

- APPLICABLE CODES AND STANDARDS
- 2023 FLORIDA BUILDING CODE, BUILDING
 - 2023 FLORIDA BUILDING CODE, RESIDENTIAL
 - ASCE 7 22 MINIMUM DESIGN LOADS ON BUILDINGS AND OTHER STRUCTURES
 - AISC STEEL CONSTRUCTION MANUAL (15TH EDITION)
 - ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
 - TMS 402-16 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
 - AWS D1.1 STRUCTURAL WELDING

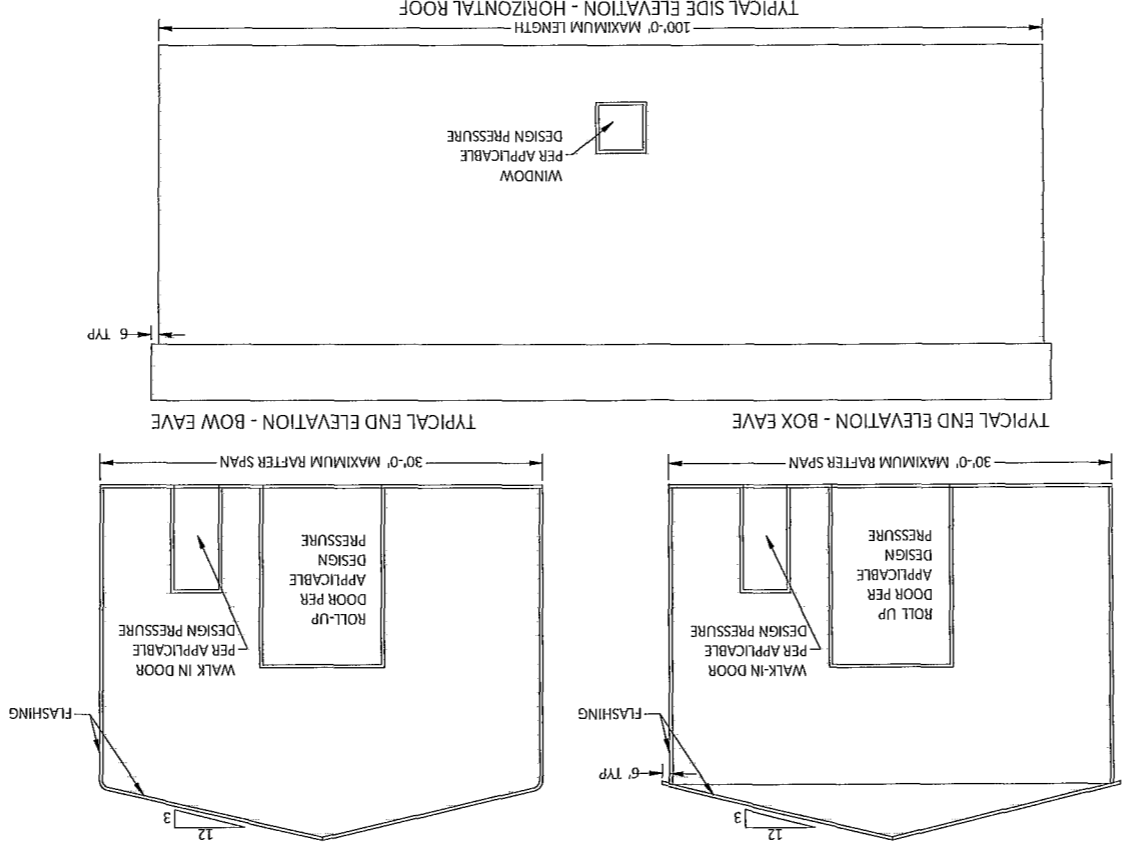
- DESIGN LOADS
- DEAD LOAD = 1.5 PSF
 - ROOF LIVE LOAD = 12 PSF
 - GROUND SNOW LOAD = 1 PSF
 - WIND LOAD
 - RISK CATEGORY = 1
 - WIND EXPOSURE CATEGORY = C
 - ULTIMATE WIND SPEED = 110 MPH - 140 MPH
 - NOMINAL WIND SPEED = 86MPH 110 MPH

- INSTALLATION NOTES AND SPECIFICATIONS
- THESE PLANS BELONG EXCLUSIVELY TO THE STRUCTURE, INCLUDING MAIN WIND FORCE RESISTING SYSTEM (MFRS), COMPONENTS AND CLADDING (C&C), AND BASE RAIL ANCHORAGE. OTHER DESIGN ISSUES, INCLUDING BUT NOT LIMITED TO PROPERTY SET-BACKS, ELECTRICAL, PLUMBING, INGRESS/EGRESS, FINISH FLOOR SLOPES AND ELEVATIONS, OR OTHER LOCAL ZONING REQUIREMENTS ARE THE LIABILITY OF OTHERS.
 - THESE STRUCTURES ARE ENGINEERED AS CAPABLE OF SUPPORTING DEAD LOAD OF THE STRUCTURE AND LIVE AND LOADS UPGRADES NOT SPECIFICALLY ADDRESSED HEREIN, SUCH AS WINDOWS, DOORS OR ANOTHER COMPONENT NOT LISTED IN THE BUILDING CODE APPROVED PRODUCT LIST, AND NOT PROVIDED AND INSTALLED BY THE CONTRACTOR, WHICH CAUSE ADDITIONAL LOADS ON THE STRUCTURE SHALL BE AT THE OWNER'S RISK. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR FAILURE OR STRUCTURAL DAMAGE DUE TO THE EXTRA LOAD.
 - ALL STEEL TUBING SHALL BE 50 KSI GALVANIZED STEEL. ALL FASTENERS SHALL BE ZINC COATED HARDWARE.
 - END WALL COLUMNS (POST) AND SIDE WALL COLUMNS ARE EQUIVALENT IN SIZE AND SPACING U N O.
 - SPECIFICATIONS APPLICABLE TO 29 GA METAL PANELS FASTENED DIRECTLY TO 2"X2.5"X14 GA TUBE STEEL (TS) FRAMING MEMBERS FOR VERTICAL PANELS. 29 GA METAL PANELS SHALL BE FASTENED DIRECTLY TO 18 GA HAT CHANNELS U N O.
 - AVERAGE FASTENER SPACING ON CENTERS ALONG RAFTERS OR PURLINS, AND POSTS, INTERIOR = 9" AND END = 6" MAX.
 - FASTENERS CONSIST OF #12-14X3/4" SELF-DRILLING SCREWS (SDS), USE CONTROL SEAL WASHER WITH EXTERIOR FASTENERS. SPECIFICATIONS APPLICABLE ONLY FOR MEAN ROOF HEIGHT OF 20'-0" OR LESS, AND ROOF SLOPES OF 14" (3 12 PITCH) OR LESS SPACING REQUIREMENTS FOR OTHER ROOF HEIGHTS AND/OR SLOPES MAY VARY.
 - ANCHORS SHALL BE INSTALLED THROUGH THE BASE RAIL WITHIN 6" OF EACH RAFTER COLUMN ALONG SIDES AND ENDS.
 - STANDARD GROUND ANCHORS (SOIL NAILS) CONSIST OF #4 REBARS WITH WELDED NUT X 36" LONG AND MAY BE USED IN SUITABLE SOILS. OPTIONAL ANCHORAGE MAY BE USED IN SUITABLE SOILS AND MUST BE USED IN UNSUITABLE SOILS AS NOTED. SOIL NAILS MAY BE USED FOR WIND SPEEDS LESS THAN OR EQUAL TO 145 MPH.
 - RAFTER SPACING IS 5'-0" MAX.
 - PURLIN SPACING IS 4'-0" MAX.
 - WIND FORCES GOVERN OVER SEISMIC FORCES.
 - CONSTRUCTION IN SPECIAL FLOOD HAZARD AREAS.
 - CONTRACTOR TO VERIFY THAT THE FINISHED FLOOR ELEVATION FOR THE PROPOSED STRUCTURE IS AT OR ABOVE THE GREATER OF THE FOLLOWING ELEVATIONS
 - BASE FLOOD ELEVATION + 2'-0"
 - DFE (DESIGN FLOOD ELEVATION)
 - THE MINIMUM ELEVATION MANDATED BY THE BUILDING CODES ADOPTED BY THE AUTHORITY HAVING JURISDICTION.

DRAWING INDEX

PAGE NO	DESCRIPTION
1	TITLE PAGE WITH INDEX
2	TRUSS DESIGN FOR RAFTER SPAN
3	CONNECTION DETAILS (1-2)
4	BASE RAIL AND FOUNDATION ANCHORAGE
5	RAFTER END WALL, SIDE WALL AND OPENING FRAMING
6	CONNECTION DETAILS (4-14)
7	BOX EAVE RAFTER LEAN TO OPTIONS
8	CONNECTION DETAILS (16-18)
9	BOX EAVE RAFTER VERTICAL ROOF/SIDING OPTION
10	OPTIONAL HELICAL ANCHORING ON GRADE DETAIL
11	OPTIONAL CONCRETE STRIP FOOTING
12	OPTIONAL HELICAL ANCHORING ON TIMBER BEAM DETAIL

ENCLOSED METAL BUILDING DESIGN
 MAXIMUM 30'-0" WIDE X 100'-0" LONG X 20'-0" HIGH (EAVE)
 BOX EAVE FRAME / BOW EAVE FRAME

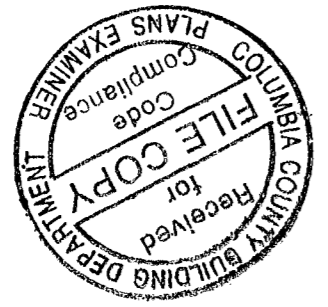


ADJUSTED C & C WIND PRESSURES (ROOF, ASD, PSF)

ADJUSTED C & C WIND PRESSURES (WALL, ASD, PSF)	EFFECTIVE WIND AREA (SQ. FT)	ALL ZONES (POSITIVE) =	ALL ZONES (NEGATIVE) =	EFFECTIVE WIND AREA (SQ. FT)	ALL ZONES (POSITIVE) =	ALL ZONES (NEGATIVE) =
10.00	10.00	NA	NA	1000.00	NA	NA
20.00	20.00	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
30.00	30.00	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
40.1	40.1	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
50.00	50.00	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
60.1	60.1	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
70.5	70.5	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
83.6	83.6	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
97.7	97.7	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
100.1	100.1	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
121.2	121.2	ZONE 1 (NEGATIVE) = 60.1	ZONE 1 (OVERHANG) = 29	500.00	ALL ZONES (POSITIVE) = 500.00	ALL ZONES (NEGATIVE) = 500.00
107.3	107.3	ZONE 3 (OVERHANG) = 84.9	ZONE 3 (NEGATIVE) = 84.9	200.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
89.6	89.6	ZONE 2 (OVERHANG) = 67.3	ZONE 2 (NEGATIVE) = 67.3	300.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
75.2	75.2	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	400.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
64	64	ZONE 1 (OVERHANG) = 43.4	ZONE 1 (NEGATIVE) = 43.4	500.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
55.1	55.1	ZONE 2 (OVERHANG) = 36.5	ZONE 2 (NEGATIVE) = 36.5	600.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
88.7	88.7	ZONE 3 (OVERHANG) = 84.9	ZONE 3 (NEGATIVE) = 84.9	700.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
74.7	74.7	ZONE 3 (OVERHANG) = 84.9	ZONE 3 (NEGATIVE) = 84.9	800.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
55.4	55.4	ZONE 2 (OVERHANG) = 67.3	ZONE 2 (NEGATIVE) = 67.3	900.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
45.8	45.8	ZONE 2 (OVERHANG) = 67.3	ZONE 2 (NEGATIVE) = 67.3	1000.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
36.5	36.5	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	1100.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
24.8	24.8	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	1200.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
NA	NA	ALL ZONES (POSITIVE) = NA	ALL ZONES (NEGATIVE) = NA	1300.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
NA	NA	ALL ZONES (POSITIVE) = NA	ALL ZONES (NEGATIVE) = NA	1400.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
NA	NA	ALL ZONES (POSITIVE) = NA	ALL ZONES (NEGATIVE) = NA	1500.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
24.8	24.8	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	1600.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
31.8	31.8	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	1700.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
31.8	31.8	ZONE 1 (OVERHANG) = 52.9	ZONE 1 (NEGATIVE) = 52.9	1800.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	1900.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2000.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2100.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2200.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2300.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2400.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2500.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2600.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2700.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2800.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	2900.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)
29.5	29.5	ALL ZONES (POSITIVE) = 29.5	ALL ZONES (NEGATIVE) = 29.5	3000.00	EFFECTIVE WIND AREA (SQ. FT)	EFFECTIVE WIND AREA (SQ. FT)

CONTRACTOR TO PROVIDE BUILDING CODE APPROVED PRODUCTS TO MEET OR EXCEED THE DESIGN PRESSURES AS TABULATED

GENERIC PLANS ARE NOT VALID WITHOUT A RAISED SEAL & BLUE INK SIGNATURE.
 THE ENGINEERING ON THESE PLANS IS SITE SPECIFIC FOR (1) STRUCTURE ONLY AT THE PROVIDED ADDRESS(ES)



SCALE: NTS
 DRAWN BY: JS
 SHEET: 1 OF 12

REVISION 2 DATE
 REVISION 1 DATE

DESIGN DATE: 09/26/2025

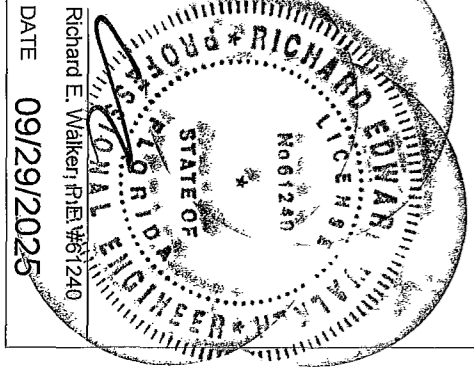
CONTRACTOR: STEEL BUILDINGS AND STRUCTURES INC., 800 PIEDMONT TRIAD WEST DR., MOUNT AIRY, NC 27030

PROJECT ADDRESS: 12-30' WIDE ENCLOSED GENERIC PLANS

FLORIDA ENGINEERING LLC
 4161 TAMiami TRAIL, UNIT 101
 PORT CHARLOTTE, FLORIDA 33952
 (941) 391-5980
 FILEng.com
 Orders@FILEng.com

PROJECT NO. 2526885

CA CERT #30782



DATE: 09/29/2025

MAX EAVE HEIGHT	20'
END WALL COLUMN DIMENSIONS	(2) 2.5X2.5X14 GA
	2.5X2.5X14 GA

- MEMBER LEGEND
- 1 SIDEWALL TS COLUMN = 2.5X2.5X14 GA U N O
 - 2 SIDEWALL TS DOUBLE COLUMN = (2) 2.5X2.5X14 GA U N O
 - 3 TRUSS MEMBERS = 2.5X2.5X14 GA U N O
 4. KNEE-BRACE = 2.5"X2"X18GA CHANNEL
 - 5 PURLIN = 1.125"X1.8GA HAT CHANNEL
 - 6 U BRACE = 2.5"X2"X16GA CHANNEL
 - 7 ENDWALL COLUMN

TRUSS LAYOUT- BOX EAVE

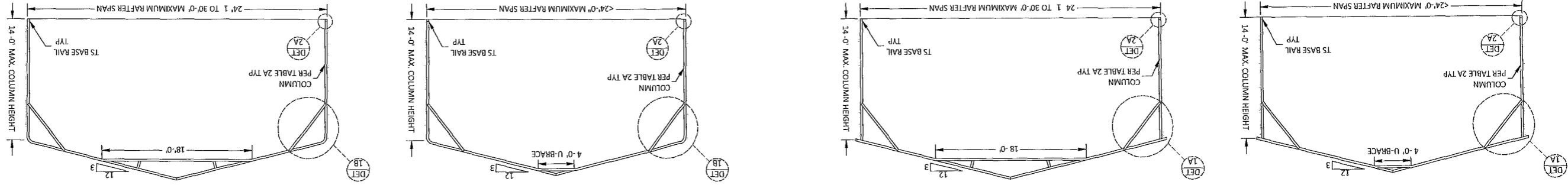


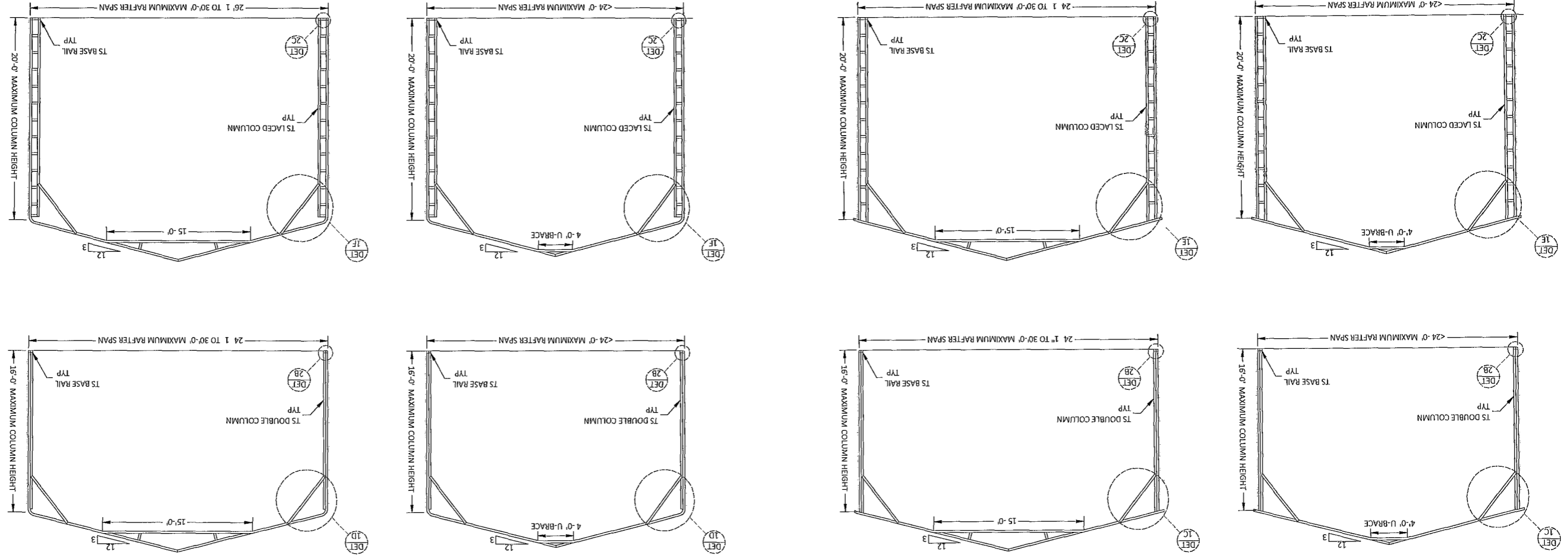
TABLE 2A.

BUILDING LENGTH	COLUMN DIMENSIONS
< 60'-0"	ALL COLUMNS TO BE 2.5X2.5X14 GA
60'-1" TO 100'-0"	(N 10) CENTRAL COLUMNS TO BE (2) 2.5X2.5X14 GA REST 2.5X2.5X14 GA

*N = NO. OF COLUMNS PER SIDE ELEVATION

FOR COLUMN HEIGHT OF MAX 14'-0"

TRUSS LAYOUT- BOW EAVE

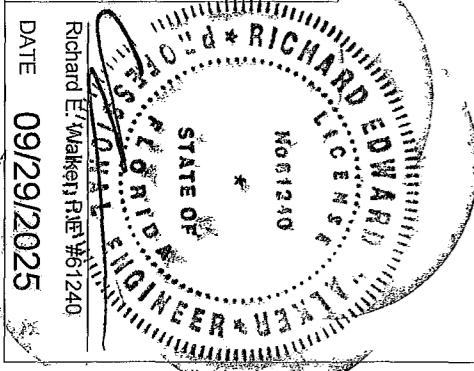


SCALE NTS
DRAWN BY JS
REVISION 2 DATE
REVISION 1 DATE
DESIGN DATE 09/26/2025

CONTRACTOR:
STEEL BUILDINGS AND STRUCTURES INC.
800 PIEDMONT TRIAD WEST DR.,
MOUNT AIRY, NC 27030

PROJECT ADDRESS:
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DATE 09/29/2025

SCALE	NTS
DRAWN BY	JS
REVISION 2 DATE	
REVISION 1 DATE	
DESIGN DATE	09/26/2025
SHEET	3
OF	12

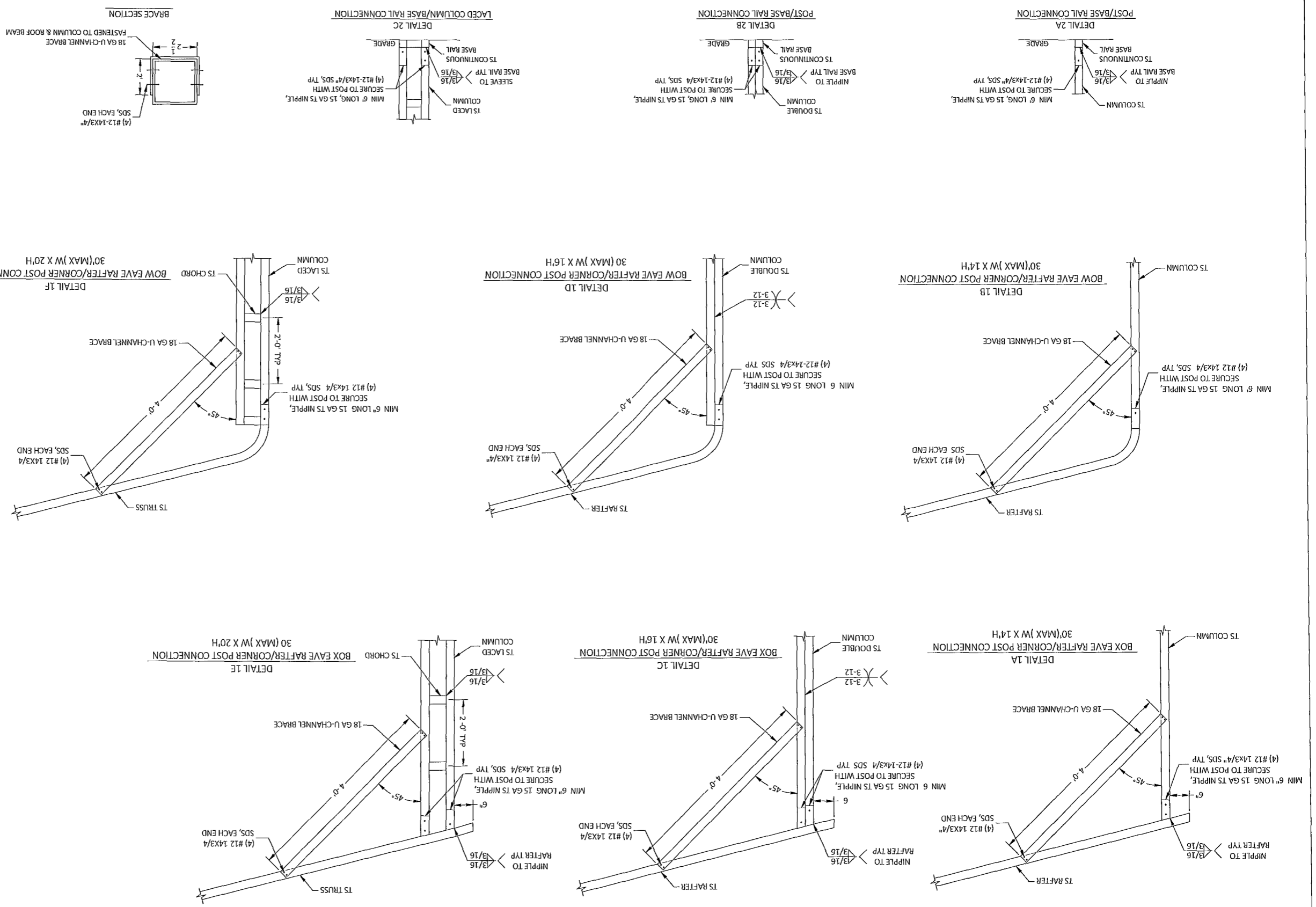
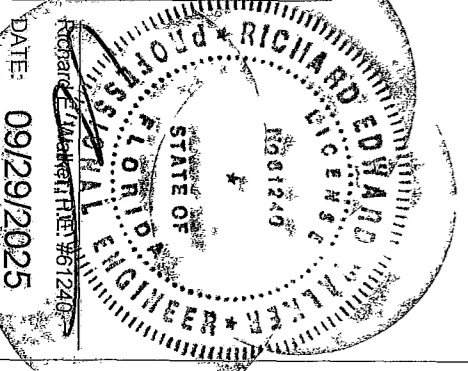
CONTRACTOR
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 800 PIEDMONT TRAIL WEST DR.,
 MOUNT AIRY, NC 27030

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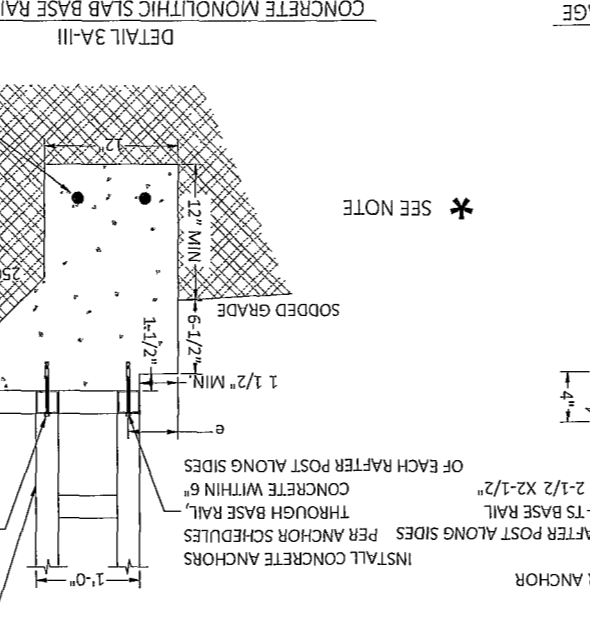
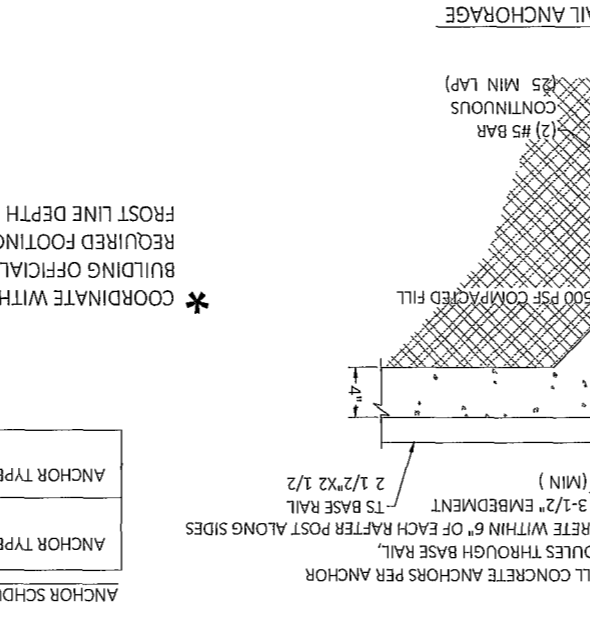
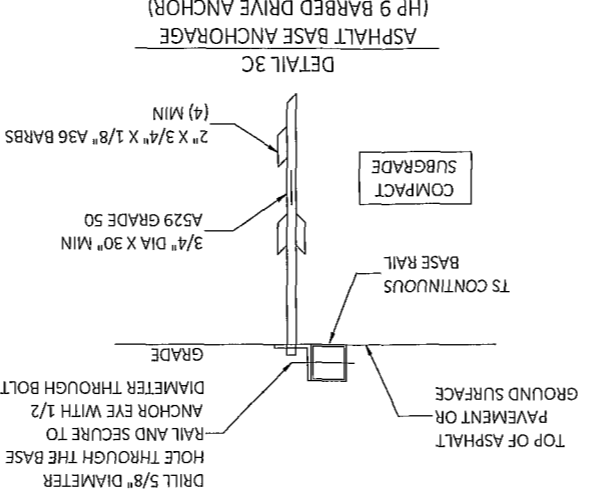
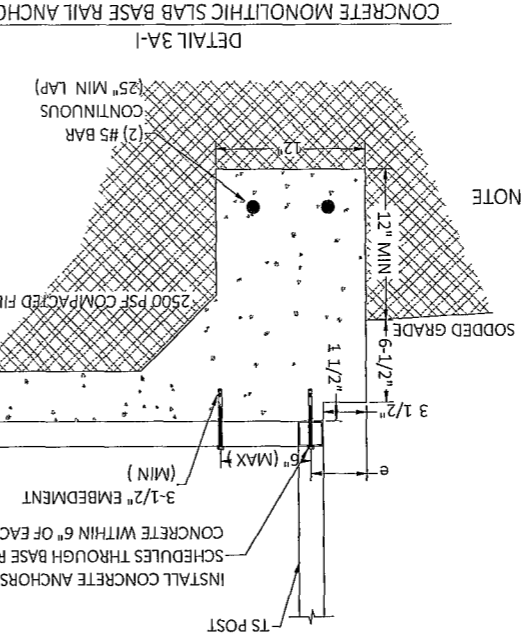
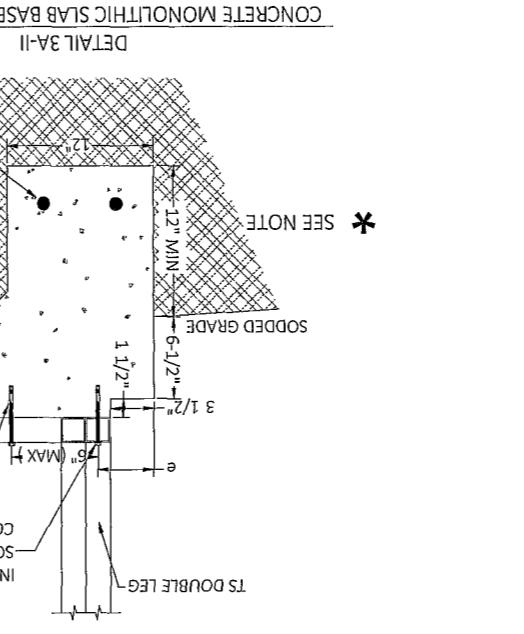
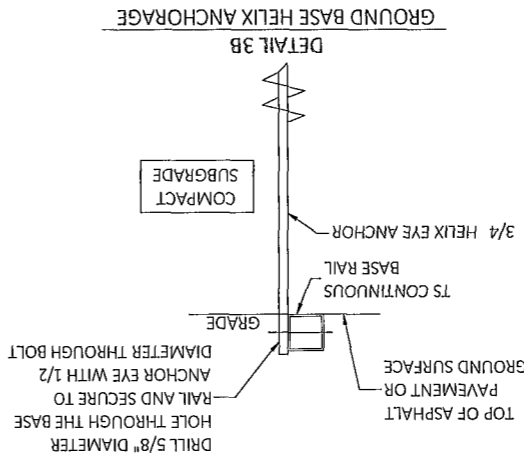


REQUIRED UPLIFT / BEARING CAPACITY	OF HELICAL ANCHORS	RAFTER SPACING (FT)
5	4	5
6	6	9.5
8	8	11.5
10	10	13.0
12	12	14.5
14	14	16.5
16	16	18.0
18	18	20.0
20	20	21.5

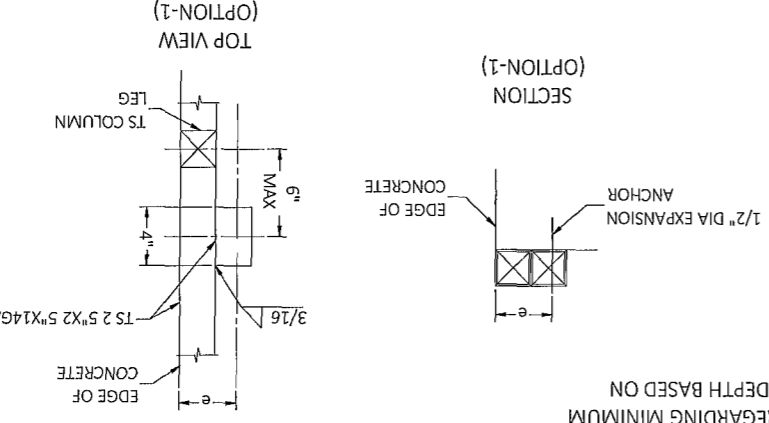
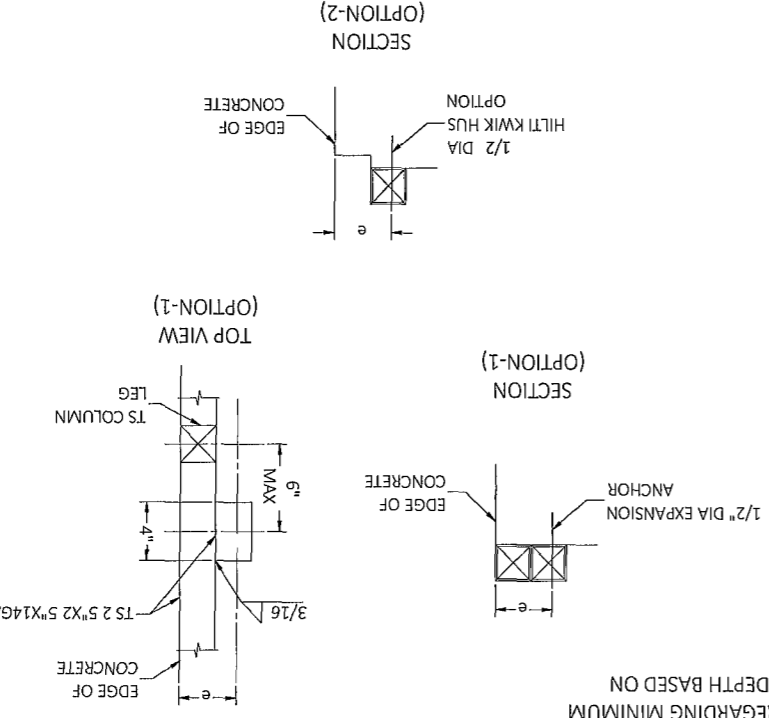
TABLE A

GENERAL NOTES:

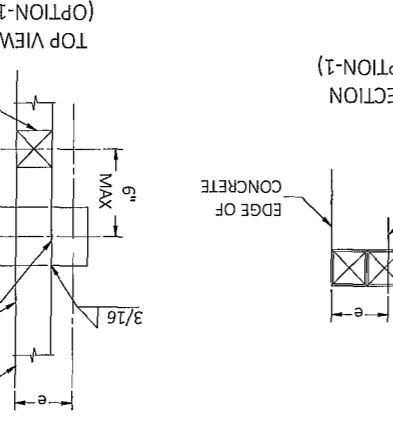
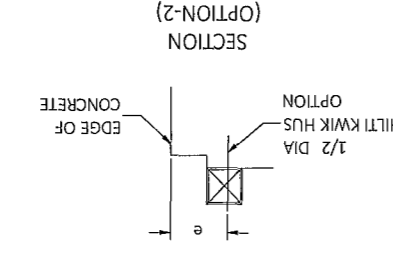
- CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.
- ALL OPEN AREAS OF CONCRETE OUTSIDE OF THE PROPOSED STRUCTURE SHALL BE DESIGNED TO SLOPE AWAY FROM THE STRUCTURE.
- WHERE CONCRETE SPECIFICATIONS ARE REQUIRED BY ONE OR MORE REGULATORY AGENCY THE FOLLOWING SPECIFICATIONS ARE APPLICABLE.
- CONCRETE SHALL CONFORM TO ASTM C94 FOR THE FOLLOWING COMPONENTS:
 - PORTLAND CEMENT TYPE 1 ASTM C150
 - AGGREGATES LARGE AGGREGATE 3/4" MAX. ASTM C33
 - AIR ENTRAINING 1% ASTM C260
 - WATER REDUCING AGENT ASTM C494
 - CLEAN POTABLE WATER
 - OTHER ADMIXTURES NOT PERMITTED
 - CONCRETE SLUMP AT DISCHARGE CHUTE NOT LESS THAN 3 OR MORE THAN 5 WATER
 - ADDED AFTER BATCHING IS NOT PERMITTED
 - PREPARE & PLACE CONCRETE PER AMERICAN CONCRETE INSTITUTE MANUAL OF STANDARD PRACTICE PART 1.2 & 3 INCLUDING NOT WEATHER RECOMMENDATIONS.
 - MUST CURE OR POLYETHYLENE CURING PERMITTED
 - PRIOR TO PLACING CONCRETE, TREAT THE ENTIRE SUBSURFACE AREA FOR TERMITES IN COMPLIANCE WITH THE BUILDING CODE (FOR RISK CATEGORY II, III, & IV STRUCTURES ONLY).
 - CONCRETE SLAB SHALL BE PLACED OVER A MIN 6 MIL POLYETHYLENE VAPOR BARRIER
 - CONTROL JOINTS SHALL BE PROVIDED AT EVERY 12' O.C. OR 18' O.C. FOR 4" THICK OR 6" (SLAB ONLY)
 - THICK CONCRETE SLAB RESPECTIVELY
- REINFORCEMENT SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT
- REINFORCEMENT MAY BE BENT IN THE FIELD OR SHOP AS LONG AS
 - IT IS BENT COLD
 - REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT
 - THE DIAMETER OF THE BEND MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS
 - FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-308 3 INCHES WHERE THE CONCRETE IS POURED AGAINST AND TEMPORARY CONTACT WITH THE EARTH OR UNPROTECTED FROM THE EARTH OR WEATHER, OTHERWISE 1 1/2 INCHES.
- FROST PROTECTION.
 - FOUNDATION SHALL BE PROTECTED AGAINST FROST USING RIGID FOAM INSULATION (EPS OR EQUIVALENT), FOR NO FROST PROTECTION OPTION COORDINATE WITH LOCAL BUILDING CODE AND/OR BUILDING OFFICIAL REGARDING REQUIRED FOOTING DEPTH BASED ON FROST LINE DEPTH
 - HELIUM ANCHOR NOTES.
 - FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CAUCHE, PRELOADED SILTS AND CLAYS, MEDIUM DENSE COARSE SANDS, SANDY GRAVELS, PRELOADED SILTS AND CLAYS, MEDIUM TO VERY LOOSE DENSE SANDS, FIRM TO STIFF CLAYS VERY STIFF SILTS AND CLAYS, MEDIUM TO VERY LOOSE DENSE SANDS, FIRM TO STIFF CLAYS PRELOADED SILTS AND CLAYS, CORALS, MEDIUM DENSE COARSE SANDS, SANDY GRAVELS, SANDY GRAVELS, MEDIUM DENSE COARSE SANDS, FIRM TO STIFF CLAYS
 - FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CAUCHE AS SHOWN IN DETAIL 3C.
 - FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CAUCHE INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING
 - THE UPLIFT/BEARING CAPACITY OF HELICAL ANCHOR MUST BE EQUAL TO OR GREATER THAN 8 KIPS FOR ANCHORS INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING.
 - THE UPLIFT/BEARING CAPACITY OF EACH ANCHOR MUST BE EQUAL TO OR GREATER THAN 5 KIPS FOR ANCHORS INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING
 - ANCHORS PROVIDED AT THE JAMBS OF DOOR OPENINGS, THE INCREASE IN ANCHOR CAPACITY MAY BE ACHIEVED BY INCREASING THE DIAMETER AND/OR THE EMBEDMENT OF THE ANCHORS OR BY USING DIFFERENT ANCHORS DEPENDING ON THE MANUFACTURER'S SPECIFICATIONS.
 - HP 9 BARBED DRIVE ANCHOR NOTES.
 - ANCHOR TO BE 3/4" DIA (A529 GRADE 50) WITH 30" MIN EMBEDMENT & (4) MIN BARBS AS SHOWN IN DETAIL 3C.
 - FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CAUCHE AS SHOWN IN DETAIL 3C.
 - FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CAUCHE INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING
 - THE UPLIFT/BEARING CAPACITY OF EACH ANCHOR MUST BE EQUAL TO OR GREATER THAN 8 KIPS FOR ANCHORS INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING.
 - THE UPLIFT/BEARING CAPACITY OF HELICAL ANCHOR MUST BE EQUAL TO OR GREATER THAN 5 KIPS FOR ANCHORS INSTALLED AT EVERY POST (LEG) / MAX. RAFTER SPACING.
 - ANCHORS PROVIDED AT THE JAMBS OF DOOR OPENINGS, THE INCREASE IN ANCHOR CAPACITY MAY BE ACHIEVED BY INCREASING THE DIAMETER AND/OR THE EMBEDMENT OF THE ANCHORS OR BY USING DIFFERENT ANCHORS DEPENDING ON THE MANUFACTURER'S SPECIFICATIONS.



ANCHOR TYPE #1	ANCHOR TYPE #2
1/2" DIA WEDGE ANCHOR WITH 5" MIN. EMBEDMENT INTO 3KSI MIN CONCRETE, 4" MIN. EDGE DISTANCE (e)	1/2" DIA HILTI KWIK HUS ANCHOR WITH 4" MIN. EMBEDMENT INTO 3KSI MIN CONCRETE 2.75 MIN. EDGE DISTANCE (e)

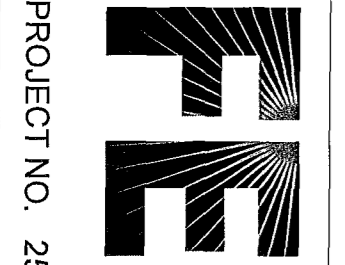


TYPICAL ANCHOR DETAIL WHEN BASE RAIL IS NEAR EDGE OF CONCRETE

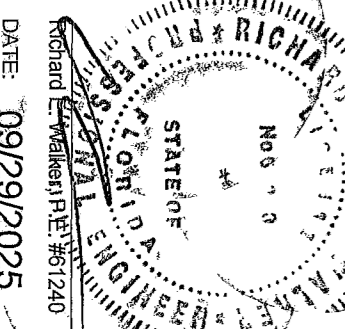


SCALE	NTS
DRAWN BY	JS
REVISION 2	DATE
REVISION 1	DATE
DESIGN DATE	09/26/2025

CONTRACTOR
STEEL BUILDINGS AND STRUCTURES INC.
 800 PIEDMONT TRIAD WEST DR.,
 MOUNT AIRY, NC 27030
 PROJECT ADDRESS:
 12'-30" WIDE ENCLOSED
 GENERIC PLANS



PROJECT NO. 2526885
FLORIDA ENGINEERING LLC
 4161 TAMAMI TRAIL, UNIT 101
 PORT CHARLOTTE, FLORIDA 33952
 (941) 391-5980
 FLEng.com
 Orders@FLEng.com



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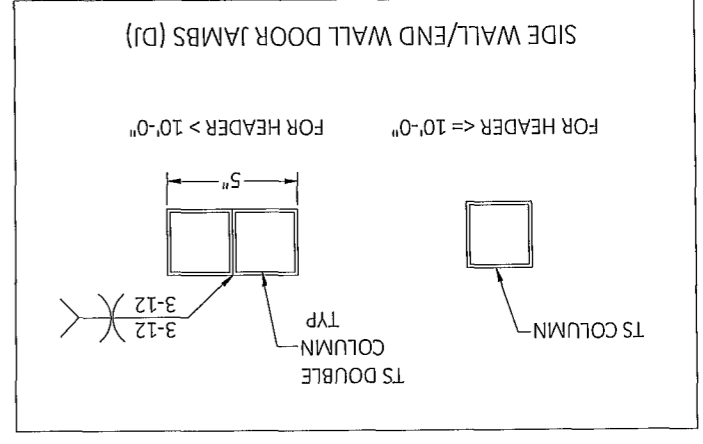
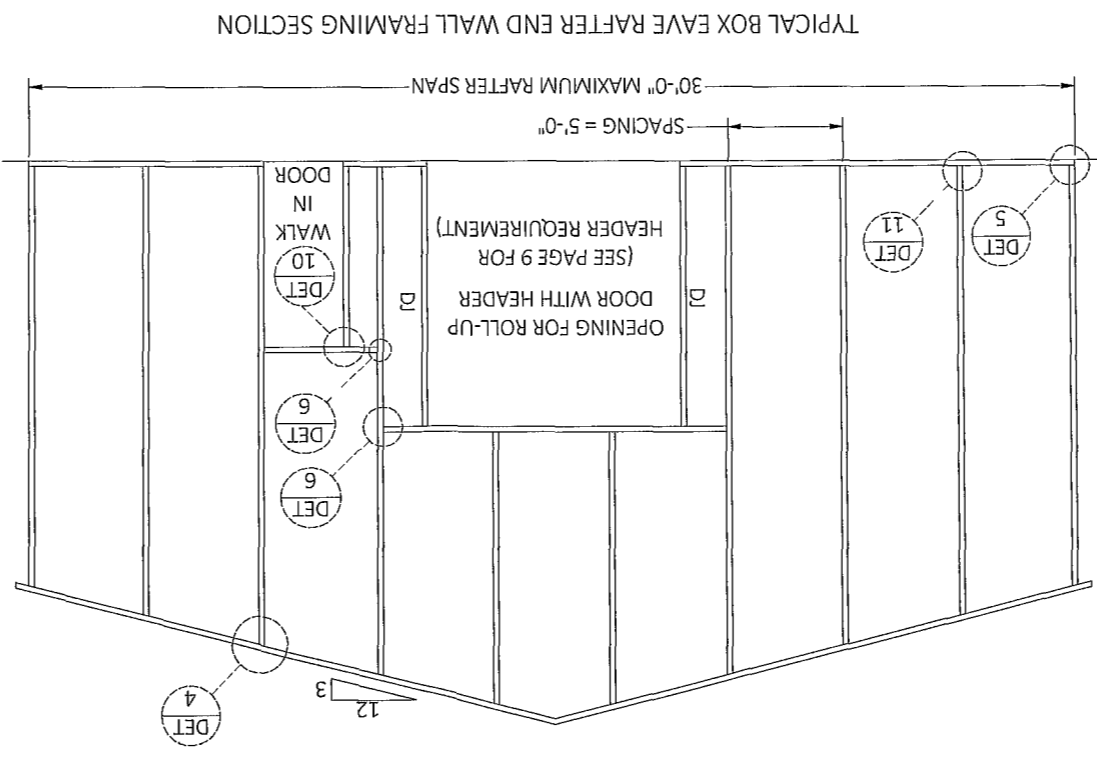
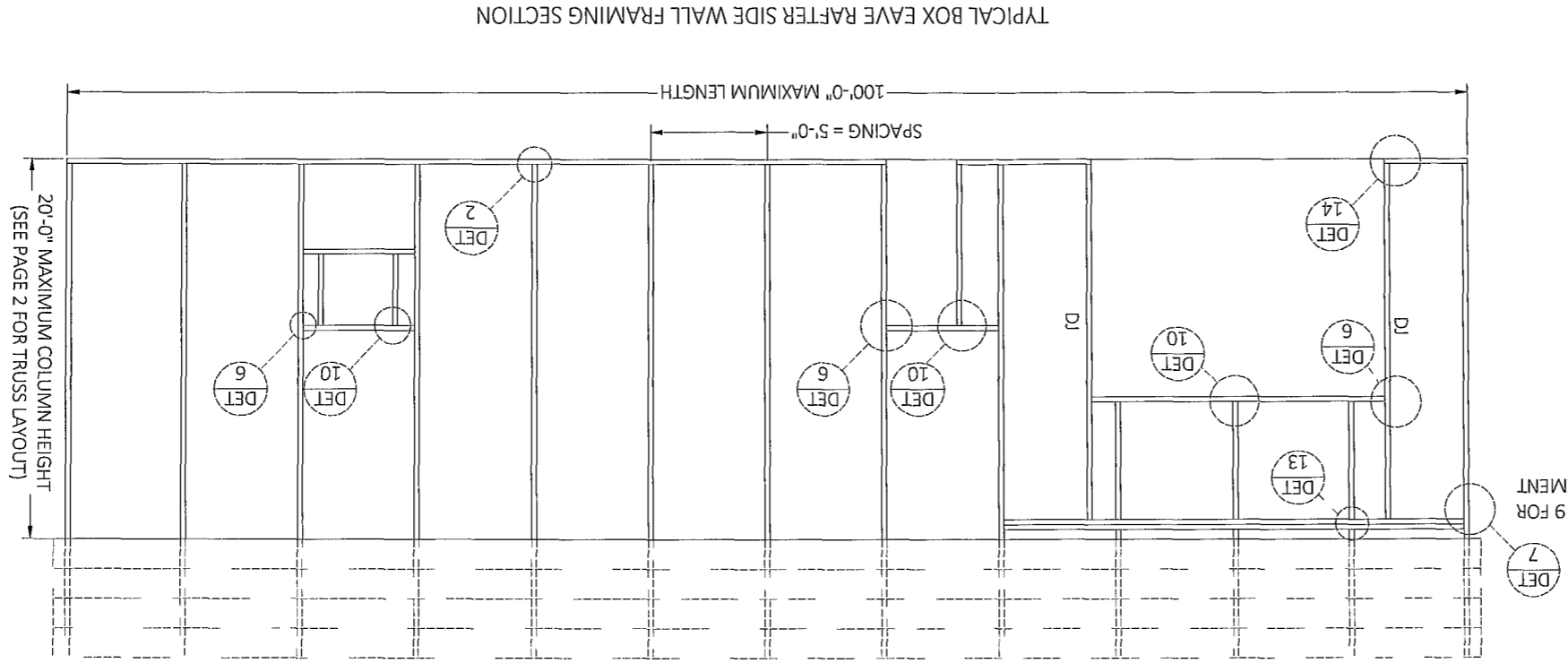
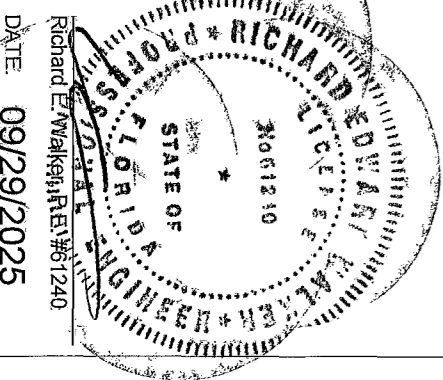
PROJECT ADDRESS
 12-30' WIDE ENCLOSED
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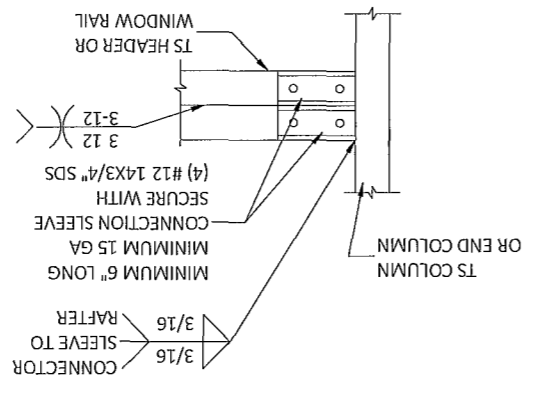
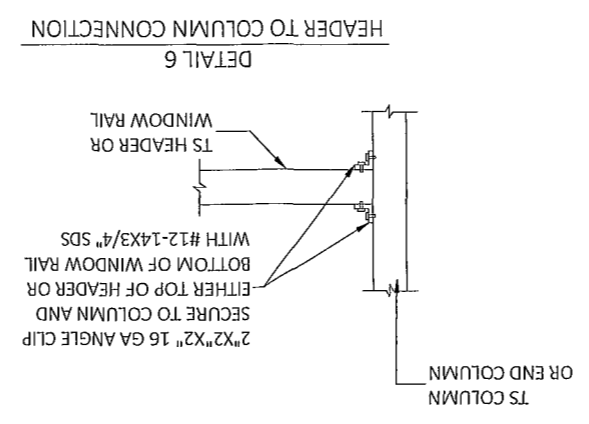
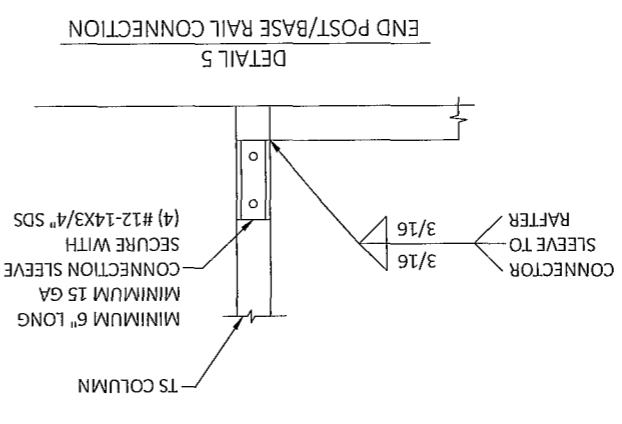
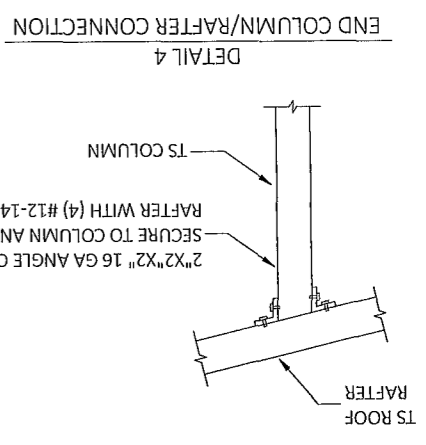
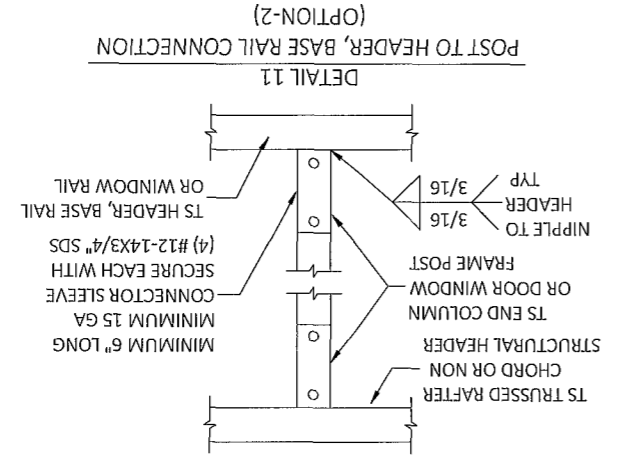
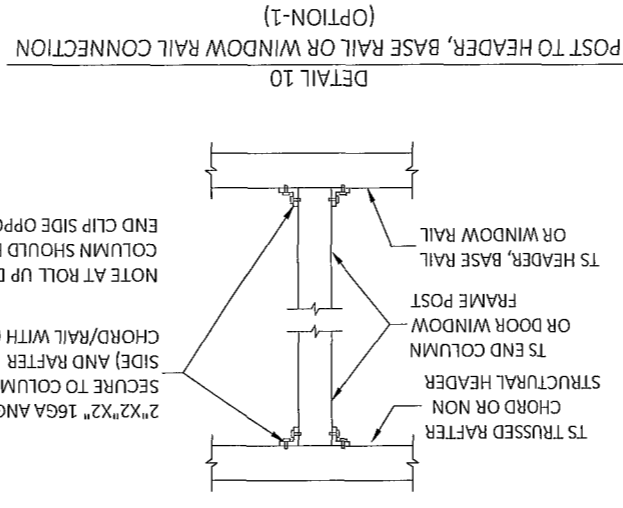
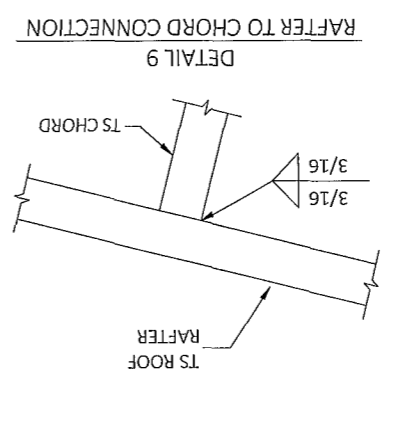
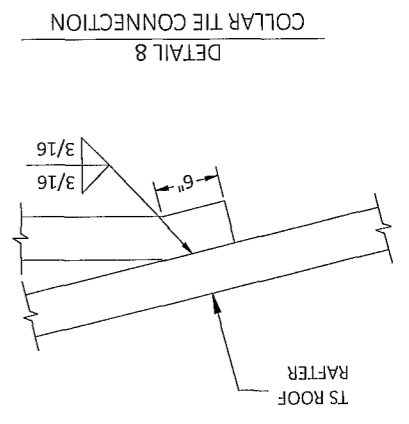
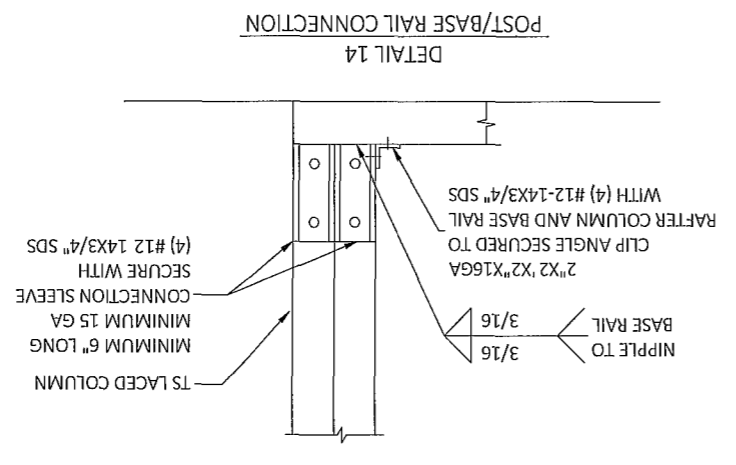
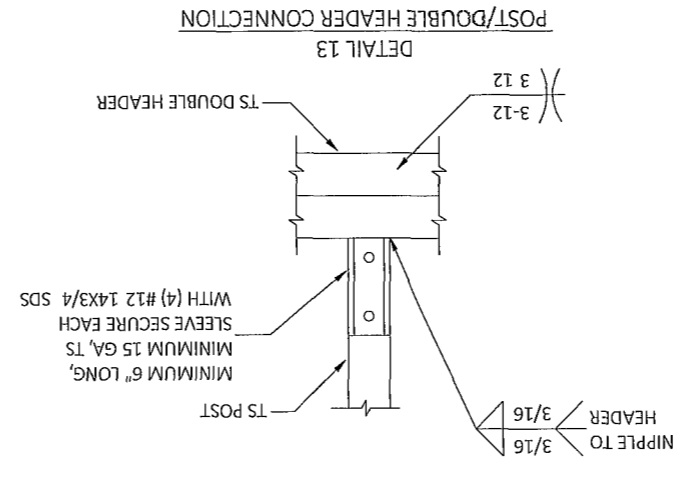
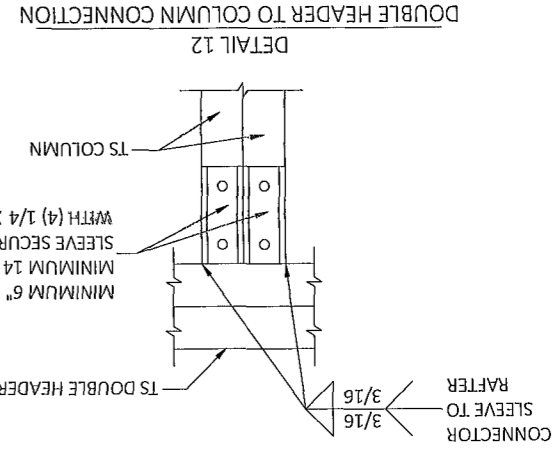
PROJECT NO. 2526885

CA CERT #30782



*SEE PAGE 9 FOR
 HEADER REQUIREMENT

6	OF 12
SHEET	DATE
REVISION 2	DATE
REVISION 1	DATE
DESIGN DATE	09/26/2025
CONTRACTOR	
STEEL BUILDINGS AND STRUCTURES INC. 800PIEDMONT TRIAD WEST DR., MOUNT AIRY, NC 27030	
PROJECT ADDRESS	
12-30' WIDE ENCLOSED GENERIC PLANS	
SCALE	
NTS	
DRAWN BY JS	

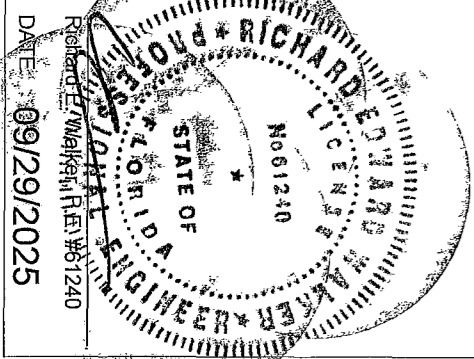


CONNECTION DETAILS

FLORIDA ENGINEERING LLC
 4161 TAMiami TRAIL, UNIT 101
 PORT CHARLOTTE, FLORIDA 33952
 (941) 391-5980
 FILEng.com
 Orders@FILEng.com

PROJECT NO. 2526885

CA CERT #30782

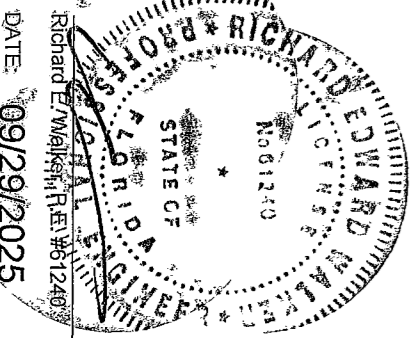


DATE 09/29/2025

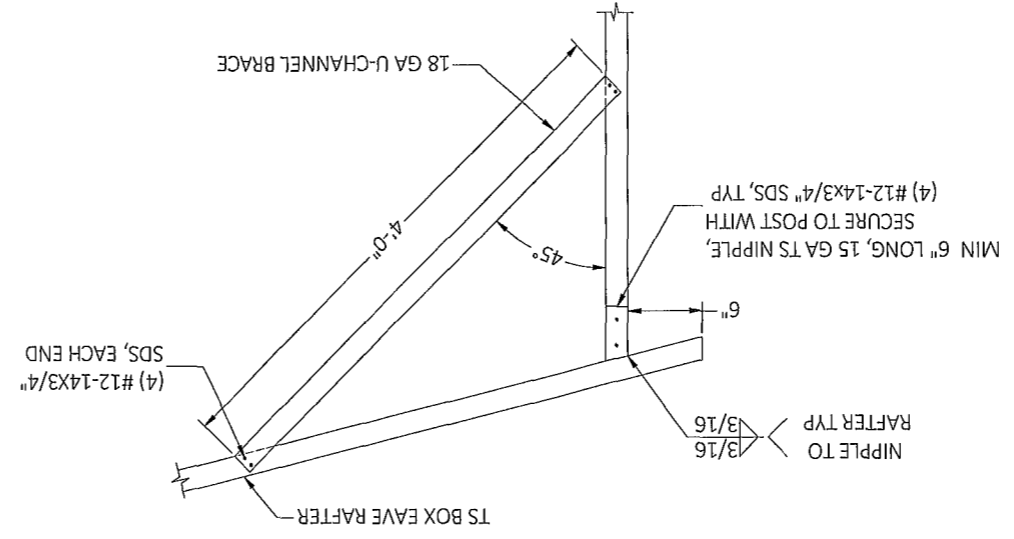
SCALE	NTS
DRAWN BY	JS
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REVISION 1	DATE
DESIGN DATE	09/26/2025

CONTRACTOR
STEEL BUILDINGS AND STRUCTURES INC.
 800 PIEDMONT TRIAD WEST DR.,
 MOUNT AIRY, NC 27030
 PROJECT ADDRESS
 12'-30" WIDE ENCLOSED
 GENERIC PLANS

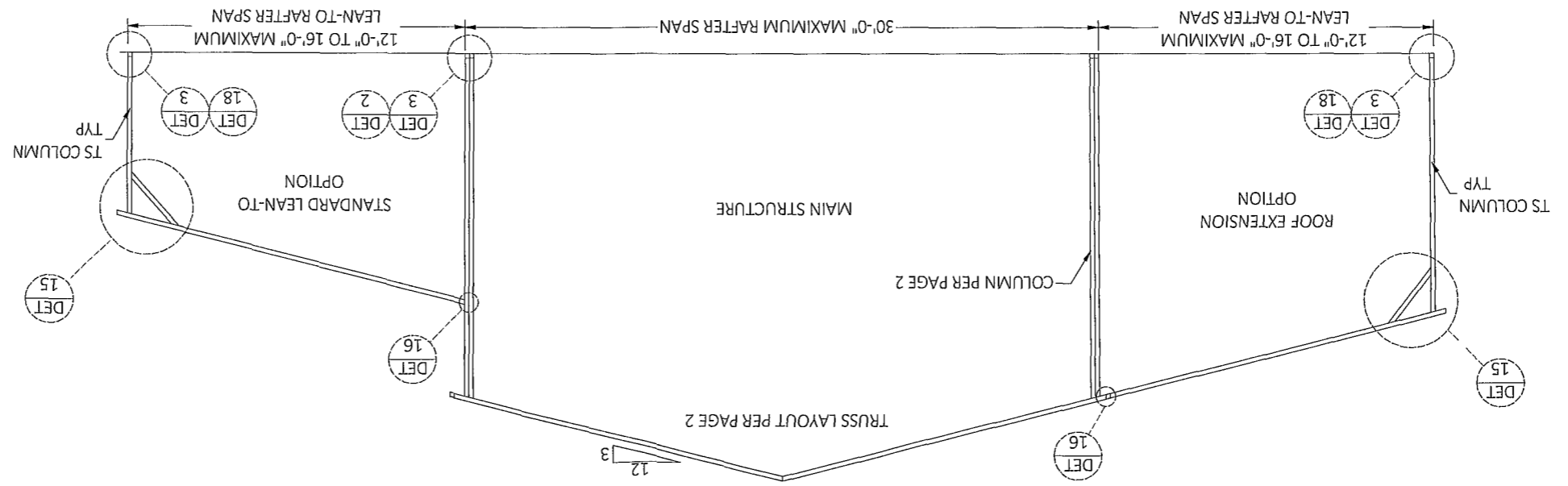
FILE
FLORIDA ENGINEERING LLC
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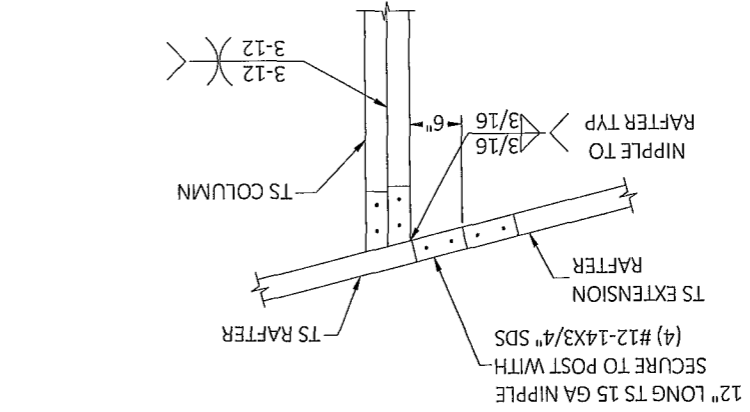
LEAN-TO RAFTER/CORNER POST CONNECTION
 DETAIL 15



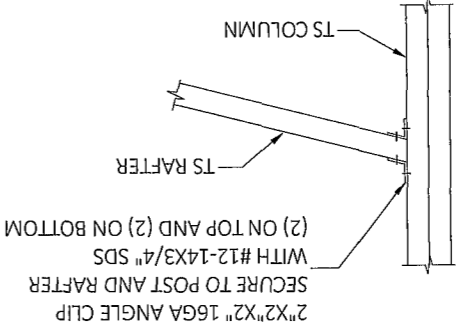
TYPICAL BOX EAVE RAFTER LEAN-TO OPTIONS FRAMING SECTION



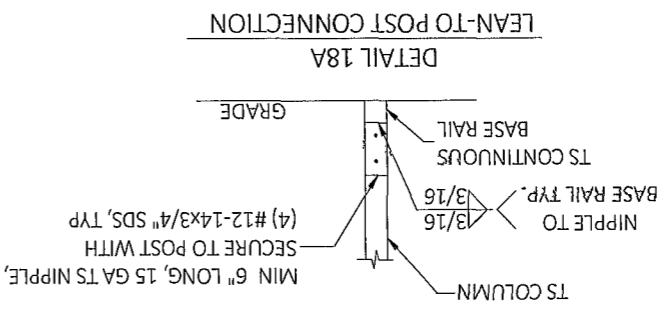
CONNECTION DETAILS



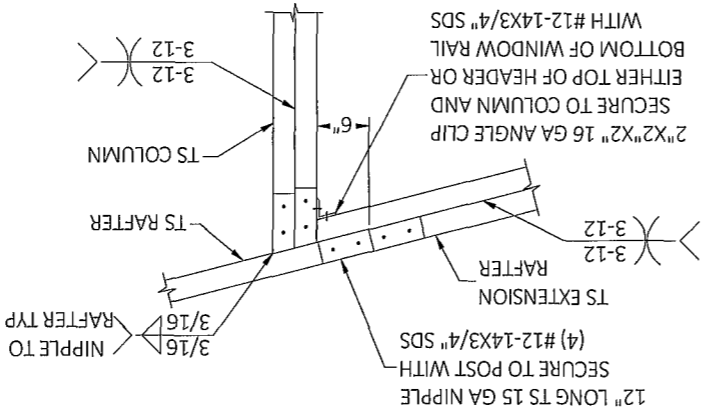
DETAIL 16A
 SIDE EXTENSION RAFTER/COLUMN CONNECTION
 FOR RAFTER SPANS LESS THAN 12'-0"



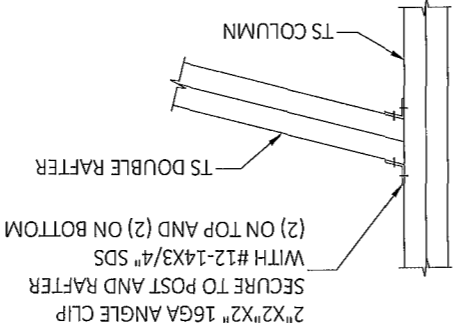
DETAIL 17A
 LEAN TO RAFTER/COLUMN CONNECTION
 FOR RAFTER SPANS LESS THAN 12'-0"



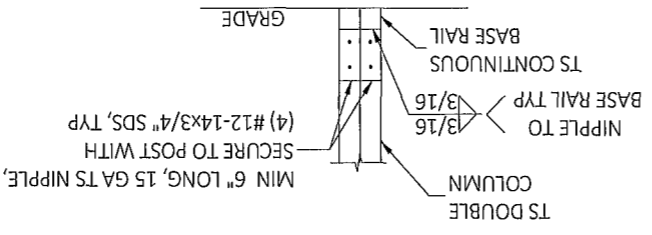
DETAIL 18A
 LEAN-TO POST CONNECTION



DETAIL 16B
 SIDE EXTENSION RAFTER/COLUMN CONNECTION
 FOR RAFTER SPANS BETWEEN 12'-0" AND 16'-0"



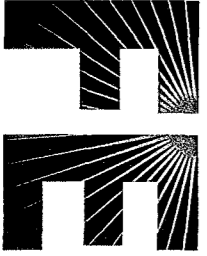
DETAIL 17B
 LEAN TO RAFTER/COLUMN CONNECTION
 FOR RAFTER SPANS BETWEEN 12'-0" AND 16'-0"



DETAIL 18B
 LEAN-TO DOUBLE POST CONNECTION

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REVISION 1	DATE
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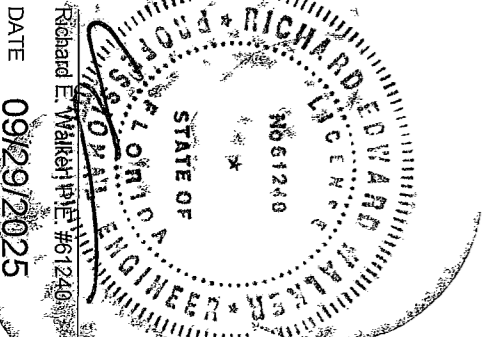
CONTRACTOR:
STEEL BUILDINGS AND STRUCTURES INC.
 800PIEDMONT TRIAD WEST DR.,
 MOUNT AIRY, NC 27030
 PROJECT ADDRESS
 12'-30" WIDE ENCLOSED
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PROJECT NO. 2526885

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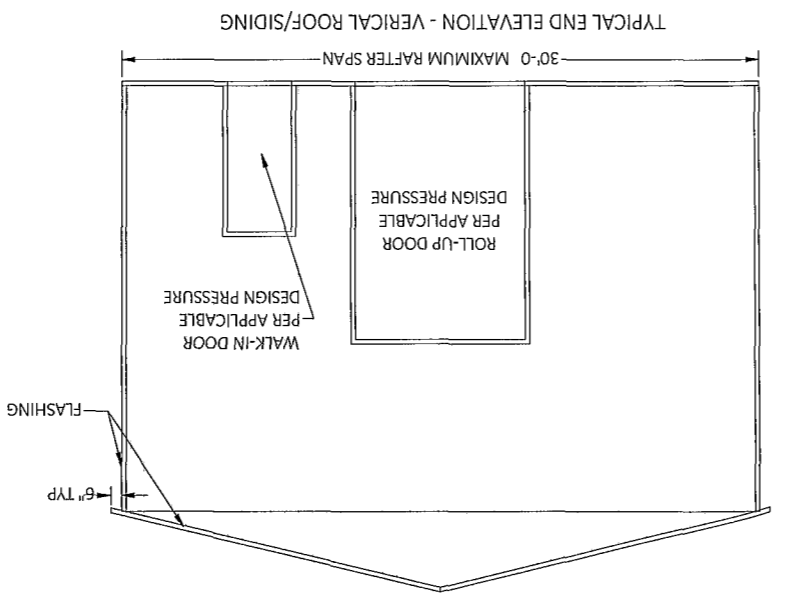
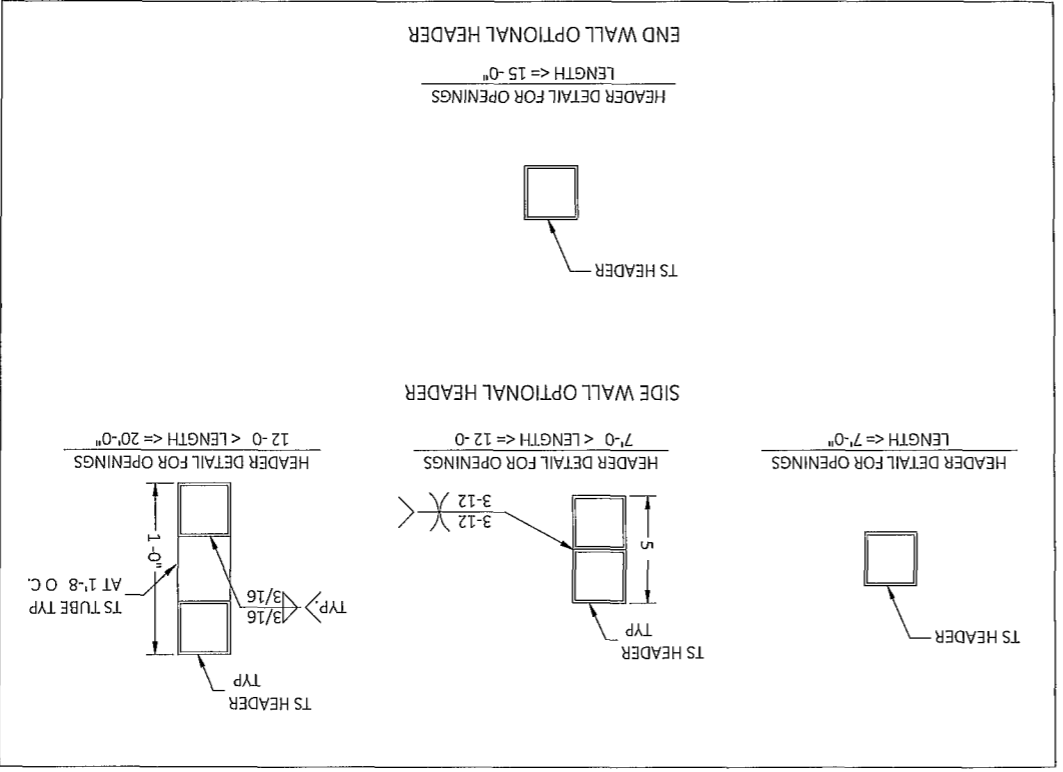
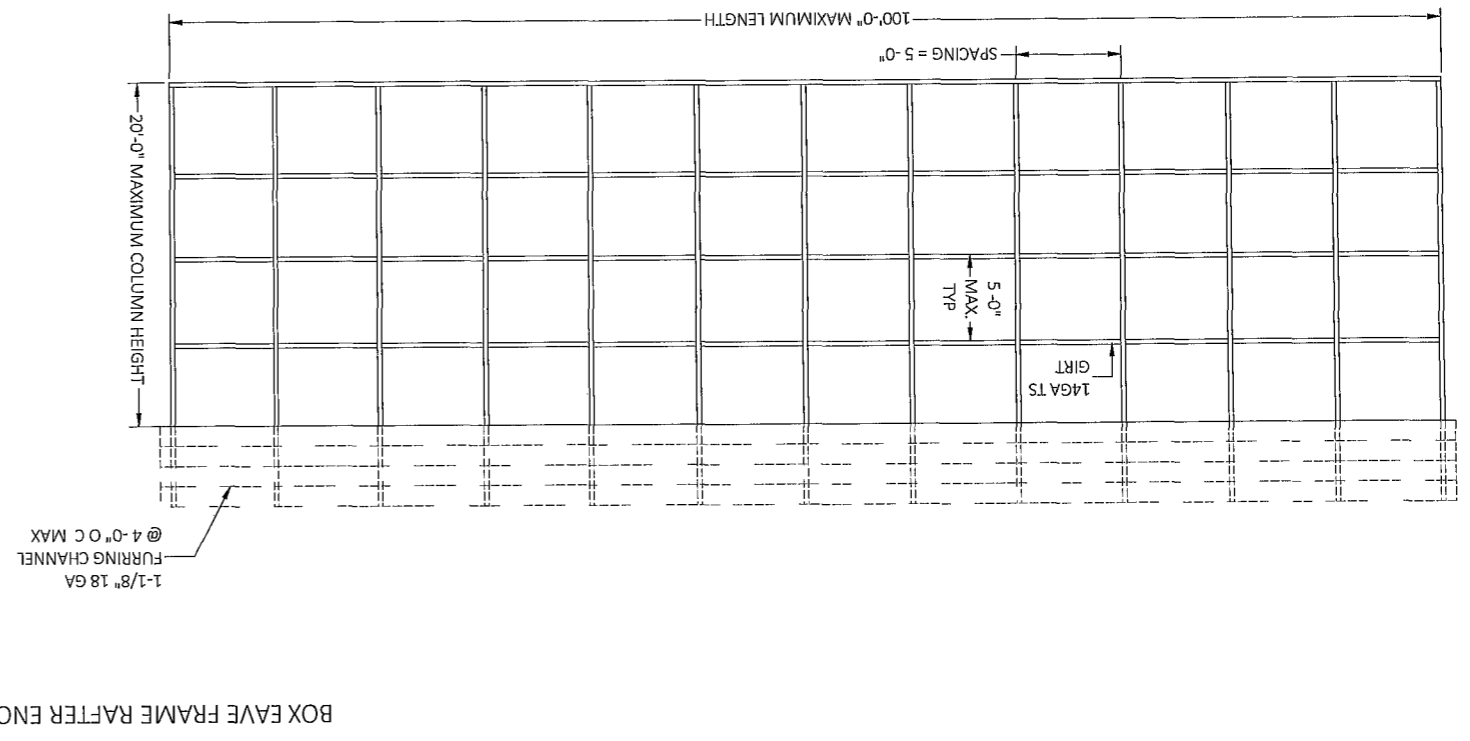
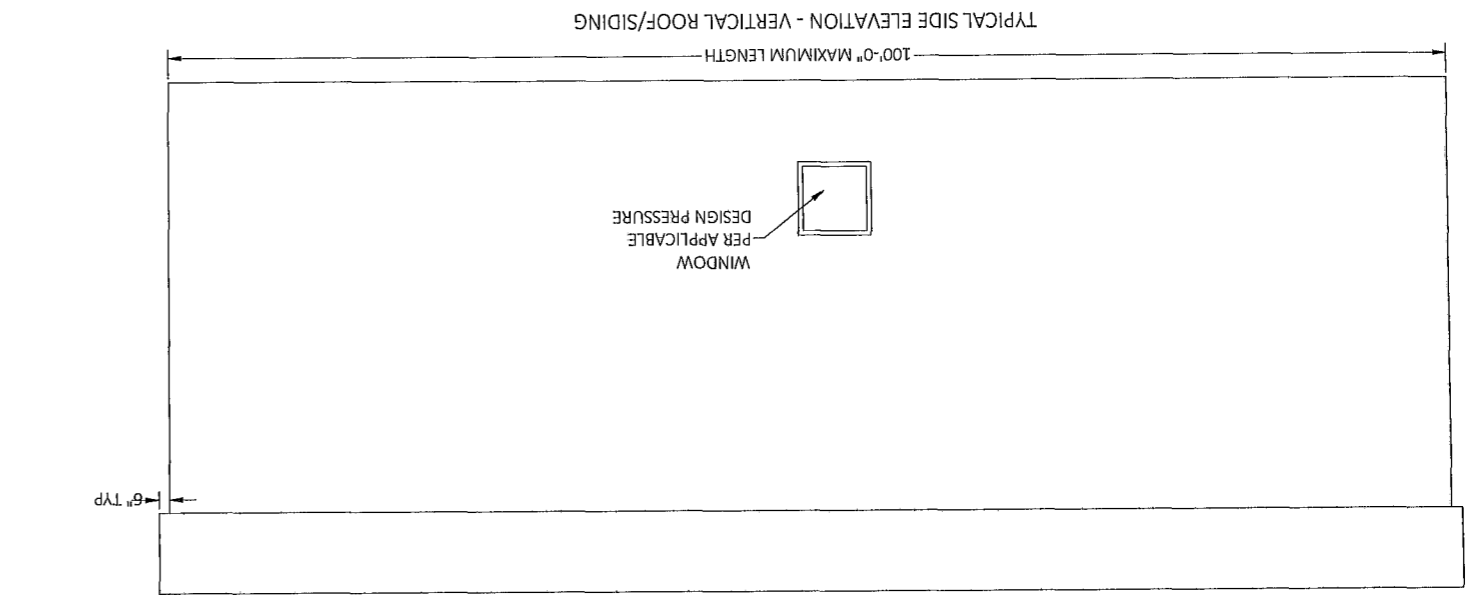


SCALE	NTS
DRAWN BY	JS
REVISION 2	DATE
REVISION 1	DATE
DESIGN DATE	09/28/2025

CONTRACTOR
STEEL BUILDINGS AND STRUCTURES INC.
 800 PIEDMONT TRIAD WEST DR.,
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 GENERIC PLANS

FL FLORIDA ENGINEERING LLC
 4161 TAMAMI TRAIL, UNIT 101
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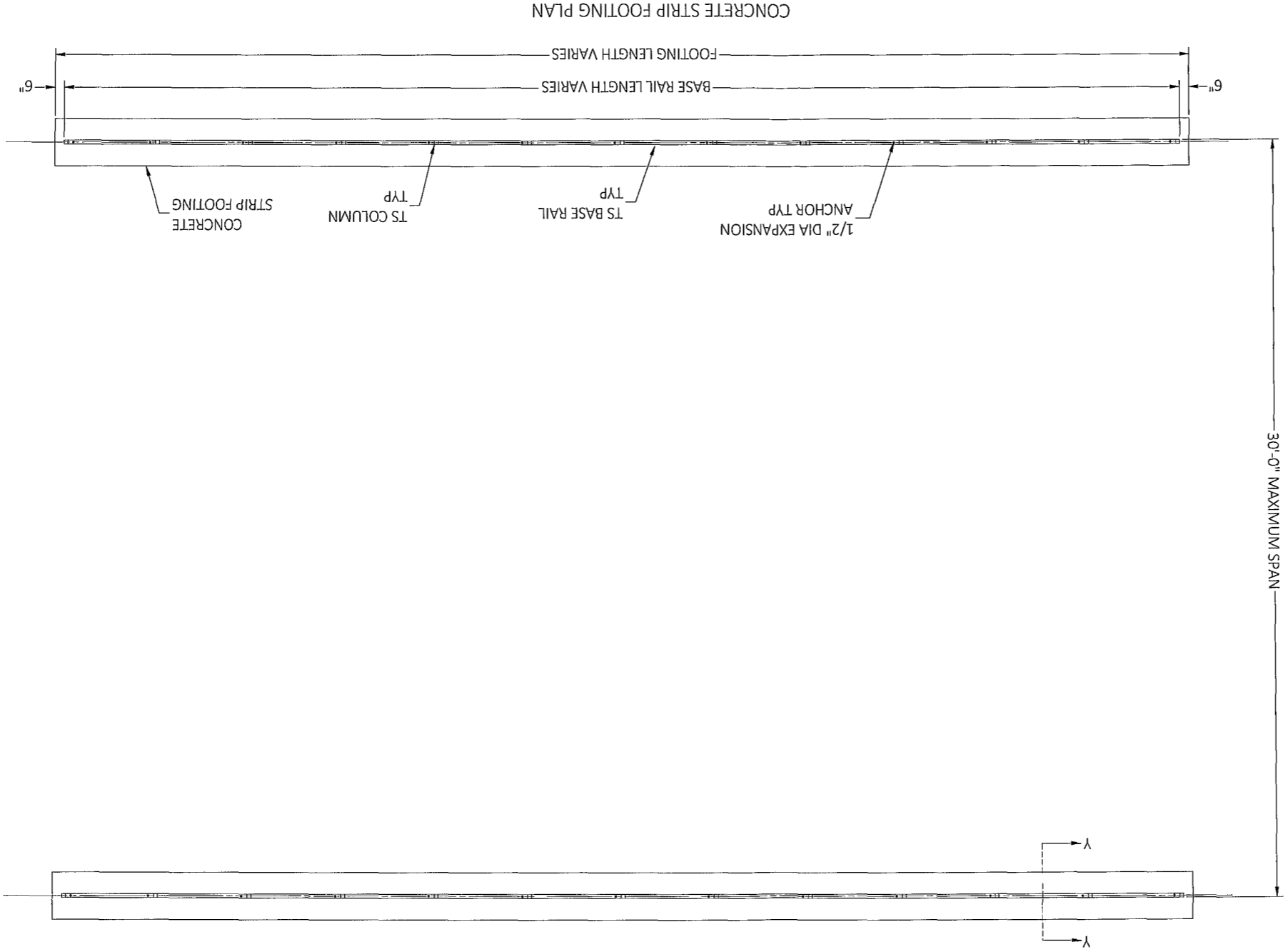
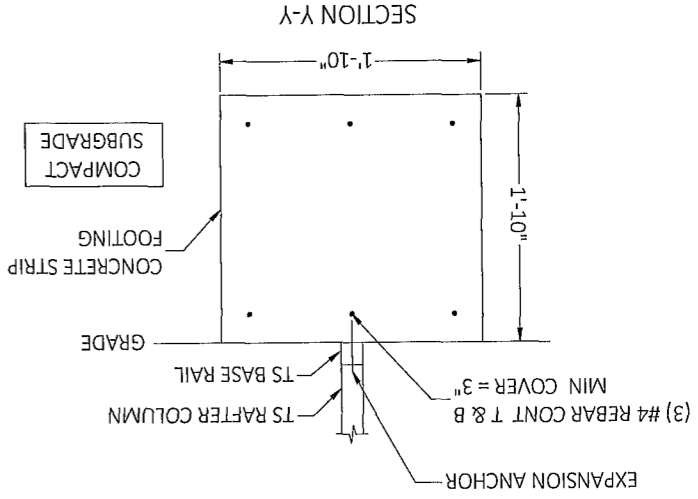
PROJECT NO. 2526885
 CA CERT. #30782
 DATE 09/29/2025
 Richard E. Walker, P.E. #61240



GENERAL NOTES
 CONCRETE MONOLITHIC SLAB DESIGN IS BASED ON A MINIMUM SOIL BEARING CAPACITY OF 2500 PSF

CONCRETE
 MINIMUM 28-DAY SPECIFIED COMPRESSIVE STRENGTH = 3000 PSI

- REINFORCING STEEL
1. TURNDOWN REINFORCING STEEL = ASTM A615 GRADE 60
 2. SLAB REINFORCEMENT = WELDED WIRE FABRIC PER ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT
 3. REINFORCING STEEL COVER = 3" WHERE CASE AGAINST AND PERMANENTLY EXPOSED TO SOIL OR WATER, 1.5" EVERYWHERE ELSE
 4. REINFORCEMENT IS BENT COLD
 5. MINIMUM INSIDE DIAMETER OF BEND = (6) BAR DIAMETERS
 6. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT



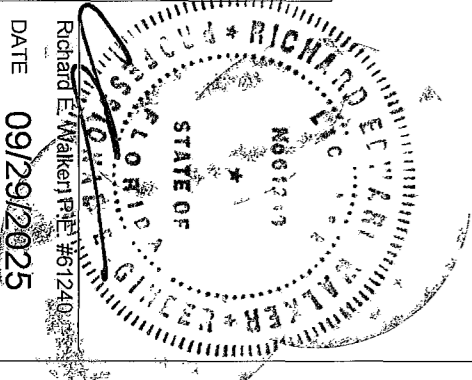
OPTIONAL CONCRETE STRIP FOOTING

SCALE	NTS
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REVISION 2	DATE
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CONTRACTOR
STEEL BUILDINGS AND STRUCTURES INC.
 800PIEDMONT TRIAD WEST DR.,
 MOUNT AIRY, NC 27030
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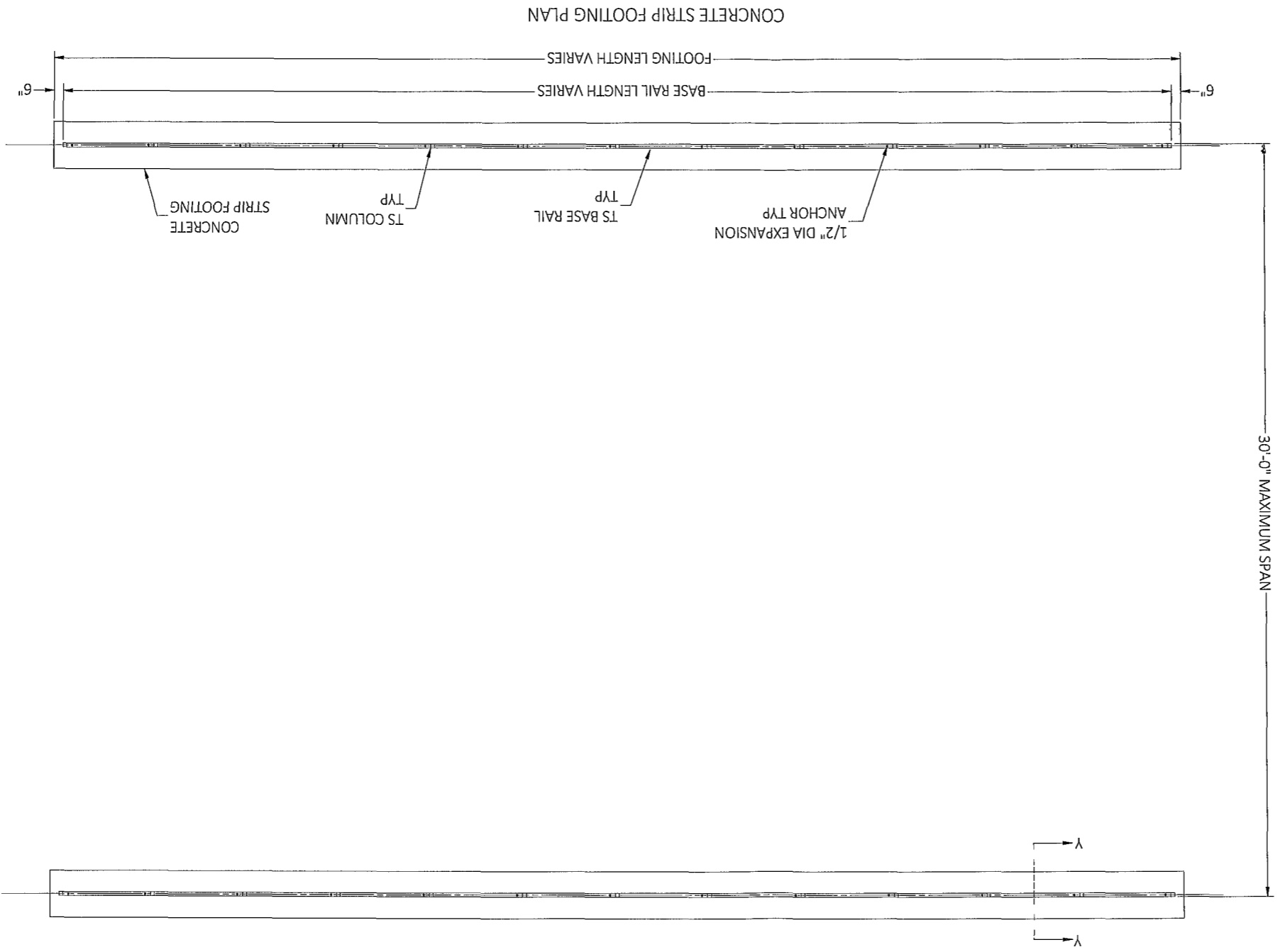
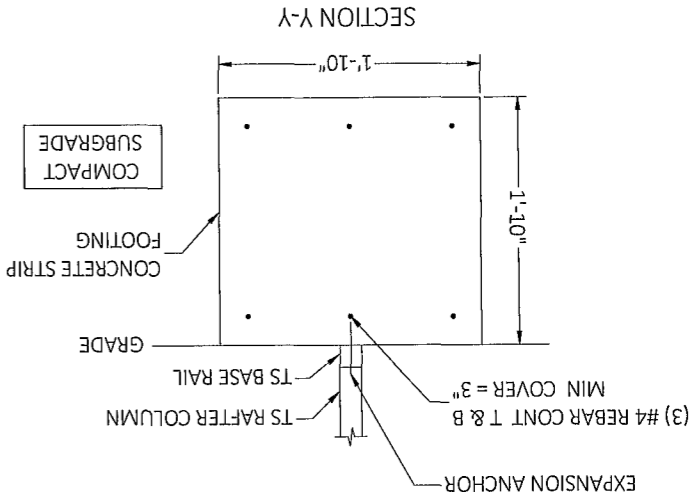
PROJECT NO. 2526885
 CA CERT #30782



GENERAL NOTES
 CONCRETE MONOLITHIC SLAB DESIGN IS BASED ON A MINIMUM SOIL BEARING CAPACITY OF 2500 PSF

CONCRETE
 MINIMUM 28-DAY SPECIFIED COMPRESSIVE STRENGTH = 3000 PSI

- REINFORCING STEEL
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 - 3 REINFORCING STEEL COVER = 3" WHERE CASE AGAINST AND PERMANENTLY EXPOSED TO SOIL OR WATER, 1.5" EVERYWHERE ELSE
 - 4 REINFORCEMENT IS BENT COLD
 - 5 MINIMUM INSIDE DIAMETER OF BEND = (6) BAR DIAMETERS
 - 6 REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT

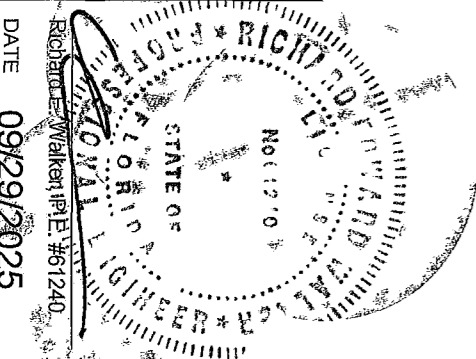


OPTIONAL CONCRETE STRIP FOOTING

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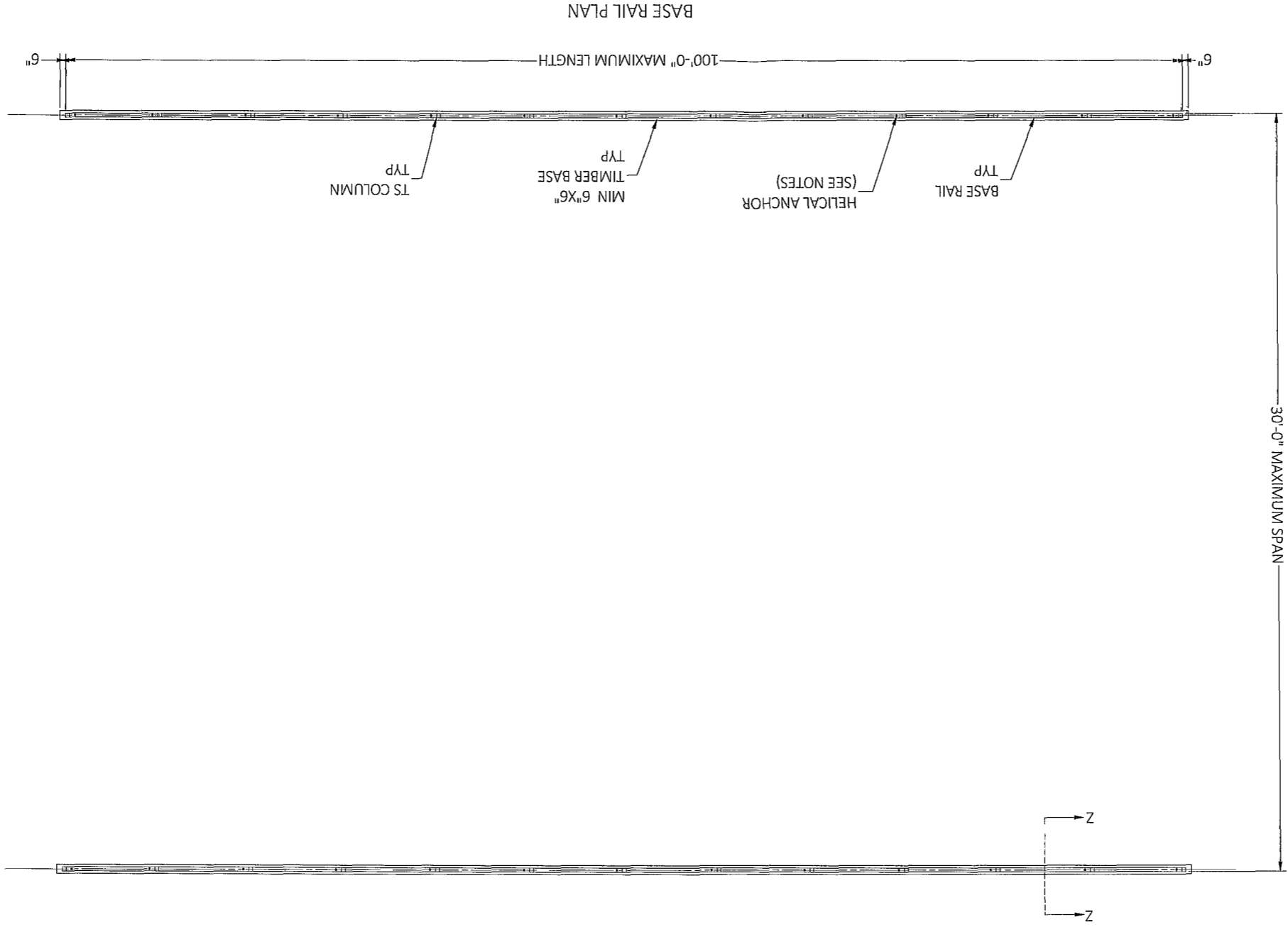
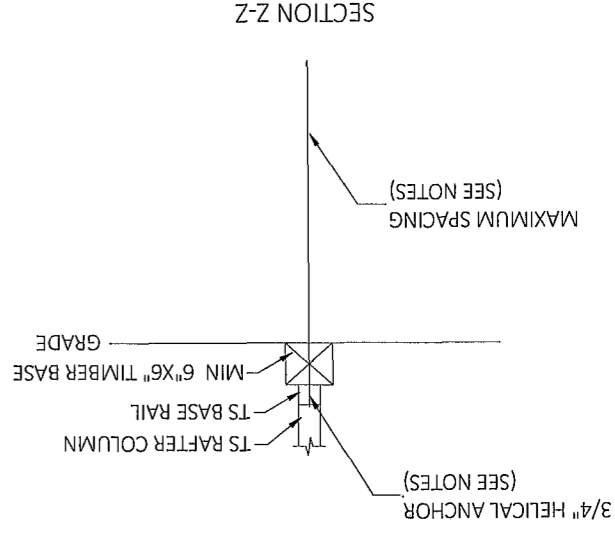


TIMBER NOTES
 1 TIMBER BASE TO BE NO. 2 SYP PT OR EQUIVALENT

HELIX ANCHOR NOTES.
 1 FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CALICHE, PRELOADED SILTS AND CLAYS, CORALS, MEDIUM DENSE COARSE SANDS, SANDY GRAVELS, VERY STIFF SILTS AND CLAYS, MEDIUM TO VERY LOOSE DENSE SANDS, FIRM TO STIFF CLAYS AND SILTS, ALLUVIAL FILL, USE MINIMUM (2) 4" HELICES WITH MINIMUM 30" EMBEDMENT INSTALLED AT EVERY POST (LEG) / MAX RAFTER SPACING
 2 THE UPLIFT/BEARING CAPACITY OF HELICAL ANCHOR MUST BE EQUAL TO OR GREATER THAN 8 KIPS FOR ANCHORS INSTALLED AT EVERY POST (LEG) / MAX RAFTER SPACING
 3 THE UPLIFT/BEARING CAPACITY OF HELICAL ANCHORS MUST BE AS SHOWN IN TABLE A FOR ANCHORS PROVIDED AT THE JAMBS OF DOOR OPENINGS THE INCREASE IN HELICAL ANCHOR CAPACITY MAY BE ACHIEVED BY INCREASING THE DIAMETER AND/OR THE EMBEDMENT OF THE ANCHORS, OR BY USING DIFFERENT ANCHORS DEPENDING ON THE MANUFACTURER'S SPECIFICATIONS

TABLE A

REQUIRED UPLIFT / BEARING CAPACITY OF HELICAL ANCHORS		RAFTER SPACING (FT)
4	5	
6	11.0	9.5
8	13.0	11.5
10	15.0	13.0
12	17.0	14.5
14	19.5	16.5
16	21.5	18.0
18	23.5	20.0
20	25.5	21.5



OPTIONAL HELICAL ANCHORING ON TIMBER BEAM DETAIL

SCALE NTS
 DRAWN BY JS
 REVISION 2 DATE
 REVISION 1 DATE
 DESIGN DATE 09/26/2025
 CONTRACTOR
 STEEL BUILDINGS AND STRUCTURES INC.
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