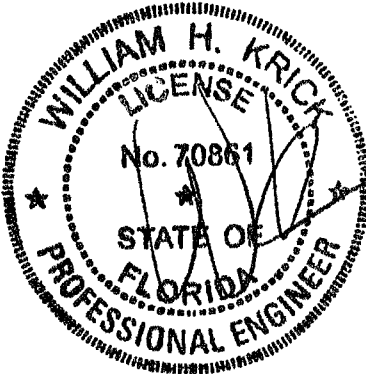




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 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com



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COA #0 278

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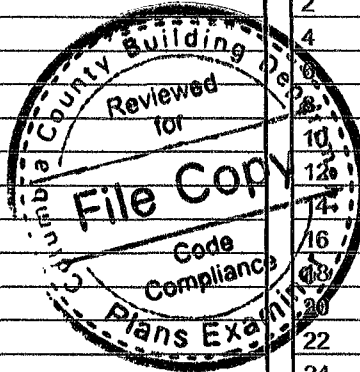
03/03/2025

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 25-2306
Job Description: MOSS	
Address: Lot 29 cobblestone S/D, Lake City, FL 32055	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 23.02.04 JRef #: 1Y7W2150003
Wind Standard: ASCE 7-22 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

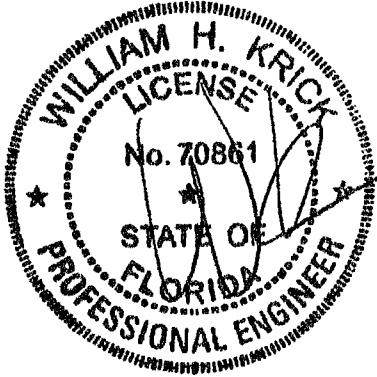
This package contains general notes pages, 64 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	062.25.0537.33483	A01	2	062.25.0537.35603	B01
3	062.25.0537.36953	B02	4	062.25.0537.38430	C01
5	062.25.0537.42987	D01	6	062.25.0537.44920	D02
7	062.25.0537.47620	D03	8	062.25.0537.50690	D04
9	062.25.0537.52153	D05	10	062.25.0537.55290	D06
11	062.25.0537.56780	D07	12	062.25.0537.58773	D08
13	062.25.0538.00887	D09	14	062.25.0538.02260	D10
15	062.25.0538.13660	G01	16	062.25.0538.16637	G02
17	062.25.0538.19950	G03	18	062.25.0538.22553	G04
19	062.25.0538.25033	G05	20	062.25.0538.27467	G06
21	062.25.0538.29130	G07	22	062.25.0538.34000	G08
23	062.25.0538.37640	G09	24	062.25.0538.40797	G10
25	062.25.0538.43843	G11	26	062.25.0538.52993	G12
27	062.25.0539.10740	G13	28	062.25.0539.15353	G14
29	062.25.0539.23567	G15	30	062.25.0539.25293	G16
31	062.25.0539.31520	G17	32	062.25.0539.32987	G18
33	062.25.0539.34650	G19	34	062.25.0539.36340	G20
35	062.25.0539.38520	G21	36	062.25.0539.39867	G22
37	062.25.0539.41487	G23	38	062.25.0539.42973	G24
39	062.25.0541.06840	G25	40	062.25.0541.09887	G26
41	062.25.0541.11140	G27	42	062.25.0541.19283	G28
43	062.25.0541.27847	H01	44	062.25.0541.29170	H02
45	062.25.0541.59533	H03	46	062.25.0542.18760	HJ01
47	062.25.0542.20400	HJ02	48	062.25.0542.21760	J01





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03/03/2025

Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 25-2306
Job Description: MOSS	
Address: Lot 29 cobblestone S/D, Lake City, FL 32055	

Item	Drawing Number	Truss
49	062.25.0542.23027	J02
51	059.25.1513.19464	J04
53	062.25.0542.33153	J06
55	062.25.0542.35757	J08
57	062.25.0542.46640	J10
59	062.25.0542.50557	PB02
61	062.25.0542.53210	PB05
63	062.25.0542.55673	PB07
65	BRCLBSUB0119	
67	VAL180220723	

Item	Drawing Number	Truss
50	062.25.0542.24457	J03
52	062.25.0542.31477	J05
54	062.25.0542.34520	J07
56	062.25.0542.40297	J09
58	062.25.0542.49377	PB01
60	062.25.0542.51567	PB03
62	062.25.0542.54597	PB06
64	062.25.0542.58653	PB08
66	PB160220723	
68	VALTN220723	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

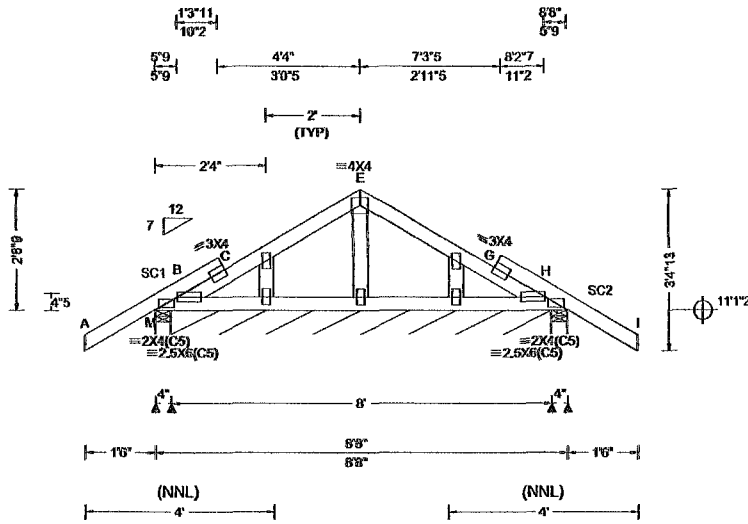
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 801983 FROM: CDM	GABL Ply 1 Qty 1	Job Number: 25-2306 MOSS Truss Label: A01	Cust: R 215 JRef: 1Y7W2150003 T9 DrawNo: 062.25.0537.33483 KD / DF 03/03/2025
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Loading Criteria (psf) TCLL 20.00 TCCL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCCL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lir: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): -0.001 C 999 240 VERT(CL): -0.002 C 999 180 HORZ(LL): 0.000 G - - HORZ(TL): 0.001 G - - Creep Factor: 2.0 Max TC CSI: 0.239 Max BC CSI: 0.031 Max Web CSI: 0.070 VIEW Ver 23.02.04.0123.14	A Maximum Reactions (lbs), or *P.L.F <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>M</td> <td>261</td> <td>-</td> <td>-</td> <td>182</td> <td>153</td> <td>106</td> </tr> <tr> <td>B*</td> <td>50</td> <td>-</td> <td>-</td> <td>132</td> <td>16</td> <td>-</td> </tr> <tr> <td>H</td> <td>260</td> <td>-</td> <td>-</td> <td>194</td> <td>153</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS M Brg Wid = 4.0 Min Req = 1.5 (Truss) B Brg Wid = 96.0 Min Req = - H Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings M, B, & H are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	M	261	-	-	182	153	106	B*	50	-	-	132	16	-	H	260	-	-	194	153	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
M	261	-	-	182	153	106																																
B*	50	-	-	132	16	-																																
H	260	-	-	194	153	-																																

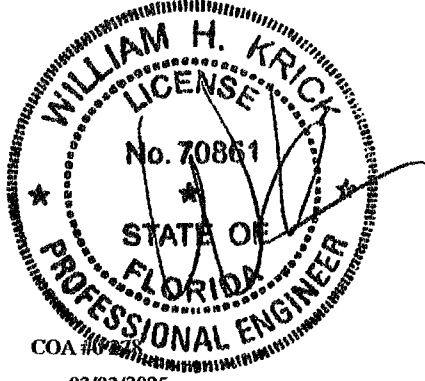
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes
 All plates are 2X4 except as noted

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

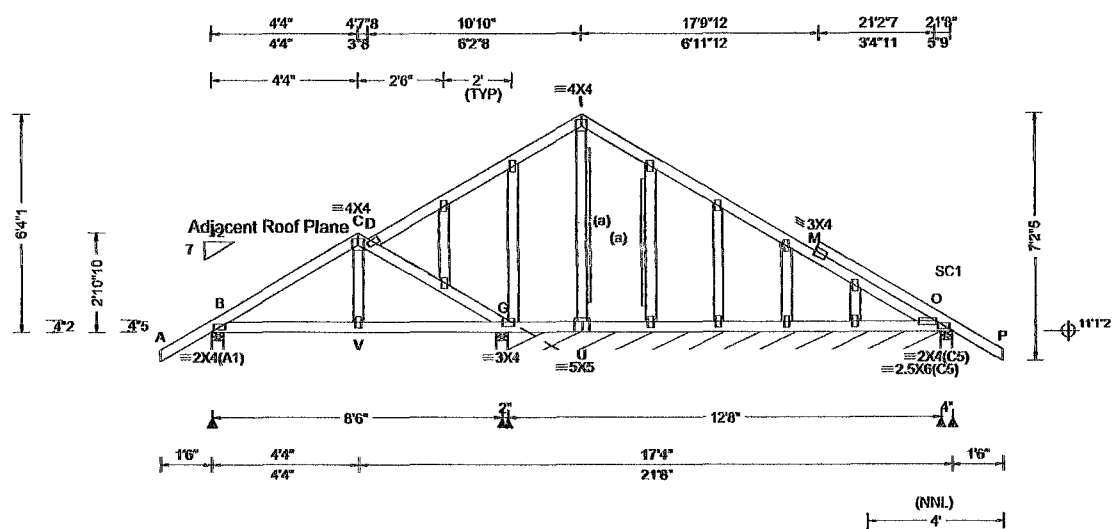
Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 2-6-9.



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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	A Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 E 999 240 VERT(CL): 0.022 E 999 180 HORZ(LL): 0.005 E - - HORZ(TL): 0.010 E - - Creep Factor: 2.0 Max TC CSI: 0.280 Max BC CSI: 0.160 Max Web CSI: 0.837 VIEW Ver 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 429 /- /- /269 /73 /207 G 183 /- /- /100 /- /- G* 96 /- /- /49 /22 /- O 281 /- /- /175 /27 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) G Brg Wid = 4.0 Min Req = 1.5 (Truss) G Brg Wid = 152 Min Req = - O Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B, G, G, & O are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. V - G 488 - 189

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3,
Stack Chord: SC1 2x4 SP #2;

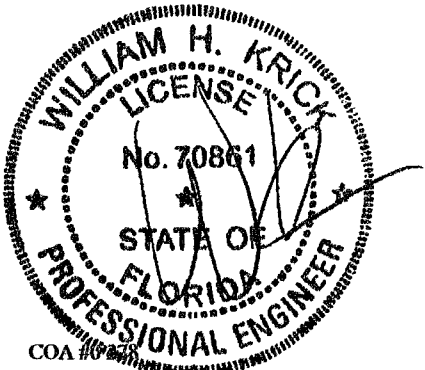
Plating Notes
All plates are 2X4 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/237

Gable Reinforcement
(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2' oc at each end for the first 18" and then 4' oc for the remainder.

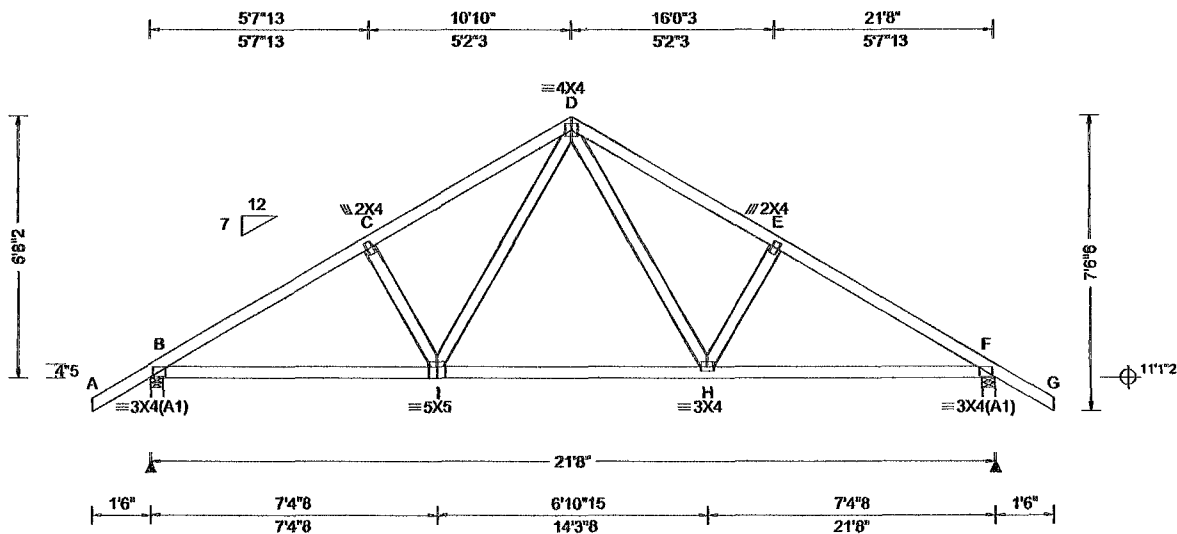
Additional Notes
Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.
The overall height of this truss excluding overhang is 6-4-1.



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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





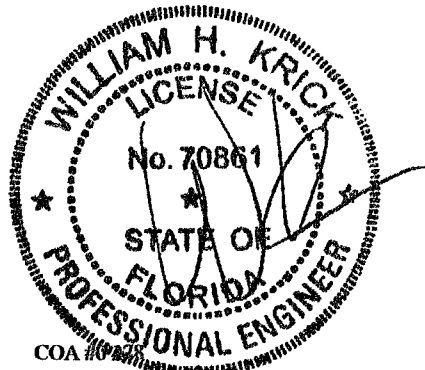
Loading Criteria (psf) TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld 40.00 NCBCLL 10.00 Soffit 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lur: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.045 H 999 240 VERT(CL): 0.088 H 999 180 HORZ(LL): 0.019 F - - HORZ(TL): 0.036 F - - Creep Factor: 2.0 Max TC CSI 0.279 Max BC CSI: 0.526 Max Web CSI: 0.196 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1054 /- /- /605 /174 /208 F 1054 /- /- /605 /174 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 435 -1464 D - E 472 -1308 C - D 472 -1307 E - F 434 -1465
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Lumber
Top chord: 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

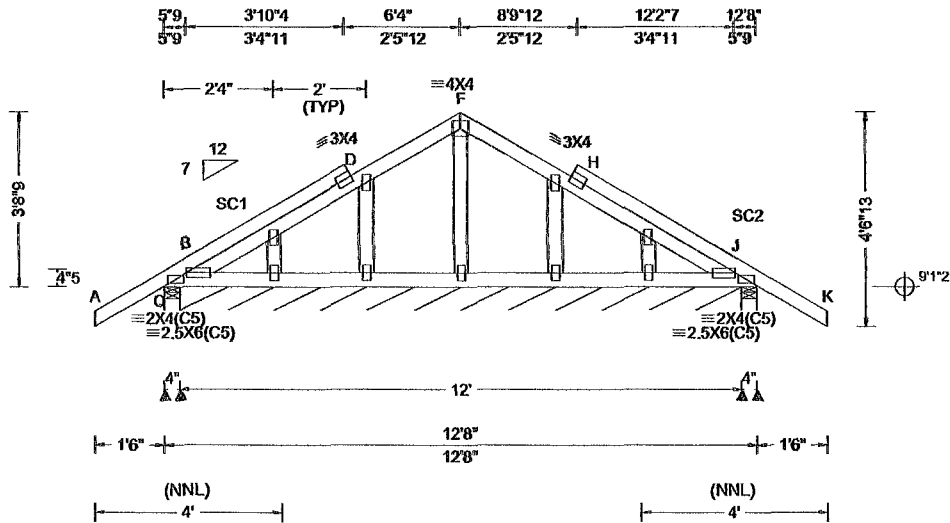
Additional Notes
The overall height of this truss excluding overhang is 6-8-2.



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Loading Criteria (psf) TCLL. 20.00 TCDL. 10.00 BCLL. 0.00 BCDL. 10.00 Des Ld. 40.00 NCBCLL. 10.00 Soffit. 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category II EXP-C Kzt: NA Mean Height: 15.00 ft TCDL. 5.0 psf BCDL. 5.0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpt: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.000 I - - HORZ(TL): 0.001 H - - Creep Factor: 2.0 Max TC CSI: 0.293 Max BC CSI: 0.027 Max Web CSI: 0.392 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs), or *PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Q 292 /- /- /184 /67 /136 B* 56 /- /- /33 /6 /- J 232 /- /- /206 /67 /- Wind reactions based on MWFRS Q Brg Wid = 4.0 Min Req = 1.5 (Truss) B Brg Wid = 144 Min Req = - J Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings Q, B, & J are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2,
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types.

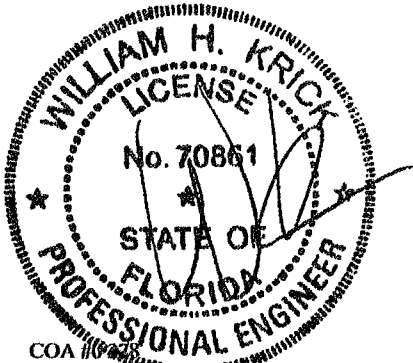
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/753.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 3-8-9.



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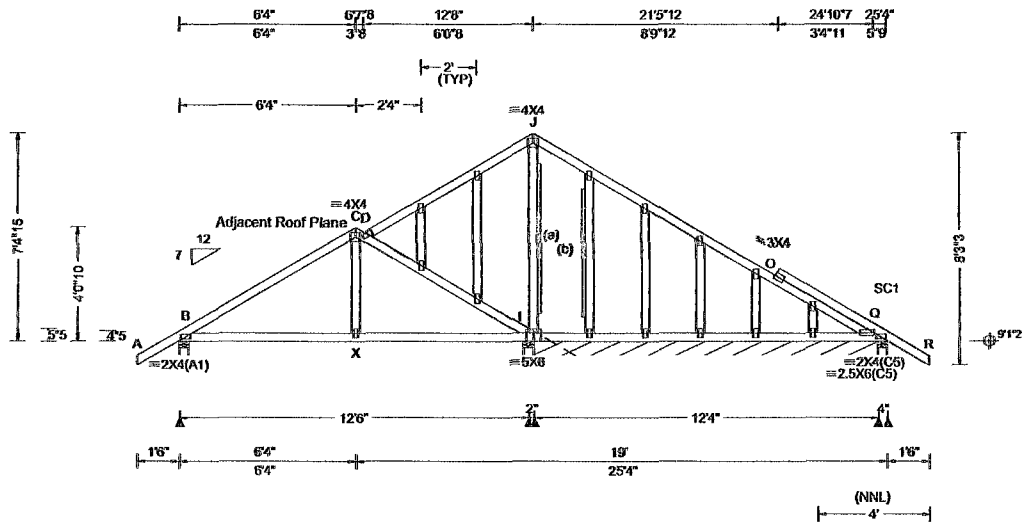
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Loading Criteria (psf) TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: 0 to 1/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT-20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.039 G 999 240 VERT(CL): 0.080 G 999 180 HORZ(LL): 0.021 G - - HORZ(TL): 0.044 G - - Creep Factor: 2.0 Max TC CSI: 0.362 Max BC CSI: 0.359 Max Web CSI: 0.992 VIEW Ver 23.02.04.0123.14	A Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rw</th> <th>U</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>569</td> <td>-</td> <td>1361</td> <td>1235</td> </tr> <tr> <td>I</td> <td>741</td> <td>-</td> <td>1522</td> <td>138</td> </tr> <tr> <td>I*</td> <td>62</td> <td>-</td> <td>144</td> <td>120</td> </tr> <tr> <td>Q</td> <td>270</td> <td>-</td> <td>1166</td> <td>18</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) I Brg Wid = 4.0 Min Req = 1.5 (Truss) I Brg Wid = 147 Min Req = - Q Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B, I, I, & Q are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>152</td> <td>-519</td> <td>D - I</td> <td>278</td> <td>-606</td> </tr> <tr> <td>C - D</td> <td>161</td> <td>-397</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	R-	Rw	U	B	569	-	1361	1235	I	741	-	1522	138	I*	62	-	144	120	Q	270	-	1166	18	Chords	Tens. Comp.		Chords	Tens. Comp.						B - C	152	-519	D - I	278	-606	C - D	161	-397			
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;

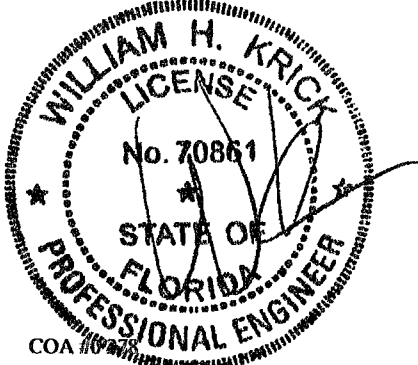
Plating Notes
 All plates are 2X4 except as noted

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/216.

Gable Reinforcement
 (a) 1x4 SP/DF #2 or better "L" reinforcement. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
 (b) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3", min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder

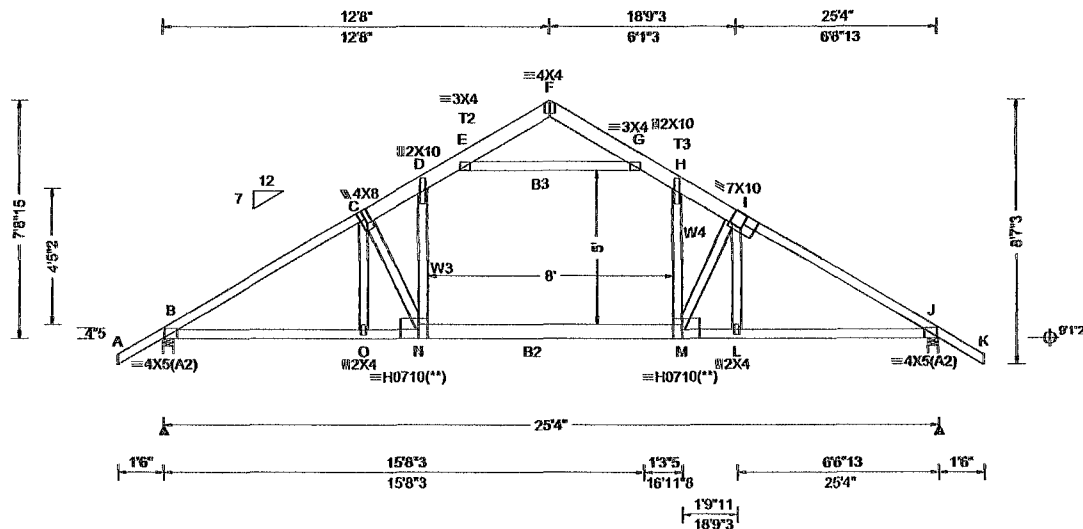
Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 7'-4-15".



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall Any GCpt: 0.18 Wind Duration: 1.60	Pg NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.134 M 999 240 VERT(CL): 0.319 M 941 180 HORZ(LL): 0.058 D - - HORZ(TL): 0.139 D - - Creep Factor: 2.0 Max TC CSI: 0.758 Max BC CSI: 0.540 Max Web CSI: 0.284 VIEW Ver 23.02.04.0123.14	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1727 / - / - / 1692 / 198 / 1235 J 1727 / - / - / 1692 / 198 / - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) J Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp B - C 447 - 2716 G - H 419 - 1972 C - D 491 - 2644 H - I 477 - 2616 D - E 419 - 1971 I - J 450 - 2723

Lumber
Top chord: 2x4 SP #2; T2,T3 2x6 SP #2;
Bot chord: 2x4 SP M-31, B2 2x6 SP 2400F-2.0E;
B3 2x4 SP #2;
Webs: 2x4 SP #3, W3,W4 2x4 SP #2;

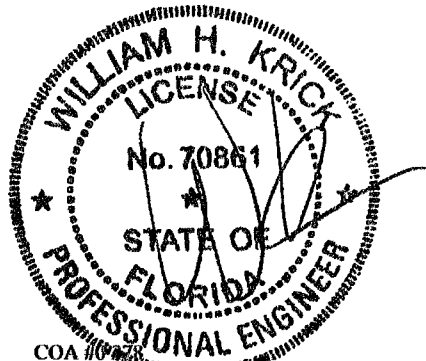
Plating Notes
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

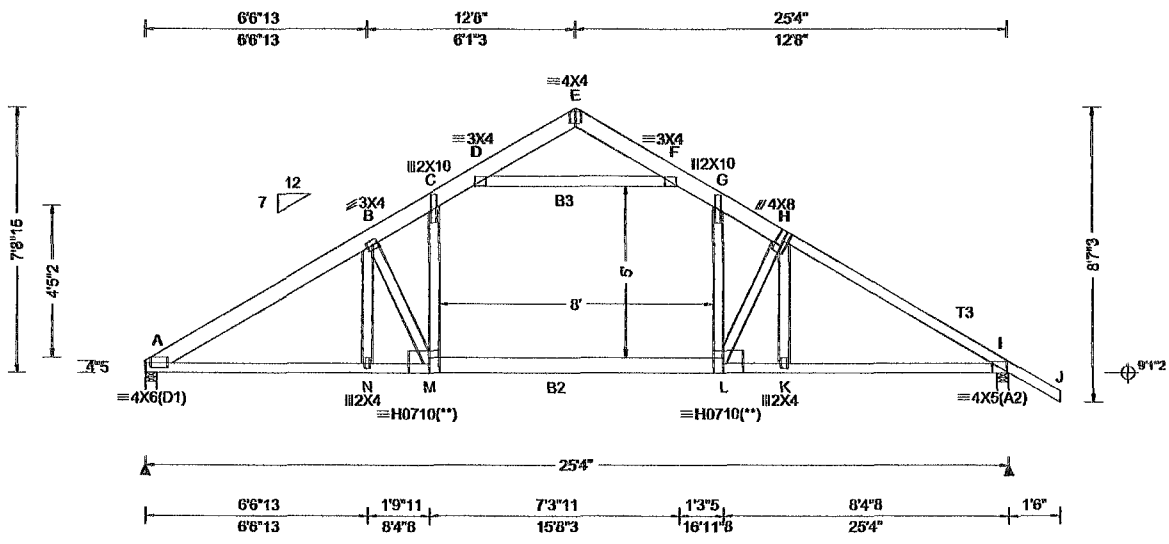
Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA PE: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.131 L 999 240 VERT(CL): 0.310 M 970 180 HORZ(LL): 0.061 C - - HORZ(TL): 0.148 C - - Creep Factor: 2.0 Max TC CSI: 0.768 Max BC CSI: 0.541 Max Web CSI: 0.506 VIEW Ver: 23.02.04.0123.14	A Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1622 - / - / 805 / 170 / 217 I 1731 - / - / 691 / 199 - Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) I Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 343 -2774 F - G 320 -1979 B - C 382 -2702 G - H 356 -2648 C - D 325 -1977 H - I 318 -2723
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Lumber
Top chord: 2x6 SP #2; T3 2x4 SP #2;
Bot chord: 2x4 SP M-31, B2 2x6 SP 2400F-2.0E;
B3 2x4 SP #2;
Webs: 2x4 SP #3;

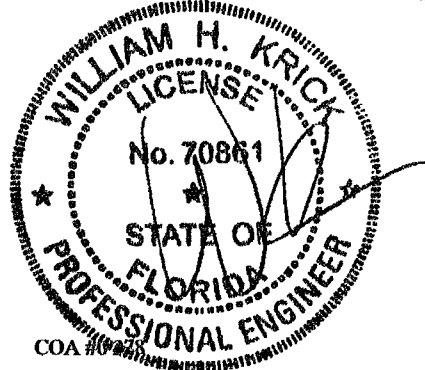
Plating Notes
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Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

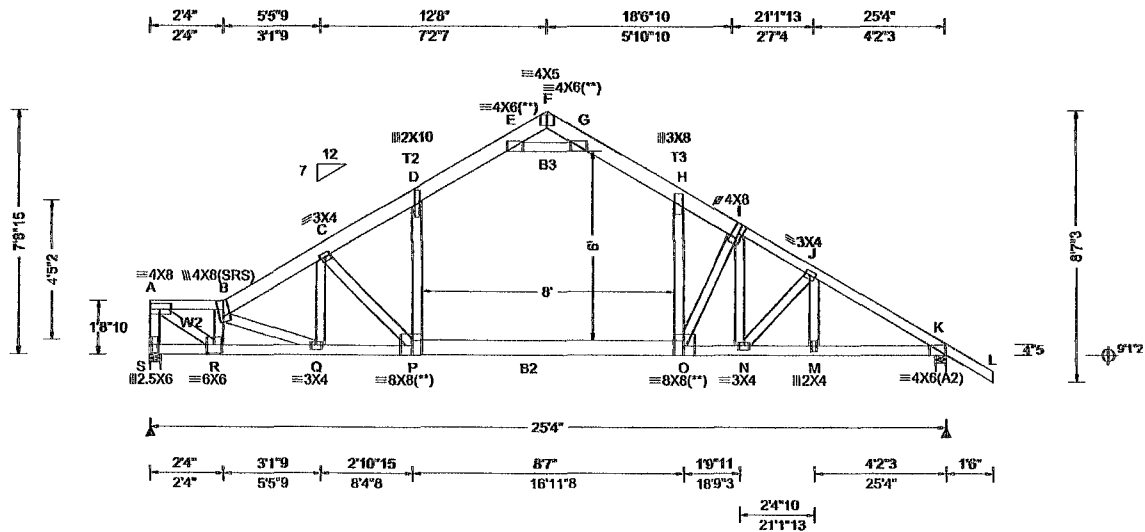
Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40 00 NCBCLL 10.00 Softit: 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Std: ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP: C Kzi: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg. NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.225 P 999 240 VERT(CL): 0.486 P 622 180 HORZ(LL): 0.107 D - - HORZ(TL): 0.239 D - - Creep Factor: 2.0 Max TC CSI 0.561 Max BC CSI 0.588 Max Web CSI 0.604 VIEW Ver 23.02.04.0123.14	Gravity Loc R+ /R- /Rh /Rw /U /RL S 1624 - / - /568 /175 /206 K 1752 - / - /696 /198 - Non-Gravity Wind reactions based on MWFRS S Brg Wid = 4.0 Min Req = 1.5 (Truss) K Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings S & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens Comp. A - B 343 -2207 F - G 1299 - 151 B - C 408 -2873 G - H 324 - 1880 C - D 358 -2623 H - I 348 -2673 D - E 327 -1879 I - J 350 -2638 E - F 1301 -148 J - K 324 -2774

Lumber
Top chord: 2x4 SP #2; T2,T3 2x6 SP 2400f-2.0E,
Bot chord: 2x4 SP M-31; B2 2x6 SP 2400f-2.0E;
B3 2x4 SP #2;
Webs: 2x4 SP #3; W2 2x4 SP #2;

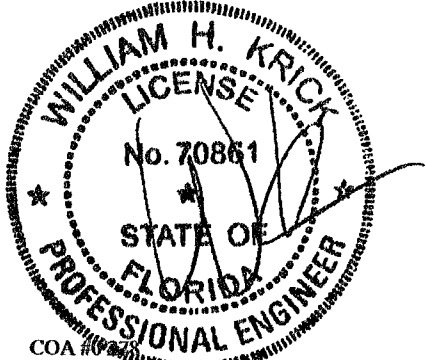
Plating Notes
(**) 4 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load. 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	2527 - 323	O - N	2283 - 149
Q - P	2502 - 235	N - M	2323 - 190
P - O	1963 - 77	M - K	2323 - 189

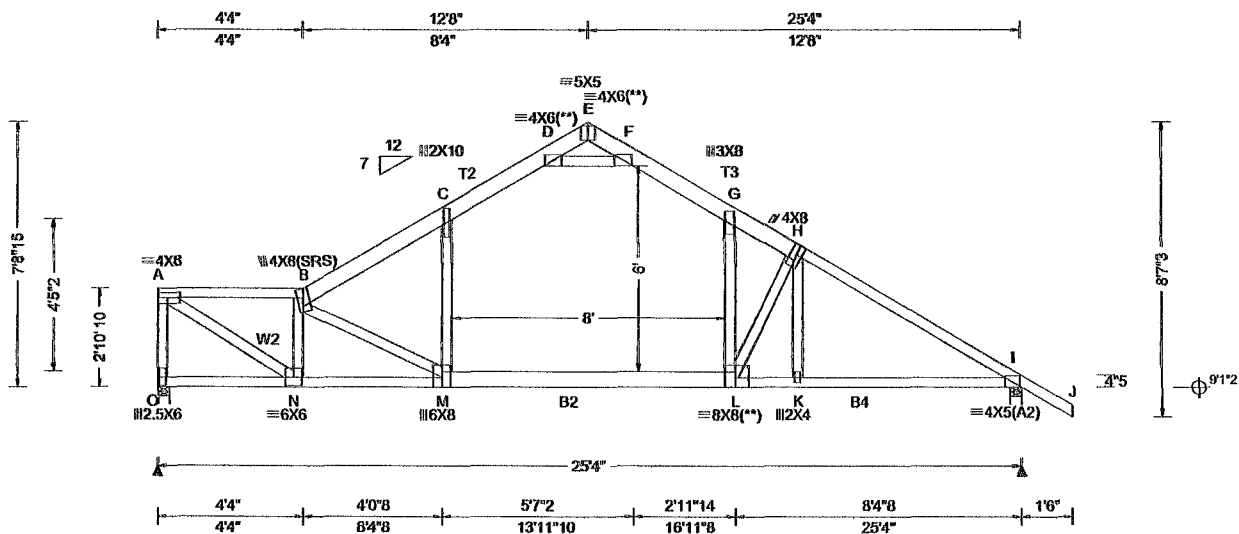
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - S	297 - 1579	P - D	1279 - 96
A - R	2677 - 412	E - G	549 - 3468
R - B	286 - 1544	H - O	1474 - 88
C - P	231 - 850	O - I	179 - 614

03/03/2025
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lur: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.252 M 999 240 VERT(CL): 0.547 M 552 180 HORZ(LL): 0.115 C - - HORZ(TL): 0.260 C - - Creep Factor: 2.0 Max TC CSI: 0.570 Max BC CSI: 0.881 Max Web CSI: 0.625 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs) Gravily Non-Gravily Loc R+ / R- / Rh / Rw / U / RL O 1624 /- /- /543 /42 /206 I 1752 /- /- /700 /17 /- Wind reactions based on MWFRS O Brg Wid = 4.0 Min Req = 1.9 (Truss) I Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings O & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 433 -2342 E - F 1362 -179 B - C 365 -2518 F - G 347 -1879 C - D 345 -1889 G - H 306 -2711 D - E 1352 -182 H - I 349 -2763
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Lumber
Top chord: 2x4 SP #2; T2, T3 2x6 SP 2400F-2.0E;
Bot chord: 2x4 SP #2; B2 2x6 SP 2400F-2.0E;
B4 2x4 SP M-31,
Webs: 2x4 SP #3; W2 2x4 SP #2;

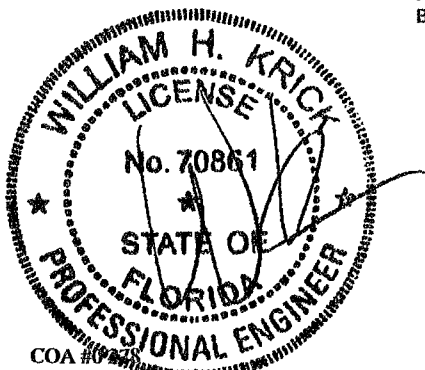
Plating Notes
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40 PSF
Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

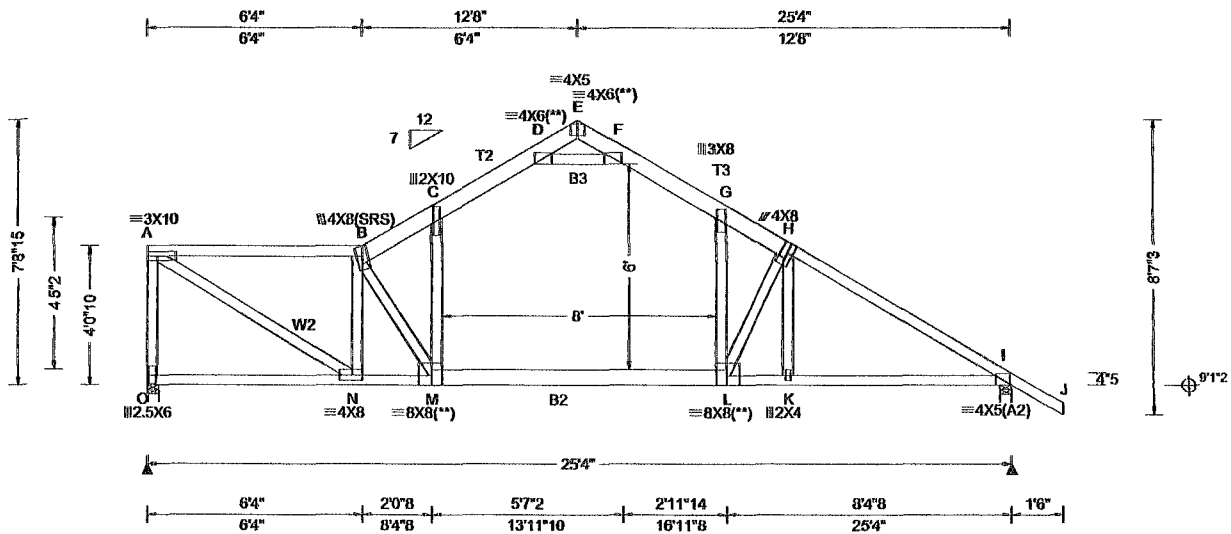
Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



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Loading Criteria (psf) TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.225 M 999 240 VERT(CL): 0.483 M 625 180 HORZ(LL): 0.090 C - - HORZ(TL): 0.206 C - - Creep Factor: 2.0 Max TC CSI: 0.718 Max BC CSI: 0.565 Max Web CSI: 0.602 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL O 1624 /- /- /546 /82 /206 I 1752 /- /- /706 /25 /- Wind reactions based on MWFRS O Brg Wid = 4.0 Min Req = 1.5 (Truss) I Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings O & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 477 -2281 E - F 1264 -174 B - C 493 -2684 F - G 375 -1878 C - D 381 -1873 G - H 424 -2700 D - E 1269 -188 H - I 375 -2764
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Lumber
Top chord 2x4 SP #2; T2,T3 2x6 SP 2400F-2.0E;
Bot chord 2x4 SP M-31, B2 2x6 SP 2400F-2.0E;
B3 2x4 SP #2;
Webs: 2x4 SP #3; W2 2x4 SP #2;

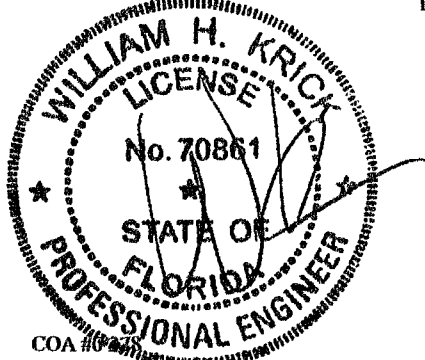
Plating Notes
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

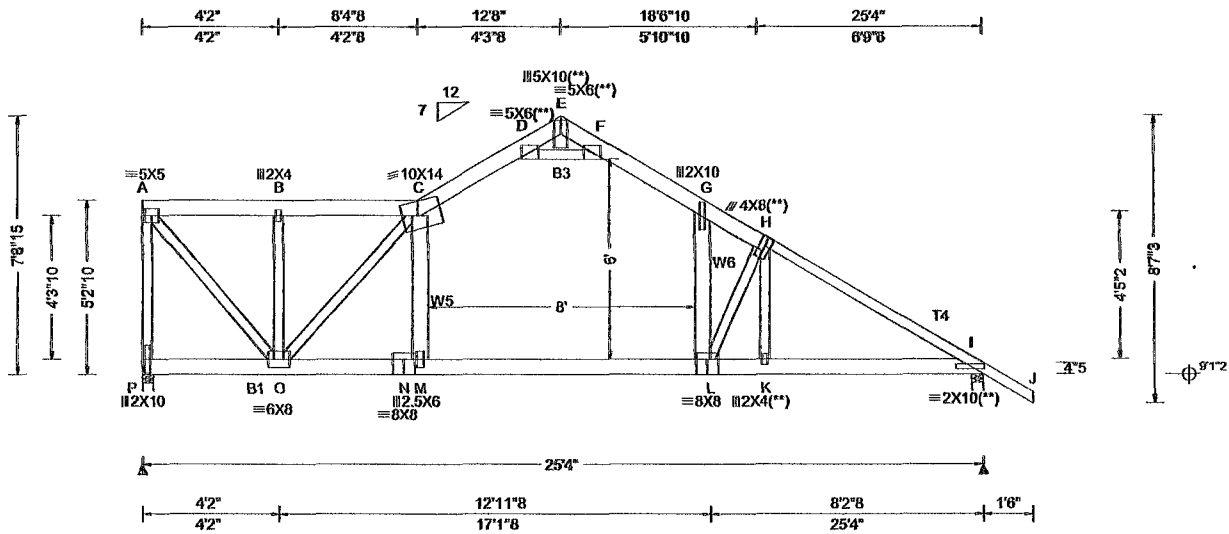
Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Δ Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.249 M 999 240 VERT(CL): 0.515 M 586 180 HORZ(LL): 0.125 A - - HORZ(TL): 0.264 A - - Creep Factor: 2.0 Max TC CSI 0.604 Max BC CSI 0.565 Max Web CSI: 0.816 VIEW Ver 23.02.04.0123.14	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL P 1623 /- /- /558 /118 /207 I 1751 /- /- /716 /37 /- Wind reactions based on MWFRS P Brg Wid = 4.0 Min Req = 1.9 (Truss) I Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings P & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 367 -1331 E - F 1456 -240 B - C 367 -1331 F - G 410 -1915 C - D 338 -1938 G - H 481 -2848 D - E 1432 -252 H - I 418 -2808

Lumber
Top chord 2x6 SP 2400F-2.0E, T4 2x4 SP #2;
Bot chord 2x6 SP 2400F-2.0E; B1 2x6 SP #2;
B3 2x4 SP #2;
Webs: 2x4 SP #3; W5,W6 2x6 SP #2;

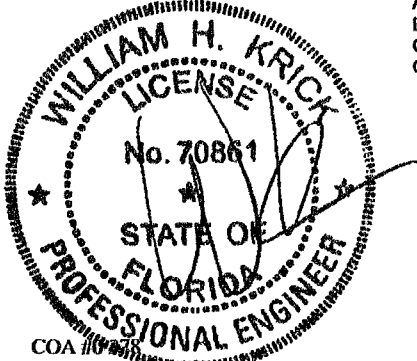
Plating Notes
(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load: 40 PSF Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

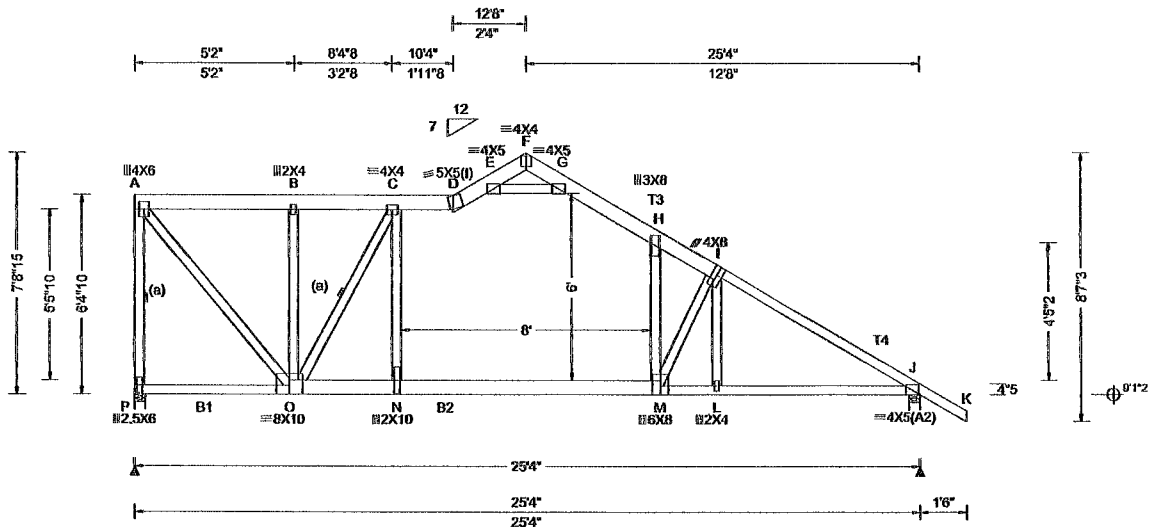
Additional Notes
The overall height of this truss excluding overhang is 7-8-15.



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Loading Criteria (psf) TCLL. 20.00 TCDL. 10.00 BCLL. 0.00 BCDL. 10.00 Des Ld: 40 00 NCBCLL. 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP- C Kzt: NA Mean Height: 15.00 ft TCDL. 5.0 psf BCDL. 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT-20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.206 M 999 240 VERT(CL): 0.452 M 669 180 HORZ(LL): -0.076 H - - HORZ(TL): 0.179 H - - Creep Factor: 2.0 Max TC CSI: 0.847 Max BC CSI: 0.808 Max Web CSI: 0.701 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P 1594 /- /- /568 /152 /207 J 1731 /- /- /726 /50 /- Wind reactions based on MWFRS P Brg Wid = 4.0 Min Req = 1.5 (Truss) J Brg Wid = 4.0 Min Req = 2.0 (Truss) Bearings P & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 353 -1185 F - G 817 -130 B - C 361 -1194 G - H 436 -1868 C - D 429 -1894 H - I 484 -2595 D - E 394 -1685 I - J 438 -2731 E - F 933 -177
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Lumber
Top chord: 2x6 SP #2; T3 2x6 SP 2400F-2.0E,
T4 2x4 SP #2;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31,
B2 2x6 SP 2400F-2.0E;
Webs: 2x4 SP #3,

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Loading
Attic room loading from 8-8-0 to 16-8-0: Live Load. 40 PSF Dead Load 10 PSF Ceiling: 10 PSF, Knee walls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 7-8-15

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Maximum Bot Chord Forces Per Ply (lbs)

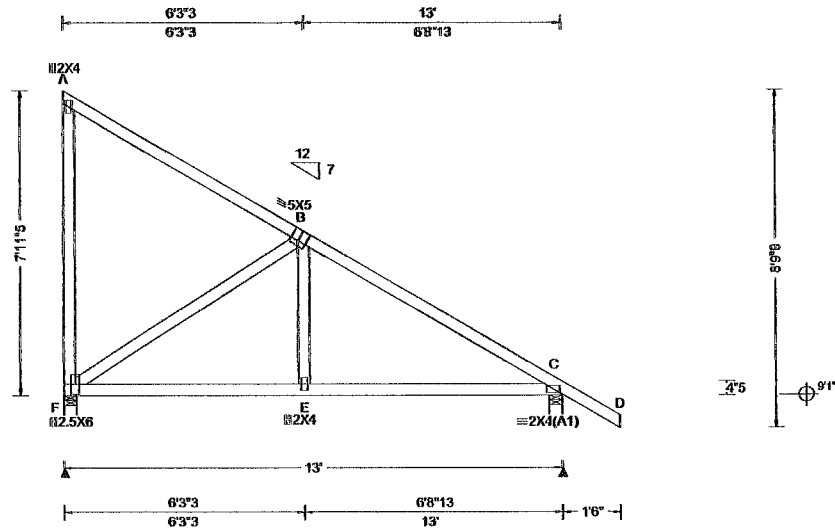
Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	1874 -181	M - L	2279 -261
N - M	1898 -179	L - J	2277 -261

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - P	560 -1544	C - N	1050 0
A - O	1841 -549	E - G	637 -2896
O - B	336 -436	H - M	1338 -140
O - C	143 -1486	M - I	189 -944

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 E 999 240 VERT(CL): 0.021 E 999 180 HORZ(LL): 0.007 A - - HORZ(TL): 0.014 A - - Creep Factor: 2.0 Max TC CSI 0.688 Max BC CSI 0.543 Max Web CSI 0.610 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 527 /- /- /389 /85 /218 C 655 /- /- /399 /- /- Wind reactions based on MWFRS F Brg Wid = 4.0 Min Req = 1.5 (Truss) C Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings F & C are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 8 -600
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Lumber

Top chord: 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

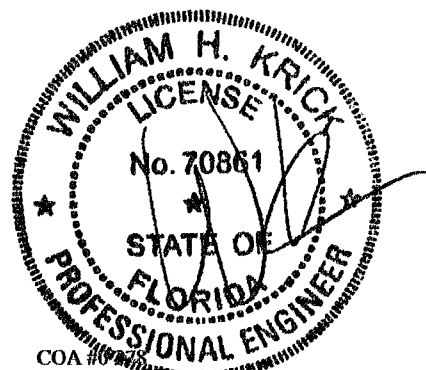
The overall height of this truss excluding overhang is 7-11-5.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens.Comp.
F - E	507 0	E - C	510 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
F - B	303 -604

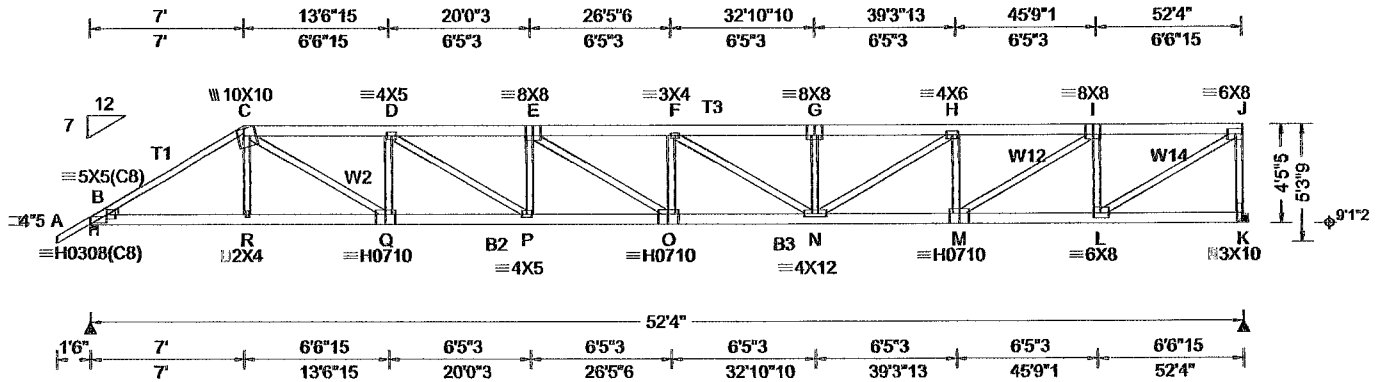


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



2 Complete Trusses Required



Loading Criteria (psf) TCLL. 20.00 TCCL. 10.00 BCLL. 0.00 BCDL. 10.00 Des Ld. 40.00 NCBCLL. 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP-C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist: 5.23 ft Loc. from endwall: not in 6 50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): HS, WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.665 O 941 240 VERT(CL): 1.339 O 467 180 HORZ(LL): 0.134 C - - HORZ(TL): 0.270 C - - Creep Factor 2.0 Max TC CSI: 0.899 Max BC CSI: 0.840 Max Web CSI: 0.713 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) <table border="1"> <tr> <th colspan="2">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U / RL</th> </tr> <tr> <td>B</td> <td>5287 / -</td> <td>- / -</td> <td>- / -</td> <td>/ 1243 / -</td> </tr> <tr> <td>K</td> <td>5255 / -</td> <td>- / -</td> <td>- / -</td> <td>/ 1302 / -</td> </tr> </table> Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 3.1 (Truss) K Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>B - C</td> <td>1157 - 4882</td> <td>F - G</td> <td>2019 - 8190</td> </tr> <tr> <td>C - D</td> <td>1651 - 6905</td> <td>G - H</td> <td>2019 - 8190</td> </tr> <tr> <td>D - E</td> <td>2027 - 8295</td> <td>H - I</td> <td>1606 - 6506</td> </tr> <tr> <td>E - F</td> <td>2158 - 8782</td> <td>I - J</td> <td>945 - 3829</td> </tr> </table>	Gravity		Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U / RL	B	5287 / -	- / -	- / -	/ 1243 / -	K	5255 / -	- / -	- / -	/ 1302 / -	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1157 - 4882	F - G	2019 - 8190	C - D	1651 - 6905	G - H	2019 - 8190	D - E	2027 - 8295	H - I	1606 - 6506	E - F	2158 - 8782	I - J	945 - 3829
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E - F	2158 - 8782	I - J	945 - 3829																																									

Lumber
 Top chord: 2x6 SP #2; T1 2x4 SP #2; T3 2x6 SP 2400F-2.0E;
 Bot chord: 2x6 SP #2; B2,B3 2x6 SP 2400F-2.0E,
 Webs: 2x4 SP #3; W2,W12 2x4 SP #2;
 W14 2x4 SP M-31,
 Lt Wedge 2x4 SP #3,
Nailnote
 Nail Schedule: 0 131"x3", min nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 1 Row @ 12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

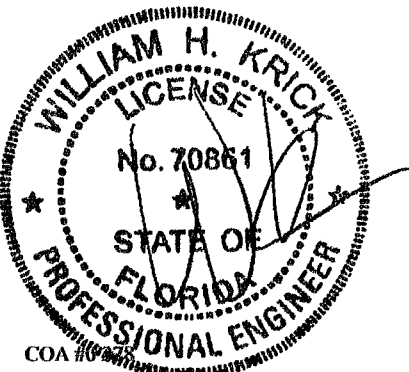
Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Wind
 Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	4182 - 985	O - N	8785 - 2167
R - Q	4193 - 984	N - M	6608 - 1638
Q - P	6905 - 1684	M - L	3982 - 991
P - O	8344 - 2047		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Q	3077 - 785	N - H	1872 - 451
Q - D	455 - 1375	H - M	440 - 1401
D - P	1657 - 409	M - I	3008 - 733
P - E	270 - 679	I - L	634 - 2104
E - O	522 - 133	L - J	4493 - 1108
F - N	175 - 704	J - K	665 - 2532
G - N	196 - 389		



03/03/2025
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SEQN: 802220	HIPM	Ply 2	Job Number: 25-2306	Cust: R 215 JRef: 1Y7W2150003 T1
FROM: CDM		Qty 1	MOSS	DrwNo: 062.25.0538.13660
Page 2 of 2			Truss Label: G01	KD / DF 03/03/2025

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

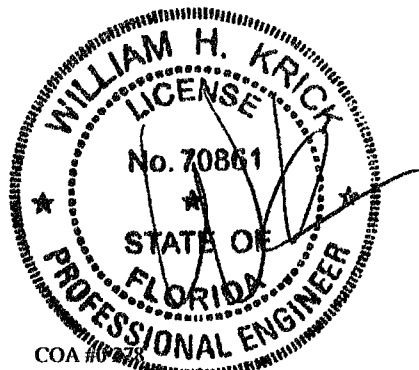
Bearing at location x=52'1" uses the following support conditions: 52'1"

- Bearing K (52'1", 9'1"2) HGUS26-2
- Supporting Member: (2)2x6 SP 2400F-2.0E
- (20) 0.162"x3.5" nails into supporting member,
- (8) 0.162"x3.5" nails into supported member

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

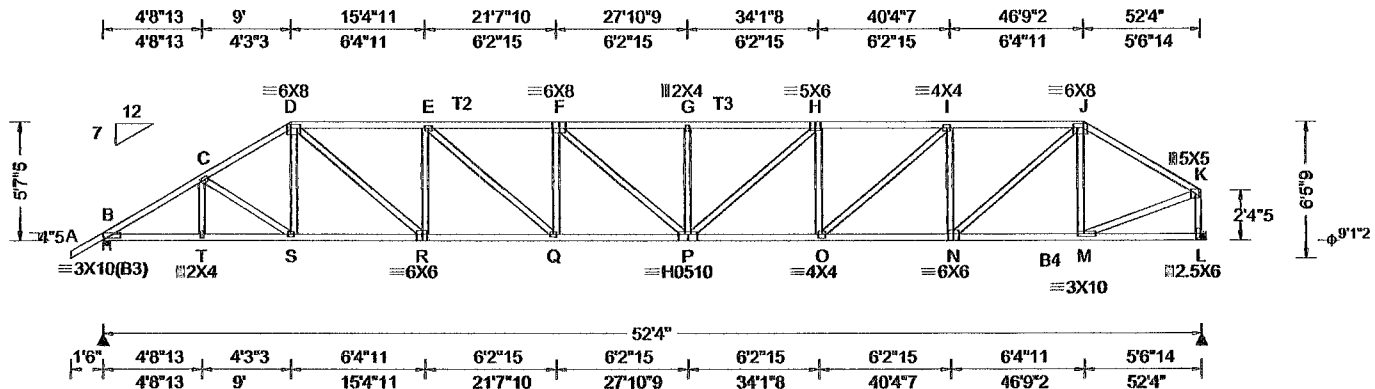
The overall height of this truss excluding overhang is 4-5-5.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Def/CSI Criteria	Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL 20.00	Wind Std. ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.447 G 999 240	B	2285	-	-	11304	1416	1160
BCLL 0.00	Enclosure: Closed	Lr: NA Cs: NA	VERT(CL): 0.924 G 677 180	L	2168	-	-	11150	1393	-
BCDL 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.108 D - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP- C Kzt: NA	Building Code:	HORZ(TL): 0.223 D - -	B	Brg Wid = 4.0		Min Req = 1.9 (Truss)			
NCBCLL 10.00	Mean Height: 15.00 ft	FBC 8th Ed. 2023 Res.	Creep Factor 2.0	L	Brg Wid = -		Min Req = -			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI 0.931	Bearing B is a rigid surface.						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.595	Members not listed have forces less than 375#						
Spacing: 24.0"	MWFRS Parallel Dist: h/2 to h	FT/RT-20(0)/10(0)	Max Web CSI: 0.926	Maximum Top Chord Forces Per Ply (lbs)						
	C&C Dist a: 5.23 ft	Plate Type(s):	VIEW Ver: 23.02.04.0123.14	Chords	Tens.Comp.		Chords	Tens. Comp.		
	Loc. from endwall: not in 6.50 ft	WAVE, HS		B - C	1185	-3776	G - H	1781	-5338	
	GCpi: 0.18			C - D	1222	-3568	H - I	1667	-4846	
	Wind Duration: 1.60			D - E	1588	-4406	I - J	1381	-3766	

Lumber
 Top chord: 2x4 SP #2; T2, T3 2x4 SP M-31;
 Bot chord: 2x4 SP M-31, B4 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes
 All plates are 3X4 except as noted

Hangers / Ties
 Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
 Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
 Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
 Bearing at location x=52'1" uses the following support conditions: 52'1"
 Bearing L (52'1", 9'1"2) HUS26
 Supporting Member (2)2x6 SP 2400f-2.0E
 (14) 0.148"x3" nails into supporting member,
 (6) 0.148"x3" nails into supported member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

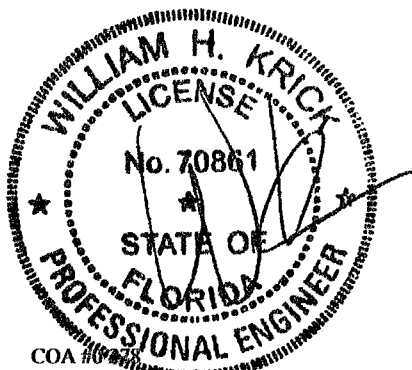
Wind
 Wind loads based on MWFRS with additional C&C member design
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - T	3181 - 1008	Q - P	5197 - 1637
T - S	3182 - 1010	P - O	4888 - 1542
S - R	3034 - 969	O - N	3837 - 1269
R - Q	4463 - 1462	N - M	2082 - 680

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - R	1800 - 619	O - I	1352 - 432
R - E	468 - 1046	I - N	539 - 1309
E - Q	947 - 310	N - J	2208 - 737
Q - F	270 - 504	J - M	311 - 626
G - P	211 - 388	M - K	2195 - 714
P - H	597 - 219	K - L	749 - 2122
H - O	353 - 776		



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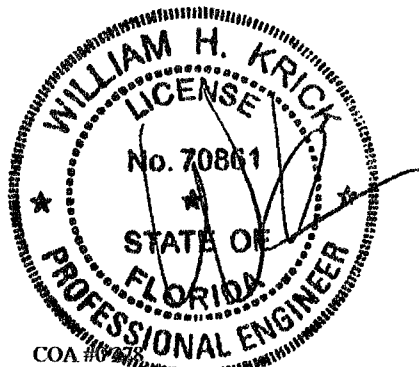


SEQN: 802205	HIPS	Ply: 1	Job Number: 25-2306	Cust: R 215	JRef: 1Y7W2150003	T68
FROM: CDM		Qty: 1	MOSS	DrwNo: 062.25.0538.16637		
Page 2 of 2			Truss Label: G02	KD / DF	03/03/2025	

Additional Notes

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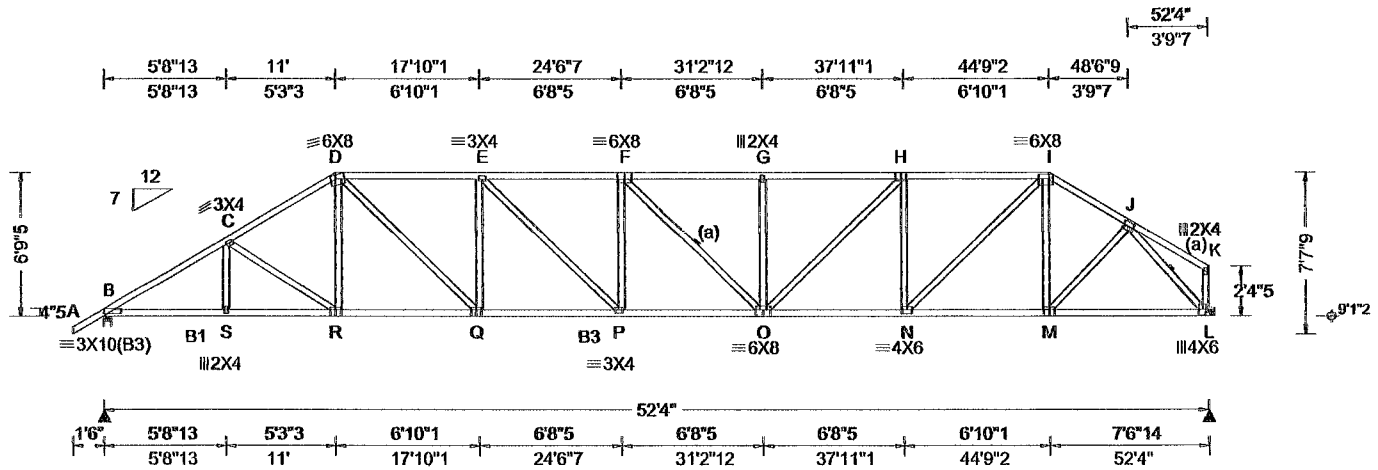
The overall height of this truss excluding overhang is 5-7-5.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40 00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.23 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.371 F 999 240 VERT(CL): 0.768 F 815 180 HORZ(LL): 0.110 L - - HORZ(TL): 0.227 L - - Creep Factor 2.0 Max TC CSI 0.999 Max BC CSI 0.874 Max Web CSI 0.992 VIEW Ver 23.02.04.0123.14	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 2285 /- /- /1327 /413 /193 L 2168 /- /- /1175 /390 /- Wind reactions based on MWFRS B Brg Wid = 4 0 Min Req = 1.9 (Truss) L Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp B - C 1155 -3782 F - G 1478 -4243 C - D 1175 -3446 G - H 1478 -4243 D - E 1414 -3928 H - I 1296 -3503 E - F 1510 -4370 I - J 946 -2607

Lumber
Top chord 2x4 SP #2;
Bot chord 2x4 SP #2; B1,B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 5X6 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

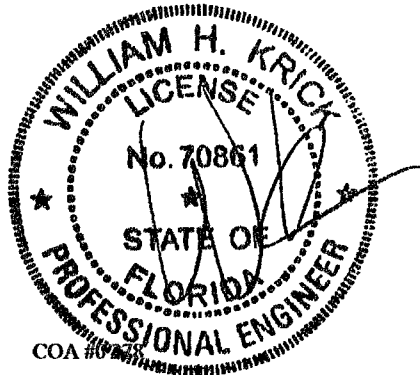
Additional Notes
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The overall height of this truss excluding overhang is 6-9-5.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - S	3180 -972	P - O	4382 -1338
S - R	3179 -973	O - N	3553 -1135
R - Q	2911 -892	N - M	2209 -676
Q - P	3966 -1257	M - L	1767 -581

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - R	394 -21	O - H	970 -327
D - Q	1414 -490	H - N	487 -1123
Q - E	411 -852	N - I	1800 -608
E - P	574 -209	M - J	653 -161
G - O	223 -407	J - L	877 -2640



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Hangers / Ties

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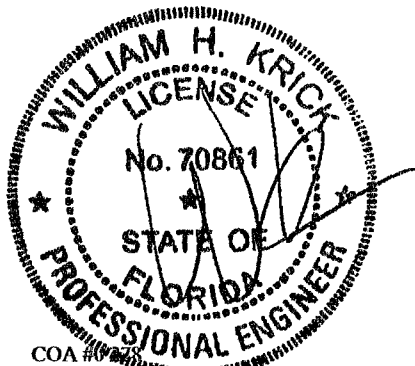
Bearing at location x=52"1" uses the following support conditions: 52"1"

Bearing L (52"1", 9"1"2) HUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(14) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member



Florida Certificate of Product Approval #FL 1999

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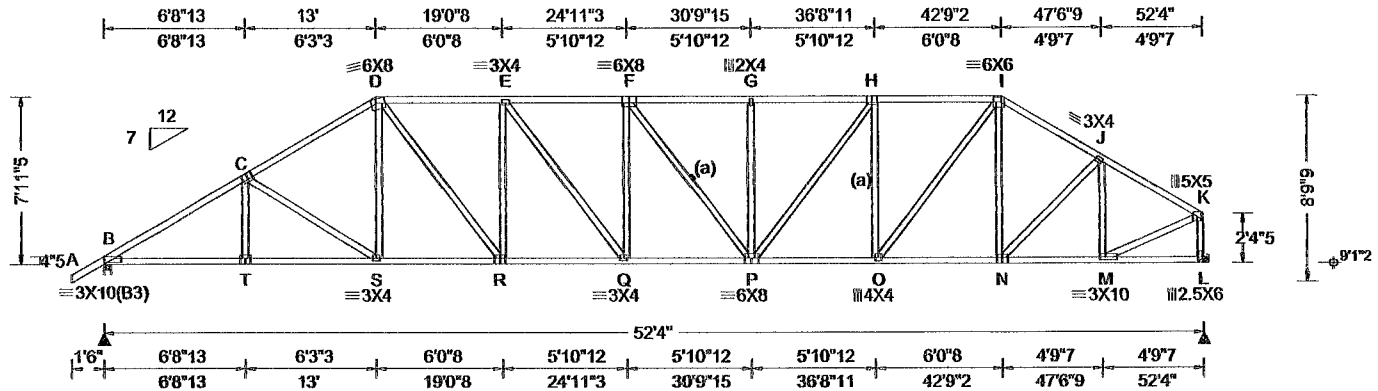
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)	
TCLL: 20.00	Wind Std ASCE 7-22	Pg: NA Ct: NA CAT NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.315 F 999 240	Loc R+ /R- /Rh /Rw /U /RL	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.651 F 961 180	B 2285 /- /- /1347 /410 /226	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(TL): 0.112 L - -	L 2168 /- /- /1196 /387 /-	
Des Ld: 40.00	EXP: C Kzt: NA		Creep Factor 2.0	Wind reactions based on MWFRS	
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Max TC CSI: 0.672	B Brg Wid = 4 0 Min Req = 2 7 (Truss)	
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed 2023 Res.	Max BC CSI: 0.942	L Brg Wid = - Min Req = -	
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max Web CSI: 0.846	Bearing B is a rigid surface.	
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes		Members not listed have forces less than 375#	
	C&C Dist a: 5.23 ft	FT/RT.20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.	
	GCpi: 0.18	WAVE		B - C 1120 -3774 G - H 1296 -3608	
	Wind Duration: 1.60		VIEW Ver 23.02.04.0123.14	C - D 1125 -3314 H - I 1172 -3108	

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

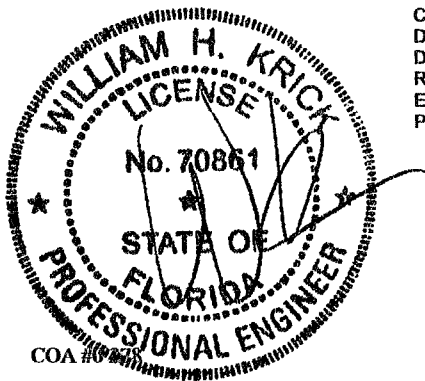
Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
 All plates are 5X6 except as noted.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 7-11-5.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - T	3164 -932	Q - P	3713 -1101
T - S	3163 -933	P - O	3146 -963
S - R	2777 -808	O - N	2243 -663
R - Q	3451 -1051	N - M	1988 -603

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - S	176 -460	H - O	443 -1000
D - S	446 -39	O - I	1416 -481
D - R	1058 -384	J - M	311 -759
R - E	363 -726	M - K	2117 -635
E - Q	429 -169	K - L	697 -2125
P - H	770 -270		

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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

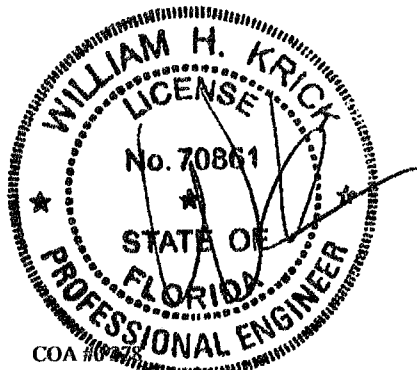
Bearing at location x=52"1" uses the following support conditions: 52"1"

Bearing L (52"1", 9"1"2) HUS26

Supporting Member: (2)2x6 SP 2400F-2.0E

(14) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member



Florida Certificate of Product Approval #FL 1999

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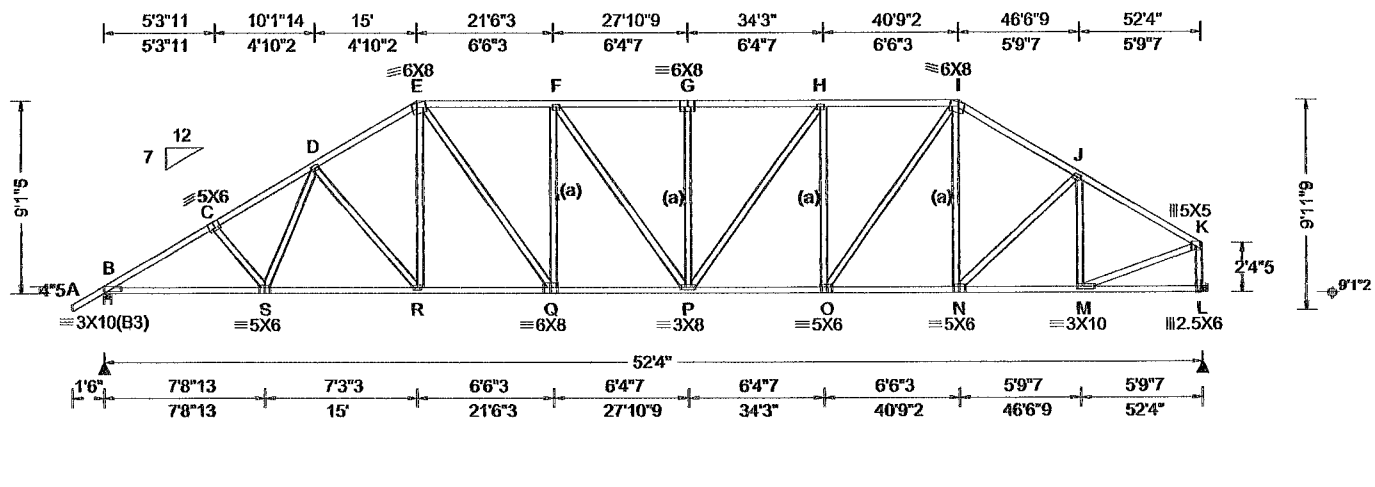
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL 20.00 TCCL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40 00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist: 5.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.266 G 999 240 VERT(CL): 0.551 G 999 180 HORZ(LL): 0.102 L - - HORZ(TL): 0.211 L - - Creep Factor: 2.0 Max TC CSI: 0.695 Max BC CSI: 0.997 Max Web CSI: 0.886 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 2285 /- /- /1364 /145 /259 L 2168 /- /- /1214 /108 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 2.7 (Truss) L Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1099 -3796 G - H 1171 -3220 C - D 1105 -3609 H - I 1113 -2921 D - E 1085 -3133 I - J 969 -2693 E - F 1161 -3119 J - K 784 -2490 F - G 1171 -3220
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord 2x4 SP #2;
 Webs: 2x4 SP #3;

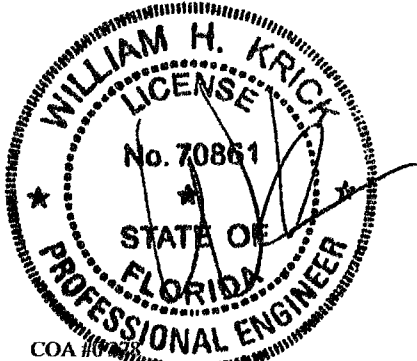
Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
 All plates are 3X4 except as noted.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
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 The overall height of this truss excluding overhang is 9'-1-5/8".



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - S	3194 -927	P - O	2947 -858
S - R	2943 -833	O - N	2243 -613
R - Q	2639 -724	N - M	2108 -608
Q - P	3137 -908		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - R	197 -476	H - O	385 -808
E - R	540 -88	O - I	1155 -395
E - Q	818 -307	J - M	278 -626
Q - F	312 -541	M - K	2189 -622
P - H	473 -184	K - L	677 -2118

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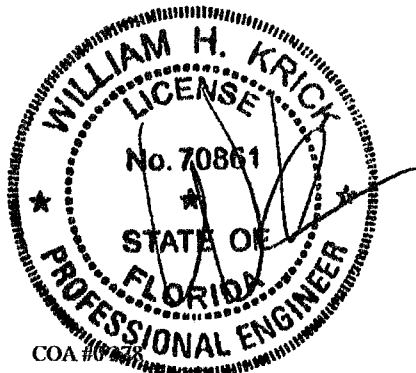
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Supporting Member: (2)2x6 SP 2400F-2.0E

(14) 0.148"x3" nails into supporting member,

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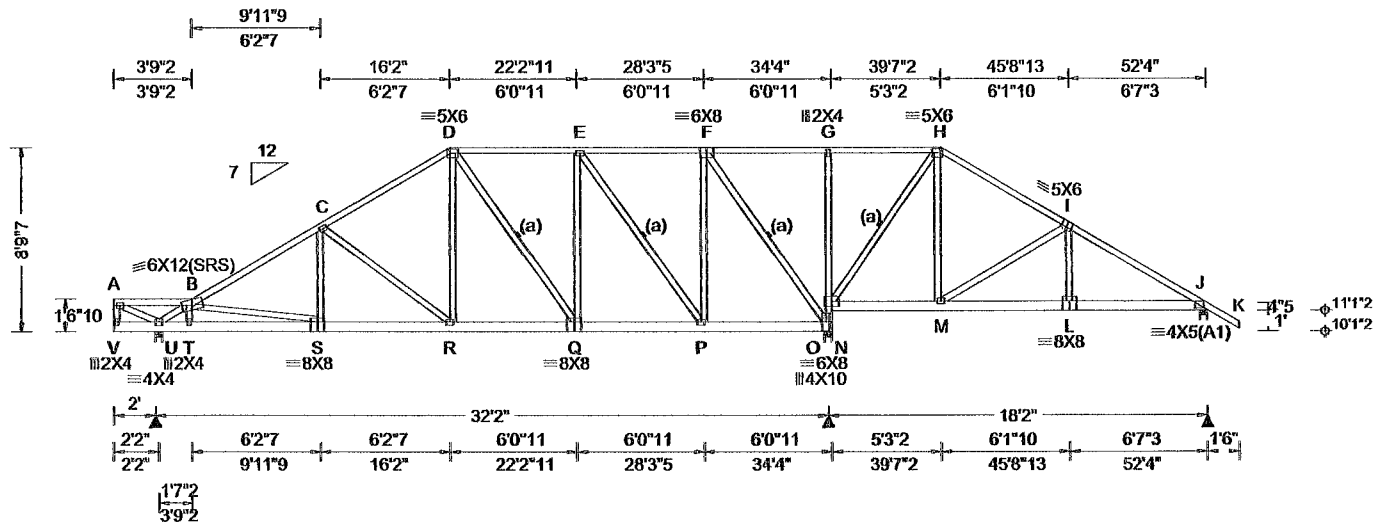


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.059 R 999 240 VERT(CL): 0.123 R 999 180 HORZ(LL): 0.020 O - - HORZ(TL): 0.042 O - - Creep Factor: 2.0 Max TC CSI: 0.467 Max BC CSI: 0.365 Max Web CSI: 0.730 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity U 1484 /- /- /893 /65 /238 O 2167 /- /- /1144 /150 /- J 830 /- /- /622 /86 /- Wind reactions based on MWFRS U Brg Wid = 4.0 Min Req = 1.5 (Truss) O Brg Wid = 4.0 Min Req = 2.6 (Truss) J Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings U, O, & J are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3;

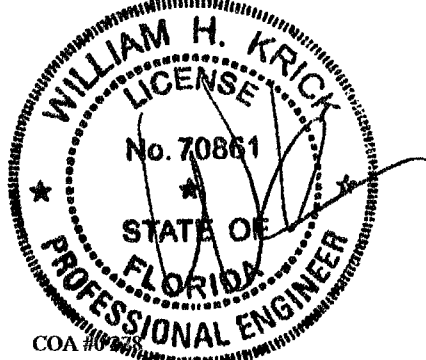
Bracing
(a) Continuous lateral restraint equally spaced on member

Plating Notes
All plates are 3X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design
Left end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

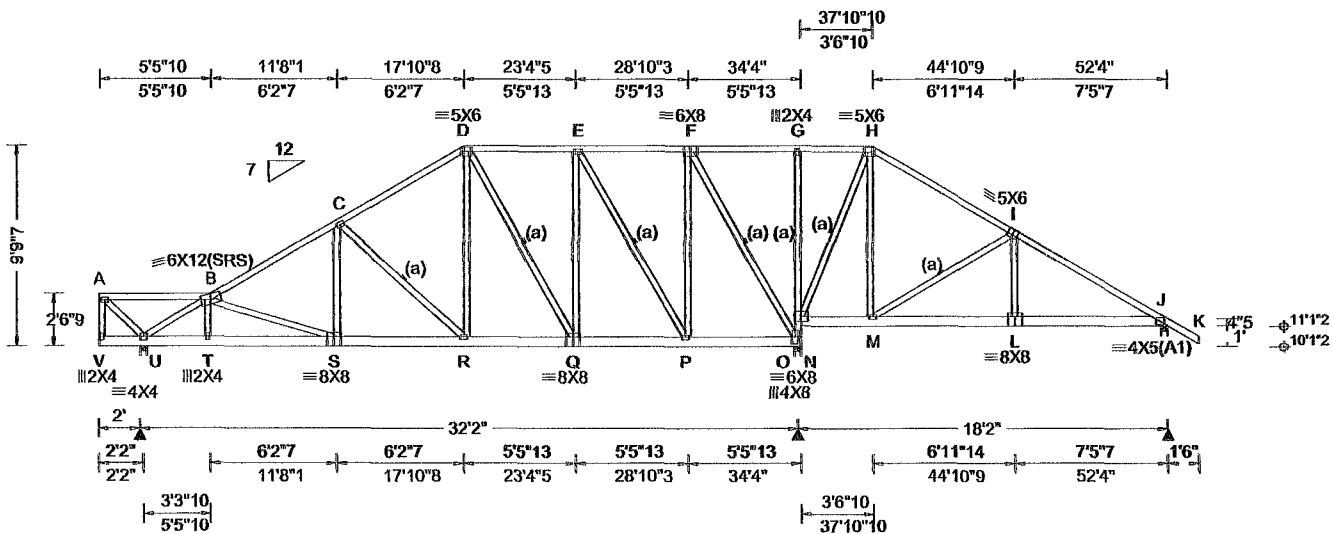
Additional Notes
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The overall height of this truss excluding overhang is 8-9.7



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category II EXP-C Kzt: NA Mean Height: 15.23 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce NA Lur: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.062 S 999 240 VERT(CL): 0.130 S 999 180 HORZ(LL): 0.020 O - - HORZ(TL): 0.041 O - - Creep Factor: 2.0 Max TC CSI: 0.558 Max BC CSI: 0.376 Max Web CSI: 0.779 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity U 1483 /- /- /871 /104 /254 O 2169 /- /- /1159 /112 /- J 830 /- /- /636 /99 /- Wind reactions based on MWFRS U Brg Wid = 4.0 Min Req = 1.5 (Truss) O Brg Wid = 4.0 Min Req = 2.6 (Truss) J Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings U, O, & J are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens Comp.
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #3,	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp U - T 1761 -522 Q - P 942 -253 T - S 1760 -528 P - O 534 -128 S - R 1504 -405 M - L 765 -169 R - Q 1087 -242 L - J 788 -168
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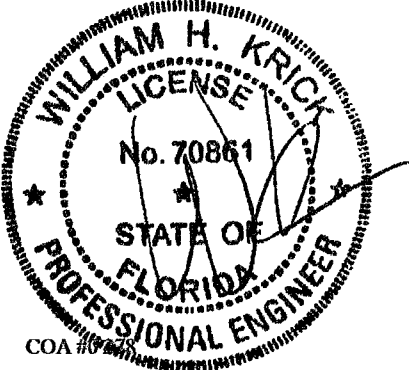
Bracing (a) Continuous lateral restraint equally spaced on member	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. U - B 823 -2205 F - O 406 -1324 C - R 223 -585 O - N 279 -960 D - R 530 -94 N - H 94 -703 Q - E 391 -25 H - M 466 -49 E - P 248 -804 M - I 231 -648 P - F 825 -157
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Plating Notes All plates are 3X4 except as noted.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp U - T 1761 -522 Q - P 942 -253 T - S 1760 -528 P - O 534 -128 S - R 1504 -405 M - L 765 -169 R - Q 1087 -242 L - J 788 -168
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Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. U - B 823 -2205 F - O 406 -1324 C - R 223 -585 O - N 279 -960 D - R 530 -94 N - H 94 -703 Q - E 391 -25 H - M 466 -49 E - P 248 -804 M - I 231 -648 P - F 825 -157
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Wind Wind loads based on MWFRS with additional C&C member design Left end vertical not exposed to wind pressure. Left cantilever is exposed to wind Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp U - T 1761 -522 Q - P 942 -253 T - S 1760 -528 P - O 534 -128 S - R 1504 -405 M - L 765 -169 R - Q 1087 -242 L - J 788 -168
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Additional Notes WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. The overall height of this truss excluding overhang is 9-9-7	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. U - B 823 -2205 F - O 406 -1324 C - R 223 -585 O - N 279 -960 D - R 530 -94 N - H 94 -703 Q - E 391 -25 H - M 466 -49 E - P 248 -804 M - I 231 -648 P - F 825 -157
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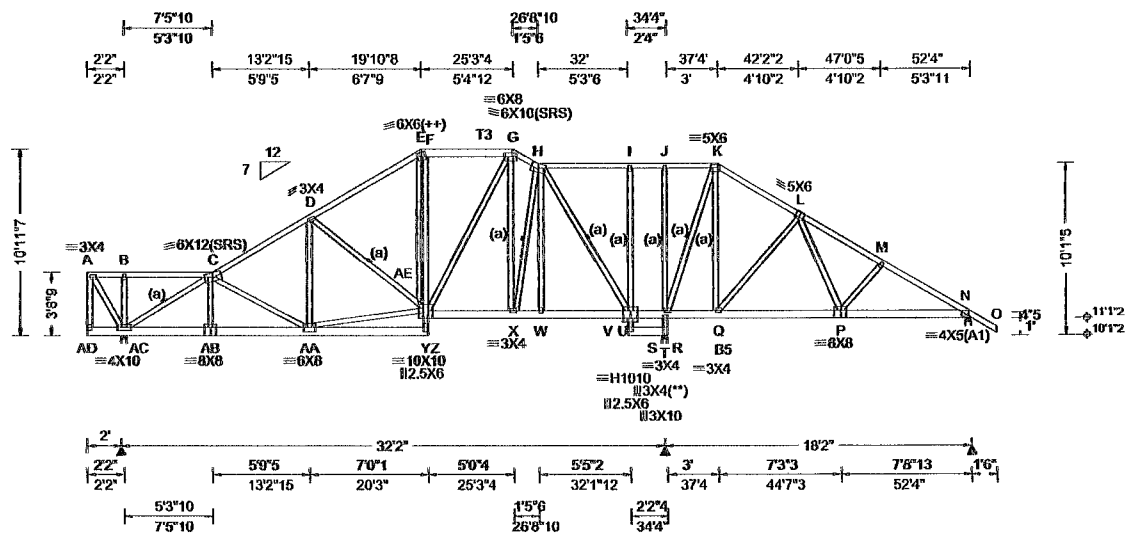


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SEQN: 802253 SPEC Qty 1 Job Number: 25-2306 Cust: R 215 JRef: 1Y7W2150003 T52
 FROM: CDM Truss Label, G08 DrwNo: 062.25.0538.34000
 KD / DF 03/03/2025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40 00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.81 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall not in 13.00 ft GCpi: 0.18 Wind Duration: 1 60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.151 W 999 240 VERT(CL): 0.310 W 999 180 HORZ(LL): 0.157 S - - HORZ(TL): 0.326 S - - Creep Factor: 2.0 Max TC CSI: 0.856 Max BC CSI: 0.938 Max Web CSI: 0.722 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AC 1571 - / - / 1910 / 110 / 283 S 1985 - / - / 1057 / 90 - N 917 - / - / 622 / 38 - Wind reactions based on MWFRS AC Brg Wid = 4.0 Min Req = 1.5 (Truss) S Brg Wid = 4.0 Min Req = 2.3 (Truss) N Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AC, S, & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
 Top chord: 2x4 SP #2; T3 2x6 SP #2;
 Bot chord 2x6 SP #2; B5 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3,

Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
 All plates are 2X4 except as noted.
 (++) - This plate works for both joints covered.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

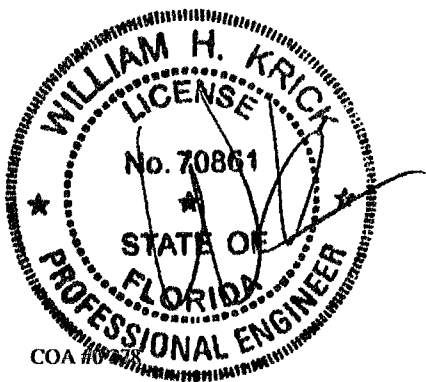
Wind
 Wind loads based on MWFRS with additional C&C member design
 Left end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).

Additional Notes
 WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 10-11-7

Chords	Tens.Comp.	Chords	Tens. Comp.
C - D	624 - 1882	G - H	630 - 1242
D - E	617 - 1542	K - L	270 - 437
E - F	572 - 1174	L - M	322 - 1023
F - G	600 - 1246	M - N	320 - 1211

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
AC-AB	1831 - 475	W - U	1069 - 174
AB-AA	1828 - 478	Q - P	660 - 44
Y - X	1041 - 142	P - N	988 - 160
X - W	1062 - 172		

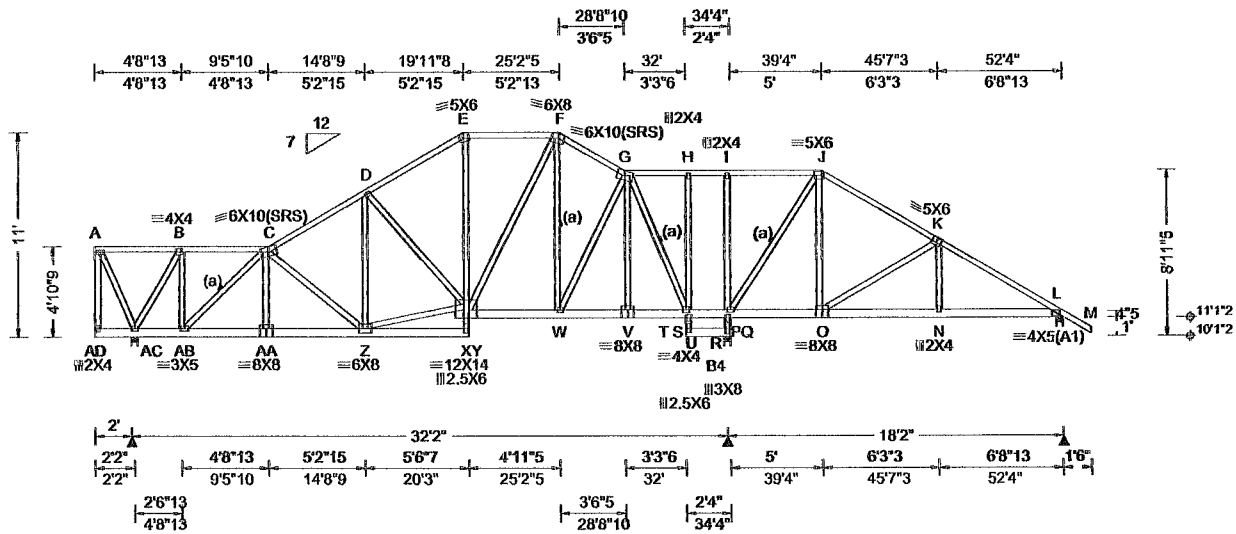
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
AC - C	711 - 2144	H - U	466 - 1369
AA - Y	1546 - 332	J - R	223 - 486
D - AE	200 - 409	T - S	707 - 2015
AE - Y	187 - 378	Q - L	227 - 558
Y - G	453 - 100	L - P	462 - 58
H - W	393 - 95		



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40 00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.39 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lir: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.136 U 999 240 VERT(CL): 0.281 V 999 180 HORZ(LL): 0.153 Q - - HORZ(TL): 0.318 Q - - Creep Factor: 2.0 Max TC CSI: 0.702 Max BC CSI: 0.477 Max Web CSI: 0.781 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AC</td> <td>1561</td> <td>-</td> <td>-</td> <td>/874</td> <td>/105</td> <td>/283</td> </tr> <tr> <td>Q</td> <td>2003</td> <td>-</td> <td>-</td> <td>/1061</td> <td>/39</td> <td>-</td> </tr> <tr> <td>L</td> <td>904</td> <td>-</td> <td>-</td> <td>/618</td> <td>/31</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS AC Brg Wid = 4.0 Min Req = 1.5 (Truss) Q Brg Wid = 4.0 Min Req = 2.4 (Truss) L Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AC, Q, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>196 -749</td> <td>F - G</td> <td>313 -1224</td> </tr> <tr> <td>C - D</td> <td>365 -1732</td> <td>J - K</td> <td>217 -599</td> </tr> <tr> <td>D - E</td> <td>322 -1499</td> <td>K - L</td> <td>251 -1171</td> </tr> <tr> <td>E - F</td> <td>325 -1226</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AC	1561	-	-	/874	/105	/283	Q	2003	-	-	/1061	/39	-	L	904	-	-	/618	/31	-	Chords	Tens.Comp.	Chords	Tens.Comp.	B - C	196 -749	F - G	313 -1224	C - D	365 -1732	J - K	217 -599	D - E	322 -1499	K - L	251 -1171	E - F	325 -1226		
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP #2; B4 2x6 SP 2400F-2.0E;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
 All plates are 3X4 except as noted.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design
 Left end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
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 The overall height of this truss excluding overhang is 11-0-0.

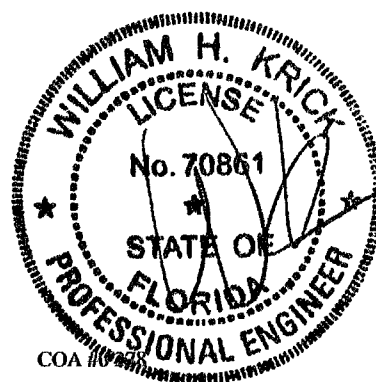
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens.Comp.
AC-AB	684 -157	V - T	980 0
AB-AA	1730 -217	P - O	423 0
AA-Z	1727 -219	O - N	931 -97
X - W	1016 0	N - L	935 -96
W - V	970 0		

Maximum Web Forces Per Ply (lbs)

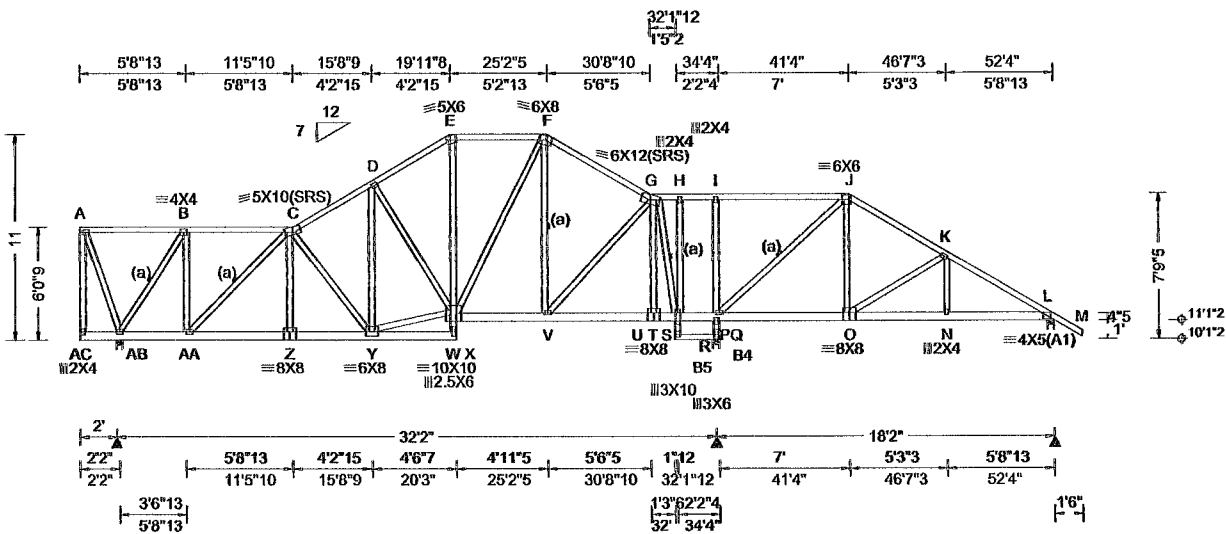
Webs	Tens.Comp.	Webs	Tens.Comp.
AC - B	468 -1522	G - V	487 -52
B - AB	1001 -141	G - T	255 -1615
AB - C	279 -1383	I - P	258 -533
C - Z	199 -405	R - Q	583 -2034
Z - X	1455 -67	P - J	146 -496
E - X	401 -130	O - K	216 -601
X - F	464 -81		



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP C Kzt: NA Mean Height: 16.60 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lur: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.123 T 999 240 VERT(CL): 0.257 T 999 180 HORZ(LL): 0.158 Q - - HORZ(TL): 0.328 Q - - Creep Factor 2.0 Max TC CSI: 0.790 Max BC CSI: 0.455 Max Web CSI: 0.571 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity AB 1545 /- /- /837 /147 /281 Q 2052 /- /- /1090 /36 /- L 873 /- /- /598 /34 /- Wind reactions based on MWFRS AB Brg Wid = 4.0 Min Req = 1.5 (Truss) Q Brg Wid = 4.0 Min Req = 2.4 (Truss) L Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AB, Q, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2; B4 2x6 SP 2400F-2.0E;
B5 2x4 SP #2;
Webs: 2x4 SP #3,

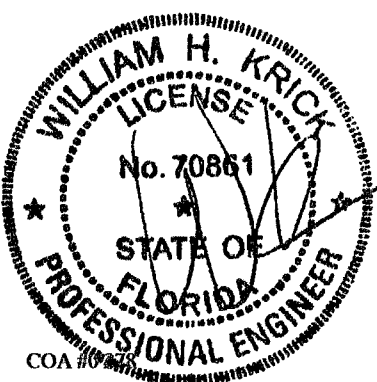
Bracing
(a) Continuous lateral restraint equally spaced on member

Plating Notes
All plates are 3X4 except as noted

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
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The overall height of this truss excluding overhang is 11'-0".

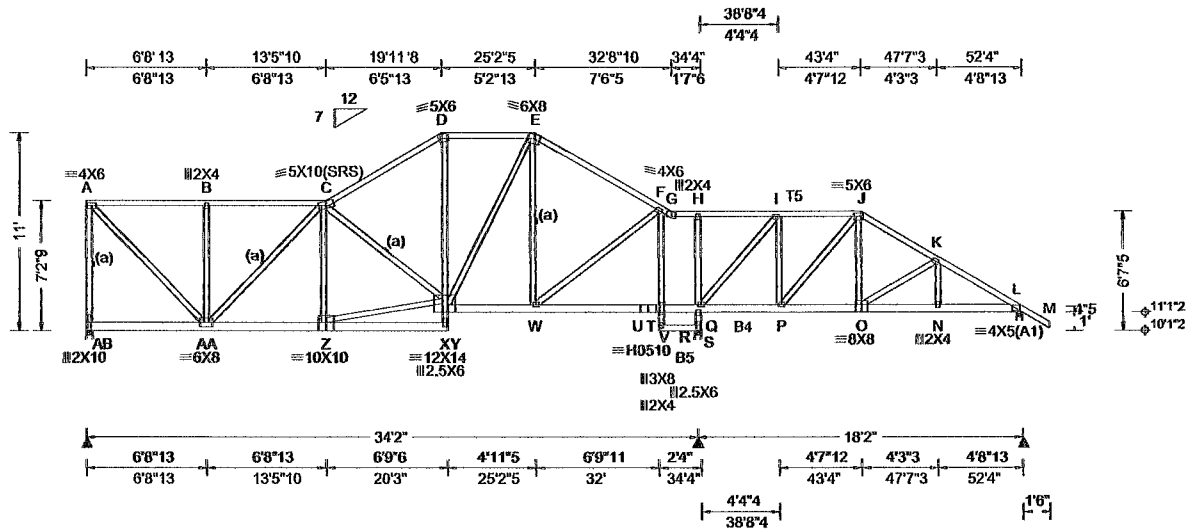


Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.			
B - C	238 -792	F - G	288 -1222
C - D	363 -1602	J - K	239 -679
D - E	319 -1425	K - L	265 -1143
E - F	312 -1180		
Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.			
AB-AA	747 -107	U - S	769 0
AA-Z	1592 -133	P - O	515 0
Z - Y	1590 -134	O - N	918 -119
W - V	987 0	N - L	921 -119
V - U	751 0		
Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.			
AB - B	495 -1528	G - U	791 -32
B - AA	885 -87	G - S	336 -2156
AA - C	206 -1142	S - H	517 -154
C - Y	212 -453	I - P	382 -694
Y - W	1361 -13	R - Q	607 -2085
E - W	398 -151	P - J	222 -587
W - F	427 -78	O - K	170 -479
V - G	404 0		

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Loading Criteria (psf) TCLL. 20.00 TC DL. 10.00 BC LL. 0.00 BC DL. 10.00 Des Ld. 40.00 NCBCLL. 10.00 Soffit. 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std. ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP: C Kzt: NA Mean Height: 16.60 ft TC DL. 5.0 psf BC DL. 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.23 ft Loc. from endwall. not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.154 U 999 240 VERT(CL): 0.311 U 999 180 HORZ(LL): 0.172 R - - HORZ(TL): 0.356 R - - Creep Factor: 2.0 Max TC CSI: 0.752 Max BC CSI: 0.740 Max Web CSI: 0.976 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs)																									
				<table border="1"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AB</td> <td>1529</td> <td>-</td> <td>-</td> <td>788</td> <td>142 1278</td> </tr> <tr> <td>R</td> <td>1885</td> <td>-</td> <td>-</td> <td>998</td> <td>117 -</td> </tr> <tr> <td>L</td> <td>1044</td> <td>-</td> <td>-</td> <td>690</td> <td>146 -</td> </tr> </tbody> </table> Wind reactions based on MWFRS AB Brg Wid = 4.0 Min Req = 1.8 (Truss) R Brg Wid = 4.0 Min Req = 2.2 (Truss) L Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AB, R, & L are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)		Gravity		Non-Gravity		Loc	R+	/Rh	/Rw	/U	/RL	AB	1529	-	-	788	142 1278	R	1885	-	-	998	117 -	L	1044
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Lumber
 Top chord: 2x4 SP #2; T5 2x4 SP M-31,
 Bot chord 2x6 SP #2; B4 2x6 SP 2400f-2.0E;
 B5 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
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Purins
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Wind
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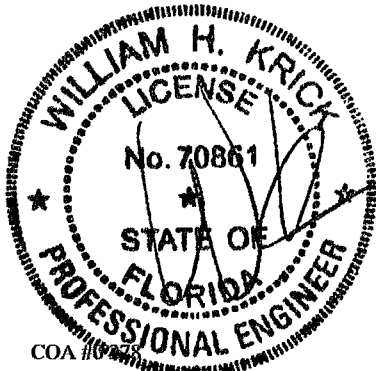
Additional Notes
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Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Chords	Tens.Comp.	Chords	Tens Comp
A - B	409 - 1282	G - H	284 - 993
B - C	409 - 1282	H - I	279 - 980
C - D	419 - 1862	I - J	374 - 897
D - E	422 - 1526	J - K	405 - 1187
E - F	376 - 1623	K - L	414 - 1486
F - G	252 - 1014		

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens Comp
AA - Z	1947 - 240	S - Q	995 - 18
X - W	1318 0	Q - P	908 - 128
W - V	1008 - 21	P - O	968 - 148
V - T	1008 - 21	O - N	1228 - 261
T - S	901 0	N - L	1229 - 260

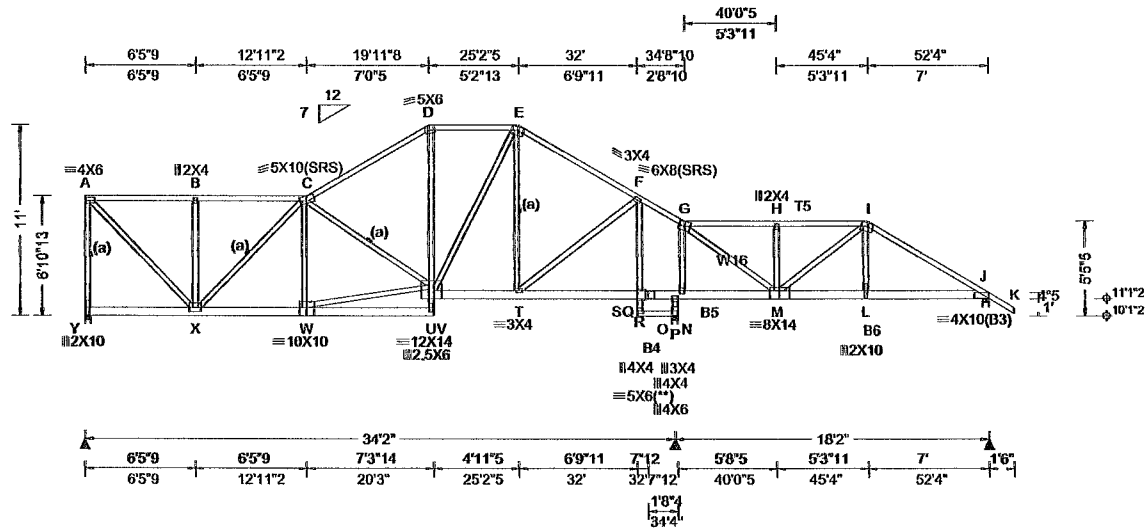
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens Comp.
A - AB	532 - 1470	X - E	442 - 86
A - AA	1827 - 583	W - F	424 - 2
B - AA	374 - 483	F - T	247 - 956
AA - C	179 - 947	H - Q	179 - 638
C - X	353 - 558	S - R	539 - 1912
Z - X	1945 - 239	Q - I	428 - 359
D - X	532 - 145		



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Loading Criteria (psf) TCLL: 20.00 TCDD: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 16.60 ft TCDD: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist: 5.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lr: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.130 U 999 240 VERT(CL): 0.262 U 999 180 HORZ(LL): 0.222 O - - HORZ(TL): 0.454 O - - Creep Factor: 2.0 Max TC CSI: 0.831 Max BC CSI: 0.910 Max Web CSI: 0.937 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/Rh</th> <th>/Rw</th> <th>/U /RL</th> </tr> </thead> <tbody> <tr> <td>Y</td> <td>1534</td> <td>-</td> <td>-</td> <td>- /272</td> </tr> <tr> <td>O</td> <td>3290</td> <td>-</td> <td>-</td> <td>- /445</td> </tr> <tr> <td>J</td> <td>2509</td> <td>-</td> <td>-</td> <td>- /375</td> </tr> </tbody> </table> Wind reactions based on MWFRS Y Brg Wid = 4.0 Min Req = 1.8 (Truss) O Brg Wid = 4.0 Min Req = 3.9 (Truss) J Brg Wid = 4.0 Min Req = 2.1 (Truss) Bearings Y, O, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>231 - 1302</td> <td>F - G</td> <td>244 - 1286</td> </tr> <tr> <td>B - C</td> <td>231 - 1302</td> <td>G - H</td> <td>426 - 3408</td> </tr> <tr> <td>C - D</td> <td>366 - 1885</td> <td>H - I</td> <td>426 - 3409</td> </tr> <tr> <td>D - E</td> <td>278 - 1540</td> <td>I - J</td> <td>603 - 4390</td> </tr> <tr> <td>E - F</td> <td>314 - 1632</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	/Rh	/Rw	/U /RL	Y	1534	-	-	- /272	O	3290	-	-	- /445	J	2509	-	-	- /375	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	231 - 1302	F - G	244 - 1286	B - C	231 - 1302	G - H	426 - 3408	C - D	366 - 1885	H - I	426 - 3409	D - E	278 - 1540	I - J	603 - 4390	E - F	314 - 1632		
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Lumber
 Top chord: 2x4 SP #2; T5 2x4 SP M-31,
 Bot chord: 2x6 SP #2, B4 2x4 SP #2; B5,
 B6 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3, W16 2x4 SP #2,

Bracing
 (a) Continuous lateral restraint equally spaced on member

Special Loads
 (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 42.46
 TC: From 32 plf at 42.46 to 32 plf at 45.33
 TC: From 63 plf at 45.33 to 63 plf at 53.83
 BC: From 20 plf at 0.00 to 20 plf at 42.46
 BC: From 10 plf at 42.46 to 10 plf at 45.30
 BC: From 20 plf at 45.30 to 20 plf at 52.33
 BC: From 5 plf at 52.33 to 5 plf at 53.83
 TC: 327 lb Conc. Load at 42.46
 TC: 190 lb Conc. Load at 43.27
 TC: 546 lb Conc. Load at 45.30
 BC: 1397 lb Conc. Load at 42.46
 BC: 130 lb Conc. Load at 43.27
 BC: 401 lb Conc. Load at 45.30

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

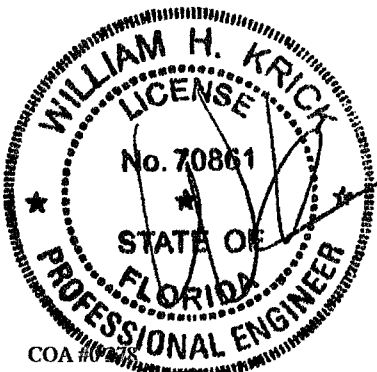
Wind
 Wind loads and reactions based on MWFRS.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
X - W	2019 - 357	P - N	1096 - 204
U - T	1321 - 237	N - M	1176 - 216
T - R	1104 - 206	M - L	3756 - 486
R - Q	909 - 173	L - J	3709 - 488
Q - P	909 - 173		

Maximum Web Forces Per Ply (lbs)

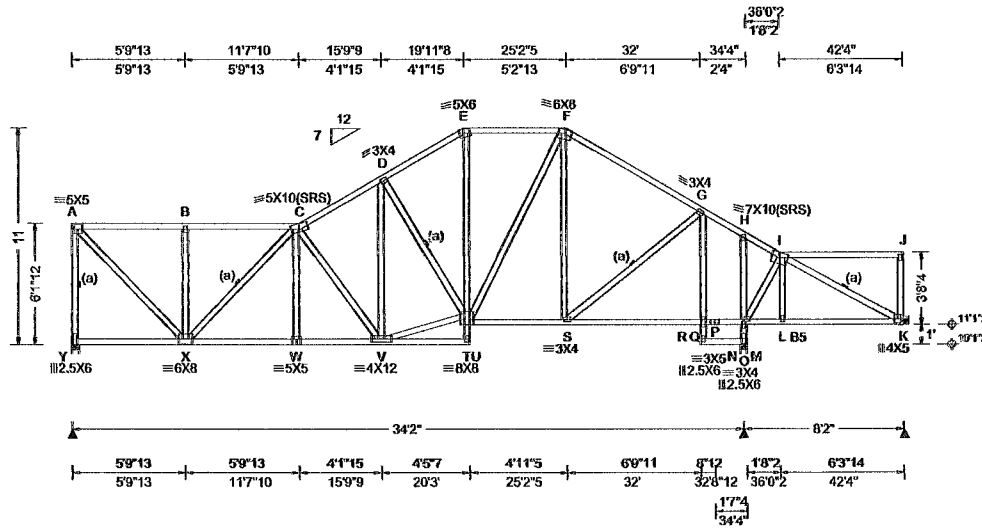
Webs	Tens.Comp.	Webs	Tens. Comp.
A - Y	297 - 1477	F - R	175 - 659
A - X	1850 - 329	P - O	469 - 3359
B - X	179 - 456	N - G	349 - 2560
X - C	178 - 1048	G - M	3210 - 263
C - U	97 - 603	H - M	180 - 606
W - U	2010 - 355	M - I	163 - 647
D - U	550 - 12	L - I	1314 - 0
U - E	468 - 90		



Plating Notes
 All plates are 6X8 except as noted
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

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Loading Criteria (psf) TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing " 24.0 "	Wind Criteria Wind Std. ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP- C Kzt: NA Mean Height: 18.70 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.23 ft Loc. from endwall not in 13.00 ft GCpi: 0.18 Wind Duration: 1 60	Snow Criteria (Pg,Pf in PSF) Pg NA Ct: NA CAT NA Pf NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.130 V 999 240 VERT(CL): 0.269 V 999 180 HORZ(LL): 0.114 N - - HORZ(TL): 0.237 N - - Creep Factor: 2.0 Max TC CSI 0.730 Max BC CSI 0.662 Max Web CSI: 0.758 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL Y 1602 - / - /823 /112 /173 N 831 - / - /451 /3 - K 1085 - / - /578 /8 - Wind reactions based on MWFRS Y Brg Wid = 4.0 Min Req = 1.9 (Truss) N Brg Wid = 4.0 Min Req = 1.5 (Truss) K Brg Wid = - Min Req = - Bearings Y & N are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B5 2x4 SP M-31,
Webs: 2x4 SP #3,

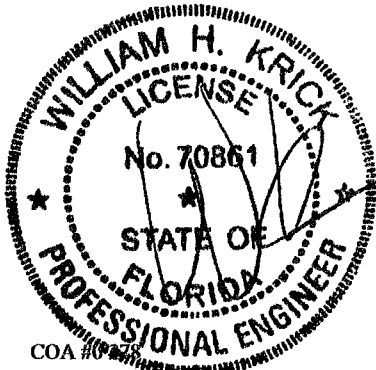
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 11-0-0.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).



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SEQN: 802287	SPEC	Ply: 1	Job Number: 25-2306	Cust: R 215	JRef: 1Y7W2150003	T24
FROM: CDM		Qty: 1	MOSS	DrawNo: 062.25.0539.10740		
Page 2 of 2			Truss Label: G13	KD / DF		03/03/2025

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

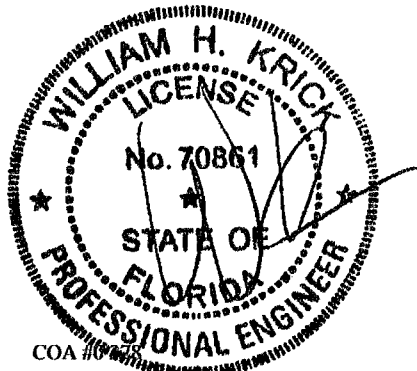
Bearing at location x=42'1" uses the following support conditions: 42'1"

Bearing K (42'1", 11'1"2) LUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member



COA #03/03/2025

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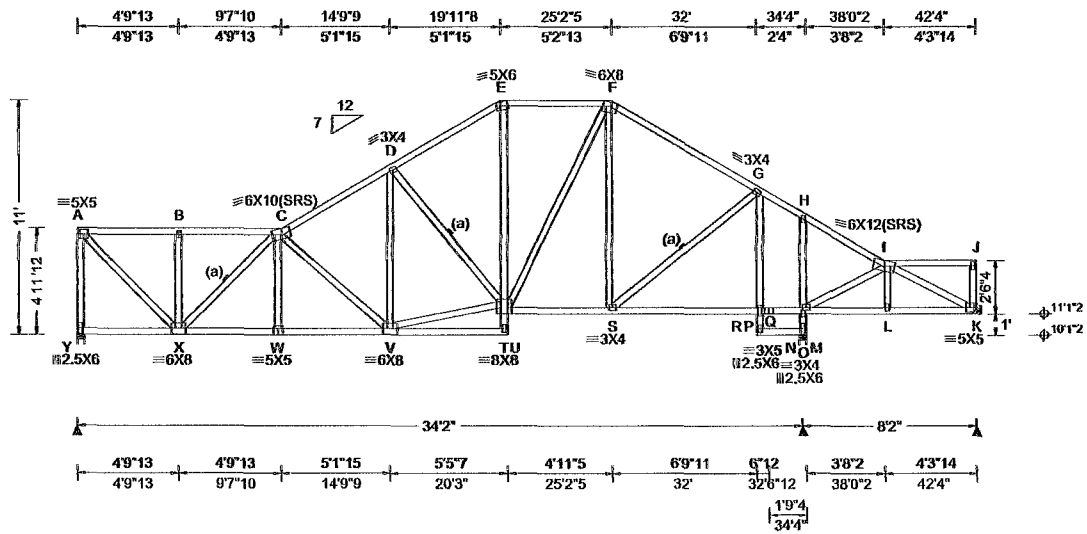
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.12 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.143 T 999 240 VERT(CL): 0.296 V 999 180 HORZ(LL): 0.123 N - - HORZ(TL): 0.255 N - - Creep Factor: 2.0 Max TC CSI: 0.783 Max BC CSI: 0.973 Max Web CSI: 0.825 VIEW Ver: 23.02.04.0123.14	Gravity Loc R+ /R- /Rh Y 1612 - /- /- /850 /70 /205 N 801 - /- /- /466 /11 /- K 1109 - /- /- /587 - /- /- Non-Gravity Loc R+ /R- /Rh Y 1612 - /- /- /850 /70 /205 N 801 - /- /- /466 /11 /- K 1109 - /- /- /587 - /- /- Wind reactions based on MWFRS Y Brg Wid = 4.0 Min Req = 1.9 (Truss) N Brg Wid = 4.0 Min Req = 1.5 (Truss) K Brg Wid = - Min Req = - Bearings Y & N are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber Top chord: 2x4 SP #2, Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. X-W 2520 -530 P-O 1534 -158 W-V 2517 -531 O-M 1568 -173 T-S 1507 -76 M-L 1792 -273 S-Q 1570 -174 L-K 1791 -269 Q-P 1534 -158
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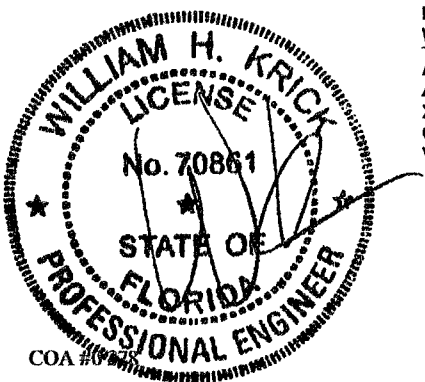
Bracing
(a) Continuous lateral restraint equally spaced on member

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

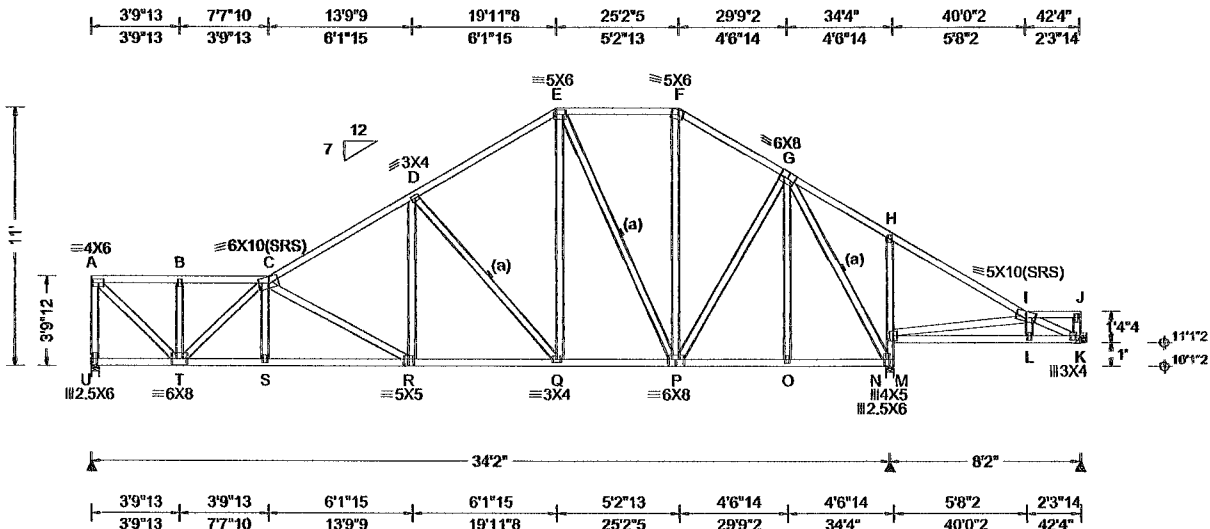
Additional Notes
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Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).



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 Webs: 2x4 SP #3;

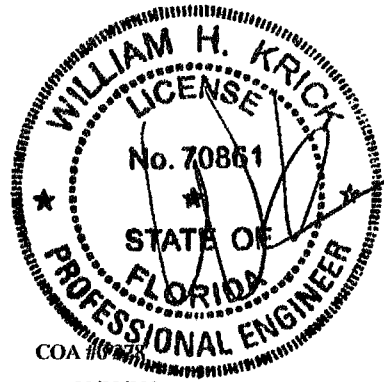
Bracing
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Plating Notes
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Purlins
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 Wind loads based on MWFRS with additional C&C member design
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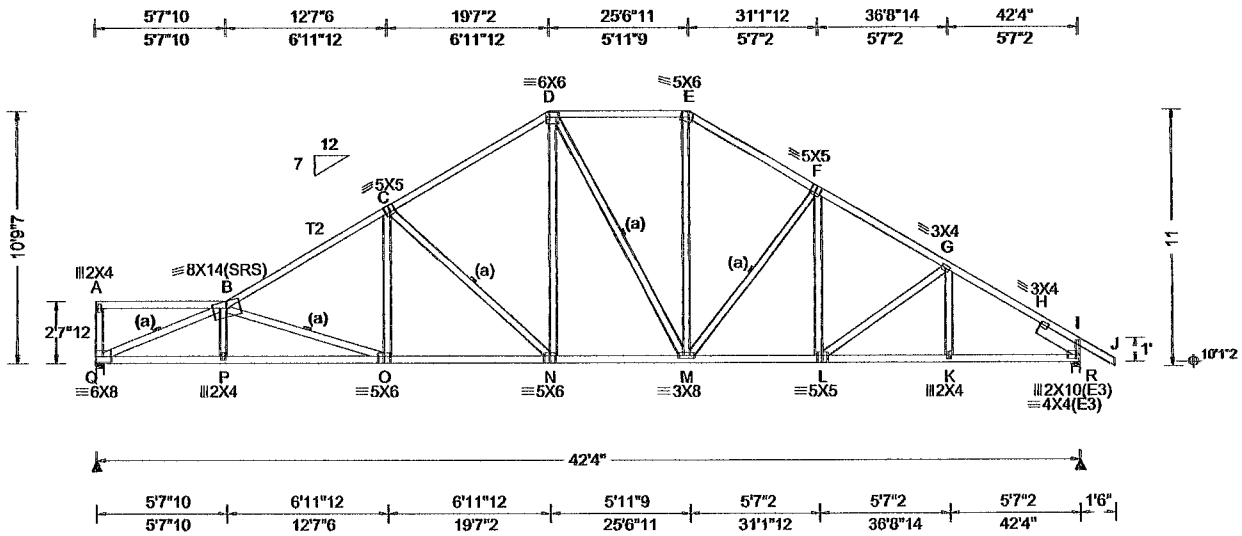
Additional Notes
 The overall height of this truss excluding overhang is 11-0-0.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL. 20 00 TCDL. 10 00 BCLL. 0.00 BCDL. 10.00 Des Ld. 40 00 NCBCLL. 10.00 Soffit. 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP-C Kzt: NA Mean Height: 16.42 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 4.23 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.222 O 999 240 VERT(CL): 0.458 O 999 180 HORZ(LL): 0.130 H - - HORZ(TL): 0.269 H - - Creep Factor: 2.0 Max TC CSI: 0.949 Max BC CSI: 0.955 Max Web CSI: 0.946 VIEW Ver 23.02.04.0123.14	Gravity Loc R+ /R- /Rh /Rw /U /RL Q 1757 /- /- /971 /- /284 R 1862 /- /- /1113 /- /- Non-Gravity Wind reactions based on MWFRS Q Brg Wid = 4.0 Min Req = 2.1 (Truss) R Brg Wid = 4.0 Min Req = 2.2 (Truss) Bearings Q & R are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 475 -2982 F - G 383 -2426 C - D 392 -2188 G - H 364 -2616 D - E 373 -1715 H - I 387 -2695 E - F 375 -2070

Lumber
Top chord: 2x4 SP #2; T2 2x4 SP M-31,
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Slider: 2x6 SP #2; block length = 2.093'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 10-9-7.

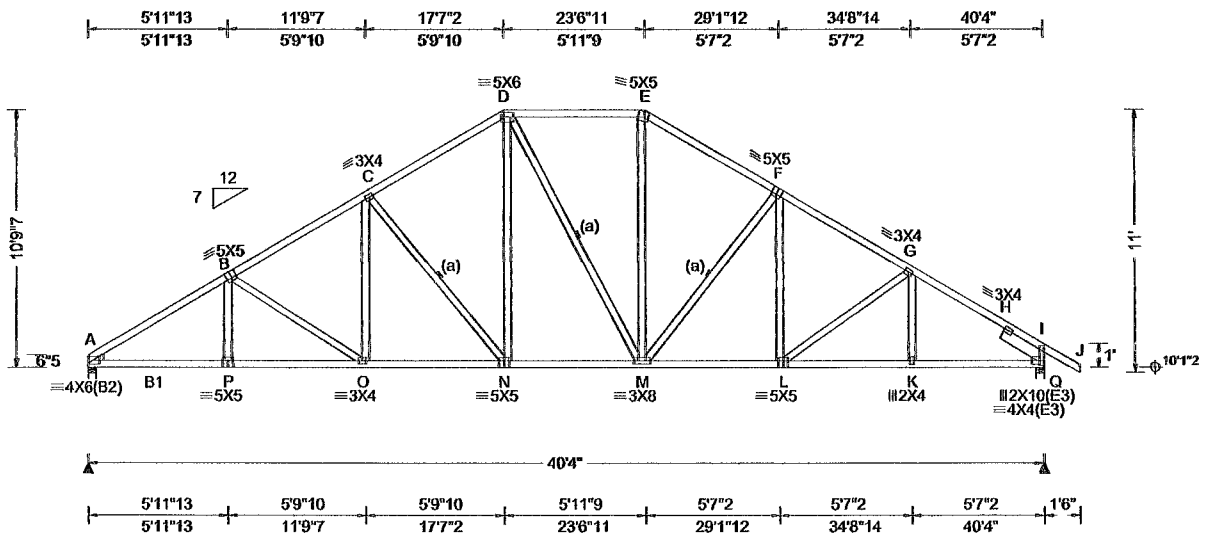


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SEQN: 802269 SPEC Ply 1 Job Number: 25-2306 Cust: R 215 JRef: 1Y7W2150003 T30
 FROM: CDM Qty 1 MOSS Truss Label: G17 DwgNo: 062.25.0539.31520
 KD / DF 03/03/2025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL. 20.00 TCDL. 10.00 BCLL. 0.00 BCDL. 10.00 Des Ld: 40 00 NCBCLL. 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.42 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 4.03 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.155 N 999 240 VERT(CL): 0.319 N 999 180 HORZ(LL): 0.107 H - - HORZ(TL) 0.221 H - - Creep Factor: 2.0 Max TC CSI: 0.882 Max BC CSI: 0.791 Max Web CSI: 0.320 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 1679 /- /- /1024 /- /309 Q 1777 /- /- /1119 /- /- Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) Q Brg Wid = 4.0 Min Req = 2.1 (Truss) Bearings A & Q are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 347 -2765 E - F 289 -1911 B - C 331 -2397 F - G 290 -2274 C - D 305 -1942 G - H 282 -2475 D - E 291 -1578 H - I 301 -2552

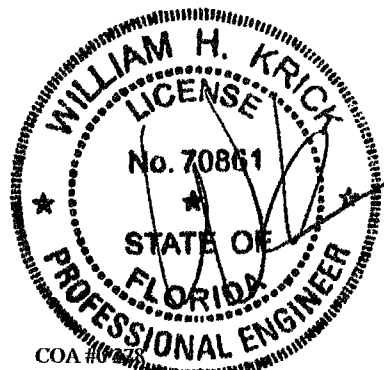
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2; B1 2x4 SP M-31,
 Webs: 2x4 SP #3;
 Rt Slider: 2x6 SP #2; block length = 2.093'
 Lt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design
 Wind loading based on both gable and hip roof types.

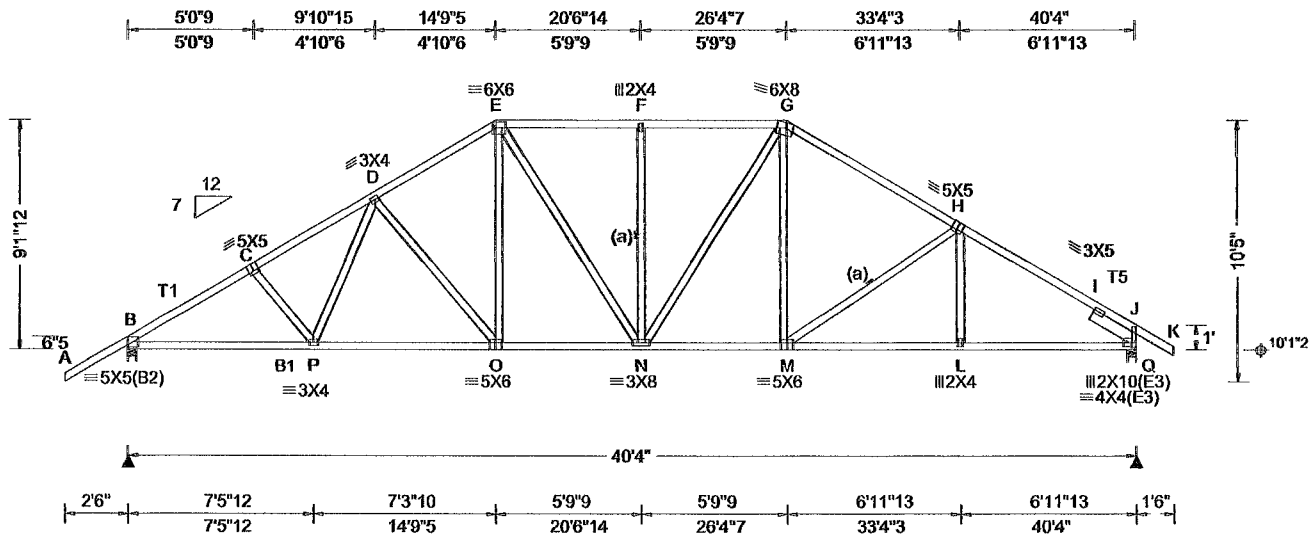
Additional Notes
 The overall height of this truss excluding overhang is 10'-9-7/8"



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)																																																															
TCLL 20 00 TCDL 10 00 BCLL 0.00 BCDL 10.00 Des Ld 40 00 NCBCLL 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.60 ft TCDL 5.0 psf BCDL 5.0 psf MWFERS Parallel Dist: h to 2h C&C Dist a: 4.03 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.188 F 999 240 VERT(CL): 0.353 F 999 180 HORZ(LL): 0.127 I - - HORZ(TL): 0.239 I - - Creep Factor: 2.0 Max TC CSI: 0.783 Max BC CSI: 0.771 Max Web CSI: 0.560 VIEW Ver: 23.02.04.0123.14	<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rh</th> <th>Rw</th> <th>U</th> <th>RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>2044</td> <td>-</td> <td>-</td> <td>1142</td> <td>-</td> <td>1295</td> </tr> <tr> <td>Q</td> <td>1917</td> <td>-</td> <td>-</td> <td>1066</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFERS B Brg Wid = 4.0 Min Req = 1.7 (Truss) Q Brg Wid = 4.0 Min Req = 2.3 (Truss) Bearings B & Q are a rigid surface. Members not listed have forces less than 375#</p> <table border="1"> <thead> <tr> <th colspan="6">Maximum Top Chord Forces Per Ply (lbs)</th> </tr> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>152</td> <td>-3076</td> <td>F - G</td> <td>102</td> <td>-2186</td> </tr> <tr> <td>C - D</td> <td>160</td> <td>-2901</td> <td>G - H</td> <td>118</td> <td>-2386</td> </tr> <tr> <td>D - E</td> <td>136</td> <td>-2452</td> <td>H - I</td> <td>135</td> <td>-2743</td> </tr> <tr> <td>E - F</td> <td>102</td> <td>-2186</td> <td>I - J</td> <td>147</td> <td>-2823</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	R-	Rh	Rw	U	RL	B	2044	-	-	1142	-	1295	Q	1917	-	-	1066	-	-	Maximum Top Chord Forces Per Ply (lbs)						Chords	Tens.Comp.	Chords	Tens.	Comp.		B - C	152	-3076	F - G	102	-2186	C - D	160	-2901	G - H	118	-2386	D - E	136	-2452	H - I	135	-2743	E - F	102	-2186	I - J	147	-2823
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Lumber
Top chord: 2x4 SP #2; T1, T5 2x4 SP M-31,
Bot chord 2x4 SP #2; B1 2x4 SP M-31,
Webs: 2x4 SP #3,
RT Slider: 2x6 SP #2; block length = 2.093'

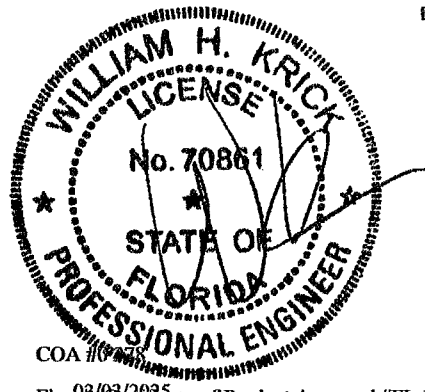
Bracing
(a) Continuous lateral restraint equally spaced on member

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFERS with additional C&C member design
Wind loading based on both gable and hip roof types.

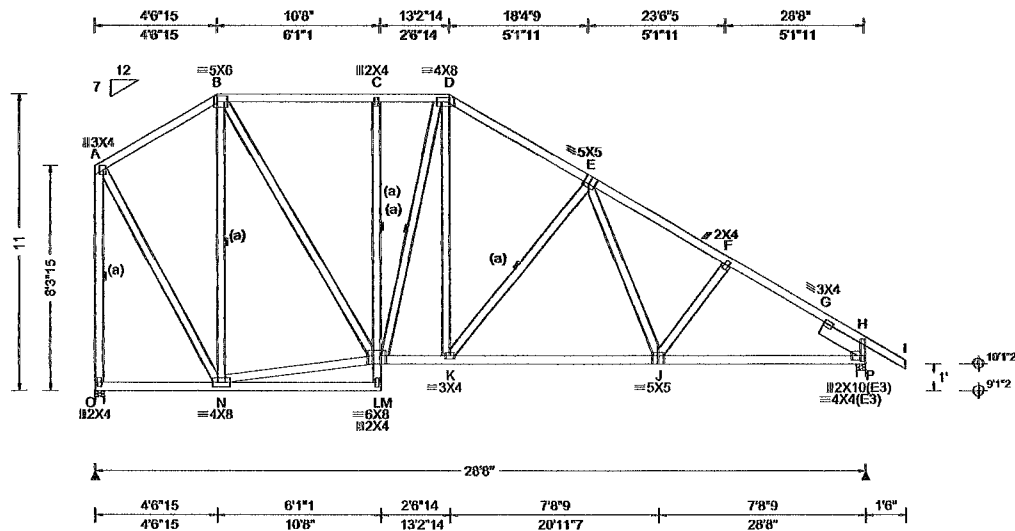
Additional Notes
The overall height of this truss excluding overhang is 9-1-12.



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02/03/2025

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.42 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.068 E 999 240 VERT(CL): 0.139 E 999 180 HORZ(LL): 0.048 G - - HORZ(TL): 0.100 G - - Creep Factor: 2.0 Max TC CSI: 0.561 Max BC CSI: 0.633 Max Web CSI: 0.436 VIEW Ver: 23.02.04.0123.14	Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> </tr> </thead> <tbody> <tr> <td>O</td> <td>1189</td> <td>-</td> <td>-</td> <td>1254</td> </tr> <tr> <td>P</td> <td>1296</td> <td>-</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS O Brg Wid = 4.0 Min Req = 1.5 (Truss) P Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings O & P are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>70</td> <td>-586</td> <td>E - F</td> <td>153</td> <td>-1549</td> </tr> <tr> <td>B - C</td> <td>95</td> <td>-840</td> <td>F - G</td> <td>138</td> <td>-1681</td> </tr> <tr> <td>C - D</td> <td>95</td> <td>-841</td> <td>G - H</td> <td>166</td> <td>-1759</td> </tr> <tr> <td>D - E</td> <td>111</td> <td>-1121</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	/Rh	/Rw	/U	O	1189	-	-	1254	P	1296	-	-	-	Chords	Tens.	Comp.	Chords	Tens.	Comp.	A - B	70	-586	E - F	153	-1549	B - C	95	-840	F - G	138	-1681	C - D	95	-841	G - H	166	-1759	D - E	111	-1121			
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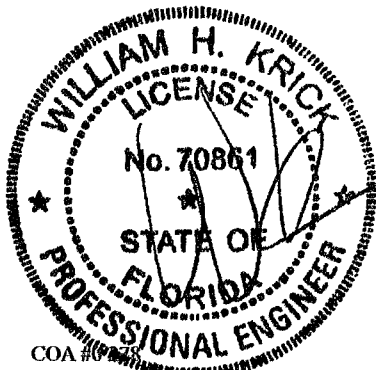
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Rt Slider: 2x6 SP #2; block length = 1.923'

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

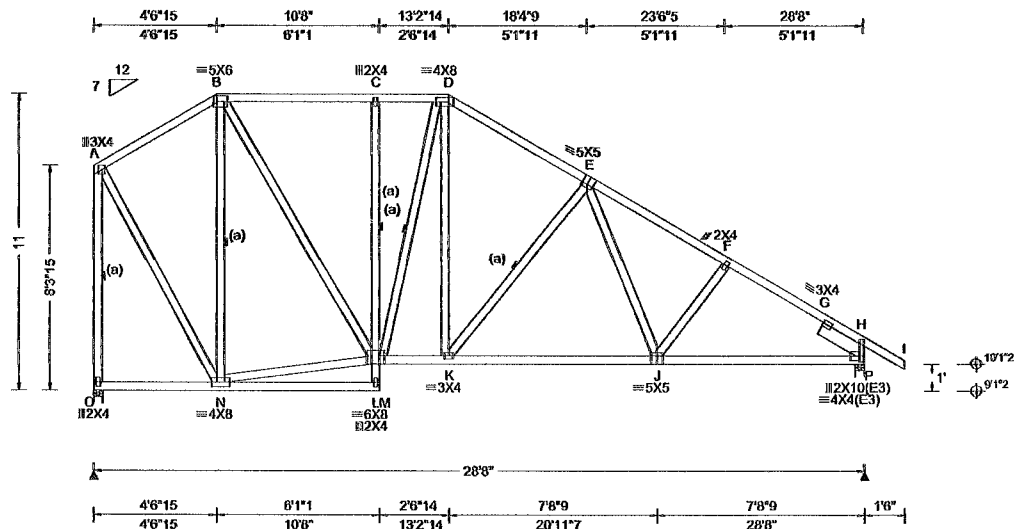
Additional Notes
 The overall height of this truss excluding overhang is 11-0-0.



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Loading Criteria (psf) TCLL. 20.00 TCDL. 10.00 BCLL. 0.00 BCDL. 10.00 Des Ld: 40.00 NCBCLL. 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0"	Wind Criteria Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP-C Kzt: NA Mean Height: 16.42 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpt: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT'20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.068 E 999 240 VERT(CL): 0.139 E 999 180 HORZ(LL): 0.048 G - - HORZ(TL): 0 100 G - - Creep Factor: 2.0 Max TC CSI 0.561 Max BC CSI 0.633 Max Web CSI: 0.436 VIEW Ver 23.02.04.0123.14	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL O 1189 / - / - / 1658 / - / 254 P 1296 / - / - / 1813 / - / - Wind reactions based on MWFRS O Brg Wid = 4.0 Min Req = 1.5 (Truss) P Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings O & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 70 -586 E - F 153 -1549 B - C 95 -840 F - G 138 -1681 C - D 95 -841 G - H 166 -1759 D - E 111 -1121
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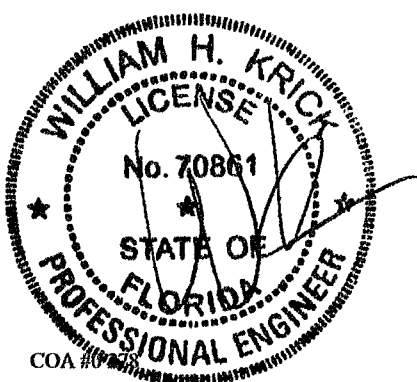
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3,
Rt Slider: 2x6 SP #2; block length = 1.923'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
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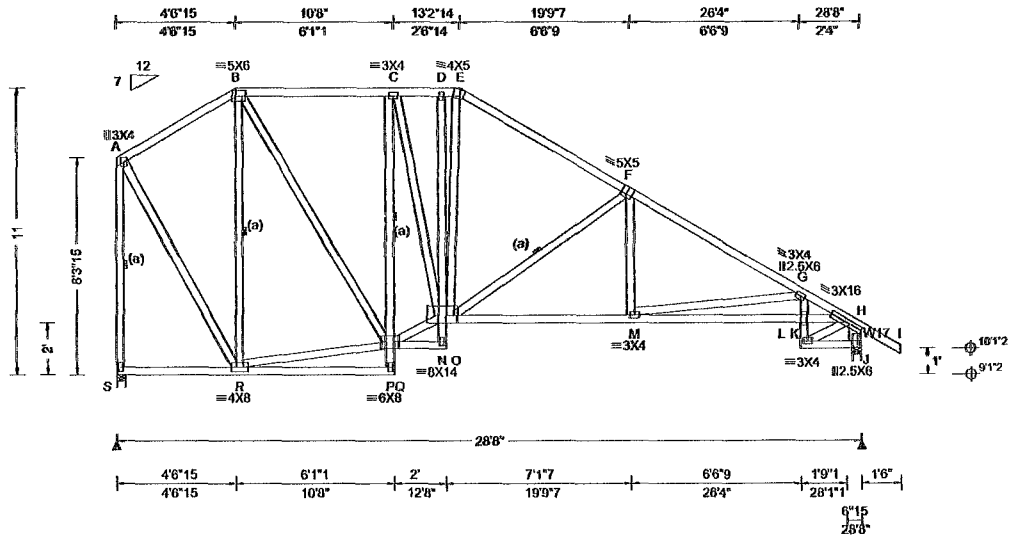
Additional Notes
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)			
				Gravity		Non-Gravity	
TCLL 20.00	Wind Std ASCE 7-22	Pg. NA Ct: NA CAT. NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh /Rw /U /RL
TCDL 10.00	Speed 130 mph	Pf NA Ce. NA	VERT(LL): 0.095 M 999 240	S	1189	-	- /651 - /252
BCLL 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.195 M 999 180	J	1296	-	- /809 - -
BCDL 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.078 L - -	Wind reactions based on MWFRS			
Des Ld: 40.00	EXP C Kzi: NA	Building Code:	HORZ(TL): 0.161 L - -	S Brg Wid = 4.0 Min Req = 1.5 (Truss)			
NCBCLL 10.00	Mean Height: 15.96 ft	FBC 8th Ed. 2023 Res.	Creep Factor: 2.0	J Brg Wid = 4.0 Min Req = 1.5 (Truss)			
Soffit: 2.00	TCDL 5.0 psf	TPI Std: 2014	Max TC CSI 0.561	Bearings S & J are a rigid surface.			
Load Duration: 1.25	BCDL 5.0 psf	Rep Fac: Yes	Max BC CSI 0.832	Members not listed have forces less than 375#			
Spacing 24.0"	MWFRS Parallel Dist: h to 2h	FT/RT.20(0)/10(0)	Max Web CSI 0.767	Maximum Top Chord Forces Per Ply (lbs)			
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 23.02.04.0123.14	Chords	Tens.Comp.	Chords	Tens. Comp.
	Loc. from endwall: not in 9.00 ft	WAVE		A - B	71 -586	E - F	64 -1235
	GCpi: 0.18			B - C	97 -841	F - G	99 -1916
	Wind Duration: 1.60			C - D	101 -977	G - H	195 -2880
				D - E	102 -983		

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3, W17 2x6 SP #2;

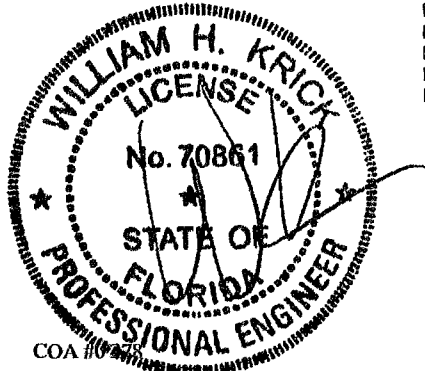
Bracing
 (a) Continuous lateral restraint equally spaced on member

Plating Notes
 All plates are 2X4 except as noted.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 11-0-0.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (if no rigid diaphragm exists at that point).



Florida Certificate of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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