

Columbia County Remodel Permit Application

#19276

For Office Use Only Application # 1909-63 Date Received 9/19 By MG Permit # 38678
 Zoning Official LW/LH Date 9-19-19 Flood Zone X Land Use Ag Zoning A-3
 FEMA Map # N/A Elevation N/A MFE 1' Above River N/A Plans Examiner T.C. Date 9.23.19
 Comments Floor 1' Above Rd., NON Habitable Structure
 NOC Deed or PA Dev Permit # _____ In Floodway Letter of Auth. from Contractor
 F W Comp. letter Owner Builder Disclosure Statement Land Owner Affidavit Ellisville Water App Fee Paid
 Site Plan Env. Health Approval 19-0720 Sub VF Form
9.27.19

Fax 386-755-1220

Applicant (Who will sign/pickup the permit) Mike Todd Phone 386-755-4387

Address 129 NE Colburn Ave Lake City FL 32055

Owners Name Robert + Andrea Smith Phone 386-303-2667

911 Address 2872 NW Brown Rd. Lake City FL 32055

Contractors Name Mike Todd Construction Inc. Phone 386-755-4387

Address 129 NE Colburn Ave Lake City FL 32055

Contractor Email mike@miketoddconstruction.com ***Include to get updates on this job.

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Marty J. Humphries 7932 240th St O'Brien FL 32071

Mortgage Lenders Name & Address _____

Circle the correct power company FL Power & Light Clay Elec. Suwannee Valley Elec. Duke Energy no electrical

Property ID Number 20-35-16-02203-002 Estimated Construction Cost 52,000

Subdivision Name N/A Lot _____ Block _____ Unit _____ Phase _____

Driving Directions from a Major Road 90 W to Brown Rd, 90 right about 3 miles,

Address is on the left

Construction of Carport Detached Commercial OR Residential

Type of Structure (House; Mobile Home; Garage; Exxon) Carport

Use/Occupancy of the building now N/A Is this changing _____

If Yes, Explain, Proposed Use/Occupancy N/A 33.92

Is the building Fire Sprinkled? NO If Yes, blueprints included _____ Or Explain N/A ACREAGE

Entrance Changes (Ingress/Egress) NO If Yes, Explain 1440 TOTAL

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

Columbia County Building Permit Application

CODE: Florida Building Code 2017 6th Edition and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Robert Smith
Print Owners Name

Robert Smith
Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

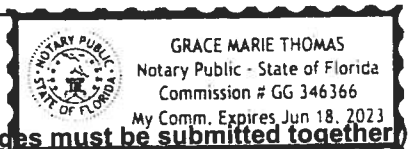
[Signature]
Contractor's Signature

Contractor's License Number CGC006209
Columbia County
Competency Card Number 539 ✓

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 18th day of Sept. 2019
Personally known or Produced Identification

Grace Marie Thomas
State of Florida Notary Signature (For the Contractor)

SEAL:



NOTICE OF COMMENCEMENT

Tax Parcel Identification Number:

20-35-16-02203-002

Clerk's Office Stamp
Inst: 201912022013 Date: 09/19/2019 Time: 1:50PM
Page 1 of 1 B: 1394 P: 2606, P.DeWitt Cason, Clerk of Court
Columbia, County, By: BD
Deputy Clerk

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

- 1. Description of property (legal description):
a) Street (job) Address: 2872 NW Brown Rd Lake City FL 32055
2. General description of improvements: CARPORT
3. Owner Information or Lessee information if the Lessee contracted for the improvements:
a) Name and address: Robert & Andrea Smith 2872 NW Brown Rd Lake City FL 32055
b) Name and address of fee simple titleholder (if other than owner)
c) Interest in property OWNERS
4. Contractor Information
a) Name and address: Mike Todd Construction 129 NE Colburn Ave Lake City FL 32055
b) Telephone No.: 386-755-4387
5. Surety Information (if applicable, a copy of the payment bond is attached):
a) Name and address: N/A
b) Amount of Bond:
c) Telephone No.:
6. Lender
a) Name and address: N/A
b) Phone No.
7. Person within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
a) Name and address: Mike Todd Construction 129 NE Colburn Ave Lake City FL 32055
b) Telephone No.: 386-755-4387
8. In addition to himself or herself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name: Mike Todd OF Mike Todd Construction
b) Telephone No.: 386-755-4387
9. Expiration date of Notice of Commencement (the expiration date will be 1 year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

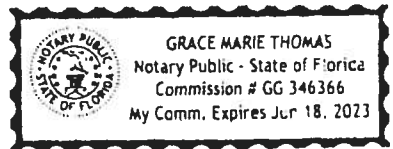
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Lessee, or Owner's or Lessee's Authorized Office/Director/Partner/Manager
Mike Todd - Contractor
Printed Name and Signatory's Title/Office

The foregoing instrument was acknowledged before me, a Florida Notary, this 18th day of Sept. 2009 by:
Grace Marie Thomas as Notary for Mike Todd
(Name of Person) (Type of Authority) (name of party on behalf of whom instrument was executed)

Personally Known OR Produced Identification Type

Notary Signature [Signature] Notary Stamp or Seal:



SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 1909-63 JOB NAME Smith

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL <input type="checkbox"/>	Print Name <u>Matt Burns Electric</u> Signature _____ Company Name: <u>Matt Burns Electric</u> License #: <u>ECT3006531</u> Phone #: <u>386-365-8948</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/A/C <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS <input type="checkbox"/>	Print Name <u>Don Bills</u> Signature <u>Don Bills</u> Company Name: <u>Hometown Plumbing</u> License #: <u>CFC1438890</u> Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input checked="" type="checkbox"/>	Print Name <u>Mike Todd Const.</u> Signature <u>[Signature]</u> Company Name: <u>Mike Todd Construction</u> License #: <u>CBC 006209</u> Phone #: <u>386-755-4387</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/SPRINKLER <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE

Legend

Parcels

2018Aerials



Roads

Roads

others

Dirt

Interstate

Main

Other

Paved

Private

SectionTownshipAndRange

SRWMD Wetlands



2018 Flood Zones

0.2 PCT ANNUAL CHANCE

A

AE

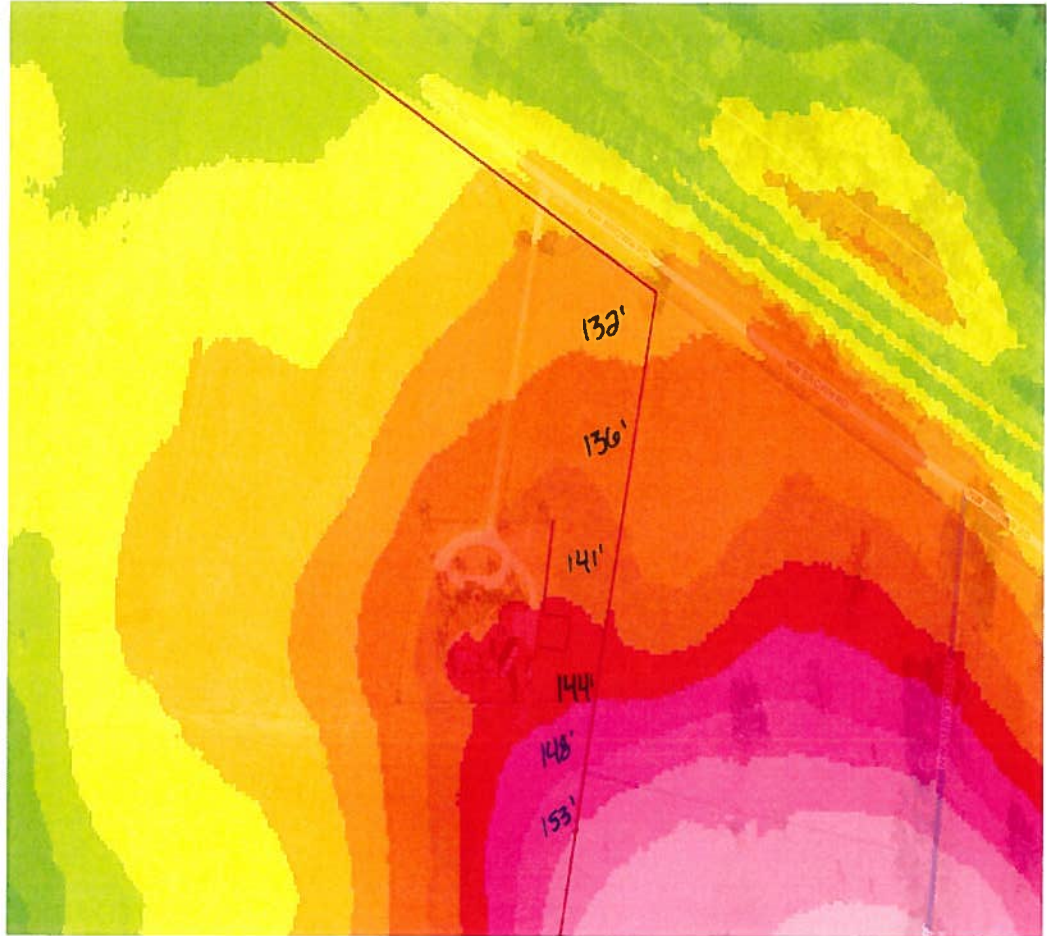
AH

LidarElevations



Columbia County, FLA - Building & Zoning Property Map

Printed: Thu Sep 19 2019 16:14:45 GMT-0400 (Eastern Daylight Time)



Parcel Information

Parcel No: 20-3S-16-02203-002
Owner: SMITH ROBERT D & ANDREA G
Subdivision:
Lot:
Acres: 33.4630928
Deed Acres: 33.72 Ac
District: District 3 Bucky Nash
Future Land Uses: Agriculture - 3
Flood Zones:
Official Zoning Atlas: A-3, PRD

All data, information, and maps are provided "as is" without warranty or any representation of accuracy, timeliness of completeness. Columbia County, FL makes no warranties, express or implied, as to the use of the information obtained here. There are no implied warranties of merchantability or fitness for a particular purpose. The requester acknowledges and accepts all limitations, including the fact that the data, information, and maps are dynamic and in a constant state of maintenance, and update.

Columbia County Property Appraiser

Jeff Hampton

2019 Preliminary Certified Values

updated: 8/14/2019

Parcel: << 20-3S-16-02203-002 >>

Aerial Viewer Pictometry Google Maps

Owner & Property Info

Result: 209 of 299

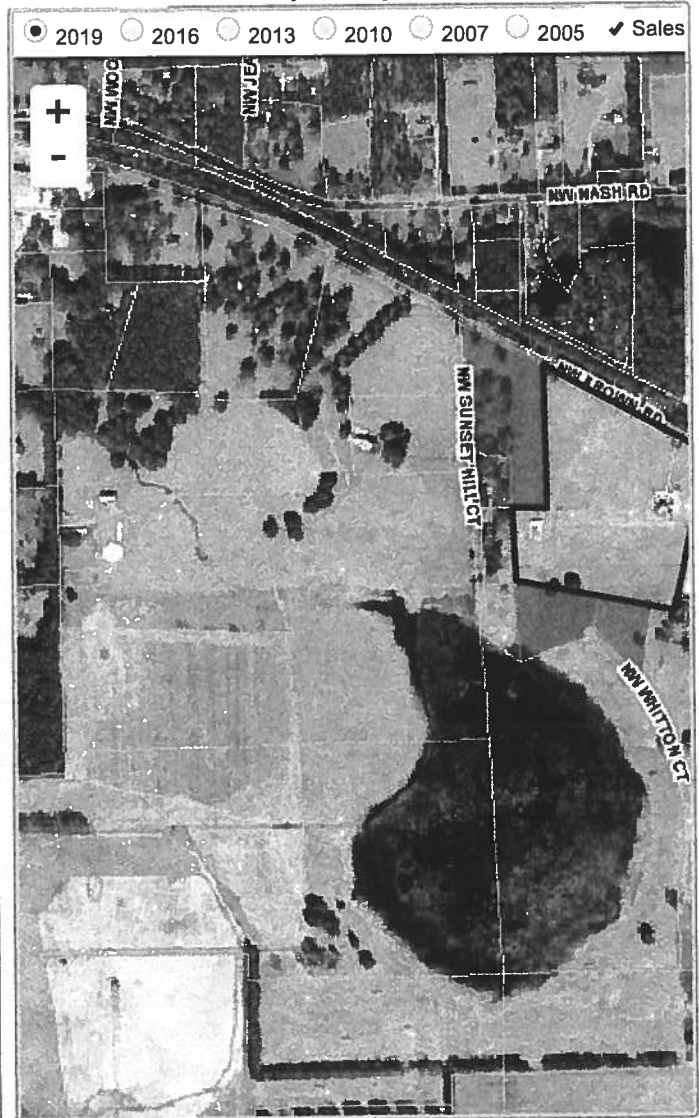
Owner	SMITH ROBERT D & ANDREA G 2872 NW BROWN RD LAKE CITY, FL 32055		
Site	2872 BROWN RD, LAKE CITY		
Description*	COMM NW COR, RUN S 855.43 FT, SE 60 DEG 677.73 FT FOR POB, CONT SE 60 DEG 1270.11 FT, SW 13 DEG 1109.31 FT, NW 77 DEG 1154.78 FT, N 481.05 FT, E 238.42 FT, N 975.44 FT TO POB. ORB 893-1637,		
Area	33.72 AC	S/T/R	20-3S-16
Use Code**	IMPROVED A (005000)	Tax District	3

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.

**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Property & Assessment Values

2018 Certified Values		2019 Preliminary Certified	
Mkt Land (1)	\$6,742	Mkt Land (1)	\$6,742
Ag Land (1)	\$7,852	Ag Land (1)	\$7,852
Building (1)	\$224,469	Building (1)	\$242,436
XFOB (6)	\$47,855	XFOB (6)	\$47,076
Just	\$524,181	Just	\$541,369
Class	\$286,918	Class	\$304,106
Appraised	\$286,918	Appraised	\$304,106
SOH Cap [?]	\$5,829	SOH Cap [?]	\$15,310
Assessed	\$283,558	Assessed	\$288,796
Exempt	HX H3 \$50,000	Exempt	HX H3 \$50,000
Total Taxable	county:\$233,558 city:\$233,558 other:\$233,558 school:\$258,558	Total Taxable	county:\$238,796 city:\$238,796 other:\$238,796 school:\$263,796



▼ Sales History

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
12/15/1999	\$188,900	893/1637	WD	V	Q	

▼ Building Characteristics

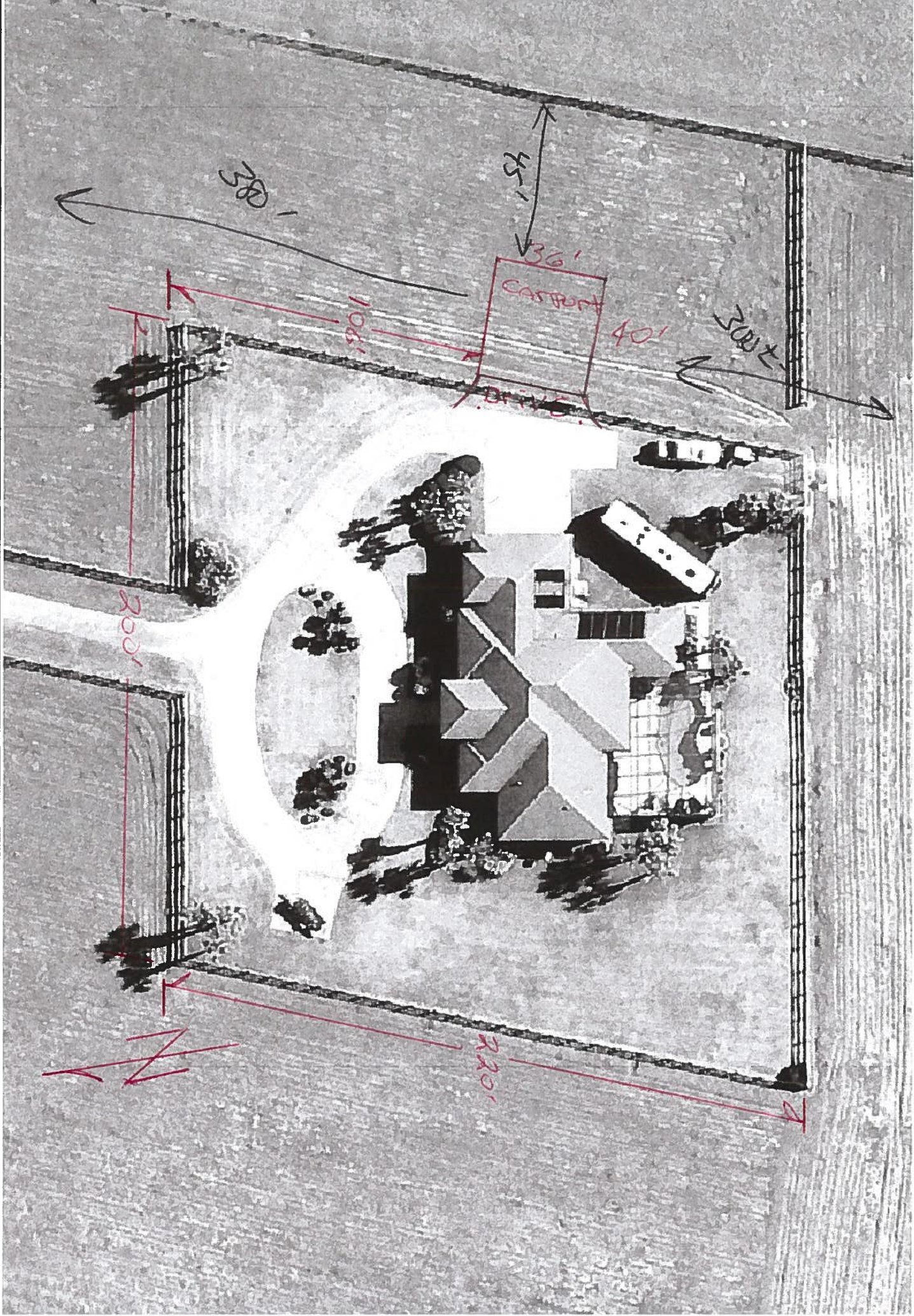
Bldg Sketch	Bldg Item	Bldg Desc*	Year Blt	Base SF	Actual SF	Bldg Value
Sketch	1	SINGLE FAM (000100)	2004	2861	4331	\$242,436

*Bldg Desc determinations are used by the Property Appraisers office solely for the purpose of determining a property's Just Value for ad valorem tax purposes and should not be used for any other purpose.

▼ Extra Features & Out Buildings (Codes)

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0190	FPLC PF	2004	\$2,400.00	2.000	0 x 0 x 0	(000.00)
0280	POOL R/CON	2004	\$10,895.00	512.000	16 x 32 x 0	(000.00)
0282	POOL ENCL	2004	\$7,403.00	1645.000	35 x 47 x 0	(000.00)
0166	CONC,PAVMT	2004	\$19,128.00	9564.000	0 x 0 x 0	(000.00)
0040	BARN,POLE	2005	\$4,000.00	1.000	0 x 0 x 0	(000.00)

Brown Road

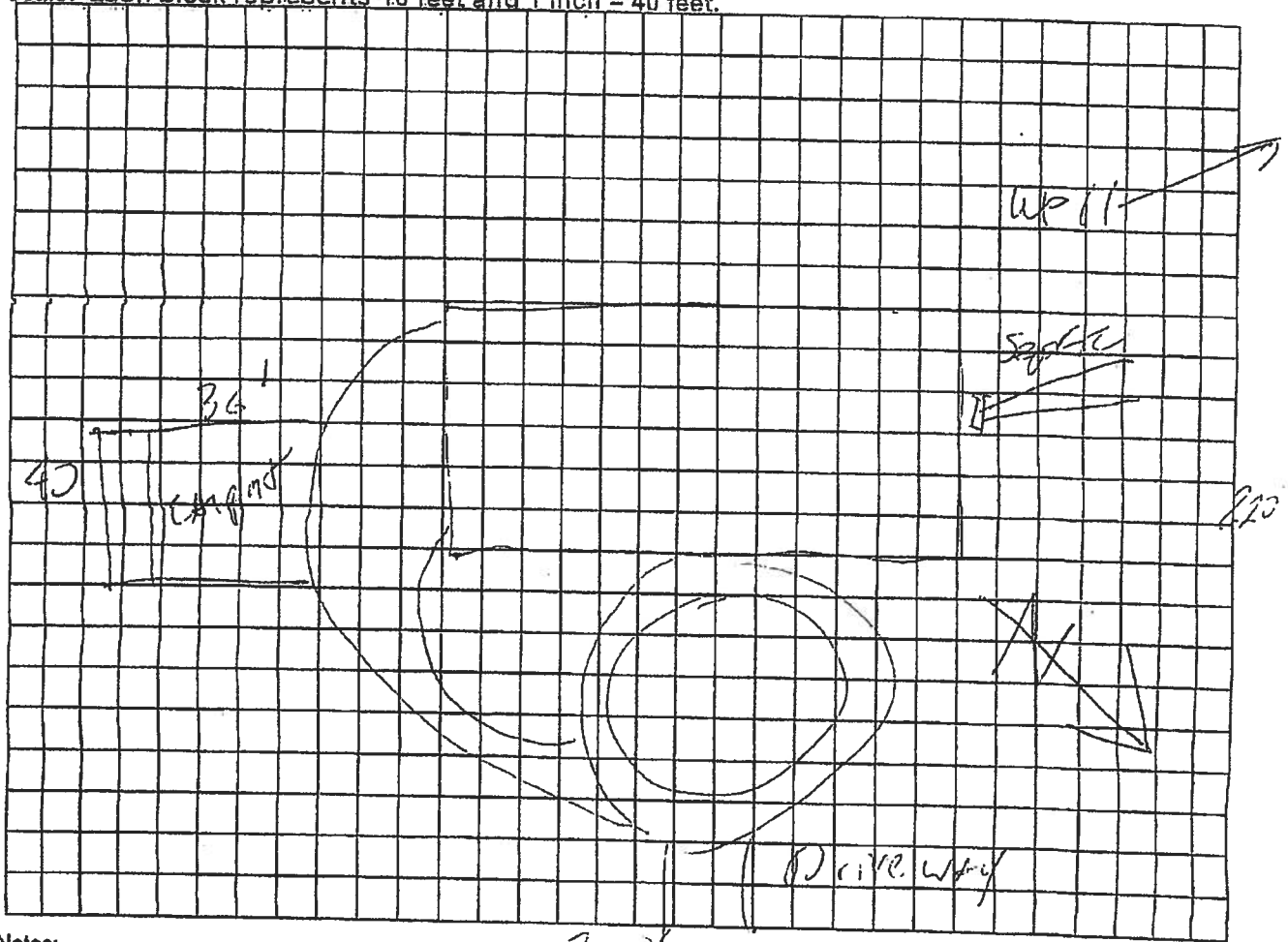


STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number 19-0720

----- PART II - SITEPLAN -----

Scale: Each block represents 10 feet and 1 inch = 40 feet.

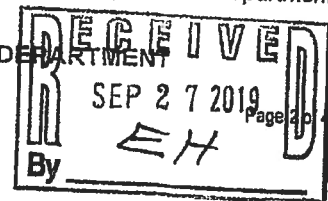


Notes: _____

* Site Plan submitted by: [Signature] TITLE Agent DATE: 9/26/19
 Plan Approved [Signature] Not Approved _____ Date _____
 By [Signature] EST. Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 08/08 (Obsoletes previous editions which may not be used) Incorporated: 64E-8.001, FAC (Stock Number: 5744-002-4015-8)





STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 19-0720
DATE PAID: 9/25/19
FEE PAID: 1000
RECEIPT #: 142533

APPLICATION FOR:

- New System
- Existing System
- Holding Tank
- Innovative
- Repair
- Abandonment
- Temporary

APPLICANT: Bob + Candrea Smith

AGENT: [Signature]

TELEPHONE: 386-755-458

MAILING ADDRESS: 2872 NW Brown Rd
Lake City, FL 32055

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3) (m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 1 BLOCK: 1 SUBDIVISION: _____ PLATTED: 1

PROPERTY ID #: 20-35-16-02203-002 ZONING: R/S I/M OR EQUIVALENT: [Y] N

PROPERTY SIZE: 33 ACRES WATER SUPPLY: PRIVATE PUBLIC [] <=2000GPD [] >2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y] N DISTANCE TO SEWER: _____ FT

PROPERTY ADDRESS: 2872 NW Brown Rd Lake City FL 32055

DIRECTIONS TO PROPERTY: turn 90 West to Brown Rd
Ground curve 1.2 mi on Rd.

Heart Land Ranch sign on left

BUILDING INFORMATION

RESIDENTIAL [] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	<u>Carpenter</u>	<u>0</u>	<u>1440</u>	
2				
3				
4				

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	<u>Carpenter</u>	<u>0</u>	<u>1440</u>	
2				
3				
4				

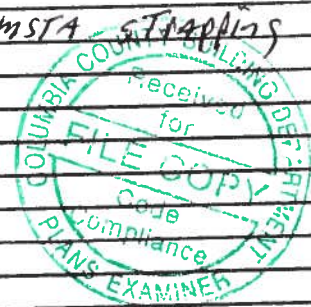
Floor/Equipment Drains [] Other (Specify) _____

SIGNATURE: [Signature]


DATE: 9/25/19

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	plastpro	Fiberglass 6 panel	FL 17347.9
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	PBT 5-400 Series	Vinyl Low E	FL 239
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS	Kaycan	Aluminum Soffit	FL 16503
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	GAF	Asphalt Shingles	FL 10124-R11
B. NON-STRUCTURAL METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCTURAL COMPONENTS			
A. WOOD CONNECTORS	Simpson	LSTA - MSTA STAPLES	FL-13872-R1
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			



The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite: 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.


 Contractor OR Agent Signature _____ Date 9/17/15

NOTES: _____



BC CALC® Member Report

PASSED

Build 7192

August 30, 2019 15:22:24

Job name: 2067658 Smith

File name:

Address:

Specifier:

City, State, Zip: Lake City, FL

Designer: Brian Cannady

Builder: Mike Todd Construction

Company: Builders FirstSource

Code reports: ESR-1040

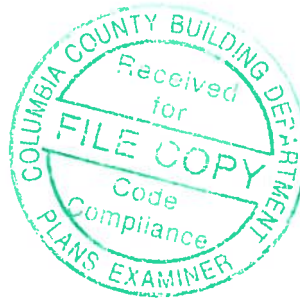
Member Summary

Design	Description	Results	Product	Plies	Control
Design					
RB01		Passed	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	Pos. Moment 99.9%
RB02		Passed	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	Pos. Moment 78.6%
RB03		Passed	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	Pos. Moment 94.1%

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

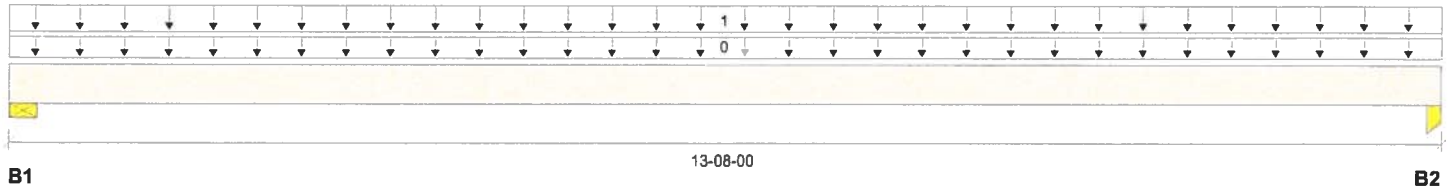


BC CALC® Member Report

August 30, 2019 15:22:24

Description:

This report has a cover page. See the cover page(s) for project data and important information regarding this analysis and product installation.



Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 5-1/2"		2427 / 0		1872 / 0	2708 / 0
B2, 2-1/2"		2340 / 0		1805 / 0	2611 / 0

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-08-00	Top		18				00-00-00
1	36' span roof load	Unf. Area (lb/ft²)	L	00-00-00	13-08-00	Top		331		269	389	01-00-00

Controls Summary

	Value	% Allowable	Duration	Case	Location
Pos. Moment	15893 ft-lbs	99.9%	125%	4	06-11-08
End Shear	4067 lbs	27.5%	125%	4	01-05-06
Total Load Deflection	L/442 (0.356")	40.7%	n/a	7	06-11-08
Live Load Deflection	L/798 (0.197")	30.1%	n/a	16	06-11-08
Max Defl.	0.356"	35.6%	n/a	7	06-11-08
Span / Depth	13.3				

Bearing Supports

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate 5-1/2" x 5-1/4"	5862 lbs	n/a	27.1%	Unspecified
B2	Column 2-1/2" x 5-1/4"	5652 lbs	n/a	57.4%	Unspecified

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is braced at all supports. See engineering report for the unbraced length.

BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Install screws from both sides, staggering screws by half of the spacing to avoid splitting.

Member has no side loads.

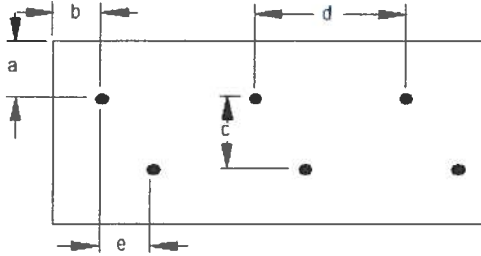


BC CALC® Member Report

August 30, 2019 15:22:24

Description:

This report has a cover page. See the cover page(s) for project data and important information regarding this analysis and product installation.

Connection Diagram: Full Length of Member

a minimum = 1-1/2"

c = 8-7/8"

b minimum = 4"

d = 24"

e minimum = 1"

Install screws from both sides, staggering screws by half of the spacing to avoid splitting.

Member has no side loads.

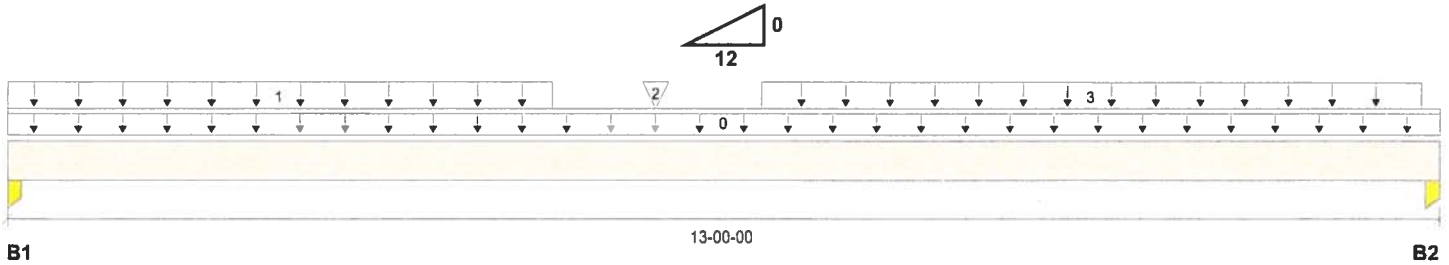
Connectors are: SDS 1/4 x 3-1/2

BC CALC® Member Report

August 30, 2019 15:22:24

Description:

This report has a cover page. See the cover page(s) for project data and important information regarding this analysis and product installation.



Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 2-1/2"		2201 / 0		2050 / 0	2453 / 0
B2, 5-1/4"		1288 / 0		1338 / 0	1375 / 0

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 160%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-00-00	Top	18					00-00-00
1	36' span roof load	Unf. Area (lb/ft ²)	L	00-00-00	04-11-04	Top	331		269	389		01-00-00
2	7' Setback GT	Conc. Pt. (lbs)	L	05-10-08	05-10-08	Top	1231		1613	1449		n/a
3	Corner set	Trapezoidal (lb/ft)	L	06-10-00	12-10-00	Top	72		68	84		n/a
							58		81	84		

Controls Summary

	Value	% Allowable	Duration	Case	Location
Pos. Moment	14251 ft-lbs	78.6%	125%	4	05-10-08
End Shear	3770 lbs	25.5%	125%	4	01-02-06
Total Load Deflection	L/545 (0.275")	33.0%	n/a	7	06-01-06
Live Load Deflection	L/941 (0.159")	25.5%	n/a	16	06-01-06
Max Defl.	0.275"	27.5%	n/a	7	06-01-06
Span / Depth	12.6				

Bearing Supports

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column 2-1/2" x 5-1/4"	5578 lbs	n/a	56.7%	Unspecified
B2	Column 5-1/4" x 5-1/4"	3323 lbs	n/a	16.1%	Unspecified

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is braced at all supports. See engineering report for the unbraced length.

BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Install screws from both sides, staggering screws by half of the spacing to avoid splitting.

Member has no side loads.



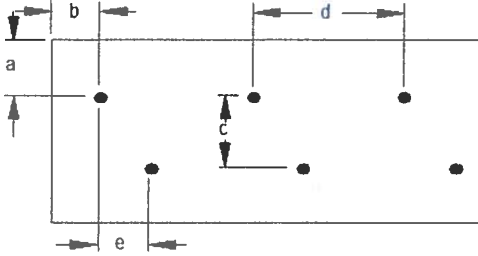
BC CALC® Member Report

August 30, 2019 15:22:24

Description:

This report has a cover page. See the cover page(s) for project data and important information regarding this analysis and product installation.

Connection Diagram: Full Length of Member



a minimum = 1-1/2"

c = 8-7/8"

b minimum = 4"

d = 24"

e minimum = 1"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

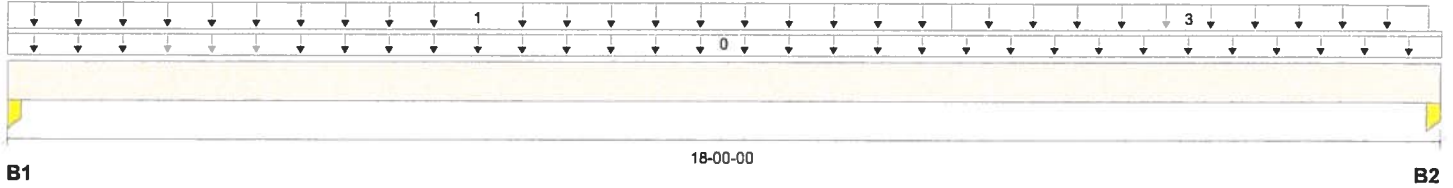
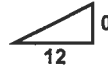
Install screws from both sides, staggering screws by half of the spacing to avoid splitting.

Member has no side loads.

Connectors are: SDS 1/4 x 3-1/2

Description:

This report has a cover page. See the cover page(s) for project data and important information regarding this analysis and product installation.



Total Horizontal Product Length = 18-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 2-1/2"		873 / 0		529 / 0	900 / 0
B2, 2-1/2"		766 / 0		593 / 0	774 / 0

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 160%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	18-00-00	Top	12					00-00-00
1	7' End Jack	Unf. Area (lb/ft²)	L	00-00-00	11-10-00	Top	87			57	103	01-00-00
3	Corner set	Trapezoidal (lb/ft)	L	11-10-00	17-10-00	Top	72			68	84	n/a
							58			81	84	

Controls Summary

	Value	% Allowable	Duration	Case	Location
Pos. Moment	7527 ft-lbs	94.1%	125%	4	08-08-05
End Shear	1532 lbs	15.5%	125%	4	01-02-06
Total Load Deflection	L/472 (0.45")	38.1%	n/a	7	09-00-00
Live Load Deflection	L/898 (0.237")	26.7%	n/a	16	09-00-00
Max Defl.	0.45"	45.0%	n/a	7	09-00-00
Span / Depth	17.9				

Bearing Supports

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column 2-1/2" x 3-1/2"	1945 lbs	n/a	29.6%	Unspecified
B2	Column 2-1/2" x 3-1/2"	1791 lbs	n/a	27.3%	Unspecified

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is braced at all supports. See engineering report for the unbraced length.

BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Install Screws with screw heads in the loaded ply.

Member has no side loads.



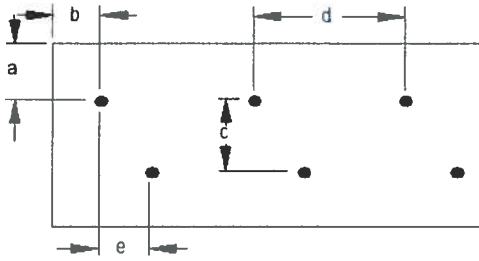
BC CALC® Member Report

August 30, 2019 15:22:24

Description:

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Connection Diagram: Full Length of Member



a minimum = 1-1/2"

c = 8-7/8"

b minimum = 4"

d = 24"

e minimum = 1"

Install Screws with screw heads in the loaded ply.

Member has no side loads.

Connectors are: SDS 1/4 x 3-1/2

BEARING HEIGHT SCHEDULE

10'-0"

NOTES:

- 1) ALL WORK IS TO BE COMPLETED WITHIN THE SPECIFIED TIME FRAME.
- 2) ALL MATERIALS AND METHODS TO BE USED SHALL BE APPROVED BY THE ENGINEER.
- 3) ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL ORDINANCES.
- 4) ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL ORDINANCES.
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- 10) ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE LATEST EDITIONS OF THE INTERNATIONAL BUILDING CODES AND ALL APPLICABLE LOCAL ORDINANCES.

Builders
FirstSource

Jacksonville
PHONE 904-772-5100 FAX 904-772-7473

Tampa
PHONE 813-447-8871 FAX 813-447-8996

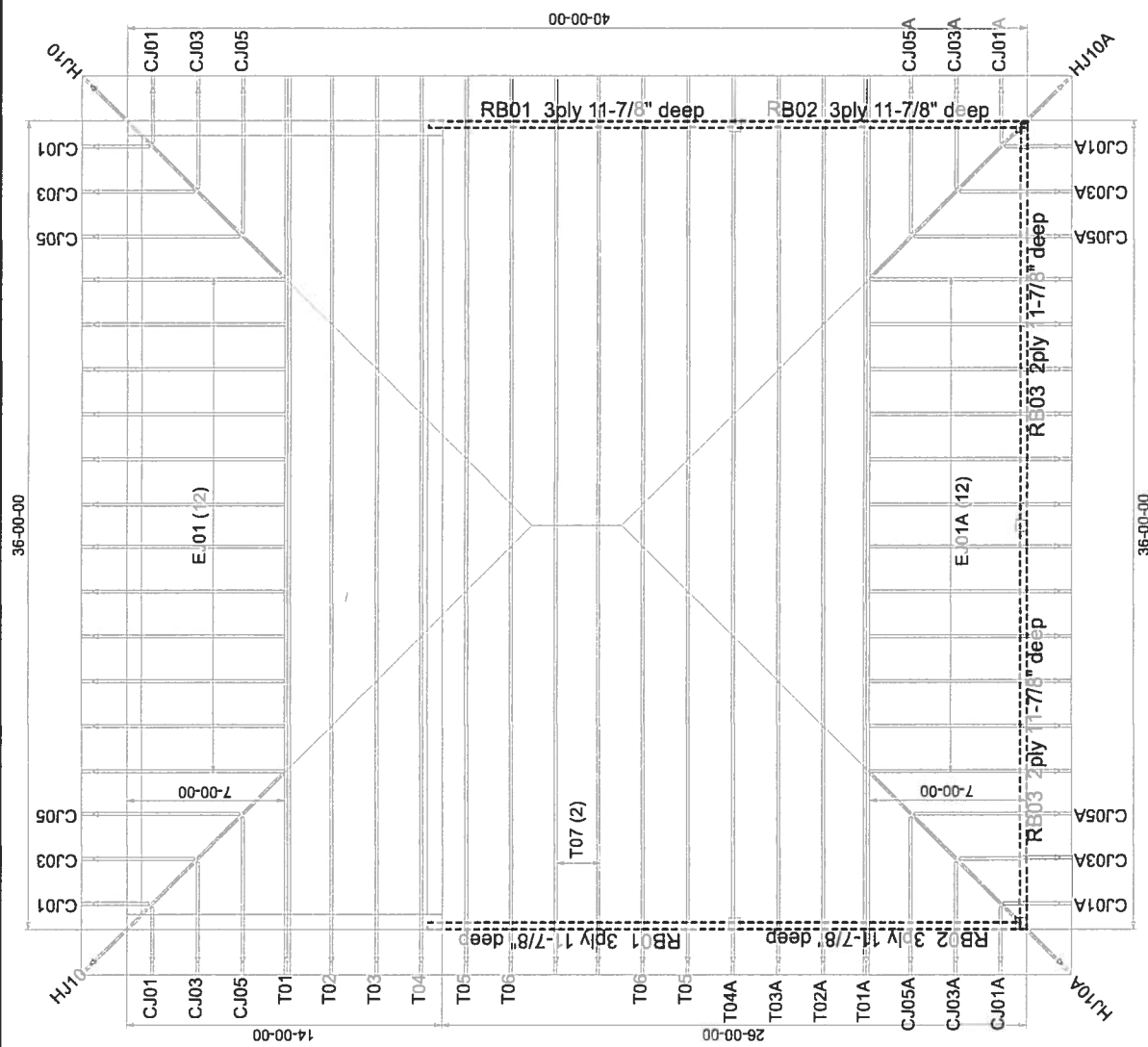
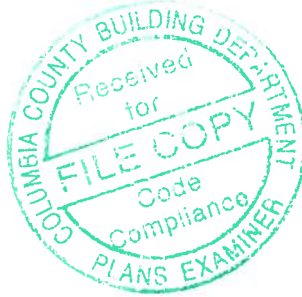
Lake City
PHONE 386-755-6004 FAX 386-755-7473

MIKE TODD CONST.

SMITH CARPORT

DATE: 8-30-19
BY: KLH
PROJECT: 2067658
JOB NO: 2067658

6/12 PITCH 24" O/H



FL Approval Codes - Mitek Plates #'s 2197.2 - 2197.4, Versa-Lam #1644-R4 & BCI Joists #1392-R4

Smith Carport

Columbia County, Florida

Wind Load Analysis Requirements

(In Compliance with the 2017 Florida Building Code)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Carport:

Footprint: 40' deep x 36' wide overall dimensions with 36' wide x 14' deep enclosed work/storage areas and 36' wide x 26' deep carport area (See plans by Mike Todd)

Walls: 8" masonry block wall with stucco exterior and furring strips, rigid insulation and blue-board hard-coat interior.

Roof Structure: Pre-engineered roof trusses at 2' on center and 15/32" OSB sheathing with Synthetic roof underlayment and architectural shingles

Roof Type: Hip roof (analyzed for 2' eave overhangs and carport)

Foundation: monolithic slab (see attached foundation details)

Windload Data and Exposure:

Basic Wind Speed = 120 mph(Vult)

Importance Factor = 1.0

Exposure category = B

Height and Exposure Adjustment Coefficient = 1.0

Residential Occupancy = Group R3

Mean roof height = 16'

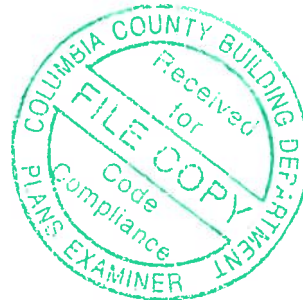
Roof Cross Slope = 6:12

Eave Overhang= (Analyzed for 2"eave overhangs)

Wall Height = 10'

Shear Wall locations = exterior walls only

Component and Cladding Pressures = Roof(Zone 1=10.0,-14.0, Zone 2=10.0,-24.0, Zone 3=10.0,-36.0), Wall(Zone 4=15.5,-16.0, Zone 5=15.5,-20.0)(units are psf)



The following requirements supercede the plan requirements by "Mike Todd" where applicable.

Nailing Pattern Requirements:

Roof sheathing: Shall be 15/32" OSB min. nailed with 8d ring-shank nails 6" on center including overhang areas

Strapping and Anchor Requirements:

truss to exterior wall plate, porch header locations: Install one Simpson model H10A hurricane anchor each location for common/tier trusses and one Simpson model H2.5A hurricane clip at jack trusses. Install Simpson model H10A-2 anchor for double trusses with 1-H2.5A clip in addition. At 8" masonry walls

Marty J. Humphries
8-26-19

install Simpson Hetal12 embedded anchors at each truss location.

Carport columns: Install Simpson HD5B hold down anchor connected to top of masonry column with 1 – 5/8”x12” anchor bolt or 1 – 5/8” steel all-thread embedded 12” into top of masonry column and epoxied with Simpson SET-XP epoxy. Connect HD5B to LVL header with 2-3/4” bolts. At block wall cast a block-out 5” deep into masonry block wall and install 1 – 5/8” steel all-threads embedded 12” into masonry wall and install nut, lock washer, and 2”x 2” plate washer at all-thread.

Footer Requirements:

Exterior wall Monolithic footer: 18” min. x 16” wide with 3-#5 rebar continuous (See attached Detail)

Carport Monolithic footer: 18” deep x 16” wide with 3-#5 rebar continuous (See attached Detail)

Header Requirements:

Carport: Header shall be 3 – 1.75”x14” LVL’s(Fb=2250 min., 1.5E min.) nailed together with 4-12d nails 10” on center

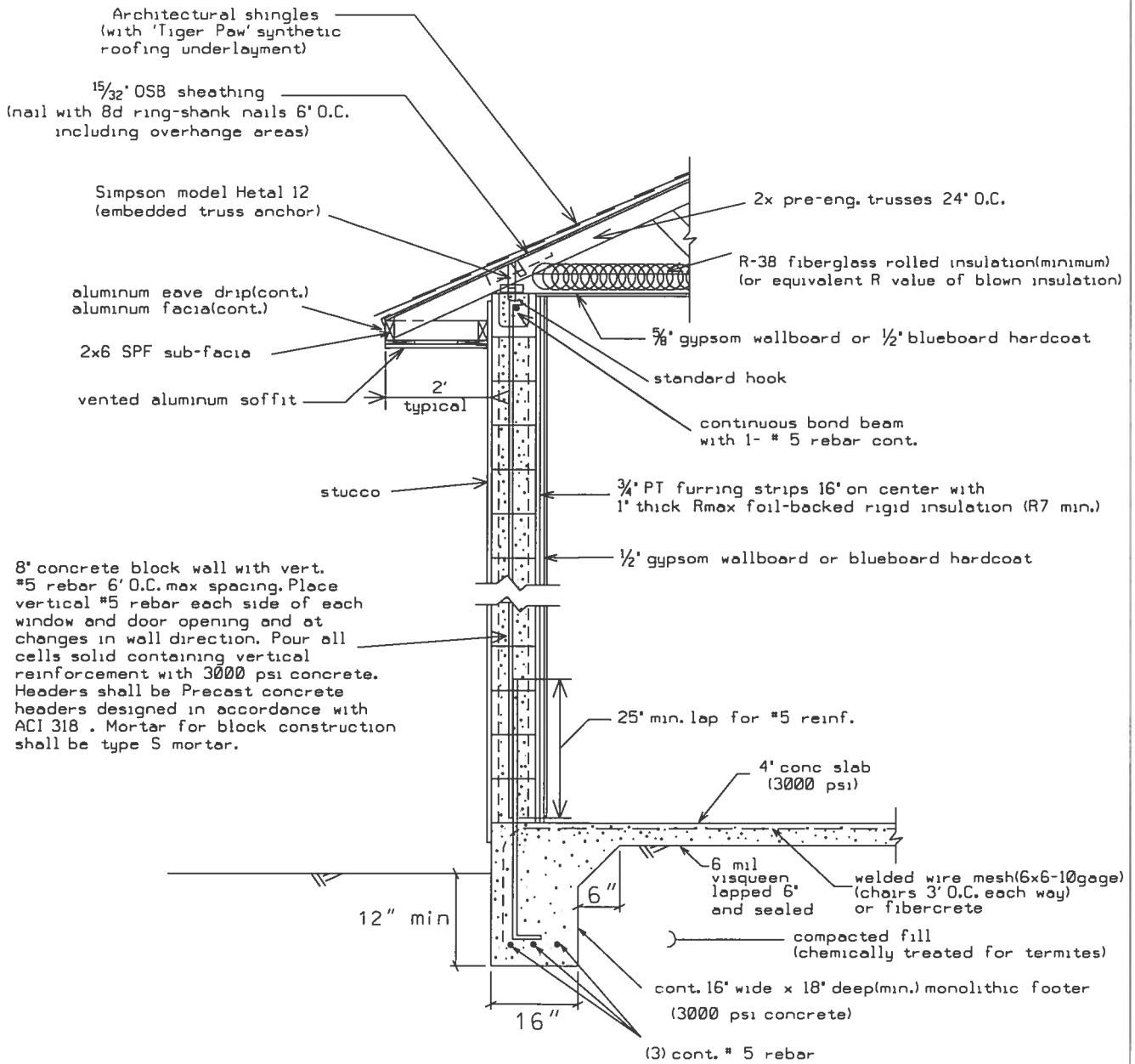
Porch/Carport Ceilings:

Install 1x4 SPF lathes 16” on center connected to trusses with 2-8d nails each location and covered with solid aluminum soffit material, vinyl soffit material or hardi-panel.

Note: Masonry column height shall be constructed such that finished top plate of masonry wall matches finished top height of LVL carport header.

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

Muty 571
8-26-19

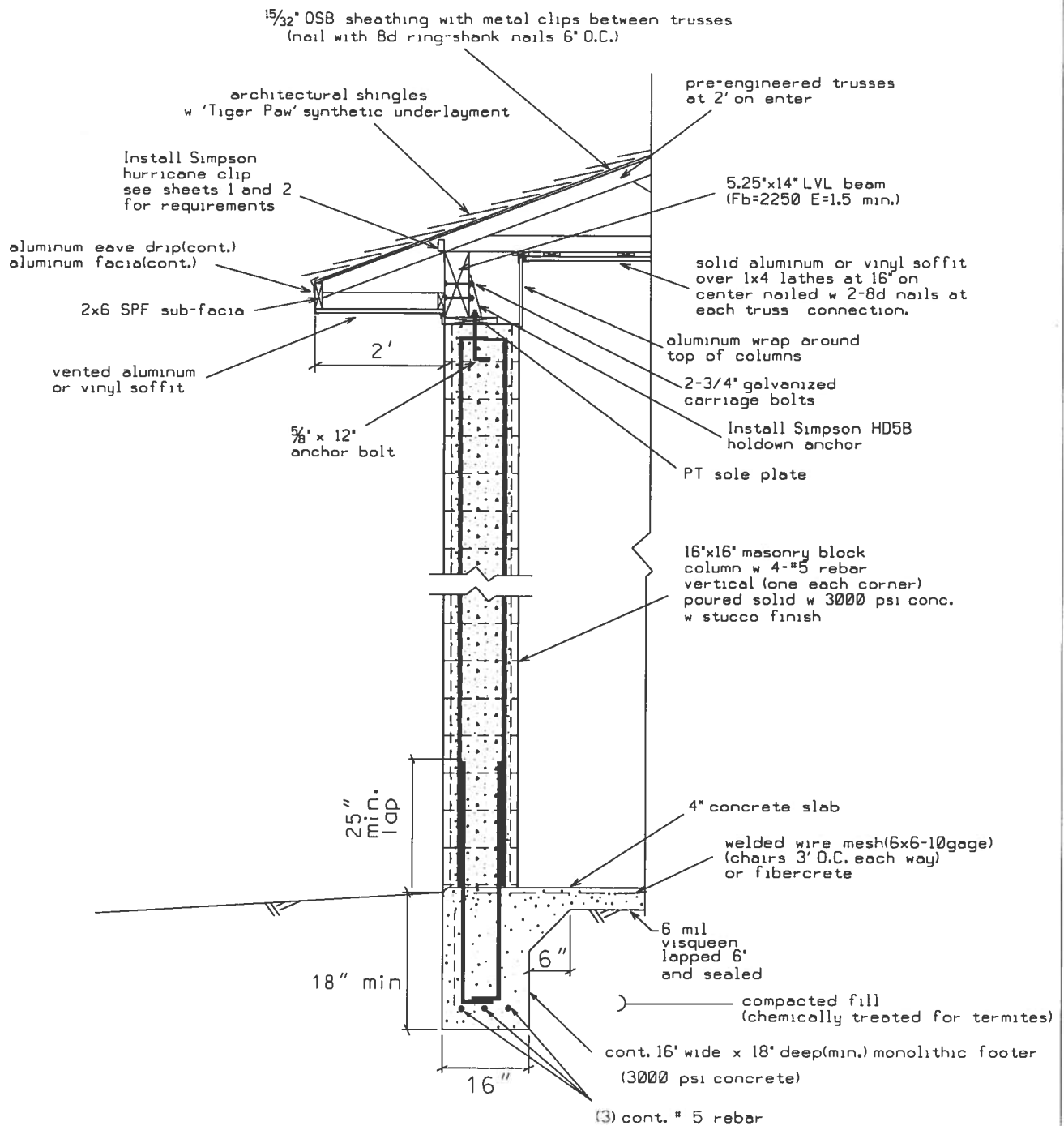


TYPICAL WALL DETAIL (N.T.S.)

(enclosed area)

Marty J. H
8-26-19

<p>Smith Carport Columbia County, FL</p>	<p>DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071</p>	<p>Sheet 3 of 4</p>
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CARPORT COLUMN/FOUNDATION DETAIL (N.T.S.)

Marty 572
8-26-19

Smith Carport Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071	Sheet 4 of 4
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Lumber design values are in accordance with ANSI/TPI 1 section 6.3
 These truss designs rely on lumber values established by others.

RE: 2067658 - MIKE TODD CONST - SMITH CARPORT

MiTek USA, Inc.

6904 Parke East Blvd.
 Tampa, FL 33610-4115

Site Information:

Customer Info: Mike Todd Const Project Name: Smith Carport Model: Custom
 Lot/Block: N/A Subdivision: N/A
 Address: 2872 NW Brown Road, N/A
 City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, if there is one, for the building.

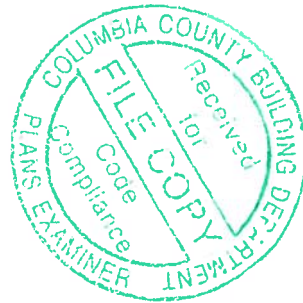
Name: License #:
 Address:
 City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
 Wind Code: ASCE 7-10 Wind Speed: 130 mph
 Roof Load: 37.0 psf Floor Load: N/A psf

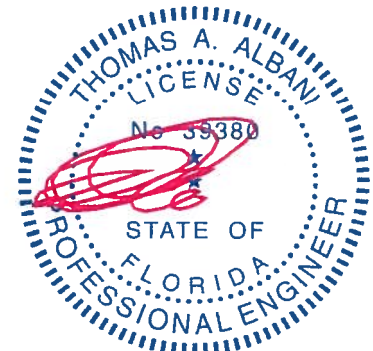
This package includes 21 individual, Truss Design Drawings and 0 Additional Drawings.
 With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T18132971	CJ01	9/17/19
2	T18132972	CJ01A	9/17/19
3	T18132973	CJ03	9/17/19
4	T18132974	CJ03A	9/17/19
5	T18132975	CJ05	9/17/19
6	T18132976	CJ05A	9/17/19
7	T18132977	EJ01	9/17/19
8	T18132978	EJ01A	9/17/19
9	T18132979	HJ10	9/17/19
10	T18132980	HJ10A	9/17/19
11	T18132981	T01	9/17/19
12	T18132982	T01A	9/17/19
13	T18132983	T02	9/17/19
14	T18132984	T02A	9/17/19
15	T18132985	T03	9/17/19
16	T18132986	T03A	9/17/19
17	T18132987	T04	9/17/19
18	T18132988	T04A	9/17/19
19	T18132989	T05	9/17/19
20	T18132990	T06	9/17/19
21	T18132991	T07	9/17/19



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas
 My license renewal date for the state of Florida is February 28, 2021.



IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Thomas A. Albani PE No. 39380
 MiTek USA, Inc. FL Cert 6634
 6904 Parke East Blvd. Tampa FL 33610
 Date:

September 17, 2019

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132971
2067658	CJ01	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:42 2019 Page 1

ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-RUOgPuxVcgYJ9OcwiYHL0Ec78NBwclG0bePj4sycjYB



Scale = 1:9.5

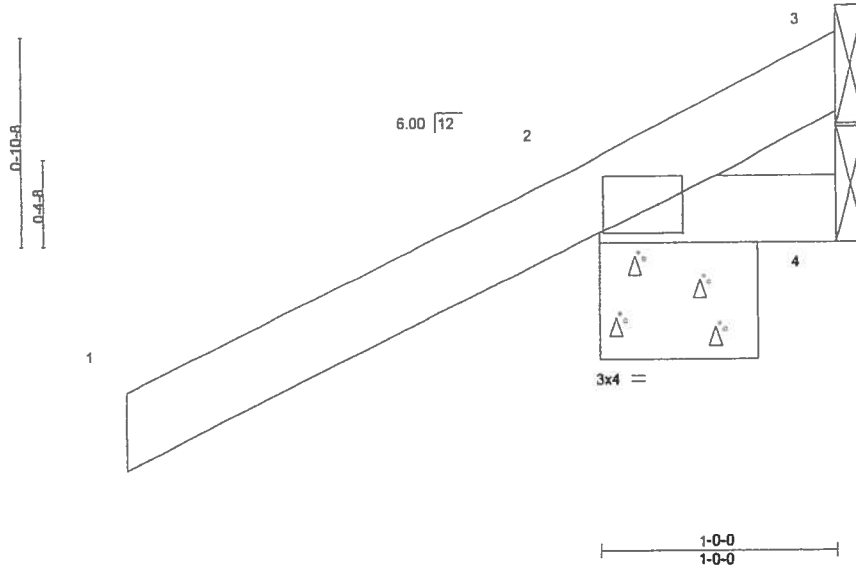


Plate Offsets (X,Y)- [2:0-1-4,0-1-9]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.32	Vert(LL)	0.00	7	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.07	Vert(CT)	0.00	7	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MP						Weight: 7 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-27/Mechanical, 2=254/0-8-0, 4=-46/Mechanical
Max Horz 2=66(LC 12)
Max Uplift 3=-27(LC 1), 2=-162(LC 12), 4=-46(LC 1)
Max Grav 3=25(LC 16), 2=254(LC 1), 4=44(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=162.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

September 17, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

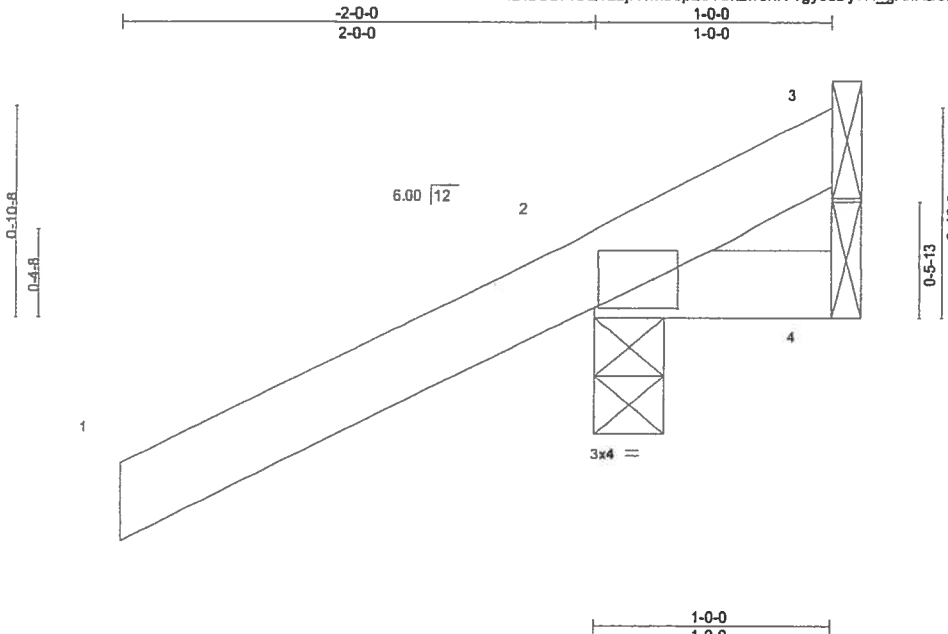


6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132972
2067658	CJ01A	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:43 2019 Page 1
 ID: BdOFrdQ122jR1Mb5pLe1CxzwohK-vgy3cDy7N_gAnXB6FFoakR9ltnXFLCW9p18HclycjYA



Scale = 1:9.5

Plate Offsets (X,Y) - [2:0-1-4,0-1-9]

LOADING (psf)	SPACING-	CSL	DEFL.	VERT (LL)	VERT (CT)	HORZ (CT)	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.32	in (loc)	0.00 7 >999	0.00 7 >999	0.00 2 n/a	240	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.07					180		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.00					n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-MP							
	Code FBC2017/TPI2014							Weight: 7 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-27/Mechanical, 2=254/0-3-8, 4=-46/Mechanical
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 Max Grav 3=25(LC 16), 2=254(LC 1), 4=44(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=162.



Thomas A. Albani PE No.39380
 MiTek USA, Inc. FL Cert 6634
 6904 Parke East Blvd. Tampa FL 33610
 Date:

September 17, 2019

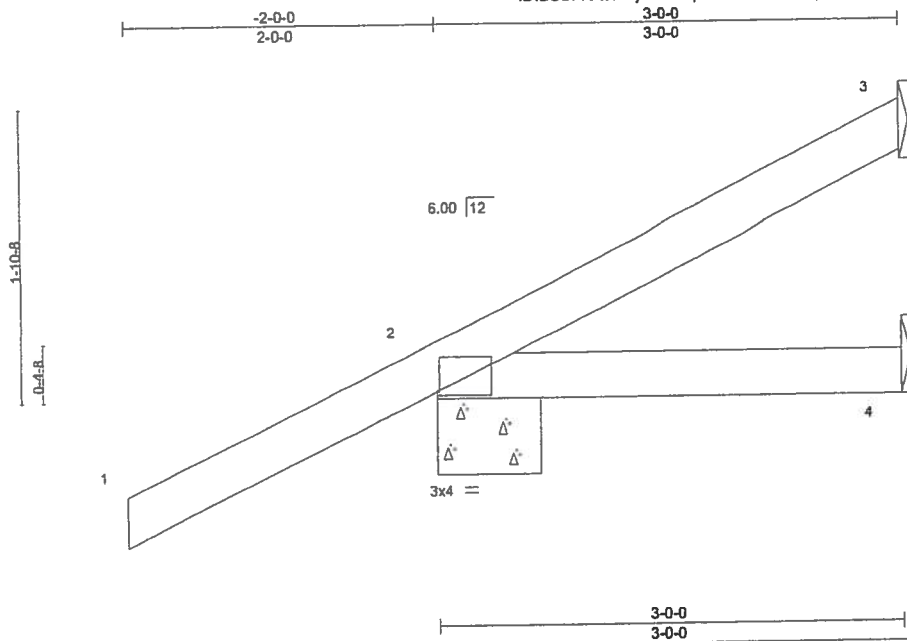
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132973
2067658	CJ03	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:43 2019 Page 1
ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-vgy3cDy7N_gAnXB6FFoaKR9ltnX9LCW9pl8HclycjYA



Scale = 1:14.6

Plate Offsets (X,Y) - [2:0-0-3,0-0-5]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.32	Vert(LL)	-0.00	4-7	>999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.07	Vert(CT)	-0.01	4-7	>999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	3	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MP					Weight: 13 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=52/Mechanical, 2=253/0-8-0, 4=20/Mechanical
Max Horz 2=113(LC 12)
Max Uplift 3=48(LC 12), 2=126(LC 12)
Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (l=lb) 2=126.



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Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132974
2067658	CJ03A	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:44 2019 Page 1
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Scale = 1:14.6

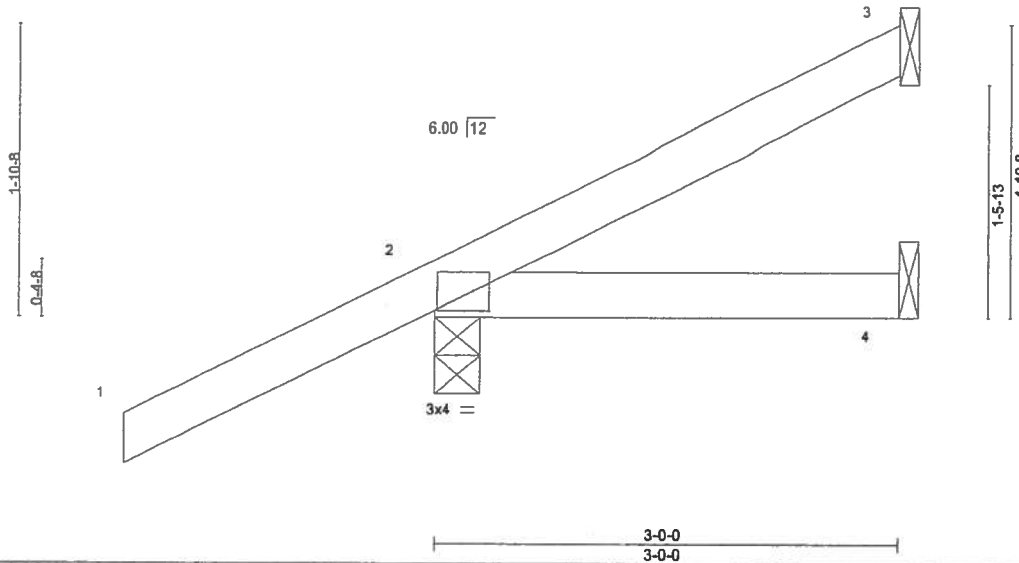


Plate Offsets (X,Y) - [2:0-1-4,0-1-9]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 2-0-0	TC 0.32	Vert(LL)	0.01	4-7	>999	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.07	Vert(CT)	-0.01	4-7	>999		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00	3	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MP					Weight: 13 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=52/Mechanical, 2=253/0-3-8, 4=20/Mechanical
Max Horz 2=113(LC 12)
Max Uplift 3=48(LC 12), 2=126(LC 12), 4=22(LC 9)
Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=126.



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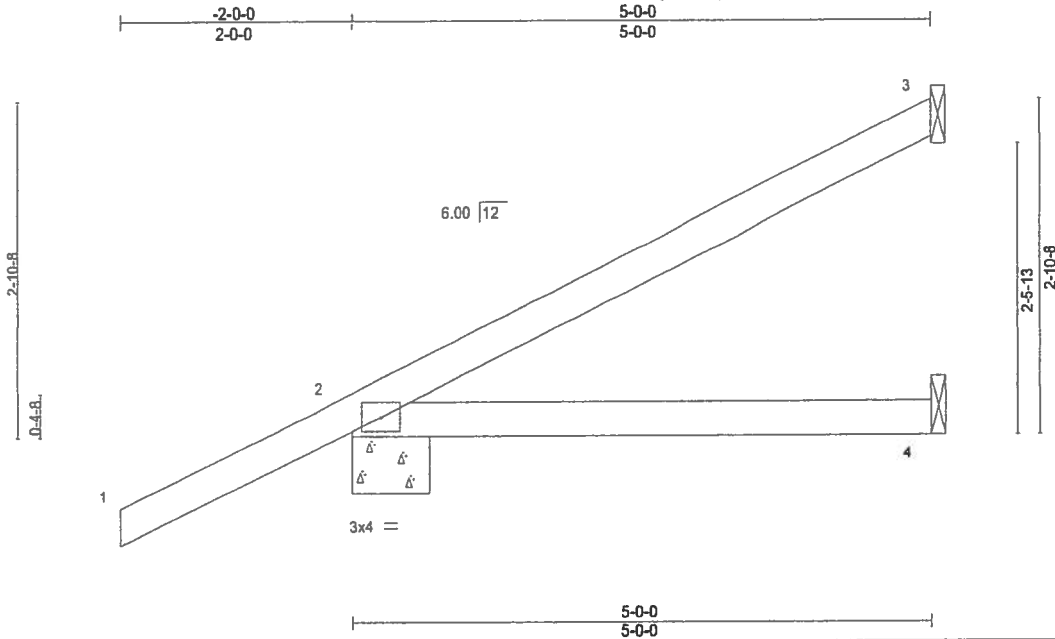


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Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132975
2067658	CJ05	Jack-Open	4	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244.

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:45 2019 Page 1
 ID: BdOFrdQ122jR1Mb5pLe1CxzwohK-r34p1v_Ovbwu0rKVNGr2QsEeNaACp60SHcdOhBycjY8



Scale = 1:19.5

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/def L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.32	Vert(LL) 0.03 4-7 >999 240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.23	Vert(CT) -0.05 4-7 >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MP		Weight: 19 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=108/Mechanical, 2=313/0-8-0, 4=53/Mechanical
 Max Horz 2=162(LC 12)
 Max Uplift 3=98(LC 12), 2=137(LC 12), 4=1(LC 12)
 Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (l=lb) 2=137.



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 Tampa, FL 33610

Job 2067658	Truss CJ05A	Truss Type Jack-Open	Qty 4	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132976
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:45 2019 Page 1
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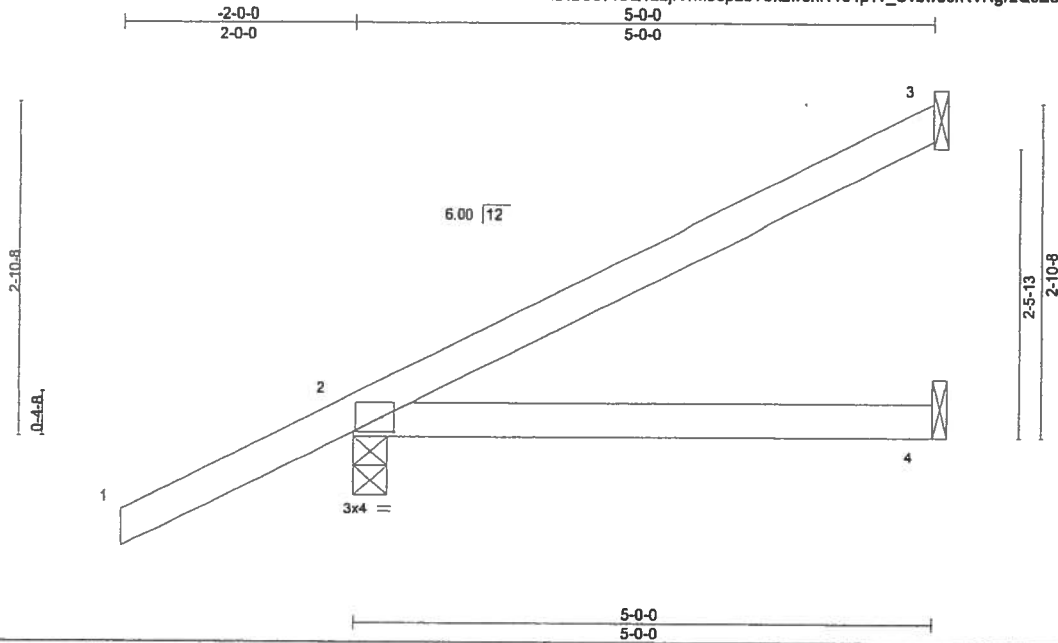


Plate Offsets (X,Y)- [2:0-1-4,0-1-9]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.38	Vert(LL) 0.08 4-7 >750 240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.34	Vert(CT) 0.07 4-7 >856 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 3 n/a n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MP		Weight: 19 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=108/Mechanical, 2=313/0-3-8, 4=53/Mechanical
Max Horz 2=162(LC 12)
Max Uplift 3=98(LC 12), 2=-137(LC 12), 4=-44(LC 9)
Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=137.



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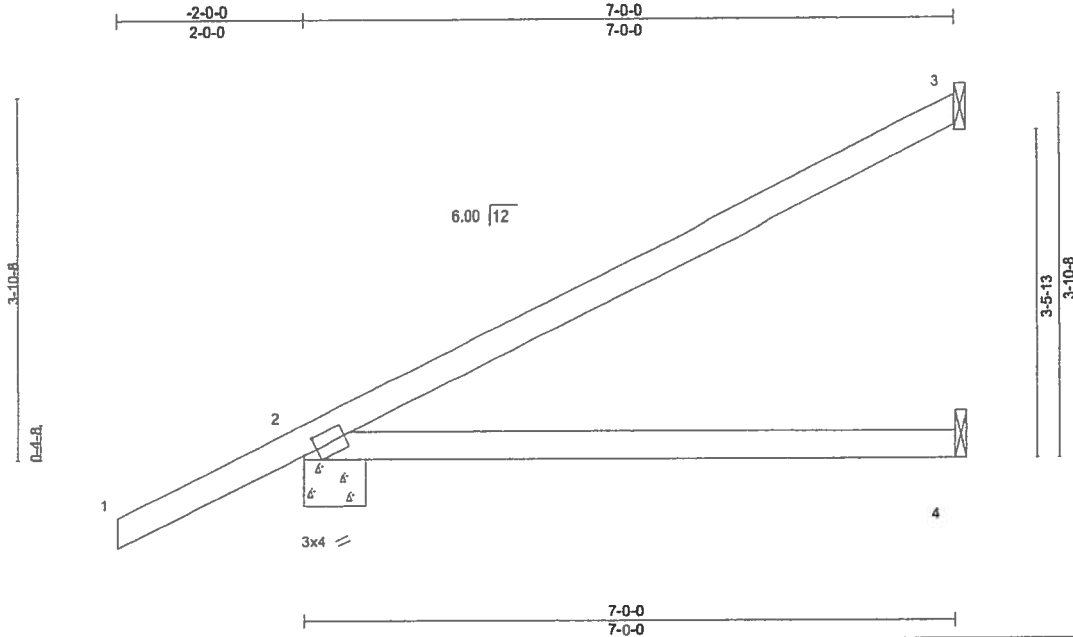
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132977
2067658	EJ01	Jack-Partial	12	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:46 2019 Page 1

ID: BdOfRdQ122jR1Mb5pLe1CxzwohK-JFdBEF_Ogv2le7vhxOMHy4njl_S7YZGbWGNxDdycjY7



Scale: 1/2"=1'

Plate Offsets (X,Y) - [2:0-1-13,0-1-8]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	2-0-0	TC 0.67	Vert(LL) 0.12	4-7	>670	240		MT20	244/190
TCDL 7.0	Lumber DOL 1.25		BC 0.51	Vert(CT) -0.21	4-7	>393	180			
BCLL 0.0 *	Rep Stress Incr YES		WB 0.00	Horz(CT) 0.01	3	n/a	n/a			
BCDL 10.0	Code FBC2017/TP12014		Matrix-MS						Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=160/Mechanical, 2=380/0-8-0, 4=81/Mechanical
Max Horz 2=144(LC 12)
Max Uplift 3=94(LC 12), 2=81(LC 12)
Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCCL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



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September 17, 201

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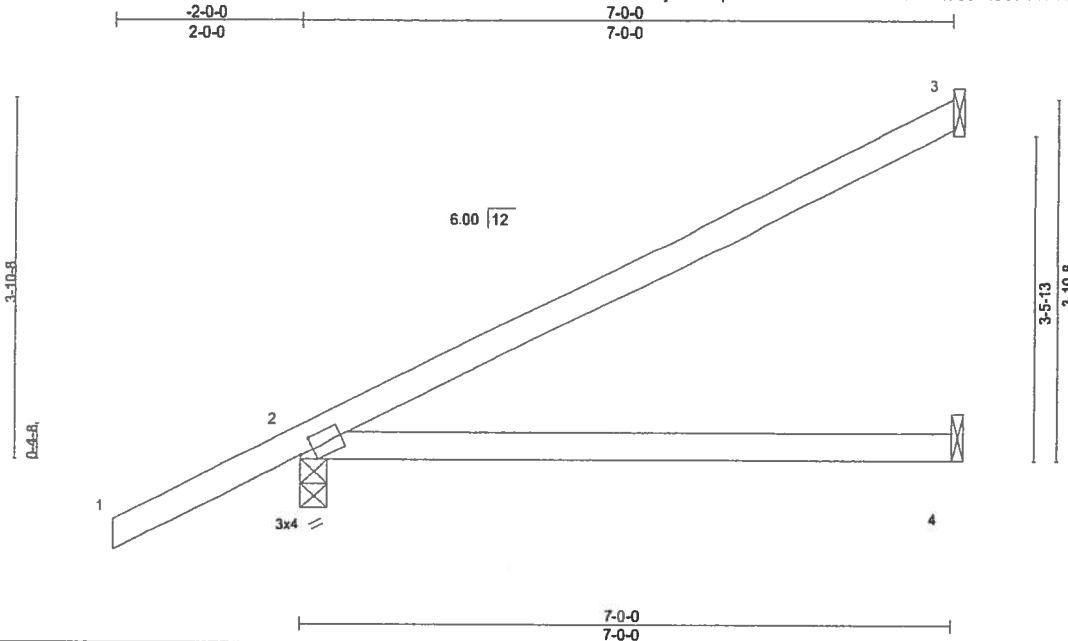
6904 Parke East Blvd.
Tampa, FL 36610

Job 2067658	Truss EJ01A	Truss Type Jack-Partial	Qty 12	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132978
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:47 2019 Page 1

ID: BdOfFrdQ122JR1Mb5pLe1CxzwohK-nSBZSb?eRCAcG9UitU5tWVHKrzOkRH0Wlkw6UI3ycjY6



Scale: 1/2"=1'

Plate Offsets (X,Y) - (2:0-1-13, 0-1-8)

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.89	Vert(LL)	0.33	4-7	>250	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.76	Vert(CT)	0.29	4-7	>287		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.01	3	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-2 oc bracing.

REACTIONS.

(lb/size) 3=160/Mechanical, 2=380/0-3-8, 4=81/Mechanical
Max Horz 2=144(LC 12)
Max Uplift 3=94(LC 12), 2=115(LC 9), 4=62(LC 9)
Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (it=lb) 2=115.



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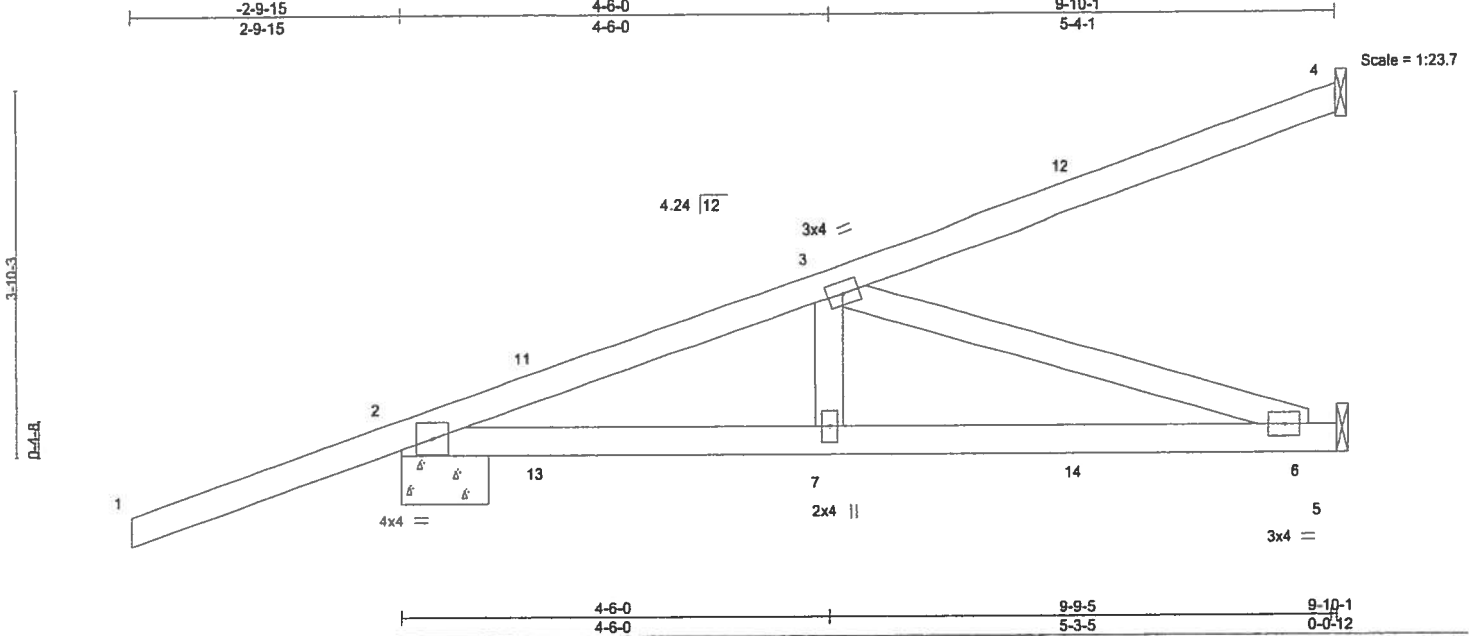


6904 Parke East Blvd.
Tampa, FL 33610

Job 2067658	Truss HJ10	Truss Type Diagonal Hip Girder	Qty 2	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132979
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:48 2019 Page 1
ID:BdOfrdQ122jR1Mb5pLe1CzxwohK-Felyfx0GCWITUJ342pOI1Vs4Vo6100Muzas2HWycjY5



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.59	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.61	Vert(LL) -0.06 7-10 >999 240		
BCLL 0.0	Lumber DOL 1.25	WB 0.35	Vert(CT) -0.12 6-7 >967 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-MS	Horz(CT) 0.01 5 n/a n/a	Weight: 44 lb	FT = 20%
	Code FBC2017/TPI2014				

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-0 oc bracing.

REACTIONS.

(lb/size) 4=150/Mechanical, 2=463/0-10-15, 5=251/Mechanical
Max Horz 2=233(LC 22)
Max Uplift 4=141(LC 4), 2=264(LC 4), 5=103(LC 8)
Max Grav 4=150(LC 1), 2=463(LC 1), 5=266(LC 3)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-638/221
BOT CHORD 2-7=-327/573, 6-7=-327/573
WEBS 3-6=-603/345

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=141, 2=264, 5=103.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 86 lb down and 103 lb up at 1-6-1, 86 lb down and 103 lb up at 1-6-1, 89 lb down and 21 lb up at 4-4-0, 89 lb down and 21 lb up at 4-4-0, and 50 lb down and 97 lb up at 7-1-15, and 50 lb down and 97 lb up at 7-1-15 on top chord, and 36 lb down and 74 lb up at 1-6-1, 36 lb down and 74 lb up at 1-6-1, 28 lb down and 2 lb up at 4-4-0, 28 lb down and 2 lb up at 4-4-0, and 44 lb down and 15 lb up at 7-1-15, and 44 lb down and 15 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 5-8=-20
Concentrated Loads (lb)
Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 13=70(F=35, B=35) 14=-49(F=-24, B=-24)



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Date:

September 17, 201

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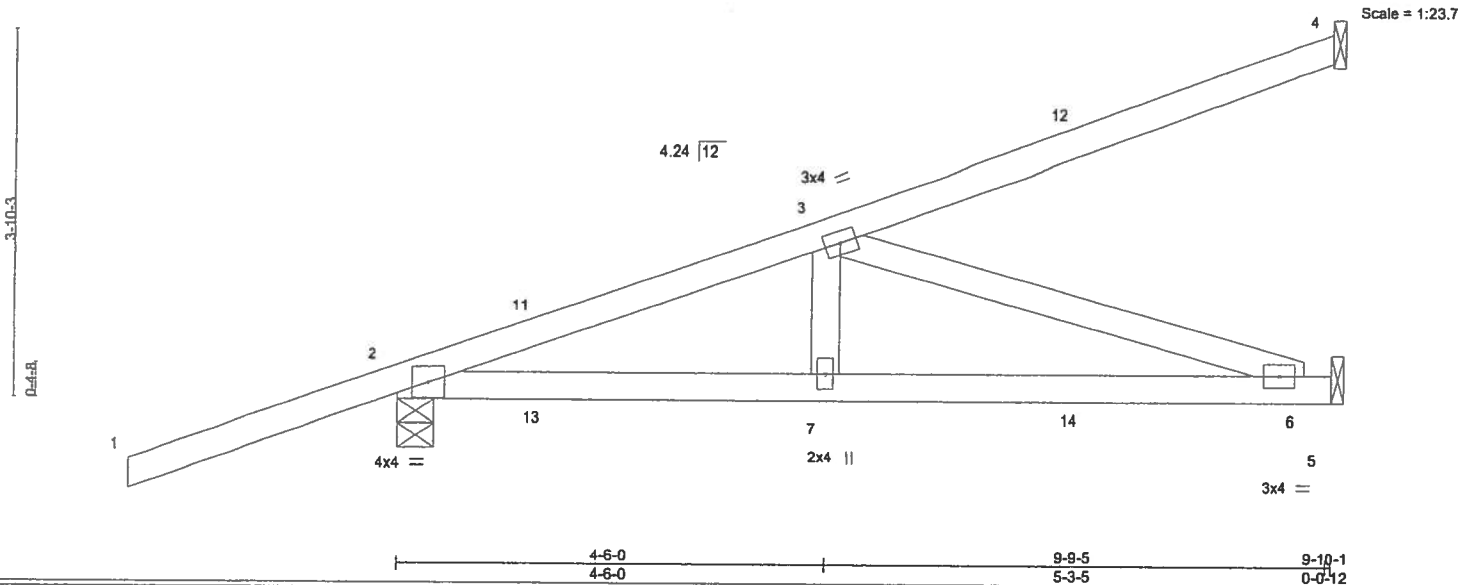
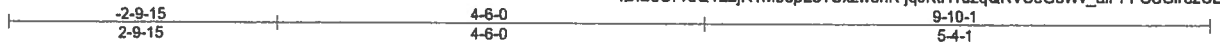


6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132980
2067658	HJ10A	Diagonal Hip Girder	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MITek Industries, Inc. Tue Sep 17 12:50:49 2019 Page 1
ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-jqJKH1uzqQKVSeGcWw_aiPFFCSGlrC2CEbbqqyqY4



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.59	Vert(LL)	0.12	6-7	>999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.61	Vert(CT)	-0.12	6-7	>967		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.35	Horz(CT)	-0.01	5	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MS					Weight: 44 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.

REACTIONS. (lb/size) 4=150/Mechanical, 2=463/0-4-9, 5=251/Mechanical
Max Horz 2=233(LC 4)
Max Uplift 4=143(LC 4), 2=-345(LC 4), 5=-216(LC 5)
Max Grav 4=150(LC 1), 2=463(LC 1), 5=266(LC 3)

FORCES. (lb) - Max. Comp /Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-628/448
BOT CHORD 2-7=-510/573, 6-7=-510/573
WEBS 3-6=-603/537

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=143, 2=345, 5=216.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 83 lb down and 103 lb up at 1-6-1, 83 lb down and 103 lb up at 1-6-1, 26 lb down and 38 lb up at 4-4-0, 26 lb down and 38 lb up at 4-4-0, and 50 lb down and 97 lb up at 7-1-15, and 50 lb down and 97 lb up at 7-1-15 on top chord, and 69 lb down and 74 lb up at 1-6-1, 69 lb down and 74 lb up at 1-6-1, 53 lb down and 30 lb up at 4-4-0, 53 lb down and 30 lb up at 4-4-0, and 40 lb down and 59 lb up at 7-1-15, and 40 lb down and 59 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 5-8=-20
Concentrated Loads (lb)
Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 13=70(F=35, B=35) 14=-49(F=-24, B=-24)



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September 17, 2019

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6904 Parke East Blvd.
Tampa, FL 38610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132981
2067658	TD1	HIP GIRDER	1	2	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

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ID: BdOfFrQ122JR1Mb5pLe1CxzwohK-8P7SVI3nGlpvMwNrHeThCL1nmPR9yBKUuCqFQHycjY1



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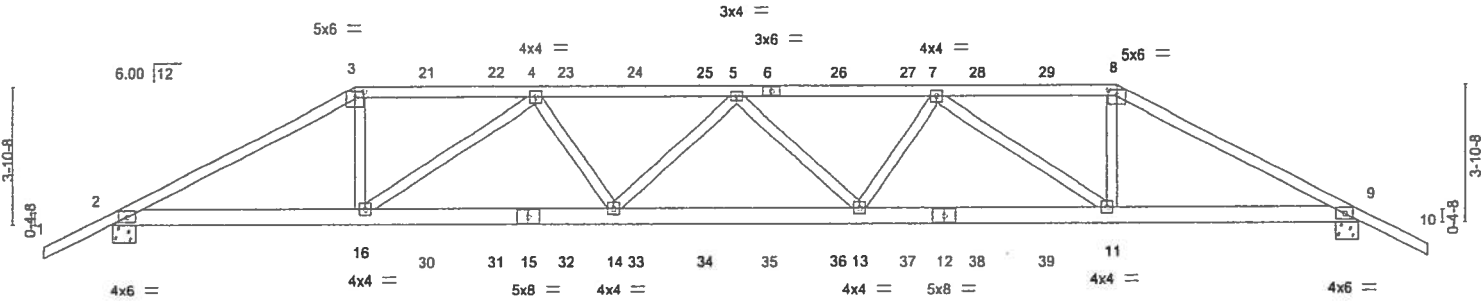


Plate Offsets (X,Y) =	[3:0-3-0,0-2-0], [8:0-3-0,0-2-0]
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.57	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.72	Vert(LL) -0.26 13-14 >999 240		
BCLL 0.0	Lumber DOL 1.25	WB 0.41	Vert(CT) -0.50 13-14 >873 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-MS	Horz(CT) 0.12 9 n/a n/a		
	Code FBC2017/TPI2014			Weight: 399 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=2680/0-8-0, 9=2725/0-8-0
Max Horz 2=61(LC 6)
Max Uplift 2=-763(LC 8), 9=-831(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-5287/1565, 3-4=-4734/1446, 4-5=-7070/2118, 5-7=-7100/2155, 7-8=-4822/1575, 8-9=-5387/1712
BOT CHORD 2-16=-1360/4656, 14-16=-2000/6629, 13-14=-2243/7405, 11-13=-2043/6678, 9-11=-1453/4746
WEBS 3-16=-480/1877, 4-16=-2370/806, 4-14=-126/851, 5-14=-494/291, 5-13=-444/226, 7-13=-73/834, 7-11=-2313/729, 8-11=-435/1847

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - 3) Unbalanced roof live loads have been considered for this design.
 - 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
 - 5) Provide adequate drainage to prevent water ponding.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=763, 9=831.
 - 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 100 lb up at 7-0-0, 106 lb down and 100 lb up at 9-0-12, 106 lb down and 100 lb up at 11-0-12, 106 lb down and 100 lb up at 13-0-12, 106 lb down and 100 lb up at 15-0-12, 106 lb down and 100 lb up at 17-0-12, 106 lb down and 100 lb up at 18-11-4, 106 lb down and 100 lb up at 20-11-4, 106 lb down and 100 lb up at 22-11-4, 106 lb down and 100 lb up at 24-11-4, and 106 lb down and 100 lb up at 26-11-4, and 227 lb down and 250 lb up at 29-0-0 on top chord, and 294 lb down and 131 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-12, 85 lb down at 13-0-12, 85 lb down at 15-0-12, 85 lb down at 17-0-12, 85 lb down at 18-11-4, 85 lb down at 20-11-4, 85 lb down at 22-11-4, 85 lb down at 24-11-4, and 85 lb down at 26-11-4, and 294 lb down and 131 lb up at 28-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.



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Date: September 17, 2019

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Continued on page 2

6904 Parke East Blvd.
Tampa, FL 33610

Job 2067658	Truss T01	Truss Type HIP GIRDER	Qty 1	Ply 2	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132981
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:52 2019 Page 2
ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-8P7SVI3nGlpvMwNrHeThCL1nmPR9yBKUuCqFQHycjY1

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-54, 8-10=-54, 2-9=-20

Concentrated Loads (lb)

Vert: 3=-106(B) 6=-106(B) 8=-180(B) 16=-284(B) 11=-284(B) 21=-106(B) 22=-106(B) 23=-106(B) 24=-106(B) 25=-106(B) 26=-106(B) 27=-106(B) 28=-106(B) 29=-106(B) 30=-61(B) 31=-61(B) 32=-61(B) 33=-61(B) 34=-61(B) 35=-61(B) 36=-61(B) 37=-61(B) 38=-61(B) 39=-61(B)

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132982
2067658	T01A	HIP GIRDER	1	2	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:56 2019 Page 1

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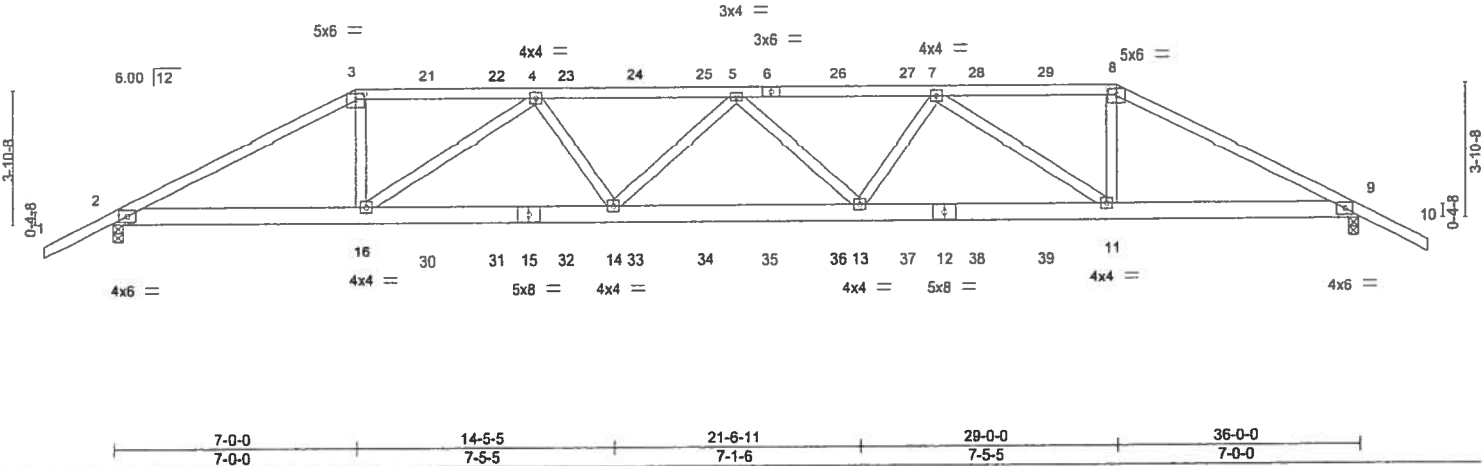


Plate Offsets (X,Y)	[3:0-3-0,0-2-0], [8:0-3-0,0-2-0]
---------------------	----------------------------------

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.57	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.72	Vert(LL) 0.42 13-14 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.41	Vert(CT) -0.50 13-14 >873 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-MS	Horz(CT) 0.12 9 n/a n/a	Weight: 399 lb	FT = 20%
	Code FBC2017/TPI2014				

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-4-5 oc bracing.

REACTIONS. (lb/size) 2=2680/0-3-8, 9=2725/0-3-8
Max Horz 2=-61(LC 25)
Max Uplift 2=-1613(LC 5), 9=-1680(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-5287/3361, 3-4=-4734/3085, 4-5=-7070/4570, 5-7=-7100/4608, 7-8=-4822/3217, 8-9=-5387/3511
BOT CHORD 2-16=-2966/4656, 14-16=-4203/6629, 13-14=-4696/7405, 11-13=-4247/6678, 9-11=-3061/4746
WEBS 3-16=-1282/1877, 4-16=-2371/1491, 4-14=-590/851, 5-14=-494/293, 5-13=-444/226, 7-13=-536/834, 7-11=-2313/1413, 8-11=-1237/1847

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1613, 9=1680.



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6904 Parke East Blvd. Tampa FL 33610
Date:

September 17,201

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132982
2067658	T01A	HIP GIRDER	1	2	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:56 2019 Page 2
ID:BdOfRdQ122jR1Mb5pLe1CzwohK-0AEzLg6HJzJKrXgcWUjXdMBCTI0c5u_J4pqrTZ2ycjXz

NOTES-

- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 100 lb up at 7-0-0, 106 lb down and 100 lb up at 9-0-12, 106 lb down and 100 lb up at 11-0-12, 106 lb down and 100 lb up at 13-0-12, 106 lb down and 100 lb up at 15-0-12, 106 lb down and 100 lb up at 17-0-12, 106 lb down and 100 lb up at 18-11-4, 106 lb down and 100 lb up at 20-11-4, 106 lb down and 100 lb up at 22-11-4, 106 lb down and 100 lb up at 24-11-4, and 106 lb down and 100 lb up at 26-11-4, and 227 lb down and 252 lb up at 29-0-0 on top chord, and 294 lb down and 335 lb up at 7-0-0, 85 lb down and 82 lb up at 9-0-12, 85 lb down and 82 lb up at 11-0-12, 85 lb down and 82 lb up at 13-0-12, 85 lb down and 82 lb up at 15-0-12, 85 lb down and 82 lb up at 17-0-12, 85 lb down and 82 lb up at 18-11-4, 85 lb down and 82 lb up at 20-11-4, 85 lb down and 82 lb up at 22-11-4, 85 lb down and 82 lb up at 24-11-4, and 85 lb down and 82 lb up at 26-11-4, and 294 lb down and 335 lb up at 28-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-54, 8-10=-54, 2-9=-20

Concentrated Loads (lb)

Vert: 3=-106(F) 6=-106(F) 8=-180(F) 16=-284(F) 11=-284(F) 21=-106(F) 22=-106(F) 23=-106(F) 24=-106(F) 25=-106(F) 26=-106(F) 27=-106(F) 28=-106(F) 29=-106(F) 30=-61(F) 31=-61(F) 32=-61(F) 33=-61(F) 34=-61(F) 35=-61(F) 36=-61(F) 37=-61(F) 38=-61(F) 39=-61(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



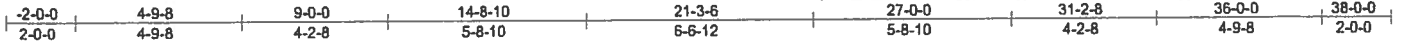
6904 Parke East Blvd.
Tampa, FL 36610

Job 2067658	Truss T02	Truss Type Hip	Qty 1	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132983
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:50:57 2019 Page 1

ID: BdOFrdQ122jR1Mb5pLe1CxzwohK-UNoLY07v4HRBShFo4B2svOkf8Q57dLxD1UX05UycjXy



Scale = 1:65.2

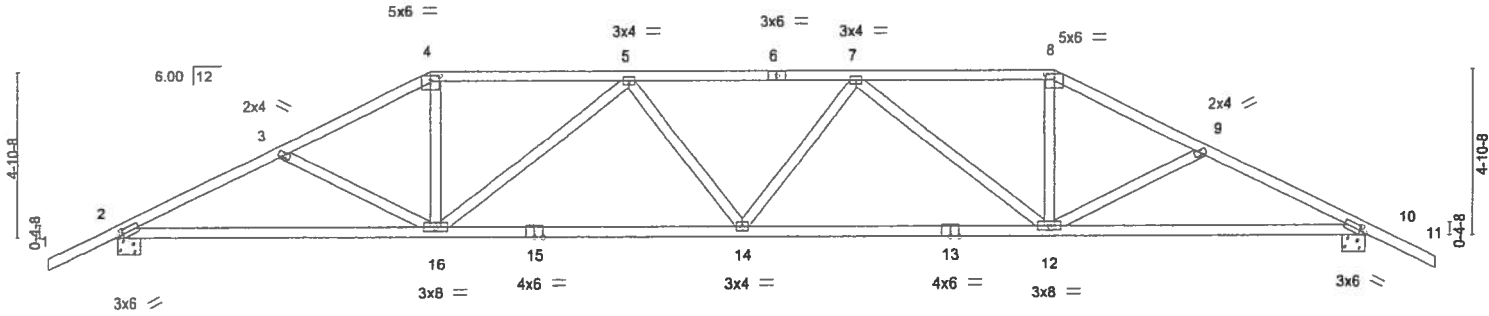


Plate Offsets (X,Y)	[2:0-1-15,0-1-8], [4:0-3-0,0-2-0], [8:0-3-0,0-2-0], [10:0-1-15,0-1-8]
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LOADING (psf)	SPACING	CSI	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.46	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.93	Vert(LL) -0.20 14 >999 240		
BCLL 0.0	Lumber DOL 1.25	WB 0.77	Vert(CT) -0.43 14-16 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0.13 10 n/a n/a		
	Code FBC2017/TPI2014			Weight: 181 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-3-7 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS. (lb/size) 2=1440/0-8-0, 10=1440/0-8-0
 Max Horz 2=-75(LC 10)
 Max Uplift 2=-264(LC 9), 10=-264(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2515/1280, 3-4=-2263/1140, 4-5=-1997/1079, 5-7=-2595/1352, 7-8=-1997/1079,
 8-9=-2263/1140, 9-10=-2515/1280
BOT CHORD 2-16=-985/2206, 14-16=-1078/2532, 12-14=-1083/2532, 10-12=-1017/2206
WEBS 3-16=-265/269, 4-16=-309/742, 5-16=-765/388, 7-12=-765/388, 8-12=-309/742,
 9-12=-265/268

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=264, 10=264.



Thomas A. Albani PE No.39380
 MiTek USA, Inc. FL Cert 6634
 6904 Parke East Blvd. Tampa FL 33610
 Date:

September 17,201

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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



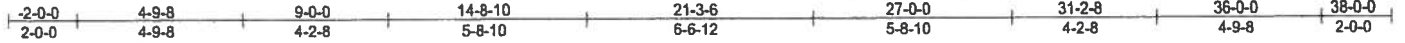
6904 Parke East Blvd.
 Tampa, FL 36610

Job 2067658	Truss T02A	Truss Type Hip	Qty 1	Ply 1	MIKE TODD CONST - SMITH CARPORT	T18132984
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MITek Industries, Inc. Tue Sep 17 12:50:58 2019 Page 1

ID: BdOFrdQ122jR1Mb5pLe1CzxwohK-zZMkmM8YrbZZ4rq?evZ5RcHnUqRMMoBMG8HadwycjXx



Scale = 1/65.2

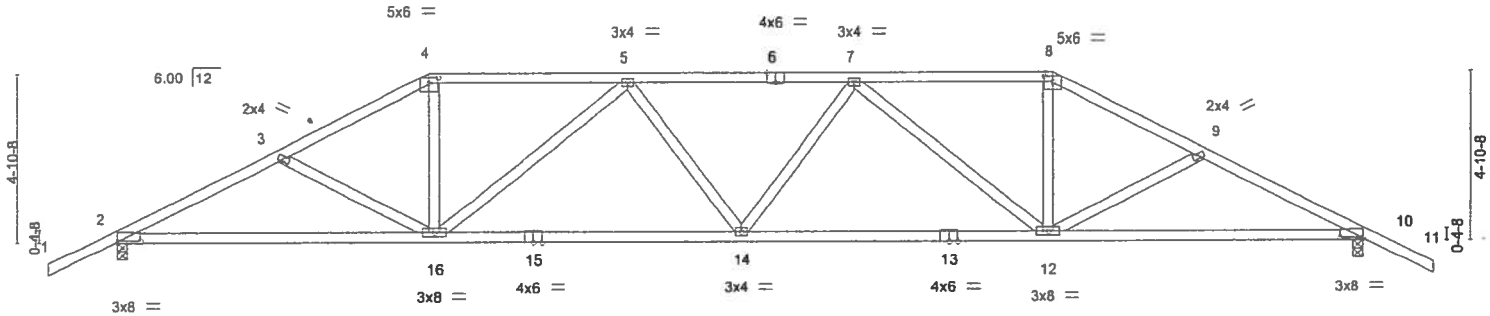


Plate Offsets (X,Y) -	9-0-0 9-0-0	18-0-0 9-0-0	27-0-0 9-0-0	36-0-0 9-0-0
	[2:0-8-0,0-0-3], [4:0-3-0,0-2-0], [6:0-3-0,Edge], [8:0-3-0,0-2-0], [10:0-8-0,0-0-3]			

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.68	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.93	Vert(LL) 0.43 14-16 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.77	Vert(CT) -0.43 14-16 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0.13 10 n/a n/a		
	Code FBC2017/TPI2014			Weight: 181 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-3-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS. (lb/size) 2=1440/0-3-8, 10=1440/0-3-8
Max Horz 2=-75(LC 10)
Max Uplift 2=-680(LC 9), 10=-680(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2515/2713, 3-4=-2263/2535, 4-5=-1997/2342, 5-7=-2595/2989, 7-8=-1997/2342, 8-9=-2263/2535, 9-10=-2515/2713
BOT CHORD 2-16=-2286/2206, 14-16=-2576/2532, 12-14=-2581/2532, 10-12=-2317/2206
WEBS 3-16=-265/334, 4-16=-943/742, 5-16=-765/693, 7-12=-765/693, 8-12=-942/742, 9-12=-265/334

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-5-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=680, 10=680.



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6904 Parke East Blvd. Tampa FL 33610
Date:

September 17,201

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



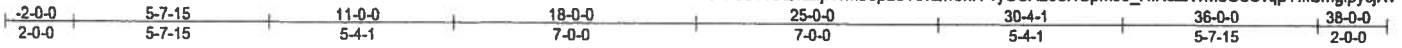
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132985
2067658	T03	Hip	1	1	Job Reference (optional)	

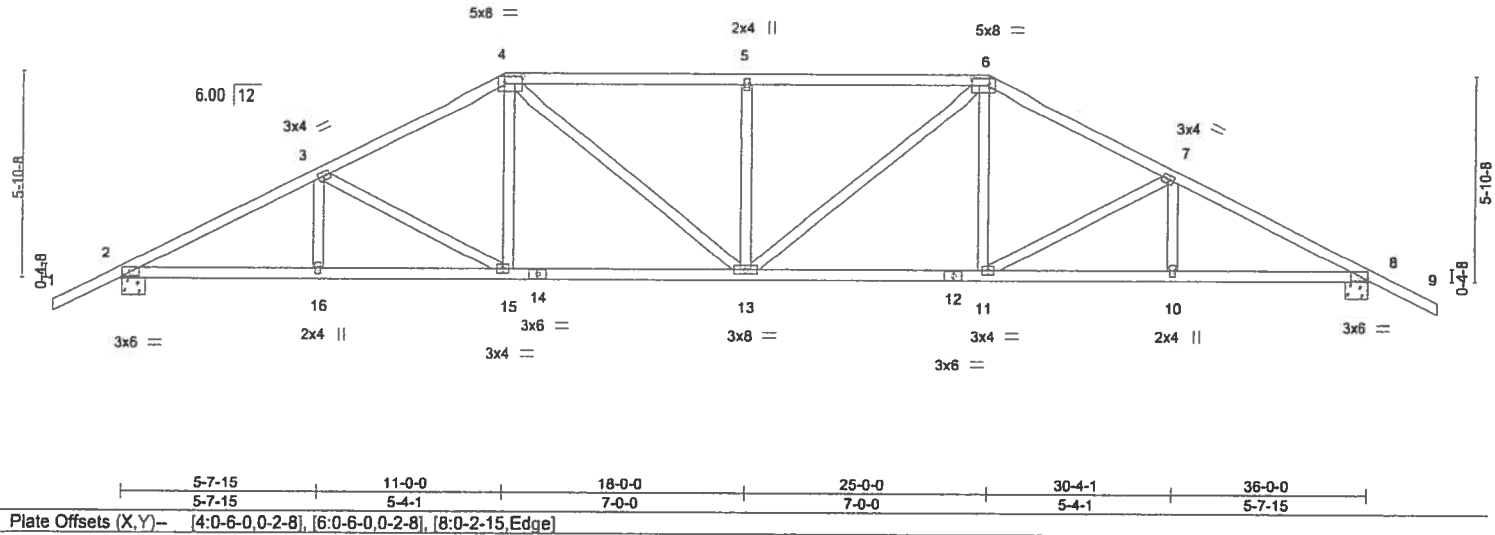
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:51:00 2019 Page 1

ID:BdOFrdQ122jR1Mb5pLe1CxzwohK-vyUUA29oNCpmJ9_NIKcZX1M8OdCvqpYfkSmgipyqjXv



Scale = 1:65.2



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.59	Vert(LL)	-0.16	13	>999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.60	Vert(CT)	-0.32	11-13	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.33	Horz(CT)	0.12	8	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MS					Weight: 192 lb	FT = 20%

LUMBER-	BRACING-	
TOP CHORD	TOP CHORD	Structural wood sheathing directly applied or 3-1-14 oc purlins.
BOT CHORD	BOT CHORD	Rigid ceiling directly applied or 5-11-6 oc bracing.
WEBS		

REACTIONS. (lb/size) 2=1440/0-8-0, 8=1440/0-8-0
 Max Horz 2=-88(LC 10)
 Max Uplift 2=-275(LC 12), 8=-275(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2523/1262, 3-4=-2111/1110, 4-5=-2190/1225, 5-6=-2190/1225, 6-7=-2111/1110, 7-8=-2523/1262
 BOT CHORD 2-16=-963/2201, 15-16=-963/2201, 13-15=-705/1835, 11-13=-711/1835, 10-11=-993/2201, 8-10=-993/2201
 WEBS 3-15=-428/324, 4-15=-118/400, 4-13=-227/566, 5-13=-434/325, 6-13=-227/566, 6-11=-118/400, 7-11=-428/323

- NOTES-
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCCL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=ib) 2=275, 8=275.



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 6904 Parke East Blvd. Tampa FL 33610
 Date:

September 17, 2019

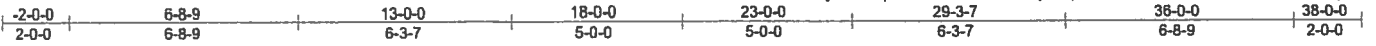
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 10/03/2015 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing Indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

6904 Parke East Blvd.
 Tampa, FL 36610

Job 2067658	Truss T04	Truss Type Hip	Qty 1	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132987
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Builders FirstSource, Jacksonville, FL - 32244,

8,240 s Jul 14 2019 MITek Industries, Inc. Tue Sep 17 12:51:02 2019 Page 1
ID: BdOfRdQ122JR1Mb5pLe1CxzwohK-rKbEbjB2vp3UZS8mle1cSSUnRoCIGNyBIFnmiycjXt



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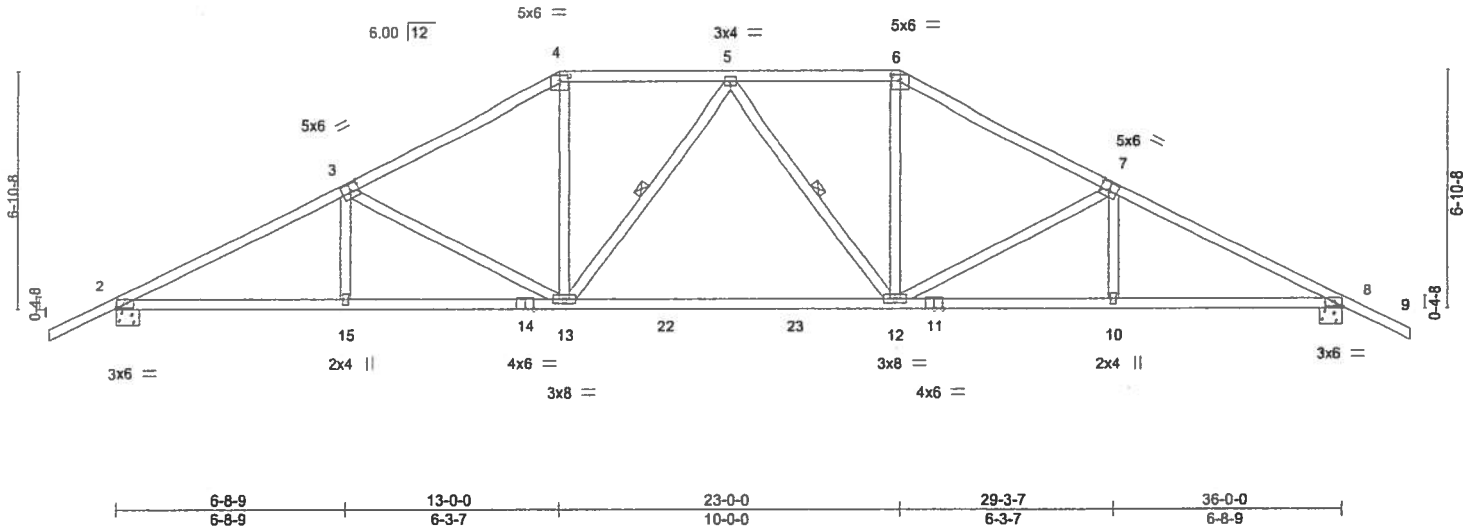


Plate Offsets (X,Y) - [3:0-3-0,0-3-0], [4:0-3-0,0-2-0], [6:0-3-0,0-2-0], [7:0-3-0,0-3-0], [8:0-2-15,Edge]

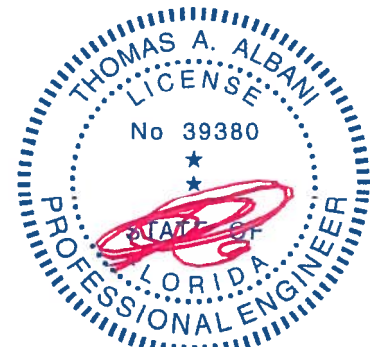
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.53	Vert(LL) -0.33	12-13	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 1.00	Vert(CT) -0.63	12-13	>689	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.54	Horz(CT) 0.11	8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS					Weight: 191 lb	FT = 20%

LUMBER-	BRACING-	
TOP CHORD 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	WEBS	1 Row at midpt 5-13, 5-12

REACTIONS. (lb/size) 2=1440/0-8-0, 8=1440/0-8-0
Max Horz 2=102(LC 11)
Max Uplift 2=-289(LC 12), 8=-289(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2484/1262, 3-4=-1974/1062, 4-5=-1702/1018, 5-6=-1702/1018, 6-7=-1974/1062, 7-8=-2484/1262
BOT CHORD 2-15=-954/2159, 13-15=-955/2156, 12-13=-697/1807, 10-12=-979/2156, 8-10=-978/2159
WEBS 3-13=-536/414, 4-13=-248/575, 5-13=-303/141, 5-12=-303/141, 6-12=-248/575, 7-12=-536/414

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=289, 8=289.



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MITek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

September 17, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132988
2067658	T04A	Hip	1	1		

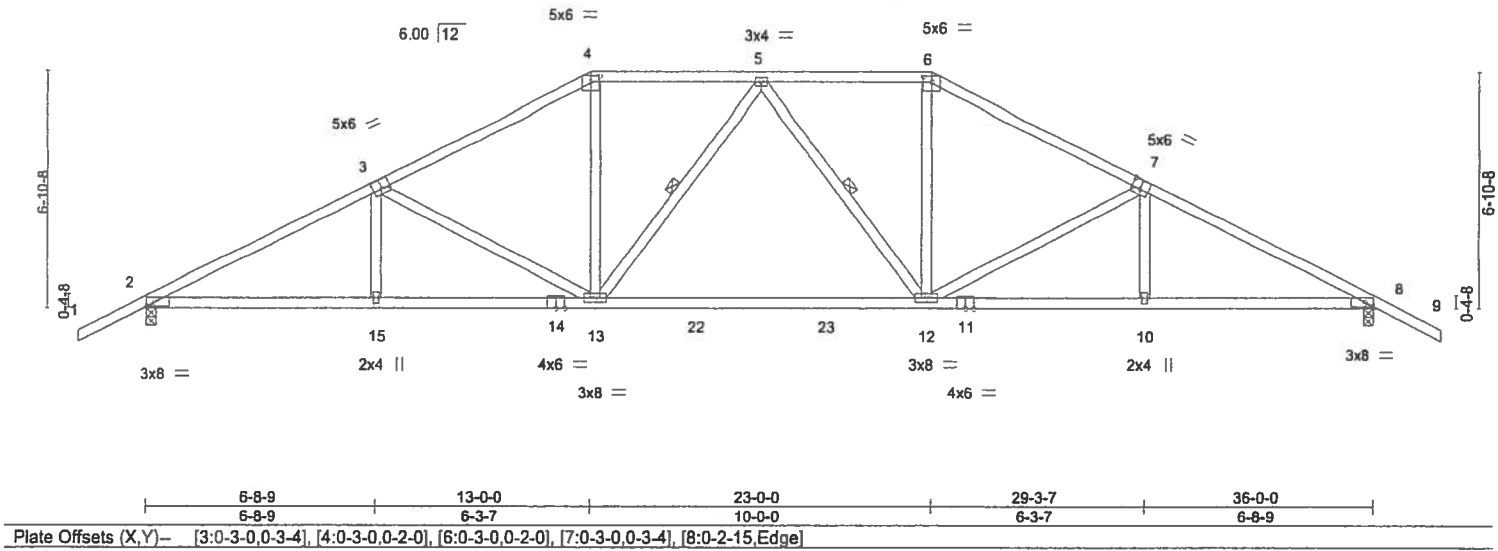
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:51:04 2019 Page 1

ID: BdOFrdQ122JR1Mb5pLe1CxzwohK-nj?0PCIRRJComH8_AgVhtXmNFTgmYeFe3kuraycjXr



Scale = 1.66.2



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL	in (loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.78	Vert(LL)	0.51 12-13	>843	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 1.00	Vert(CT)	-0.63 12-13	>689	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.65	Horz(CT)	0.11 8	n/a	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-MS					Weight: 191 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-13, 5-12

REACTIONS. (lb/size) 2=1440/0-3-8, 8=1440/0-3-8
 Max Horz 2=-102(LC 10)
 Max Uplift 2=-622(LC 9), 8=-622(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2484/2730, 3-4=-1974/2240, 4-5=-1702/2082, 5-6=-1702/2082, 6-7=-1974/2240, 7-8=-2484/2730
 BOT CHORD 2-15=-2272/2159, 13-15=-2266/2156, 12-13=-1760/1807, 10-12=-2290/2156, 8-10=-2296/2159
 WEBS 3-13=-536/709, 4-13=-777/575, 5-13=-303/140, 5-12=-303/140, 6-12=-777/575, 7-12=-536/709

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=622, 8=622.



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 Date:

September 17, 2019

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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

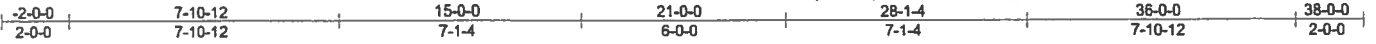
6904 Parke East Blvd.
 Tampa, FL 33610

Job 2067658	Truss T05	Truss Type Hip	Qty 2	Ply 1	MIKE TODD CONST - SMITH CARPORT Job Reference (optional)	T18132989
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Builders FirstSource, Jacksonville, FL - 32244,

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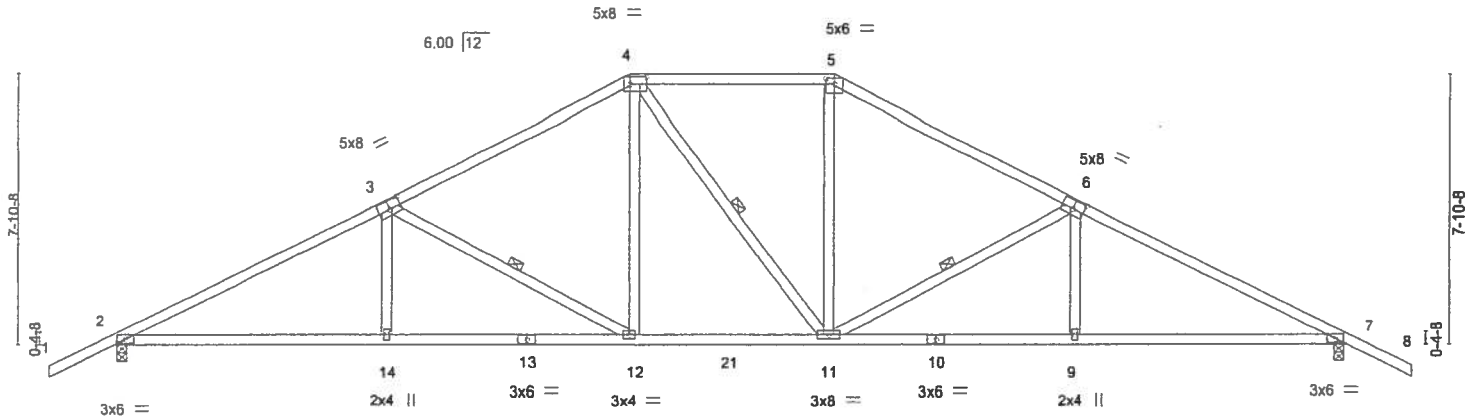


Plate Offsets (X,Y)	[3:0-4-0,0-3-0], [4:0-6-0,0-2-8], [5:0-3-0,0-2-0], [6:0-4-0,0-3-0], [7:0-2-15,Edge]
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LOADING (psf)	SPACING - 2-0-0	CSI	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.94	Vert(LL) 0.28 12-14 >999 240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.75	Vert(CT) -0.28 12-14 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.74	Horz(CT) 0.11 7 n/a n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MS		Weight: 189 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 3-4-12 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 3-12, 4-11, 6-11

REACTIONS. (lb/size) 2=1440/0-3-8, 7=1440/0-3-8
 Max Horz 2=-115(LC 10)
 Max Uplift 2=-590(LC 9), 7=-590(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2445/2695, 3-4=-1818/2074, 4-5=-1547/1938, 5-6=-1819/2075, 6-7=-2445/2695
 BOT CHORD 2-14=-2231/2115, 12-14=-2230/2114, 11-12=-1486/1547, 9-11=-2250/2114,
 7-9=-2251/2114
 WEBS 3-14=-326/322, 3-12=-662/888, 4-12=-667/484, 5-11=-666/485, 6-11=-661/886,
 6-9=-325/321

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=590. 7=590.



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September 17, 201

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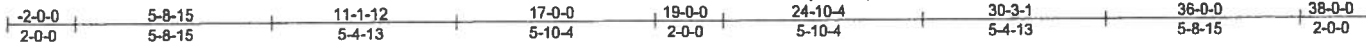
6904 Parke East Blvd.
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Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132990
2067658	T06	Hip	2	1	Job Reference (optional)	

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8.240 s Jul 14 2019 MiTek Industries, Inc. Tue Sep 17 12:51:06 2019 Page 1

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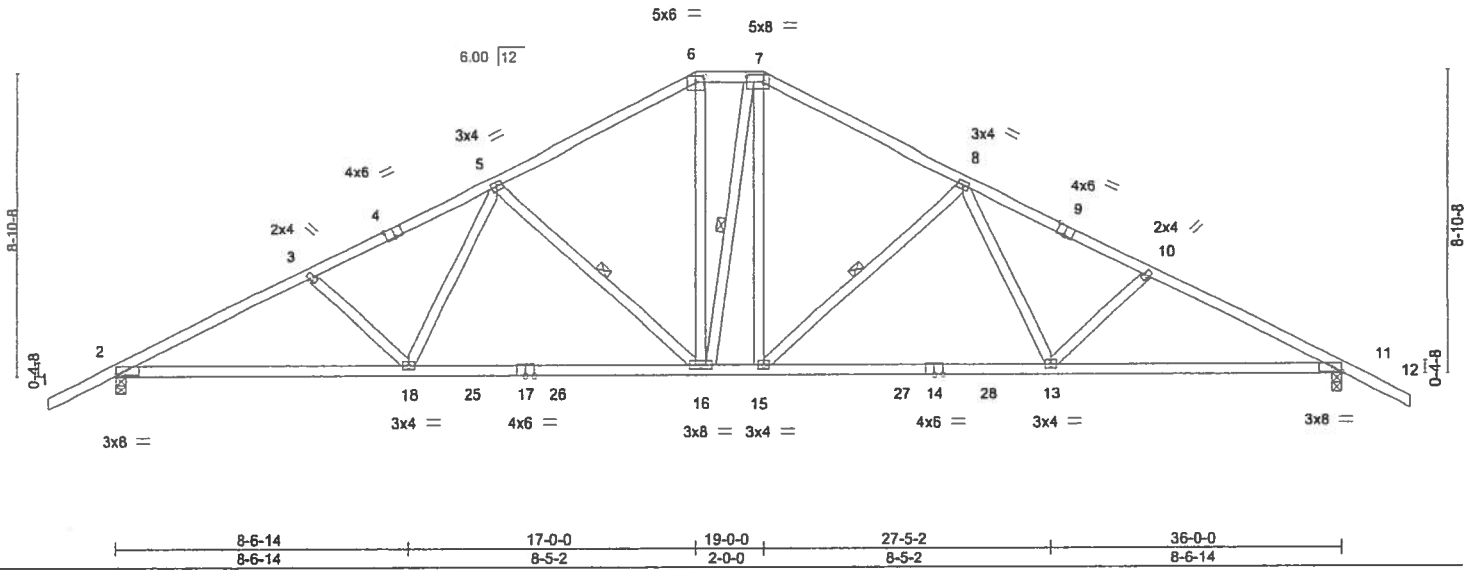


Plate Offsets (X,Y) - [4:0-3-0,Edge], [6:0-3-0,0-2-0], [7:0-6-0,0-2-8], [9:0-3-0,Edge], [11:0-2-15,Edge]

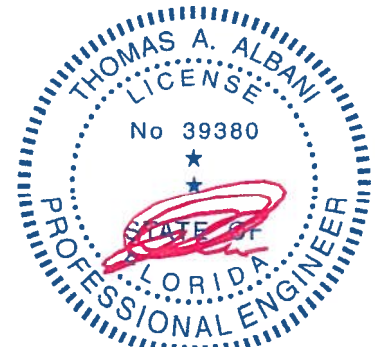
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.70	Vert(LL)	0.38 13-15	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.85	Vert(CT)	-0.42 13-15	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 1.00	Horz(CT)	0.11 11	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS					Weight: 210 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-5-11 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 3-5-5 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-16, 7-16, 8-15

REACTIONS. (lb/size) 2=1440/0-3-8, 11=1440/0-3-8
 Max Horz 2=-129(LC 10)
 Max Uplift 2=-556(LC 9), 11=-556(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2495/2758, 3-5=-2283/2676, 5-6=-1636/1951, 6-7=-1400/1817, 7-8=-1634/1948,
 8-10=-2283/2677, 10-11=-2495/2759
 BOT CHORD 2-18=-2314/2178, 16-18=-1850/1821, 15-16=-1309/1398, 13-15=-1857/1821,
 11-13=-2340/2179
 WEBS 3-18=-273/315, 5-18=-623/467, 5-16=-591/762, 6-16=-707/501, 7-15=-701/497,
 8-15=-594/767, 8-13=-626/469, 10-13=-272/313

- NOTES-
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=556, 11=556.



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 MiTek USA, Inc. FL Gert 6634
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 Date:

September 17, 2019

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Job	Truss	Truss Type	Qty	Ply	MIKE TODD CONST - SMITH CARPORT	T18132991
2067658	T07	Common	2	1		

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Scale: 3/16"=1'

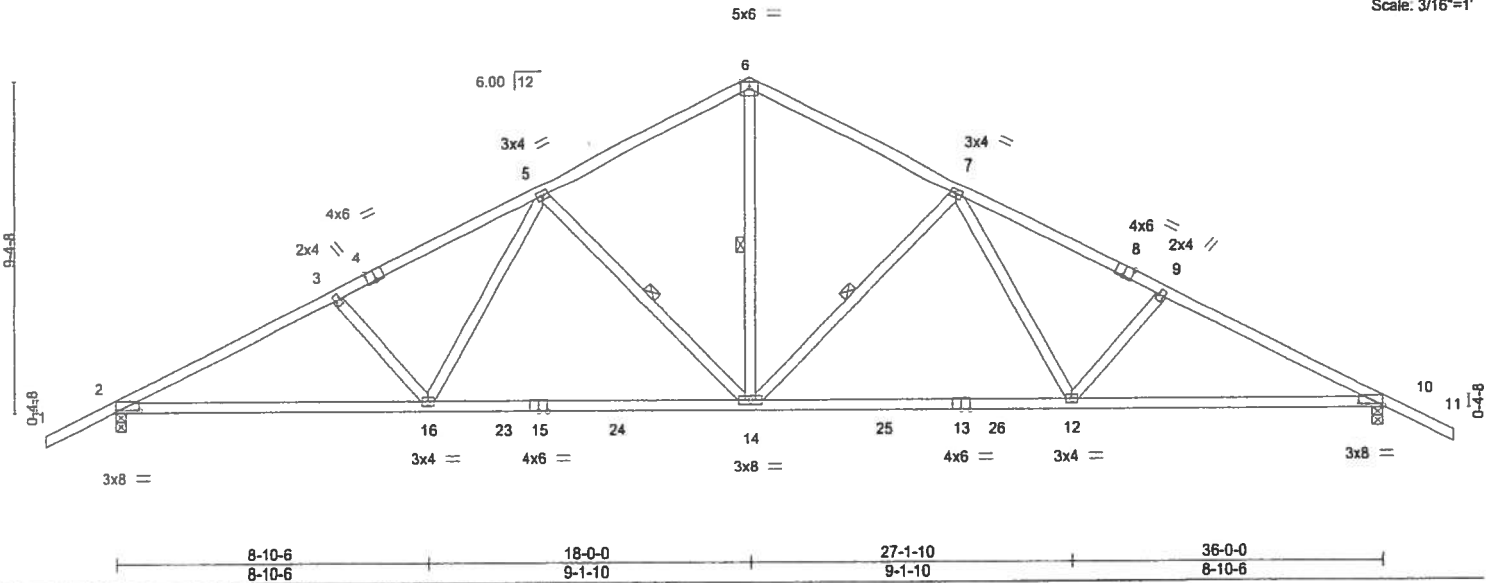


Plate Offsets (X,Y)-	2-0-8-0,0-0-7	4:0-3-0,Edge	8:0-3-0,Edge	10:0-8-0,0-0-7
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.74	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.89	Vert(LL) 0.38 14-16 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.65	Vert(CT) -0.45 14-16 >952 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0.11 10 n/a n/a		
	Code FBC2017/TPI2014			Weight: 189 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-4-11 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 3-4-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 6-14, 7-14, 5-14

REACTIONS. (lb/size) 2=1440/0-3-8, 10=1440/0-3-8
 Max Horz 2=-135(LC 10)
 Max Uplift 2=-538(LC 9), 10=-538(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2476/2741, 3-5=-2268/2682, 5-6=-1570/1906, 6-7=-1570/1906, 7-9=-2268/2682, 9-10=-2476/2741
 BOT CHORD 2-16=-2295/2157, 14-16=-1760/1748, 12-14=-1766/1748, 10-12=-2318/2157
 WEBS 6-14=-1449/1067, 7-14=-605/762, 7-12=-703/505, 9-12=-299/344, 5-14=-605/762, 5-16=-703/505, 3-16=-299/344

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCPi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=538, 10=538.



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September 17,201

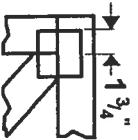
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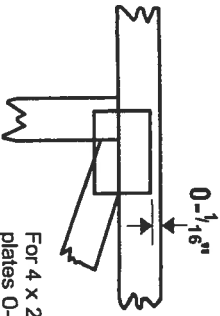
6904 Parke East Blvd. Tampa, FL 36610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

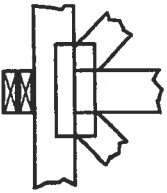
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



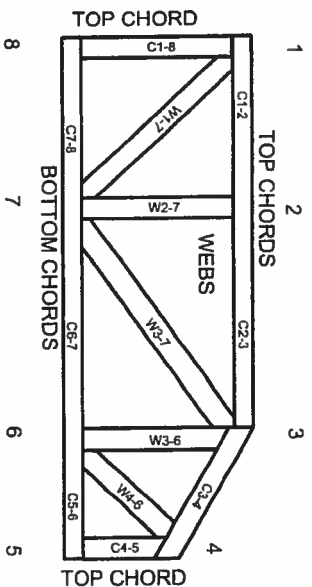
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
Design Standard for Bracing.
DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Top I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 Quality Criteria.