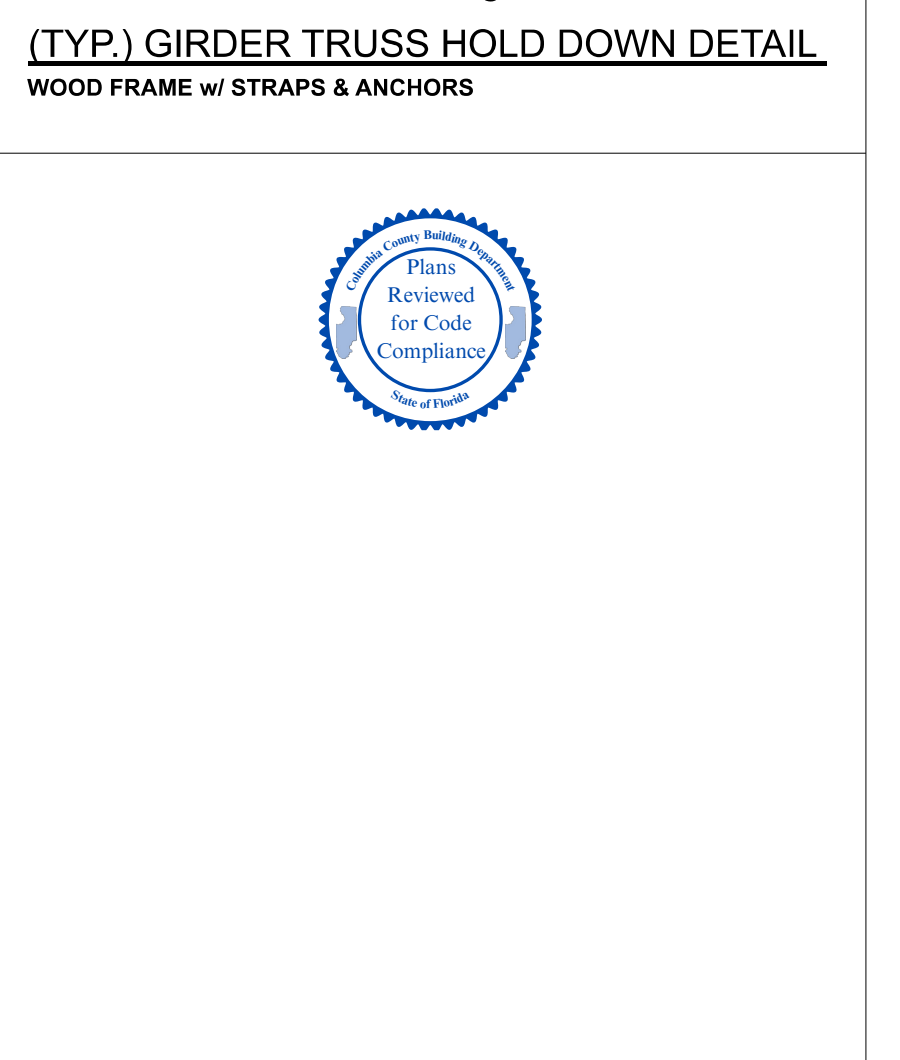
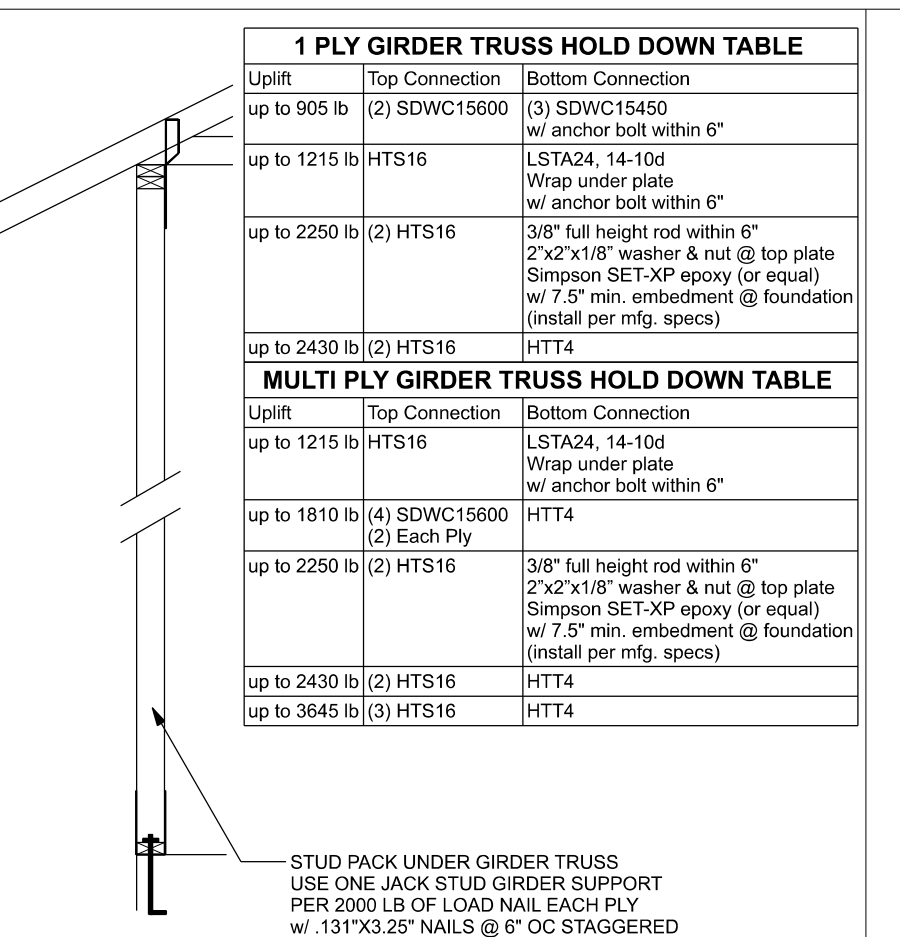
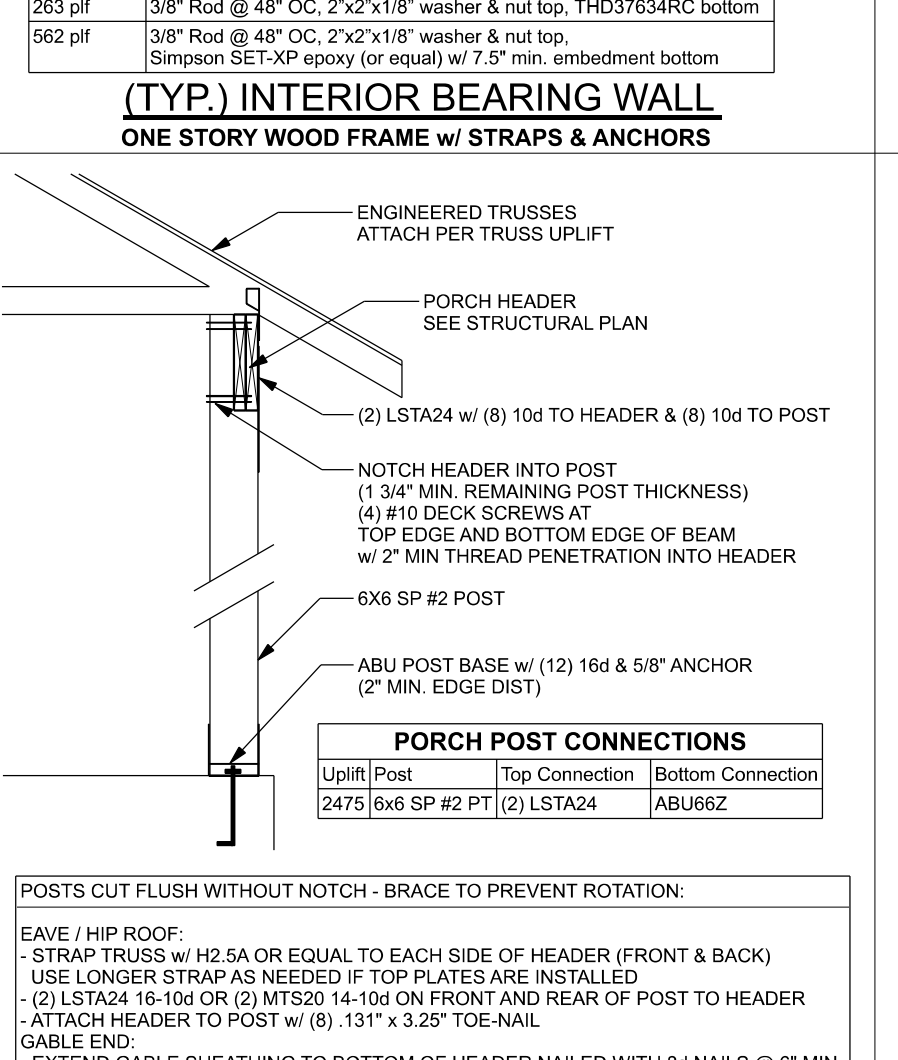
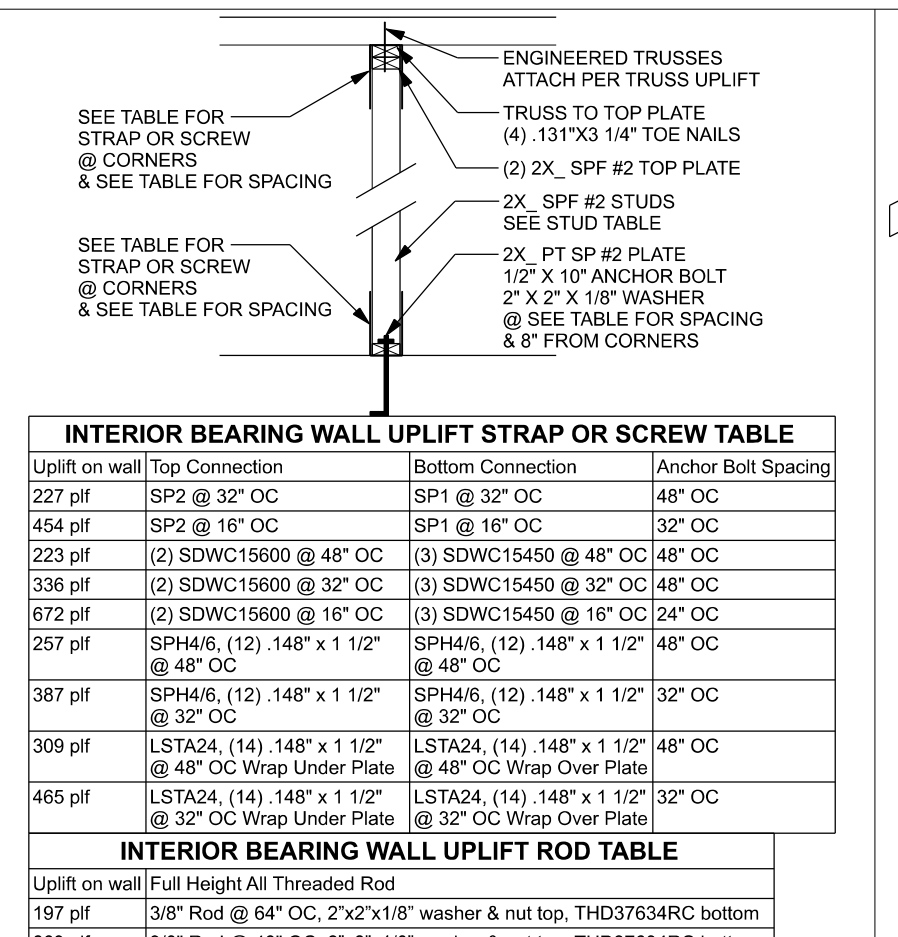
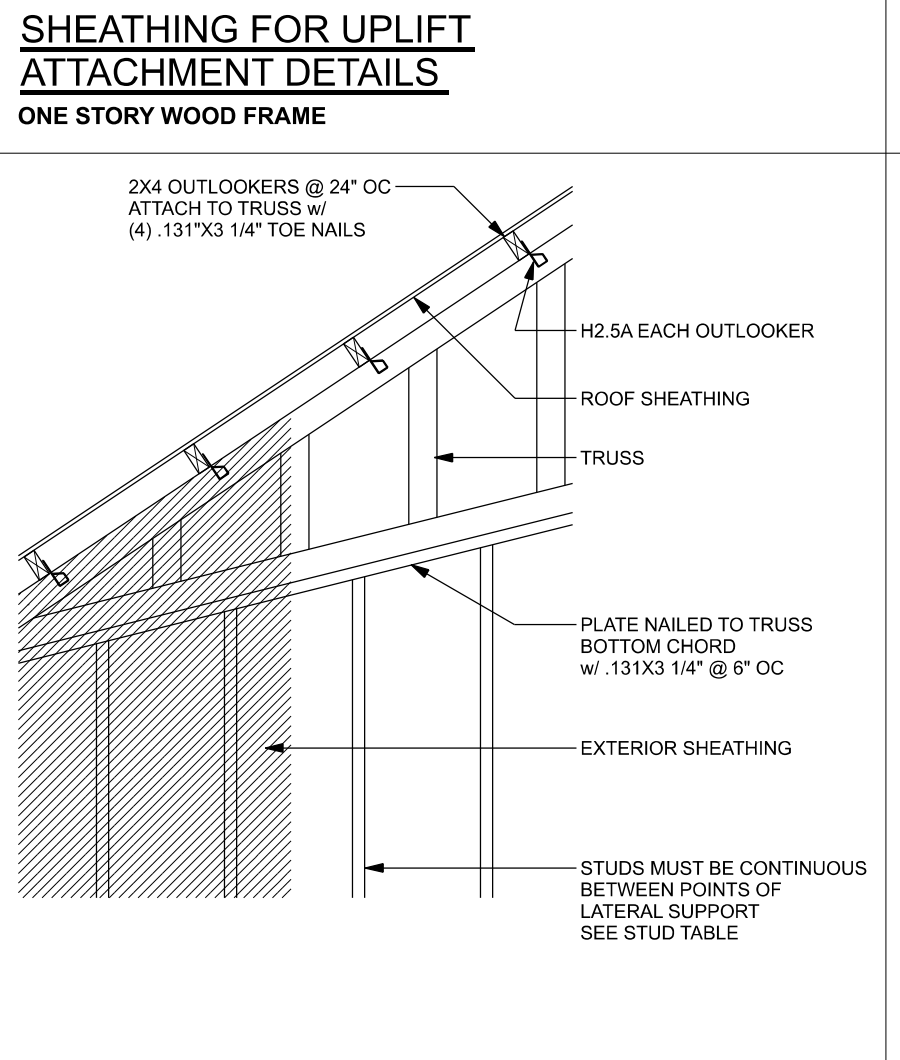
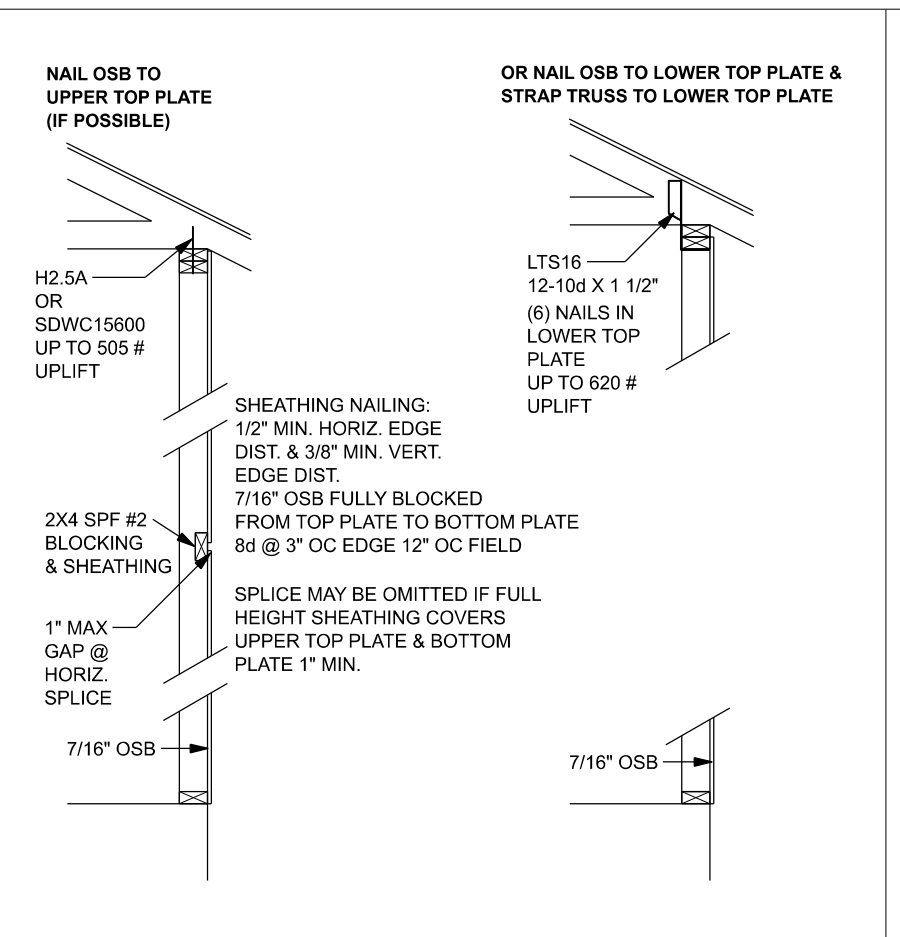


ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2.38" x 0.131")	6" oc	6" oc
120 mph Exp. D	13/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.131")	6" oc	6" oc
130 mph Exp. C	13/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2.38" x 0.131")	6" oc	6" oc
140 mph Exp. D	13/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	13/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	13/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based on the type of roofing material being used. See manufacturer Florida product approval.



CONNECTOR TABLE

Uplift SP	Uplift SPF	Uplift SPC	Uplift SPS	To Plate	To Truss/Rafter
805	505	SDWC15600		-	-
400	290	H3		4-131"x1 1/2"	4-131"x1 1/2"
625	540	H2.5A		6-131"x1 1/2"	5-131"x1 1/2"
1040	1015	H10A		9-148"x1 1/2"	9-148"x1 1/2"
645	515	LTS12-30		6-148"x1 1/2"	6-148"x1 1/2"
690	860	HTS12-30		7-148"x1 1/2"	7-148"x1 1/2"
1415	1215	HTS16-30		8-148"x1 1/2"	8-148"x1 1/2"
		HTT4		To One Member	To Other Member
1235	1235	LSTA21		8-148"x1 1/2"	8-148"x1 1/2"
1640	1460	MSTA24		9-148"x1 1/2"	9-148"x1 1/2"
1030	1030	CS20		7-148"x1 1/2"	7-148"x1 1/2"
555	535	SP1		4-148"x3"	4-148"x3"
1010	605	SP2		6-148"x3"	6-148"x3"
1280	1100	SP4H6		12-148"x1 1/2"	wrap under or over plate
771	771	LSTA24		10-148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24		14-148"x1 1/2"	wrap under or over plate
Uplift SP Uplift SPC	Uplift SPS	Uplift SPT	Uplift SPU	To Stud	To Plate
1010	605	SP2		6-148"x3"	4-148"x3"
2145	3640	HTT4		8-SDS 1/4"x1 1/2"	12"x12" Titen HD
2145	3640	HTT4		18-162"x3 1/2"	12"x12" Titen HD
4235	3640	HTT4		18-162"x3 1/2"	12"x12" Titen HD
Uplift SP Uplift SPC	Uplift SPS	Uplift SPT	Uplift SPU	To Post	To Anchor
1900	1900	ABU42		12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	2475	ABU62		12-162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP Uplift SPC	Uplift SPS	Uplift SPT	Uplift SPU	To Stud	To Anchor
1050	1050	ABU42		12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	2475	ABU62		12-162"x3 1/2"	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXEMPT 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-11" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

2x8	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES, TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS. TRUSS DESIGN, TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS CONNECTIONS UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF THE TRUSS CONNECTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 70LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 150 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4 FTB @ 8X8. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A1816. LOCATED IN MIDDLE OF THE SLAB. SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER INCH OF SLAB THICKNESS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 308. JOINTS SHALL BE MADE IN SLAB PLACEMENT. THE LENGTH WITH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10 FT. DO NOT CUT WITH OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT TO CONTROL THE LOCATION OF CRACKS ON A GIVEN LINE).

REBAR: ASTM A615, GRADE 40, DEFORMED BARS. F.Y. = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 318-96, U.O.C.

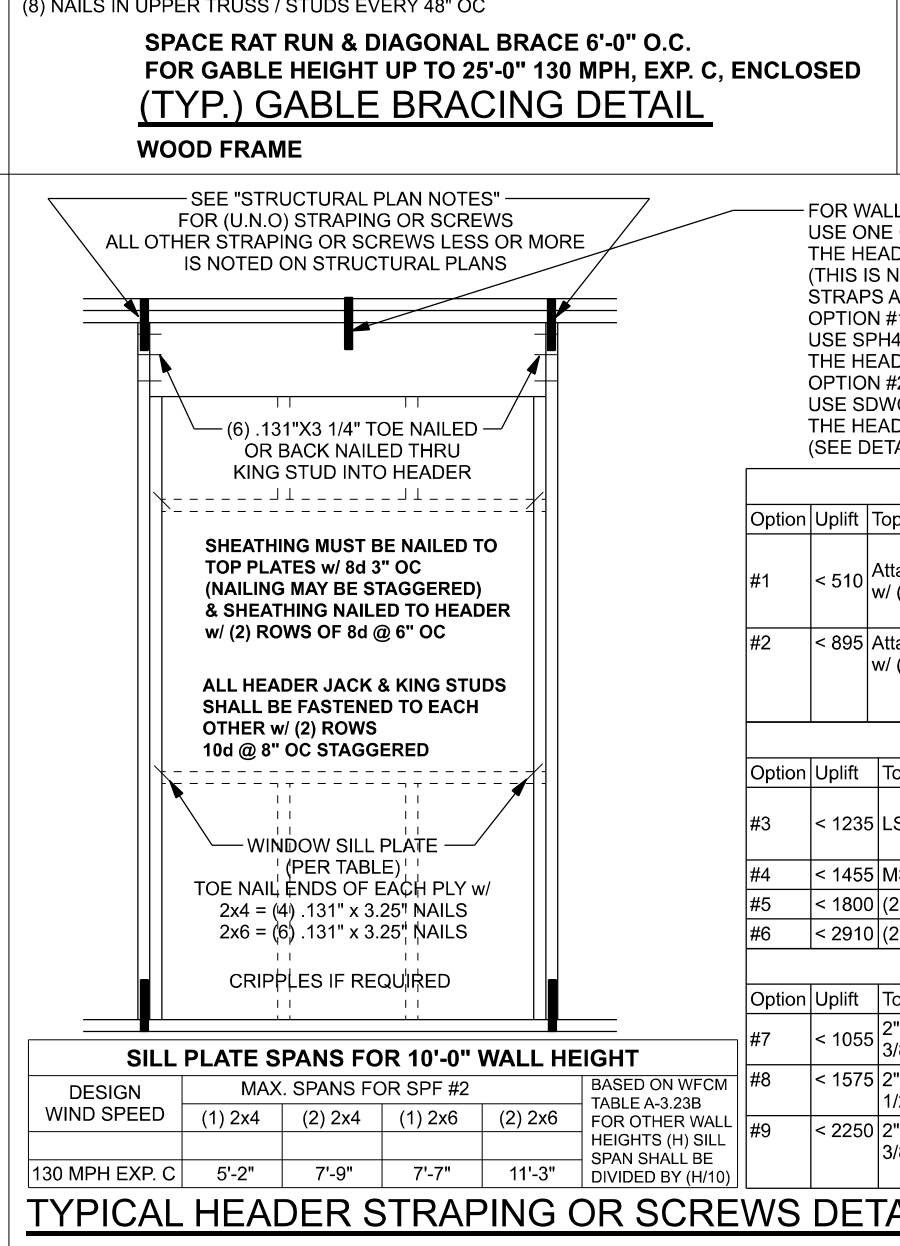
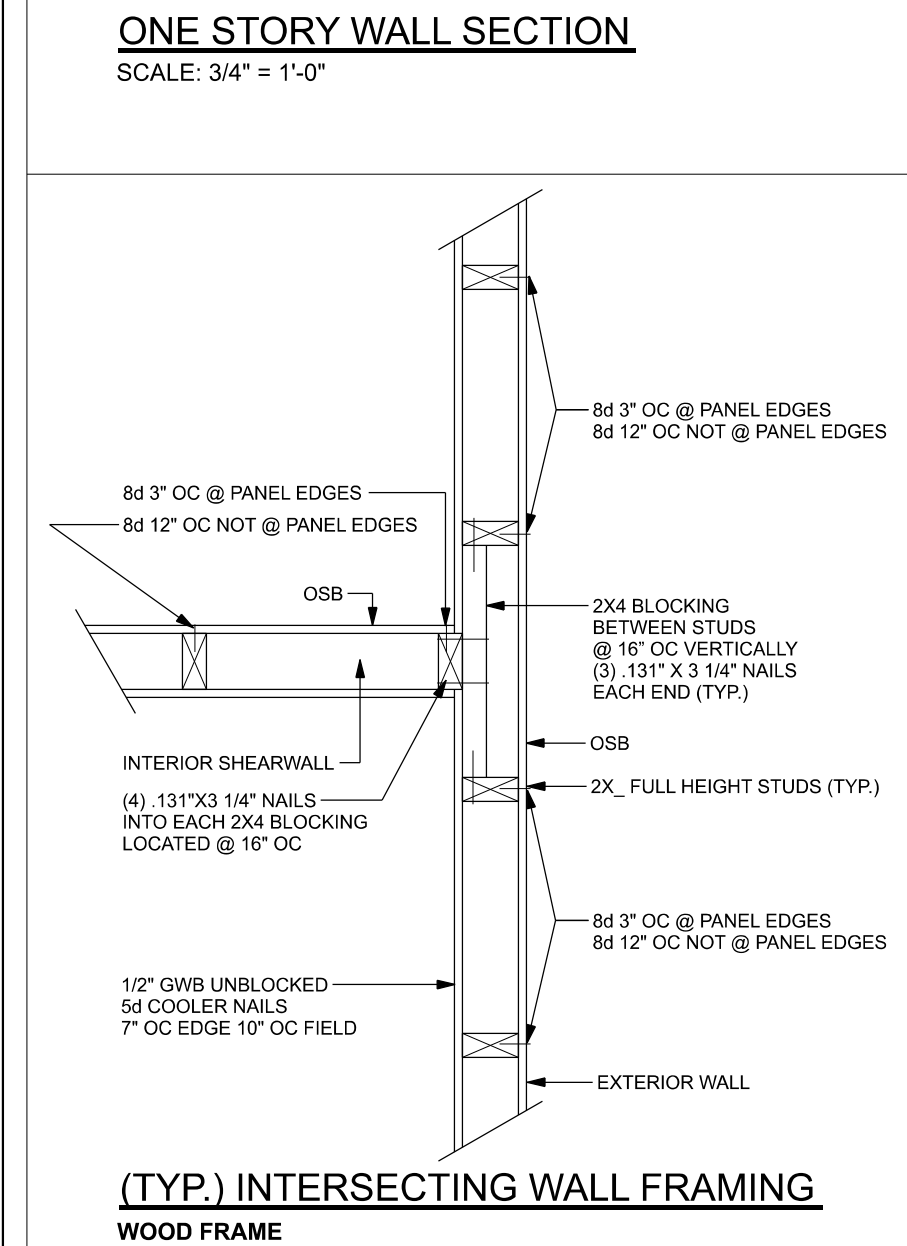
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMISSIONS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN: THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER. THE BUILDER SHOULD USE CARE CARE IN THE PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CARE IN THE DESIGN RESPONSIBILITY FOR THE TRUSS LAYOUT WHICH IS THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



HEADER SCREWS TABLE

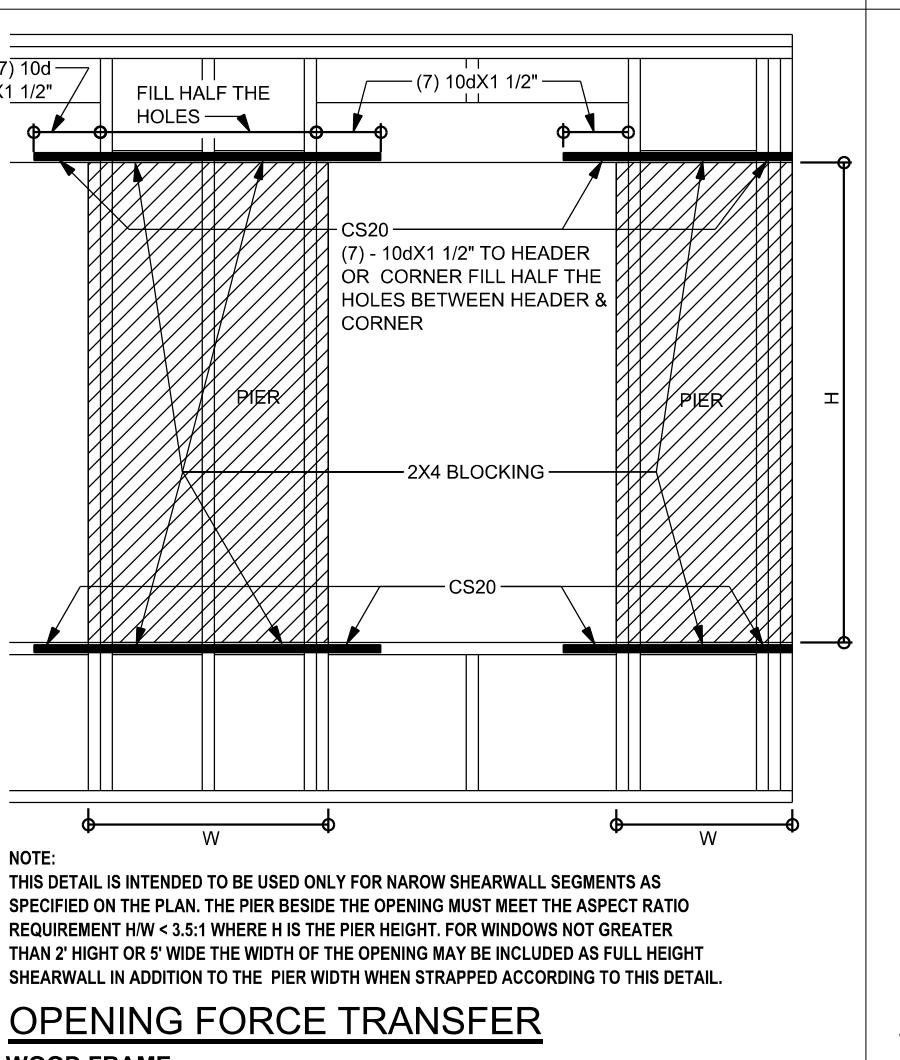
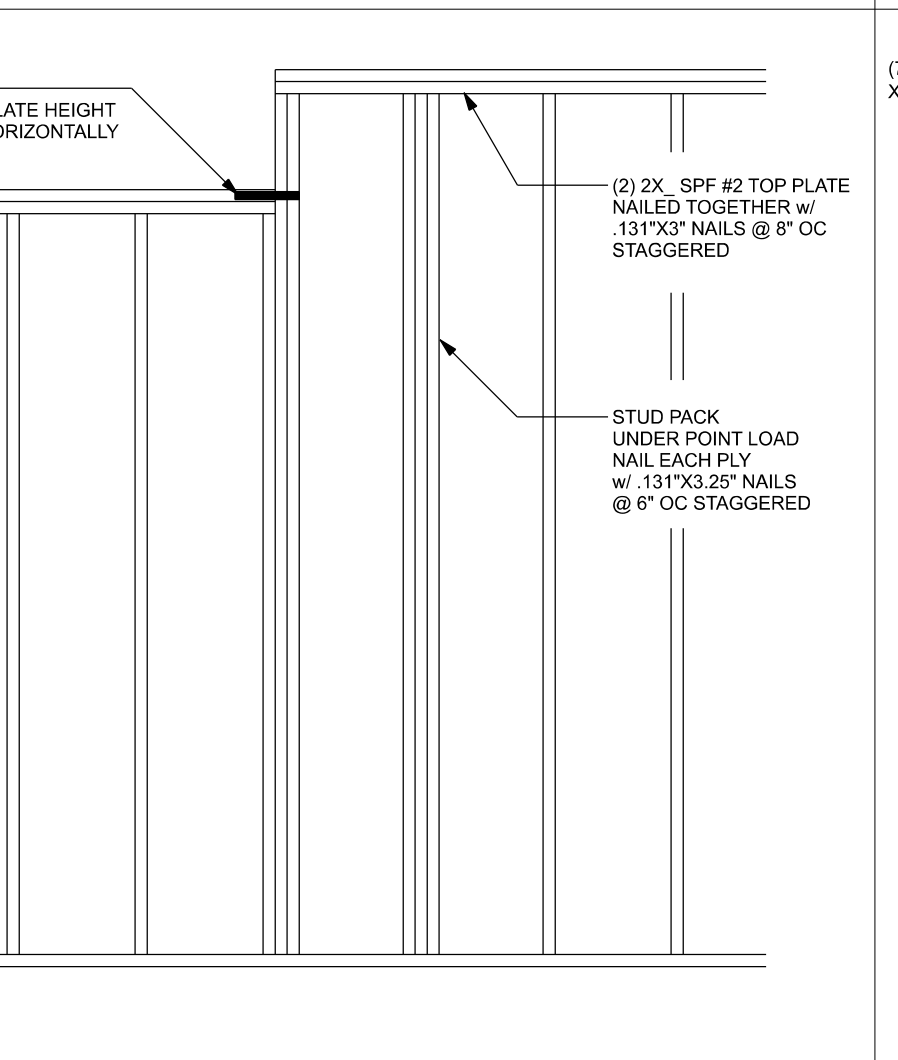
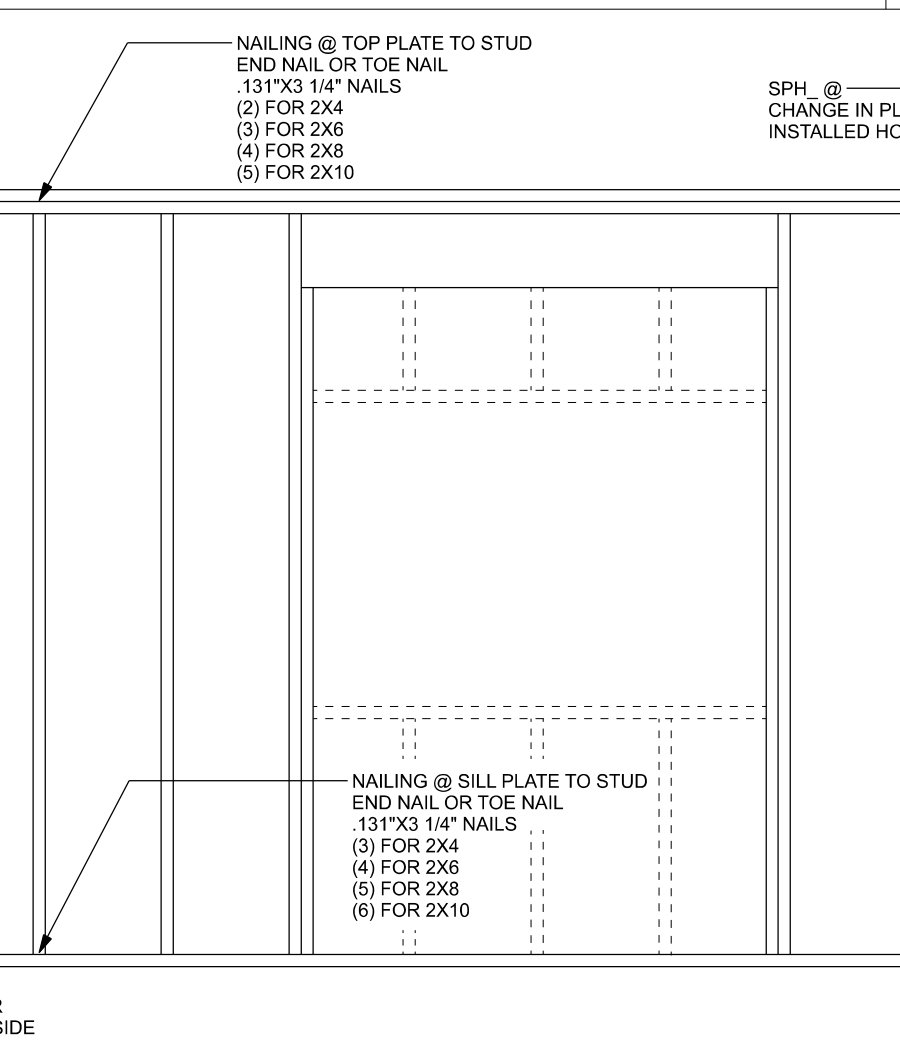
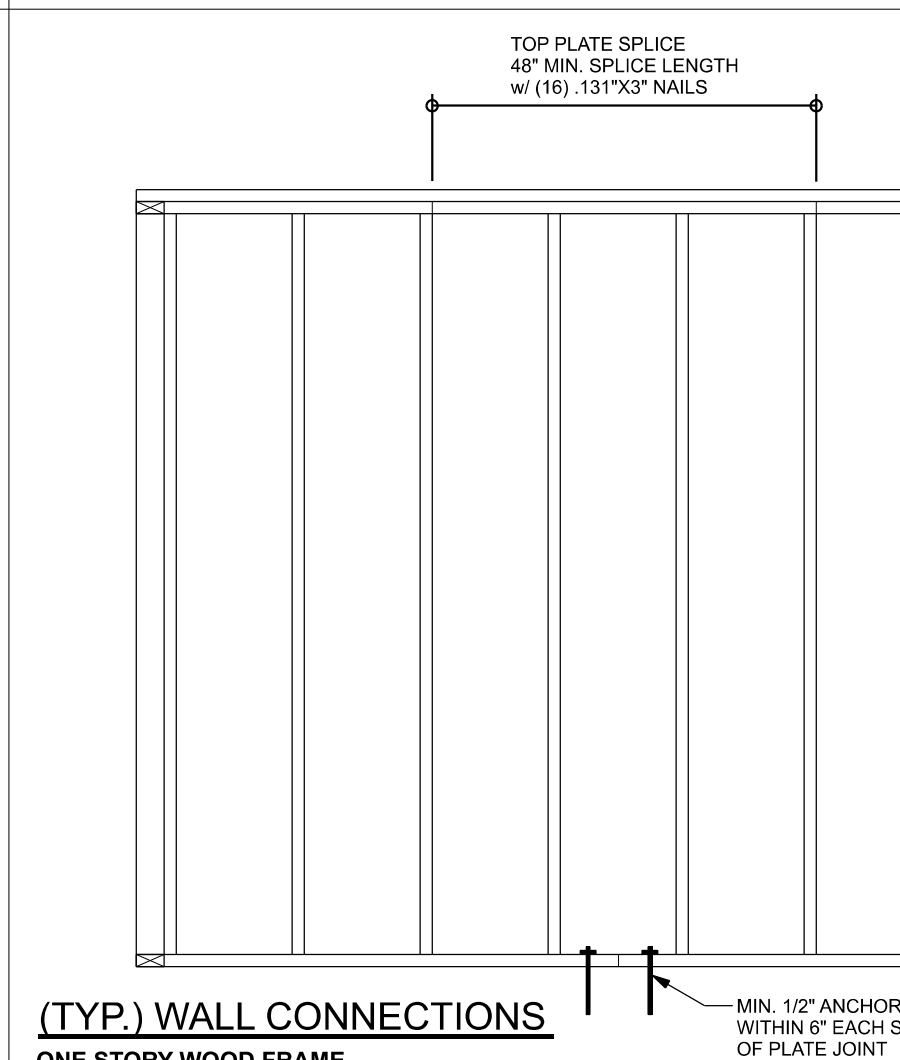
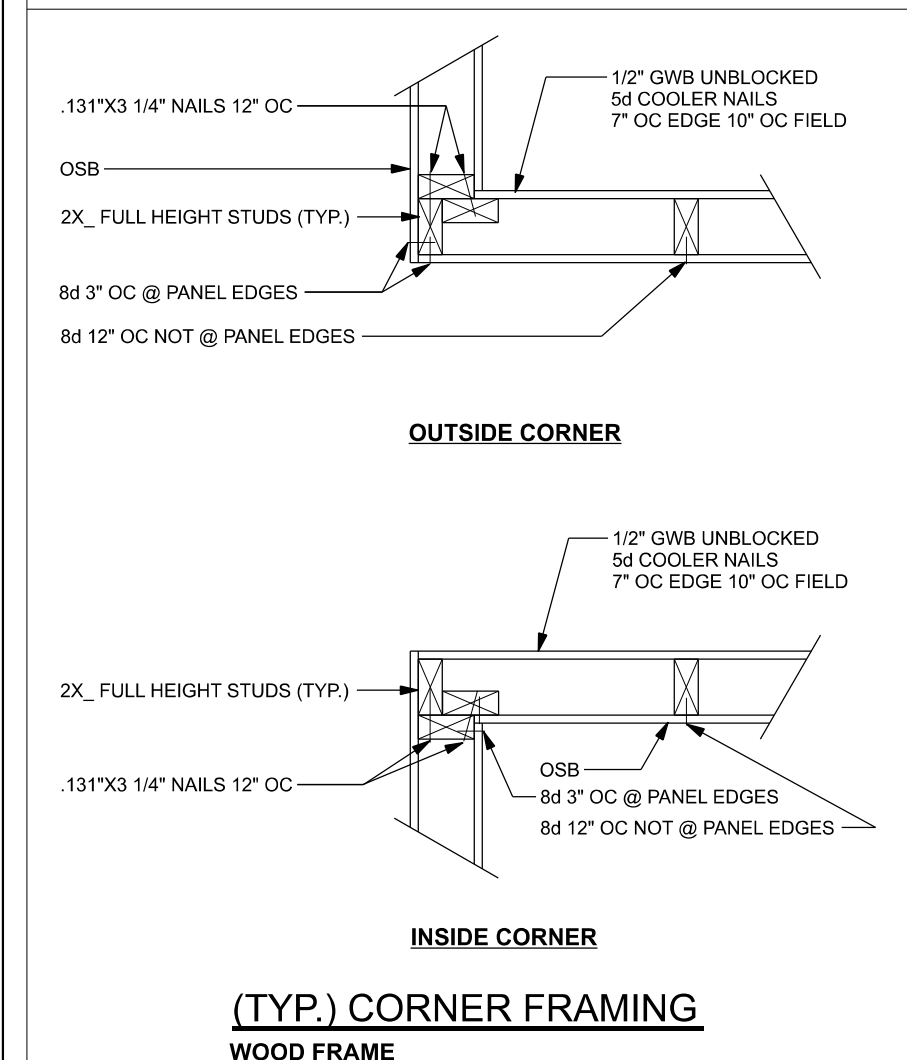
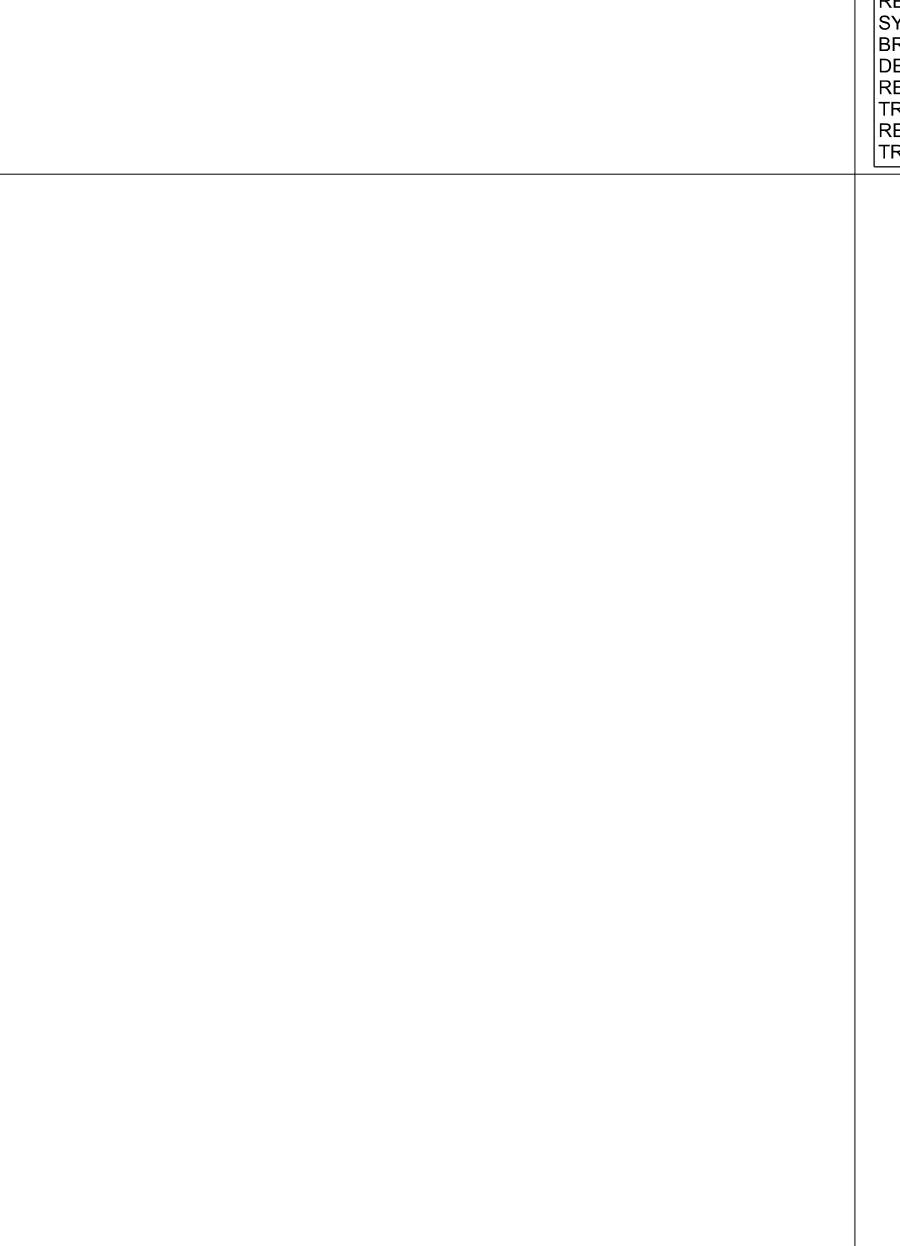
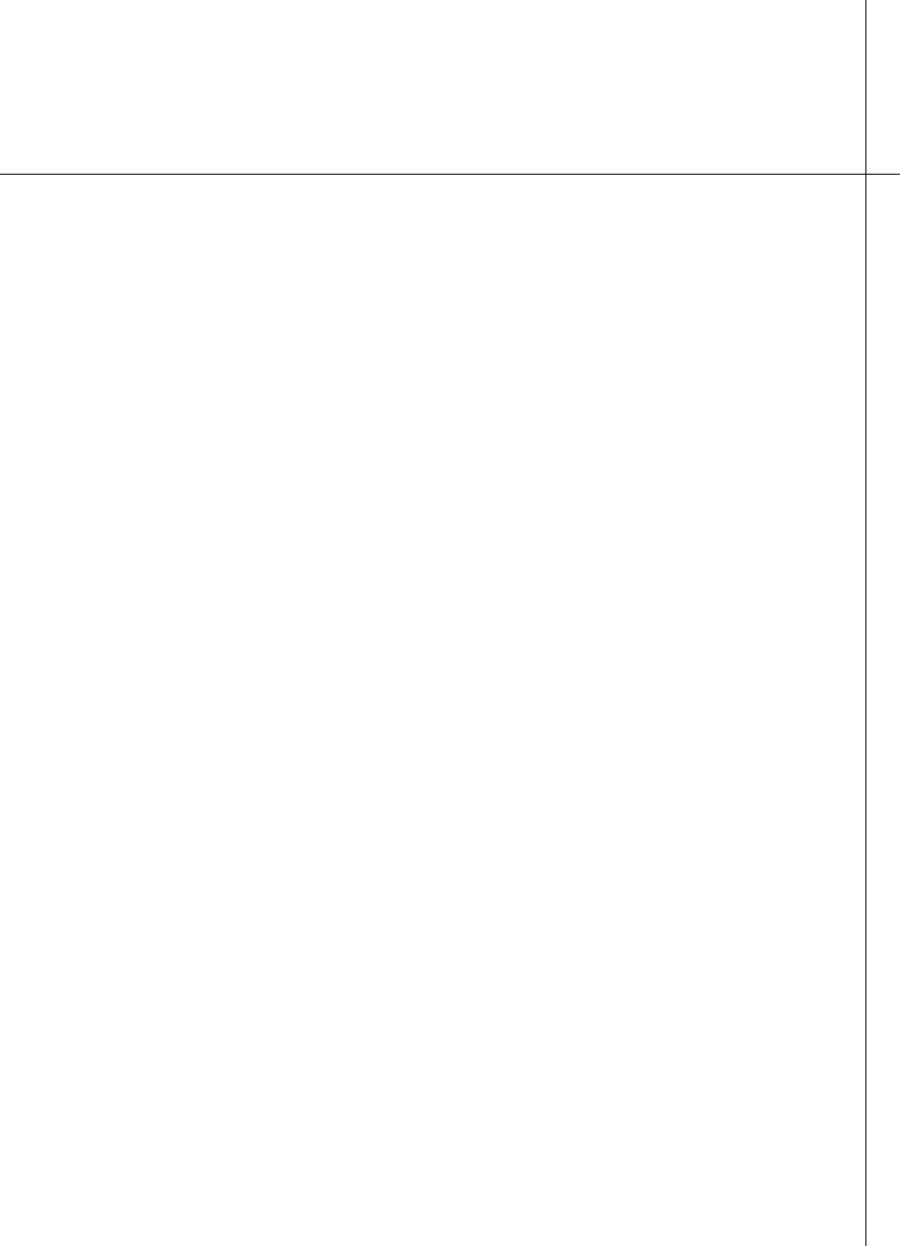
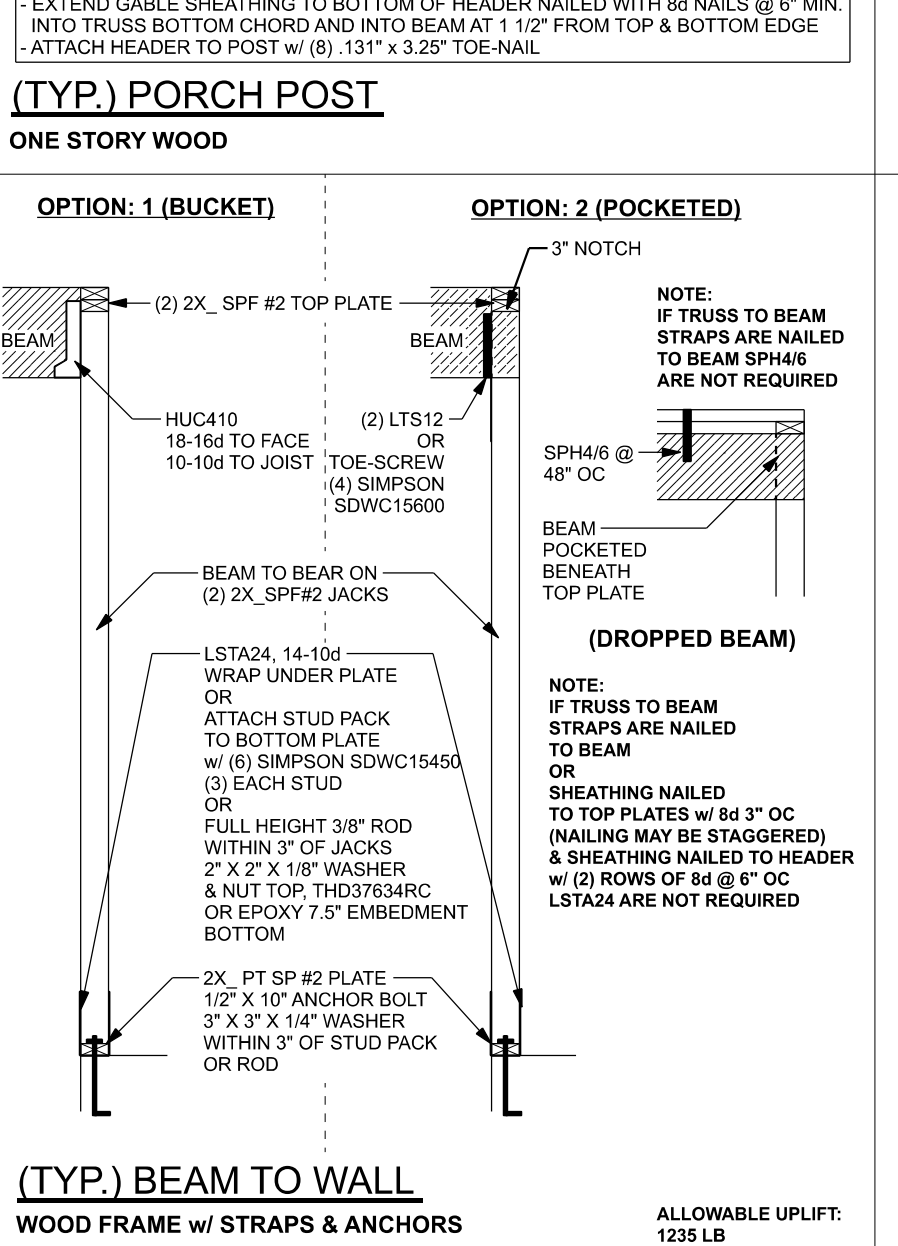
Option	Uplift	Top Connection	Bottom Connection
#1	< 516	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

HEADER STRAP TABLE

Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, (14), 148" x 1 1/2" wrap over plate (PER LABEL)	LSTA24, (14), 148" x 1 1/2" wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4	< 1455	MSTA24, 18-148"x1 1/2" header to jacks	DTT22
#5	< 1800	(2) MST24, 18-148"x1 1/2" header to jacks	DTT22
#6	< 2910	(2) MST24, 18-148"x1 1/2" header to jacks	HTT4

HEADER FULL HEIGHT ROD TABLE

Option	Uplift	Top Connection	Bottom Connection
#7	< 1055	2" x 2" x 1/8" washer & nut @ top plate Simpson THD50834PC 3/8" full height A307 all threaded rod	3/8" full height A307 all threaded rod
#8	< 1575	2" x 2" x 1/8" washer & nut @ top plate 1/2" full height A307 all threaded rod	1/2" full height A307 all threaded rod
#9	< 2250	2" x 2" x 1/8" washer & nut @ top plate Simpson SET-XP epoxy (or equal) w/ 7.5" min. embedment (install per mfg. specs) 3/8" full height A307 all threaded rod	3/8" full height A307 all threaded rod



DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR	1
COEFFICIENT	0.8
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&S DESIGN PRESSURES: SEE TABLE	
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY 1500 PSF	
FLOOD ZONE THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR	ZONE 5 END & FROM ALL OUTSIDE CORNERS
0 - 20	+25.6(Vsust) -27.8(Vsust)	+25.0(Vsust) -34.2(Vsust)
0 - 20	+42.6(Vsust) -46.2(Vsust)	+42.6(Vsust) -57(Vsust)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	16x7 GARAGE DOOR
+22.6(Vsust) -25.5(Vsust)	+21.7(Vsust) -24.1(Vsust)

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midway Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER: 250971
S-1
OF 3 SHEETS

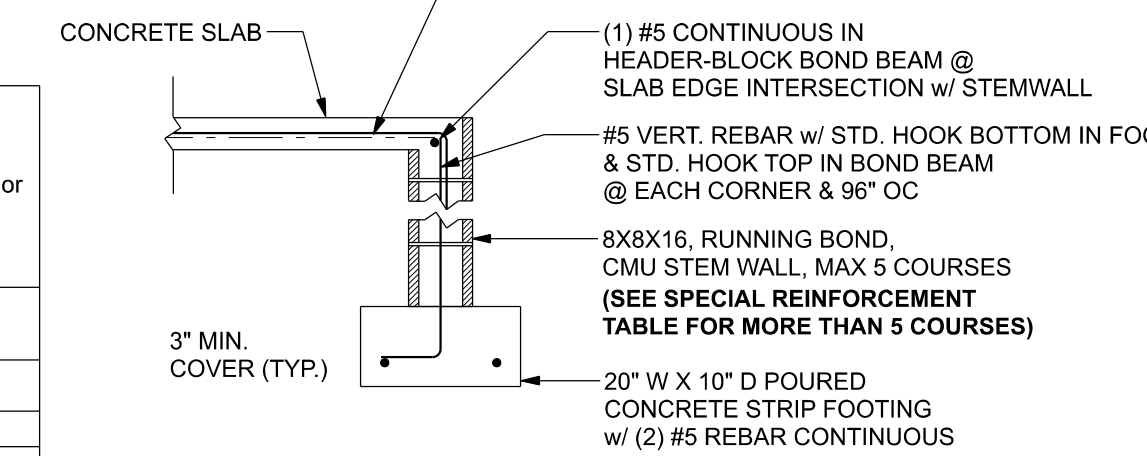
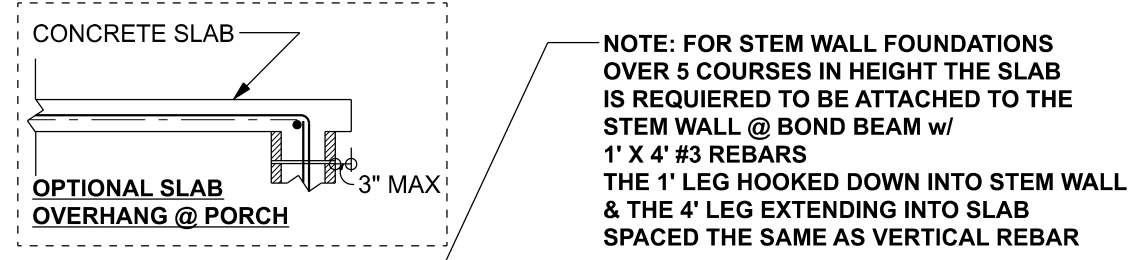
Amira Builders, Inc.
Curasco Res.
PROJECT ADDRESS: 316 SW Cypresswood Glen, Lake City, FL

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

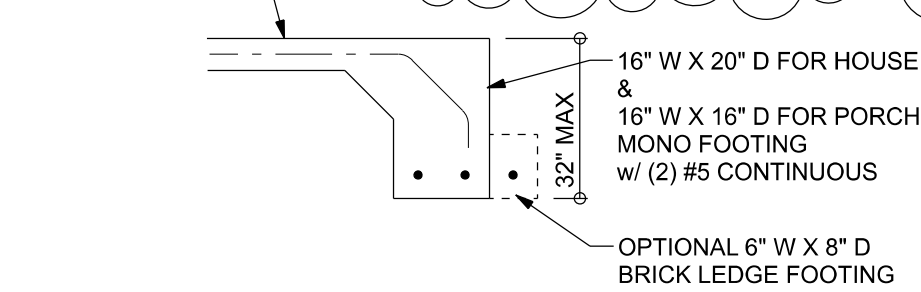
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, $F_y = 40$ ksi, Lip, silicon min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal less completely embedded in mortar or grout, ASTM A525, Class 600, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal less not completely embedded in mortar or grout, ASTM A153, Class 62, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

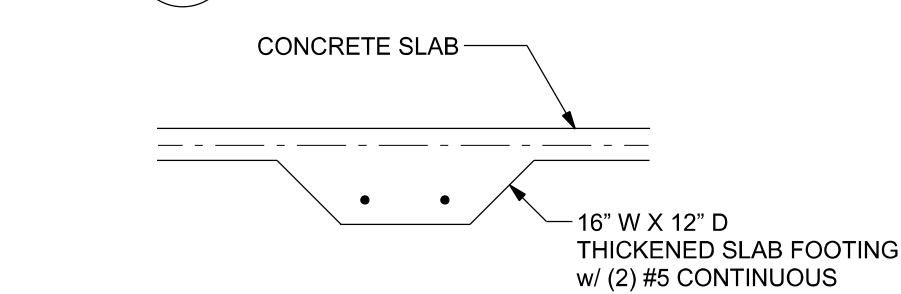


F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

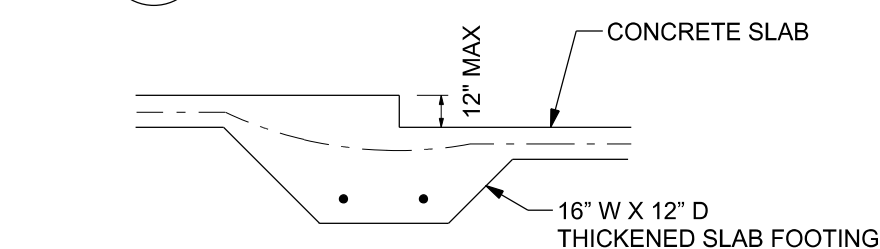
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



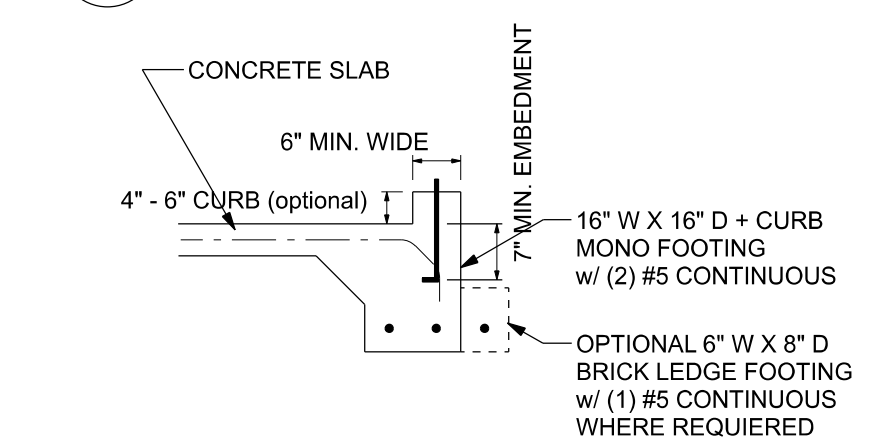
F1 OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



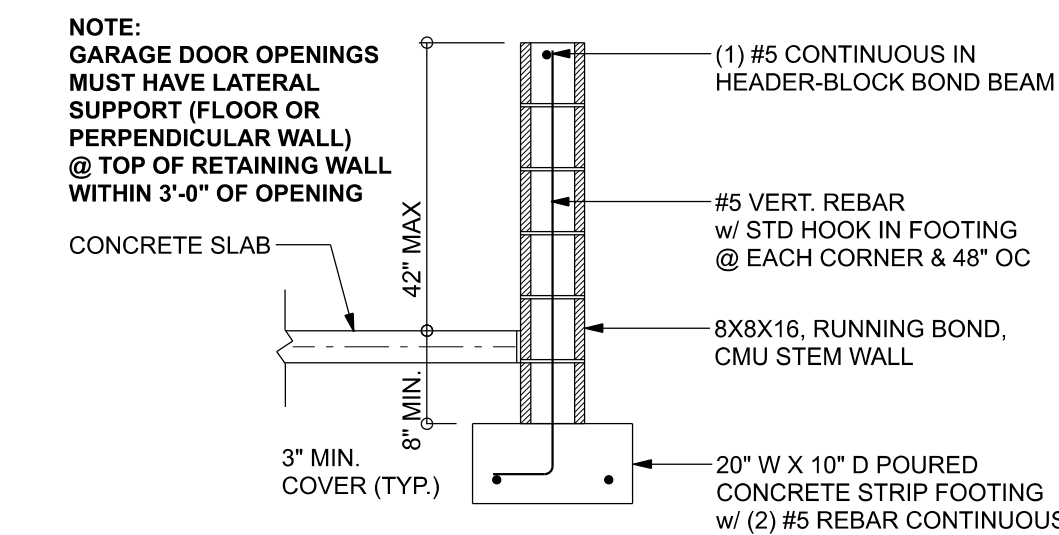
F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



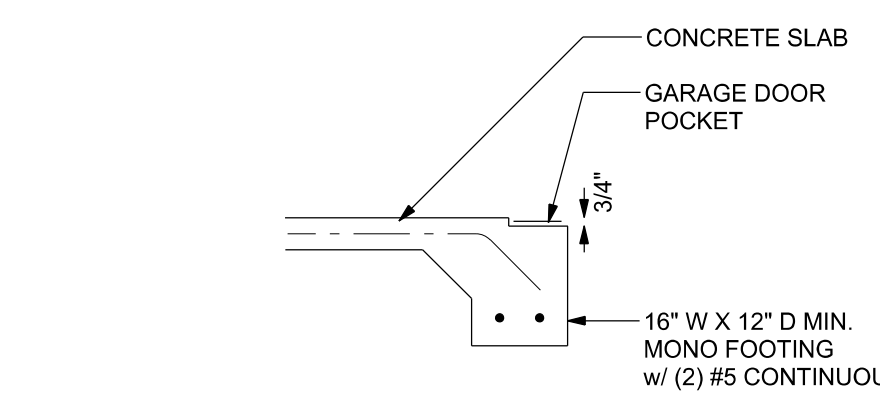
F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



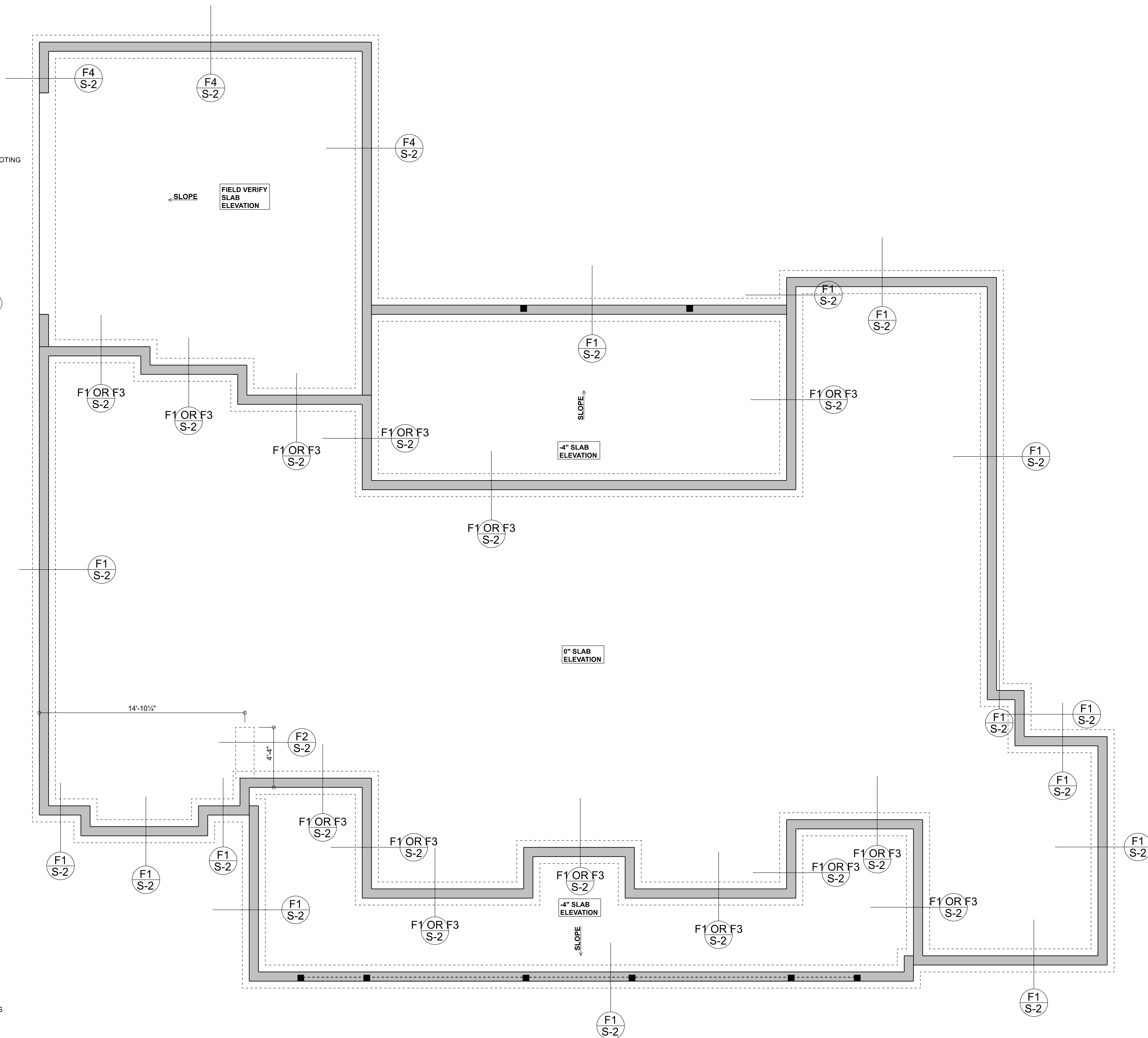
F4 OPTIONAL MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F4 STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

- FOUNDATION NOTES**
- FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
 - CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
 - FN-3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6x6-1/4" 4 WELDED WIRE MESH PLACED ON CHAIRS @ 12" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).

Amira Builders, Inc.
Curasco Res.
PROJECT ADDRESS:
316 SW Cypresswood Glen, Lake City, FL

FL PE 53915
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

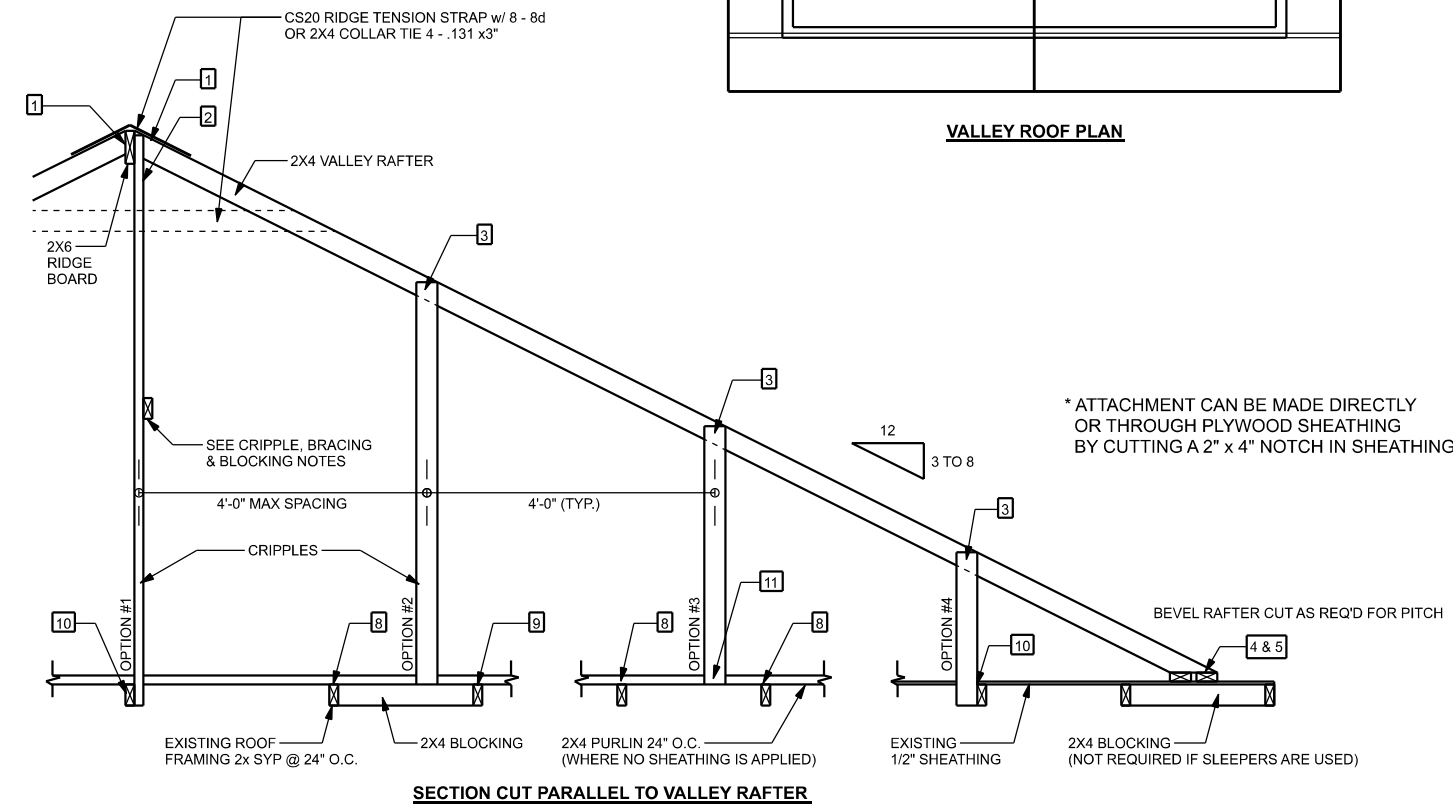
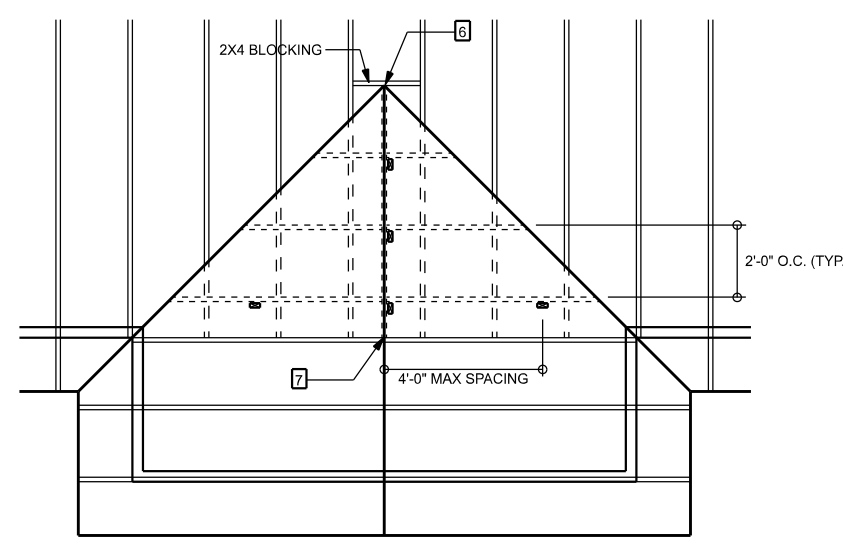
LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
250971

S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
RISE BOARD	2X4 SYP #2
RAFTER SPANS 20'0" OR LESS	2X4 SYP #2
PURLINS (LATERAL BRACING)	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SYP #2 OR 2 INVALES, 2X4 SYP #2
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL

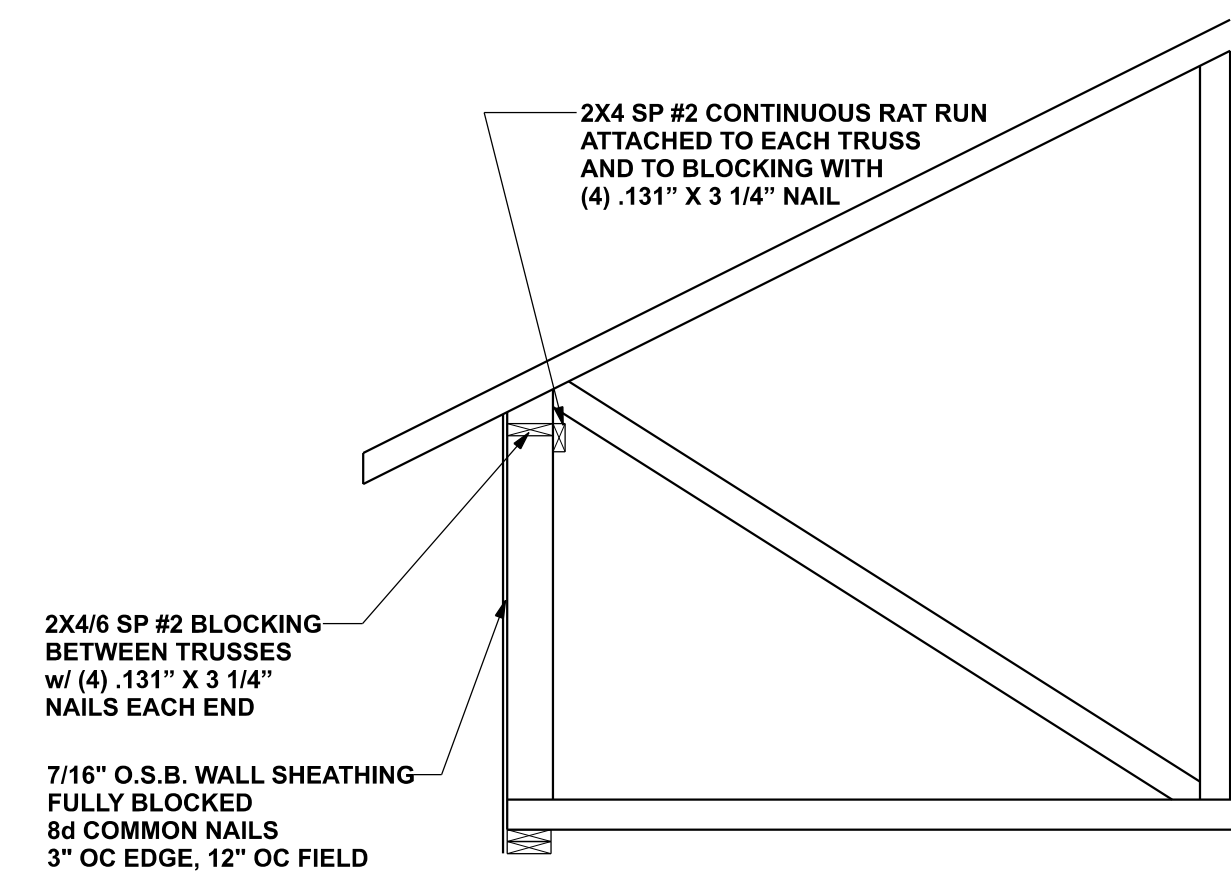


VALLEY ROOF PLAN MEMBER LEGEND

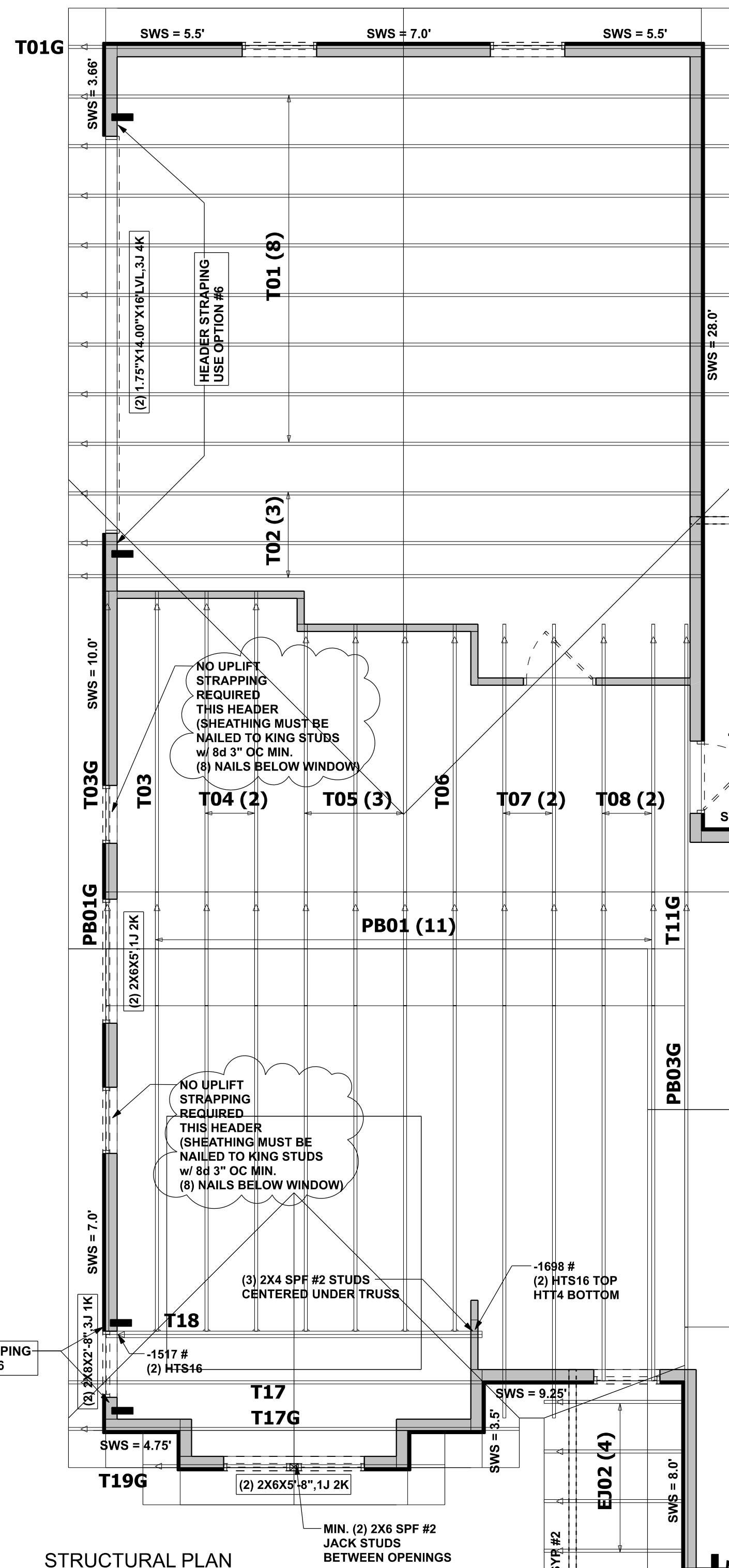
- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE
- CRIPPLES 4" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES	
1. 2X4 RAFTERS TO RIDGE	4 - 131 x 3" TOE NAILS
2. CRIPPLE TO ROOF	4 - 131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	4 - 131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	4 - 131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4 - 131 x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO ROOF BLOCK	4 - 131 x 3" FACE NAILS
7. RIDGE BOARD TO TRUSS	4 - 131 x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP)	4 - 131 x 3" NAILS
9. PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	6 - 131 x 3" NAILS
10. TRUSS TO BLOCKING	4 - 131 x 3" END NAILS
11. CRIPPLE TO TRUSS	4 - 131 x 3" FACE NAILS
12. CRIPPLE TO PURLIN	4 - 131 x 3" FACE NAILS

ROOF OVER FRAMING & BRACING DETAIL
SCALE: 1/2" = 1'-0"



DETAIL @ TRUSSES WITH RAISED HEELS
SCALE: 1/2" = 1'-0"



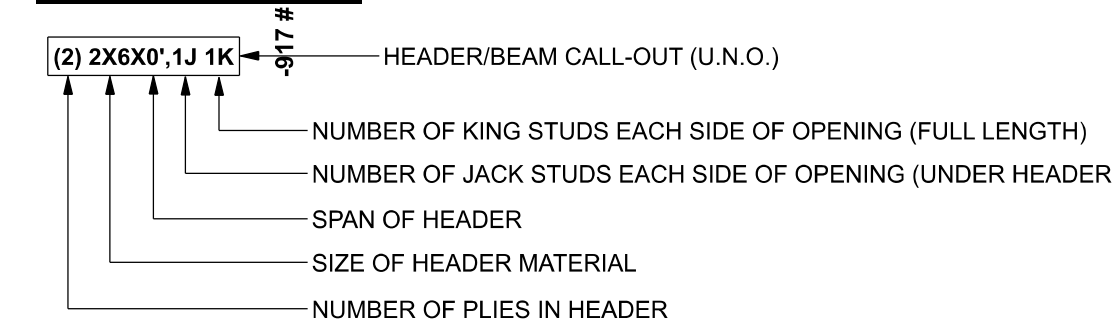
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE
- SN-2

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	3116 LBF	19557 LBF
REQUIRED	24970 LBF	18365 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #4894417

ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

NOTE: IF THE ABOVE DETAIL IS USED ON THE REAR PORCH WALL THE REAR PORCH CEILING DOES NOT NEED TO BE SHEATHED

NO UPLIFT STRAPPING REQUIRED THIS HEADER (SHEATHING MUST BE NAILED TO KING STUDS w/ 8d 3" OC MIN. (8) NAILS BELOW WINDOW)

NOTE: PORCH CEILING MUST BE SHEATHED w/ MIN. 3/8" OSB w/ 8d @ 3" OC EDGE & 12" OC FIELD OR 5/8" GYPSUM BOARD (FULLY BLOCKED) w/ #6 x 1 1/4" SCREWS OR 8d COOLER NAILS @ 4" OC (UPTO 175PSLF)

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2003) to the best of my knowledge.

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S-3
OF 3 SHEETS