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Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-6484C
Job Description: Dupree-Donny & Missy Dupree	
Address: Lake City, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.01.01A JRef #: 1XcQ2150002
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 13 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	033.22.1533.01417	A01
3	033.22.1532.56440	A2
5	033.22.1532.53203	B02
7	033.22.1532.48743	B04
9	033.22.1532.19030	B06
11	033.22.1532.12257	PB02
13	033.22.1532.07760	PB04
15	GBLLETIN0118	
17	BRCLBSUB0119	

Item	Drawing Number	Truss
2	033.22.1532.57913	A02
4	033.22.1532.54940	B01
6	033.22.1532.50797	B03
8	033.22.1532.46593	B05
10	033.22.1532.14890	PB01
12	033.22.1532.10117	PB03
14	A14015ENC160118	
16	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

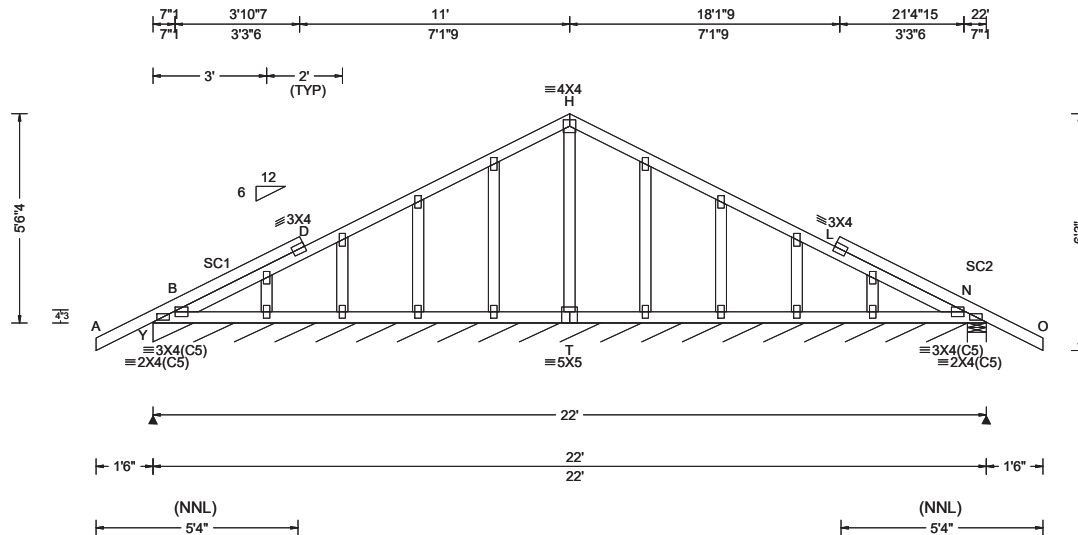
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 639375 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 21-6484C Dupree-Donny & Missy Dupree Truss Label: A01	Cust: R215 JRef: 1XcQ2150002 T2 DrwNo: 033.22.1533.01417 KD / YK 02/02/2022
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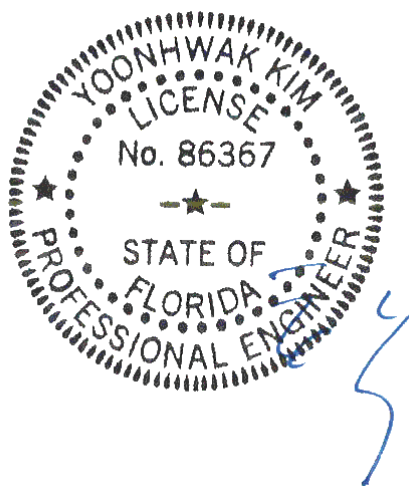
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 L 999 240 VERT(CL): 0.004 L 999 180 HORZ(LL): 0.001 L - - HORZ(TL): 0.002 L - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.041 Max Web CSI: 0.058 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Y* 80 /- /- /45 /- /4 N 290 /- /- /210 /65 /- Wind reactions based on MWFRS Y Brg Wid = 258 Min Req = - N Brg Wid = 6.0 Min Req = 1.5 Bearings Y & N are a rigid surface. Members not listed have forces less than 375#
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

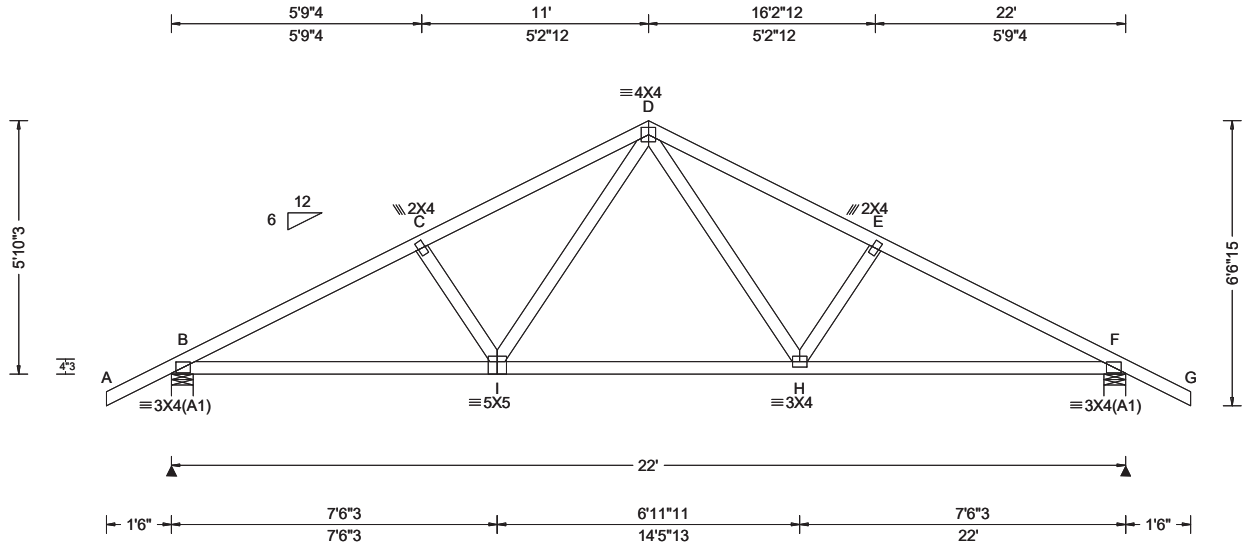
Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 5-6-4.



FL REG# 278, Yoonhwak Kim, FL PE #86367
02/02/2022

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCcp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.049 H 999 240 VERT(CL): 0.100 H 999 180 HORZ(LL): 0.020 F - - HORZ(TL): 0.039 F - - Creep Factor: 2.0 Max TC CSI: 0.288 Max BC CSI: 0.554 Max Web CSI: 0.181 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1006 /- /- /607 /180 /180 F 1006 /- /- /607 /180 /- Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 1.5 F Brg Wid = 6.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 646 - 1510 D - E 648 - 1336 C - D 649 - 1335 E - F 646 - 1511 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - I 1288 - 474 H - F 1289 - 450 I - H 874 - 221 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. I - D 474 - 196 D - H 475 - 195
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Lumber

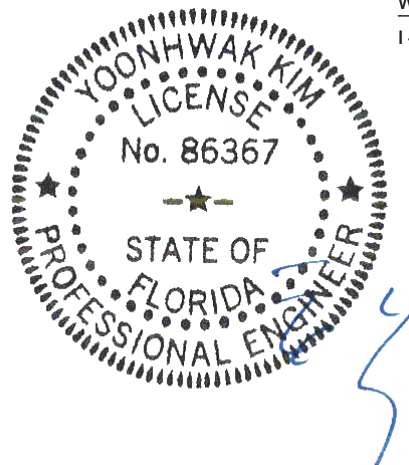
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-10-3.

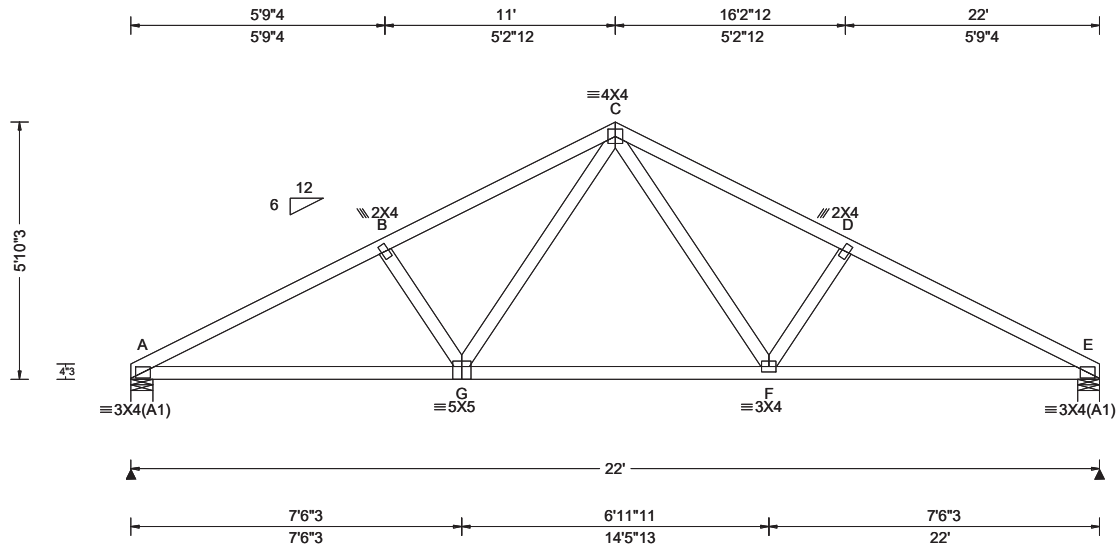


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SEQN: 639381 FROM: CDM	COMN Ply: 1 Qty: 1	Job Number: 21-6484C Dupree-Donny & Missy Dupree Truss Label: A2	Cust: R215 JRef: 1XcQ2150002 T3 DrwNo: 033.22.1532.56440 KD / YK 02/02/2022
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BC DL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.048 F 999 240 VERT(CL): 0.098 F 999 180 HORZ(LL): 0.019 E - - HORZ(TL): 0.040 E - - Creep Factor: 2.0 Max TC CSI: 0.320 Max BC CSI: 0.566 Max Web CSI: 0.189 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 906 /- /- /522 /15 /141 E 906 /- /- /522 /15 /- Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 1.5 E Brg Wid = 6.0 Min Req = 1.5 Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 463 -1544 C - D 469 -1368 B - C 469 -1367 D - E 462 -1544					
				Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - G 1323 -353 F - E 1324 -340 G - F 891 -162 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - C 495 -139 C - F 496 -138					

Lumber

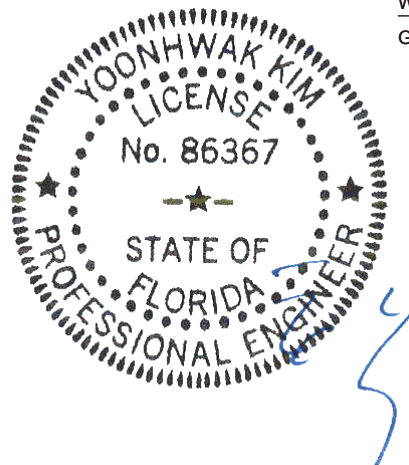
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

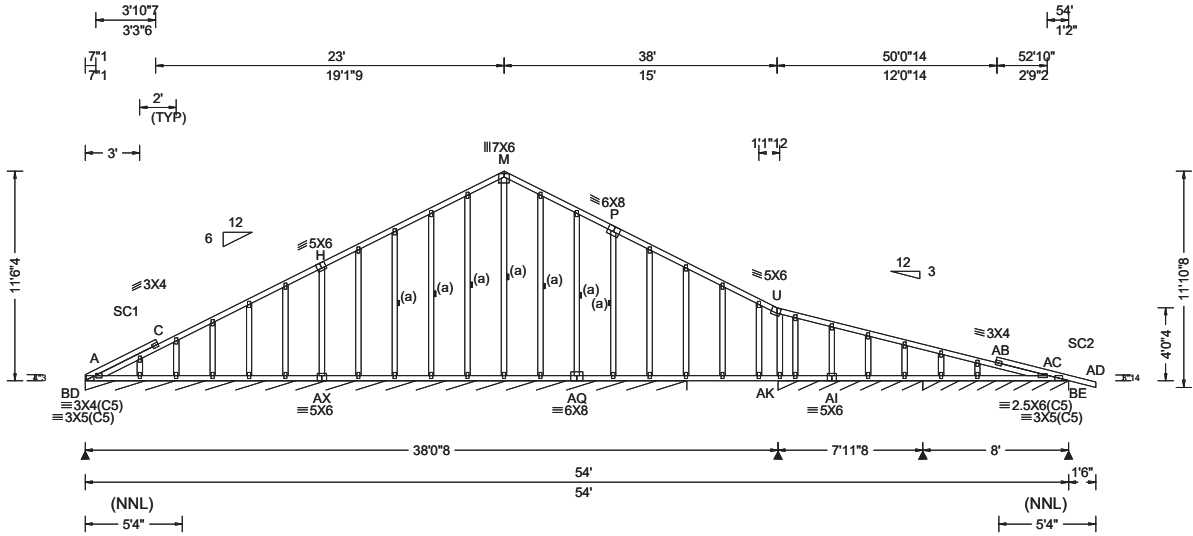
The overall height of this truss excluding overhang is 5-10-3.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.40 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.016 S 999 240 VERT(CL): 0.033 S 999 180 HORZ(LL): -0.007 S - - HORZ(TL): 0.015 S - - Creep Factor: 2.0 Max TC CSI: 0.254 Max BC CSI: 0.155 Max Web CSI: 0.194 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL BD*89 - / - / - /68 /11 /11 AK*107 - / - / - /57 /- /- BE*92 - / - / - /45 /- /- Wind reactions based on MWFRS BD Brg Wid = 396 Min Req = - AK Brg Wid = 95.5 Min Req = - BE Brg Wid = 96.0 Min Req = - Bearings BD, AK, & AF are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. H - M 425 -116 M - P 423 -36
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

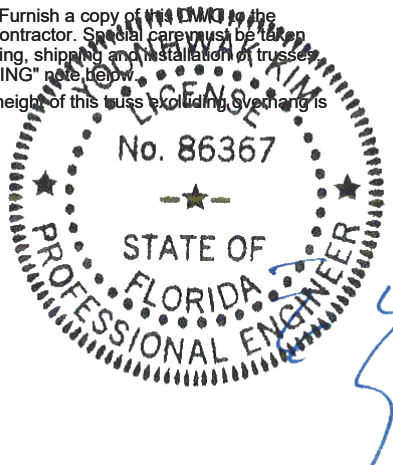
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

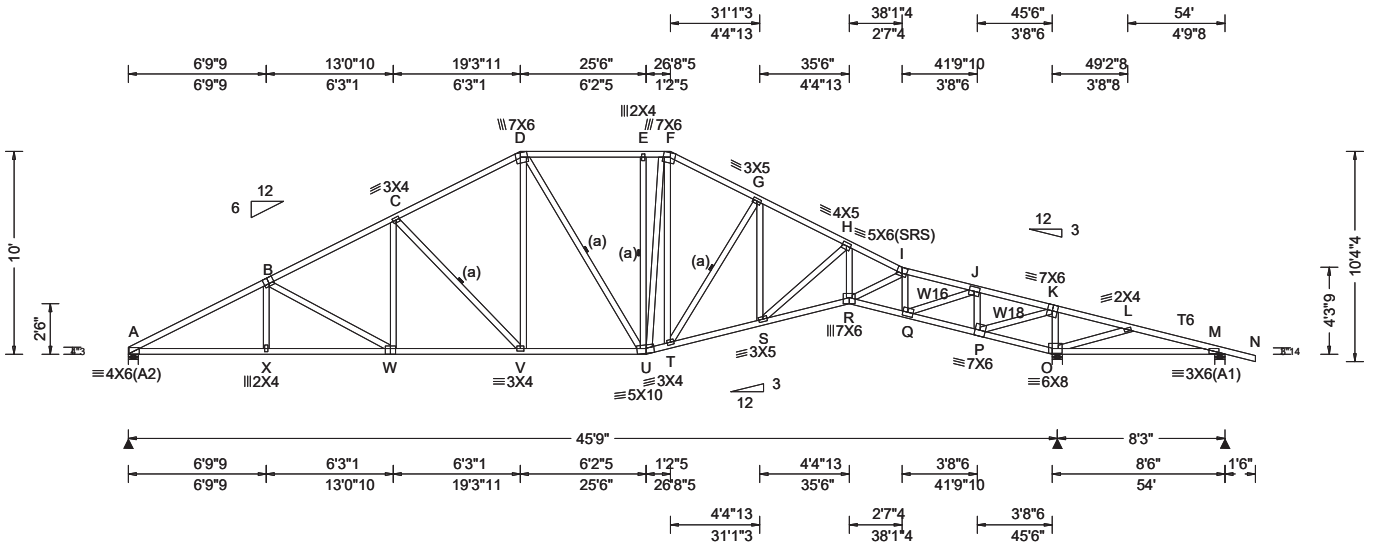
Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
WARNING: Furnish a copy of this drawing to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss including overhang is 11'-6".



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02/02/2022

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Loading Criteria (psf) TCCL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.40 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.284 S 999 240 VERT(CL): 0.557 S 977 180 HORZ(LL): 0.143 O - - HORZ(TL): 0.281 O - - Creep Factor: 2.0 Max TC CSI: 0.670 Max BC CSI: 0.839 Max Web CSI: 0.878 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1850 - / - / - /1052 /299 /258 O 3372 - / - / - /1733 /558 - M - /-684 - / - /50 /284 - Wind reactions based on MWFRS A Brg Wid = 6.0 Min Req = 2.2 O Brg Wid = 6.0 Min Req = 4.0 M Brg Wid = 6.0 Min Req = 1.5 Bearings A, O, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				A - B 1396 -3534 G - H 1298 -2773 B - C 1292 -3012 H - I 1681 -3885 C - D 1161 -2386 I - J 1220 -2687 D - E 1110 -1930 K - L 3612 -1519 E - F 1107 -1926 L - M 3155 -1266 F - G 1163 -2203 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - X 3081 -1207 T - S 2485 -821 X - W 3079 -1209 S - R 3529 -1267 W - V 2602 -954 R - Q 2857 -1095 V - U 2055 -637 P - O 1631 -3651 U - T 1953 -560 O - M 1266 -3027 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - W 306 -533 H - R 1204 -431 W - C 469 -92 R - I 813 -279 C - V 463 -801 I - Q 706 -1642 D - V 844 -270 Q - J 2766 -1079 F - T 686 -352 J - P 928 -1955 T - G 530 -955 P - K 3446 -1541 G - S 781 -244 O - K 997 -2106 S - H 580 -1350 O - L 314 -708					

Lumber
 Top chord: 2x4 SP #2; T6 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W16, W18 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

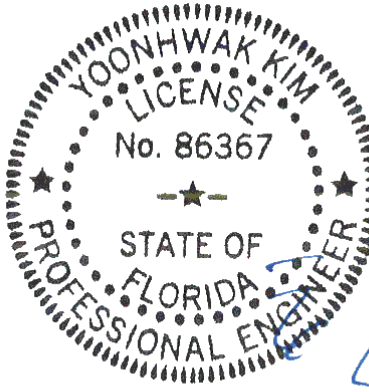
Plating Notes
 All plates are 5X6 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

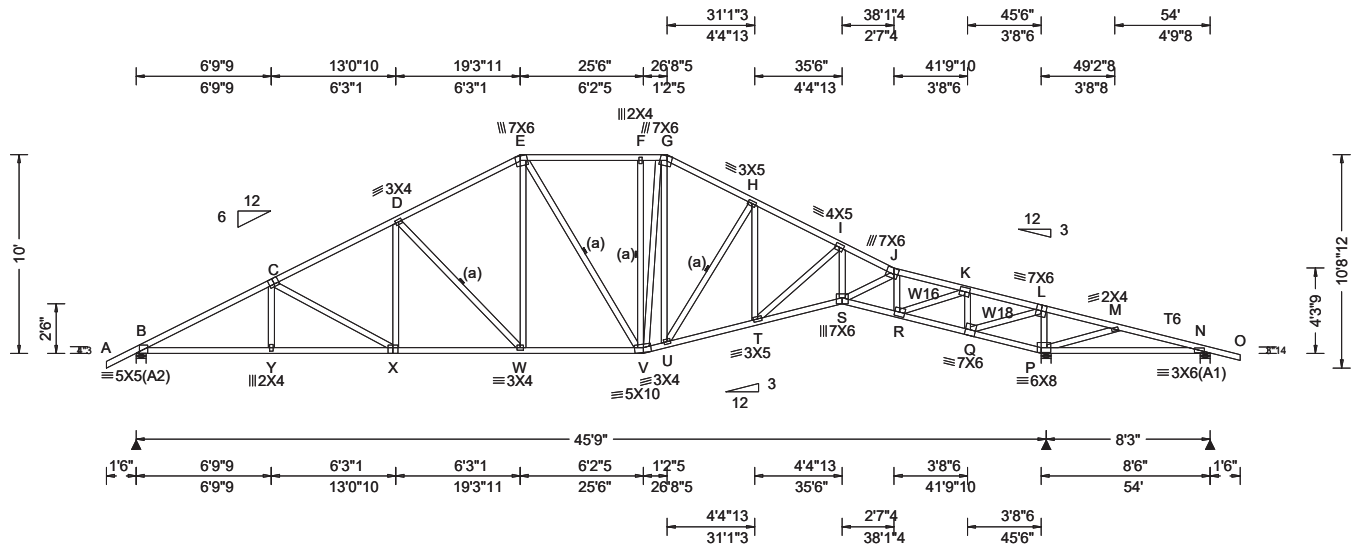
Additional Notes
 Negative reaction(s) of -684# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 10'-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/02/2022

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.284 T 999 240 VERT(CL): 0.557 T 977 180 HORZ(LL): 0.143 P - - HORZ(TL): 0.281 P - - Creep Factor: 2.0 Max TC CSI: 0.591 Max BC CSI: 0.838 Max Web CSI: 0.876 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1952 - / - / - /1140 /58 /284 P 3370 - / - / - /1733 /63 - N - /-683 - / - /284 - Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 2.3 P Brg Wid = 6.0 Min Req = 4.0 N Brg Wid = 6.0 Min Req = 1.5 Bearings B, P, & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				B - C 857 -3506 H - I 848 -2769 C - D 835 -3001 I - J 1057 -3879 D - E 784 -2381 J - K 749 -2683 E - F 760 -1927 L - M 3610 -925 F - G 756 -1922 M - N 3153 -814 G - H 787 -2199 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - Y 3052 -748 U - T 2481 -545 Y - X 3050 -750 T - S 3523 -806 X - W 2593 -616 S - R 2852 -667 W - V 2049 -435 Q - P 1029 -3648 V - U 1950 -387 P - N 812 -3024 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - X 167 -510 I - S 1202 -239 X - D 461 -25 S - J 813 -204 D - W 264 -796 J - R 455 -1640 E - W 840 -125 R - K 2762 -675 G - U 686 -202 K - Q 578 -1953 U - H 313 -954 Q - L 3442 -941 H - T 780 -124 P - L 663 -2104 T - I 346 -1348 P - M 278 -708					

Lumber
 Top chord: 2x4 SP #2; T6 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W16,W18 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

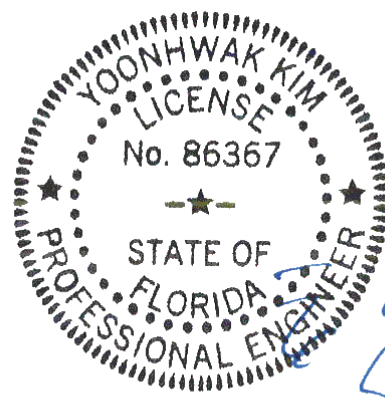
Plating Notes
 All plates are 5X6 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

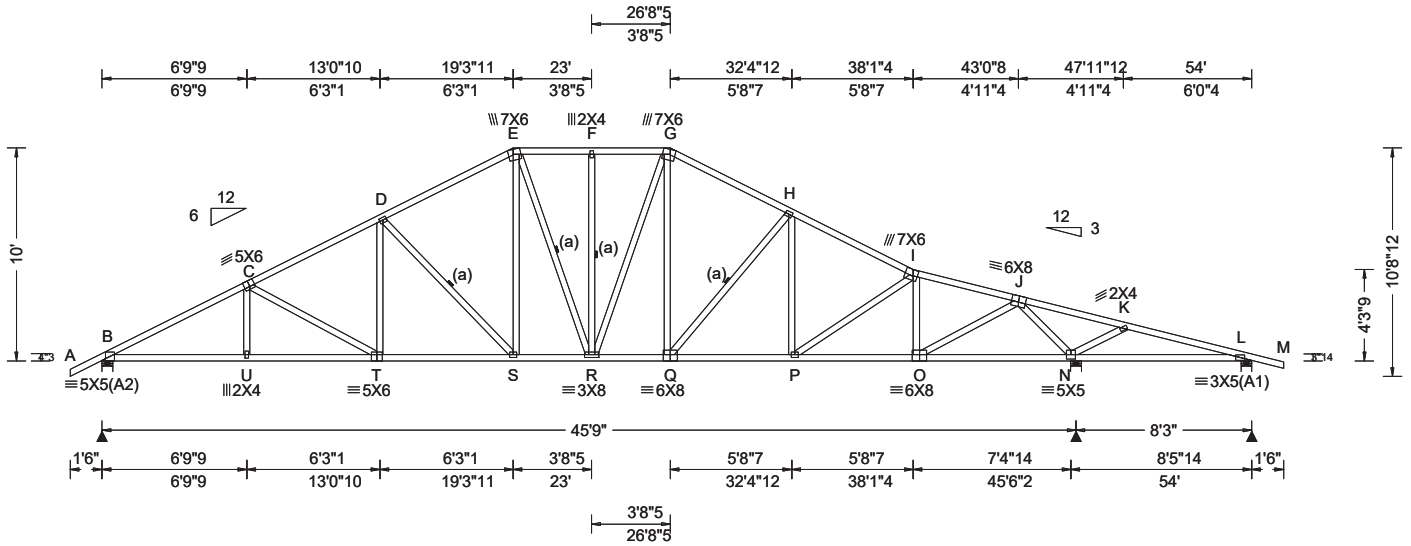
Additional Notes
 Negative reaction(s) of -683# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 10'-0".



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.40 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.219 S 999 240 VERT(CL): 0.428 S 999 180 HORZ(LL): 0.086 N - - HORZ(TL): 0.168 N - - Creep Factor: 2.0 Max TC CSI: 0.853 Max BC CSI: 0.859 Max Web CSI: 0.979 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 2001 - / - / 1181 - / 284 N 2821 - / - / 1442 - / - L 185 - / 219 - / 27 / 98 - Wind reactions based on MWFRS B Brg Wid = 6.0 Min Req = 2.4 N Brg Wid = 6.0 Min Req = 3.3 L Brg Wid = 6.0 Min Req = 1.5 Bearings B, N, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 450 -3609 G - H 377 -2388 C - D 416 -3112 H - I 405 -2741 D - E 369 -2480 I - J 392 -2369 E - F 357 -2137 J - K 1797 -279 F - G 357 -2137 K - L 1469 -185 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - U 3144 -383 Q - P 2380 -166 U - T 3142 -384 P - O 2335 -248 T - S 2693 -241 O - N 424 -45 S - R 2134 -53 N - L 205 -1401 R - Q 2059 -9 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - T 177 -500 Q - H 250 -507 T - D 473 -25 I - O 177 -1002 D - S 276 -818 O - J 2160 -221 E - S 739 -145 J - N 615 -3132 G - Q 527 -141 N - K 264 -532
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

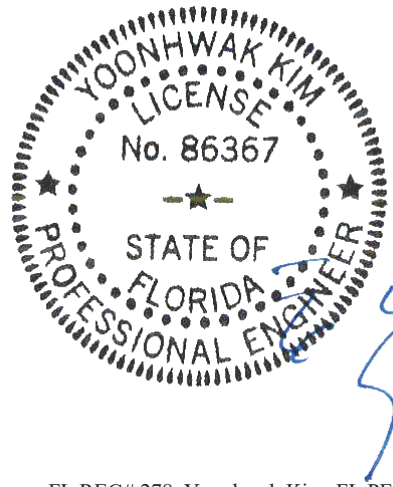
Plating Notes
All plates are 3X4 except as noted.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

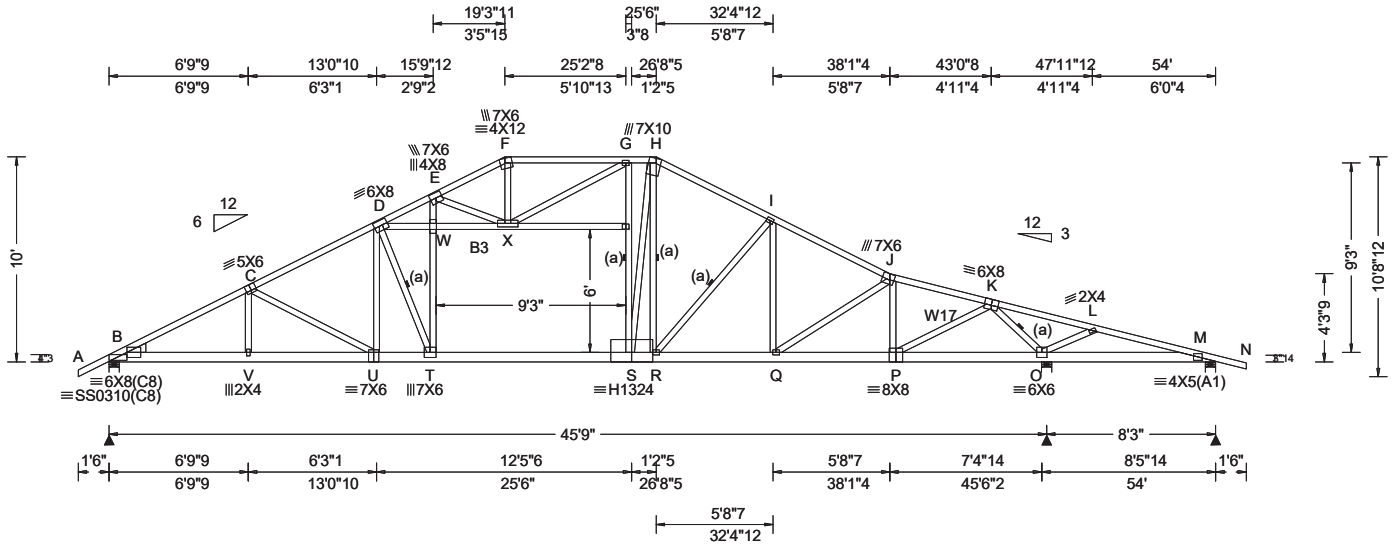
Additional Notes
Negative reaction(s) of -219# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
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The overall height of this truss excluding overhang is 10'-0-0.



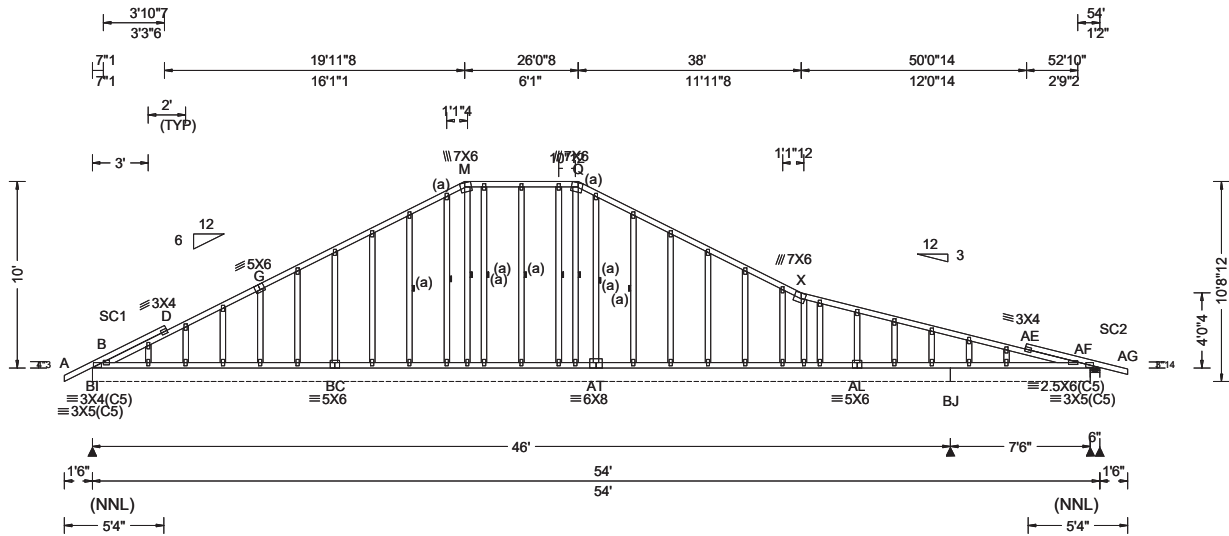
FL REG# 278, Yoonhwak Kim, FL PE #86367
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6750 Forum Drive
 Suite 305
 Orlando FL, 32821



SEQN: 639387 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 21-6484C Dupree-Donny & Missy Dupree Truss Label: B06	Cust: R215 JRef: 1XcQ2150002 T8 DrwNo: 033.22.1532.19030 KD / YK 02/02/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.40 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.006 AE 999 240 VERT(CL): 0.015 AF 999 180 HORZ(LL): 0.004 V - - HORZ(TL): 0.007 V - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.125 Max Web CSI: 0.127 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>BI* 84</td> <td>-</td> <td>-</td> <td>-</td> <td>/45</td> <td>-</td> <td>/5</td> </tr> <tr> <td>BJ* 56</td> <td>-</td> <td>-</td> <td>-</td> <td>/29</td> <td>-</td> <td>-</td> </tr> <tr> <td>AF 313</td> <td>-</td> <td>-</td> <td>-</td> <td>/163</td> <td>/68</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS BI Brg Wid = 552 Min Req = - BJ Brg Wid = 90.0 Min Req = - AF Brg Wid = 6.0 Min Req = 1.5 Bearings BI, BJ, & AF are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	BI* 84	-	-	-	/45	-	/5	BJ* 56	-	-	-	/29	-	-	AF 313	-	-	-	/163	/68	-
Loc	Gravity			Non-Gravity																																		
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BI* 84	-	-	-	/45	-	/5																																
BJ* 56	-	-	-	/29	-	-																																
AF 313	-	-	-	/163	/68	-																																

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

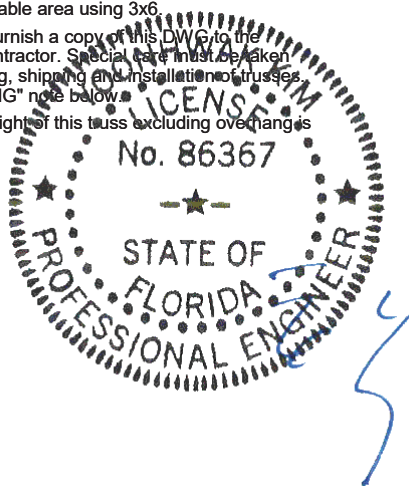
Plating Notes
 All plates are 2X4 except as noted.

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

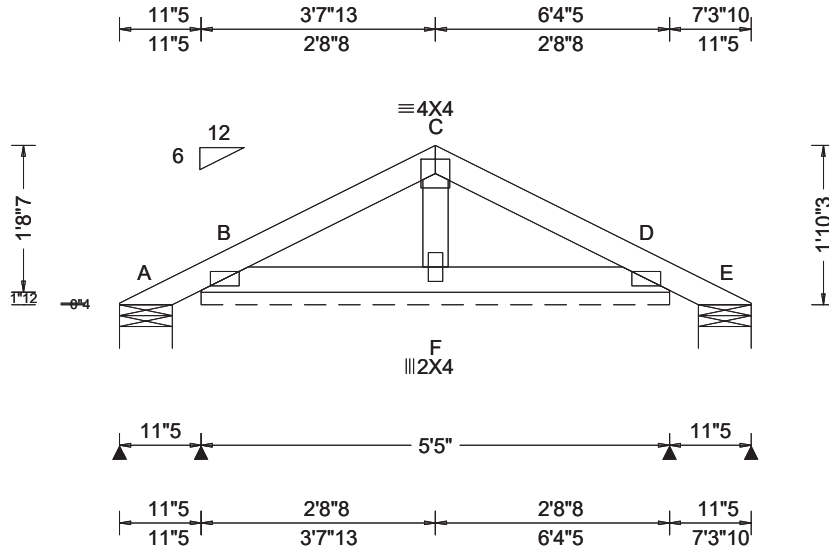
Additional Notes
 See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

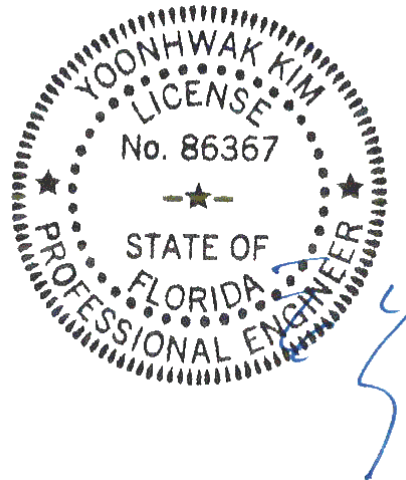
All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

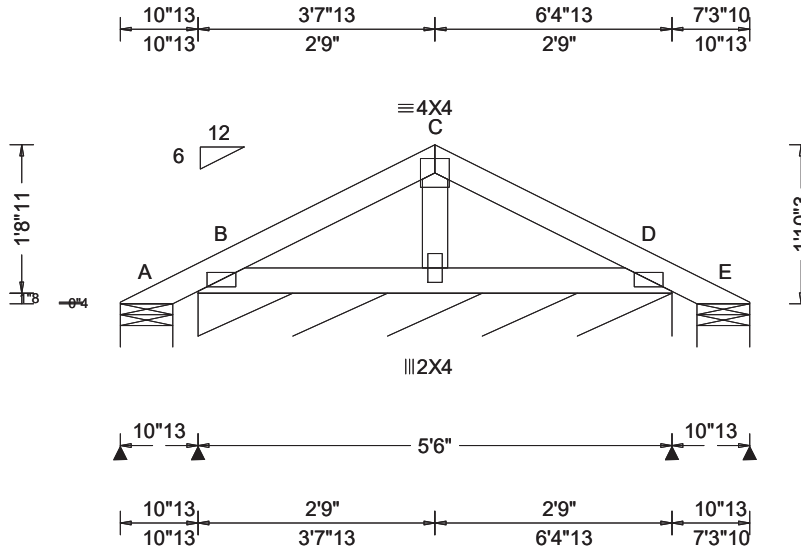
Refer to DWG PB160160118 for piggyback details.
The overall height of this truss excluding overhang is 1-10-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
02/02/2022

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.070 Max BC CSI: 0.039 Max Web CSI: 0.017 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-6</td> <td>/-</td> <td>/27</td> <td>/26</td> <td>/48</td> </tr> <tr> <td>B*</td> <td>83</td> <td>/-</td> <td>/-</td> <td>/58</td> <td>/10</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-6</td> <td>/-</td> <td>/6</td> <td>/5</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-6	/-	/27	/26	/48	B*	83	/-	/-	/58	/10	/-	E	-	/-6	/-	/6	/5	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

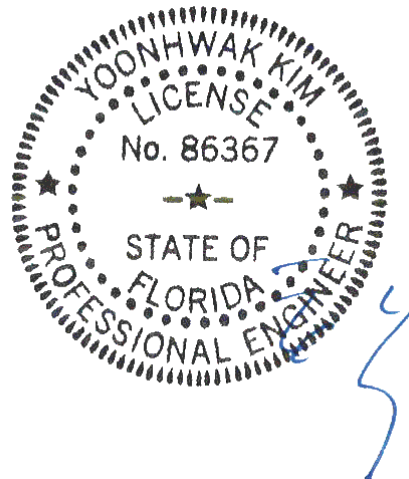
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

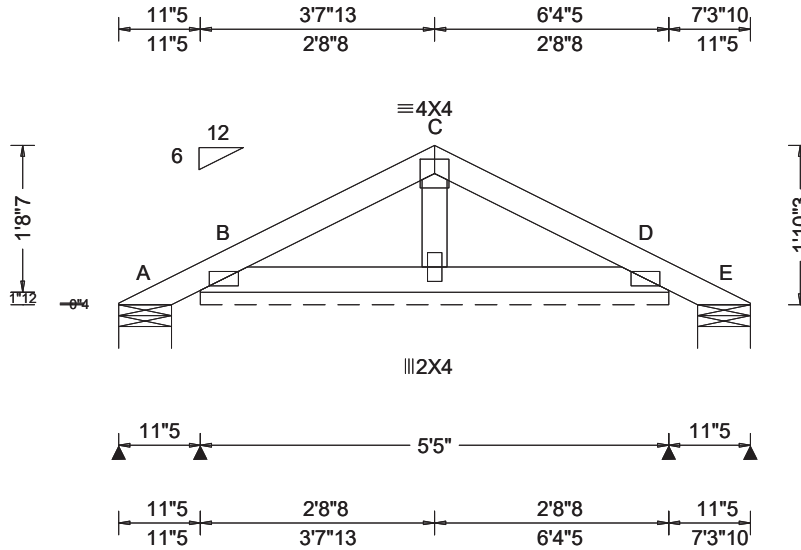
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.068 Max BC CSI: 0.038 Max Web CSI: 0.017 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-2 /- /27 /24 /48 B* 83 /- /- /58 /26 /- E - /-2 /- /6 /3 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 B Brg Wid = 65.0 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

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Loading

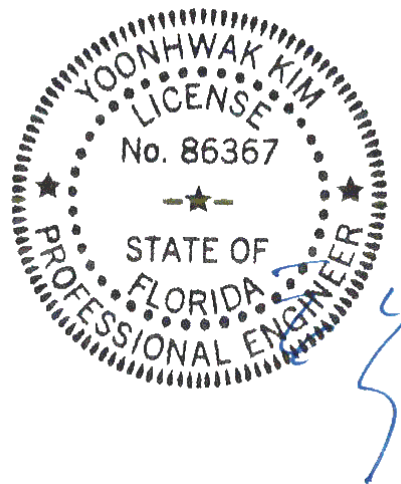
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Wind loading based on both gable and hip roof types.

Additional Notes

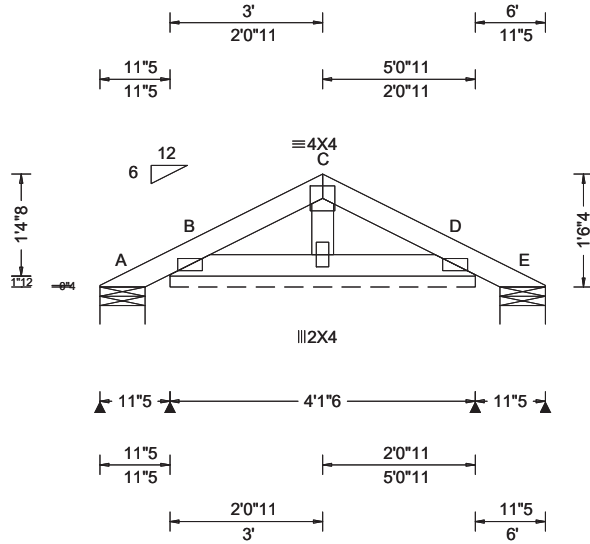
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

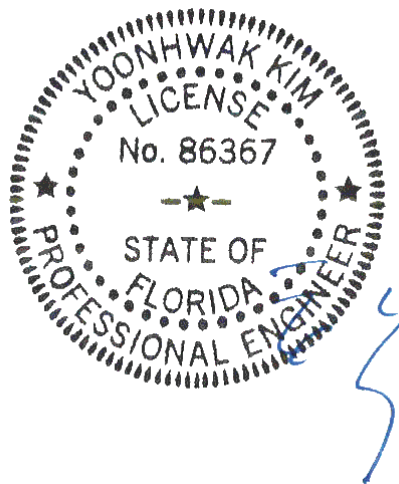
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Additional Notes

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ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00
 Gable Stud Reinforcement Detail
 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

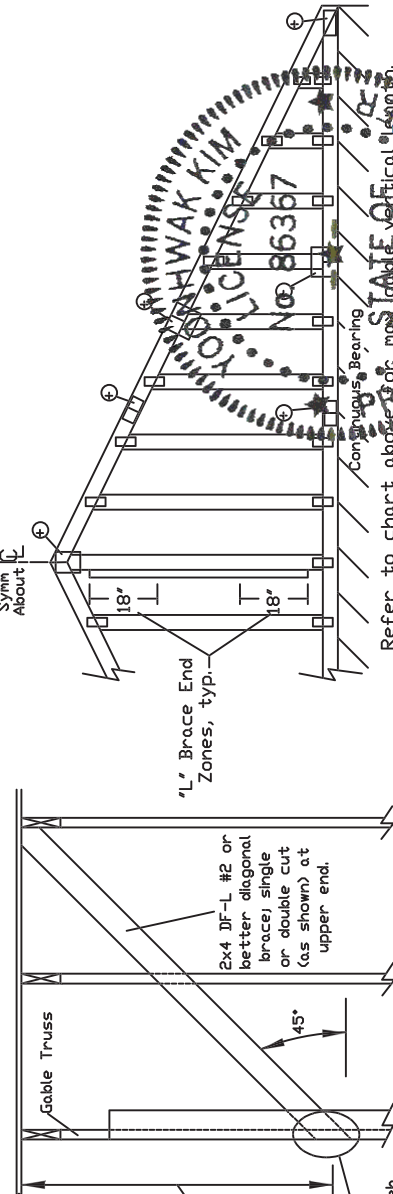
Gable Vertical Spacing	2x4 Gable Species	Brace Grade	No Braces		(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x4 'L' Brace		(2) 2x6 'L' Brace		(2) 2x6 'L' Brace		(2) 2x6 'L' Brace		
			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A
12" o.c.	SPF	#1 / #2	7' 3"	8' 7"	8' 11"	10' 3"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	6' 7"	7' 1"	8' 10"	10' 1"	10' 6"	13' 4"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	6' 7"	7' 0"	8' 10"	10' 1"	10' 6"	13' 4"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	HF	Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 6"	10' 6"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 6"	7' 4"	7' 8"	9' 0"	10' 4"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	DFL	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	HF	Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	8' 5"	8' 9"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" o.c.	SPF	Standard	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	HF	Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1 / #2	4' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	HF	Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	Standard	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	DFL	Standard	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:		Group B:	
Spruce-Pine-Fir	Hem-Fir	Southern Pine***	
#1 / #2 Standard	#2 Stud	#1 Stud	#1 Stud
#3 Standard	#3 Stud	#2 Stud	#2 Stud
Douglas Fir-Larch		Douglas Fir-Larch	
#3 Standard		#1 Stud	#1 Stud
		#2 Stud	#2 Stud

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 plf over continuous bearing (5 psf IC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF	ASCE7-16-GABI4015
DATE	01/26/2018
DRWG	A14015ENC160118

MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

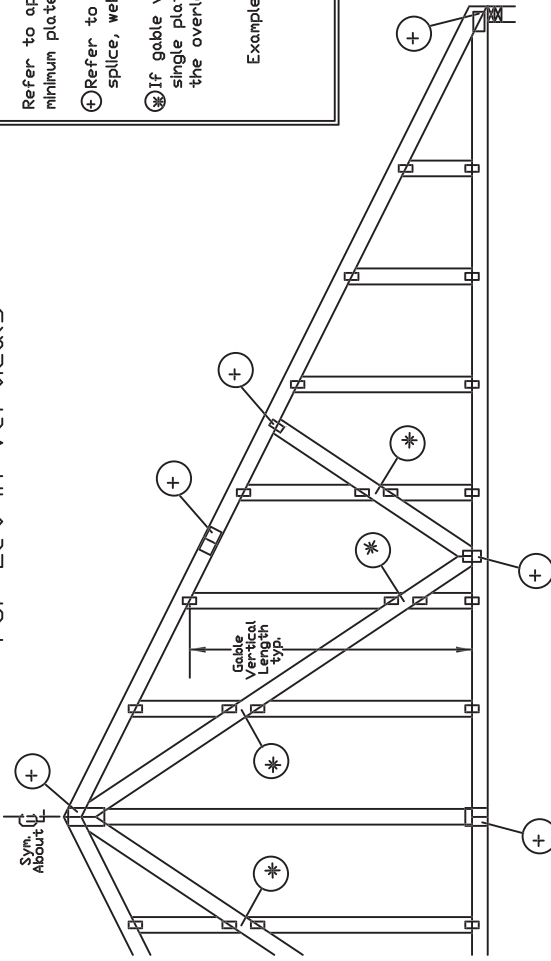


IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS.
 Trusses require extensive care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Components Safety Information (TPI and SIB) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the Joint Details, unless noted otherwise.
 Refer to drawings 10414-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss design or installation. The truss is to be installed in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
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 For more information see this job's general notes page and these web sites: www.alpineitw.com, www.alpinetw.com, TPI: www.tpi-st.org, SBCA: www.sbcccomponents.com ICC: www.iccsafe.org
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Gable Detail For Let-in Verticals

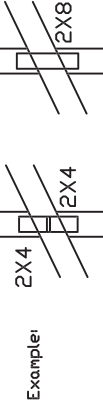


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Example:

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

- 10d Common (0.148"x3",min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:

- 10d Common (0.148"x3",min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

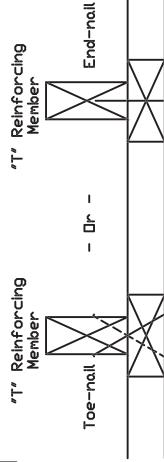
- A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
- A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

- A11515ENC100118, A12015ENC100118, A14015ENC100118, A18015ENC100118,
- A20015ENC100118, A20015ENC100118, A20015ENC100118, A20015ENC100118,
- A18030ENC100118, A20030ENC100118, A14030ENC100118, A20030ENC100118,
- S11515ENC100118, S12015ENC100118, S14015ENC100118, S20015ENC100118,
- S18030ENC100118, S12030ENC100118, S14030ENC100118, S20030ENC100118,
- S18030ENC100118, S20030ENC100118, S20030ENC100118, S20030ENC100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mem. Size	'T' Increase %
2x4	30 %
2x6	20 %

Example:

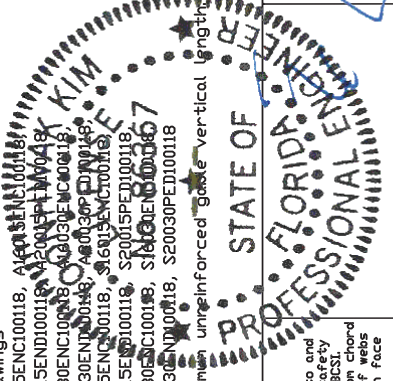
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24' o.c. SP #3
- 'T' Reinforcing Member Size = 2x4
- 'T' Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 'L' Brace Length = 8' 7"
- Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Components Installation, TPI and SPSA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a bracing attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the joint details, unless noted otherwise.
 Refer to drawings 1004-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of trusses.
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Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

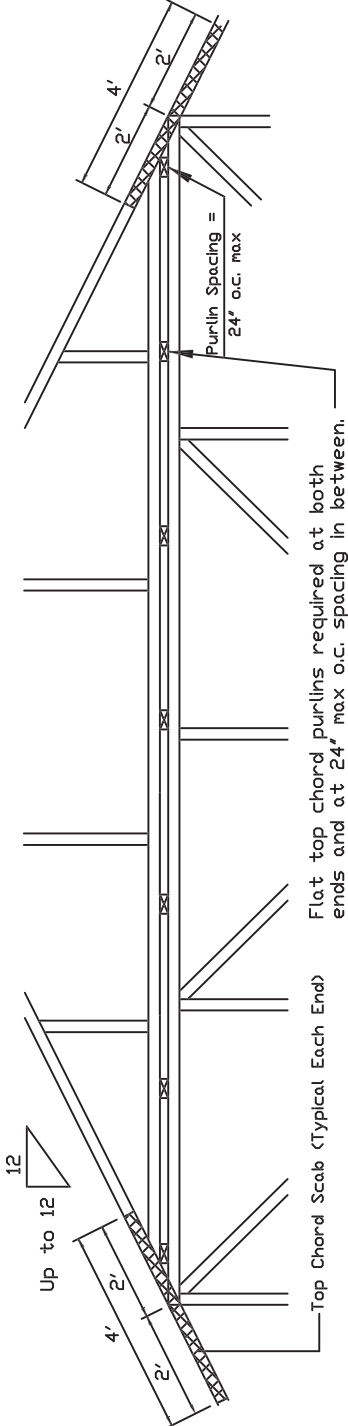
160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min), Kzt=1.0
 Or 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (min), Kzt=1.0

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

*** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



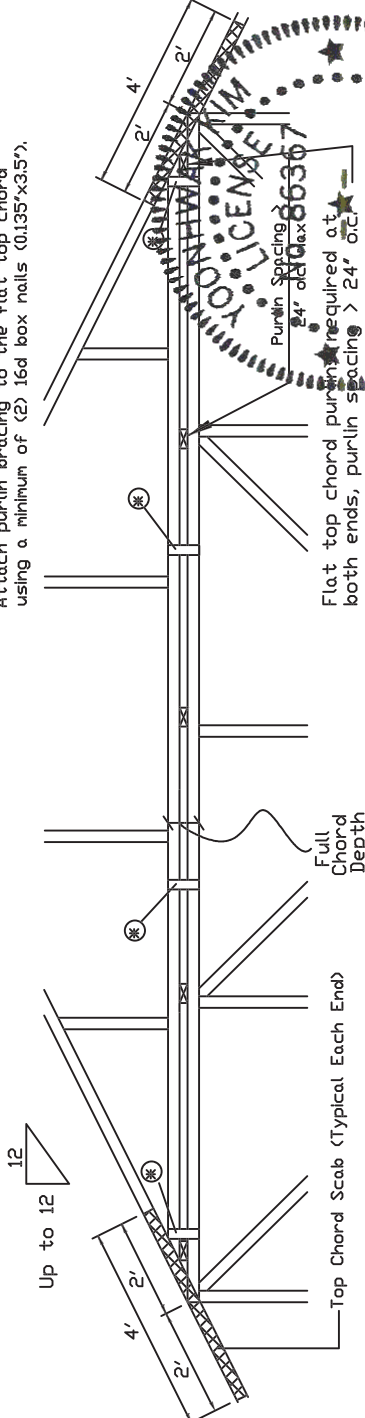
Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLERS.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSE Building Components Safety Information, TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSE. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the joint details, unless noted otherwise.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss or the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
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 For more information see this job's general notes page and these web sites: (920) 222-2778, ALPINE: www.alpinetw.com; TPI: www.tpi.net.org; SBCA: www.sbcccomponents.com; ICC: www.iccsafe.org

* In addition, provide connection with one of the following methods: Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces. APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces. 2x4 Vertical Scabs 2x4 SPF #2 full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces. 28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
--

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

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CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

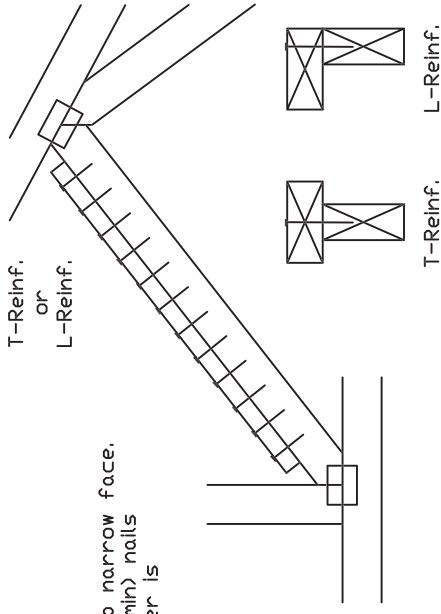
(*) Center scab on wide face of web. Apply (1) scab to each face of web.

T-Reinforcement

OR

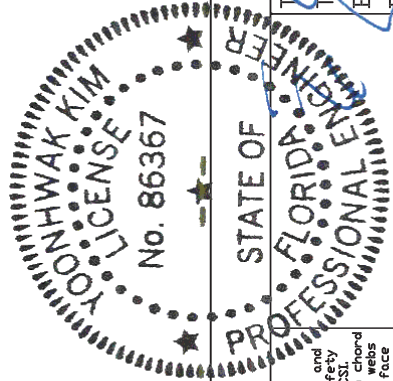
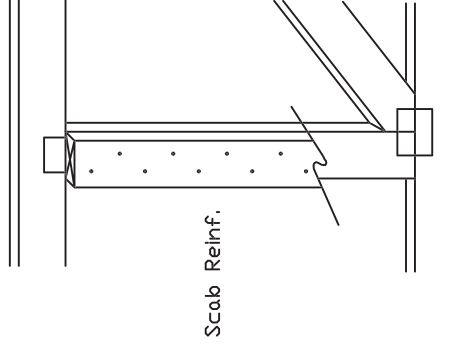
L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING! THE INSTALLERS MUST FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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TC LL	PSF	REF	CLR Subst.
TZ DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

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