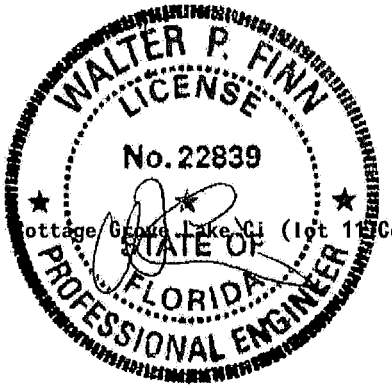
Alpine, an ITW Company

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1VEC487-Z0125130858



Truss Fabricator	Anderson Truss Company
Job Identification	15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci (lot 11 Cott
Truss Count	33
Model Code	Florida Building Code 2014 or 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 14.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

02/25/2015

Notes

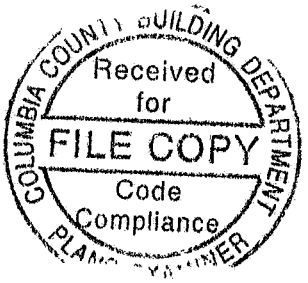
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

2400 Lake Orange Dr, Suite 150
Orlando FL, 32837

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	92673--A	21' Common	15056001	02/25/15
2	92674-A1	21' Common Gi	15056021	02/25/15
3	92675--AGE	21' Gable	15056010	02/25/15
4	92676--B	6'10" Common	15056022	02/25/15
5	92677--B1	18'2" Common	15056026	02/25/15
6	92678--BGE	6'10" Gable	15056023	02/25/15
7	92679-BGE1	18'2" Gable	15056025	02/25/15
8	92680--C	29' Mono Hip	15056069	02/25/15
9	92681--C1	29' Mono Hip	15056068	02/25/15
10	92682-C2	28'4" Special	15056063	02/25/15
11	92683-C3	28'4" Special	15056064	02/25/15
12	92684-C4	28'4" Special	15056065	02/25/15
13	92685--CJ1	1' Jack	15056004	02/25/15
14	92686--CJ1A	1' Jack	15056009	02/25/15
15	92687--CJ3	3' Jack	15056003	02/25/15
16	92688--CJ3A	2'4" Jack	15056008	02/25/15
17	92689--CJ5	5' Jack	15056002	02/25/15
18	92690--CJ5A	4'4" Jack	15056012	02/25/15
19	92691--CJ5B	5' Jack	15056016	02/25/15
20	92692--EJ7	7' End Jack	15056005	02/25/15
21	92693-H11A	28'4" Stepd	15056013	02/25/15
22	92694-H11B	29' Mono Hi	15056018	02/25/15
23	92695-H13A	28'4" Stepd	15056066	02/25/15
24	92696-H13B	29' Mono Hi	15056067	02/25/15
25	92697-H7	28' Stepdown	15056006	02/25/15
26	92698-H7A	28'4" Stepdo	15056011	02/25/15
27	92699-H7B	29' Mono Hip	15056019	02/25/15
28	92700-H8	28' Stepdown	15056020	02/25/15
29	92701-H9A	28'4" Stepdo	15056015	02/25/15
30	92702-H9B	29' Mono Hip	15056024	02/25/15
31	92703-HJ7	9'10"13 Hip	15056007	02/25/15
32	92704-HJ7A	9'10"1 Hip	15056014	02/25/15
33	92705-HJ7B	9'10"13 Hip	15056017	02/25/15



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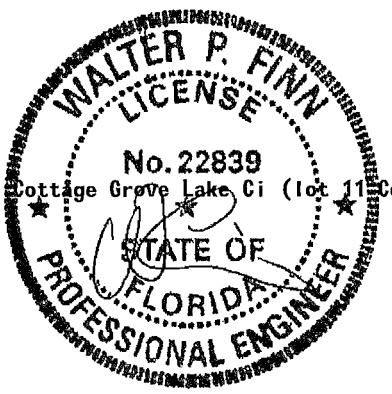
 Document ID 1VEC487-Z0125130858



Truss Fabricator	Anderson Truss Company
Job Identification	15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci (lot 11 Cott
Truss Count	1
Model Code	Florida Building Code 2014 or 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 14.03.
Structural Engineer of Record	
Address	
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration Floor - N/A Wind - 120 MPH ASCE 7-10 -Closed

02/25/2015

-Truss Design Engineer-
Walter P Finn

1950 Marley Drive
Haines City, FL 33844

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Revised Trusses

#	Ref	Description	Drawing#	Date
1	92698-H7A	28'4" Stepdo	15056011	02/25/15

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Cı - A 21 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

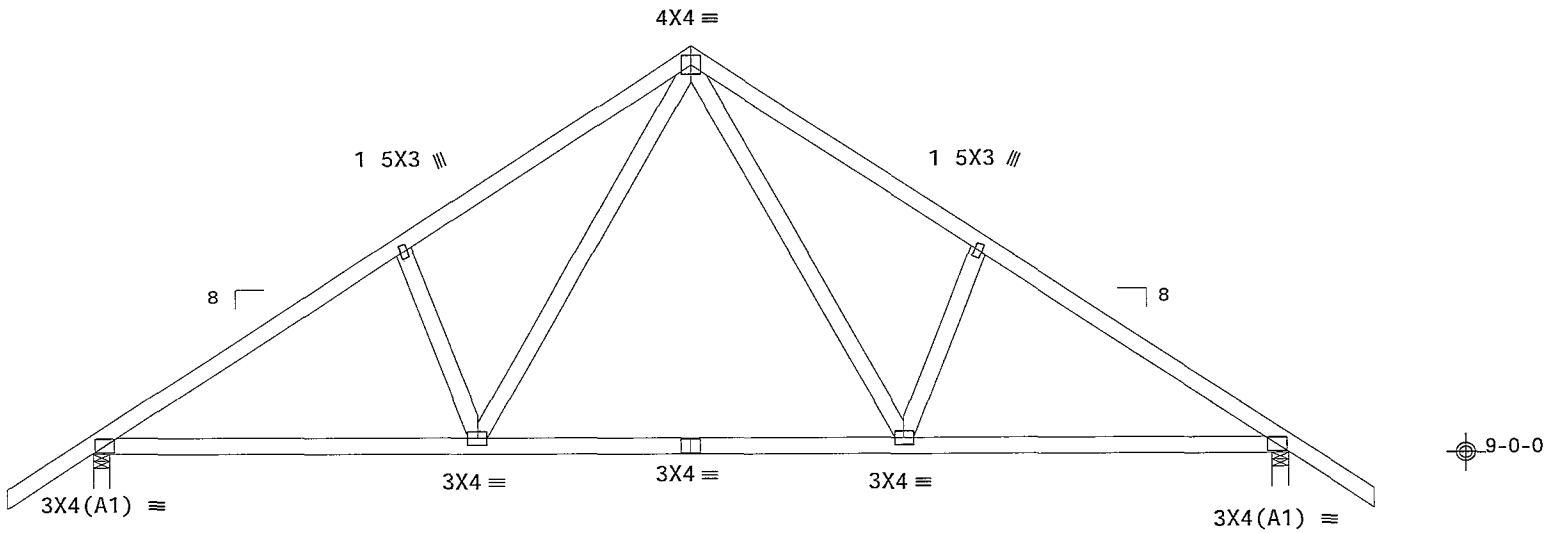
Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42 -high x 24 -wide clearance



R=964 U=32 W=3 5
RL=143/-143

R=964 U=32 W=3 5'

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14.03 01 QTY 11 FL/-/5/-/-/R/- Scale =.3125"/Ft.

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Orlando, FL 32837
FL COA #0 278

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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Trusses requ re extreme care n fabr cating handl ng sh pp ng nstal ng and brac ng Refer to and follow the latest ed t on of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for safety pract ces prior to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherw top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ce l ng Locations shown for permanent lateral restraint of webs shall have brac ng nstal ed per BCSI sections B3 B7 or B10 as appl cable Apply plates to each face of truss and pos t on as shown above and the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A Z for standard p ate pos t ons

Alp ne a d v s on of ITW Bu ld ng Components Group Inc shall not be respons ble for any dev at on from draw ng any failure to build the truss in conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on & brac ng of trusses

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For more nformation see th s job s general notes page and these web s tes
ALPINE www alp neltw com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92673
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056001
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427725
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - A1 21 Common Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 T2 2x4 SP M-30
Bot chord 2x6 SP #2 B2 2x6 SP #1 Dense
Webs 2x4 SP #3 W3 2x4 SP #2

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10 CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

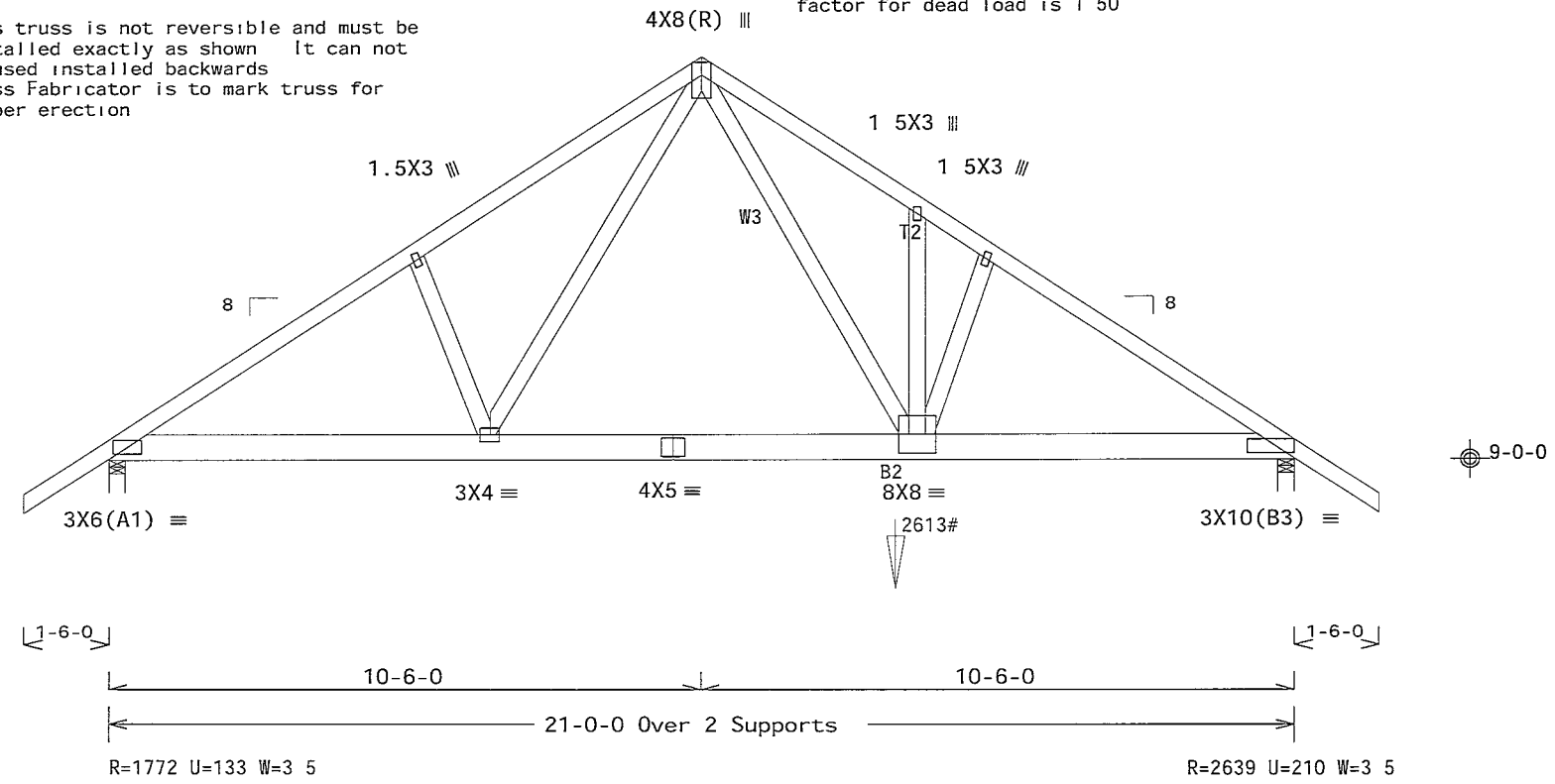
Bottom chord checked for 10 00 psf non-concurrent live load

WARNING This truss is not reversible and must be installed exactly as shown It can not be used installed backwards
Truss Fabricator is to mark truss for proper erection

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 10 50
TC- From 57 plf at 10 50 to 57 plf at 22 50
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 10 50
BC- From 20 plf at 10 50 to 20 plf at 21 00
BC- From 5 plf at 21 00 to 5 plf at 22 50
BC- 2613 14 lb Conc Load at 13 94

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

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02/25/2015

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TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056021
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427846
DUR FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - AGE 21 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

See DWGS A12015ENC101014, GBLLETIN1014, & GABRST101014 for gable wind bracing requirements

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

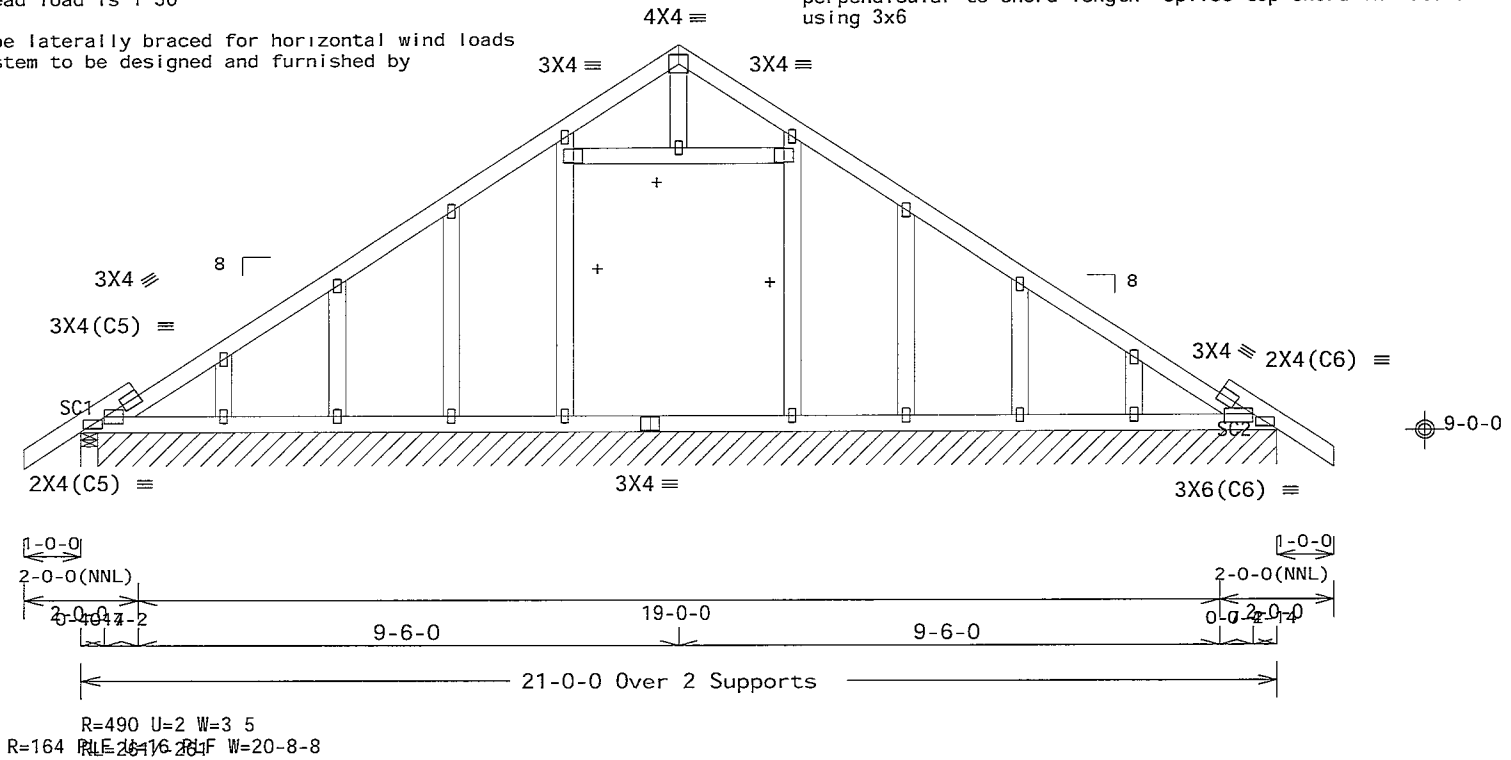
+ Member to be laterally braced for horizontal wind loads
Bracing system to be designed and furnished by others

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss designed to support 2-0-0 top chord outlookers and 10 00 PSF cladding load one face, and 24 0' span on opposite face Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



Note. All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03.01 QTY 1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

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FL COA #0278

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Trusses requ re extreme care n fabr cat ng handl ng shipping nstalling and brac ng Refer to and follow the latest edit on of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rig d ceiling Locations shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sections B3 B7 or B10 as appl cable Apply plates to each face of truss and pos t on as shown above and the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate positions

Alp ne a d v s on of ITW Building Components Group Inc shall not be respons ble for any dev at on from this draw ng any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallat on & brac ng of trusses

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02/25/2015

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TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056010
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427760
DUR FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - B 6'10 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

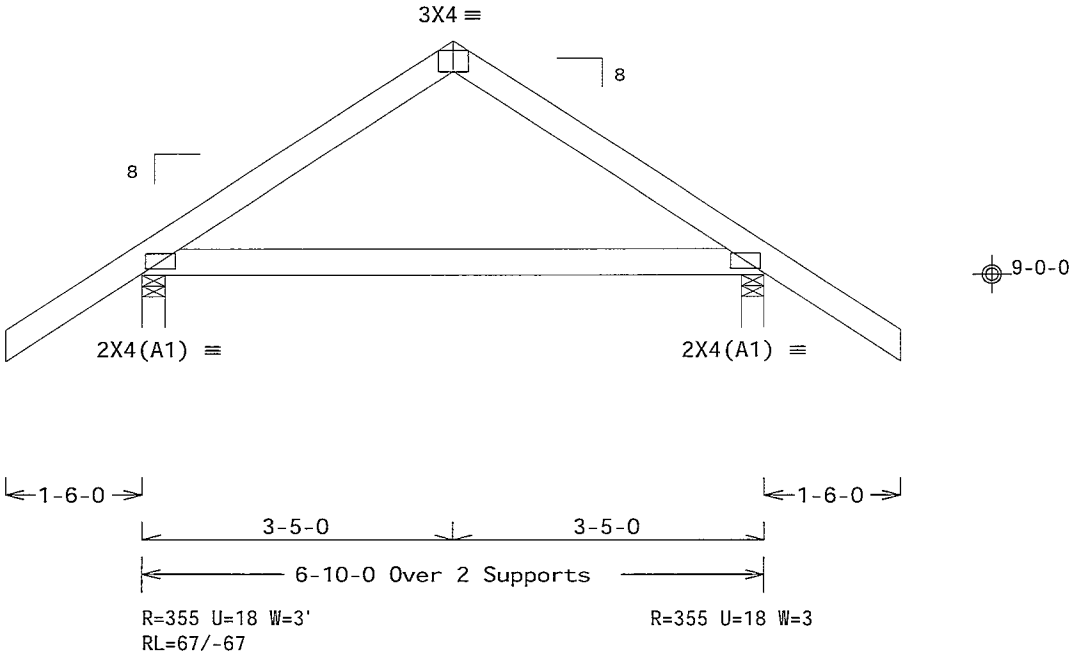
Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14.03 01 [illegible] 03 QTY·1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Alp ne a d v s on of ITW Bu lding Components Group Inc shall not be respons ble for any dev ation from this draw ng any fa lure to bu ld the truss in conformance with ANSI/TPI 1 or for handling sh pp ng nstallat on & brac ng of trusses

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02/25/2015

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TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056022
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427671
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - B1 18 2 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

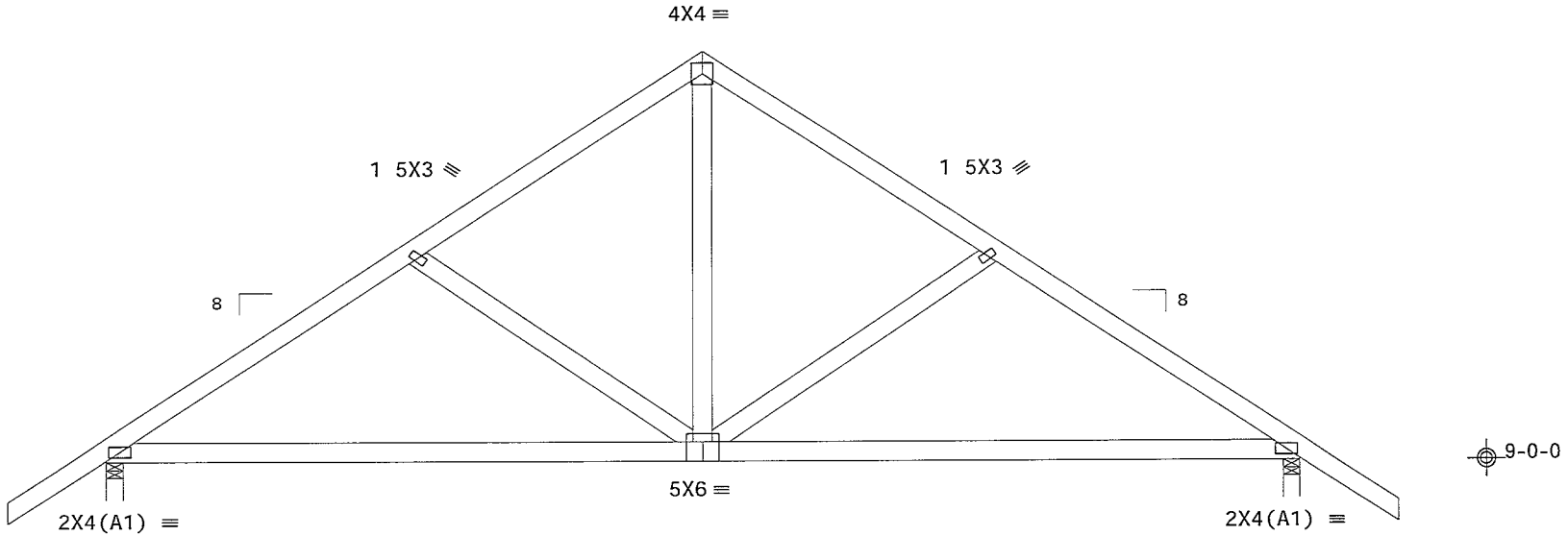
Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible]03 QTY.1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE AN ITW COMPANY

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Orlando, FL 32837
FL COA #0 278

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Trusses require extreme care n fabricat ng handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Bu lding Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rig d ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as appl cable Apply plates to each face of truss and posit on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92677
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10.0 PSF	DRW	HCUSR9114 15056026
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427669
DUR.FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - BGE 6'10" Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

See DWGS A12015ENC101014, GBLLETIN1014, & GABRST101014 for gable wind bracing requirements

In lieu of structural panels use purlins to brace TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

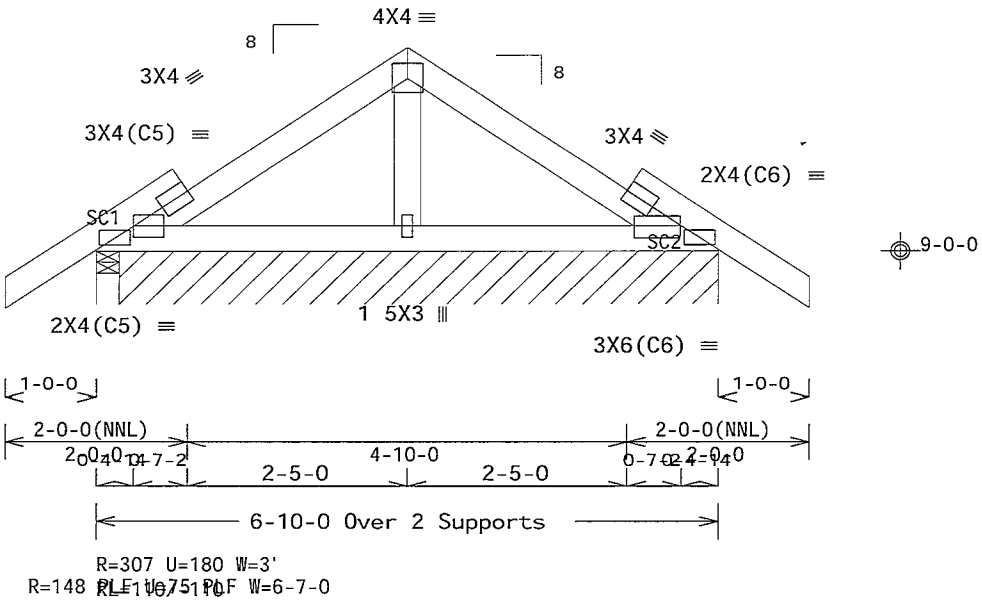
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss designed to support 2-0-0 top chord outlookers and 10 00 PSF cladding load one face, and 24 0' span on opposite face Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03.01 [illegible].23 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE AN ITW COMPANY

2400 Lake Orange Dr Suite 150
Orlando, FL 32837
FL COA #0 278

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Trusses requ re extreme care n fabricating handling sh pp ng install ng and brac ng Refer to and follow the latest ed tion of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r gid ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng nstalled per BCSI sections B3 B7 or B10 as appl cable Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A Z for standard plate pos t ons

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/25/2015

TC LL	20 0 PSF	REF	R9114- 92678
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056023
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427739
DUR FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - BGE1 18 2 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP 2850f-2 3E Stack Chord SC2 2x4 SP 2850f-2 3E

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

See DWGS A12015ENC101014 GBLLETIN1014 & GABRST101014 for gable wind bracing requirements

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

+ Member to be laterally braced for horizontal wind loads
Bracing system to be designed and furnished by others

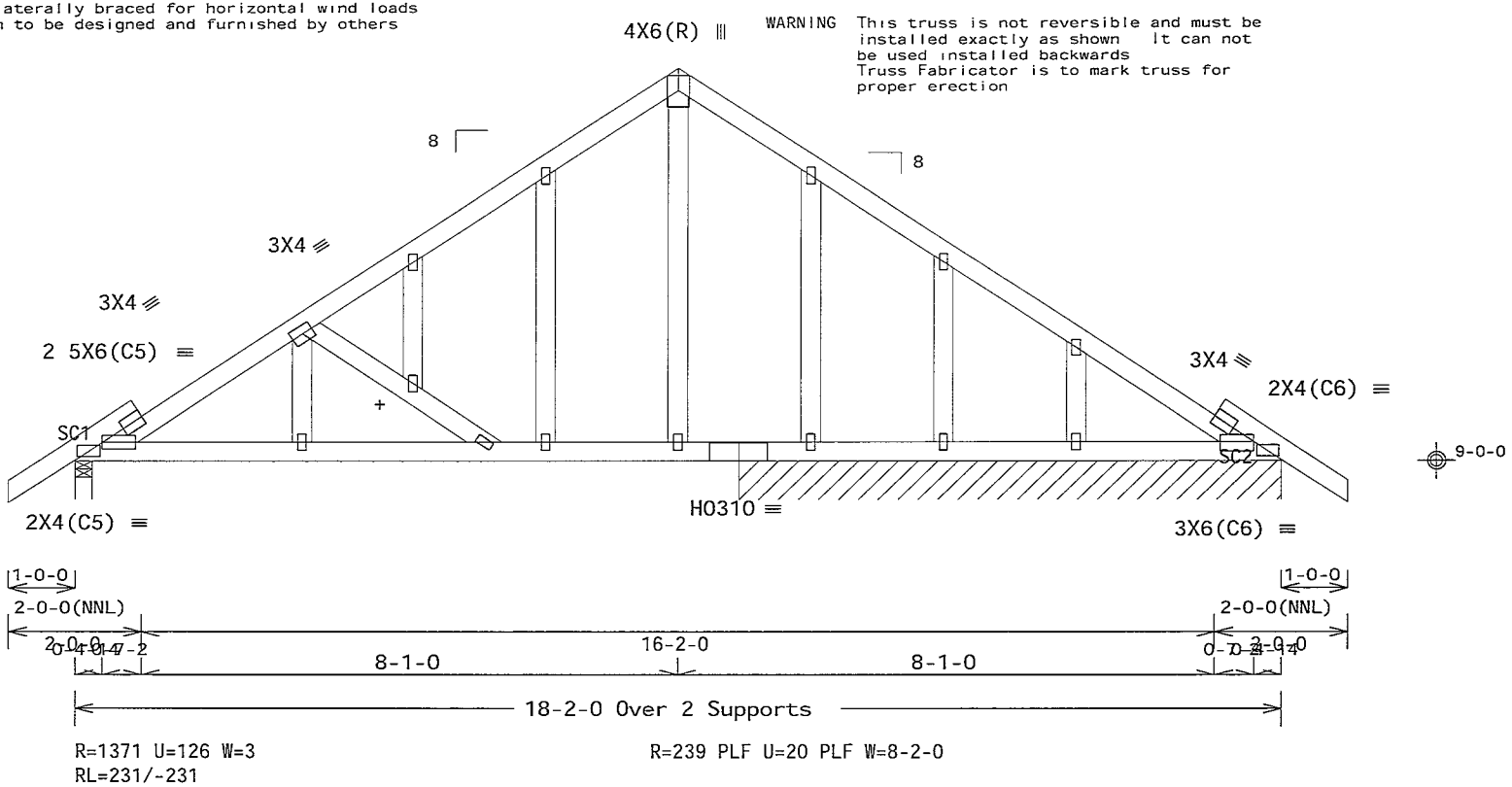
120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg Located anywhere in roof RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss designed to support 2-0-0 top chord outlookers and 10 00 PSF cladding load one face and 24 0 span on opposite face Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface plate length perpendicular to chord length Splice top chord in notchable area using 3x6

WARNING This truss is not reversible and must be installed exactly as shown It can not be used installed backwards
Truss Fabricator is to mark truss for proper erection



R=1371 U=126 W=3
RL=231/-231

R=239 PLF U=20 PLF W=8-2-0

Note All Plates Are 1 5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14 03 01 0000 23 QTY.1 FL/-/5/-/-/R/- Scale =.375"/Ft.

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FL COA #0278

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92679
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056025
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427763
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - C 29 Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B4 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (1)2x6 SP M-26 supporting member
(14) 0 148 x3 nails into supporting member
(4) 0 148 x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

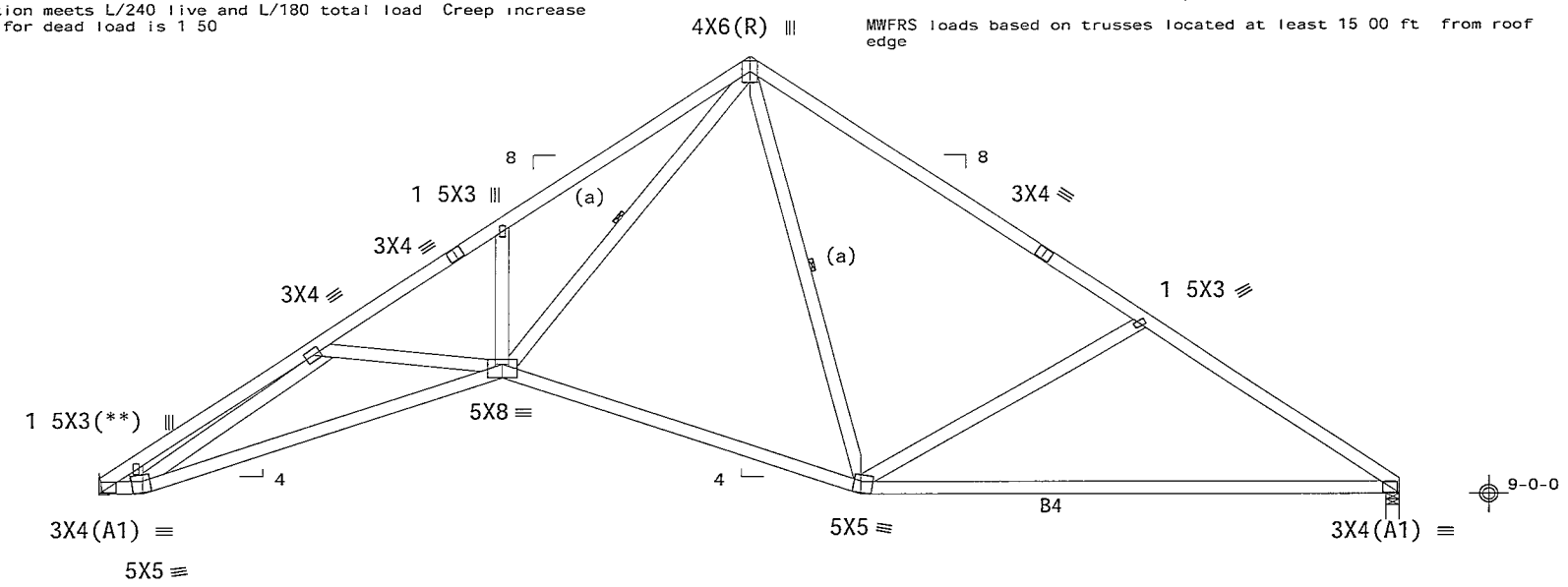
Wind loads and reactions based on MWFRS with additional C&C member design

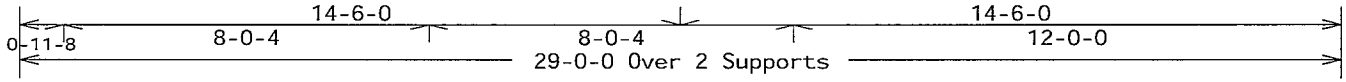
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge





R=1125 U=0
RL=154/-154
H=H1

R=1120 U=0 W=3 5"

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0000 23 QTY.3 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE AN ITW COMPANY

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Orlando, FL 32837
FL COA #0278

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Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A Z for standard plate positions

Alpine advises on of ITW Building Components Group Inc shall not be responsible for any deviation from this drawing any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92680
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056069
BC LL	0 0 PSF	HC-ENG	MAC/WPF
TOT LD	37 0 PSF	SEQN-	399841
DUR FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - C1 29 Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B4 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (1)2x6 SP M-26 supporting member
(14) 0 148 x3 nails into supporting member
(4) 0 148 x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

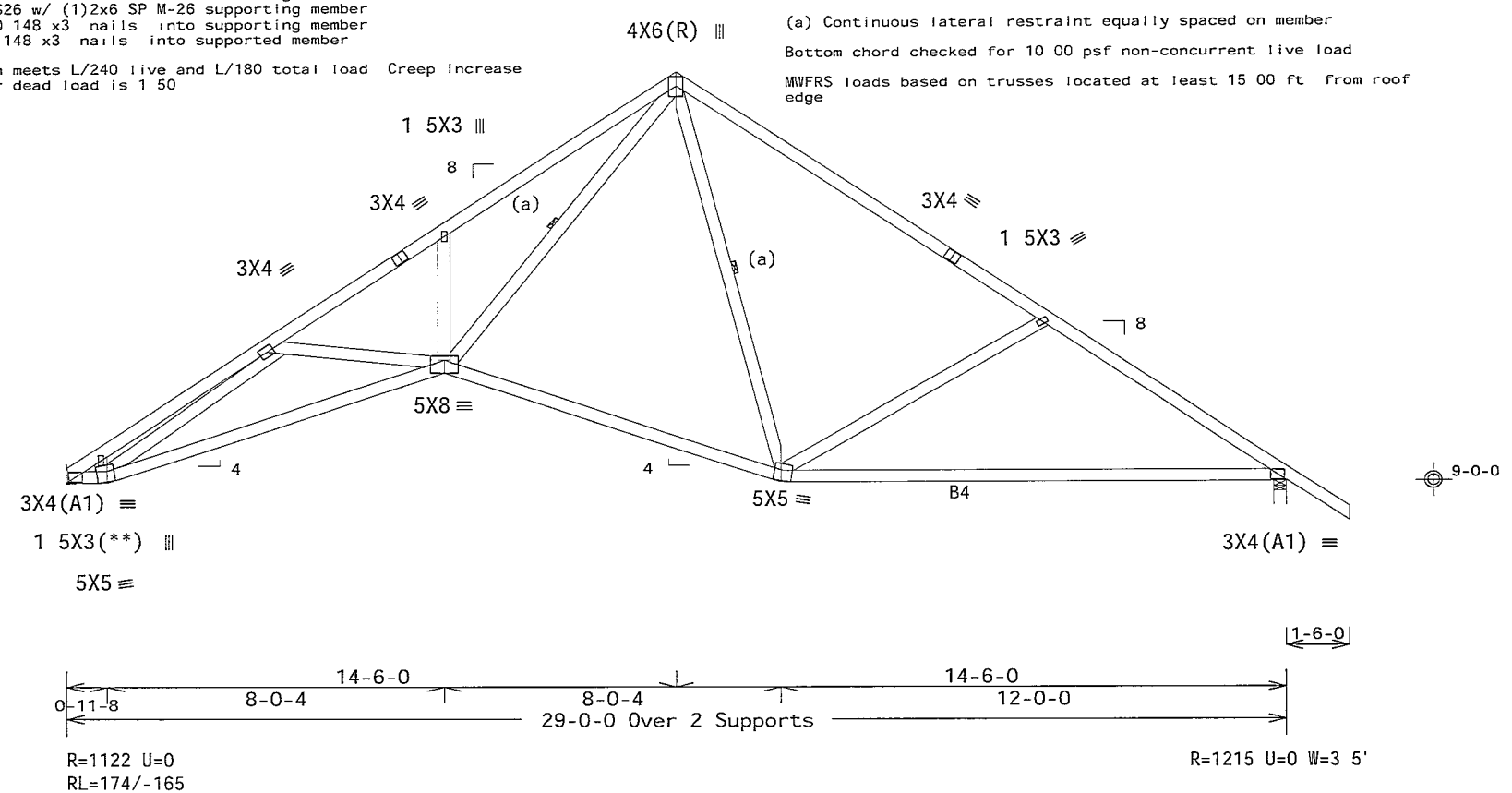
Wind loads and reactions based on MWFRS with additional C&C member design

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 2 FL/-/5/-/-/R/- Scale = 25"/Ft

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02/25/2015

TC LL	20 0 PSF	REF R9114- 92681
TC DL	7 0 PSF	DATE 02/25/15
BC DL	10 0 PSF	DRW HCUSR9114 15056068
BC LL	0 0 PSF	HC-ENG MAC/WPF
TOT LD	37 0 PSF	SEQN- 399838
DUR.FAC.	1.25	FROM JMW
SPACING	24 0"	JREF- 1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - C2 28 4 Special)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B3 2x4 SP 2850f-2 3E
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

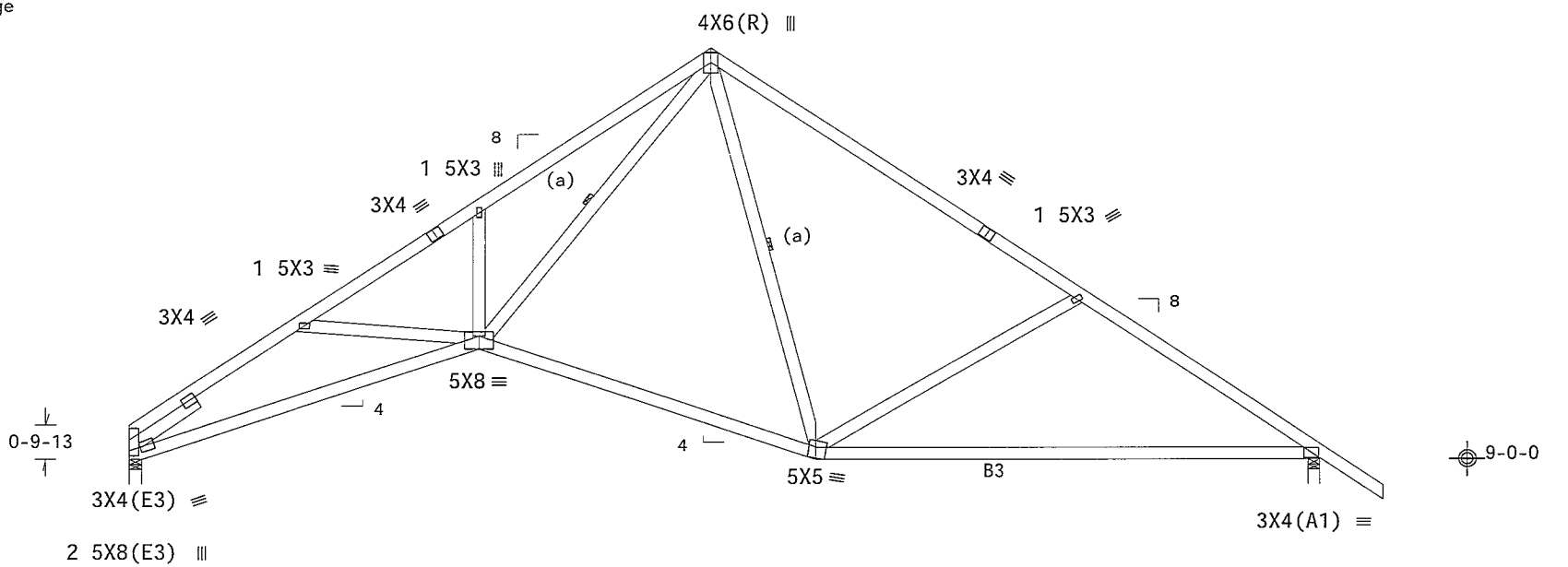
MWFRS loads based on trusses located at least 15 00 ft from roof edge

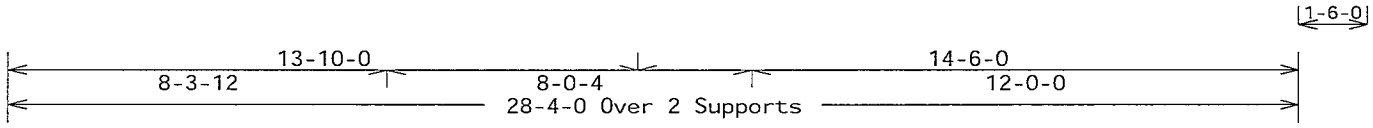
120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1092 U=0 W=3 5
RL=169/-163

R=1195 U=0 W=3 5'

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 14 03 01 0122 23 | QTY·4 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92682
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056063
BC LL	0 0 PSF	HC-ENG	MAC/WPF
TOT.LD	37.0 PSF	SEQN-	399835
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - C3 28 4 Special)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

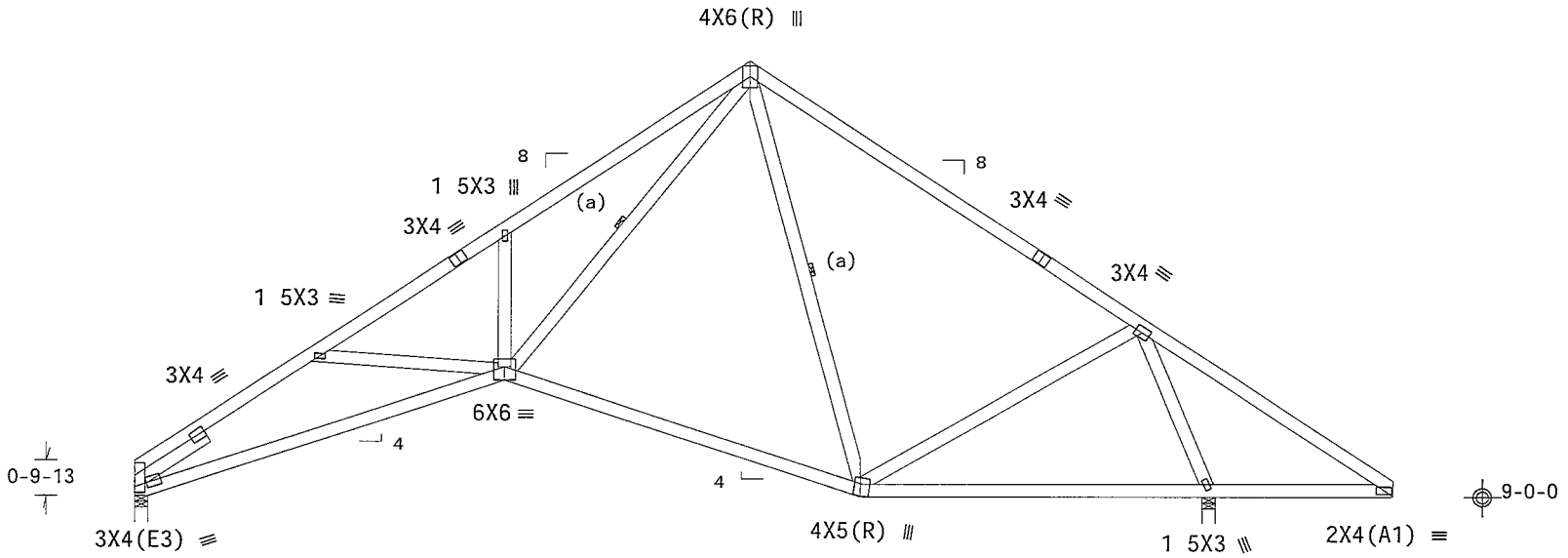
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right cantilever is exposed to wind

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge



R=925 U=0 W=3 5
RL=149/-152

R=1284 U=0 W=3 5

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY·3 FL/-/5/-/-/R/- Scale =.25"/Ft.

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92683
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056064
BC LL	0 0 PSF	HC-ENG	MAC/WPF
TOT.LD	37 0 PSF	SEQN-	399829
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - C4 28 4 Special)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP M-30 T3, T4 2x4 SP #1
Bot chord 2x4 SP #1 B3 2x4 SP 2850f-2 3E
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

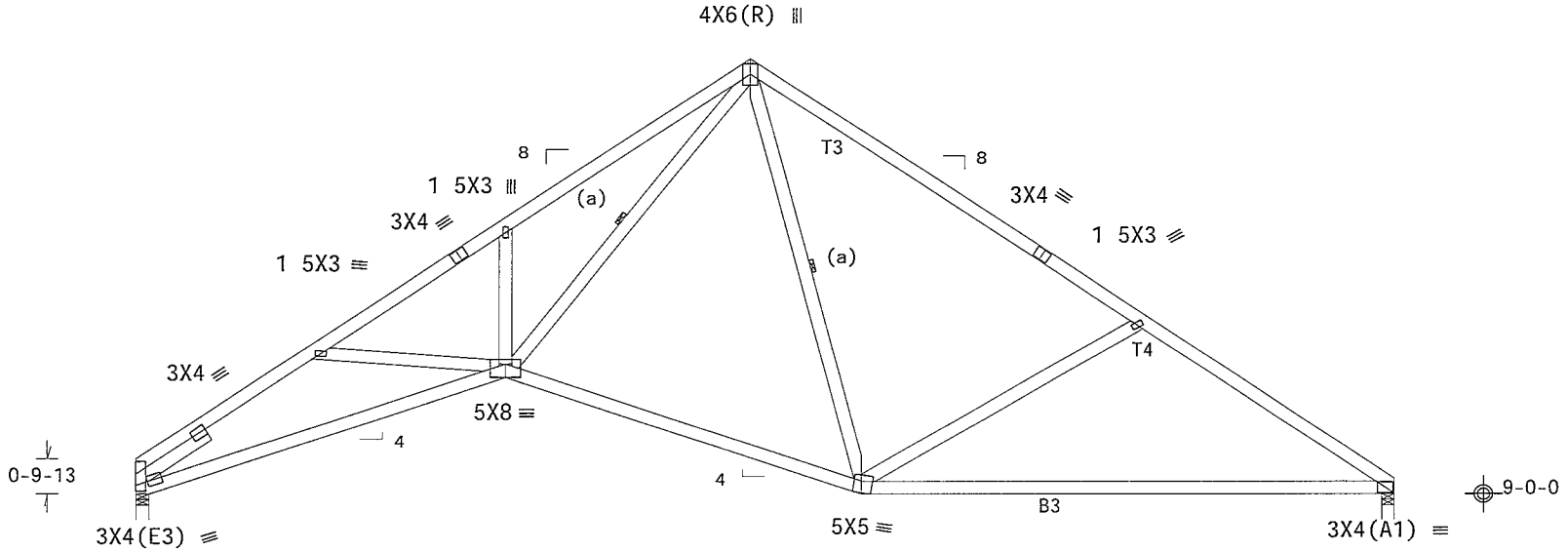
MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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For more nformation see this job s general notes page and these web s tes
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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92684
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10.0 PSF	DRW	HCUSR9114 15056065
BC LL	0.0 PSF	HC-ENG	MAC/WPF
TOT.LD.	37 0 PSF	SEQN-	399826
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - CJ1 1 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

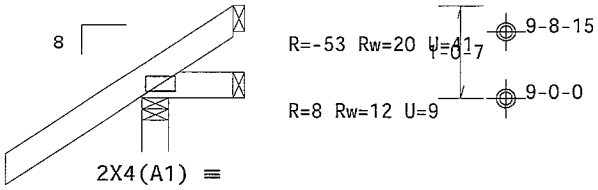
Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

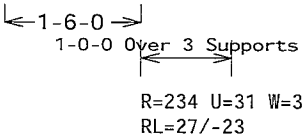
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible] QTY·9 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Trusses require extreme care n fabricat ng handling sh pping install ng and brac ng Refer to and fol the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces p r to performing these functions Installers shall provide temporary brac ng per BCSI Unless noted otherw top chord shall have properly attached structural sheathing and bottom chord shall have a properly attache r g d ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng nstalled per BCS sect ons B3 B7 or B10 as applicable Apply plates to each face of truss and pos t on as shown above and the Jo nt Deta ls unless noted otherw se Refer to drawings 160A Z for standard plate pos t ons

Alp ne a d v s on of ITW Bu lding Components Group Inc shall not be respons ble for any dev at on from th draw ng any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on & brac ng of trussos

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92685
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056004
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT.LD.	37 0 PSF	SEQN-	427655
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Cı - CJ1A 1 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

Left cantilever is exposed to wind

Bottom chord checked for 10 00 psf non-concurrent live load

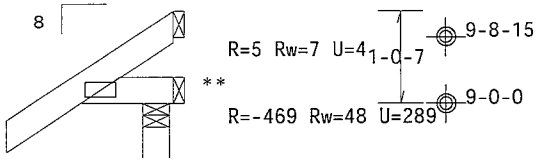
Negative reaction(s) of -469# MAX (See below) from a non-wind load case requires uplift connection

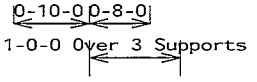
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

** Hanger required OR apply a 2x4 SP_#3_13B or better vertical scab, cut-to-fit, attached with (3) 0 128'x3 0 10d nails in each member then attach with (3) 0 162 x3 5 16d Common toe-nails in lieu of hanger





R=593 U=58 W=3 5

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 QTY 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92686
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056009
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427668
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Cı - CJ3 3 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

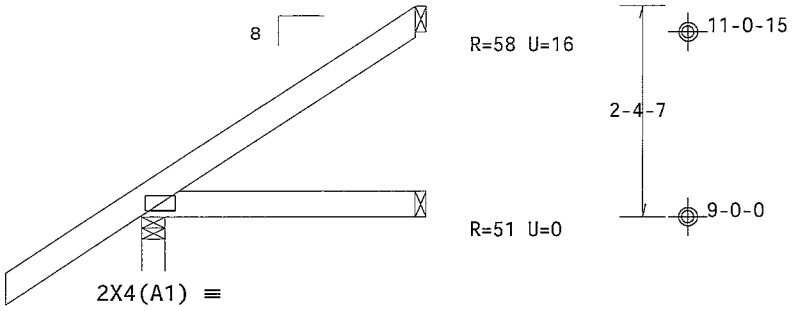
Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

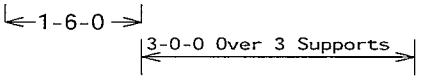
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=241 U=8 W=3"
RL=49/-30

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY:9 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Alpine a div sion of ITW Bu ld ng Components Group Inc sha l not be respons ble for any dev at on from th draw ng any failure to build the truss in conformance w th ANSI/TPI 1 or for handling sh pp ng installat on & brac ng of trusses

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TC LL	20 0 PSF	REF	R9114- 92687
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056003
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427663
DUR FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake C1 - CJ3A 2 4 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

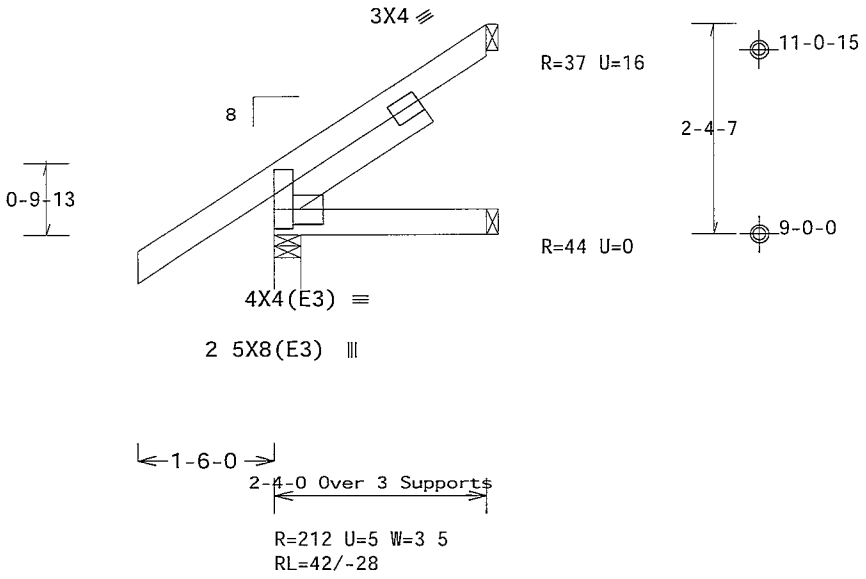
Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices to be followed in performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10 as applicable. Apply plates to each face of truss and position as shown above, unless noted otherwise on the Joint Details. Refer to drawings 160A Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc., shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92688
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056008
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427676
DUR FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - CJ5 5 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

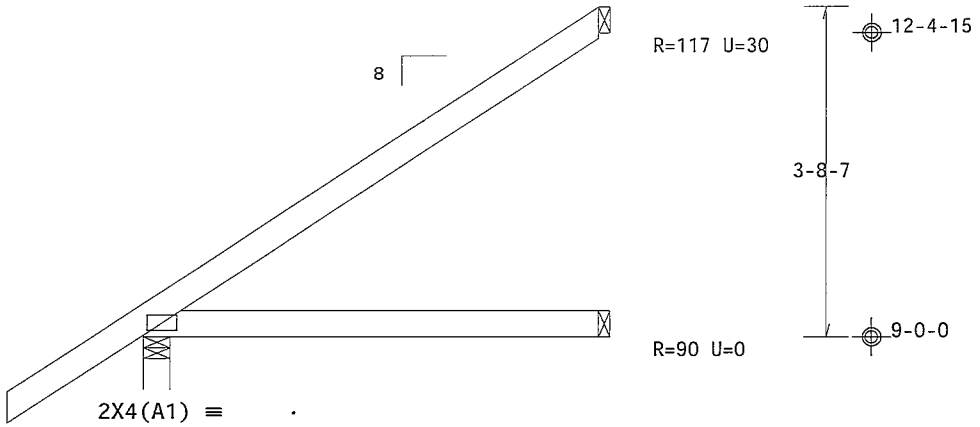
Lumber value set '13B uses design values approved 1/30/2013 by ALSC

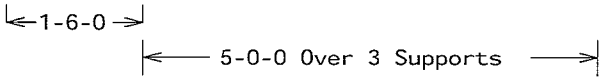
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=307 U=0 W=3 5
RL=71/-37

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 8 FL/-/5/-/-/R/- Scale =.5"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92689
TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056002
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427658
DUR FAC.	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - CJ5A 4 4 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

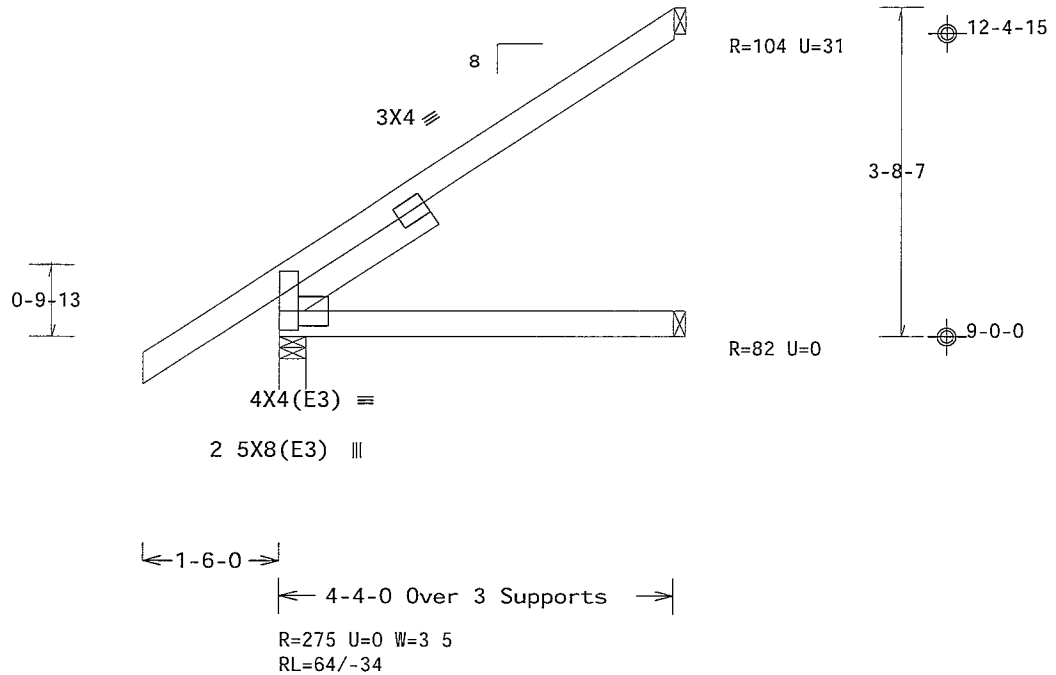
Lumber value set 13B' uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 1 FL/-/5/-/-/R/- Scale = 5"/Ft

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TC LL	20 0 PSF	REF	R9114- 92690
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056012
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD.	37 0 PSF	SEQN-	427664
DUR FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake C1 - CJ5B 5 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 #1

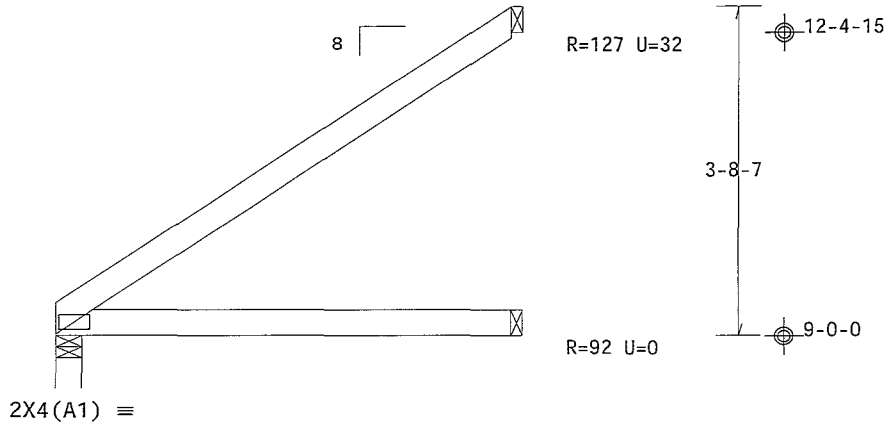
Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible] QTY 1 FL/-/5/-/-/R/- Scale = 5"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92691
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056016
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427674
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - EJ7 7 End Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

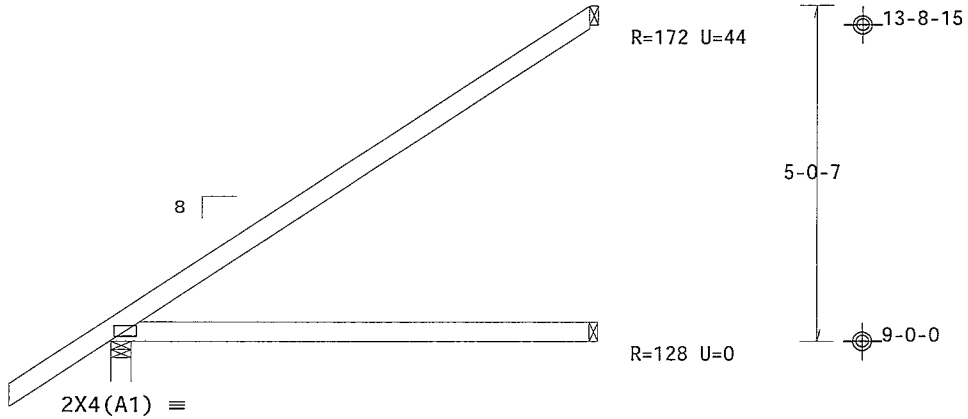
Bottom chord checked for 10 00 psf non-concurrent live load

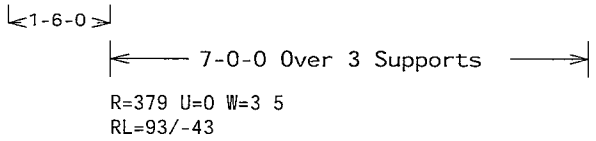
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=379 U=0 W=3 5
RL=93/-43

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible] QTY 28 FL/-/5/-/-/R/- Scale =.375"/Ft.

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TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056005
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD.	37 0 PSF	SEQN-	427652
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H11A 28 4 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B2 2x4 SP 2850f-2 3E
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000'

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

MWFRS loads based on trusses located at least 7 50 ft from roof edge

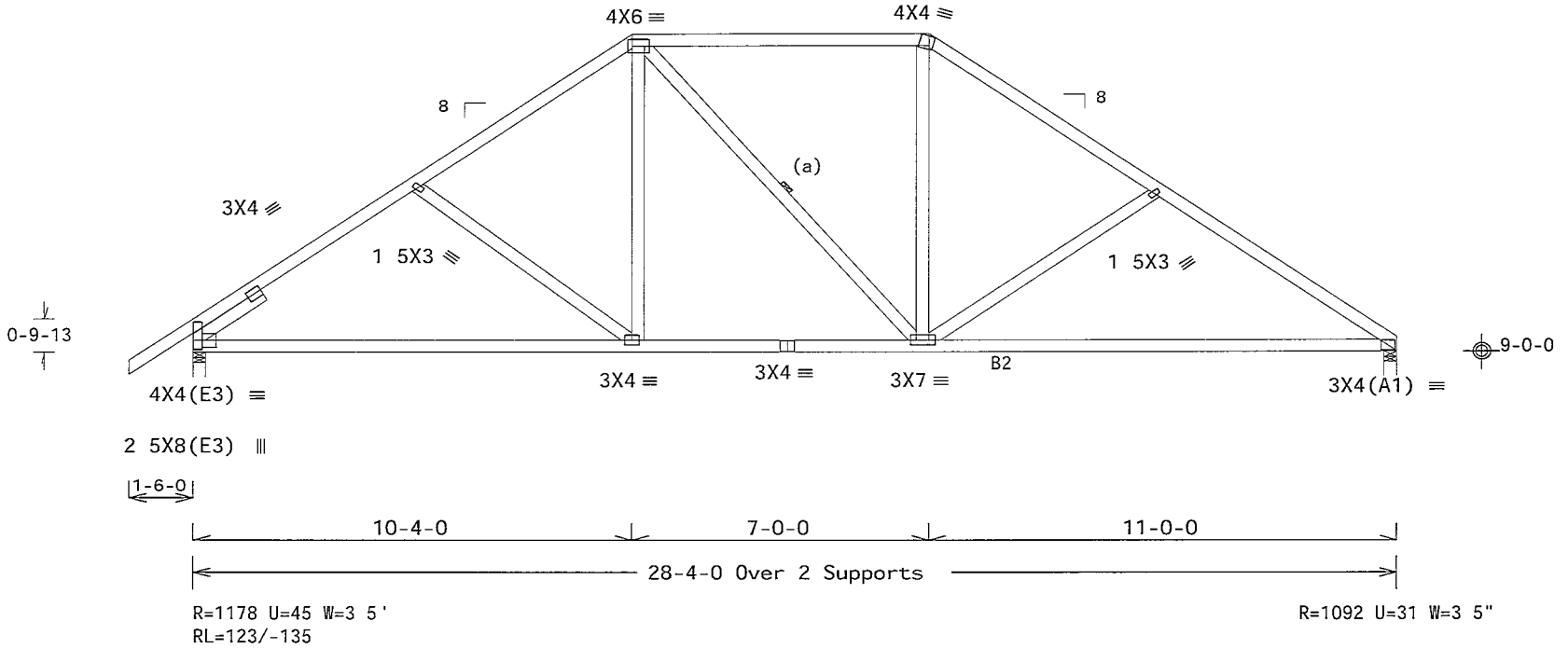
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14.03 01 QTY.1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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Orlando, FL 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/25/2015

TC LL	20 0 PSF	REF	R9114- 92693
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056013
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427781
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H11B 29 Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP M-30 B2 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

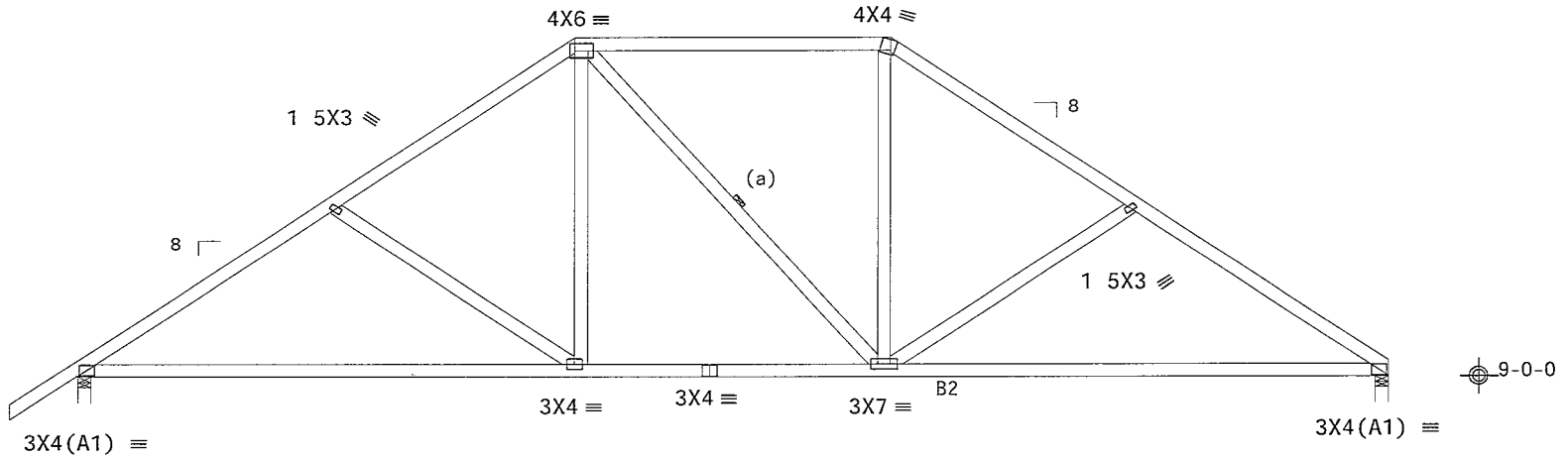
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

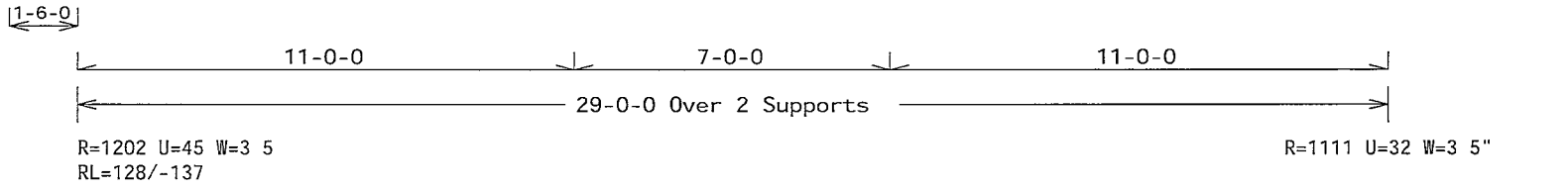
Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge





R=1202 U=45 W=3 5
RL=128/-137

R=1111 U=32 W=3 5"

Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible] QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92694
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056018
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT.LD	37 0 PSF	SEQN-	427819
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H13A 28 4" Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 7 50 ft from roof edge

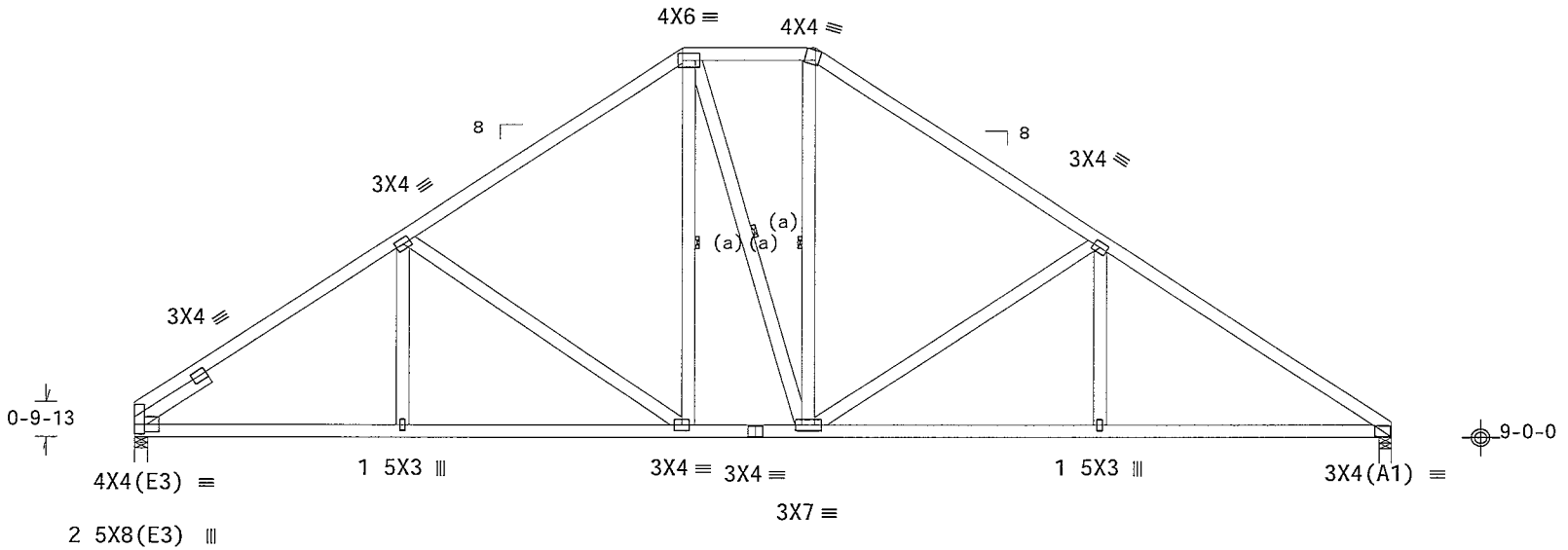
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY·1 FL/-/5/-/-/R/- Scale = 25"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92695
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056066
BC LL	0 0 PSF	HC-ENG	MAC/WPF
TOT LD	37 0 PSF	SEQN-	399815
DUR FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H13B 29 Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B4 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (1)2x6 SP M-26 supporting member
(14) 0 148 x3 nails into supporting member
(4) 0 148 x3 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

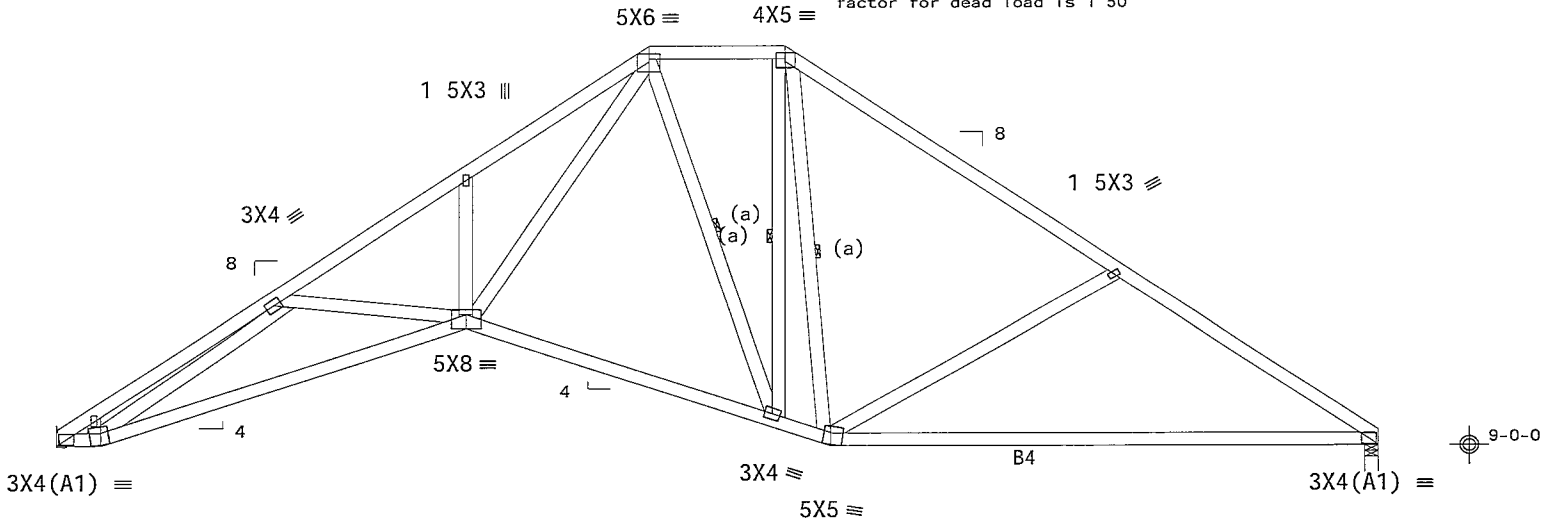
Wind loads and reactions based on MWFRS with additional C&C member design

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=1125 U=23
RL=138/-138
H=H1

R=1120 U=27 W=3 5'

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY·1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056067
BC LL	0 0 PSF		HC-ENG MAC/WPF
TOT LD	37 0 PSF	SEQN-	399844
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H7 28' Stepdown Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 T2 2x4 SP M-30
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10 CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

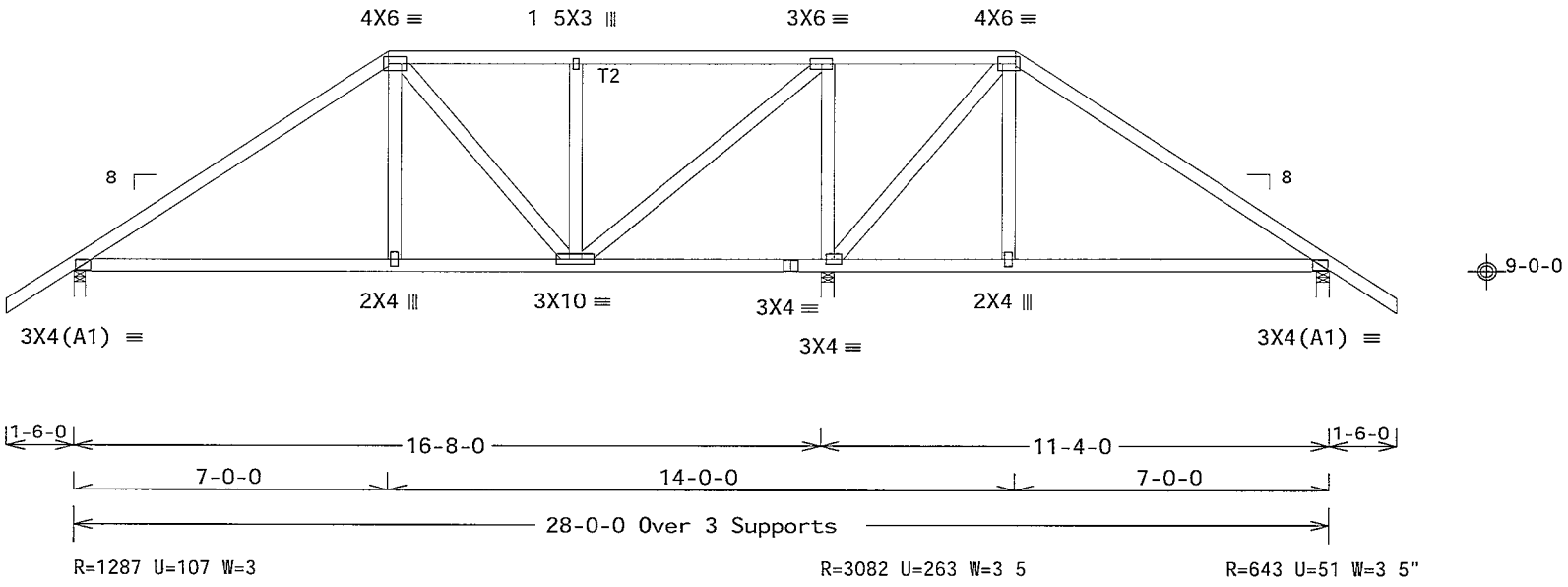
Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 21 00
TC- From 57 plf at 21 00 to 57 plf at 29 50
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 16 00
BC- From 10 plf at 16 00 to 10 plf at 20 97
BC- From 20 plf at 20 97 to 20 plf at 28 00
BC- From 5 plf at 28 00 to 5 plf at 29 50
TC- 241 88 lb Conc Load at 7 03,20 97
TC- 171 79 lb Conc Load at 9 06 11 06,13 06,14 94 16 94 18 94
BC- 458 41 lb Conc Load at 7 03,20 97
BC- 128 30 lb Conc Load at 9 06,11 06,13 06,14 94 16 94 18 94

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92697
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056006
BC LL	0 0 PSF		HC-ENG TCE/DF
TOT LD	37 0 PSF	SEQN-	427832
DUR FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

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(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H7A 28 4 Stepdown Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 T2 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E B2 2x4 SP M-30
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 6 33
TC- From 28 plf at 6 33 to 28 plf at 21 33
TC- From 57 plf at 21 33 to 57 plf at 28 33
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 6 36
BC- From 10 plf at 6 36 to 10 plf at 16 00
BC- From 10 plf at 16 00 to 10 plf at 21 30
BC- From 20 plf at 21 30 to 20 plf at 28 33
TC- 244 23 lb Conc Load at 6 36 21 30
TC- 171 79 lb Conc Load at 8 40,10 40,12 40,13 83 15 27,17 27,19 27
BC- 464 01 lb Conc Load at 6 36,21 30
BC- 128 30 lb Conc Load at 8 40,10 40,12 40,13 83 15 27,17 27,19 27

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

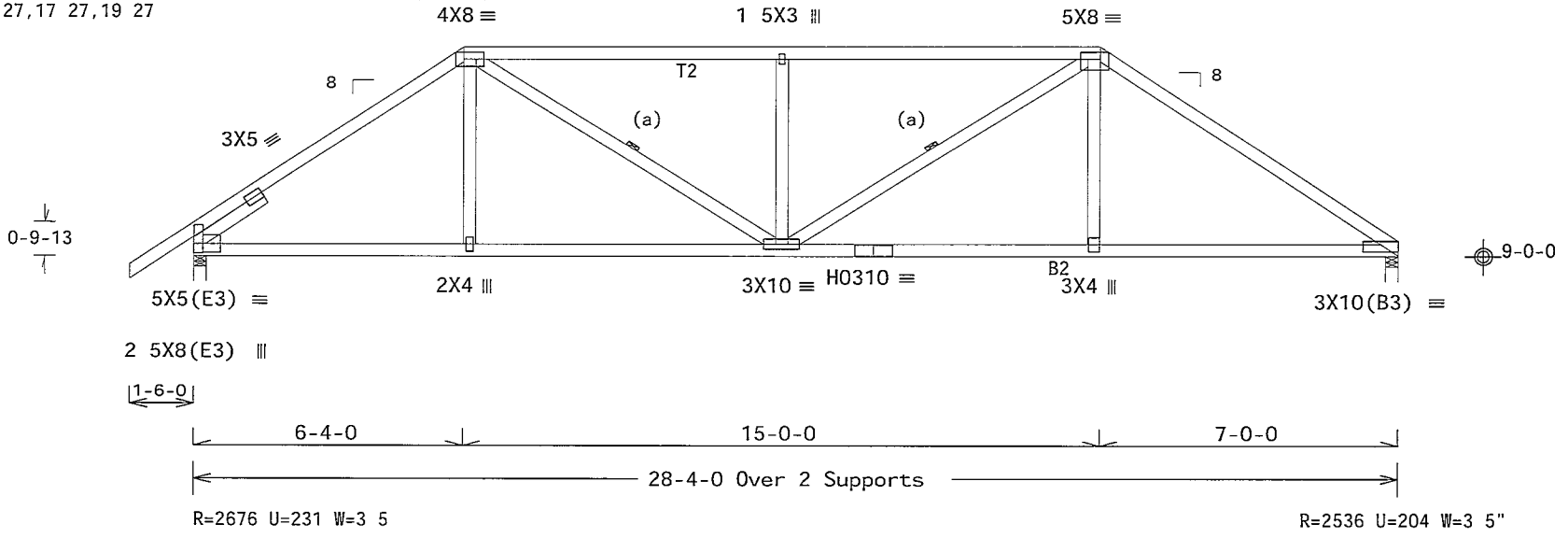
(a) or scab bracing may be used in lieu of CLR restraint substitute (1) scab for (1) CLR and (2) scabs for (2) CLR S where shown Bracing to be same size species, grade, and 80% length of web member Attach with 0 128x3 gun nails @ 6 OC

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced Attach with (2) 8d Box or Gun nails(0 113'x2 5',min) Restraint material to be supplied and attached at both ends to a suitable support by erection contractor

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible]03 QTY 1 FL/-/5/-/-/R/- Scale = 25"/Ft.

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TC LL	20 0 PSF	REF	R9114- 92698
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056011
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	1092 REV
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake C\ - H7B 29 Mono Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2 3E T1 2x4 SP #1
Bot chord 2x4 SP 2850f-2 3E B2 2x4 SP #1
Webs 2x4 SP #3 W6 2x4 SP #2

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 17 00
TC- From 28 plf at 17 00 to 28 plf at 29 00
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 16 00
BC- From 10 plf at 16 00 to 10 plf at 29 00
TC- 241 88 lb Conc Load at 7 03
TC- 171 79 lb Conc Load at 9 06 11 06 13 06 15 06
17 06 19 06 21 06 23 06 25 06 27 06
BC- 458 41 lb Conc Load at 7 03
BC- 128 30 lb Conc Load at 9 06 11 06 13 06 15 06
17 06 19 06 21 06 23 06 25 06 27 06

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3 min nails
Top Chord 1 Row @12 00 o c
Bot Chord 1 Row @12 00 o c
Webs 1 Row @ 4 o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

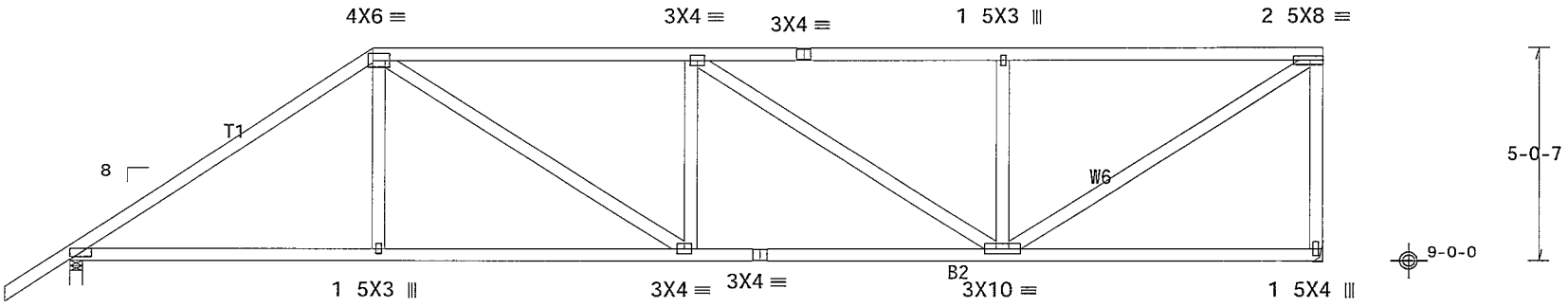
Wind loads and reactions based on MWFRS

Right end vertical not exposed to wind pressure

(J) Hanger Support Required by others

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 03 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92699
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056019
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427835
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H8 28 Stepdown Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1 T3 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1 B2 2x6 SP M-26
Webs 2x4 SP #3 W6 2x4 SP #2
Rt Wedge 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

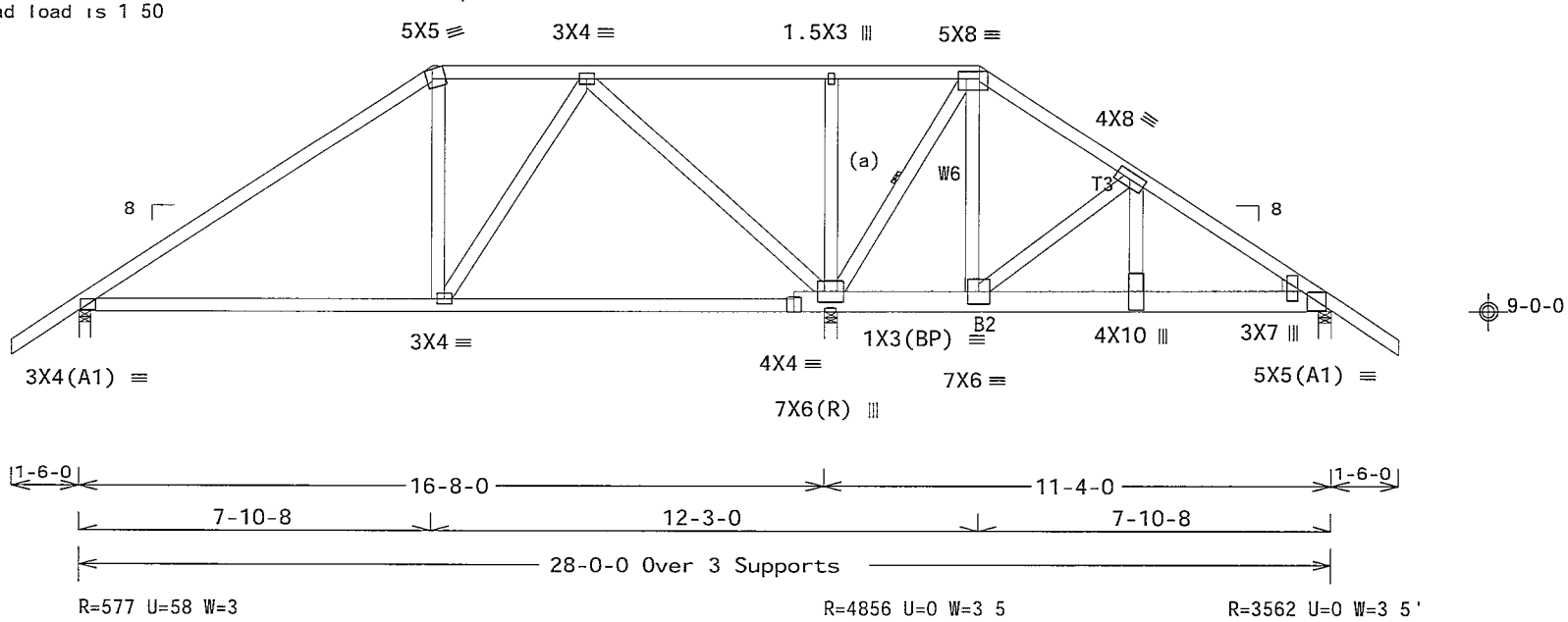
Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 88
TC- From 57 plf at 7 88 to 57 plf at 20 12
TC- From 57 plf at 20 12 to 57 plf at 29 50
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 16 00
BC- From 10 plf at 16 00 to 10 plf at 28 00
BC- From 5 plf at 28 00 to 5 plf at 29 50
BC- 1122 29 lb Conc Load at 17 27,19 27
BC- 1125 17 lb Conc Load at 21 27,23 27,25 27 27 27

Wind loads and reactions based on MWFRS

WARNING This truss is not reversible and must be installed exactly as shown It can not be used installed backwards
Truss Fabricator is to mark truss for proper erection



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible]03 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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2400 Lake Orange Dr Suite 150
Orlando, FL 32837
FL COA #0 278

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02/25/2015

TC LL	20 0 PSF	REF	R9114- 92700
TC DL	7 0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056020
BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT.LD.	37 0 PSF	SEQN-	427772
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H9A 28 4 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Lt Slider 2x4 SP #3 BLOCK LENGTH = 2 000

Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

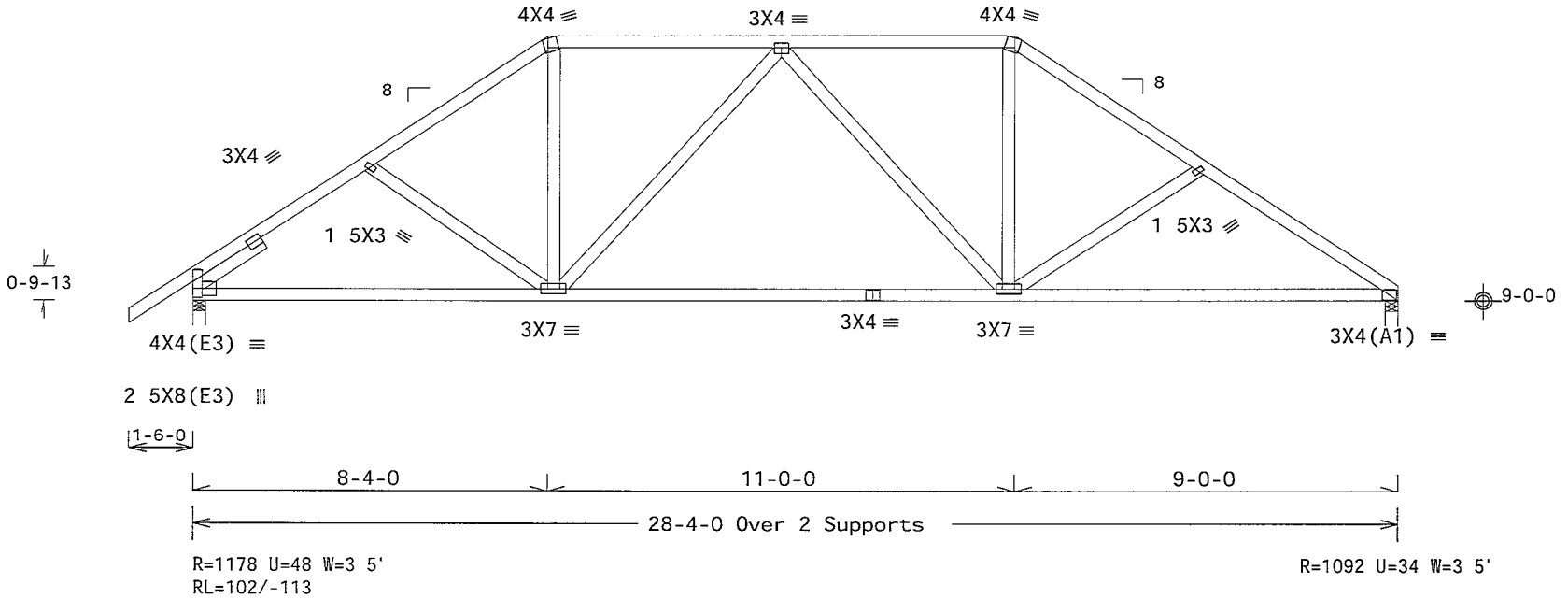
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

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TC DL	7.0 PSF	DATE	02/25/15
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TOT LD.	37 0 PSF	SEQN-	427778
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - H9B 29 Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B2 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

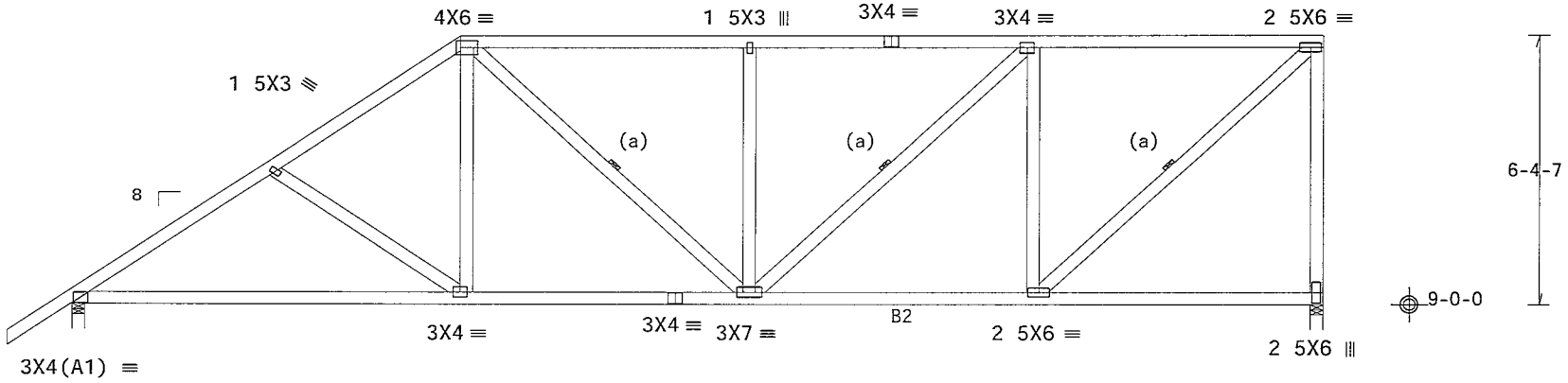
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

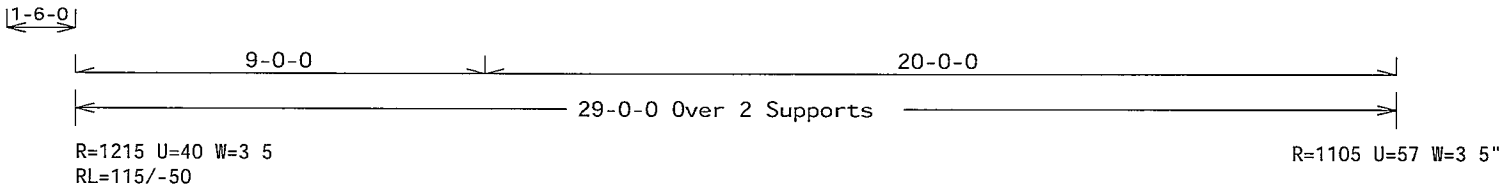
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 7 50 ft from roof edge





Design Crit FBC2010Res/TPI-2007(STD)

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TC DL	7.0 PSF	DATE	02/25/15
BC DL	10 0 PSF	DRW	HCUSR9114 15056024
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TOT LD.	37 0 PSF	SEQN-	427824
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - HJ7 9 10 13 Hip Jack Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

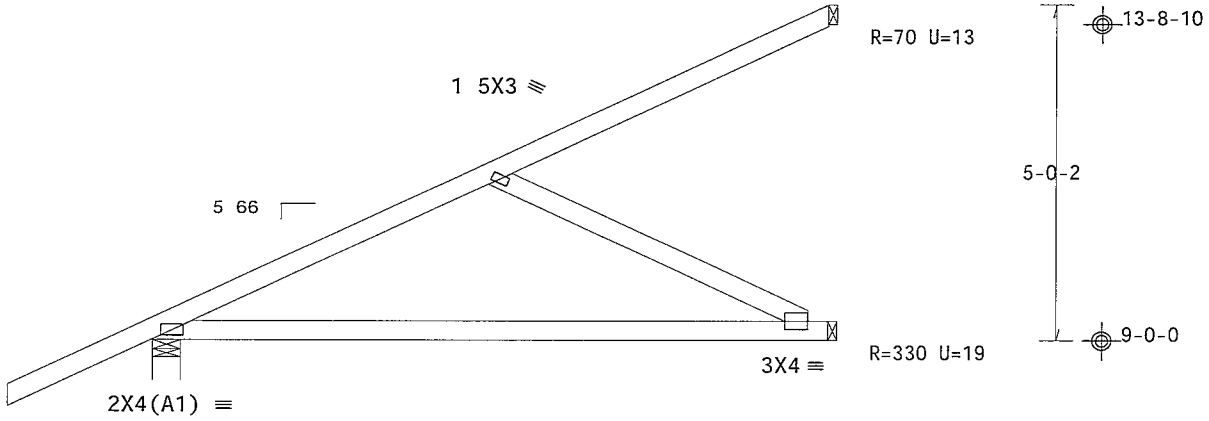
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

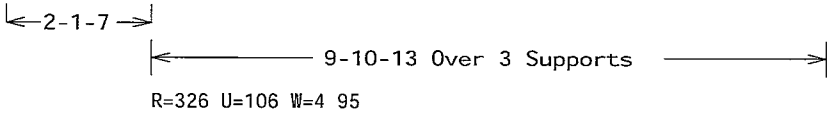
Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -36 92 lb Conc Load at 1 48
TC- 115 89 lb Conc Load at 4 31
TC- 234 06 lb Conc Load at 7 13
BC- 16 00 lb Conc Load at 1 48
BC- 101 67 lb Conc Load at 4 31
BC- 179 56 lb Conc Load at 7 13

Bottom chord checked for 10 00 psf non-concurrent live load





Design Crit· FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03.01 02/22/23 QTY 3 FL/-/5/-/-/R/- Scale =.375"/Ft.

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02/25/2015

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TC DL	7.0 PSF	DATE	02/25/15
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BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT.LD	37 0 PSF	SEQN-	427829
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Cı - HJ7A 9 10 1 Hip Jack Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

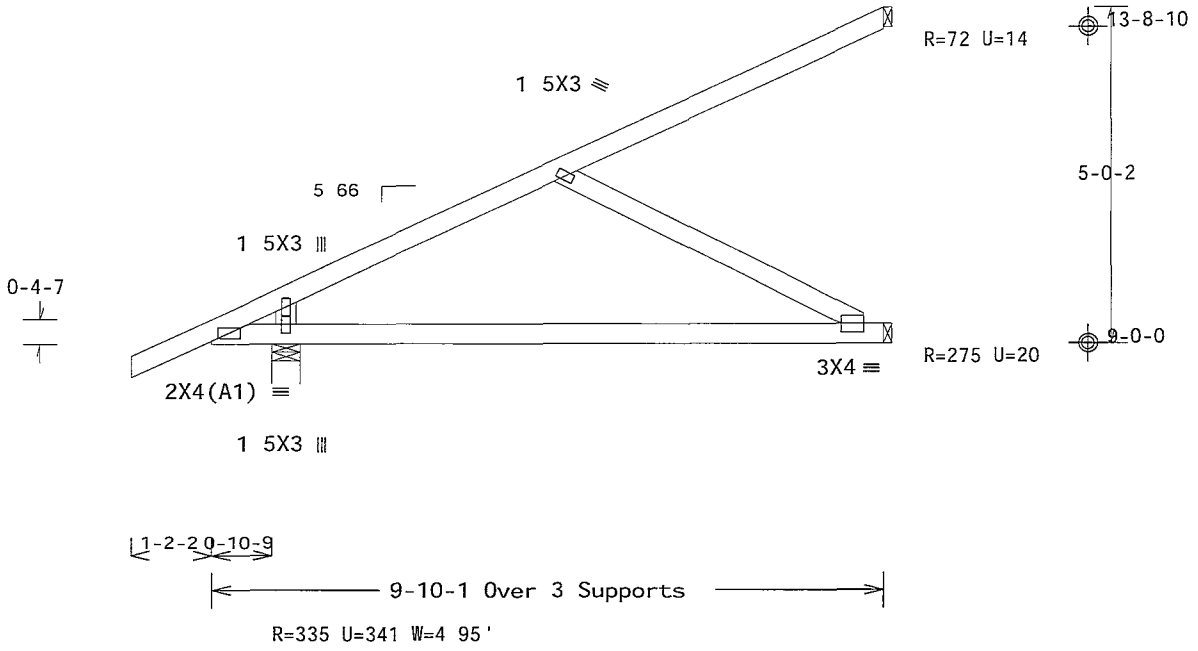
Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -1 18 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 84
BC- From 0 plf at -1 18 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 84
TC- -13 55 lb Conc Load at 1 48
TC- 94 84 lb Conc Load at 4 31
TC- 220 74 lb Conc Load at 7 13
BC- -204 65 lb Conc Load at 1 48
BC- 94 70 lb Conc Load at 4 31
BC- 171 32 lb Conc Load at 7 13

Left cantilever is exposed to wind

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit. FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 [illegible]23 QTY·1 FL/-/5/-/-/R/- Scale =.375"/Ft.

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FL COA #0 278

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TOT LD	37 0 PSF	SEQN-	427838
DUR.FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(15-036--Innovative Home Builders, /Model 1618 RH -- lot 11 Cottage Grove Lake Ci - HJ7B 9 10'13 Hip Jack Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3

Lumber value set '13B' uses design values approved 1/30/2013 by ALSC

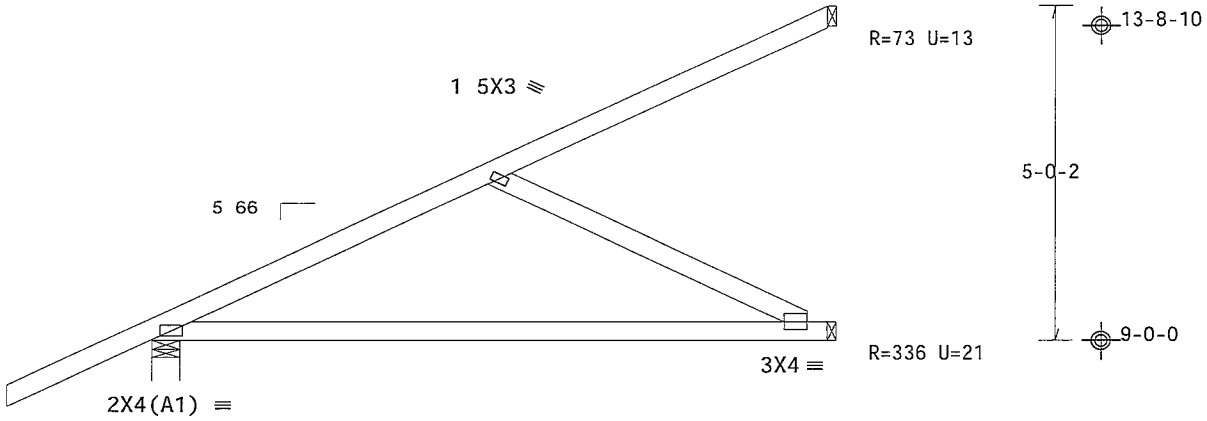
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

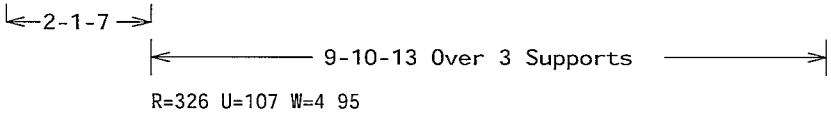
Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -36 92 lb Conc Load at 1 48
TC- 115 89 lb Conc Load at 4 31
TC- 243 75 lb Conc Load at 7 13
BC- 16 00 lb Conc Load at 1 48
BC- 101 67 lb Conc Load at 4 31
BC- 182 15 lb Conc Load at 7 13

Bottom chord checked for 10 00 psf non-concurrent live load





Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 03 01 0122 23 QTY 1 FL/-/5/-/-/R/- Scale =.375"/Ft.

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BC LL	0 0 PSF	HC-ENG	TCE/DF
TOT LD	37 0 PSF	SEQN-	427679
DUR FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1VEC487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specfied in chart below may be conservative For minimum alternative reinforcement re-run design with appropriate reinforcement type

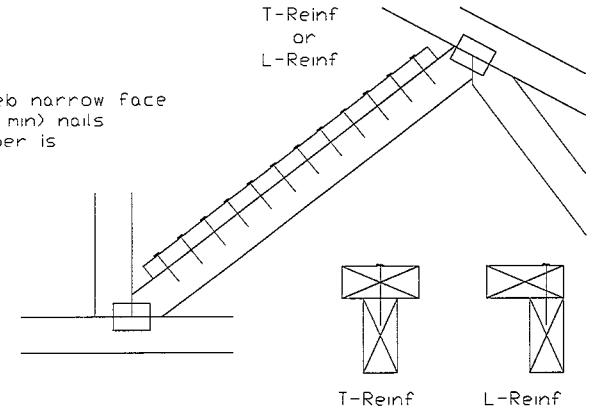
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement L-reinforcement or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineers sealed design

(*) Center scab on wide face of web Apply (1) scab to each face of web

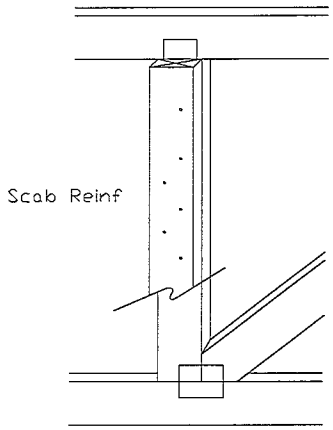
T-Reinforcement or L-Reinforcement

Apply to ether side of web narrow face Attach with 10d (0128"x3 0" min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



Scab Reinforcement

Apply scab(s) to wide face of web No more than (1) scab per face Attach with 10d (0128"x3 0" min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



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LICENSE
No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	PSF	REF	CLR Subst
TC DL	PSF	DATE	10/01/14
BC DL	PSF	DRWG	BRCLBSUB1014
BC LL	PSF		
TOT LD	PSF		
DUR FAC			
SPACING			

02/25/2015

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Enclosed Exposure D Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
	24" oc	SPF HF	#1 / #2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14 0"	14 0"	14 0"	14 0"
			#3	4 7"	7 9"	8 3"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
			Stud	4 7"	7 8"	8 2"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
			Standard	4 7"	6 7"	7 0"	8 10"	9 5"	11 5"	11 10"	13 10"	14 0"	14 0"	14 0"
		SP DFL	#1	5 0"	8 4"	8 7"	9 10"	10 2"	11 8"	12 1"	14 0"	14 0"	14 0"	14 0"
			#2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14 0"	14 0"	14 0"	14 0"
			#3	4 8"	7 0"	7 5"	9 3"	9 11"	11 5"	11 11"	14 0"	14 0"	14 0"	14 0"
			Stud	4 8"	7 0"	7 5"	9 3"	9 11"	11 5"	11 11"	14 0"	14 0"	14 0"	14 0"
			Standard	4 7"	6 2"	6 7"	8 2"	8 9"	11 1"	11 10"	12 10"	13 9"	14 0"	14 0"
	16" oc	SPF HF	#1 / #2	5 6"	9 5"	9 9"	11 1"	11 6"	13 2"	13 9"	14 0"	14 0"	14 0"	14 0"
			#3	5 3"	9 3"	9 9"	10 11"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
			Stud	5 3"	9 3"	9 7"	10 11"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
			Standard	5 3"	8 1"	8 7"	10 10"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		SP DFL	#1	5 9"	9 6"	9 10"	11 3"	11 8"	13 4"	13 10"	14 0"	14 0"	14 0"	14 0"
			#2	5 6"	9 5"	9 9"	11 1"	11 6"	13 2"	13 9"	14 0"	14 0"	14 0"	14 0"
			#3	5 5"	8 6"	9 1"	11 0"	11 5"	13 1"	13 8"	14 0"	14 0"	14 0"	14 0"
			Stud	5 5"	8 6"	9 1"	11 0"	11 5"	13 1"	13 8"	14 0"	14 0"	14 0"	14 0"
			Standard	5 3"	7 6"	8 0"	10 0"	10 9"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
	12" oc	SPF HF	#1 / #2	6 1"	10 4"	10 8"	12 2"	12 8"	13 2"	14 0"	14 0"	14 0"	14 0"	14 0"
			#3	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			Stud	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			Standard	5 9"	9 4"	9 11"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		SP DFL	#1	6 4"	10 6"	10 10"	12 4"	12 10"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			#2	6 1"	10 4"	10 8"	12 2"	12 8"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			#3	5 11"	9 10"	10 6"	12 1"	12 7"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			Stud	5 11"	9 10"	10 6"	12 1"	12 7"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
			Standard	5 9"	8 8"	9 3"	11 7"	12 5"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

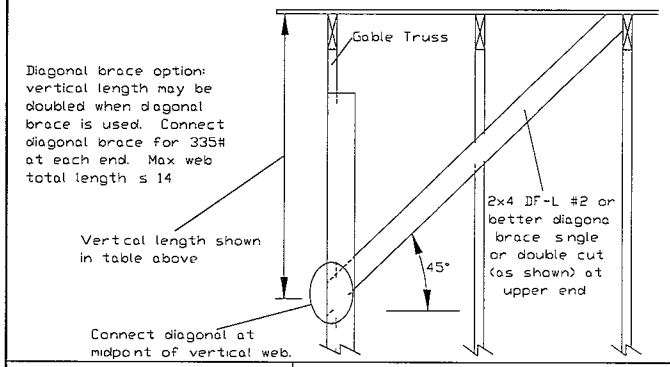
** For (2) "L" braces space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

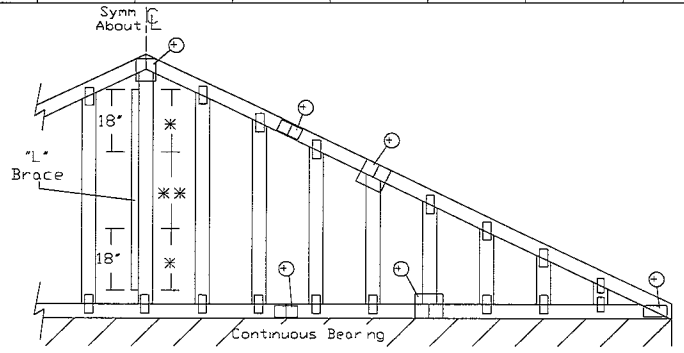
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak splice and heel plates

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length

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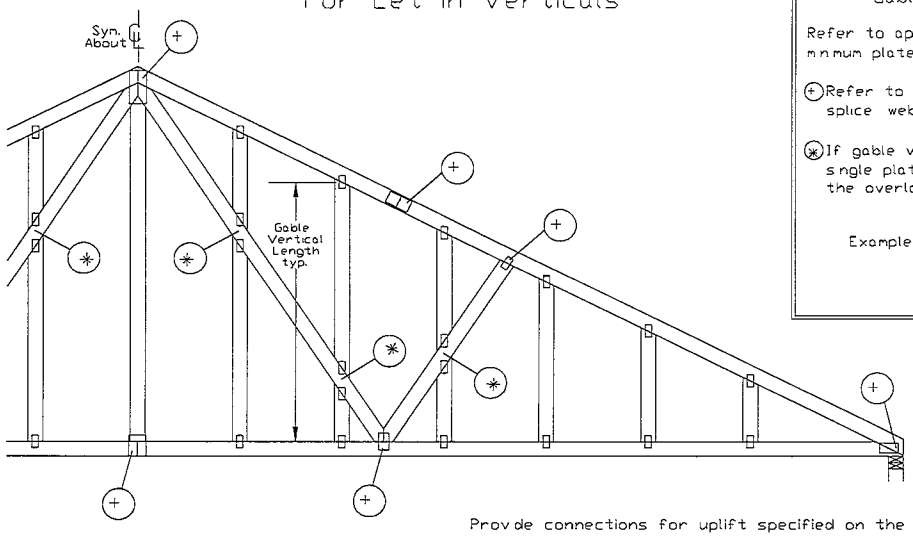
02/25/2015

MAX TOT LD 60 PSF

MAX SPACING 24 0

REF	ASCE7-10-GAB12015
DATE	10/01/14
DRWG	A12015ENC101014

Gable Detail
For Let-in Verticals

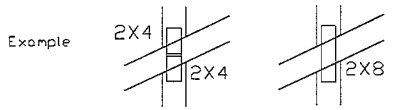


Gable Truss Plate Sizes

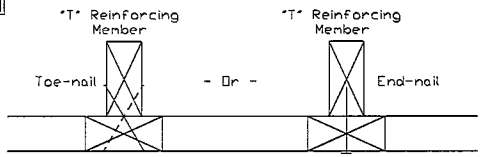
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs

(+) Refer to Engineered truss design for peak splice web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3" min) Nails at 4" oc plus
- (4) nails in the top and bottom chords

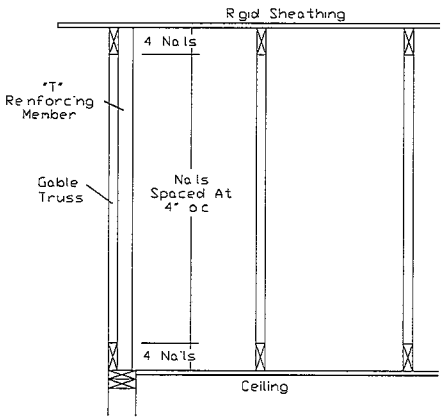
Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" oc plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate Alpine gable detail for ASCE wind load

ASCE 7-05 Gable Detail Drawings
A13015051014 A12015051014 A11015051014 A10015051014 A14015051014
A130300510 4 A12030051014 A11030051014 A10030051014 A14030051014

ASCE 7-10 Gable Detail Drawings
A11515ENC101014 A12015ENC101014 A14015ENC101014 A16015ENC101014
A18015ENC101014 A20015ENC101014 A20015END101014 A20015PED101014
A11530ENC101014 A12030ENC101014 A14030ENC101014 A16030ENC101014
A18030ENC101014 A20030ENC101014 A20030END101014 A20030PED101014

See appropriate Alpine gable detail for maximum unreinforced gable vertical length



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail)

Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord.

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft Kzt = 1.00
- Gable Vertical = 24"o.c SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"

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MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24 0"

REF	LET-IN VERT
DATE	10/01/14
DRWG	GBLLETIN1014

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt ASCE 7-10, Enclosed Exp C or
100 mph, 30ft Mean Hgt, ASCE 7-10 Enclosed, Exp D or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

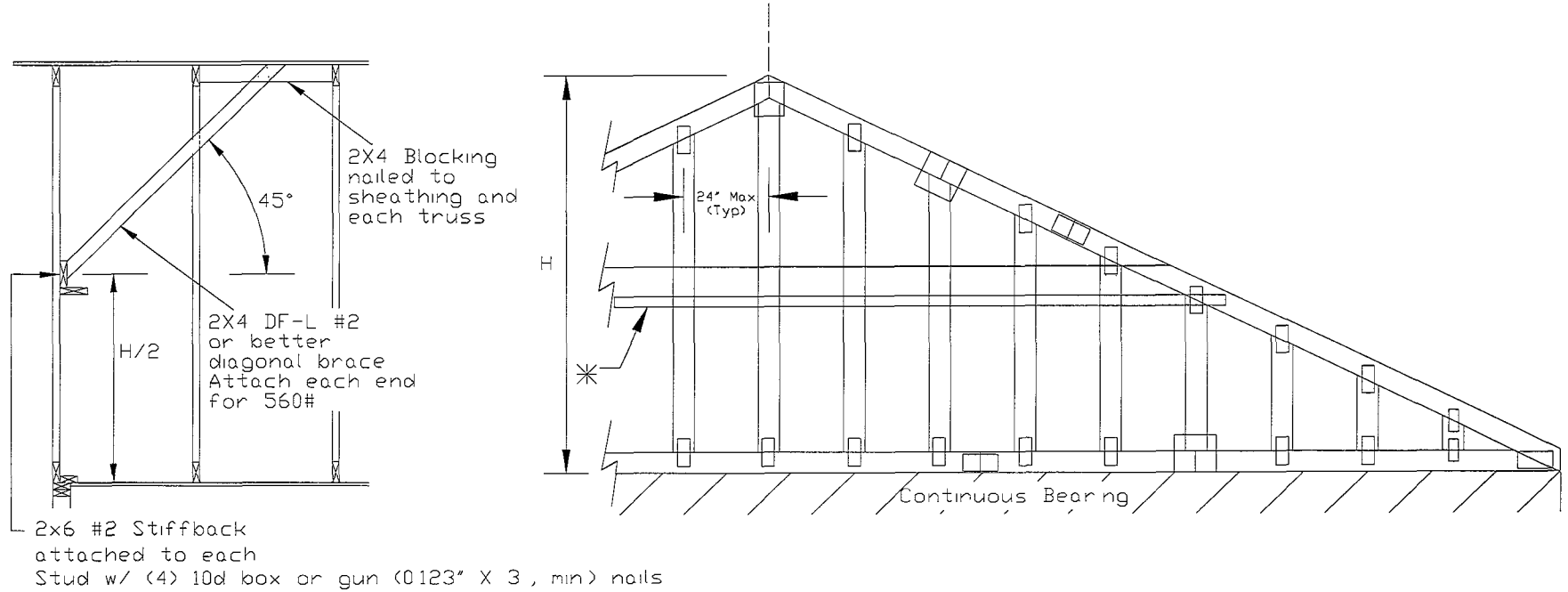
Nails 10d box or gun (0.128 x3 ,min) nails

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC101014)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC101014)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.



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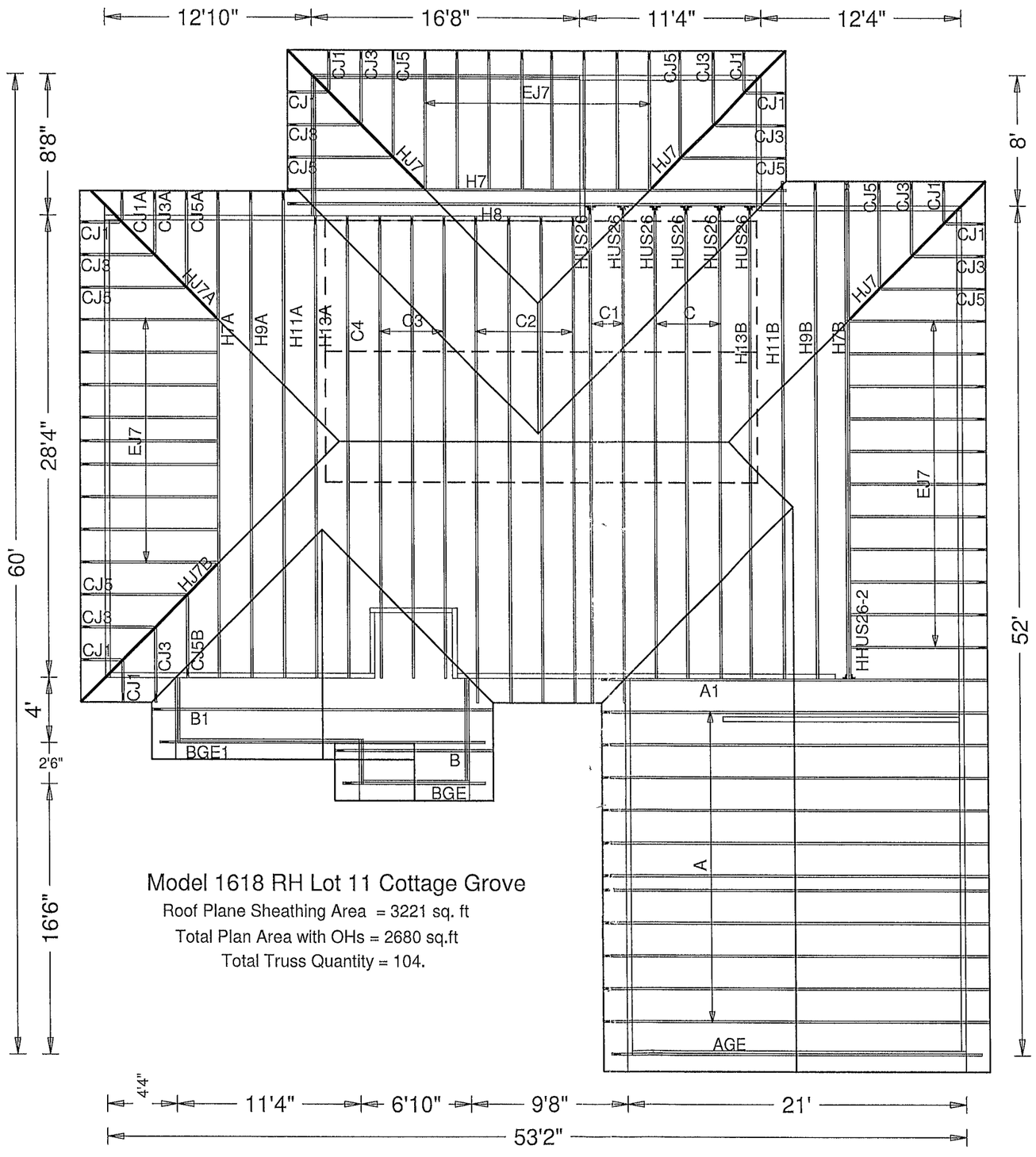
MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	10/01/14
DRWG	GABRST101014



Model 1618 RH Lot 11 Cottage Grove

Roof Plane Sheathing Area = 3221 sq. ft

Total Plan Area with OHs = 2680 sq.ft

Total Truss Quantity = 104.

ANDERSON TRUSS COMPANY
"QUALITY PRE-ENGINEERED WOOD TRUSSES"

Created : 02-25-2015
: <Not Found>

Customer: Innovative Home Builders,
Job Name: Model 1618 RH
: lot 11 Cottage Grove
Job Numb: 15-036
Designer: Jon Williams
Salesman: CVB

JOB NO:
15-036

PAGE NO:
1 OF 1

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