



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 4847424 -

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Customer Info: JOHN NORRIS CONST. Project Name: Goodenbury Res. Model: Custom
Lot/Block: N/A Subdivision: N/A
Address: TBD, TBD
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 25.4
Wind Code: ASCE 7-22 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 3 individual, Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T41346216	T01	5/28/26
2	T41346217	T01G	5/28/26
3	T41346218	T02	5/28/26

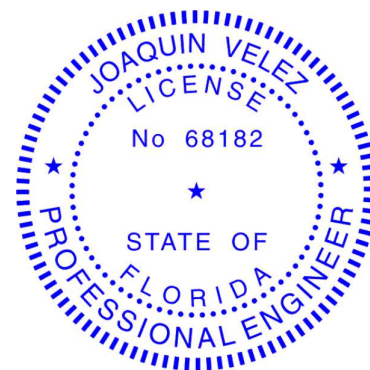
This item has been digitally signed and sealed by Velez, Joaquin, PE on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by
MiTek USA, Inc. under my direct supervision based on the parameters
provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: Velez, Joaquin
My license renewal date for the state of Florida is February 28, 2027.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

May 28,2026

Velez, Joaquin

1 of 1

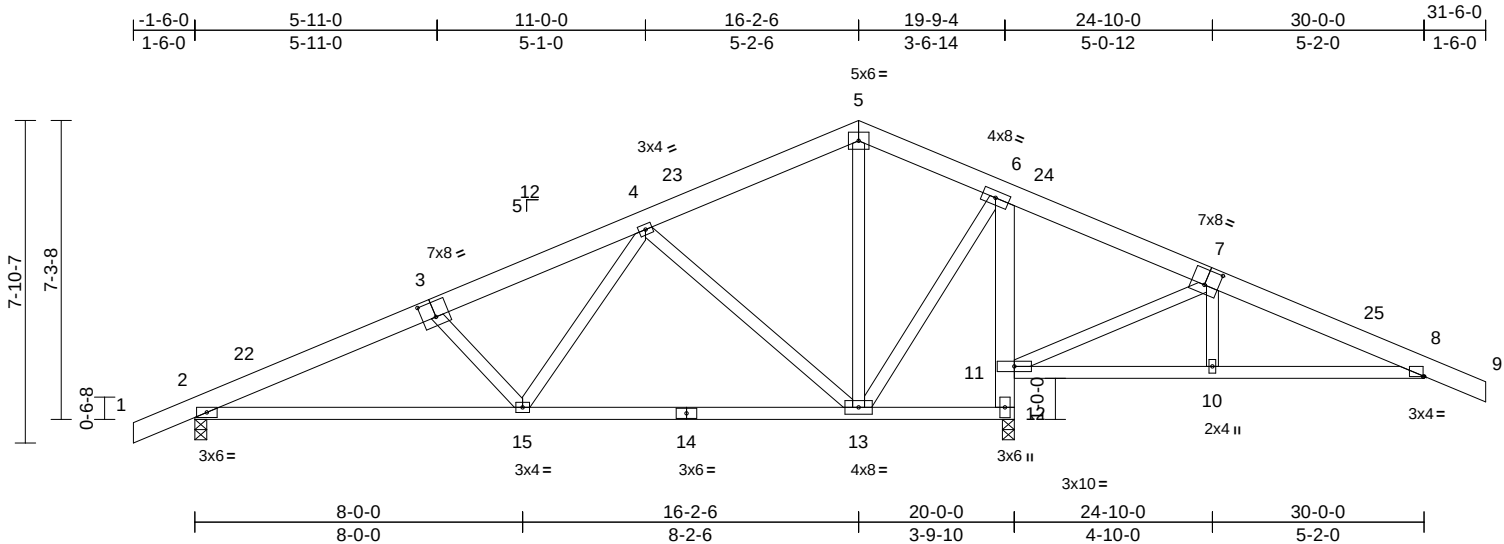
Job	Truss	Truss Type	Qty	Ply	
4847424	T01	Roof Special	10	1	T41346216
Job Reference (optional)					

Builders FirstSource (Lake City,FL), Lake City, FL - 32055,

Run: 25.40 S Apr 17 2026 Print: 25.4.0 S Apr 17 2026 MiTek Industries, Inc. Thu May 28 06:08:06

Page: 1

ID:P8T2VR4ZkFAXu32wqPRP77zD_0D-RfC?PsB70Hq3NSgPqnL8w3uITxbGKWrCDoi7J4zJC?f



Scale = 1:56.2

Plate Offsets (X, Y): [3:0-4-0,0-4-8], [7:0-4-0,0-4-8], [8:0-0-12,0-1-9]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	-0.08	13-15	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.89	Vert(CT)	-0.17	13-15	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.02	12	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 192 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 12-6:2x6 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-8-7 oc bracing.

REACTIONS

(size) 2=0-3-8, 12=0-3-8
Max Horiz 2=145 (LC 12)
Max Uplift 2=-226 (LC 12), 12=-599 (LC 9)
Max Grav 2=749 (LC 25), 12=1957 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-4=-1100/324, 4-5=-244/449, 5-6=-208/438, 6-8=-653/1267, 8-9=0/35
BOT CHORD 2-15=-366/978, 13-15=-184/515, 12-13=-918/597, 11-12=-1982/784, 6-11=-1672/604, 10-11=-603/461, 8-10=-598/457
WEBS 4-13=-659/283, 5-13=-389/223, 7-11=-577/296, 4-15=-111/517, 3-15=-328/188, 7-10=-91/206, 6-13=-397/1188

NOTES (8)

- Unbalanced roof live loads have been considered for this design.
- Designed to ASCE 7-22 basic load combinations for allowable stress design unless otherwise noted.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 16-2-6, Zone2 16-2-6 to 20-5-5, Zone1 20-5-5 to 31-6-0 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 599 lb uplift at joint 12 and 226 lb uplift at joint 2.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

This item has been digitally signed and sealed by Velez, Joaquin, PE on the date indicated here. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Joaquin Velez PE No.68182
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
Date:

May 28,2026

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinet.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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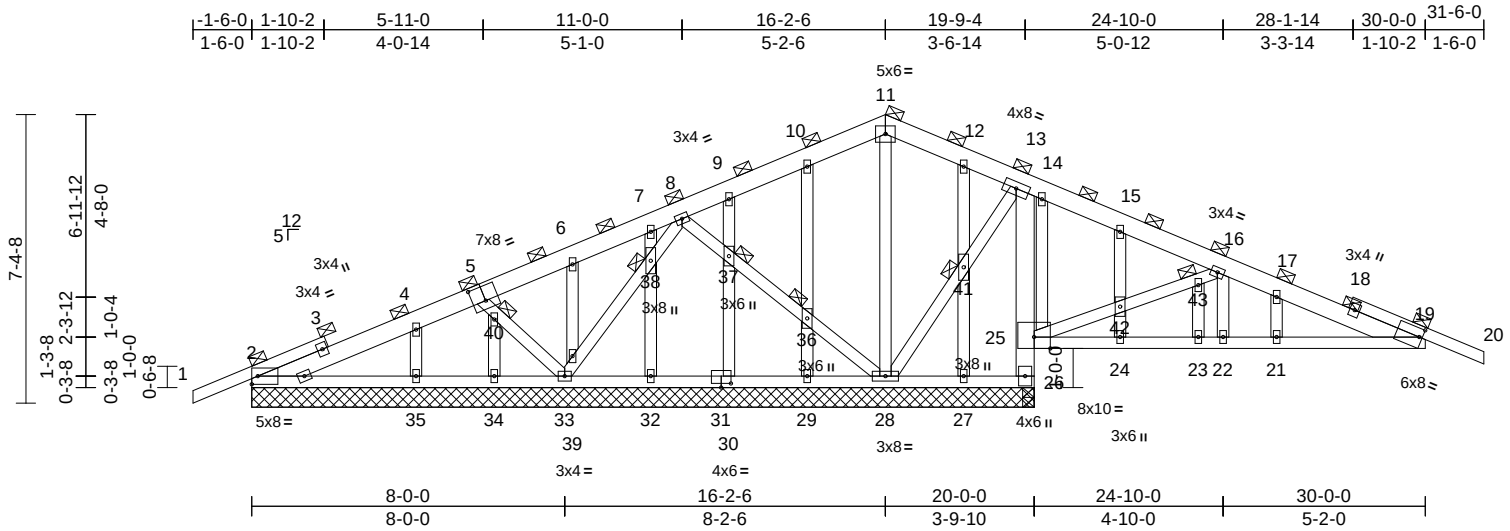
Job	Truss	Truss Type	Qty	Ply	
4847424	T01G	Roof Special Structural Gable	2	1	T41346217
Job Reference (optional)					

Builders FirstSource (Lake City,FL), Lake City, FL - 32055,

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Page: 1

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Scale = 1:58.9

Plate Offsets (X, Y): [5:0-4-0,0-4-8], [19:0-0-15,0-2-9], [31:0-3-0,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	0.00	35-49	>999	240	MT20
TCDL	10.0	Lumber DOL	1.25	BC	0.61	Vert(CT)	-0.01	35-49	>999	180	244/190
BCLL	0.0*	Rep Stress Incr	YES	WB	0.37	Horz(CT)	-0.01	26	n/a	n/a	
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							
Weight: 239 lb FT = 20%											

LUMBER
TOP CHORD 2x6 SP No.2 *Except* 1-3,18-20:2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 26-13:2x6 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-10 oc bracing.

JOINTS
1 Brace at Jt(s): 11, 36, 37, 38, 40, 41, 43

REACTIONS (size)
2=20-0-0, 26=20-0-0, 27=20-0-0, 28=20-0-0, 29=20-0-0, 30=20-0-0, 32=20-0-0, 34=20-0-0, 35=20-0-0
Max Horiz 2=138 (LC 12)
Max Uplift 2=70 (LC 8), 26=-674 (LC 9), 27=-40 (LC 13), 28=-198 (LC 26), 29=-67 (LC 12), 30=-47 (LC 12), 32=-70 (LC 12), 34=-34 (LC 12), 35=-98 (LC 12)
Max Grav 2=249 (LC 25), 26=1574 (LC 26), 27=175 (LC 26), 28=155 (LC 9), 29=230 (LC 1), 30=125 (LC 25), 32=179 (LC 25), 34=109 (LC 25), 35=291 (LC 25)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-4=-79/117, 4-6=-64/157, 6-7=-41/159, 7-8=-48/192, 8-9=-204/488, 9-10=-204/529, 10-11=-161/501, 11-12=-163/510, 12-13=-135/440, 13-14=-640/1253, 14-15=-707/1362, 15-16=-702/1301, 16-17=-491/819, 17-19=-483/776, 19-20=0/35

BOT CHORD 2-35=-92/89, 34-35=-92/89, 33-34=-92/89, 32-33=-242/181, 30-32=-242/181, 29-30=-242/181, 28-29=-242/181, 27-28=-992/625, 26-27=-992/625, 25-26=-1824/1028, 13-25=-1114/649, 24-25=-698/512, 23-24=-700/511, 22-23=-700/511, 21-22=-700/511, 19-21=-700/511
WEBS 8-37=-306/194, 36-37=-279/177, 28-36=-280/177, 11-28=-456/162, 25-42=-529/276, 42-43=-536/279, 16-43=-562/292, 33-39=-62/142, 38-39=-99/226, 8-38=-102/227, 5-40=-110/67, 33-40=-78/67, 16-22=-97/164, 28-41=-541/958, 13-41=-521/922, 10-36=-183/96, 29-36=-180/95, 9-37=-70/63, 30-37=-92/62, 7-38=-117/70, 32-38=-125/80, 6-39=-97/59, 34-40=-76/43, 4-35=-214/104, 12-41=-88/80, 27-41=-63/51, 14-25=-370/187, 15-42=-34/85, 24-42=-36/67, 23-43=-66/31, 17-21=-26/56

NOTES (12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Designed to ASCE 7-22 basic load combinations for allowable stress design unless otherwise noted.
- 3) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 1-6-0 to 1-4-2, Zone1 1-4-2 to 16-2-6, Zone2 16-2-6 to 20-2-6, Zone1 20-2-6 to 31-6-0 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 6) All plates are 2x4 (||) MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 26, 198 lb uplift at joint 28, 67 lb uplift at joint 29, 47 lb uplift at joint 30, 70 lb uplift at joint 32, 34 lb uplift at joint 34, 98 lb uplift at joint 35, 40 lb uplift at joint 27, 70 lb uplift at joint 2 and 70 lb uplift at joint 2.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Chesterfield, MO 63017
Date:

May 28,2026

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of the design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinet.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

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Job	Truss	Truss Type	Qty	Ply	
4847424	T01G	Roof Special Structural Gable	2	1	T41346217
					Job Reference (optional)

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

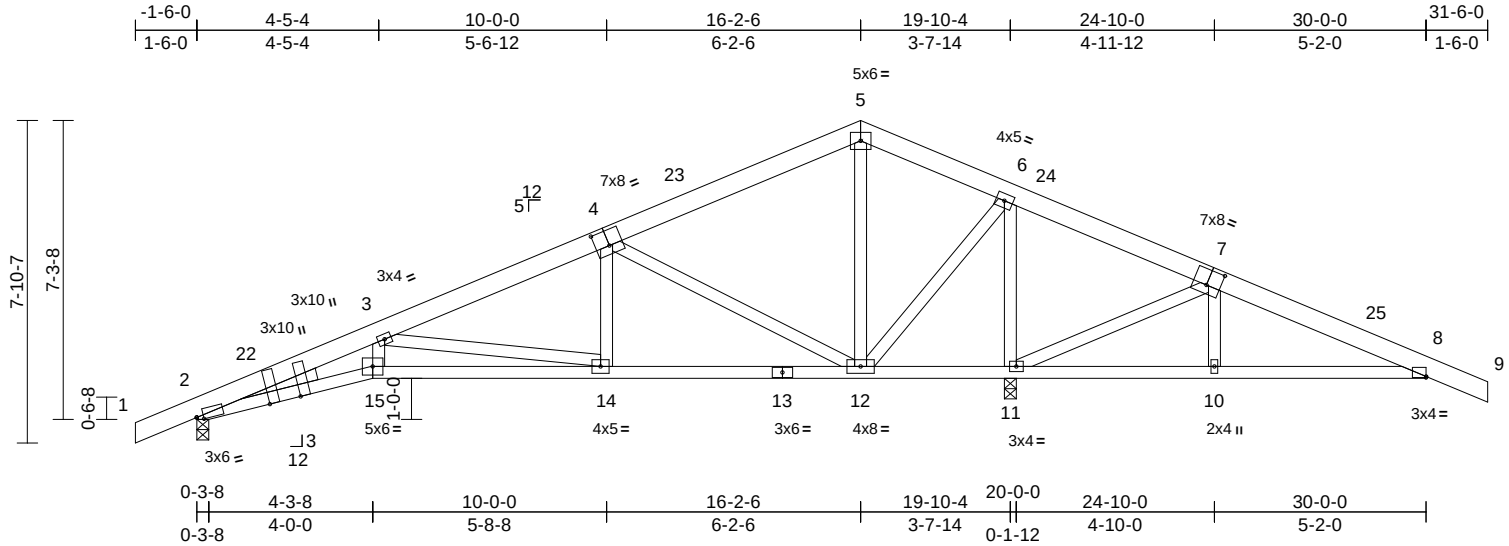
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Job	Truss	Truss Type	Qty	Ply	
4847424	T02	Roof Special	9	1	T41346218
Job Reference (optional)					

Builders FirstSource (Lake City,FL), Lake City, FL - 32055,

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Page: 1



Scale = 1:56.2

Plate Offsets (X, Y): [2:0-2-0,0-0-14], [2:0-1-6,Edge], [2:0-1-6,Edge], [4:0-4-0,0-4-8], [7:0-4-0,0-4-8], [8:Edge,0-0-5]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	-0.11	14-15	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.56	Vert(CT)	-0.20	14-15	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.08	11	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 188 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-6-2 oc bracing.

REACTIONS (size) 2=0-3-8, 11=0-3-8
Max Horiz 2=145 (LC 12)
Max Uplift 2=-226 (LC 12), 11=-595 (LC 9)
Max Grav 2=754 (LC 25), 11=1949 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2197/731, 3-5=-975/516, 5-6=-281/527, 6-8=-659/1270, 8-9=0/35
BOT CHORD 2-15=-762/2067, 14-15=-719/1926, 12-14=-301/881, 11-12=-1126/733, 10-11=-599/458, 8-10=-593/454
WEBS 3-15=-154/549, 4-12=-943/365, 5-12=-512/327, 6-12=-473/1305, 6-11=-1608/577, 7-11=-587/305, 7-10=-102/214, 4-14=-39/400, 3-14=-1064/425

NOTES (9)

- Unbalanced roof live loads have been considered for this design.
- Designed to ASCE 7-22 basic load combinations for allowable stress design unless otherwise noted.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 16-2-6, Zone2 16-2-6 to 20-5-5, Zone1 20-5-5 to 31-6-0 zone; cantilever right exposed :C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2 and 595 lb uplift at joint 11.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

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Date:

May 28,2026

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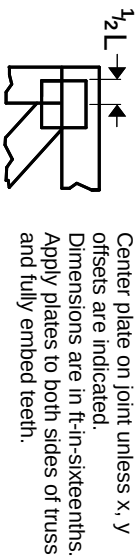
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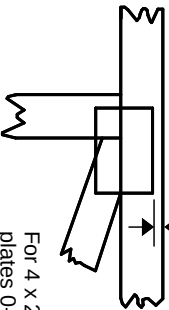
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Symbols

PLATE LOCATION AND ORIENTATION



0- $\frac{1}{16}$ "



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

—
This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

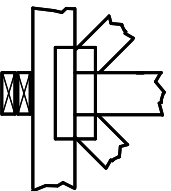
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

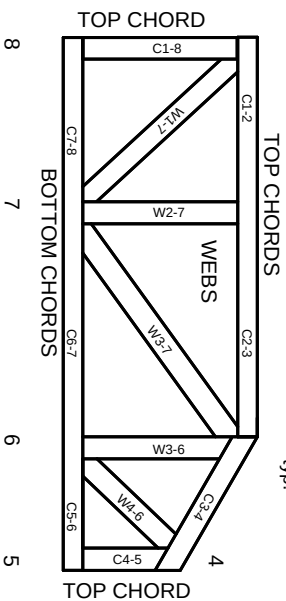
Industry Standards:

ANSI/TP1.1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)

1 2 3 Joint ID typ.



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek®

MITek Engineering Reference Sheet: MIL-7473 rev. 1/12/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.