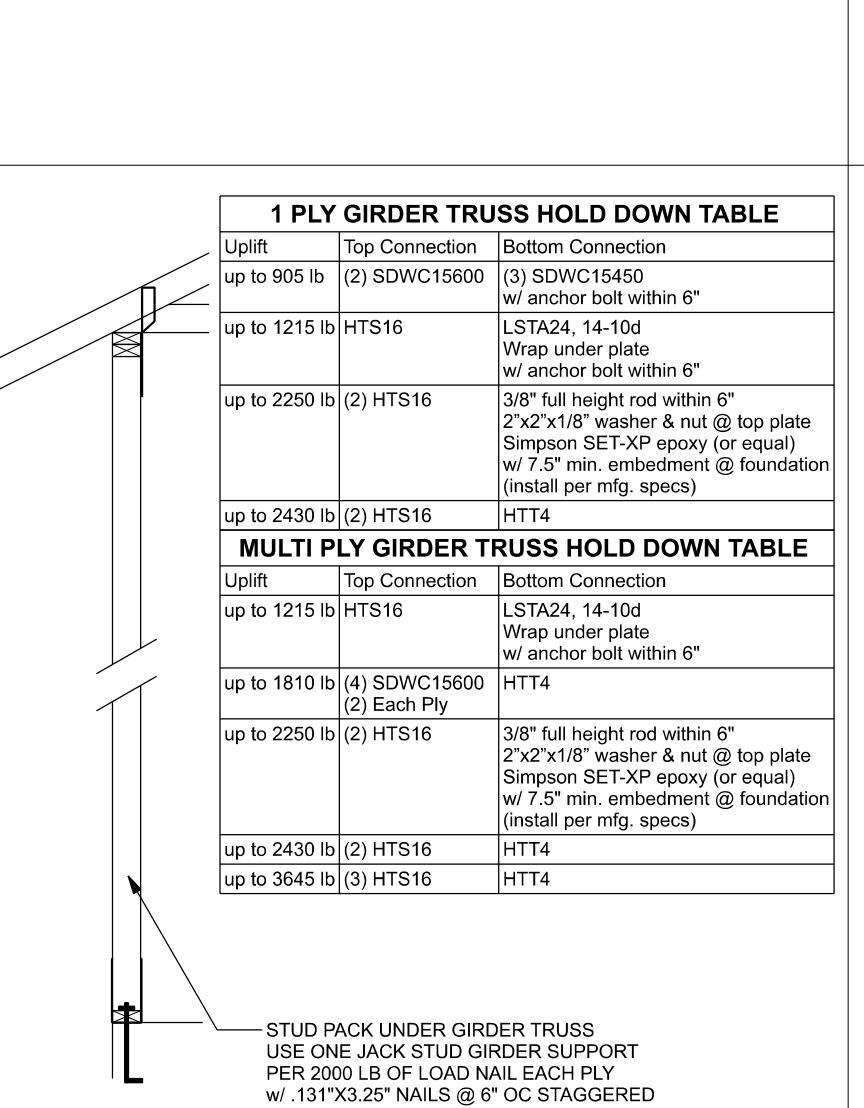
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



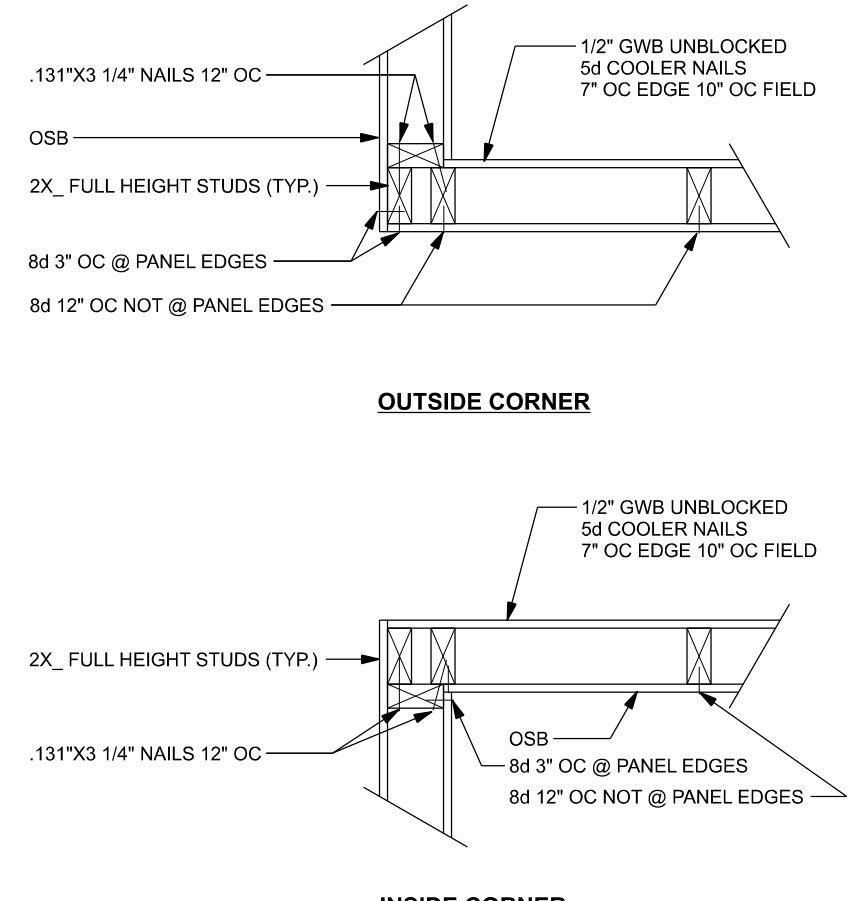
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



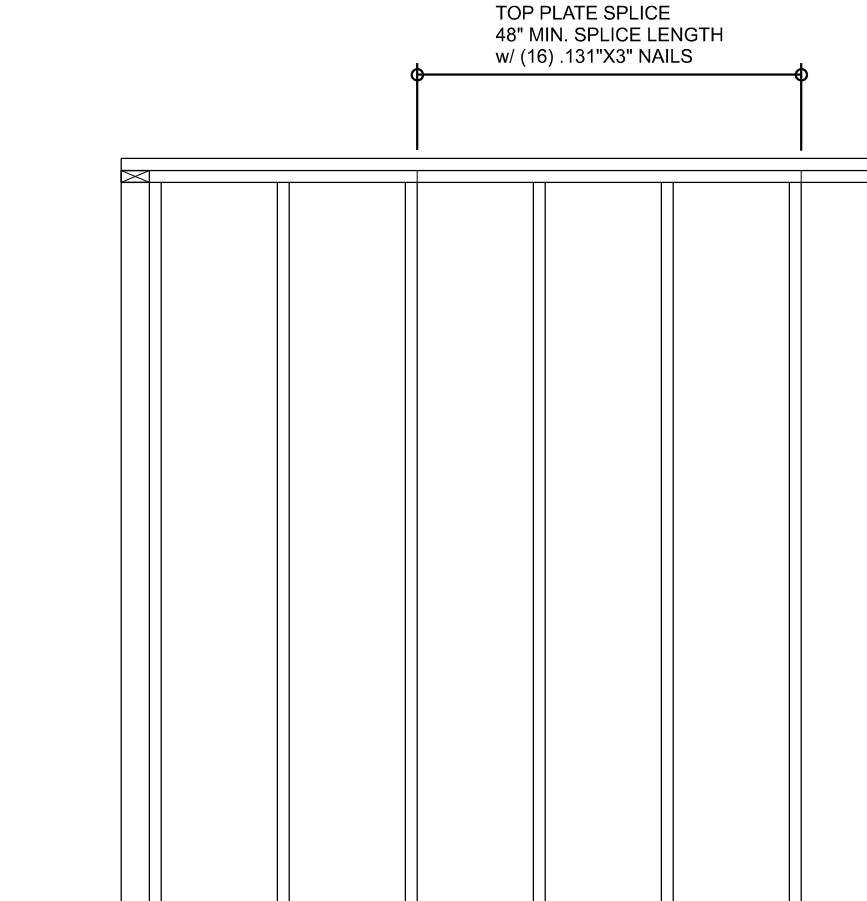
(TYP.) PORCH POST
ONE STORY WOOD



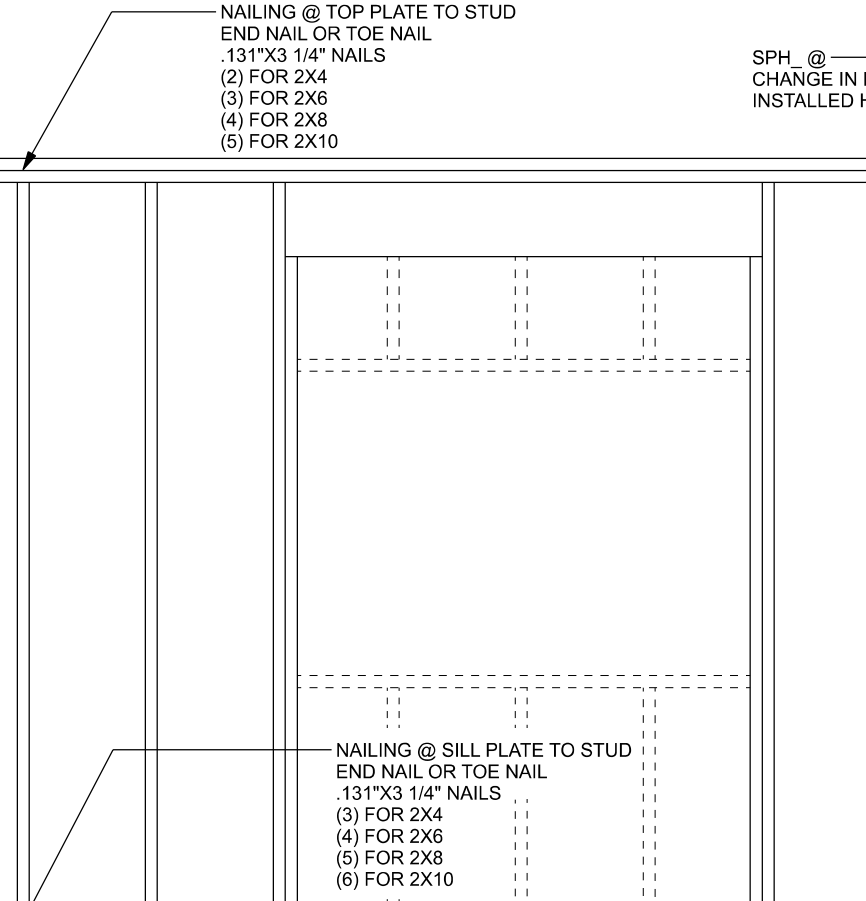
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



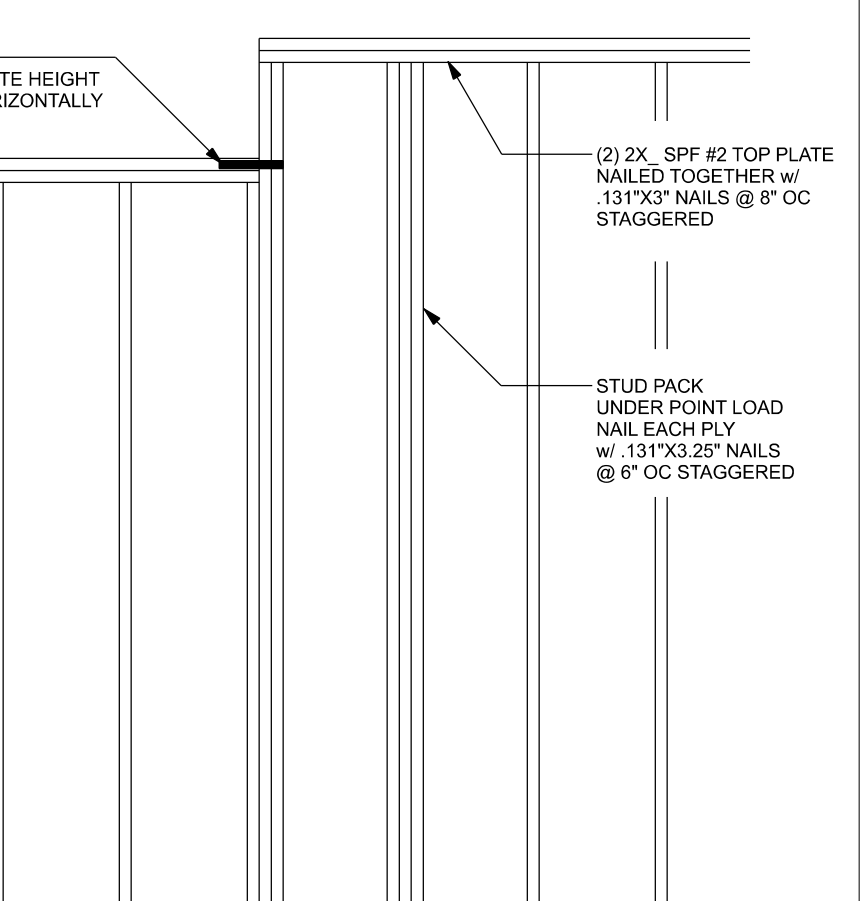
(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

1 PLY GIRDER TRUSS HOLD DOWN TABLE

Uplift	Top Connection	Bottom Connection
up to 905 lb	(2) SDWC15600	(3) SDWC15450 w/ anchor bolt within 6"
up to 1215 lb	HTS16	LSTA24, 14-10J Wrap under plate w/ anchor bolt within 6"
up to 2250 lb	(2) HTS16	3/8" full height rod within 6" 2"x2"x18" washer & nut @ top plate Simpson SET-XP epoxy (or equal) w/ 7.5" min. embedment @ foundation (install per mfg. specs)
up to 2430 lb	(2) HTS16	HTT4

MULTI PLY GIRDER TRUSS HOLD DOWN TABLE

Uplift	Top Connection	Bottom Connection
up to 1215 lb	HTS16	LSTA24, 14-10J Wrap under plate w/ anchor bolt within 6"
up to 1810 lb	(4) SDWC15600 (2) Each Ply	HTT4
up to 2250 lb	(2) HTS16	3/8" full height rod within 6" 2"x2"x18" washer & nut @ top plate Simpson SET-XP epoxy (or equal) w/ 7.5" min. embedment @ foundation (install per mfg. specs)
up to 2430 lb	(2) HTS16	HTT4
up to 3645 lb	(3) HTS16	HTT4

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WIND LOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	1
ENCLOSURE CLASSIFICATION (BUILDER MUST FIELD VERIFY)	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57.0(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

16'x7' GARAGE DOOR	16'x7' GARAGE DOOR
+22.0(Vasd) -25.5(Vasd)	+22.0(Vasd) -25.5(Vasd)
+21.7(Vasd) -24.1(Vasd)	+21.7(Vasd) -24.1(Vasd)

B&B Homes

Thomas & Alisha Alfred Res.

FL PE 53915

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JOB NUMBER: 260306

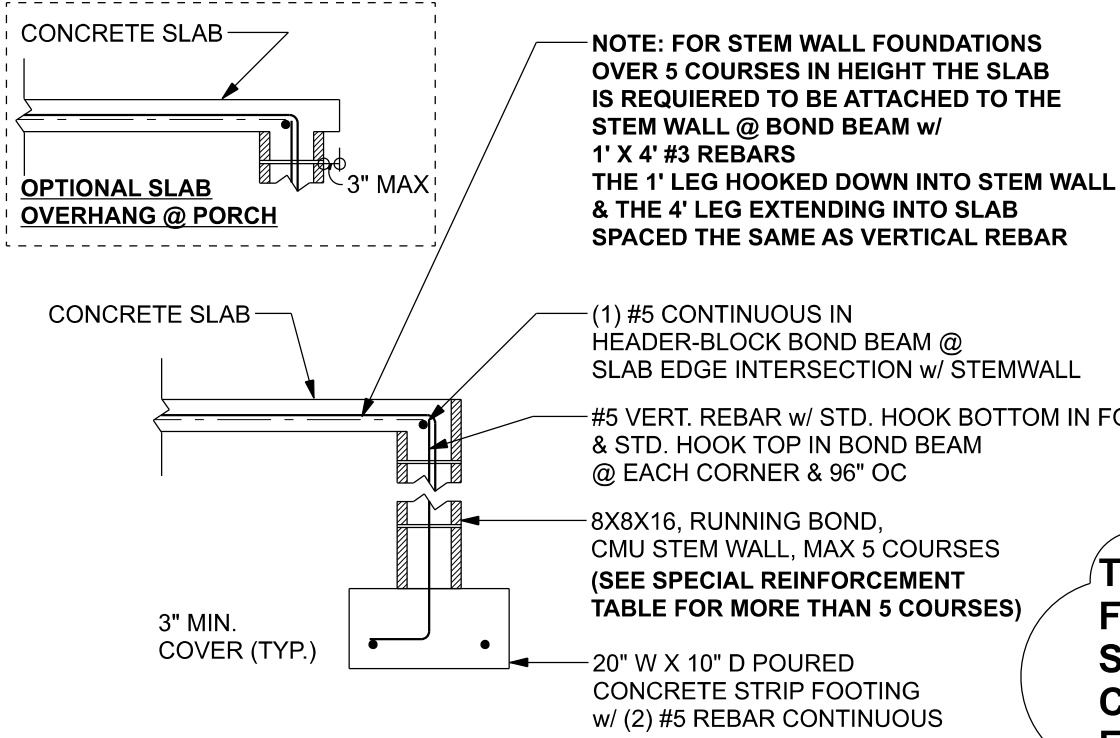
S-1
OF 3 SHEETS

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

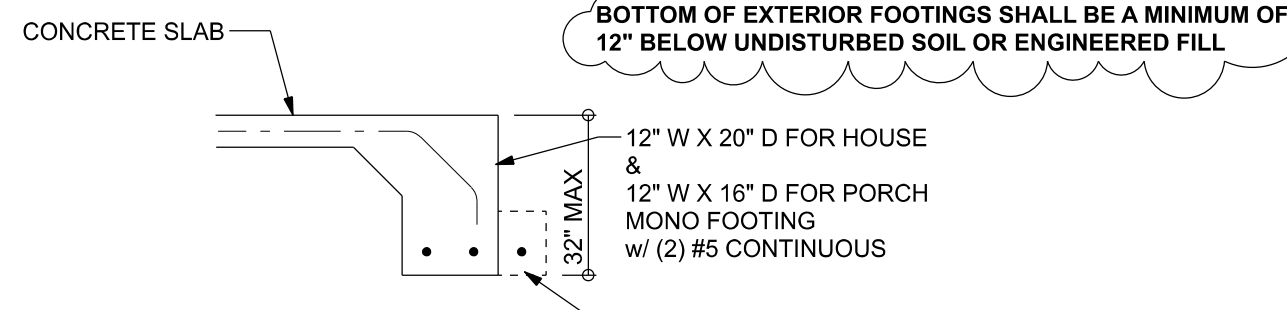
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602).
THE CONTRACTOR AND MASON MUST IMMEDIATELY BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength: 8" block bearing walls Fm = 1500 psi
2.1	Mortar: ASTM C 270, Type N, UNO
2.2	Grout: ASTM C 476, admixtures require approval
2.3	CMU standard: ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard: ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11: ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #8)
2.4F	Coating for corrosion protection: Anchors, sheet metal ties completely embedded in mortar or grout. ASTM A525, Class 680, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection: Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout. ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints: Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

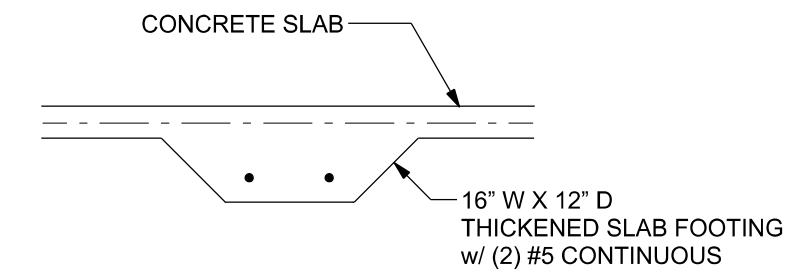


THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING

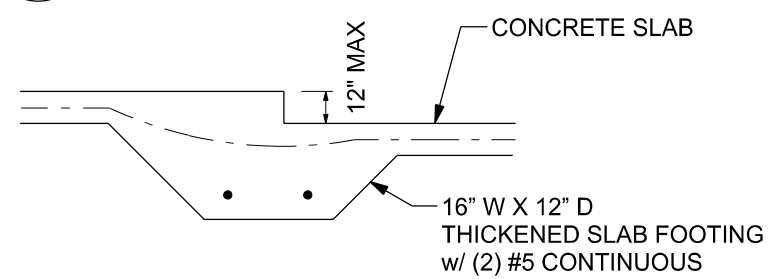
F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



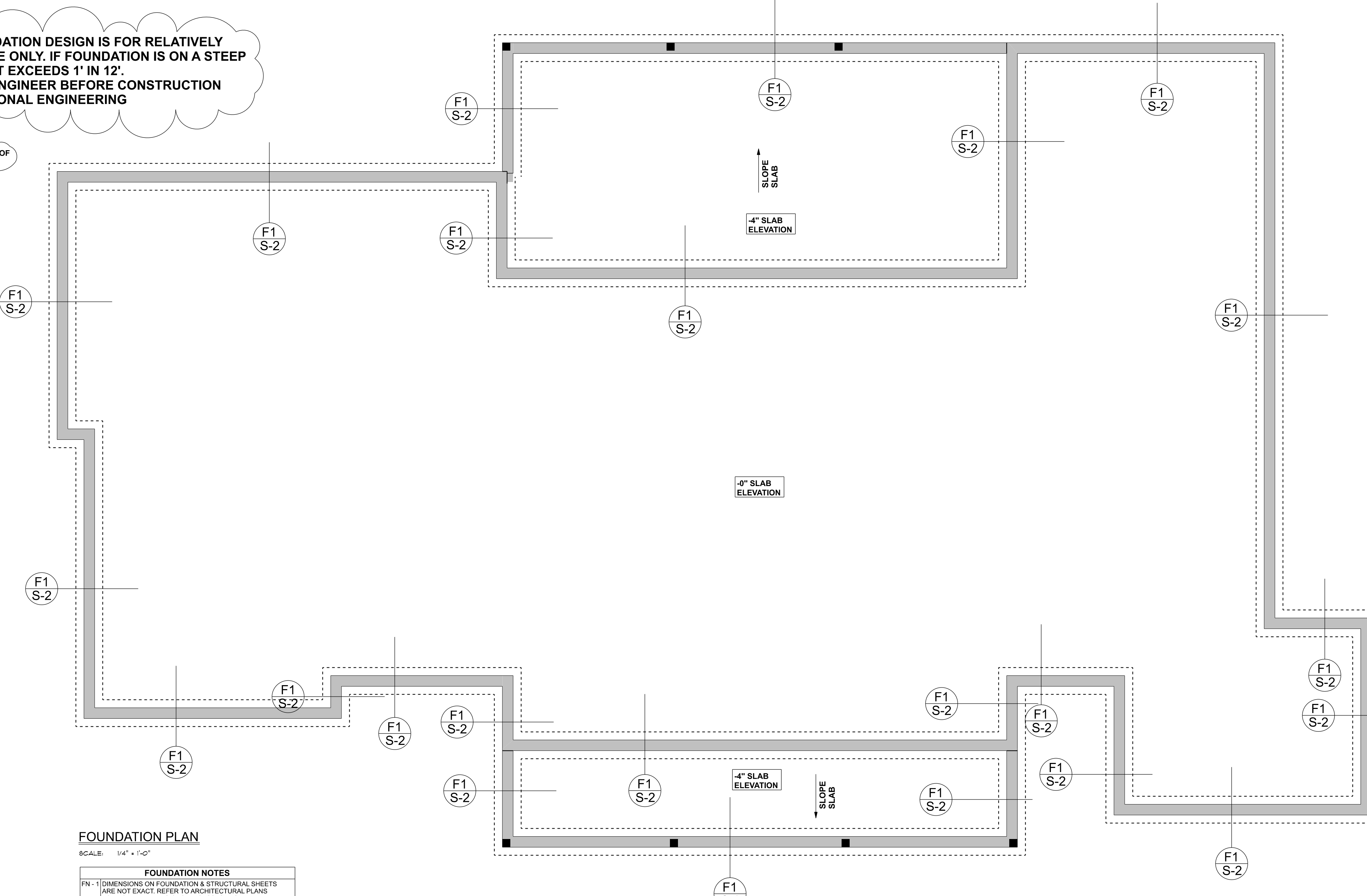
F1 OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ (805) 1/4" x 1/4" WELDED WIRE MESH PLACED ON CHAIRS @ 12" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE TREATED & COMPACTED FILL

B&B Homes
Thomas & Alisha Alfred Res.
PROJECT ADDRESS:
5276 SW 14th Corner Ave., Fort White, FL 32038

FL PE 53915
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

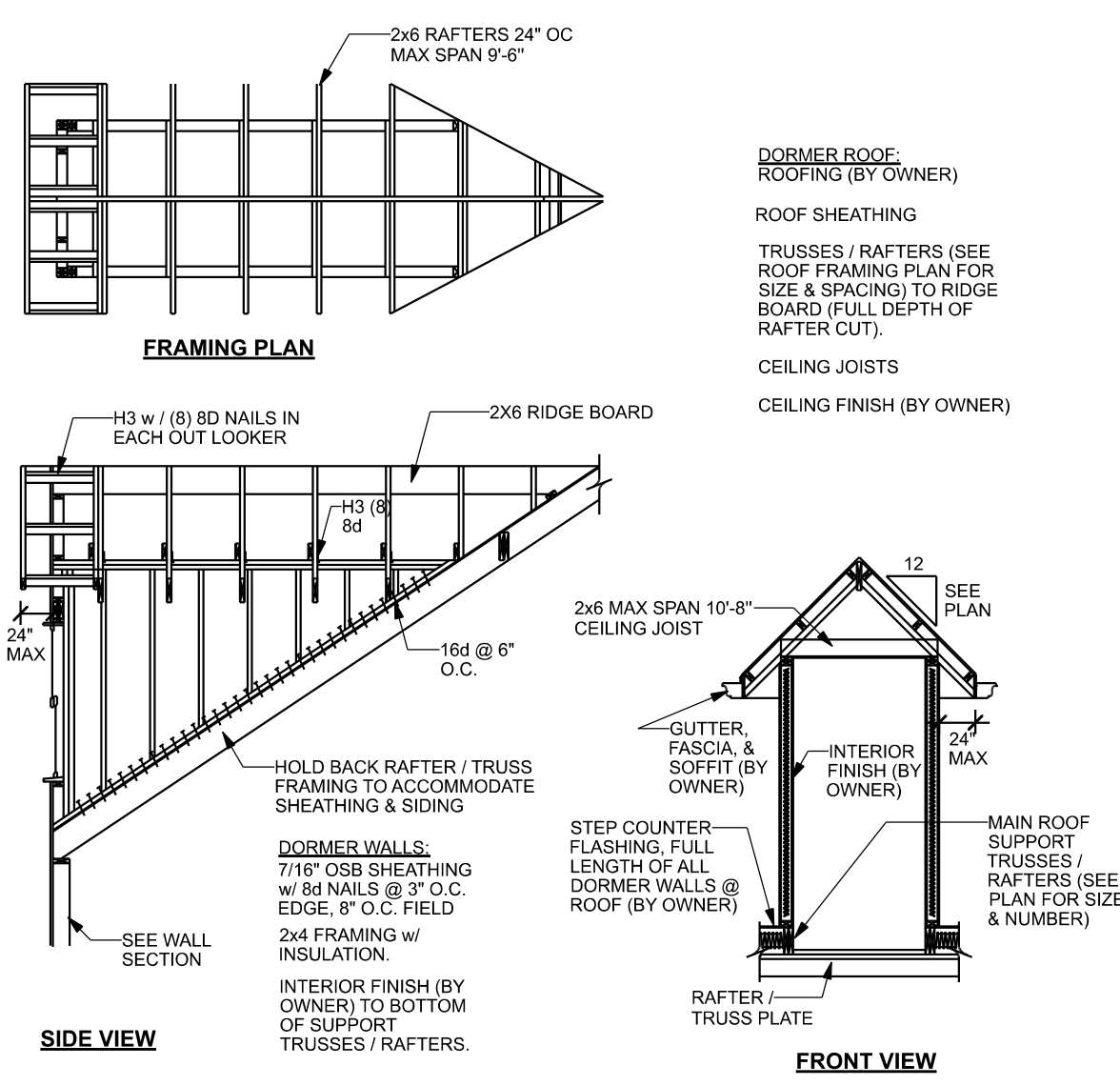
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2022) to the best of my knowledge.

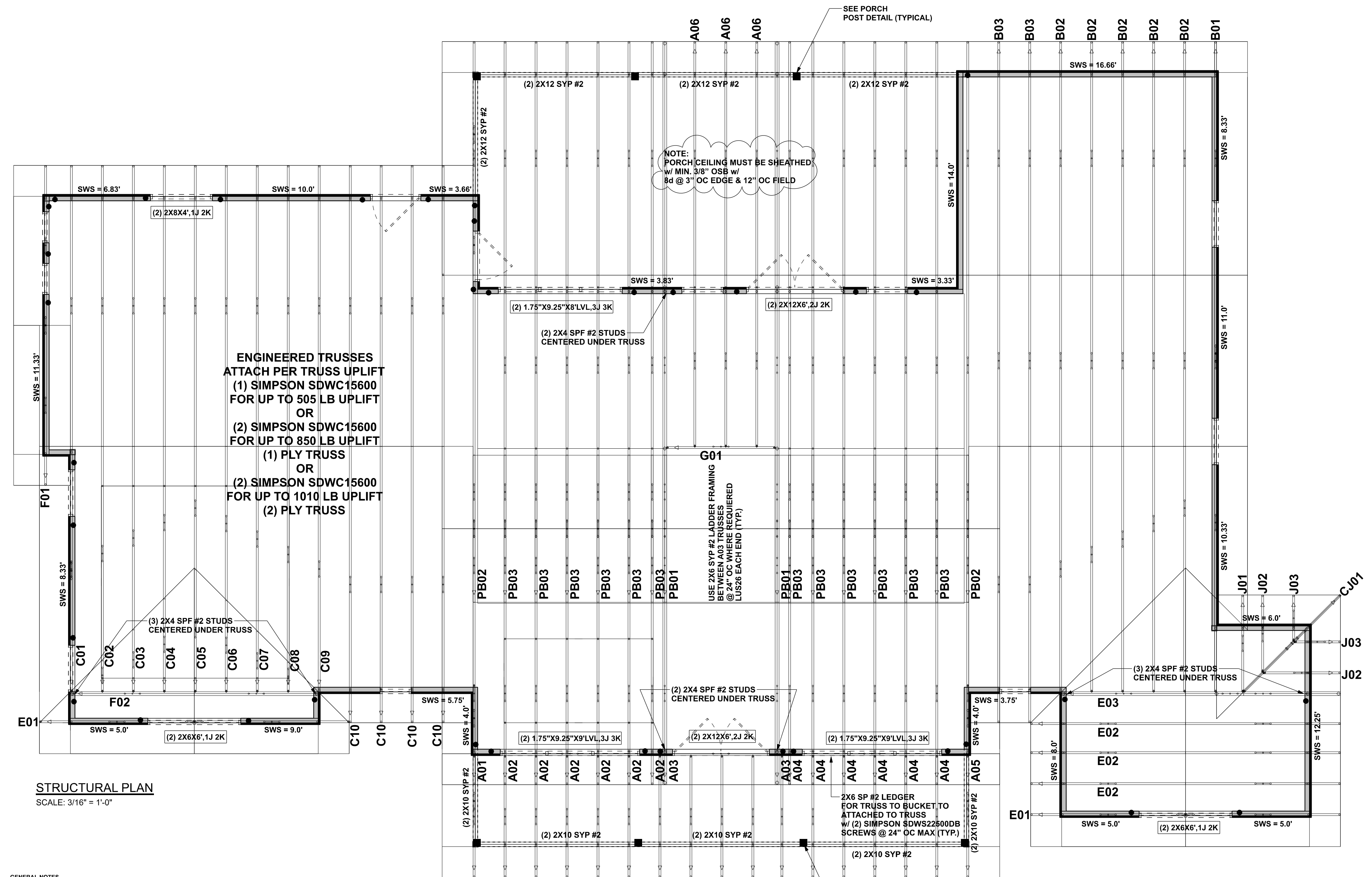
LIMITATION: This design is valid for one building, at specified location.

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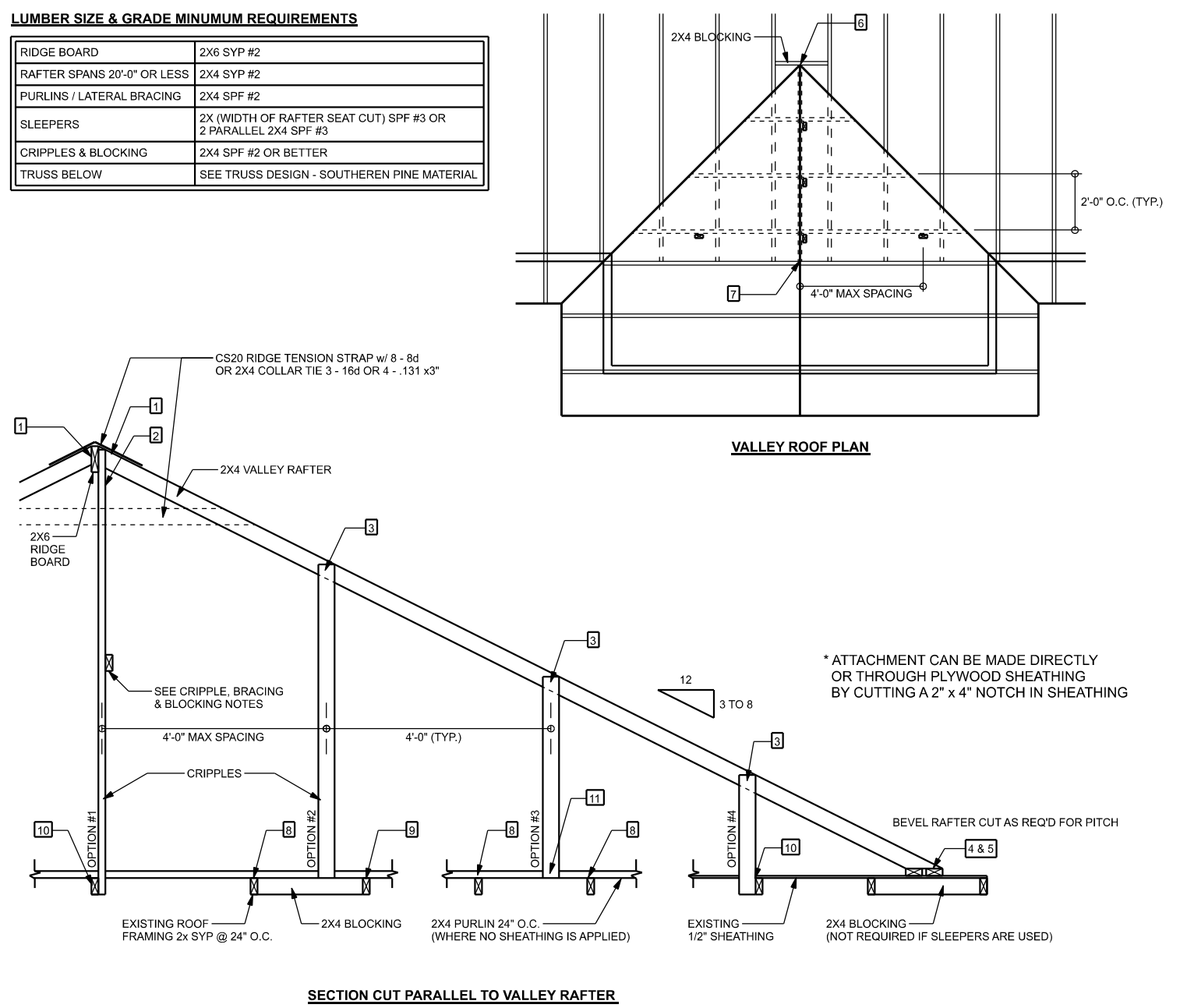
JOB NUMBER:
260306
S-2
OF 3 SHEETS



DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.

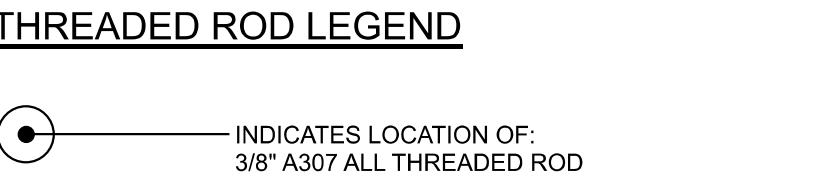
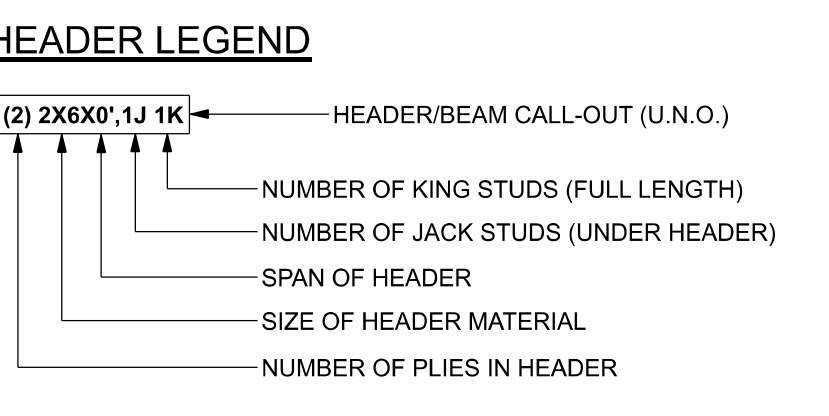


- GENERAL NOTES**
- MAXIMUM RAFTER SPAN: 16'-0" FOR 2X4, 12'-0" FOR 2X6 SYP #2 OR SYP #2. MAXIMUM ROOF AREA PER SUPPORT: 1602 IN ZONES 2 & 3, 2425 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 8'-0" SPAN = 1602)
 - PURLIN REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
 - PURLIN SHOULD OVERLAP EXISTING ONE TRUSS SPACING MINIMUM OF 1" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 3" COMMON WIRE NAILS.
 - THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS: SPACING BETWEEN TRUSSES: 4'-0" OR LESS; MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS; MAXIMUM WIND SPEED: 100 MPH; MAXIMUM MEAN ROOF HEIGHT: 30 FEET; MAXIMUM TOTAL LOADING: 40 PSF.
 - MEETS IRC CHANGES 7-10 WIND REQUIREMENTS - EXPOSURE CATEGORY "C", 15, 64/1-10 - ENCLOSURE CATEGORY.
- CRIPPLE BRACING & BLOCKING NOTES**
- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG. NAILS: W/ 2 - 16D NAILS OR 2X4 1" OR SCAB BRACE NAILS TO PLAT EDGE OF CRIPPLE WITH NAILS @ 2'-0" O.C. - 1" OR SCAB MUST BE MIN. OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO (2) 16D NAILS W/ 1" OR SCAB USE STRESS GRATED LUMBER & SOX OR COMMON NAILS.
 - MINIMUM JOIST OF CRIPPLE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO ROOF BOARD.
 - INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN.
 - LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
 - APPLY ALL BRACING ACCORDANCE TO NDS-1089 SECTION 13. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.
- VALLEY ROOF PLAN MEMBER LEGEND**
- TRUSS
 - TRUSS UNDER VALLEY FRAMING
 - VALLEY RAFTER OR RIDGE
 - CRIPPLE
- CONNECTION REQUIREMENT NOTES**
- | | | |
|----|--|---|
| 1 | 2X4 RAFTERS TO RIDGE | 3-16D OR 6 - 131 x 3" TOE NAILS |
| 2 | CRIPPLE TO RIDGE | 3-16D OR 6 - 131 x 3" TOE NAILS |
| 3 | CRIPPLE TO RAFTERS | 3-16D OR 6 - 131 x 3" TOE NAILS |
| 4 | RAFTER TO SLEEPER OR BLOCKING | 6-16D OR 12 - 131 x 3" TOE NAILS |
| 5 | SLEEPER TO TRUSS | 4-16D OR 8 - 131 x 3" FACE NAILS EACH TRUSS |
| 6 | RIDGE BOARD TO ROOF BLOCK | 3-16D OR 6 - 131 x 3" TOE NAILS |
| 7 | RIDGE BOARD TO TRUSS | 3-16D OR 6 - 131 x 3" TOE NAILS |
| 8 | PURLIN TO TRUSS (TYP) | 3-16D OR 6 - 131 x 3" NAILS |
| 9 | PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN | 4-16D OR 8 - 131 x 3" NAILS |
| 10 | TRUSS TO BLOCKING | 3-16D OR 6 - 131 x 3" END NAILS |
| 11 | CRIPPLE TO TRUSS | 3-16D OR 6 - 131 x 3" FACE NAILS |
| 12 | CRIPPLE TO PURLIN | 3-16D OR 6 - 131 x 3" FACE NAILS |



ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S.

- STRUCTURAL PLAN NOTES**
- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SYP #2 (U.N.O.)
 - SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
 - SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
 - SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
 - SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-03, BC51-B1, BC51-B2, & BC51-B3. BC51-B2, & BC51-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



2X4/6 SP #2 BLOCKING FULLY BLOCKED BETWEEN TRUSSES W/ (4) .131" X 3 1/4" NAILS EACH END

7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 3" OC EDGE, 12" OC FIELD

DETAIL @ TRUSSES WITH RAISED HEELS
SCALE: 1/2" = 1'-0"

CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO
JOB #0825-019

B&B Homes
Thomas & Alisha Allred Res.
PROJECT ADDRESS:
5276 SW Chechucknee Ave., Fort White, FL 32038

FL PE 53915
This item has been digitally signed and sealed by Mark Discoway, P.E. or digital signature 53915. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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JOB NUMBER:
260306
S-3
OF 3 SHEETS