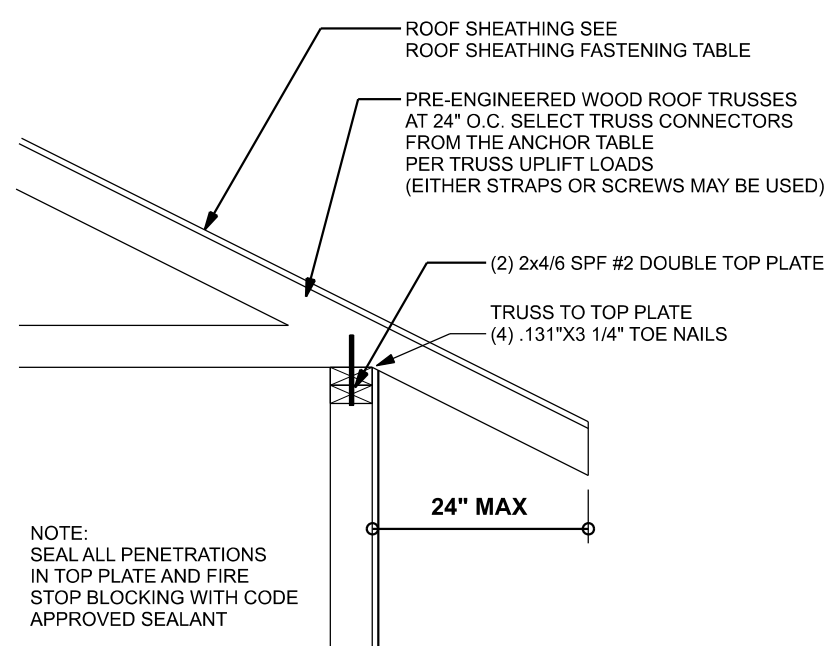
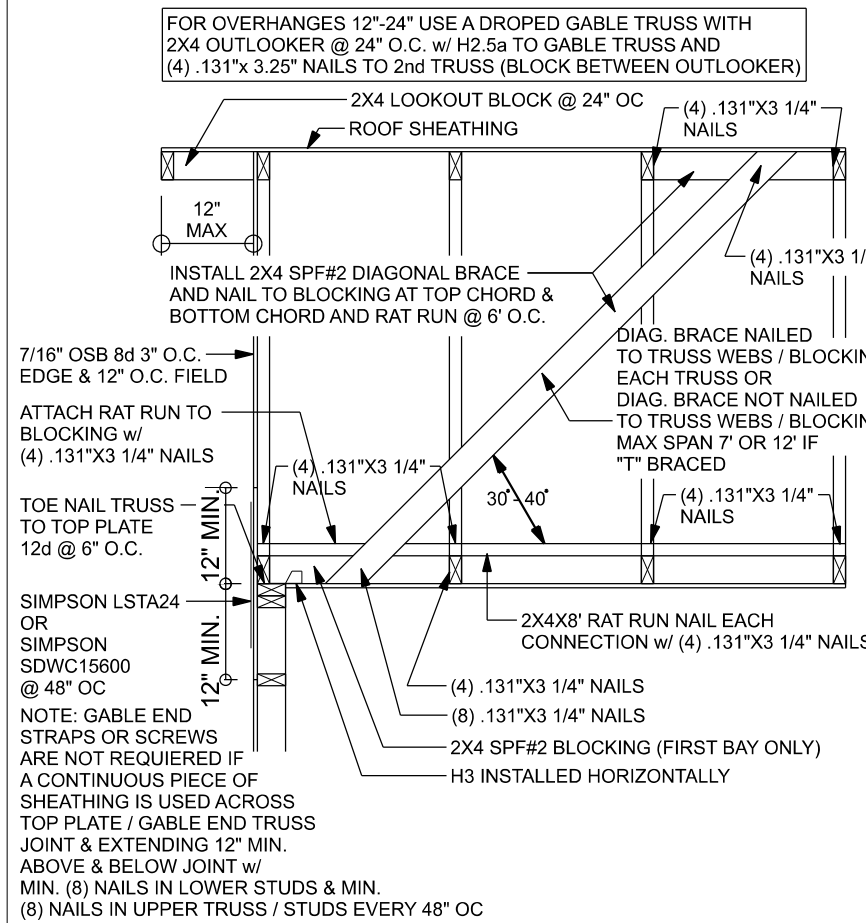
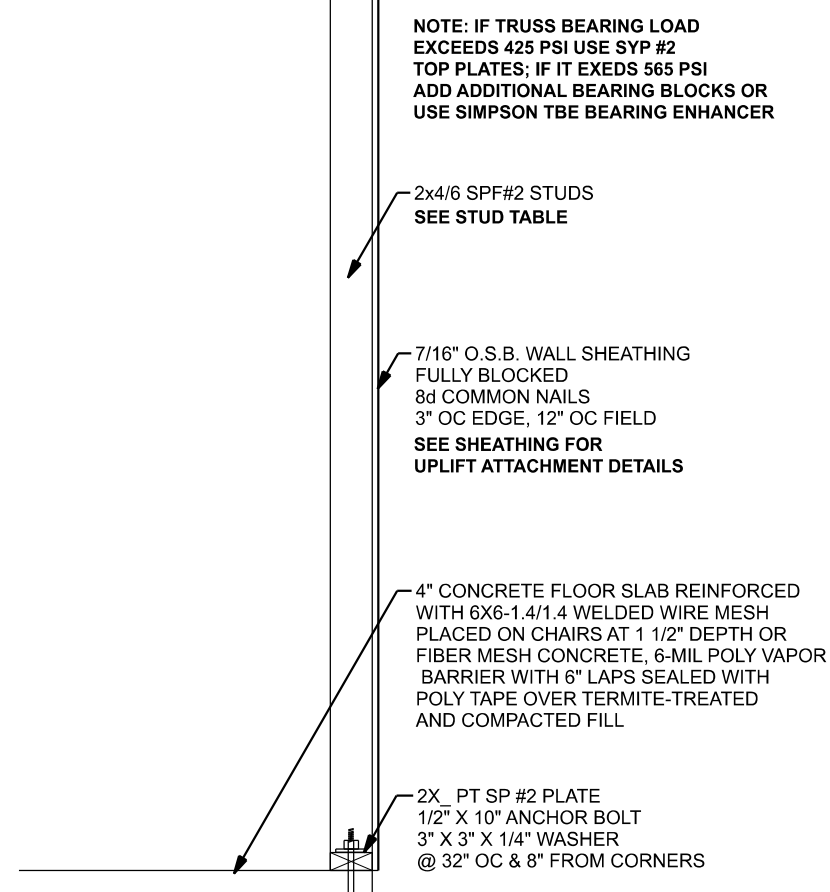


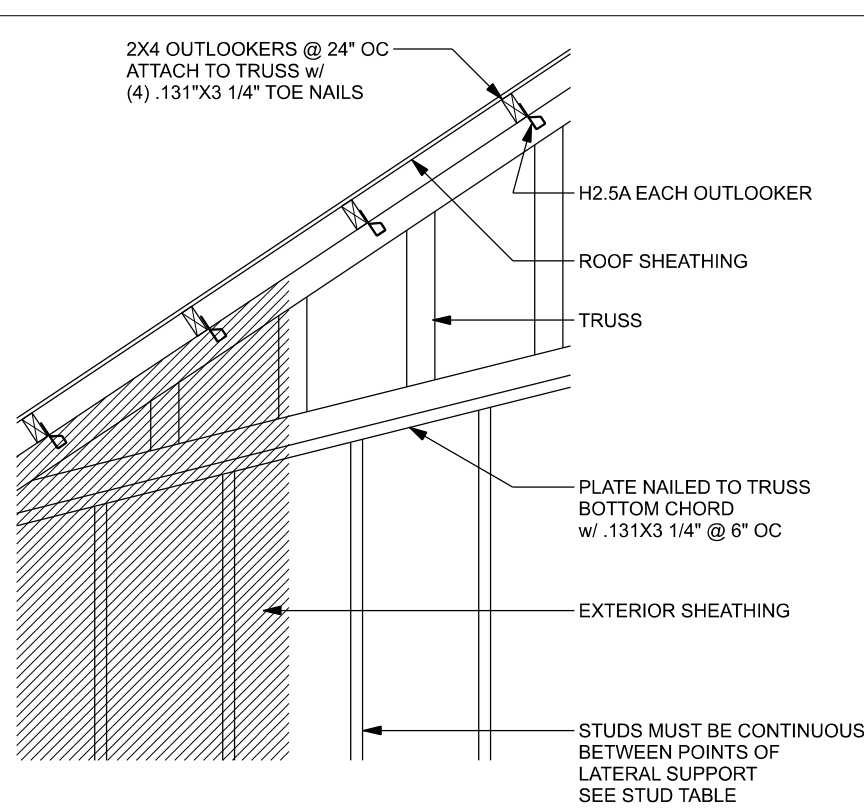
Wind Speed Exp. C	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
130 mph	15/32"	ASTM F1667 RRS-01 (2 3/8" x 6.13")	6" oc	6" oc



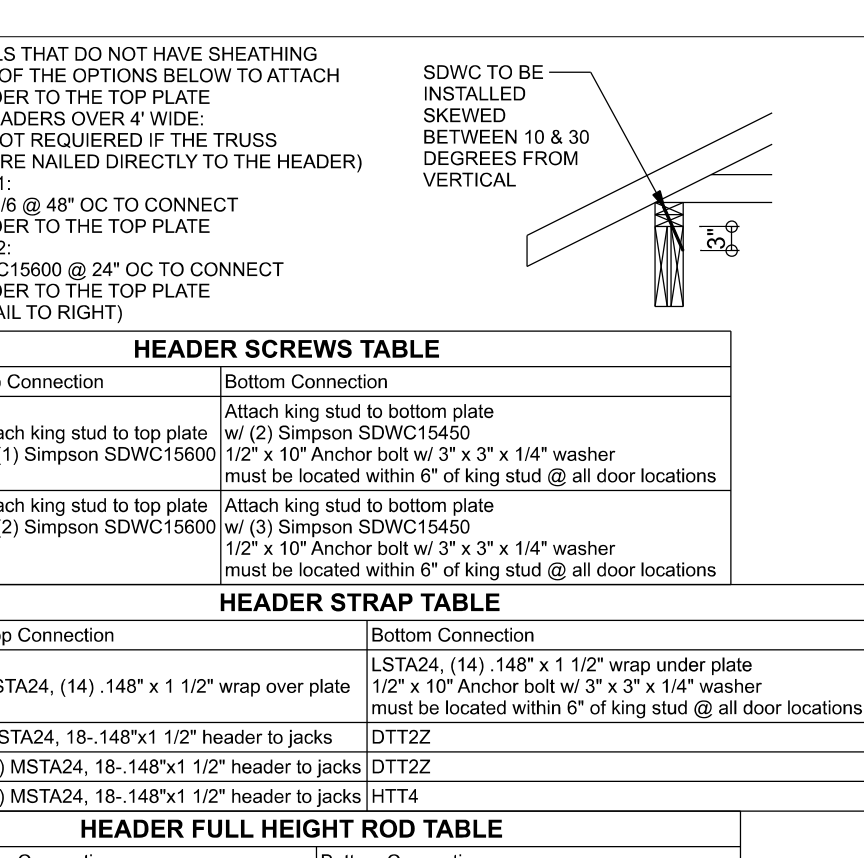
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of edges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based on the type of roofing material being used. See manufacturer Florida product approval.



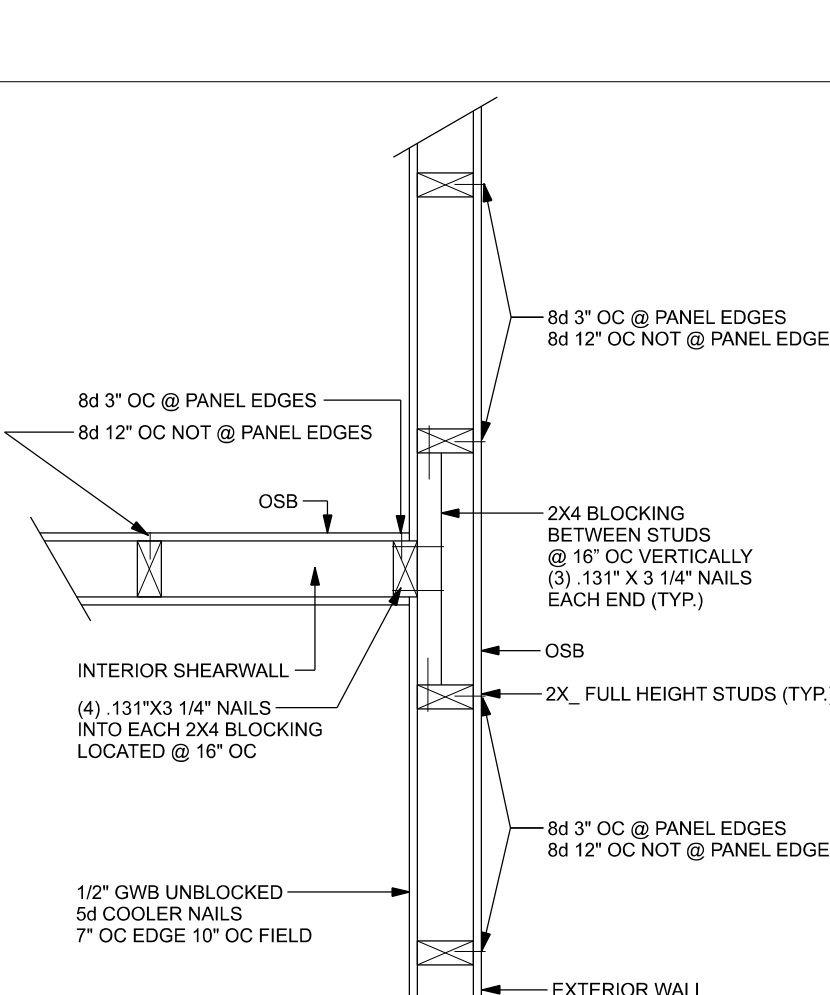
SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



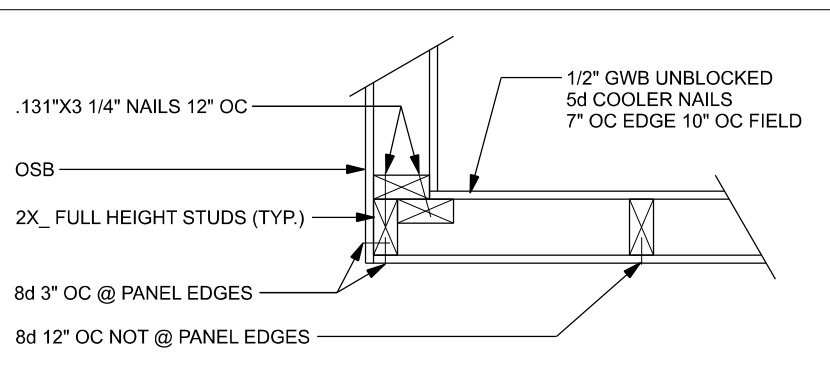
(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME



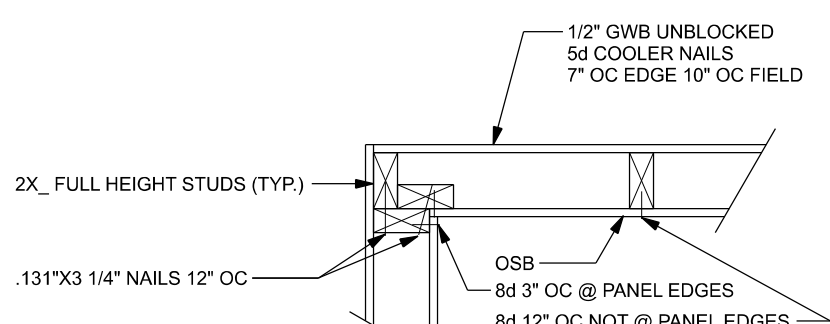
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"



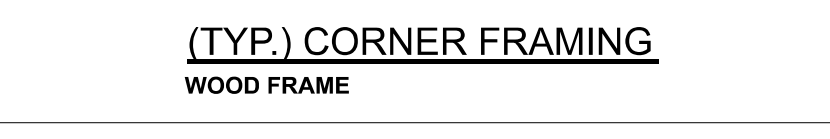
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



OUTSIDE CORNER



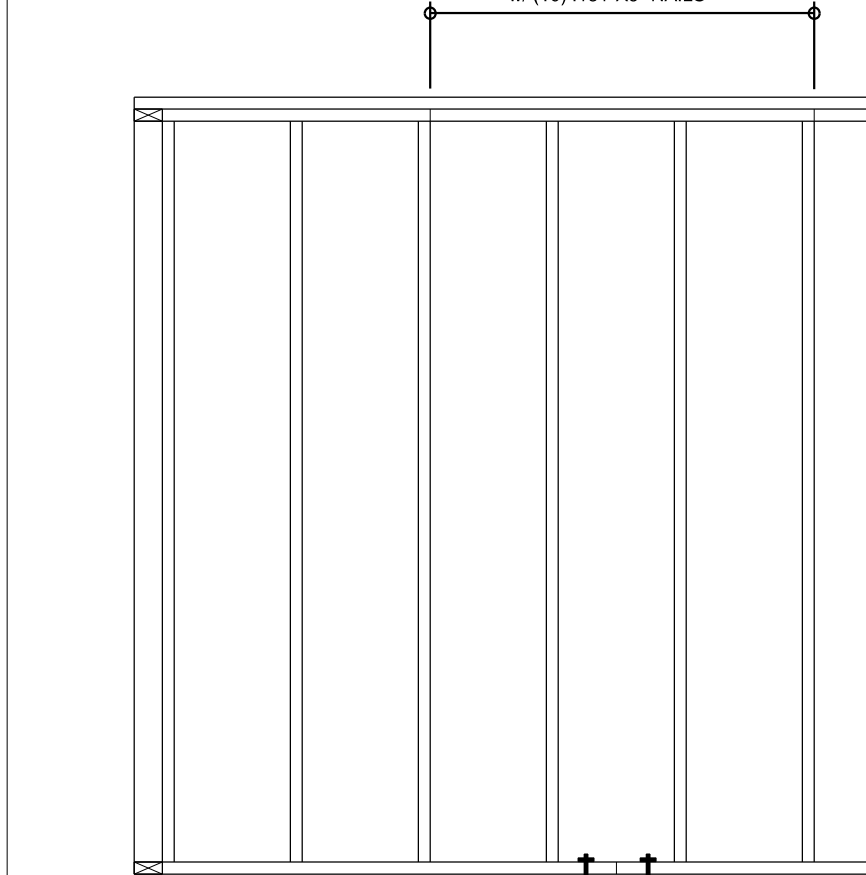
(TYP.) CORNER FRAMING WOOD FRAME



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SFF #2	BASED ON WIND HEADS W/ 239 FOR OTHER WALL HEADS W/ 239 SPAN SHALL BE DIVIDED BY 1/10
130 MPH EXP. C	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6

TYPICAL HEADER STRAPING OR SCREWS DETAIL



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

HEADING SCREWS TABLE

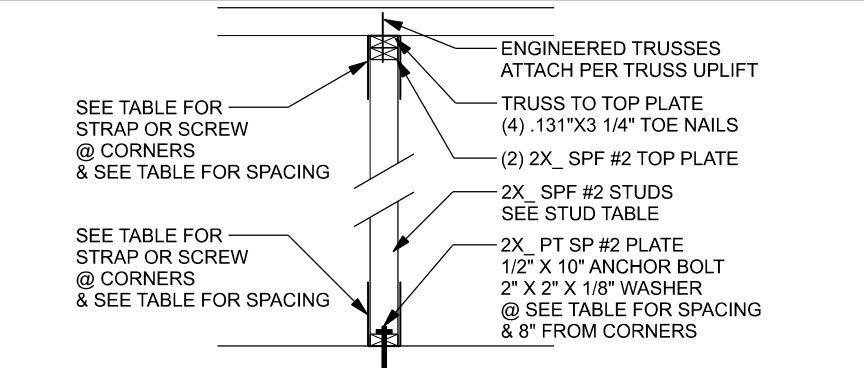
Option	Uplift	Top Connection	Bottom Connection
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	(2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

HEADING STRAP TABLE

Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, (14) 148" x 1 1/2" wrap over plate	1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4	< 1455	MSTA24, 18-148" x 1 1/2" header to jacks	DTT22
#5	< 1800	(2) MSTA24, 18-148" x 1 1/2" header to jacks	DTT22
#6	< 2910	(2) MSTA24, 18-148" x 1 1/2" header to jacks	HTT4

HEADING FULL HEIGHT ROD TABLE

Option	Uplift	Top Connection	Bottom Connection
#7	< 1055	2" x 2" x 1/8" washer & nut @ top plate Simpson THD3763ARC 3/8" full height A307 all threaded rod	3/8" full height A307 all threaded rod
#8	< 1575	2" x 2" x 1/8" washer & nut @ top plate Simpson THD3093ARC 1/2" full height A307 all threaded rod	1/2" full height A307 all threaded rod
#9	< 2250	2" x 2" x 1/8" washer & nut @ top plate Simpson SET-XP epoxy (or equal) 3/8" full height A307 all threaded rod	3/8" full height A307 all threaded rod



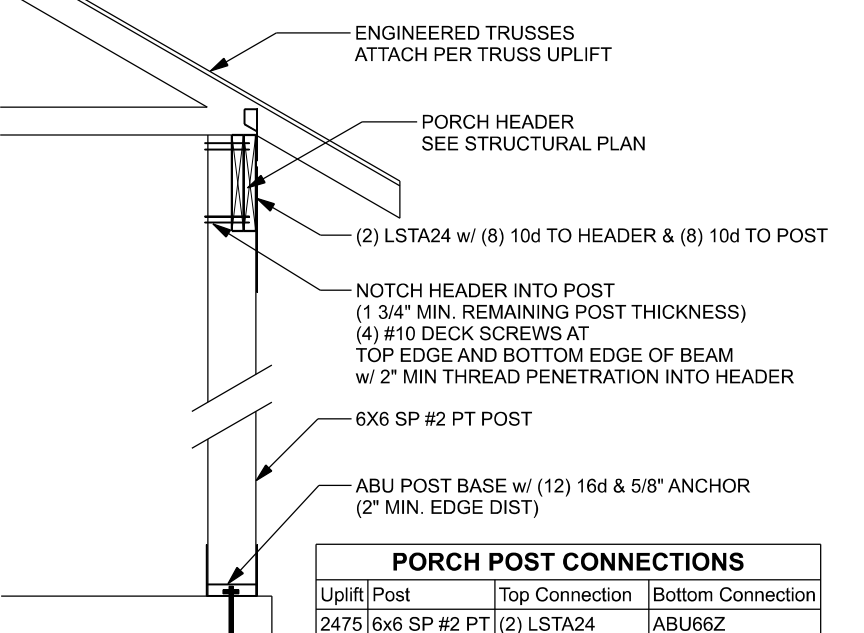
INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE

Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pcf	SP2 @ 32" OC	SP1 @ 16" OC	48" OC
454 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
223 pcf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
336 pcf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
672 pcf	(2) SDWC15600 @ 16" OC	(3) SDWC15450 @ 16" OC	24" OC
257 pcf	SP#46, (12) 148" x 1 1/2" @ 48" OC	SP#46, (12) 148" x 1 1/2" @ 48" OC	48" OC
387 pcf	SP#46, (12) 148" x 1 1/2" @ 32" OC	SP#46, (12) 148" x 1 1/2" @ 32" OC	32" OC
308 pcf	LSTA24, (14) 148" x 1 1/2" @ 48" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 48" OC Wrap Over Plate	48" OC
465 pcf	LSTA24, (14) 148" x 1 1/2" @ 32" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 32" OC Wrap Over Plate	32" OC

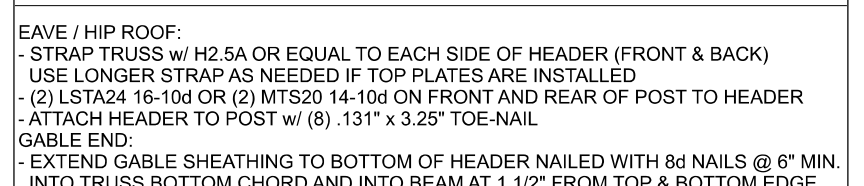
INTERIOR BEARING WALL UPLIFT ROD TABLE

Uplift on wall	Full Height All Threaded Rod
197 pcf	3/8" Rod @ 64" OC, 2" x 2" x 1/8" washer & nut top, THD3763ARC bottom
263 pcf	3/8" Rod @ 48" OC, 2" x 2" x 1/8" washer & nut top, THD3763ARC bottom
662 pcf	3/8" Rod @ 48" OC, 2" x 2" x 1/8" washer & nut top, Simpson SET-XP epoxy (or equal) w/ 7.5" min. embedment bottom

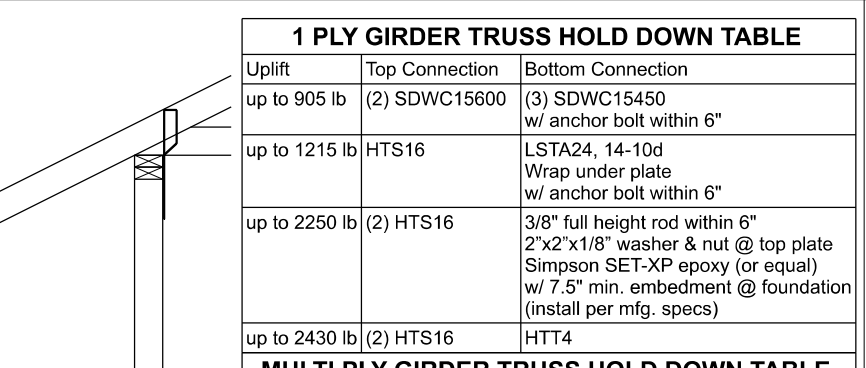
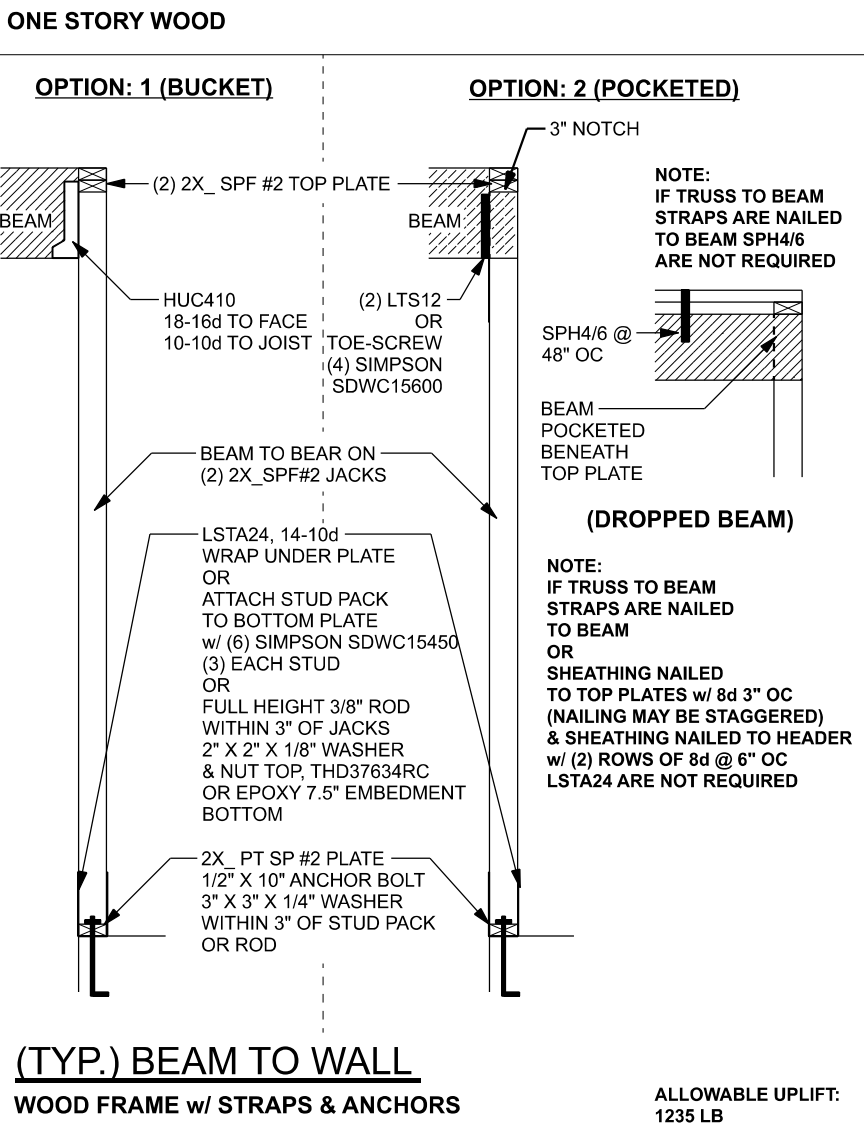
(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST ONE STORY WOOD

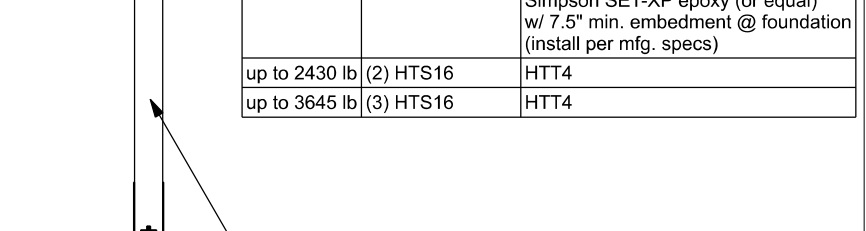


(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



MULTI PLY GIRDER TRUSS HOLD DOWN TABLE

Uplift	Top Connection	Bottom Connection
up to 1215 lb	HTS16	LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6"



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL WOOD FRAME w/ STRAPS & ANCHORS



Uplift SP	Uplift SFF	Uplift SPP	Truss Connector	To Plate	To Truss/Rafter
805	505		SDWC15600	-	-
400	290	H2.5A		4-131°x1 1/2"	4-131°x1 1/2"
625	540	H3		5-131°x1 1/2"	5-131°x1 1/2"
1040	1015	H10A		8-148°x1 1/2"	8-148°x1 1/2"
645	515	LTS12.30		6-148°x1 1/2"	6-148°x1 1/2"
990	850	MTS12.30		7-148°x1 1/2"	7-148°x1 1/2"
1415	1215	HTS16.30		8-148°x1 1/2"	8-148°x1 1/2"

EXTERIOR WALL STUD TABLE FOR SFF #2 STUDS:

Uplift SP	Uplift SFF	Uplift SPP	Holddowns @ Stenwall	To Stud / Post	Anchor
1235	1235	LSTA24		8-148°x1 1/2"	8-148°x1 1/2"
1640	1460	MSTA24		10-148°x1 1/2"	10-148°x1 1/2"
1030	1030	CS20		7-148°x1 1/2"	7-148°x1 1/2"
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Strap Ties <td>To Stud <td>To Plate</td> </td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Strap Ties <td>To Stud <td>To Plate</td> </td></td></td>	Uplift SPP <td>Strap Ties <td>To Stud <td>To Plate</td> </td></td>	Strap Ties <td>To Stud <td>To Plate</td> </td>	To Stud <td>To Plate</td>	To Plate
555	535	SP1	4-148°x3"	4-148°x3"	4-148°x3"
1010	605	SP2	6-148°x3"	6-148°x3"	6-148°x3"
1280	1100	SP#46Z		12-148°x1 1/2"	wrap under or over plate
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td></td>	Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td>	Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td>	To Stud / Post <td>Anchor </td>	Anchor
1235	1235	LSTA24		8-148°x1 1/2"	8-148°x1 1/2"
1235	1235	LSTA24		14-148°x1 1/2"	wrap under or over plate
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Holddowns @ Stenwall <td>To Stud / Post <td>Anchor </td></td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Holddowns @ Stenwall <td>To Stud / Post <td>Anchor </td></td></td></td>	Uplift SPP <td>Holddowns @ Stenwall <td>To Stud / Post <td>Anchor </td></td></td>	Holddowns @ Stenwall <td>To Stud / Post <td>Anchor </td></td>	To Stud / Post <td>Anchor </td>	Anchor
2145	1835	DTT22		8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4		18-162°x2 1/2"	1/2"x12" Titen HD
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td></td>	Uplift SPP <td>Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td></td>	Holddowns @ Mono <td>To Stud / Post <td>Anchor </td></td>	To Stud / Post <td>Anchor </td>	Anchor
2145	1835	DTT22		8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4		18-162°x2 1/2"	1/2"x12" Titen HD
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Post Bases @ Stenwall <td>To Post <td>Anchor </td></td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Post Bases @ Stenwall <td>To Post <td>Anchor </td></td></td></td>	Uplift SPP <td>Post Bases @ Stenwall <td>To Post <td>Anchor </td></td></td>	Post Bases @ Stenwall <td>To Post <td>Anchor </td></td>	To Post <td>Anchor </td>	Anchor
1900		ABU66Z		12-162°x3 1/2"	5/8"x12" Drill & Epoxy
2475		ABU66Z		12-162°x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP <td>Uplift SFF <td>Uplift SPP <td>Post Bases @ Mono <td>To Post <td>Anchor </td></td></td></td></td>	Uplift SFF <td>Uplift SPP <td>Post Bases @ Mono <td>To Post <td>Anchor </td></td></td></td>	Uplift SPP <td>Post Bases @ Mono <td>To Post <td>Anchor </td></td></td>	Post Bases @ Mono <td>To Post <td>Anchor </td></td>	To Post <td>Anchor </td>	Anchor
1900		ABU66Z		12-162°x3 1/2"	5/8"x12" Drill & Epoxy
2475		ABU66Z		12-162°x3 1/2"	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SFF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP EXTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

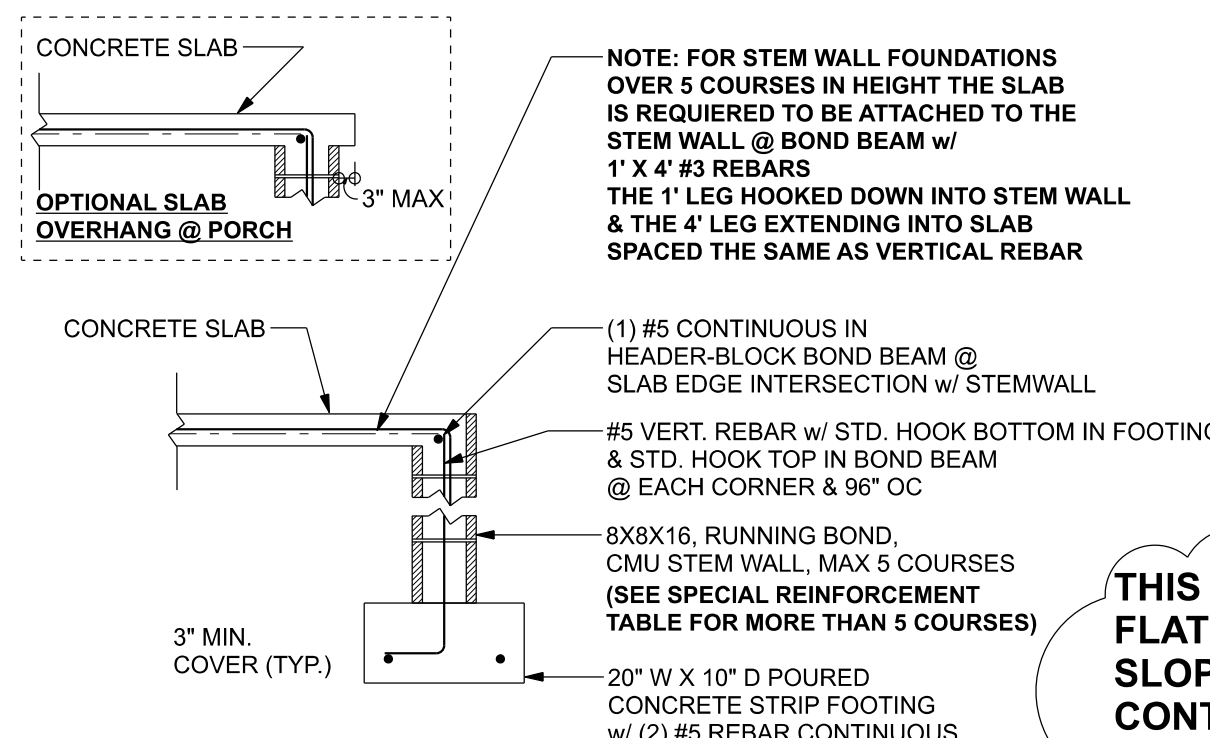
Uplift	Top Connection	Bottom Connection
(1) 2x4 @ 16" OC		TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC		TO 11'-2" STUD HEIGHT
(1) 2x4 @ 16" OC		TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC		TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

Grade	Species	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

ROOF SYSTEM DESIGN:

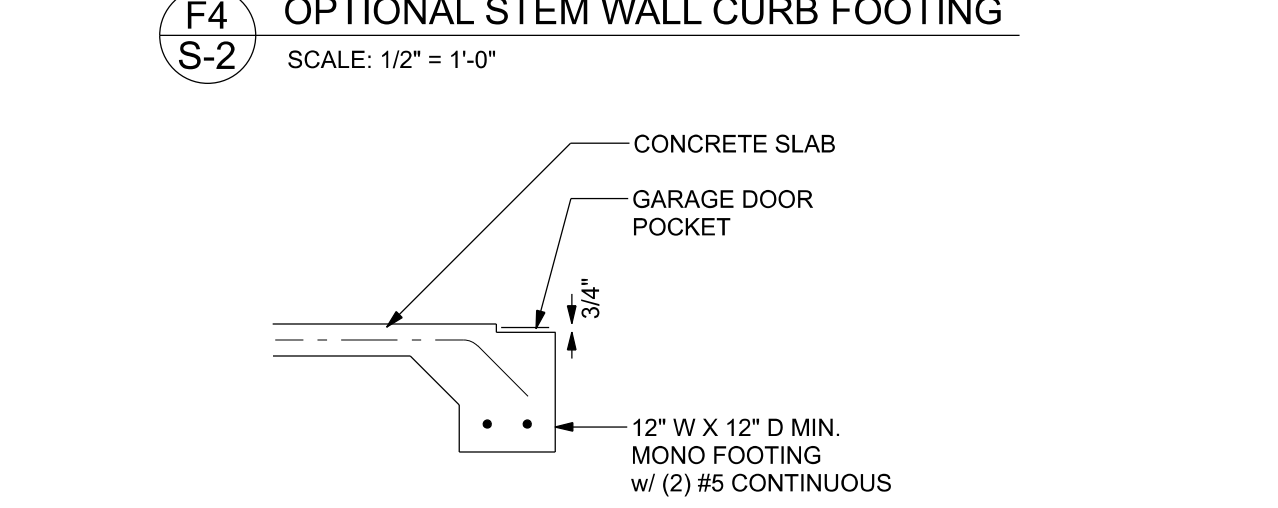
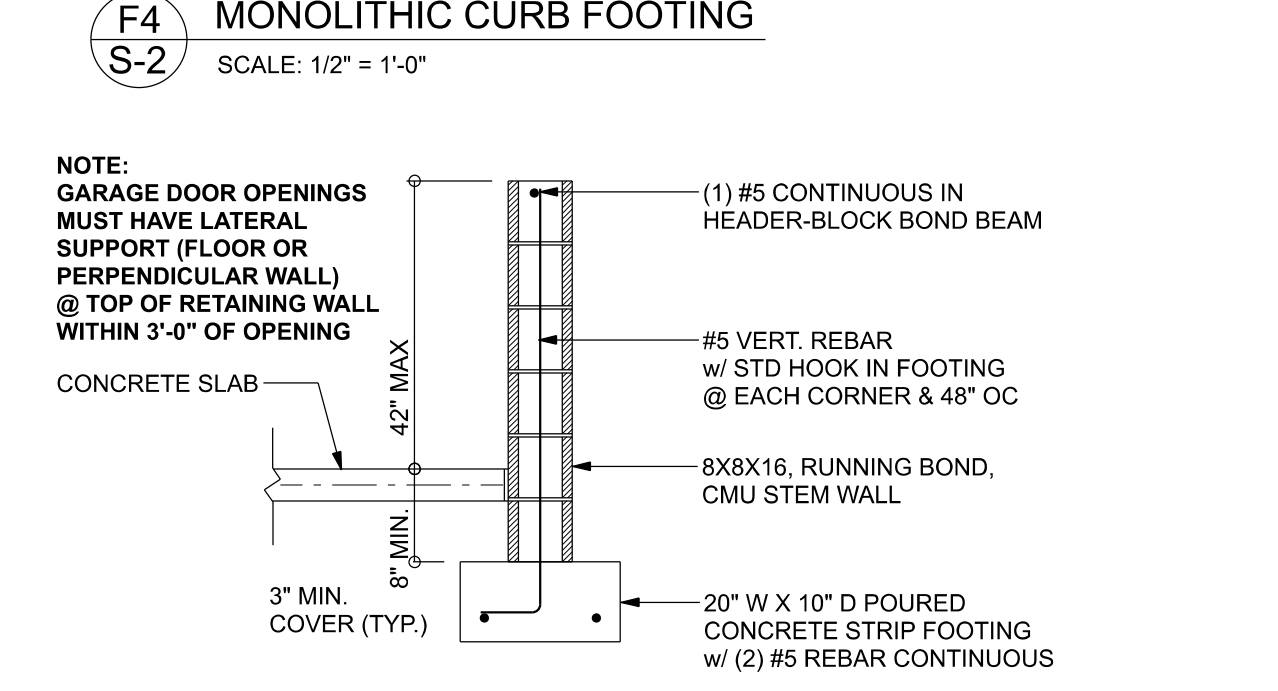
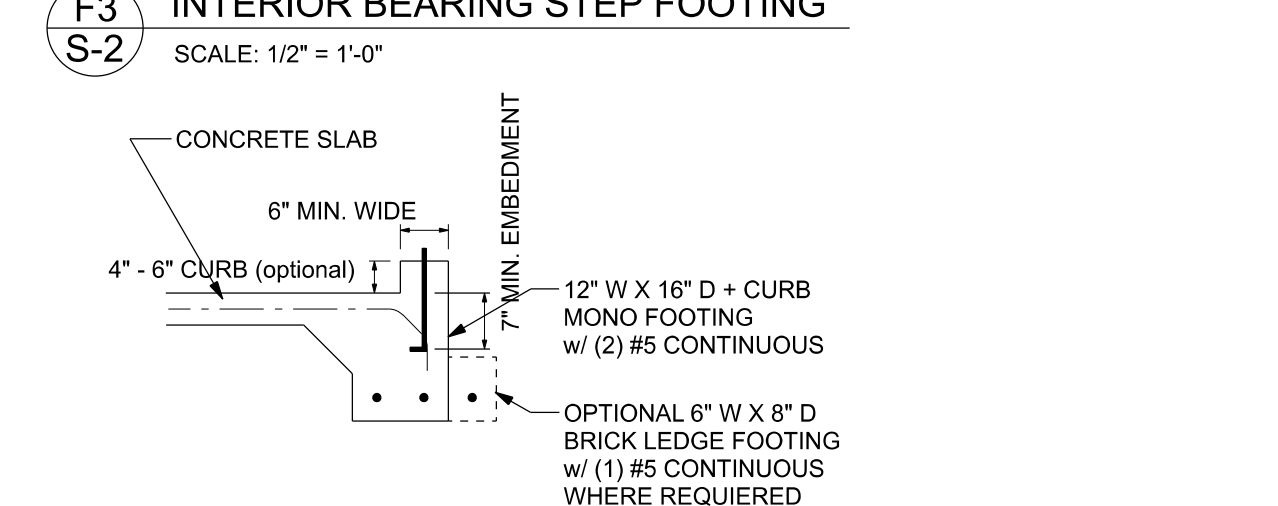
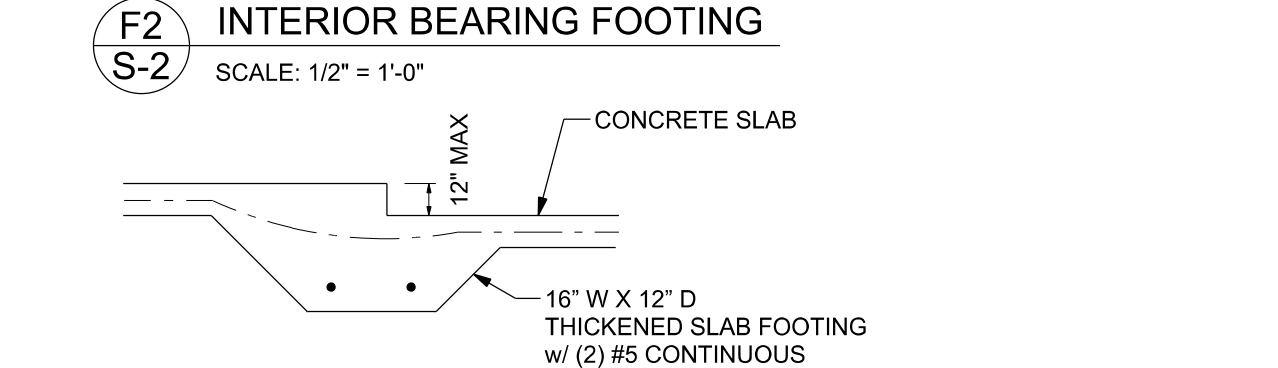
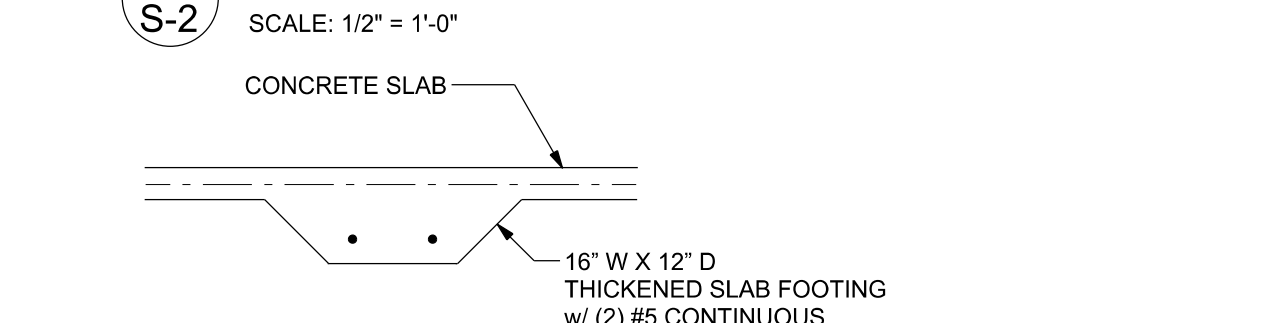
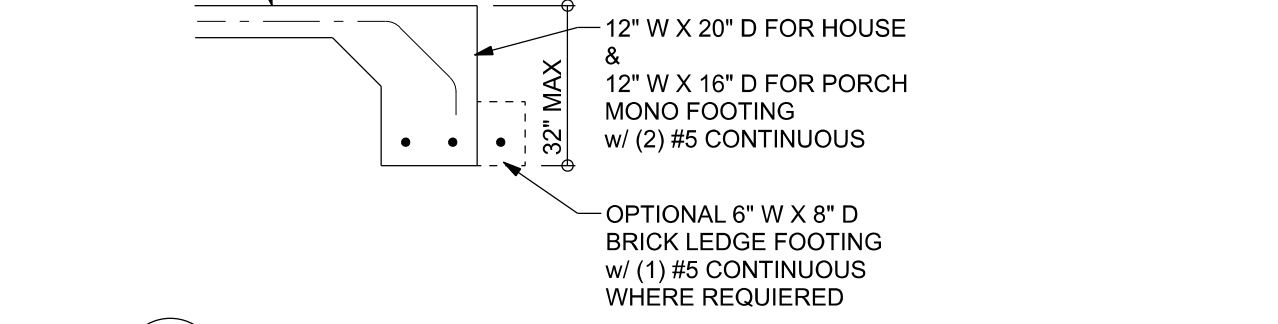
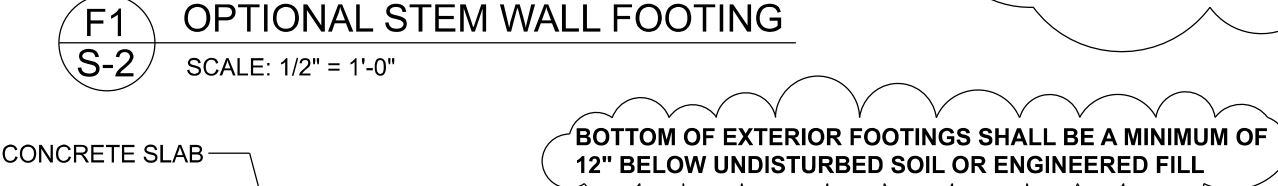
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A



TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

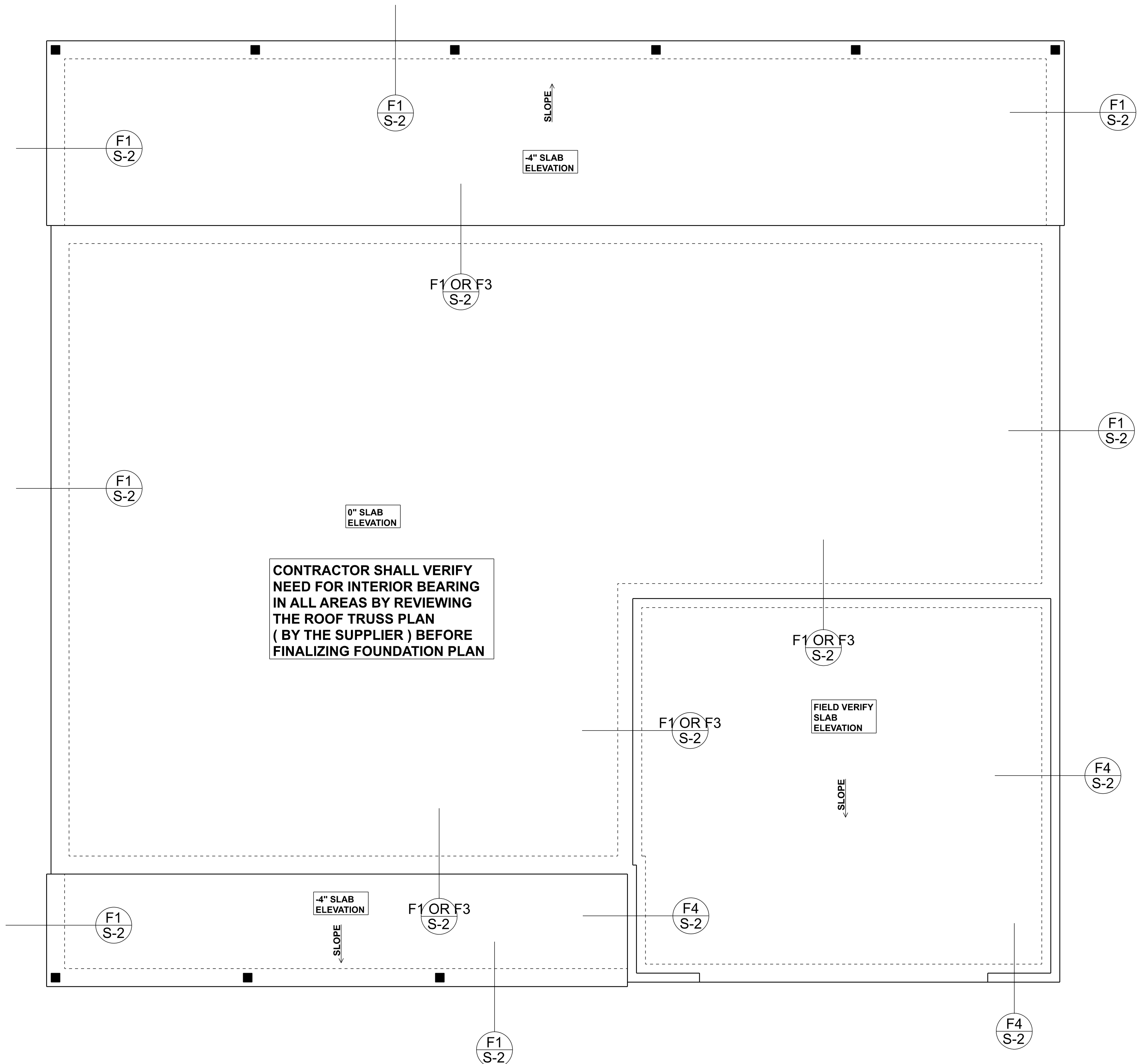
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	
5.3	5.0	56	96	96	96	96	
6.0	5.7	40	80	96	96	96	
6.7	6.3	32	56	80	96	96	

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12'. CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING



MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 518/ASCE 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls F'm = 1500 psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM A615, Grade 40, Fy = 40 ksi. Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout. ASTM A525, Class GR60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout. ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ (2) #4 @ 12" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

FOUNDATION DESIGN: Size footings per truss reactions and other loads. Locate footings per truss bearings. Interior shear walls require a thickened slab footing. For point loads > 5000 lb or repetitive loads > 3000 lb per truss provide pad footing 1' x 1' sqft, #5, 8"oc each way per 1500 lb of load.

Menendez Construction
Spec House - 217 SW Blue Jay Ct.
PROJECT ADDRESS: 217 SW Blue Jay Ct., Fort White, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disosway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

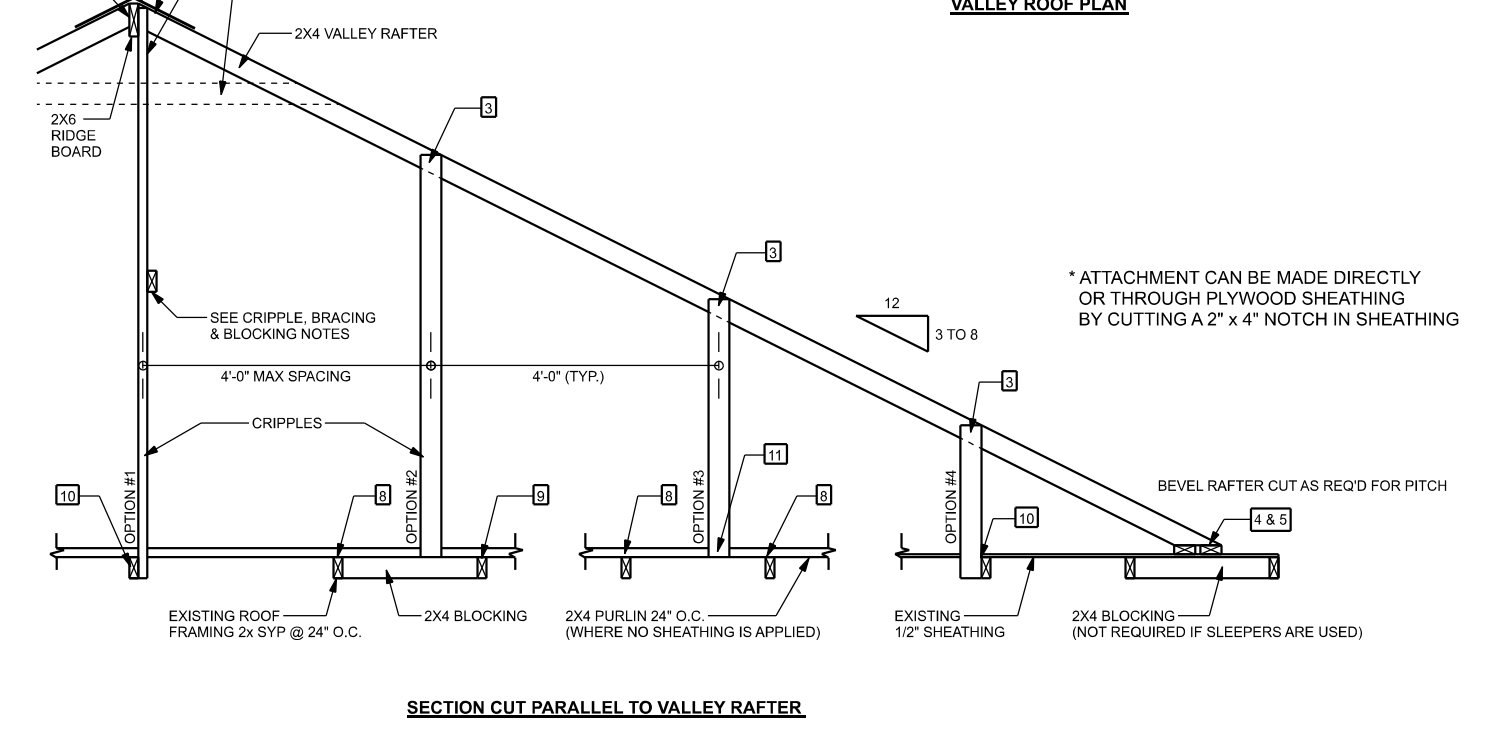
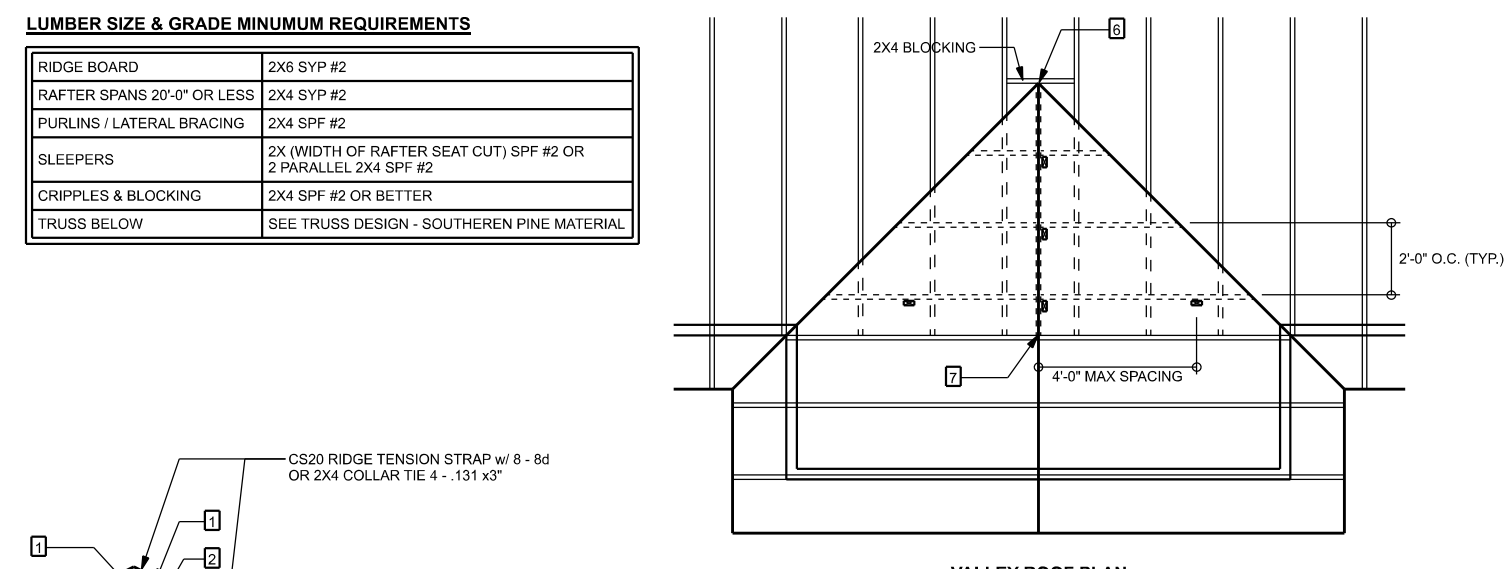
LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
260072

S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
RAIDGE BOARD	2X4 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLIN / LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER BEAT CUT) SPF #2 OR 2X (VALLEY) 2X4 SYP #2
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S.

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20' (TL) AND 10' (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

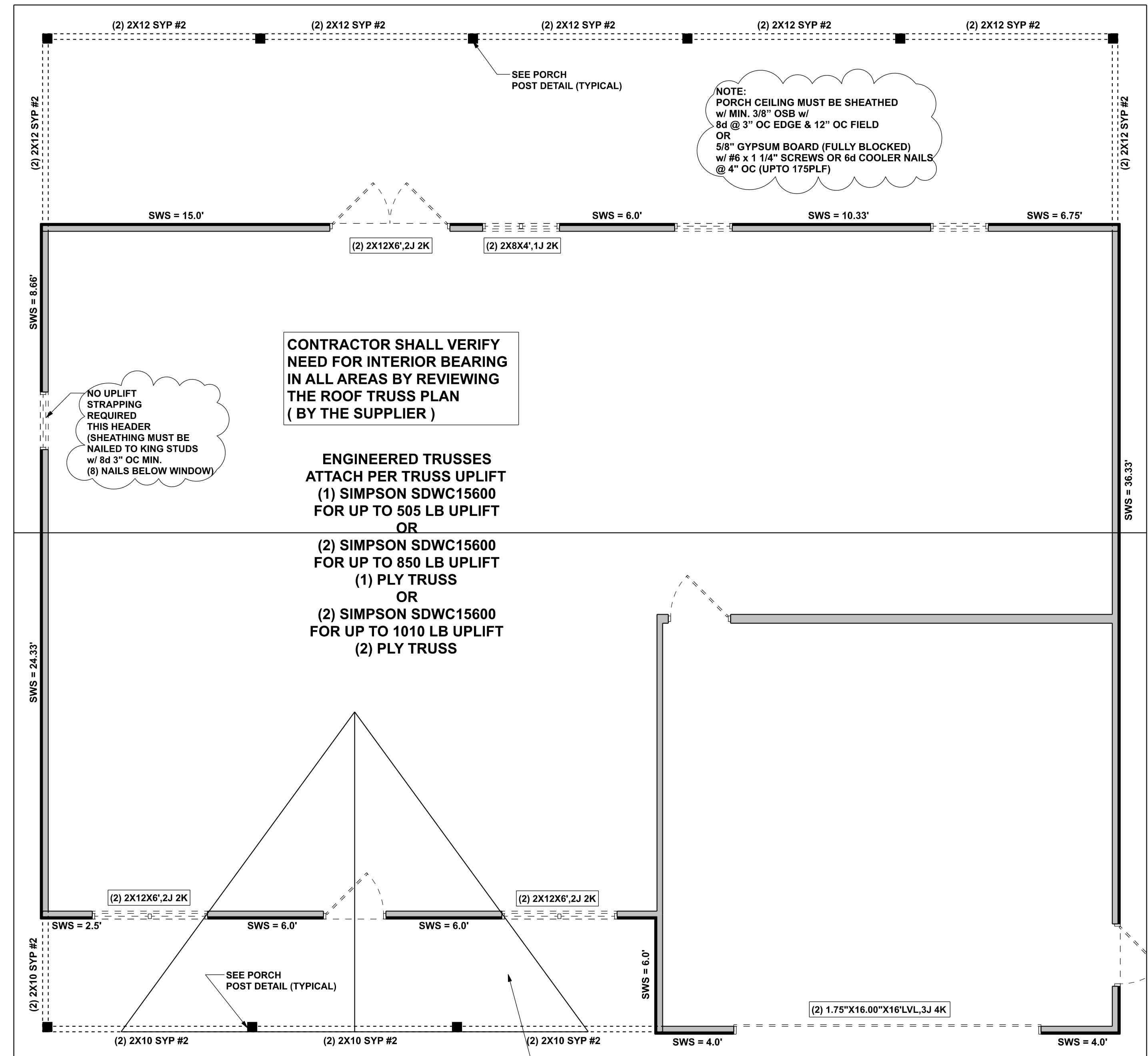
1 2X4 RAFTERS TO RIDGE	4 - 131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	4 - 131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	4 - 131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	4 - 131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4 - 131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO RIDGE BLOCK	4 - 131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	4 - 131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP)	4 - 131 x 3" NAILS
9 PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 - 131 x 3" NAILS
10 TRUSS TO BLOCKING	4 - 131 x 3" END NAILS
11 CRIPPLE TO PURLIN	4 - 131 x 3" FACE NAILS

GENERAL NOTES

- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2
- MAXIMUM ROOF AREA PER SUPPORT: 1992 IN ZONES 2 & 3, 2402 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1600 SQ FT X 2.47 SPAN = 1992)
- PURLIN REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS FROM TOE BOARD TO FINISH ONE TRUSS SPACING MINIMUM IN CASE THAT THE STRUCTURAL COVER SHEATHING IS MINIMUM OF 1/2" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 1-1/2 COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
 - MAXIMUM WIND SPEED: 135 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LENGTH: 40 FEET
- SEE T&E TABLE 7 WIND REQUIREMENTS
- EXPOSURE CATEGORY: 'C' 1 = 1.0, K4 = 1.0
- ENCLOSED BUILDING

CRIPPLE BRACING & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG
- NAILS = 2 - 10D NAILS OR 2X4 1" OR SCAB BRACE NAILS TO FLAT EDGE OF CRIPPLE WITH NAILS @ 9" O.C. "T" OR SCAB MUST BE 50% OF CRIPPLE LENGTH CRIPPLES OVER 10'-0" LONG REQUIRE TWO SCAB OR BOTH FACES 1" OR SCAB USE STRESS GRADED LUMBER A BOX OR COMMON NAILS
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1999 SECTION 12, NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE



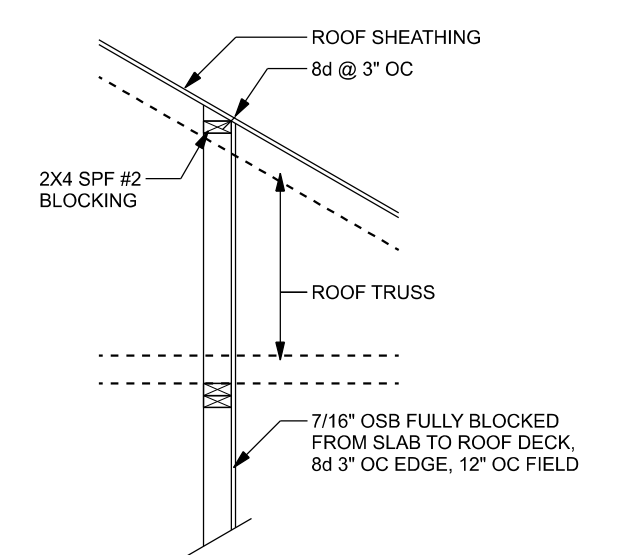
CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER)

ENGINEERED TRUSSES ATTACH PER TRUSS UPLIFT FOR UP TO 505 LB UPLIFT OR (2) SIMPSON SDWC15600 FOR UP TO 850 LB UPLIFT (1) PLY TRUSS OR (2) SIMPSON SDWC15600 FOR UP TO 1010 LB UPLIFT (2) PLY TRUSS

NO UPLIFT STRAPPING REQUIRED THIS HEADER (SHEATHING MUST BE NAILED TO KING STUDS w/ 8d 3" OC MIN. (8) NAILS BELOW WINDOW)

NOTE: PORCH CEILING MUST BE SHEATHED w/ MIN. 3/8" OSB w/ 8d @ 3" OC EDGE & 12" OC FIELD OR 5/8" GYPSUM BOARD (FULLY BLOCKED) w/ #6 x 1 1/4" SCREWS OR 6d COOLER NAILS @ 4" OC (UPTO 175PLF)

NOTE: PORCH CEILING MUST BE SHEATHED w/ MIN. 3/8" OSB w/ 8d @ 3" OC EDGE & 12" OC FIELD OR 5/8" GYPSUM BOARD (FULLY BLOCKED) w/ #6 x 1 1/4" SCREWS OR 6d COOLER NAILS @ 4" OC (UPTO 175PLF)



ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

NOTE: IF THE ABOVE DETAIL IS USED ON THE FRONT & REAR PORCH WALL THE FRONT & REAR PORCH CEILING DOES NOT NEED TO BE SHEATHED

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

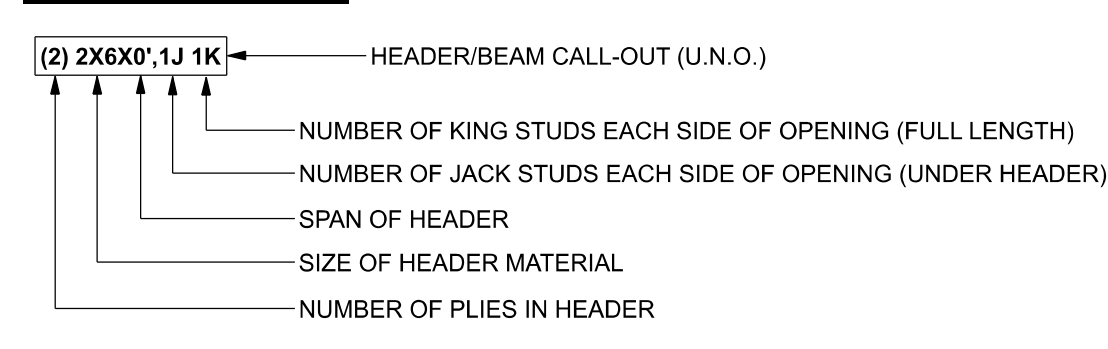
STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.
- SN-2 LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	18076 LBF	14419 LBF
REQUIRED	17232 LBF	10968 LBF

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #2 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



Mendez Construction
Spec House - 217 SW Blue Jay Ct.
PROJECT ADDRESS: 217 SW Blue Jay Ct., Fort White, FL

FL PE 53015
This item has been digitally signed and sealed by Mark Disosway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 2018 Edition Florida Building Code Residential (2023) to the best of my knowledge.
LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
260072
S-3
OF 3 SHEETS