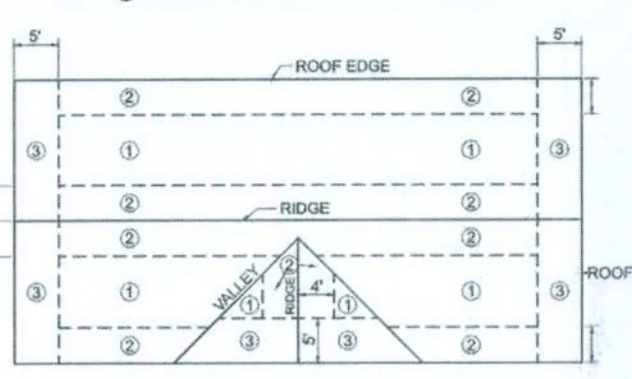
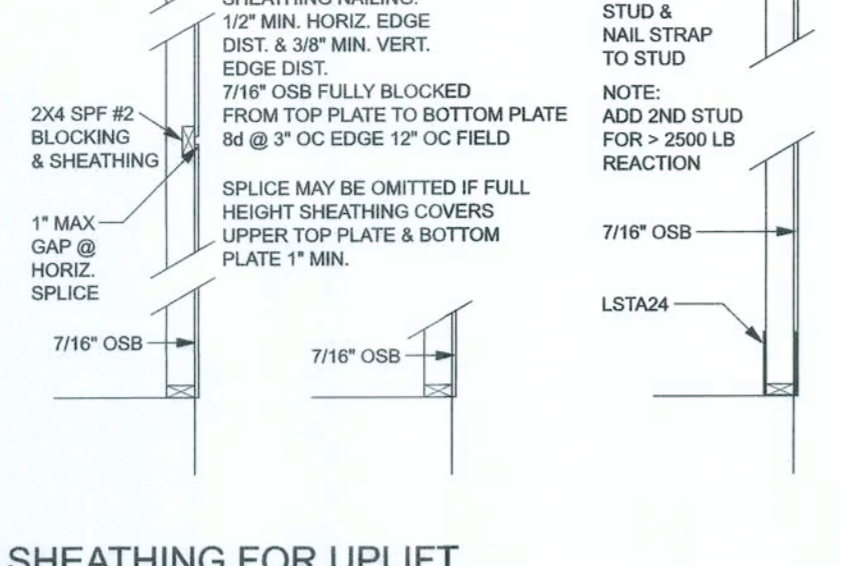
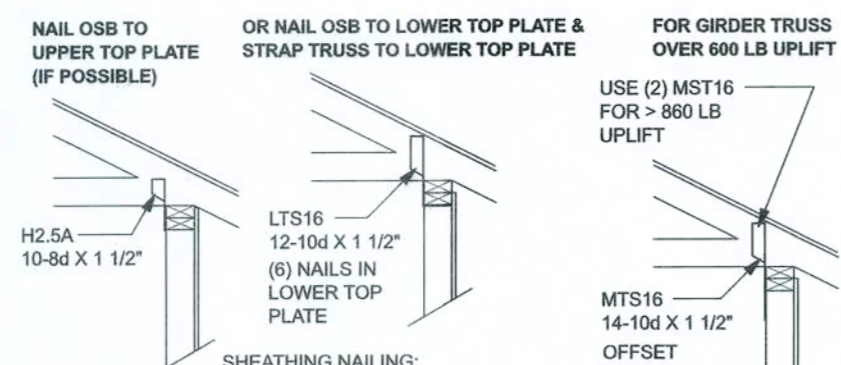


- RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
- 0.113 INCH NOMINAL ROOT SHANK DIAMETER
  - RING DIAMETER OF 0.010 OVER SHANK DIAMETER
  - 16 TO 20 RINGS PER INCH
  - 0.280 INCH FULL ROUND HEAD DIAMETER
  - 2.38 INCH NAIL LENGTH

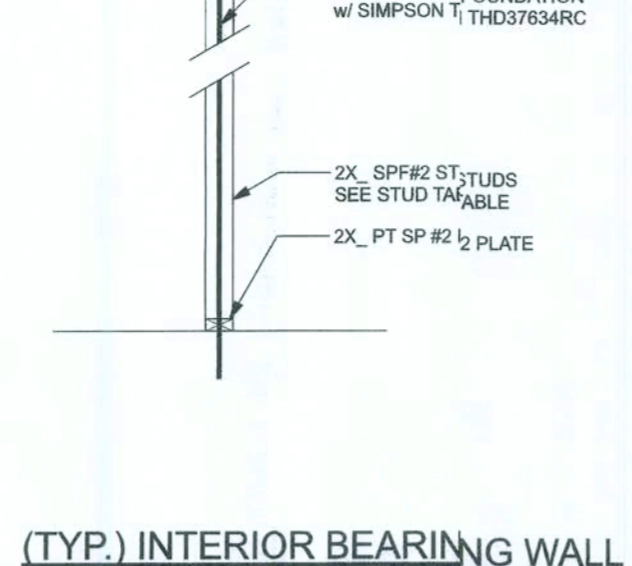
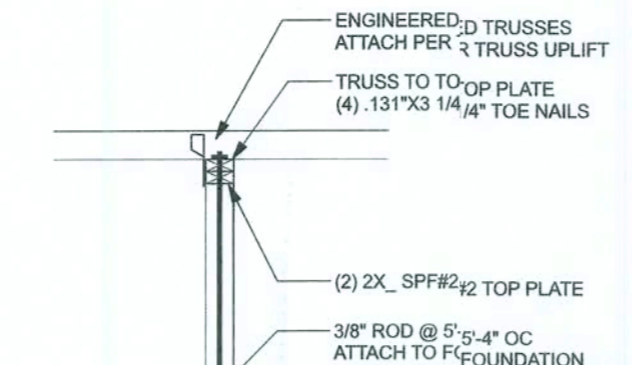
- NAILING PATTERN SHALL BE:
- 4" OC GABLE END (SEE GABLE BRACING DETAIL)
  - 6" OC @ EDGES ALL ZONES
  - 12" OC @ INTERMEDIATE FRAMING IN ZONE 3
  - 12" OC @ INTERMEDIATE FRAMING IN ZONE 1 & 2



**ROOF SHEATHING FASTENING**  
ONE STORY WOOD FRAME



**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME



**(TYP.) INTERIOR BEARING WALL**  
ONE STORY WOOD FRAME w/ RODS

**CONNECTOR TABLE**

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	250	HS	4-8x61 1/2"	4-8x61 1/2"
575	485	H2.5A	5-8x61 1/2"	5-8x61 1/2"
1340	1015	H10A	9-10x11 1/2"	9-10x11 1/2"
720	620	LTS12-20	6-10x11 1/2"	6-10x11 1/2"
1000	860	MTS12-30	7-10x11 1/2"	7-10x11 1/2"
1490	1248	HTS20-30	12-10x11 1/2"	12-10x11 1/2"
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Stud Ties</b>	<b>To One Member</b>	<b>To Other Member</b>
1235	1235	LSTA21	9-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Stud Plate Ties</b>	<b>To Stud</b>	<b>To Plate</b>
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Holdowns @ Stenwall</b>	<b>To Stud / Post</b>	<b>Anchor</b>
1325	1600	D1T22	8-SDS 14"x11 1/2"	1/2"x12" Titen HD
4235	3540	H1T4	18-16x62 1/2"	1/2"x12" Titen HD
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Holdowns @ Mono</b>	<b>To Stud / Post</b>	<b>Anchor</b>
1625	1800	D1T22	8-SDS 14"x11 1/2"	1/2"x6" Titen HD
4235	3540	H1T4	18-16x62 1/2"	1/2"x12" Titen HD
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Post Bases @ Stenwall</b>	<b>To Post</b>	<b>Anchor</b>
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy
<b>Uplift SP</b>	<b>Uplift SPF</b>	<b>Post Bases @ Mono</b>	<b>To Post</b>	<b>Anchor</b>
2200	ABU44	ABU44	12-16d	5/8"x7" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x7" Drill & Epoxy

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

FOUNDATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f'_c = 2500$  PSI. WELDED WIRE REINFORCED SLAB: 8" x 8" W/ 4" x W/ 4" #3 @ 18" O.C. WELDED WIRE REINFORCEMENT FABRIC (W.W.F.) CONFORMING TO ASTM A186 IS LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1119. SUPPLIER TO PROVIDE ASTM C 1119 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPLICES 40" DB OR 2X BAR LENGTH, UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICE LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

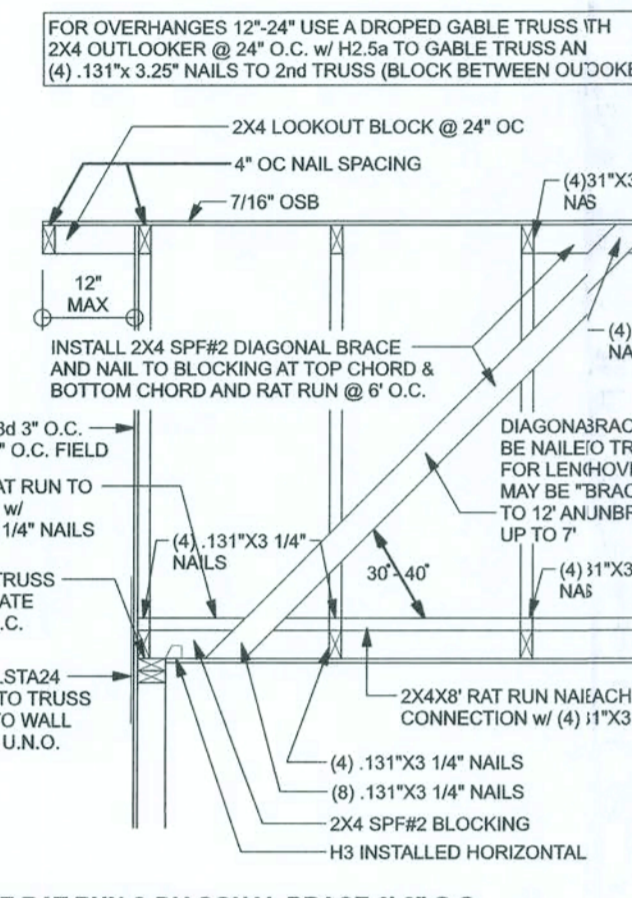
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 18" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**  
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

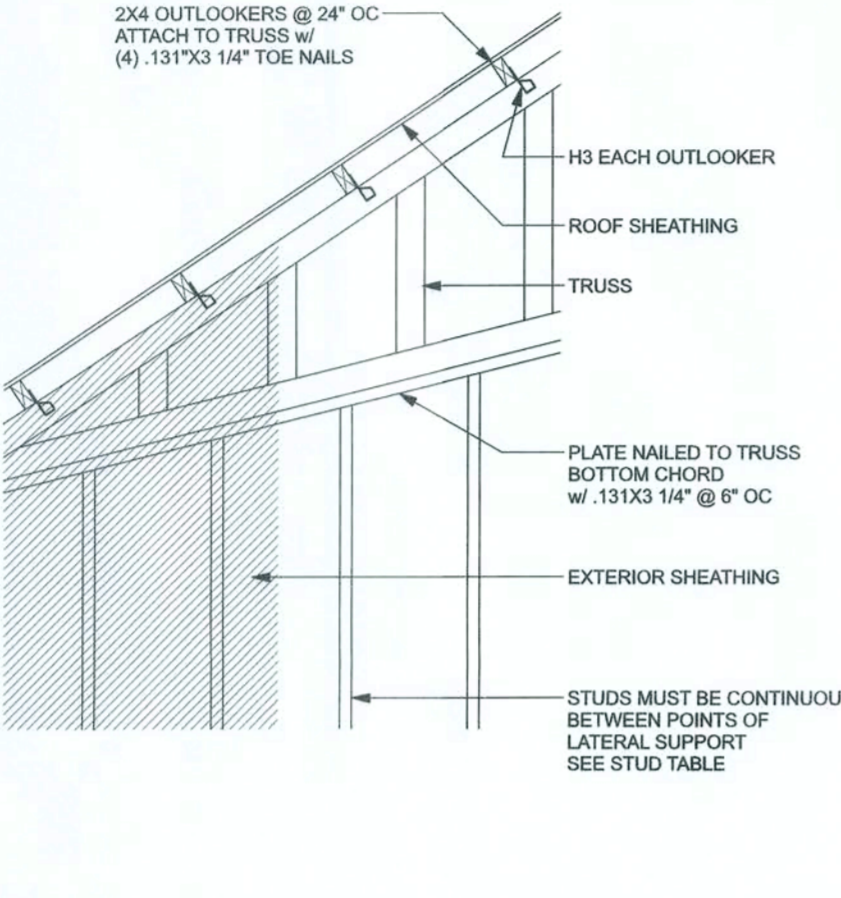
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

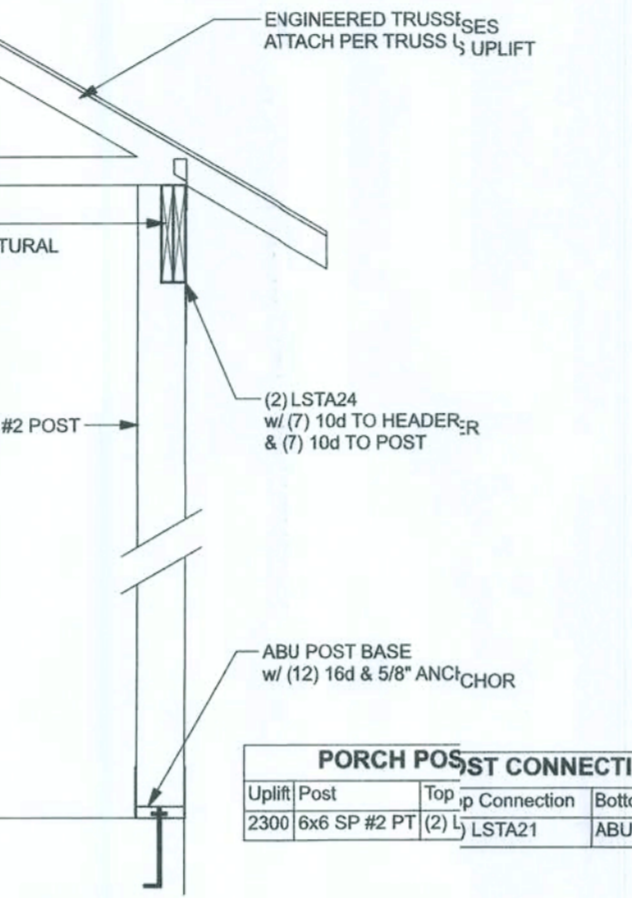
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME



**(TYP.) PORCH POST**  
ONE STORY WOOD

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

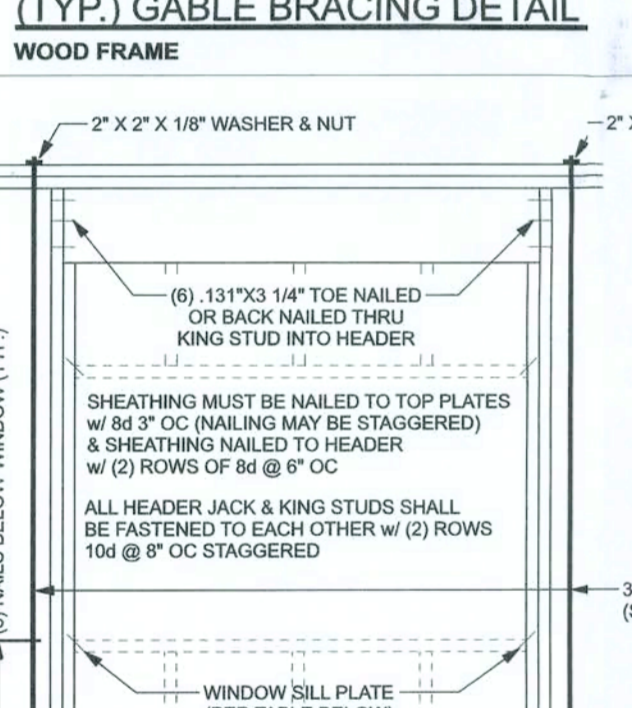
STUD HEIGHT	STUD SPACING
(1) 24" @ 16" OC	TO 10'-11" STUD HEIGHT
(1) 24" @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 24" @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

**GRADE & SPECIES TABLE**

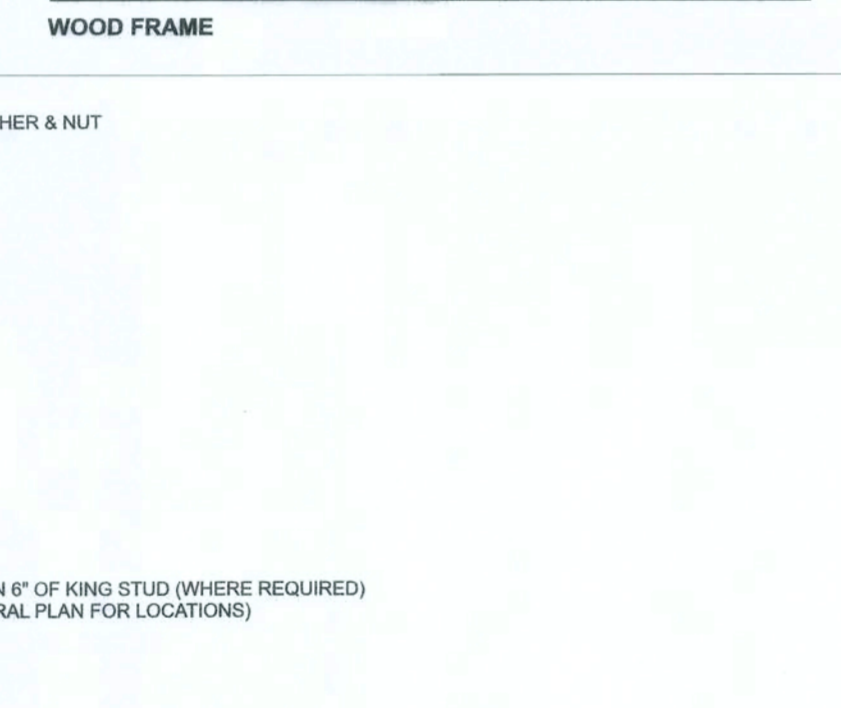
GRADE	SP #2	Fb	E
2x6	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24-F/3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**PORCH POST CONNECTIONS**

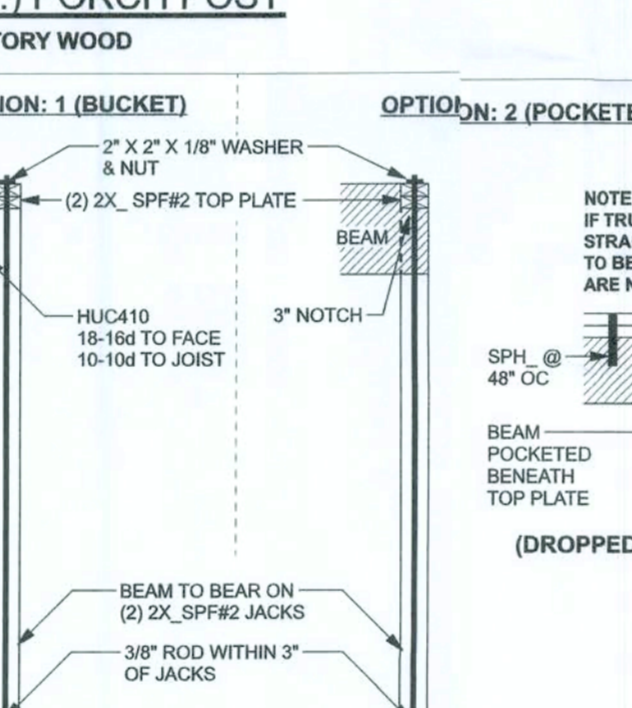
Uplift Post	Top Connection	Bottom Connection
2300 6x6 SP #2 PT [2] 1/2	LSTA21	ABU66



**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ RODS

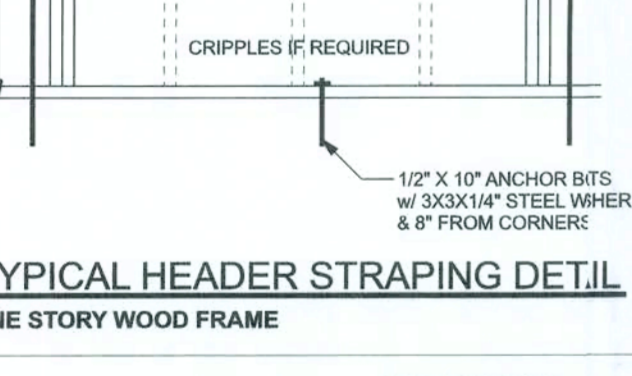


**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME

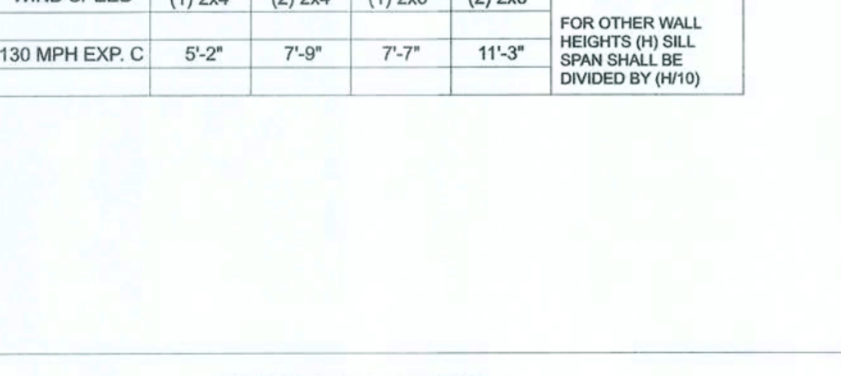
**SILL PLATE SPANS FOR 10'-0" WALL HEIGHT**

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	(BASED ON WFCM TABLE A-3.2B)
130 MPH EXP. C	5'-2"	7'-9"
	(1) 2x4	(2) 2x4
	(2) 2x4	(1) 2x6
	(2) 2x4	(2) 2x6
	(1) 2x4	(2) 2x6

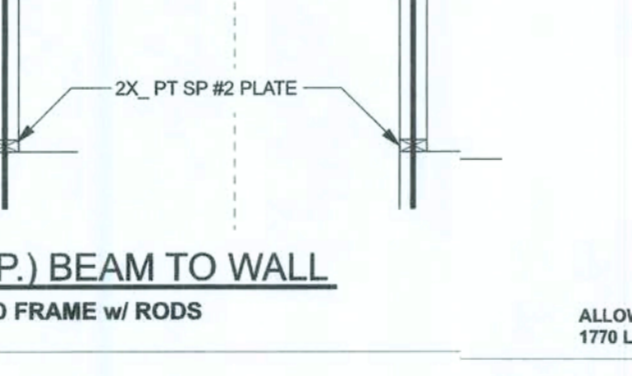
FOR OTHER WALL HEIGHTS (9' TO 11'), SILL SPAN SHALL BE DIVIDED BY (9/10)



**TYPICAL HEADER STRAPPING DETAIL**  
ONE STORY WOOD FRAME



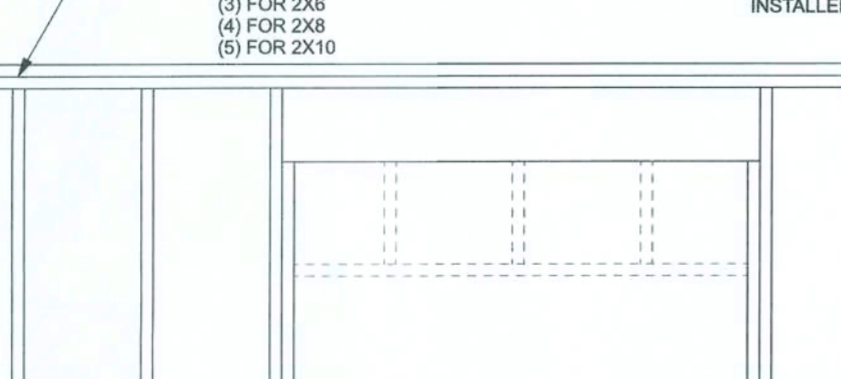
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



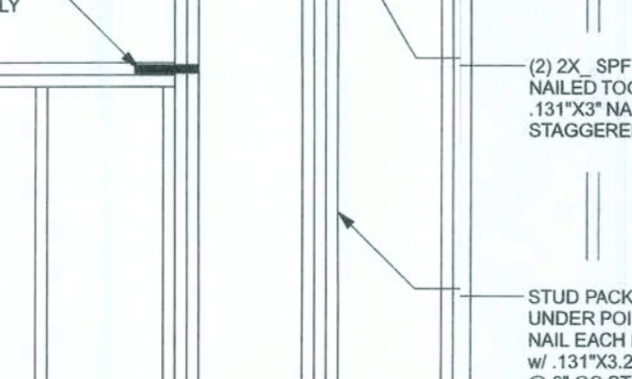
**(TYP.) CORNER FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

John Norris Construction, L.L.C.

Raleigh & Melissa Summrell Res.

PROJECT ADDRESS:  
389 NW Felling Creek Rd.  
Lake City, FL 32055

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and let the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This designs valid for one building, at specified location.

MARK DISOWAY P.E. 53915



Tuesday, March 24, 2020

Mark Disoway P.E.  
Suite 103  
163 SW Midtown Place  
Lake City, Florida 32025  
386.755.5419  
disowaydesign@gmail.com

JOB NUMBER:  
200349

S-1  
OF 3 SHEETS

**DESIGN CRITERIA & LOADS:**

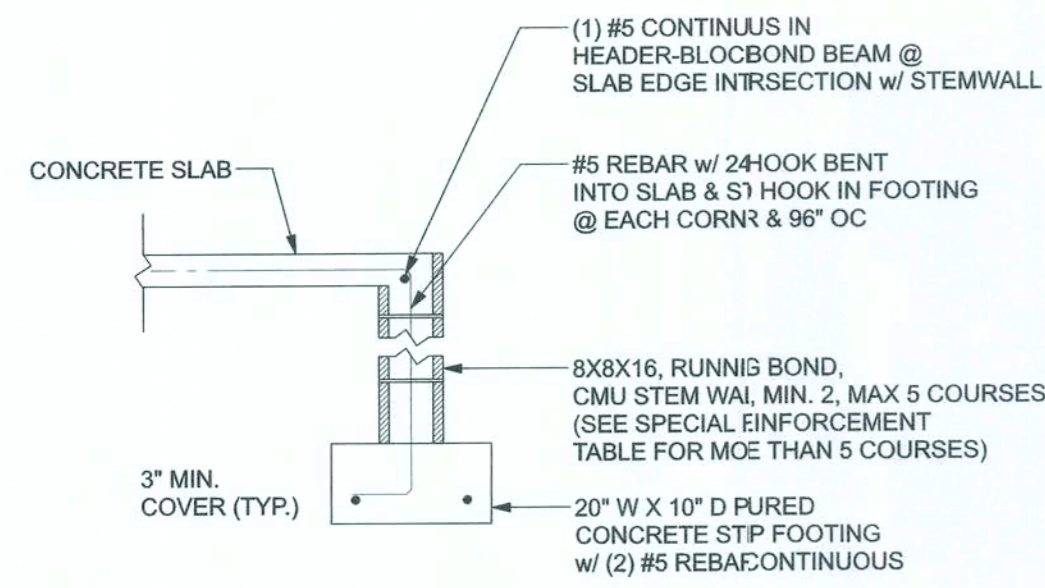
BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
RISK CATEGORY	I
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7.45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C &amp; C DESIGN PRESSURES</b>	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 AND GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

**EFFECTIVE WIND AREA (Ft²)**

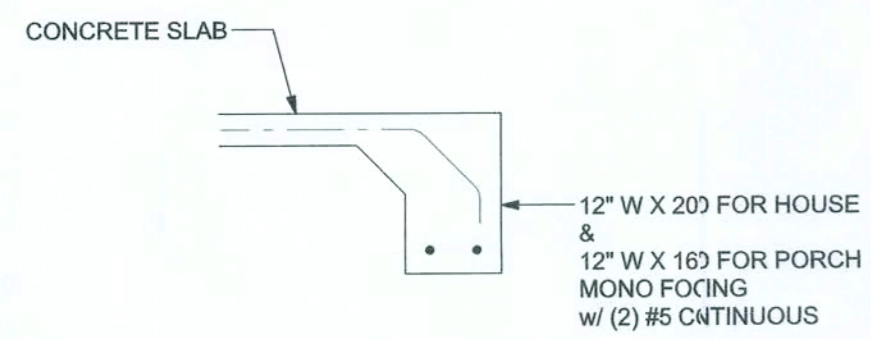
ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vassd) -27.8(Vassd) +25.6(Vassd) -34.2(Vassd)
0 - 20	+42.8(Vult) -46.2(Vult) +42.8(Vult) -57(Vult)

**GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)**

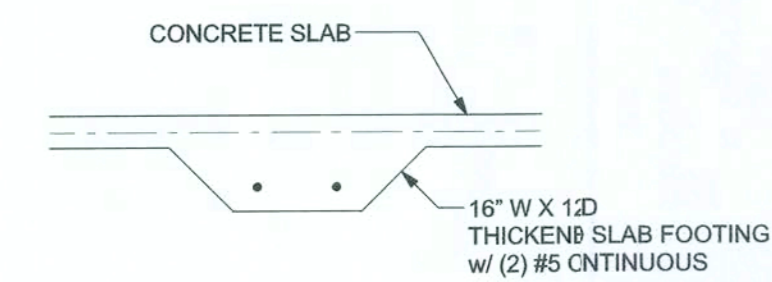
9x7 GARAGE DOOR	16x7 GARAGE DOOR
+22.8(Vassd) -25.5(Vassd)	+21.7(Vassd) -24.1(Vassd)



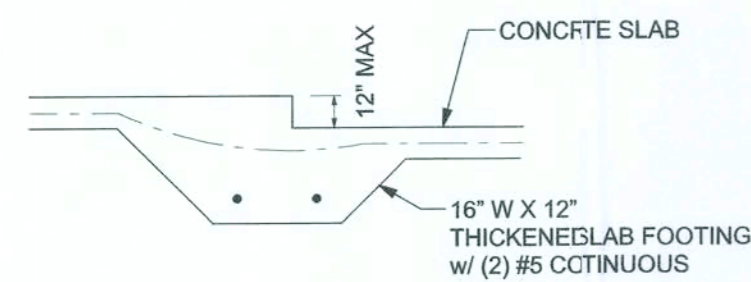
**F1 S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



**F1 S-2** OPTIONAL MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"

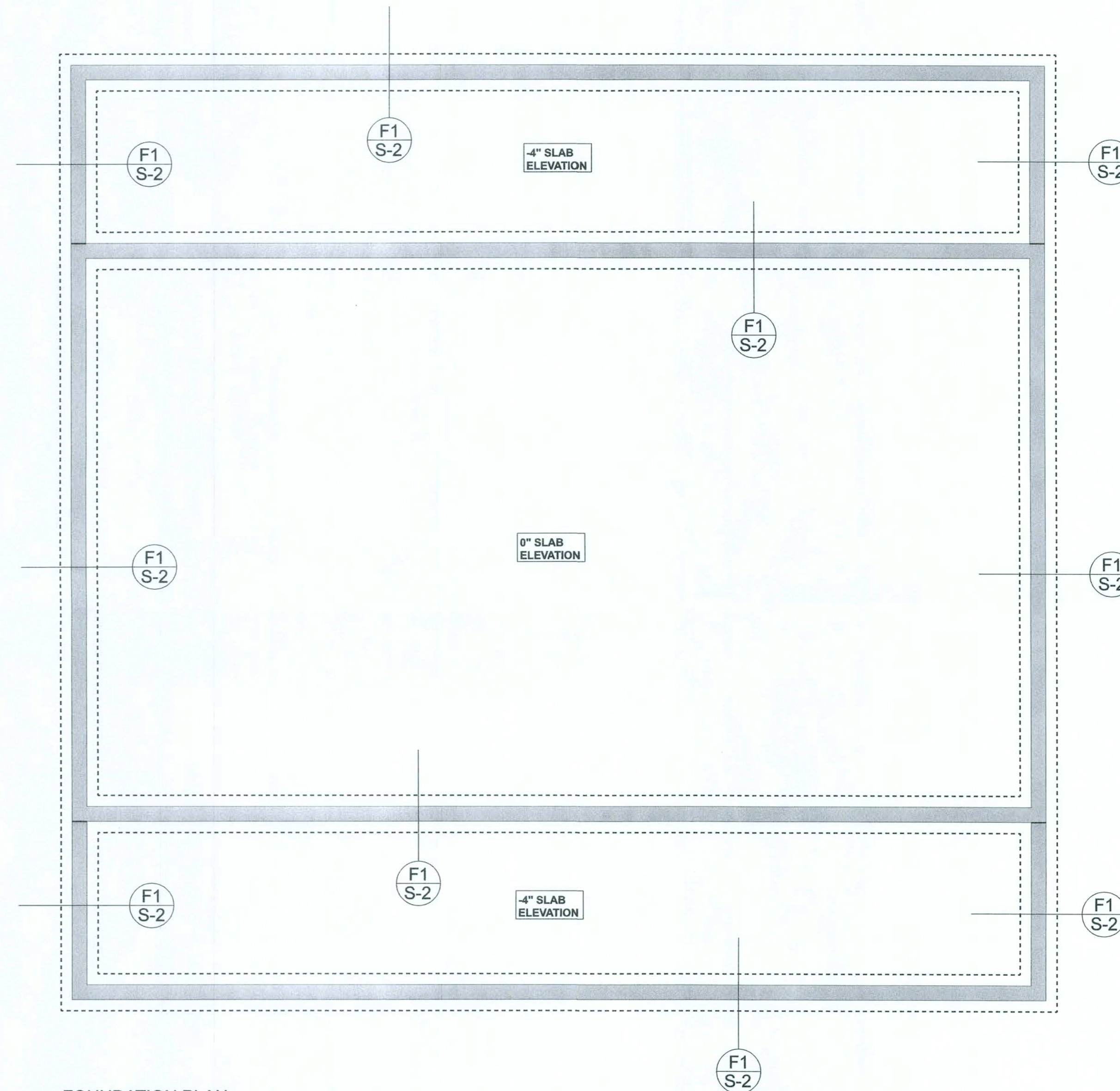
**TALL STEM WALL TABLE:**  
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements	
1.4A	Compressive strength	8" block bearing walls Fm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.9x2.75x11.9"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 40 ksi, Lap splices min 40 bar dia. (20" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints, if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017-RES. SECTION R403.1.4



**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"

- FOUNDATION NOTES**
- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
  - CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
  - FN - 2 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ #6-1.41.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1/2" DEPTH OR FIBER MESH CONCRETE, 4-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).

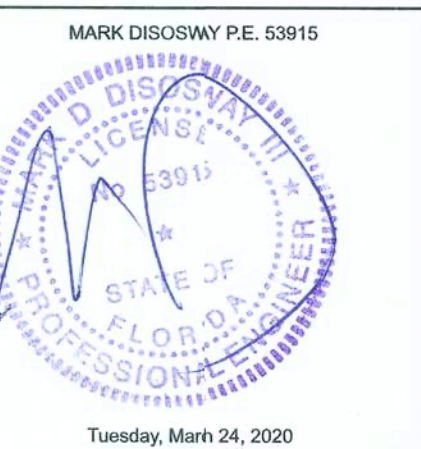
John Norris Construction L.L.C.  
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**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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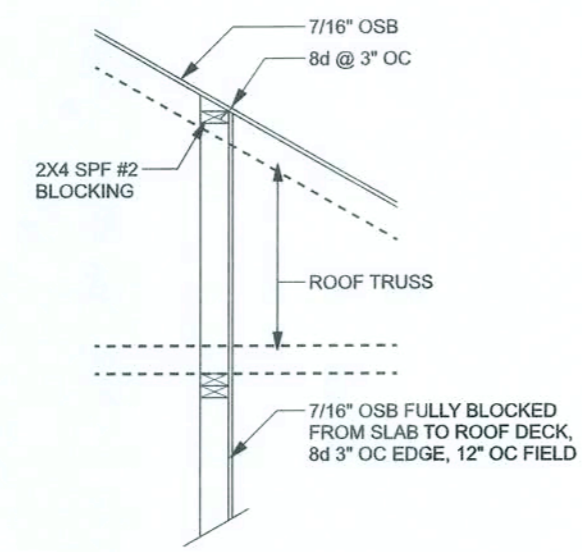
**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.



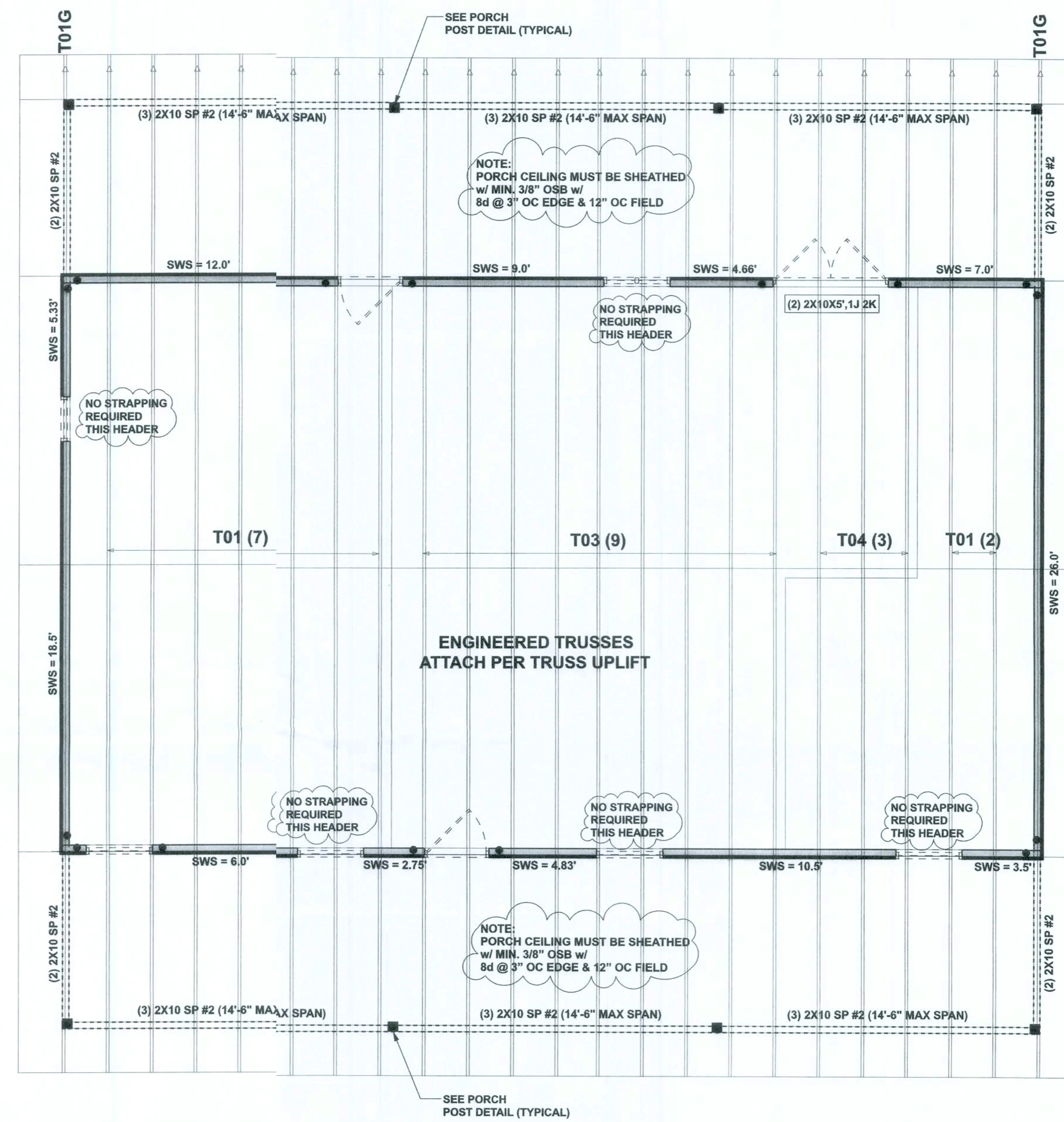
Mark Disosway P.E.  
163 SW Midown Place  
Suite 103  
Lake City, Florida 32025  
386.751.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
200/49  
**S2**  
OF 3 SHEETS



**ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL**

NOTE:  
IF THE ABOVE DETAIL IS USED ON THE FRONT & REAR WALL @ THE PORCH THE PORCH CEILING DOES NOT NEED TO BE SHEATHED



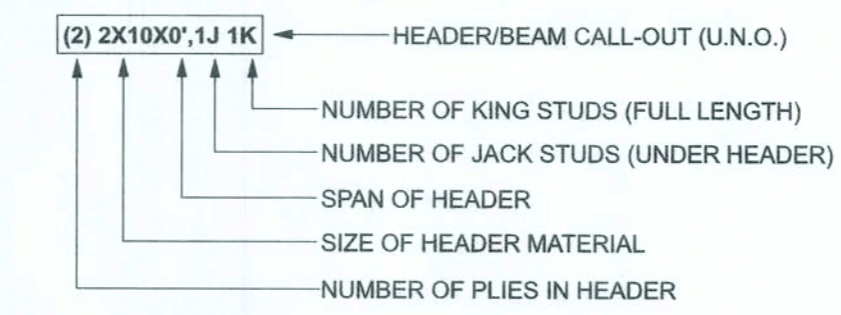
**STRUCTURAL PLAN**

SCALE: 1/4" = 1'-0"

**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & P PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-01-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**HEADER LEGEND**



**THREADED ROD LEGEND**



**ACTUAL vs REQUIRED SHEARWALL**

	TRANSVERSE	LONGITUDINAL
ACTUAL	11959 LBF	14457 LBF
REQUIRED	9154 LBF	5417 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2281688

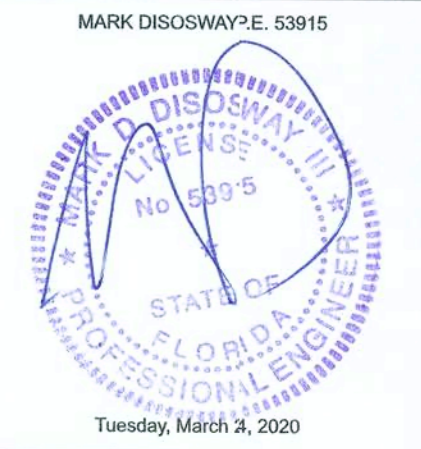
John Norris Construction L.L.C.  
Raleigh & Melissa Summerall Res.  
PROJECT ADDRESS:  
3867 W. US HWY 90  
LAKE CITY, FL 32025

**DIMENSIONS:**  
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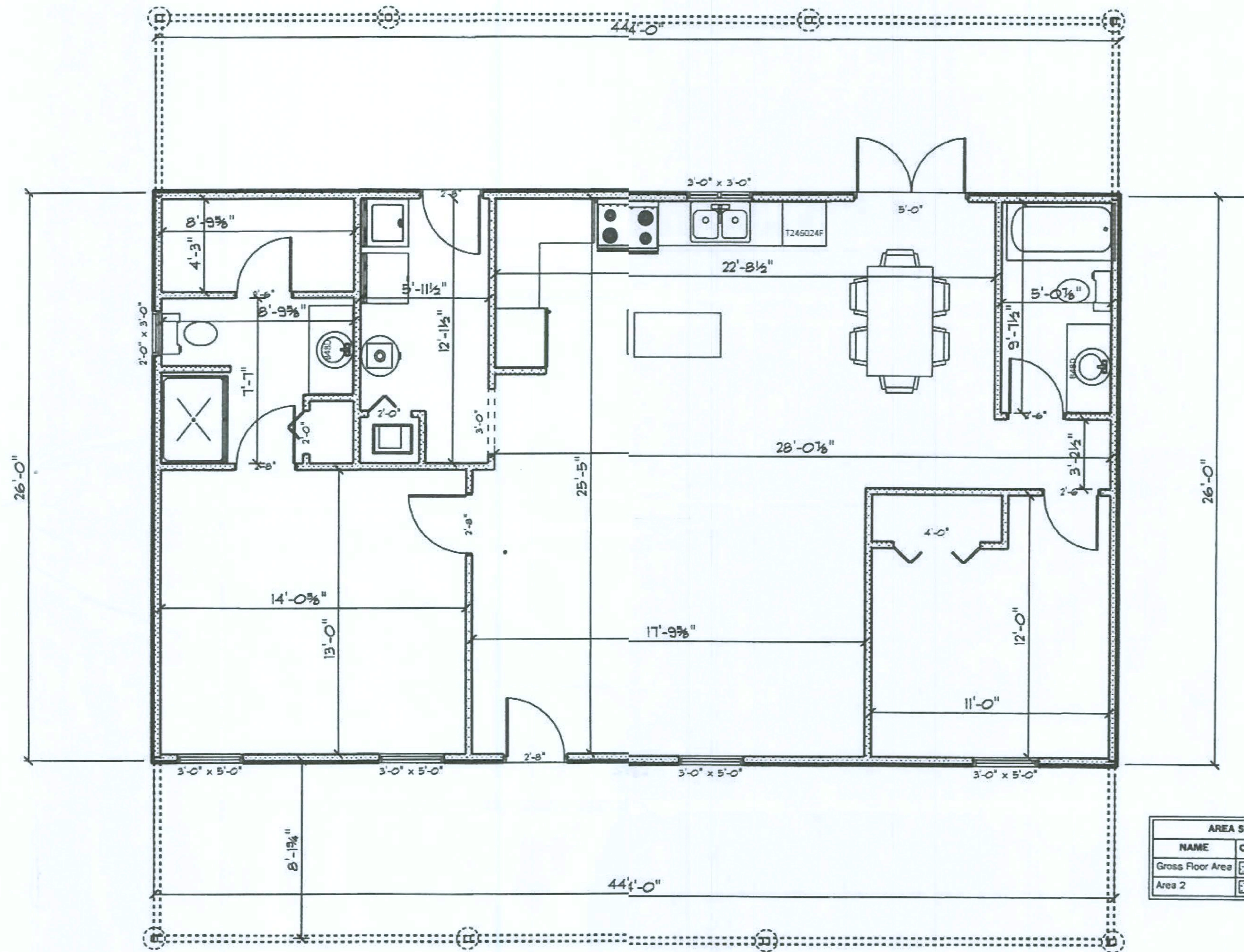
**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to my engineering comply with the 8th Edition Florida Building Code Residential (D17) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.



Mark Discovay P.E.  
163 SW Midtown Place  
Suite 113  
Lake City, Florida 32025  
386.754.4119  
discovaydesign@gmail.com

JOB NUMBER:  
200349  
**S-3**  
OF 3 SHEETS

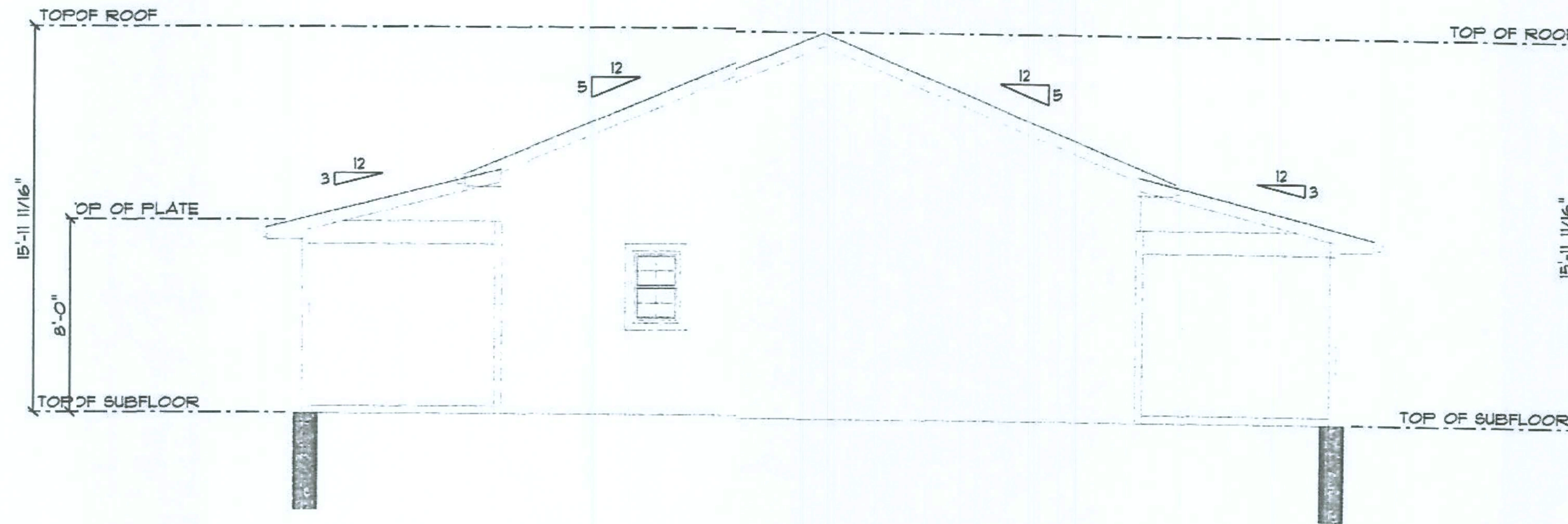


AREA SCHEDULE		
NAME	COLOR	AREA
Gross Floor Area	[Pattern]	1161.6 sq. ft.
Area 2	[Pattern]	713.2 sq. ft.

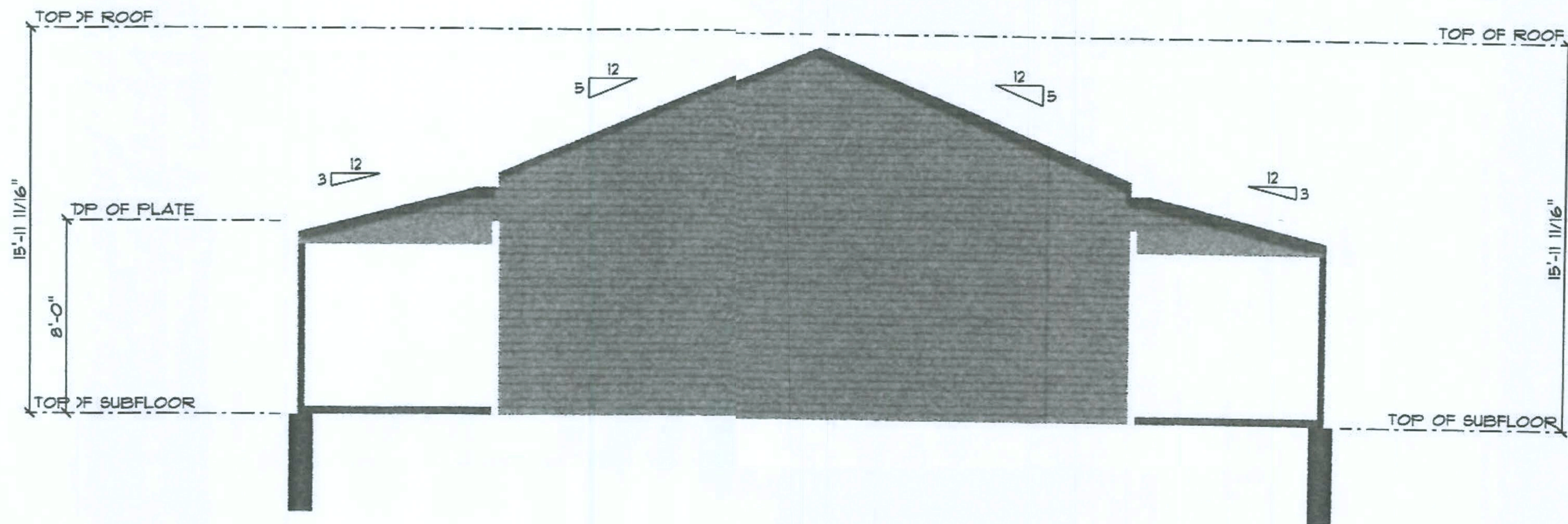
# Summerall - MAIN FLOOR

SCALE: 1/8" = 1'-0"

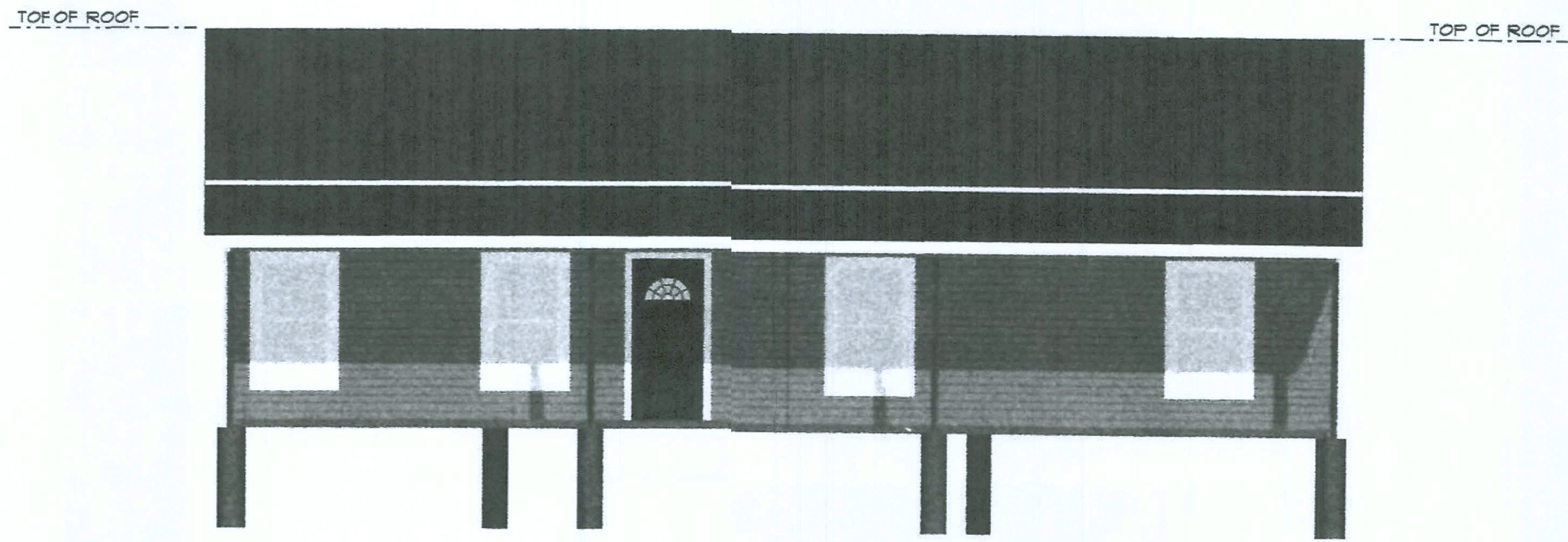
John Norris Construction  
 Raleigh and Melissa Summerall  
 682 NW Falling Creek Rd  
 Lake City, FL 32055



Left Elevation  
SCALE: 1/8" = 1'-0"

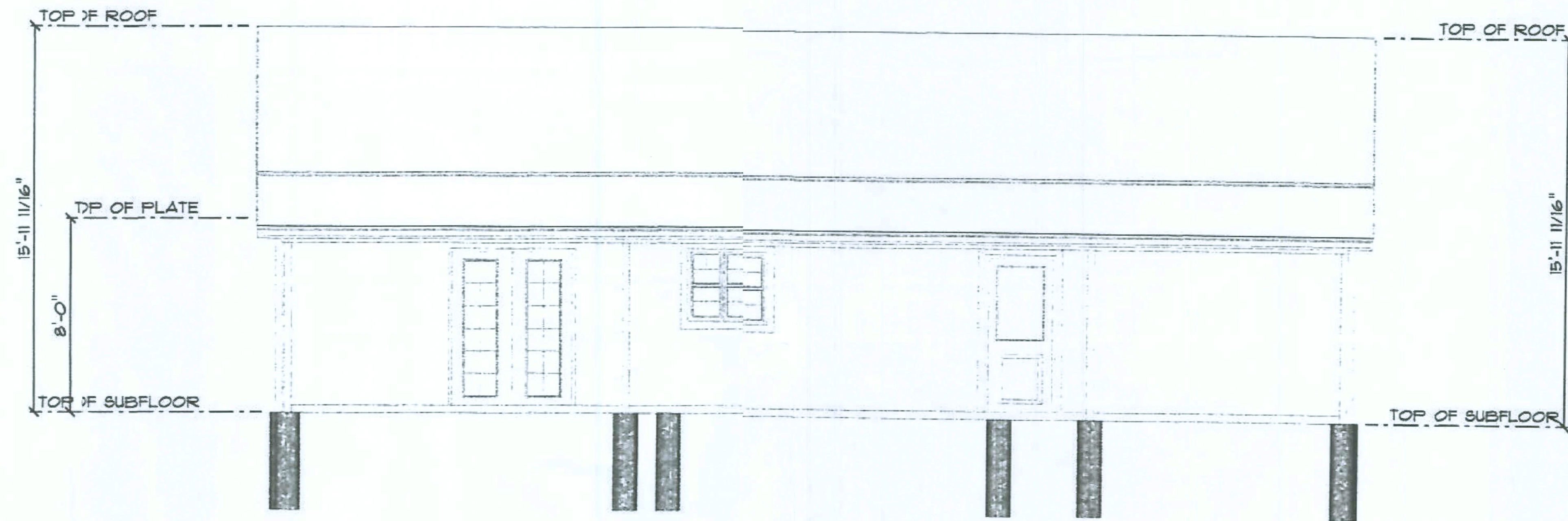


Right Elevation  
SCALE: 1/8" = 1'-0"



## Front Elevation

SCALE: 1/8" = 1'-0"



## Rear Elevation

SCALE: 1/8" = 1'-0"