DATE 10/07/2005

# Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

**PERMIT** 000023692

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER DEBRA GRIFFIN/CINDY THOMAS PHONE 752-4072

ADDRESS 920 NW SCENIC LAKE DRIVE LAKE CITY FL 32055

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY LAKE JEFFREY DRIVE, TL ON SCENIC LAKE DRIVE, LOT 23 ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 133700.00

HEATED FLOOR AREA 2674.00 TOTAL AREA 4733.00 HEIGHT .00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 25

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 22-3S-16-02268-223 SUBDIVISION WOODBOROUGH

LOT 23 BLOCK PHASE 1 UNIT TOTAL ACRES .75

| 000000836 | | CGC036224 | Katie Reed |
|---|---|---|---|
| Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor |

| CULVERT | 05-0738-N | BK | JH | Y |
|---|---|---|---|---|
| Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident |

COMMENTS: 1ST FLOOR TO BE 139.2 MIN.PER PLAT, ELEVATION LETTER REQUIRED BEFORE SLAB, NOC ON FILE, ALTERNATIVE TERMIT TREATMENT REC'D

Check # or Cash 4358

## FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 670.00 CERTIFICATION FEE $ 23.66 SURCHARGE FEE $ 23.66

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 817.32

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

**This Permit Must Be Prominently Posted on Premises During Construction**

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

# Columbia County Building Permit Application CK#4353 Revised 9-23-04

For Office Use Only Application # 0509-14 Date Received 9-2-05 By LH Permit # 836/23692

Application Approved by - Zoning Official BLK Date 9.10.05 Plans Examiner OKJTH Date 9-22-05

Flood Zone X Site Plan Development Permit YES Zoning RSF-2 Land Use Plan Map Category RES. Low Dens.

Comments 1ST Floor to be at 139.2 ft. minimum per plat Elevation letter Required

NOC

Applicants Name Katie Reed Phone 386-752-4072 755-7272

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Owners Name Debra Griffin and Cindy Thomas Phone 386-752-4072

911 Address 920 NW Scenic Lake Drive Lake City, FL 32055

Contractors Name Don Reed Construction, Inc. Phone 386-752-4072

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disosway P.E. P.O. Box 868 Lake City, FL 32056

Mortgage Lenders Name & Address Mercantile Bank 187 SW Baya Drive Lake City, FL 32025

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 22-3S-16-02268-223 Estimated Cost of Construction $406,746.00

Subdivision Name Woodborough Phase I Lot 23 Block ____ Unit ____ Phase ____

Driving Directions Lake Jeffery Rd; TL into Woodborough Subdivision on Scenic Lake Drive; follow main drive to Lot 23 on the left - on the lake side

Type of Construction single family dwelling Number of Existing Dwellings on Property 0

Total Acreage .758 Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 40' Side 35'61/2" Side 35' Rear 63'5"

Total Building Height 25' Number of Stories 2 Heated Floor Area 2,674 Roof Pitch 7/12

Porches 884 GARAGE 764 2674 + 411 = 3085 TOTAL 4733

**Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.**

**OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.**

**WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.**

Don Reed

Owner Builder or Agent (Including Contractor) Contractor Signature

Contractors License Number CGC036224

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY STAMP/SEAL

Notary Public State of Florida
Ingrid Geiger
My Commission DD385312
Expires 01/26/2009

Sworn to (or affirmed) and subscribed before me this 31st day of Aug 2005.

Personally known ✓ or Produced Identification ____

Ingrid Geiger

Notary Signature

Est. 1973

**DON REED CONSTRUCTION, INC.**

Builder of Fine Homes & Offices

2230 SE Baya Drive, Suite 101
Lake City, Florida 32025
(386) 752-4072
Fax- (386) 755-7272

August 3, 2005

Columbia County Building Department
P.O. Drawer 1529
Lake City, FL 32056

To Whom It May Concern:

We are submitting the Permit Package for the home of Debbie Griffin and Cindy Thomas. Please note the recorded Notice of Commencement is not enclosed. As soon as the closing is processed and we receive a recorded copy, we will forward a copy to you. Should you have any questions or need any other information, please contact our office. Thank you.

Sincerely,

Don Reed

Don Reed

0509-14

Inst:2005023283 Date:09/21/2005 Time:15:15
DC,P.DeWitt Cason,Columbia County B:1059 P:394

# NOTICE OF COMMENCEMENT

**STATE OF FLORIDA**
**COUNTY OF:** Columbia

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property: Lot 23
XXX NW Scenic Dr
Woodborough
Lake City, FL 32055

2. General Description of Improvements: Residential Construction

3. Name and Address of Owner: Debra Griffin and Cynthia Thomas
276 SW Wilshire Dr
Lake City, FL 32024

Interest in Property: Fee Simple

Name and Address of Fee Simple Titleholder (if other than owner): N/A

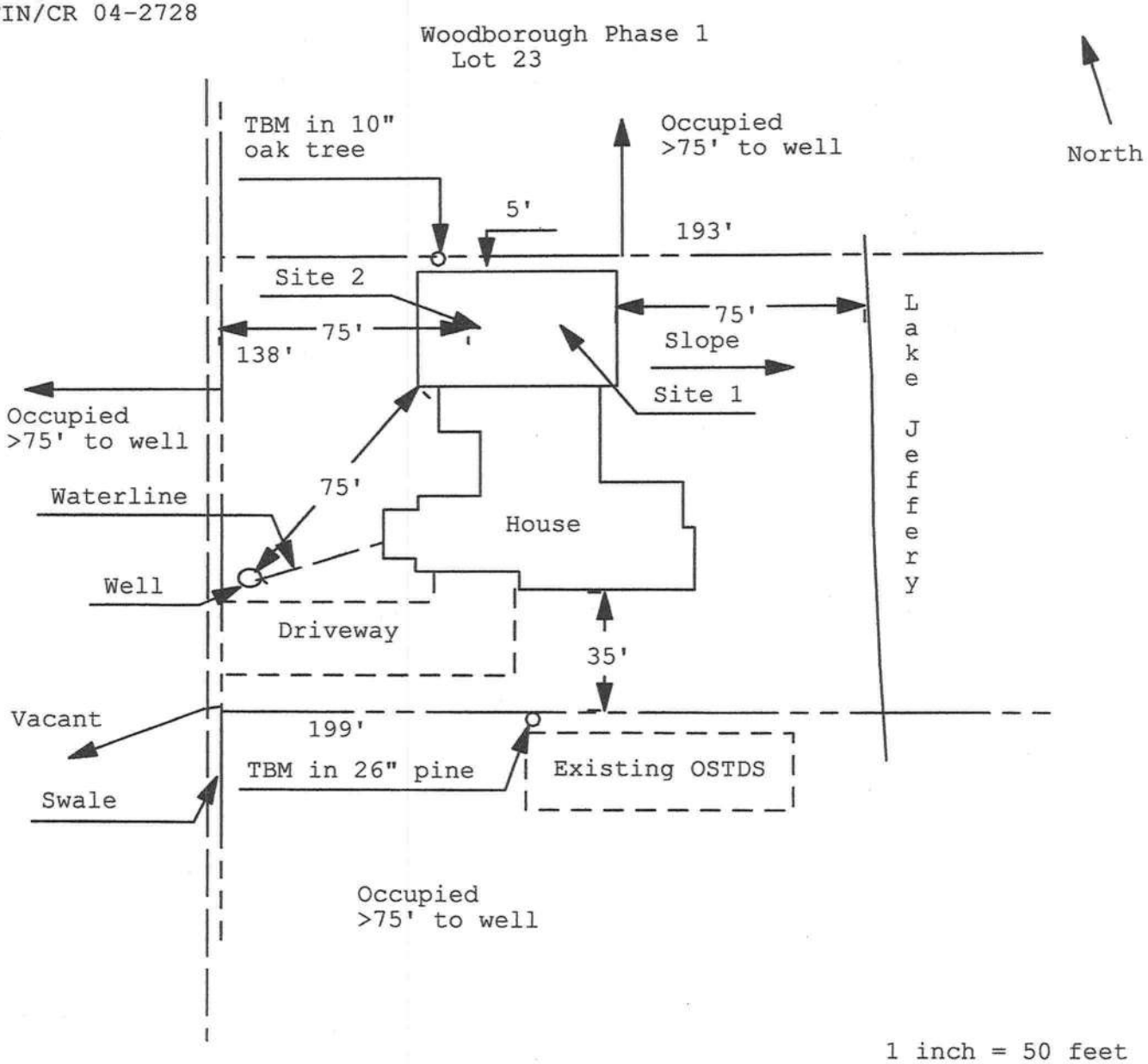
ST[illegible] OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By [signature] Deputy Clerk

# Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

**Permit Application Number:** 05-0738N

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

GRIFFIN/CR 04-2728

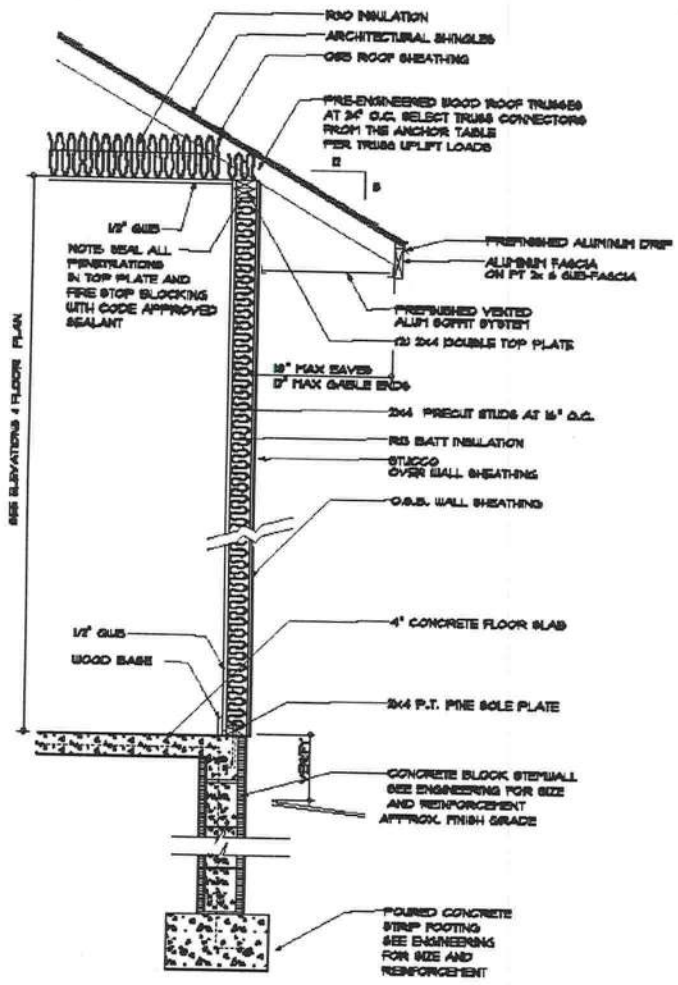



**Site Plan Submitted By** Paul Lloyd **Date** 7/6/05

**Plan Approved** ✓ **Not Approved** ____ **Date** 7-11-05

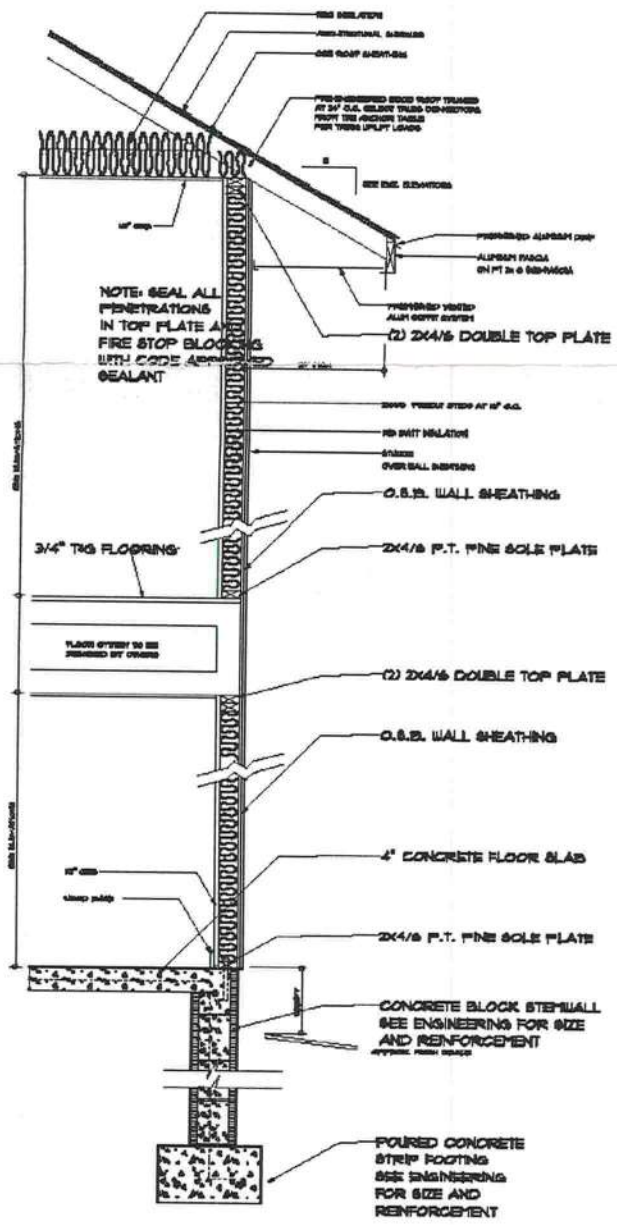
**By** [signature] (Columbia) **CPHU**

**Notes:** Reviewed by K. Kassens 7/19/05




## TYPICAL DESIGN WALL SECTION
## NON - STRUCTURAL DATA

SCALE : N.T.S.




## TYPICAL DESIGN WALL SECTION
## 2 STORY
## NON - STRUCTURAL DATA

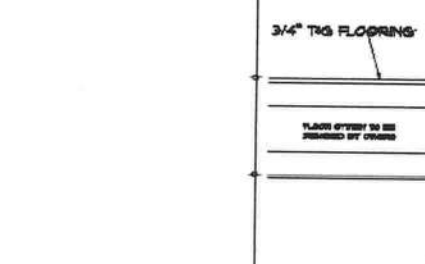
SCALE : N.T.S.



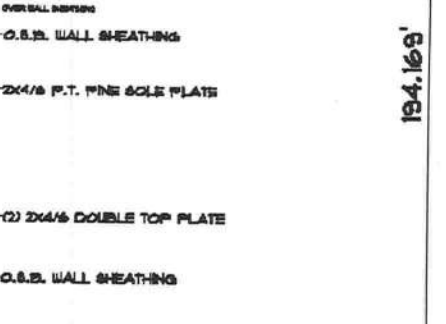
SECTION VIEW



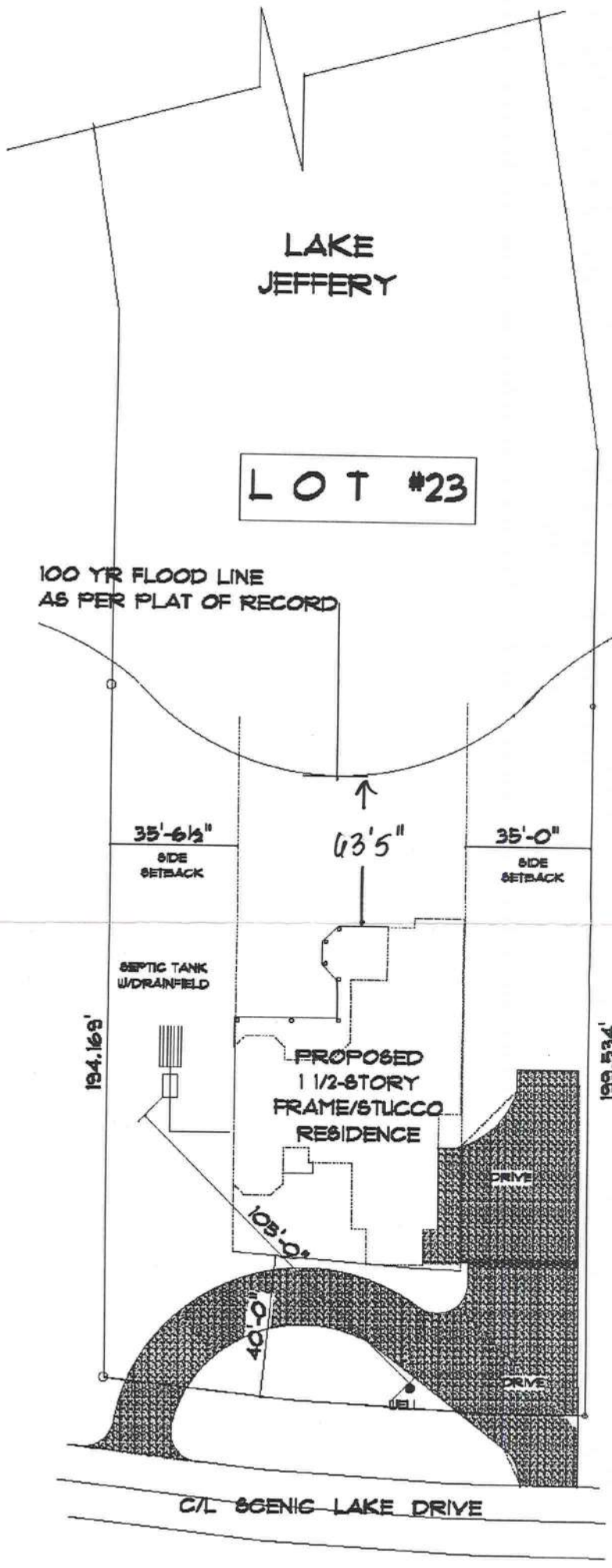
GUARDRAIL DETAIL




TYPICAL NICHE
N.T.S.




STAIR DETAIL




## * SITE PLAN *

SCALE : 1" = 20'

RESIDENCE for DEBBIE GRIFFIN & CINDY THOMAS IN WOODBORO on LAKE JEFFERY

Teresa M. Rivero
351 NW 91st Ct Pmb 121
Lake City, Florida 32055
Phone (386) 754 - 9990
Cell (386) 697 - 1181
trivero@bellsouth.net

revision date: August 26, 2008
DRAWN BY: E.M. G.S.J.
CHECKED BY:
return comment

FINAL DATE: AUGUST 2008
JOB NUMBER:

DRAWING NUMBER A-4
OF 4 SHEETS

DESCRIPTION:
LOT 23 OF "WOODBOROUGH PHASE I" AS PER PLAT THEREOF RECORDED IN PLAT BOOK 5, PAGE 114 & 114A OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
3. SOME PORTIONS OF THIS PARCEL ARE IN ZONE "A" AND MAY BE SUBJECT TO FLOODING. HOWEVER, NO BASE FLOOD ELEVATION HAS BEEN DETERMINED FOR ZONE "A". SOME PORTIONS OF THIS PARCEL ARE IN ZONE "X" AND ARE DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD INSURANCE RATE MAP, DATED 6 JAN. 1988 COMMUNITY PANEL NO. 120070 0175 B. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.

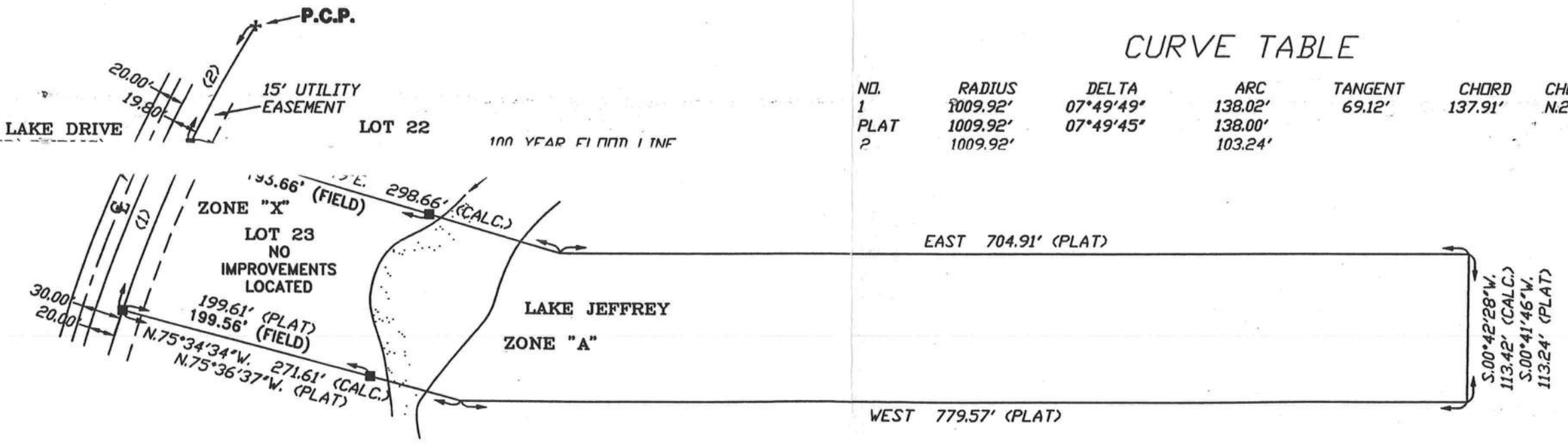
# BOUNDARY SURVEY IN SECTION 22, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.

SYMBOL LEGEND:

- ■ 4"X4" CONCRETE MONUMENT FOUND
- □ 4"X4" CONCRETE MONUMENT SET
- ● IRON PIPE FOUND
- ○ IRON PIN AND CAP SET
- ⊕ POWER POLE
- ▲ WATER METER
- ℄ CENTERLINE
- ✱ WELL
- ⍟ SATELLITE DISH
- ⊙ TELEPHONE BOX
- --E-- ELECTRIC LINES
- --X-- WIRE FENCE
- --o-- CHAIN LINK FENCE
- --□-- WOODEN FENCE

N

SCALE: 1" = 100'




## CURVE TABLE

| NO. | RADIUS | DELTA | ARC | TANGENT | CHORD | CHORD BEARING |
|---|---|---|---|---|---|---|
| 1 | 1009.92' | 07°49'49" | 138.02' | 69.12' | 137.91' | N.22°07'09"E. |
| PLAT | 1009.92' | 07°49'45" | 138.00' | | | |
| 2 | 1009.92' | | 103.24' | | | |

NOTE: ALL PROPERTY CORNERS LOCATED ARE IDENTIFIED AS DONALD LEE, P.L.S. 3628.

SPECIAL FLOOD NOTE: ACCORDONG TO THE PLAT THEREOF A 100 YEAR ELEVATION HAS BEEN ESTABLISHED FOR THIS LOT ESTABLISHED TO BE 138.20 FEET.

CERTIFIED TO:
DEBRA GRIFFIN

FIELD BOOK: SEE PAGE(S): FILE

SURVEYOR'S CERTIFICATION:

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

01/14/05 FIELD SURVEY DATE
01/16/05 DRAWING DATE

C. SCOTT BRITT, P.S.M. CERTIFICATION # 5757

NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.


BRITT SURVEYING
LAND SURVEYORS AND MAPPERS
1426 WEST DUVAL STREET LAKE CITY, FLORIDA 32055
(904)752-7163 FAX (904)752-5573


WORK ORDER # L-15719

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 04-895
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number R02268-223

Inst:2005001157 Date:01/18/2005 Time:16:49
Doc Stamp-Deed : 945.00
DC,P.DeWitt Cason,Columbia County B:1035 P:2129

**WARRANTY DEED**

**This Warranty Deed**, made this 17th day of January, 2005, BETWEEN H.C. CORBITT, III, whose post office address is 854 NW Guerdon Street, Lake City, FL 32055, of the County of Columbia, State of Florida, grantor*, and DEBRA K. GRIFFIN and CYNTHIA E. THOMAS, as joint tenants with full right of survivorship, whose post office address is 276 SW Wilshire Drive, Lake City, FL 32024, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

**Witnesseth:** that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 23, WOODBOROUGH PHASE 1, a subdivision according to the plat thereof as recorded in Plat Book 5, Pages 114-114A of the public records of Columbia County, Florida.

N.B.: Neither the Grantor nor any member of his family live on or reside on the property described herein or any adjacant land thereto or claim any part hereof or any adjacent land thereto as their homestead.

**Together** with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

**To Have and to Hold,** the same in fee simple forever.

**And** subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

**In Witness Whereof**, grantor has hereunto set grantor's hand

Signed, sealed and delivered
in our presence:

(Signature of First Witness)
Terry McDavid
(Typed Name of First Witness)

(Signature of Second Witness)
Crystal L. Brunner
(Typed Name of Second Witness)

Grantor (SEAL)
H.C. CORBITT, III
Printed Name

Inst:2005001157 Date:01/18/2005 Time:16:49
Doc Stamp-Deed : 945.00
DC,P.DeWitt Cason,Columbia County B:1035 P:2130

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 17th day of January, 2005, by H.C. CORBITT, III, who is personally known to me or who has produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY MCDAVID
MY COMMISSION EXPIRES
January 16, 2006
#DD 079305
Bonded thru
Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA

# COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-2949
PHONE: (386) 752-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

## Addressing Maintenance

To maintain the Countywide addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

**DATE ISSUED:** May 23, 2005

**ENHANCED 9-1-1 ADDRESS:**

920 NW SCENIC LAKE DR (LAKE CITY, FL 32055)

**Addressed Location 911 Phone Number:** NOT AVAIL.

**OCCUPANT NAME:** NOT AVAIL.

**OCCUPANT CURRENT MAILING ADDRESS:**

**PROPERTY APPRAISER MAP SHEET NUMBER:** 43

**PROPERTY APPRAISER PARCEL NUMBER:** 22-3S-16-02268-223

**Other Contact Phone Number (If any):**

**Building Permit Number (If known):**

**Remarks:** LOT 23 WOODBOROUGH PHASE 1 S/D

Address Issued By: _____
Columbia County 9-1-1 Addressing Department

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

22-3S-16-02268-223

LOT 23 WOODBOROUGH PHASE 1
ORB 742-237, WD 1035-2129.

GRIFFIN DEBRA K &
CYNTHIA E THOMAS (JTWRS)
276 SW WILSHIRE DR
LAKE CITY FL 32024

22-3S-16-02268-223

Columbia County 2005 R
CARD 001 of 001
PRINTED 5/02/2005 8:40 BY JEFF
APPR 5/20/2003 DF

| | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| USE | | AE? | HTD AREA | .000 | INDEX | 22316.01 | NBHD | PROP USE | 000000 VACANT |
| MOD | BATH | | EFF AREA | | E-RATE | .000 | INDX | STR 22- 3S- 16 | |
| EXW | FIXT | | RCN | | | | AYB | MKT AREA 06 | 0 BLDG |
| % | BDRM | | %GOOD | | BLDG VAL | | EYB | (PUD1 | 0 XFOB |
| RSTR | RMS | | | | | | | AC .758 | 65,000 LAND |
| RCVR | UNTS | | FIELD CK: | | | | | NTCD | 0 AG |
| % | C-W% | | LOC: LOT 23 WOODBOROUGH PHASE 1 | | | | | APPR CD | 0 MKAG |
| INT | HGHT | | | | | | | CNDO | 65,000 JUST |
| % | PMTR | | | | | | | SUBD | 0 CLAS |
| FLR | STYS | | | | | | | BLK | |
| % | ECON | | | | | | | LOT | 0 SOHD |
| HTTP | FUNC | | | | | | | MAP# | 0 ASSD |
| A/C | SPCD | | | | | | | | 0 EXPT |
| QUAL | DEPR | | | | | | | TXDT 002 | 0 COTXBL |
| FNDN | UD-1 | | | | | | | | |
| SIZE | UD-2 | | | | | | | BLDG TRAVERSE | |
| CEIL | UD-3 | | | | | | | | |
| ARCH | UD-4 | | | | | | | | |
| FRME | UD-5 | | | | | | | | |
| KTCH | UD-6 | | | | | | | | |
| WNDO | UD-7 | | | | | | | | |
| CLAS | UD-8 | | | | | | | | |
| OCC | UD-9 | | | | | | | | |
| COND | % | | | | | | | | |

SUB A-AREA % E-AREA SUB VALUE

PERMITS

| NUMBER | DESC | AMT | ISSUED |
|---|---|---|---|

SALE

| BOOK | PAGE | DATE | PRICE |
|---|---|---|---|
| 1035 | 2129 | 1/17/2005 Q V | 135000 |
| GRANTOR H C CORBITT III | | | |
| GRANTEE DEBRA K GRIFFIN & CYNTHIA E THOMAS | | | |
| 742 | 237 | 2/25/1991 Q V | 49900 |
| GRANTOR WOODBORROUGH | | | |
| GRANTEE H C CORBITT III | | | |

TOTAL

EXTRA FEATURES

| AE | BN | CODE | DESC | LEN | WID | HGHT | QTY | QL | YR | ADJ | UNITS | UT | PRICE | ADJ UT PR | SPCD % | %GOOD | XFOB VALUE |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|

FIELD CK:

| AE | LAND CODE | DESC | ZONE | TOPO | ROAD UTIL | UD1 UD2 | UD3 UD4 | FRONT BACK | DEPTH DT | ADJUSTMENTS | UNITS | UT | PRICE | ADJ UT PR | LAND VALUE |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| Y | 000133 | SFR LAKE | A-1 | 0002 | 0003 | 0002 | 0003 | 136 | 243 | 1.00 1.00 1.00 1.00 | 1.000 | LT | 65000.000 | 65000.00 | 65,000 |

L001 - 0.76 AC UPLANDS; 2.81 AC TOTAL
SALE - LOT 23, WOODBOROUGH PHASE 1
2005

# HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

FORM 600A-2001 EnergyGauge® 3.4

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

## Residential Whole Building Performance Method A

| | | | |
|---|---|---|---|
| Project Name: | **508085ReedDonGriffinDebra&Thomas** | Builder: | |
| Address: | **Scenic Lake Drive** | Permitting Office: | |
| City, State: | **Lake City, FL** | Permit Number: | 23692 |
| Owner: | **Griffin Debra & Thomas** | Jurisdiction Number: | 221000 |
| Climate Zone: | **North** | | |

| | | | |
|---|---|---|---|
| 1. New construction or existing | | New | |
| 2. Single family or multi-family | | Single family | |
| 3. Number of units, if multi-family | | 1 | |
| 4. Number of Bedrooms | | 3 | |
| 5. Is this a worst case? | | Yes | |
| 6. Conditioned floor area (ft²) | | 3085 ft² | |
| 7. Glass area & type | Single Pane | Double Pane | |
| a. Clear glass, default U-factor | 0.0 ft² | 556.0 ft² | |
| b. Default tint, default U-factor | 0.0 ft² | 0.0 ft² | |
| c. Labeled U-factor or SHGC | 0.0 ft² | 0.0 ft² | |
| 8. Floor types | | | |
| a. Slab-On-Grade Edge Insulation | | R=0.0, 276.0(p) ft | |
| b. N/A | | | |
| c. N/A | | | |
| 9. Wall types | | | |
| a. Frame, Wood, Exterior | | R=13.0, 2798.0 ft² | |
| b. Frame, Wood, Adjacent | | R=13.0, 190.0 ft² | |
| c. N/A | | | |
| d. N/A | | | |
| e. N/A | | | |
| 10. Ceiling types | | | |
| a. Under Attic | | R=30.0, 3614.0 ft² | |
| b. N/A | | | |
| c. N/A | | | |
| 11. Ducts | | | |
| a. Sup: Unc. Ret: Unc. AH: Garage | | Sup. R=6.0, 260.0 ft | |
| b. N/A | | | |

| | | |
|---|---|---|
| 12. Cooling systems | | |
| a. Central Unit | Cap: 74.0 kBtu/hr | |
| | SEER: 12.00 | |
| b. N/A | | |
| c. N/A | | |
| 13. Heating systems | | |
| a. Electric Heat Pump | Cap: 74.0 kBtu/hr | |
| | HSPF: 8.00 | |
| b. N/A | | |
| c. N/A | | |
| 14. Hot water systems | | |
| a. Electric Resistance | Cap: 40.0 gallons | |
| | EF: 0.89 | |
| b. N/A | | |
| c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) | | |
| 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) | | |

Glass/Floor Area: 0.18

Total as-built points: 44116
Total base points: 44284

**PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** Ben Sparks

**DATE:** 9/10/05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER/AGENT:** _____

**DATE:** _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

**BUILDING OFFICIAL:** _____

**DATE:** _____

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **GLASS TYPES** .18 X Conditioned Floor Area | X BSPM | = Points | | Type/SC | Overhang Ornt | Len | Hgt | Area X | SPM X | SOF | = Points | |
| .18 | 3085.0 | 20.04 | 11128.2 | Double, Clear | SE | 2.0 | 6.0 | 15.0 | 42.75 | 0.80 | 514.6 | |
| | | | | Double, Clear | S | 5.0 | 6.0 | 30.0 | 35.87 | 0.55 | 586.5 | |
| | | | | Double, Clear | SW | 8.0 | 6.0 | 15.0 | 40.16 | 0.44 | 265.0 | |
| | | | | Double, Clear | W | 99.0 | 8.0 | 10.0 | 38.52 | 0.37 | 144.3 | |
| | | | | Double, Clear | S | 12.0 | 8.0 | 30.0 | 35.87 | 0.47 | 509.1 | |
| | | | | Double, Clear | E | 99.0 | 8.0 | 10.0 | 42.06 | 0.36 | 150.1 | |
| | | | | Double, Clear | E | 4.0 | 8.0 | 30.0 | 42.06 | 0.73 | 915.5 | |
| | | | | Double, Clear | S | 15.0 | 8.0 | 30.0 | 35.87 | 0.45 | 488.9 | |
| | | | | Double, Clear | E | 10.0 | 8.0 | 10.0 | 42.06 | 0.46 | 194.8 | |
| | | | | Double, Clear | S | 1.5 | 7.0 | 30.0 | 35.87 | 0.89 | 962.5 | |
| | | | | Double, Clear | S | 1.5 | 7.0 | 30.0 | 35.87 | 0.89 | 962.5 | |
| | | | | Double, Clear | S | 1.5 | 1.5 | 15.0 | 35.87 | 0.52 | 280.1 | |
| | | | | Double, Clear | SE | 2.0 | 1.5 | 4.0 | 42.75 | 0.45 | 76.4 | |
| | | | | Double, Clear | S | 5.0 | 1.5 | 14.0 | 35.87 | 0.43 | 216.9 | |
| | | | | Double, Clear | SW | 10.0 | 1.5 | 4.0 | 40.16 | 0.37 | 59.1 | |
| | | | | Double, Clear | W | 1.5 | 1.5 | 7.0 | 38.52 | 0.53 | 143.0 | |
| | | | | Double, Clear | W | 1.5 | 4.5 | 12.0 | 38.52 | 0.85 | 392.7 | |
| | | | | Double, Clear | W | 1.5 | 2.5 | 14.0 | 38.52 | 0.67 | 361.9 | |
| | | | | Double, Clear | N | 1.5 | 6.5 | 18.0 | 19.20 | 0.95 | 327.4 | |
| | | | | Double, Clear | N | 1.5 | 8.0 | 36.0 | 19.20 | 0.97 | 668.6 | |
| | | | | Double, Clear | N | 1.5 | 1.5 | 6.0 | 19.20 | 0.71 | 81.9 | |
| | | | | Double, Clear | N | 7.5 | 9.0 | 20.0 | 19.20 | 0.74 | 283.2 | |
| | | | | Double, Clear | N | 1.5 | 1.5 | 6.0 | 19.20 | 0.71 | 81.9 | |
| | | | | Double, Clear | NW | 1.5 | 2.5 | 7.0 | 25.97 | 0.74 | 134.7 | |
| | | | | Double, Clear | N | 1.5 | 2.5 | 14.0 | 19.20 | 0.80 | 214.3 | |
| | | | | Double, Clear | NE | 1.5 | 2.5 | 7.0 | 29.56 | 0.71 | 146.5 | |
| | | | | Double, Clear | E | 1.5 | 2.5 | 21.0 | 42.06 | 0.66 | 586.9 | |
| | | | | Double, Clear | E | 1.5 | 1.5 | 7.0 | 42.06 | 0.52 | 152.5 | |
| | | | | Double, Clear | S | 15.0 | 8.0 | 40.0 | 35.87 | 0.45 | 651.9 | |
| | | | | Double, Clear | E | 17.0 | 8.0 | 10.0 | 42.06 | 0.39 | 162.5 | |
| | | | | Double, Clear | S | 1.5 | 5.5 | 20.0 | 35.87 | 0.83 | 597.0 | |
| | | | | Double, Clear | W | 1.5 | 1.5 | 4.0 | 38.52 | 0.53 | 81.7 | |
| | | | | Double, Clear | E | 1.5 | 6.0 | 30.0 | 42.06 | 0.91 | 1151.8 | |
| | | | | **As-Built Total:** | | | | **556.0** | | | **12546.8** | |

| **WALL TYPES** | Area X | BSPM | = Points | Type | R-Value | Area X | SPM | = Points |
|---|---|---|---|---|---|---|---|---|
| Adjacent | 190.0 | 0.70 | 133.0 | Frame, Wood, Exterior | 13.0 | 2798.0 | 1.50 | 4197.0 |
| Exterior | 2798.0 | 1.70 | 4756.6 | Frame, Wood, Adjacent | 13.0 | 190.0 | 0.60 | 114.0 |
| **Base Total:** | **2988.0** | | **4889.6** | **As-Built Total:** | | **2988.0** | | **4311.0** |

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | | AS-BUILT | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| **DOOR TYPES** | Area | X BSPM | = | Points | Type | | Area | X SPM | = | Points |
| Adjacent | 20.0 | 2.40 | | 48.0 | Exterior Insulated | | 190.0 | 4.10 | | 779.0 |
| Exterior | 190.0 | 6.10 | | 1159.0 | Adjacent Insulated | | 20.0 | 1.60 | | 32.0 |
| **Base Total:** | **210.0** | | | **1207.0** | **As-Built Total:** | | **210.0** | | | **811.0** |
| **CEILING TYPES** | Area | X BSPM | = | Points | Type | R-Value | Area | X SPM X SCM | = | Points |
| Under Attic | 3085.0 | 1.73 | | 5337.1 | Under Attic | 30.0 | 3614.0 | 1.73 X 1.00 | | 6252.2 |
| **Base Total:** | **3085.0** | | | **5337.1** | **As-Built Total:** | | **3614.0** | | | **6252.2** |
| **FLOOR TYPES** | Area | X BSPM | = | Points | Type | R-Value | Area | X SPM | = | Points |
| Slab | 276.0(p) | -37.0 | | -10212.0 | Slab-On-Grade Edge Insulation | 0.0 | 276.0(p | -41.20 | | -11371.2 |
| Raised | 0.0 | 0.00 | | 0.0 | | | | | | |
| **Base Total:** | | | | **-10212.0** | **As-Built Total:** | | **276.0** | | | **-11371.2** |
| **INFILTRATION** | Area | X BSPM | = | Points | | | Area | X SPM | = | Points |
| | 3085.0 | 10.21 | | 31497.8 | | | 3085.0 | 10.21 | | 31497.8 |
| **Summer Base Points:** | | | | **43847.7** | **Summer As-Built Points:** | | | | | **44047.7** |
| Total Summer Points | X System Multiplier | = | Cooling Points | | Total Component | X Cap Ratio | X Duct Multiplier (DM x DSM x AHU) | X System Multiplier | X Credit Multiplier | = Cooling Points |
| **43847.7** | **0.4266** | | **18705.4** | | 44047.7<br>**44047.7** | 1.000<br>**1.00** | (1.090 x 1.147 x 1.00)<br>**1.250** | 0.284<br>**0.284** | 1.000<br>**1.000** | 15662.8<br>**15662.8** |

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

## BASE

**GLASS TYPES**

| .18 X | Conditioned Floor Area | X BWPM | = Points |
|---|---|---|---|
| **.18** | **3085.0** | **12.74** | **7074.5** |

| Type/SC | Overhang Ornt | Len | Hgt | Area X | WPM X | WOF | = Points |
|---|---|---|---|---|---|---|---|
| Double, Clear | SE | 2.0 | 6.0 | 15.0 | 14.71 | 1.18 | 259.2 |
| Double, Clear | S | 5.0 | 6.0 | 30.0 | 13.30 | 2.46 | 979.6 |
| Double, Clear | SW | 8.0 | 6.0 | 15.0 | 16.74 | 1.76 | 440.8 |
| Double, Clear | W | 99.0 | 8.0 | 10.0 | 20.73 | 1.24 | 256.6 |
| Double, Clear | S | 12.0 | 8.0 | 30.0 | 13.30 | 3.28 | 1306.6 |
| Double, Clear | E | 99.0 | 8.0 | 10.0 | 18.79 | 1.51 | 283.2 |
| Double, Clear | E | 4.0 | 8.0 | 30.0 | 18.79 | 1.12 | 631.4 |
| Double, Clear | S | 15.0 | 8.0 | 30.0 | 13.30 | 3.48 | 1387.9 |
| Double, Clear | E | 10.0 | 8.0 | 10.0 | 18.79 | 1.35 | 253.3 |
| Double, Clear | S | 1.5 | 7.0 | 30.0 | 13.30 | 1.07 | 428.4 |
| Double, Clear | S | 1.5 | 7.0 | 30.0 | 13.30 | 1.07 | 428.4 |
| Double, Clear | S | 1.5 | 1.5 | 15.0 | 13.30 | 2.73 | 544.8 |
| Double, Clear | SE | 2.0 | 1.5 | 4.0 | 14.71 | 2.19 | 128.9 |
| Double, Clear | S | 5.0 | 1.5 | 14.0 | 13.30 | 3.66 | 681.4 |
| Double, Clear | SW | 10.0 | 1.5 | 4.0 | 16.74 | 2.03 | 136.0 |
| Double, Clear | W | 1.5 | 1.5 | 7.0 | 20.73 | 1.17 | 169.3 |
| Double, Clear | W | 1.5 | 4.5 | 12.0 | 20.73 | 1.04 | 259.4 |
| Double, Clear | W | 1.5 | 2.5 | 14.0 | 20.73 | 1.11 | 321.0 |
| Double, Clear | N | 1.5 | 6.5 | 18.0 | 24.58 | 1.00 | 443.3 |
| Double, Clear | N | 1.5 | 8.0 | 36.0 | 24.58 | 1.00 | 885.6 |
| Double, Clear | N | 1.5 | 1.5 | 6.0 | 24.58 | 1.02 | 150.2 |
| Double, Clear | N | 7.5 | 9.0 | 20.0 | 24.58 | 1.02 | 499.5 |
| Double, Clear | N | 1.5 | 1.5 | 6.0 | 24.58 | 1.02 | 150.2 |
| Double, Clear | NW | 1.5 | 2.5 | 7.0 | 24.30 | 1.02 | 172.9 |
| Double, Clear | N | 1.5 | 2.5 | 14.0 | 24.58 | 1.01 | 348.1 |
| Double, Clear | NE | 1.5 | 2.5 | 7.0 | 23.57 | 1.03 | 169.9 |
| Double, Clear | E | 1.5 | 2.5 | 21.0 | 18.79 | 1.16 | 456.7 |
| Double, Clear | E | 1.5 | 1.5 | 7.0 | 18.79 | 1.29 | 169.1 |
| Double, Clear | S | 15.0 | 8.0 | 40.0 | 13.30 | 3.48 | 1850.6 |
| Double, Clear | E | 17.0 | 8.0 | 10.0 | 18.79 | 1.46 | 274.2 |
| Double, Clear | S | 1.5 | 5.5 | 20.0 | 13.30 | 1.15 | 305.1 |
| Double, Clear | W | 1.5 | 1.5 | 4.0 | 20.73 | 1.17 | 96.7 |
| Double, Clear | E | 1.5 | 6.0 | 30.0 | 18.79 | 1.04 | 583.8 |
| **As-Built Total:** | | | | **556.0** | | | **15451.9** |

| **WALL TYPES** | Area X | BWPM | = Points |
|---|---|---|---|
| Adjacent | 190.0 | 3.60 | 684.0 |
| Exterior | 2798.0 | 3.70 | 10352.6 |
| **Base Total:** | **2988.0** | | **11036.6** |

| Type | R-Value | Area X | WPM | = Points |
|---|---|---|---|---|
| Frame, Wood, Exterior | 13.0 | 2798.0 | 3.40 | 9513.2 |
| Frame, Wood, Adjacent | 13.0 | 190.0 | 3.30 | 627.0 |
| **As-Built Total:** | | **2988.0** | | **10140.2** |

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| **DOOR TYPES** | Area X | BWPM | = Points | Type | | Area X | WPM | = | Points |
| Adjacent | 20.0 | 11.50 | 230.0 | Exterior Insulated | | 190.0 | 8.40 | | 1596.0 |
| Exterior | 190.0 | 12.30 | 2337.0 | Adjacent Insulated | | 20.0 | 8.00 | | 160.0 |
| **Base Total:** | **210.0** | | **2567.0** | **As-Built Total:** | | **210.0** | | | **1756.0** |
| **CEILING TYPES** | Area X | BWPM | = Points | Type | R-Value | Area X | WPM X WCM | = | Points |
| Under Attic | 3085.0 | 2.05 | 6324.3 | Under Attic | 30.0 | 3614.0 | 2.05 X 1.00 | | 7408.7 |
| **Base Total:** | **3085.0** | | **6324.3** | **As-Built Total:** | | **3614.0** | | | **7408.7** |
| **FLOOR TYPES** | Area X | BWPM | = Points | Type | R-Value | Area X | WPM | = | Points |
| Slab | 276.0(p) | 8.9 | 2456.4 | Slab-On-Grade Edge Insulation | 0.0 | 276.0(p | 18.80 | | 5188.8 |
| Raised | 0.0 | 0.00 | 0.0 | | | | | | |
| **Base Total:** | | | **2456.4** | **As-Built Total:** | | **276.0** | | | **5188.8** |
| **INFILTRATION** | Area X | BWPM | = Points | | | Area X | WPM | = | Points |
| | 3085.0 | -0.59 | -1820.1 | | | 3085.0 | -0.59 | | -1820.1 |
| **Winter Base Points:** | | | **27638.6** | **Winter As-Built Points:** | | | | | **38125.5** |

| Total Winter Points | X System Multiplier | = Heating Points | Total Component | X Cap Ratio | X Duct Multiplier (DM x DSM x AHU) | X System Multiplier | X Credit Multiplier | = Heating Points |
|---|---|---|---|---|---|---|---|---|
| | | | 38125.5 | 1.000 | (1.069 x 1.169 x 1.00) | 0.426 | 1.000 | 20308.2 |
| **27638.6** | **0.6274** | **17340.5** | **38125.5** | **1.00** | **1.250** | **0.426** | **1.000** | **20308.2** |

# WATER HEATING & CODE COMPLIANCE STATUS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| **WATER HEATING** | | | | | | | | | | | |
| Number of Bedrooms | X | Multiplier | = Total | Tank Volume | EF | Number of Bedrooms | X Tank Ratio | X Multiplier | X Credit Multiplier | = Total | |
| 3 | | 2746.00 | 8238.0 | 40.0 | 0.89 | 3 | 1.00 | 2715.15 | 1.00 | 8145.4 | |
| | | | | **As-Built Total:** | | | | | | **8145.4** | |

## CODE COMPLIANCE STATUS

| BASE | | | | AS-BUILT | | | |
|---|---|---|---|---|---|---|---|
| Cooling Points + | Heating Points + | Hot Water Points = | Total Points | Cooling Points + | Heating Points + | Hot Water Points = | Total Points |
| **18705** | **17340** | **8238** | **44284** | **15663** | **20308** | **8145** | **44116** |

# PASS

# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

| COMPONENTS | SECTION | REQUIREMENTS FOR EACH PRACTICE | CHECK |
|---|---|---|---|
| Exterior Windows & Doors | 606.1.ABC.1.1 | Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area. | |
| Exterior & Adjacent Walls | 606.1.ABC.1.2.1 | Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate. | |
| Floors | 606.1.ABC.1.2.2 | Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams. | |
| Ceilings | 606.1.ABC.1.2.3 | Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams. | |
| Recessed Lighting Fixtures | 606.1.ABC.1.2.4 | Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested. | |
| Multi-story Houses | 606.1.ABC.1.2.5 | Air barrier on perimeter of floor cavity between floors. | |
| Additional Infiltration reqts | 606.1.ABC.1.3 | Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air. | |

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

| COMPONENTS | SECTION | REQUIREMENTS | CHECK |
|---|---|---|---|
| Water Heaters | 612.1 | Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required. | |
| Swimming Pools & Spas | 612.1 | Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. | |
| Shower heads | 612.1 | Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG. | |
| Air Distribution Systems | 610.1 | All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation. | |
| HVAC Controls | 607.1 | Separate readily accessible manual or automatic thermostat for each system. | |
| Insulation | 604.1, 602.1 | Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11. | |

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE* = 82.4**

**The higher the score, the more efficient the home.**

Griffin Debra & Thomas, Scenic Lake Drive, Lake City, FL,

| | | | |
|---|---|---|---|
| 1. New construction or existing | | New | |
| 2. Single family or multi-family | | Single family | |
| 3. Number of units, if multi-family | | 1 | |
| 4. Number of Bedrooms | | 3 | |
| 5. Is this a worst case? | | Yes | |
| 6. Conditioned floor area (ft²) | | 3085 ft² | |
| 7. Glass area & type | Single Pane | Double Pane | |
| a. Clear glass, default U-factor | 0.0 ft² | 556.0 ft² | |
| b. Default tint, default U-factor | 0.0 ft² | 0.0 ft² | |
| c. Labeled U-factor or SHGC | 0.0 ft² | 0.0 ft² | |
| 8. Floor types | | | |
| a. Slab-On-Grade Edge Insulation | | R=0.0, 276.0(p) ft | |
| b. N/A | | | |
| c. N/A | | | |
| 9. Wall types | | | |
| a. Frame, Wood, Exterior | | R=13.0, 2798.0 ft² | |
| b. Frame, Wood, Adjacent | | R=13.0, 190.0 ft² | |
| c. N/A | | | |
| d. N/A | | | |
| e. N/A | | | |
| 10. Ceiling types | | | |
| a. Under Attic | | R=30.0, 3614.0 ft² | |
| b. N/A | | | |
| c. N/A | | | |
| 11. Ducts | | | |
| a. Sup: Unc. Ret: Unc. AH: Garage | | Sup. R=6.0, 260.0 ft | |
| b. N/A | | | |

| | | |
|---|---|---|
| 12. Cooling systems | | |
| a. Central Unit | Cap: 74.0 kBtu/hr | |
| | SEER: 12.00 | |
| b. N/A | | |
| c. N/A | | |
| 13. Heating systems | | |
| a. Electric Heat Pump | Cap: 74.0 kBtu/hr | |
| | HSPF: 8.00 | |
| b. N/A | | |
| c. N/A | | |
| 14. Hot water systems | | |
| a. Electric Resistance | Cap: 40.0 gallons | |
| | EF: 0.89 | |
| b. N/A | | |
| c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) | | |
| 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) | | |

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-7824.*

23692

# Columbia County Building Department
# Culvert Permit

**Culvert Permit No.**
**000000836**

DATE 10/07/2005 PARCEL ID # 22-3S-16-02268-223

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DR SUITE 101 LAKE CITY FL 32025

OWNER DEBRA GRIFFIN/CINDY THOMAS PHONE 752-4072

ADDRESS 920 NW SCENIC LAKE DRIVE LAKE CITY FL 32055

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY LAKE JEFFREY RD, L ON SCENIC LAKE DR, LOT 23 ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WOODBOROUGH 23

SIGNATURE Katie Reed

**INSTALLATION REQUIREMENTS**

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

**Amount Paid 25.00**

COLUMBIA COUNTY · GATEWAY TO FLORIDA ·

# CERTIFICATE OF OCCUPANCY

## COLUMBIA COUNTY, FLORIDA

## Department of Building and Zoning Inspection

***This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.***

**Parcel Number** 22-3S-16-02268-223 **Building permit No.** 000023692

**Use Classification** SFD,UTILITY **Fire:** 23.68

**Permit Holder** DON REED **Waste:** 49.00

**Owner of Building** DEBRA GRIFFIN/CINDY THOMAS **Total:** 72.68

**Location:** 920 NW SCENIC LAKE DRIVE

**Date:** 06/23/2006

Harry Dicks

**Building Inspector**

**POST IN A CONSPICUOUS PLACE**
***(Business Places Only)***

FORM 600A-2001 EnergyGauge® 3.4

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

## Residential Whole Building Performance Method A

| | | | |
|---|---|---|---|
| Project Name: | **508085ReedDonGriffinDebra&Thomas** | Builder: | |
| Address: | **Scenic Lake Drive** | Permitting Office: | |
| City, State: | **Lake City, FL** | Permit Number: | |
| Owner: | **Griffin Debra & Thomas** | Jurisdiction Number: | |
| Climate Zone: | **North** | | |

| | | | |
|---|---|---|---|
| 1. New construction or existing | | New | |
| 2. Single family or multi-family | | Single family | |
| 3. Number of units, if multi-family | | 1 | |
| 4. Number of Bedrooms | | 3 | |
| 5. Is this a worst case? | | Yes | |
| 6. Conditioned floor area (ft²) | | 3085 ft² | |
| 7. Glass area & type | Single Pane | Double Pane | |
| a. Clear glass, default U-factor | 0.0 ft² | 556.0 ft² | |
| b. Default tint, default U-factor | 0.0 ft² | 0.0 ft² | |
| c. Labeled U-factor or SHGC | 0.0 ft² | 0.0 ft² | |
| 8. Floor types | | | |
| a. Slab-On-Grade Edge Insulation | | R=0.0, 276.0(p) ft | |
| b. N/A | | | |
| c. N/A | | | |
| 9. Wall types | | | |
| a. Frame, Wood, Exterior | | R=13.0, 2798.0 ft² | |
| b. Frame, Wood, Adjacent | | R=13.0, 190.0 ft² | |
| c. N/A | | | |
| d. N/A | | | |
| e. N/A | | | |
| 10. Ceiling types | | | |
| a. Under Attic | | R=30.0, 3614.0 ft² | |
| b. N/A | | | |
| c. N/A | | | |
| 11. Ducts | | | |
| a. Sup: Unc. Ret: Unc. AH: Garage | | Sup. R=6.0, 260.0 ft | |
| b. N/A | | | |

| | | |
|---|---|---|
| 12. Cooling systems | | |
| a. Central Unit | Cap: 74.0 kBtu/hr | |
| | SEER: 12.00 | |
| b. N/A | | |
| c. N/A | | |
| 13. Heating systems | | |
| a. Electric Heat Pump | Cap: 74.0 kBtu/hr | |
| | HSPF: 8.00 | |
| b. N/A | | |
| c. N/A | | |
| 14. Hot water systems | | |
| a. Electric Resistance | Cap: 40.0 gallons | |
| | EF: 0.89 | |
| b. N/A | | |
| c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) | | |
| 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) | | |

Glass/Floor Area: 0.18

Total as-built points: 44116
Total base points: 44284

**PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** Ben Sparks

**DATE:** 8/10/05 Ben Sparks

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER/AGENT:** _____

**DATE:** _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

**BUILDING OFFICIAL:** _____

**DATE:** _____

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| **GLASS TYPES** .18 X Conditioned Floor Area | X BSPM | = Points | | Type/SC | Overhang Ornt | Len | Hgt | Area X | SPM X | SOF | = Points | |
| .18 | 3085.0 | 20.04 | 11128.2 | Double, Clear | SE | 2.0 | 6.0 | 15.0 | 42.75 | 0.80 | 514.6 | |
| | | | | Double, Clear | S | 5.0 | 6.0 | 30.0 | 35.87 | 0.55 | 586.5 | |
| | | | | Double, Clear | SW | 8.0 | 6.0 | 15.0 | 40.16 | 0.44 | 265.0 | |
| | | | | Double, Clear | W | 99.0 | 8.0 | 10.0 | 38.52 | 0.37 | 144.3 | |
| | | | | Double, Clear | S | 12.0 | 8.0 | 30.0 | 35.87 | 0.47 | 509.1 | |
| | | | | Double, Clear | E | 99.0 | 8.0 | 10.0 | 42.06 | 0.36 | 150.1 | |
| | | | | Double, Clear | E | 4.0 | 8.0 | 30.0 | 42.06 | 0.73 | 915.5 | |
| | | | | Double, Clear | S | 15.0 | 8.0 | 30.0 | 35.87 | 0.45 | 488.9 | |
| | | | | Double, Clear | E | 10.0 | 8.0 | 10.0 | 42.06 | 0.46 | 194.8 | |
| | | | | Double, Clear | S | 1.5 | 7.0 | 30.0 | 35.87 | 0.89 | 962.5 | |
| | | | | Double, Clear | S | 1.5 | 7.0 | 30.0 | 35.87 | 0.89 | 962.5 | |
| | | | | Double, Clear | S | 1.5 | 1.5 | 15.0 | 35.87 | 0.52 | 280.1 | |
| | | | | Double, Clear | SE | 2.0 | 1.5 | 4.0 | 42.75 | 0.45 | 76.4 | |
| | | | | Double, Clear | S | 5.0 | 1.5 | 14.0 | 35.87 | 0.43 | 216.9 | |
| | | | | Double, Clear | SW | 10.0 | 1.5 | 4.0 | 40.16 | 0.37 | 59.1 | |
| | | | | Double, Clear | W | 1.5 | 1.5 | 7.0 | 38.52 | 0.53 | 143.0 | |
| | | | | Double, Clear | W | 1.5 | 4.5 | 12.0 | 38.52 | 0.85 | 392.7 | |
| | | | | Double, Clear | W | 1.5 | 2.5 | 14.0 | 38.52 | 0.67 | 361.9 | |
| | | | | Double, Clear | N | 1.5 | 6.5 | 18.0 | 19.20 | 0.95 | 327.4 | |
| | | | | Double, Clear | N | 1.5 | 8.0 | 36.0 | 19.20 | 0.97 | 668.6 | |
| | | | | Double, Clear | N | 1.5 | 1.5 | 6.0 | 19.20 | 0.71 | 81.9 | |
| | | | | Double, Clear | N | 7.5 | 9.0 | 20.0 | 19.20 | 0.74 | 283.2 | |
| | | | | Double, Clear | N | 1.5 | 1.5 | 6.0 | 19.20 | 0.71 | 81.9 | |
| | | | | Double, Clear | NW | 1.5 | 2.5 | 7.0 | 25.97 | 0.74 | 134.7 | |
| | | | | Double, Clear | N | 1.5 | 2.5 | 14.0 | 19.20 | 0.80 | 214.3 | |
| | | | | Double, Clear | NE | 1.5 | 2.5 | 7.0 | 29.56 | 0.71 | 146.5 | |
| | | | | Double, Clear | E | 1.5 | 2.5 | 21.0 | 42.06 | 0.66 | 586.9 | |
| | | | | Double, Clear | E | 1.5 | 1.5 | 7.0 | 42.06 | 0.52 | 152.5 | |
| | | | | Double, Clear | S | 15.0 | 8.0 | 40.0 | 35.87 | 0.45 | 651.9 | |
| | | | | Double, Clear | E | 17.0 | 8.0 | 10.0 | 42.06 | 0.39 | 162.5 | |
| | | | | Double, Clear | S | 1.5 | 5.5 | 20.0 | 35.87 | 0.83 | 597.0 | |
| | | | | Double, Clear | W | 1.5 | 1.5 | 4.0 | 38.52 | 0.53 | 81.7 | |
| | | | | Double, Clear | E | 1.5 | 6.0 | 30.0 | 42.06 | 0.91 | 1151.8 | |
| | | | | **As-Built Total:** | | | | **556.0** | | | **12546.8** | |

| **WALL TYPES** | Area X | BSPM | = Points | Type | R-Value | Area X | SPM | = Points |
|---|---|---|---|---|---|---|---|---|
| Adjacent | 190.0 | 0.70 | 133.0 | Frame, Wood, Exterior | 13.0 | 2798.0 | 1.50 | 4197.0 |
| Exterior | 2798.0 | 1.70 | 4756.6 | Frame, Wood, Adjacent | 13.0 | 190.0 | 0.60 | 114.0 |
| **Base Total:** | **2988.0** | | **4889.6** | **As-Built Total:** | | **2988.0** | | **4311.0** |

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | | AS-BUILT | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|
| **DOOR TYPES** | Area X | BSPM | = Points | | Type | | Area X | SPM | = | Points |
| Adjacent | 20.0 | 2.40 | 48.0 | | Exterior Insulated | | 190.0 | 4.10 | | 779.0 |
| Exterior | 190.0 | 6.10 | 1159.0 | | Adjacent Insulated | | 20.0 | 1.60 | | 32.0 |
| **Base Total:** | **210.0** | | **1207.0** | | **As-Built Total:** | | **210.0** | | | **811.0** |
| **CEILING TYPES** | Area X | BSPM | = Points | | Type | R-Value | Area X | SPM X SCM | = | Points |
| Under Attic | 3085.0 | 1.73 | 5337.1 | | Under Attic | 30.0 | 3614.0 | 1.73 X 1.00 | | 6252.2 |
| **Base Total:** | **3085.0** | | **5337.1** | | **As-Built Total:** | | **3614.0** | | | **6252.2** |
| **FLOOR TYPES** | Area X | BSPM | = Points | | Type | R-Value | Area X | SPM | = | Points |
| Slab | 276.0(p) | -37.0 | -10212.0 | | Slab-On-Grade Edge Insulation | 0.0 | 276.0(p | -41.20 | | -11371.2 |
| Raised | 0.0 | 0.00 | 0.0 | | | | | | | |
| **Base Total:** | | | **-10212.0** | | **As-Built Total:** | | **276.0** | | | **-11371.2** |
| **INFILTRATION** | Area X | BSPM | = Points | | | | Area X | SPM | = | Points |
| | 3085.0 | 10.21 | 31497.8 | | | | 3085.0 | 10.21 | | 31497.8 |
| **Summer Base Points:** | | | **43847.7** | | **Summer As-Built Points:** | | | | | **44047.7** |

| Total Summer Points | X System Multiplier | = Cooling Points | Total Component | X Cap Ratio | X Duct Multiplier (DM x DSM x AHU) | X System Multiplier | X Credit Multiplier | = Cooling Points |
|---|---|---|---|---|---|---|---|---|
| **43847.7** | **0.4266** | **18705.4** | 44047.7<br>**44047.7** | 1.000<br>**1.00** | (1.090 x 1.147 x 1.00)<br>**1.250** | 0.284<br>**0.284** | 1.000<br>**1.000** | 15662.8<br>**15662.8** |

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

## BASE

**GLASS TYPES**

| .18 X | Conditioned Floor Area | X BWPM | = Points |
|---|---|---|---|
| **.18** | **3085.0** | **12.74** | **7074.5** |

| WALL TYPES | Area | X BWPM | = Points |
|---|---|---|---|
| Adjacent | 190.0 | 3.60 | 684.0 |
| Exterior | 2798.0 | 3.70 | 10352.6 |
| **Base Total:** | **2988.0** | | **11036.6** |

## AS-BUILT

| Type/SC | Overhang Ornt | Len | Hgt | Area X | WPM X | WOF = | Points |
|---|---|---|---|---|---|---|---|
| Double, Clear | SE | 2.0 | 6.0 | 15.0 | 14.71 | 1.18 | 259.2 |
| Double, Clear | S | 5.0 | 6.0 | 30.0 | 13.30 | 2.46 | 979.6 |
| Double, Clear | SW | 8.0 | 6.0 | 15.0 | 16.74 | 1.76 | 440.8 |
| Double, Clear | W | 99.0 | 8.0 | 10.0 | 20.73 | 1.24 | 256.6 |
| Double, Clear | S | 12.0 | 8.0 | 30.0 | 13.30 | 3.28 | 1306.6 |
| Double, Clear | E | 99.0 | 8.0 | 10.0 | 18.79 | 1.51 | 283.2 |
| Double, Clear | E | 4.0 | 8.0 | 30.0 | 18.79 | 1.12 | 631.4 |
| Double, Clear | S | 15.0 | 8.0 | 30.0 | 13.30 | 3.48 | 1387.9 |
| Double, Clear | E | 10.0 | 8.0 | 10.0 | 18.79 | 1.35 | 253.3 |
| Double, Clear | S | 1.5 | 7.0 | 30.0 | 13.30 | 1.07 | 428.4 |
| Double, Clear | S | 1.5 | 7.0 | 30.0 | 13.30 | 1.07 | 428.4 |
| Double, Clear | S | 1.5 | 1.5 | 15.0 | 13.30 | 2.73 | 544.8 |
| Double, Clear | SE | 2.0 | 1.5 | 4.0 | 14.71 | 2.19 | 128.9 |
| Double, Clear | S | 5.0 | 1.5 | 14.0 | 13.30 | 3.66 | 681.4 |
| Double, Clear | SW | 10.0 | 1.5 | 4.0 | 16.74 | 2.03 | 136.0 |
| Double, Clear | W | 1.5 | 1.5 | 7.0 | 20.73 | 1.17 | 169.3 |
| Double, Clear | W | 1.5 | 4.5 | 12.0 | 20.73 | 1.04 | 259.4 |
| Double, Clear | W | 1.5 | 2.5 | 14.0 | 20.73 | 1.11 | 321.0 |
| Double, Clear | N | 1.5 | 6.5 | 18.0 | 24.58 | 1.00 | 443.3 |
| Double, Clear | N | 1.5 | 8.0 | 36.0 | 24.58 | 1.00 | 885.6 |
| Double, Clear | N | 1.5 | 1.5 | 6.0 | 24.58 | 1.02 | 150.2 |
| Double, Clear | N | 7.5 | 9.0 | 20.0 | 24.58 | 1.02 | 499.5 |
| Double, Clear | N | 1.5 | 1.5 | 6.0 | 24.58 | 1.02 | 150.2 |
| Double, Clear | NW | 1.5 | 2.5 | 7.0 | 24.30 | 1.02 | 172.9 |
| Double, Clear | N | 1.5 | 2.5 | 14.0 | 24.58 | 1.01 | 348.1 |
| Double, Clear | NE | 1.5 | 2.5 | 7.0 | 23.57 | 1.03 | 169.9 |
| Double, Clear | E | 1.5 | 2.5 | 21.0 | 18.79 | 1.16 | 456.7 |
| Double, Clear | E | 1.5 | 1.5 | 7.0 | 18.79 | 1.29 | 169.1 |
| Double, Clear | S | 15.0 | 8.0 | 40.0 | 13.30 | 3.48 | 1850.6 |
| Double, Clear | E | 17.0 | 8.0 | 10.0 | 18.79 | 1.46 | 274.2 |
| Double, Clear | S | 1.5 | 5.5 | 20.0 | 13.30 | 1.15 | 305.1 |
| Double, Clear | W | 1.5 | 1.5 | 4.0 | 20.73 | 1.17 | 96.7 |
| Double, Clear | E | 1.5 | 6.0 | 30.0 | 18.79 | 1.04 | 583.8 |
| **As-Built Total:** | | | | **556.0** | | | **15451.9** |

| Type | R-Value | Area X | WPM = | Points |
|---|---|---|---|---|
| Frame, Wood, Exterior | 13.0 | 2798.0 | 3.40 | 9513.2 |
| Frame, Wood, Adjacent | 13.0 | 190.0 | 3.30 | 627.0 |
| **As-Built Total:** | | **2988.0** | | **10140.2** |

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| **DOOR TYPES** | Area X | BWPM = | Points | Type | | Area X | WPM = | | Points |
| Adjacent | 20.0 | 11.50 | 230.0 | Exterior Insulated | | 190.0 | 8.40 | | 1596.0 |
| Exterior | 190.0 | 12.30 | 2337.0 | Adjacent Insulated | | 20.0 | 8.00 | | 160.0 |
| **Base Total:** | **210.0** | | **2567.0** | **As-Built Total:** | | **210.0** | | | **1756.0** |
| **CEILING TYPES** | Area X | BWPM = | Points | Type | R-Value | Area X | WPM X | WCM = | Points |
| Under Attic | 3085.0 | 2.05 | 6324.3 | Under Attic | 30.0 | 3614.0 | 2.05 X | 1.00 | 7408.7 |
| **Base Total:** | **3085.0** | | **6324.3** | **As-Built Total:** | | **3614.0** | | | **7408.7** |
| **FLOOR TYPES** | Area X | BWPM = | Points | Type | R-Value | Area X | WPM = | | Points |
| Slab | 276.0(p) | 8.9 | 2456.4 | Slab-On-Grade Edge Insulation | 0.0 | 276.0(p | 18.80 | | 5188.8 |
| Raised | 0.0 | 0.00 | 0.0 | | | | | | |
| **Base Total:** | | | **2456.4** | **As-Built Total:** | | **276.0** | | | **5188.8** |
| **INFILTRATION** | Area X | BWPM = | Points | | | Area X | WPM = | | Points |
| | 3085.0 | -0.59 | -1820.1 | | | 3085.0 | -0.59 | | -1820.1 |
| **Winter Base Points:** | | | **27638.6** | **Winter As-Built Points:** | | | | | **38125.5** |
| Total Winter Points | X System Multiplier | = | Heating Points | Total Component | X Cap Ratio | X Duct Multiplier (DM x DSM x AHU) | X System Multiplier | X Credit Multiplier | = Heating Points |
| **27638.6** | **0.6274** | | **17340.5** | 38125.5<br>**38125.5** | 1.000<br>**1.00** | (1.069 x 1.169 x 1.00)<br>**1.250** | 0.426<br>**0.426** | 1.000<br>**1.000** | 20308.2<br>**20308.2** |

# WATER HEATING & CODE COMPLIANCE STATUS

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

| BASE | | | | AS-BUILT | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| **WATER HEATING** | | | | | | | | | | | |
| Number of Bedrooms | X Multiplier | = Total | | Tank Volume | EF | Number of Bedrooms | X Tank Ratio | X Multiplier | X Credit Multiplier | = Total | |
| 3 | 2746.00 | 8238.0 | | 40.0 | 0.89 | 3 | 1.00 | 2715.15 | 1.00 | 8145.4 | |
| | | | | **As-Built Total:** | | | | | | | **8145.4** |

## CODE COMPLIANCE STATUS

| BASE | | | | AS-BUILT | | | |
|---|---|---|---|---|---|---|---|
| Cooling Points + | Heating Points + | Hot Water Points = | Total Points | Cooling Points + | Heating Points + | Hot Water Points = | Total Points |
| **18705** | **17340** | **8238** | **44284** | **15663** | **20308** | **8145** | **44116** |

# PASS

# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: **Scenic Lake Drive, Lake City, FL,** PERMIT #:

### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

| COMPONENTS | SECTION | REQUIREMENTS FOR EACH PRACTICE | CHECK |
|---|---|---|---|
| Exterior Windows & Doors | 606.1.ABC.1.1 | Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area. | |
| Exterior & Adjacent Walls | 606.1.ABC.1.2.1 | Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate. | |
| Floors | 606.1.ABC.1.2.2 | Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams. | |
| Ceilings | 606.1.ABC.1.2.3 | Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams. | |
| Recessed Lighting Fixtures | 606.1.ABC.1.2.4 | Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested. | |
| Multi-story Houses | 606.1.ABC.1.2.5 | Air barrier on perimeter of floor cavity between floors. | |
| Additional Infiltration reqts | 606.1.ABC.1.3 | Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air. | |

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

| COMPONENTS | SECTION | REQUIREMENTS | CHECK |
|---|---|---|---|
| Water Heaters | 612.1 | Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required. | |
| Swimming Pools & Spas | 612.1 | Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. | |
| Shower heads | 612.1 | Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG. | |
| Air Distribution Systems | 610.1 | All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation. | |
| HVAC Controls | 607.1 | Separate readily accessible manual or automatic thermostat for each system. | |
| Insulation | 604.1, 602.1 | Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11. | |

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE* = 82.4**

**The higher the score, the more efficient the home.**

Griffin Debra & Thomas, Scenic Lake Drive, Lake City, FL,

| | | | |
|---|---|---|---|
| 1. New construction or existing | | New | |
| 2. Single family or multi-family | | Single family | |
| 3. Number of units, if multi-family | | 1 | |
| 4. Number of Bedrooms | | 3 | |
| 5. Is this a worst case? | | Yes | |
| 6. Conditioned floor area (ft²) | | 3085 ft² | |
| 7. Glass area & type | Single Pane | Double Pane | |
| a. Clear glass, default U-factor | 0.0 ft² | 556.0 ft² | |
| b. Default tint, default U-factor | 0.0 ft² | 0.0 ft² | |
| c. Labeled U-factor or SHGC | 0.0 ft² | 0.0 ft² | |
| 8. Floor types | | | |
| a. Slab-On-Grade Edge Insulation | | R=0.0, 276.0(p) ft | |
| b. N/A | | | |
| c. N/A | | | |
| 9. Wall types | | | |
| a. Frame, Wood, Exterior | | R=13.0, 2798.0 ft² | |
| b. Frame, Wood, Adjacent | | R=13.0, 190.0 ft² | |
| c. N/A | | | |
| d. N/A | | | |
| e. N/A | | | |
| 10. Ceiling types | | | |
| a. Under Attic | | R=30.0, 3614.0 ft² | |
| b. N/A | | | |
| c. N/A | | | |
| 11. Ducts | | | |
| a. Sup: Unc. Ret: Unc. AH: Garage | | Sup. R=6.0, 260.0 ft | |
| b. N/A | | | |

| | | |
|---|---|---|
| 12. Cooling systems | | |
| a. Central Unit | Cap: 74.0 kBtu/hr | |
| | SEER: 12.00 | |
| b. N/A | | |
| c. N/A | | |
| 13. Heating systems | | |
| a. Electric Heat Pump | Cap: 74.0 kBtu/hr | |
| | HSPF: 8.00 | |
| b. N/A | | |
| c. N/A | | |
| 14. Hot water systems | | |
| a. Electric Resistance | Cap: 40.0 gallons | |
| | EF: 0.89 | |
| b. N/A | | |
| c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) | | |
| 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) | | |

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-7824.*

# Residential System Sizing Calculation

## Summary

Griffin Debra & Thomas
Scenic Lake Drive
Lake City, FL

Project Title:
508085ReedDonGriffinDebra&Thomas

Class 3 Rating
Registration No. 0
Climate: North

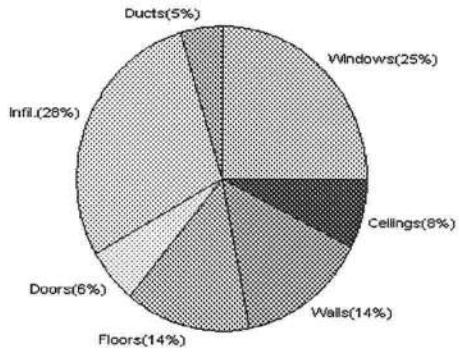
8/10/2005

| Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.) | | | | | |
|---|---|---|---|---|---|
| Winter design temperature | 31 | F | Summer design temperature | 93 | F |
| Winter setpoint | 70 | F | Summer setpoint | 75 | F |
| Winter temperature difference | 39 | F | Summer temperature difference | 18 | F |
| **Total heating load calculation** | **62459** | **Btuh** | **Total cooling load calculation** | **58496** | **Btuh** |
| Submitted heating capacity | % of calc | Btuh | Submitted cooling capacity | % of calc | Btuh |
| Total (Electric Heat Pump) | 118.5 | 74000 | Sensible (SHR = 0.5) | 82.9 | 37000 |
| Heat Pump + Auxiliary(0.0kW) | 118.5 | 74000 | Latent | 266.4 | 37000 |
| | | | Total (Electric Heat Pump) | 126.5 | 74000 |

## WINTER CALCULATIONS

Winter Heating Load (for 3085 sqft)

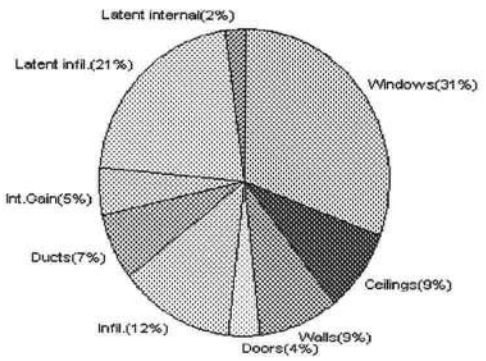
| Load component | | | Load | |
|---|---|---|---|---|
| Window total | 556 | sqft | 15735 | Btuh |
| Wall total | 2988 | sqft | 8978 | Btuh |
| Door total | 210 | sqft | 3671 | Btuh |
| Ceiling total | 3614 | sqft | 4698 | Btuh |
| Floor total | 276 | ft | 8722 | Btuh |
| Infiltration | 412 | cfm | 17681 | Btuh |
| **Subtotal** | | | **59485** | **Btuh** |
| Duct loss | | | 2974 | Btuh |
| **TOTAL HEAT LOSS** | | | **62459** | **Btuh** |




## SUMMER CALCULATIONS

Summer Cooling Load (for 3085 sqft)

| Load component | | | Load | |
|---|---|---|---|---|
| Window total | 556 | sqft | 18086 | Btuh |
| Wall total | 2988 | sqft | 5066 | Btuh |
| Door total | 210 | sqft | 2129 | Btuh |
| Ceiling total | 3614 | sqft | 5132 | Btuh |
| Floor total | | | 0 | Btuh |
| Infiltration | 361 | cfm | 7141 | Btuh |
| Internal gain | | | 3000 | Btuh |
| **Subtotal(sensible)** | | | **40554** | **Btuh** |
| Duct gain | | | 4055 | Btuh |
| **Total sensible gain** | | | **44609** | **Btuh** |
| Latent gain(infiltration) | | | 12507 | Btuh |
| Latent gain(internal) | | | 1380 | Btuh |
| **Total latent gain** | | | **13887** | **Btuh** |
| **TOTAL HEAT GAIN** | | | **58496** | **Btuh** |




EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: Glenn Lowe
DATE: 8/10/05

# System Sizing Calculations - Winter

## Residential Load - Component Details

Griffin Debra & Thomas
Scenic Lake Drive
Lake City, FL

Project Title:
508085ReedDonGriffinDebra&Thomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

8/10/2005

| Window | Panes/SHGC/Frame/U | Orientation | Area X | HTM= | Load |
|---|---|---|---|---|---|
| 1 | 2, Clear, Metal, DEF | NW | 15.0 | 28.3 | 424 Btuh |
| 2 | 2, Clear, Metal, DEF | N | 30.0 | 28.3 | 849 Btuh |
| 3 | 2, Clear, Metal, DEF | NE | 15.0 | 28.3 | 424 Btuh |
| 4 | 2, Clear, Metal, DEF | E | 10.0 | 28.3 | 283 Btuh |
| 5 | 2, Clear, Metal, DEF | N | 30.0 | 28.3 | 849 Btuh |
| 6 | 2, Clear, Metal, DEF | W | 10.0 | 28.3 | 283 Btuh |
| 7 | 2, Clear, Metal, DEF | W | 30.0 | 28.3 | 849 Btuh |
| 8 | 2, Clear, Metal, DEF | N | 30.0 | 28.3 | 849 Btuh |
| 9 | 2, Clear, Metal, DEF | W | 10.0 | 28.3 | 283 Btuh |
| 10 | 2, Clear, Metal, DEF | N | 30.0 | 28.3 | 849 Btuh |
| 11 | 2, Clear, Metal, DEF | N | 30.0 | 28.3 | 849 Btuh |
| 12 | 2, Clear, Metal, DEF | N | 15.0 | 28.3 | 424 Btuh |
| 13 | 2, Clear, Metal, DEF | NW | 4.0 | 28.3 | 113 Btuh |
| 14 | 2, Clear, Metal, DEF | N | 14.0 | 28.3 | 396 Btuh |
| 15 | 2, Clear, Metal, DEF | NE | 4.0 | 28.3 | 113 Btuh |
| 16 | 2, Clear, Metal, DEF | E | 7.0 | 28.3 | 198 Btuh |
| 17 | 2, Clear, Metal, DEF | E | 12.0 | 28.3 | 340 Btuh |
| 18 | 2, Clear, Metal, DEF | E | 14.0 | 28.3 | 396 Btuh |
| 19 | 2, Clear, Metal, DEF | S | 18.0 | 28.3 | 509 Btuh |
| 20 | 2, Clear, Metal, DEF | S | 36.0 | 28.3 | 1019 Btuh |
| 21 | 2, Clear, Metal, DEF | S | 6.0 | 28.3 | 170 Btuh |
| 22 | 2, Clear, Metal, DEF | S | 20.0 | 28.3 | 566 Btuh |
| 23 | 2, Clear, Metal, DEF | S | 6.0 | 28.3 | 170 Btuh |
| 24 | 2, Clear, Metal, DEF | SE | 7.0 | 28.3 | 198 Btuh |
| 25 | 2, Clear, Metal, DEF | S | 14.0 | 28.3 | 396 Btuh |
| 26 | 2, Clear, Metal, DEF | SW | 7.0 | 28.3 | 198 Btuh |
| 27 | 2, Clear, Metal, DEF | W | 21.0 | 28.3 | 594 Btuh |
| 28 | 2, Clear, Metal, DEF | W | 7.0 | 28.3 | 198 Btuh |
| 29 | 2, Clear, Metal, DEF | N | 40.0 | 28.3 | 1132 Btuh |
| 30 | 2, Clear, Metal, DEF | W | 10.0 | 28.3 | 283 Btuh |
| 31 | 2, Clear, Metal, DEF | N | 20.0 | 28.3 | 566 Btuh |
| 32 | 2, Clear, Metal, DEF | E | 4.0 | 28.3 | 113 Btuh |
| 33 | 2, Clear, Metal, DEF | W | 30.0 | 28.3 | 849 Btuh |
| | Window Total | | 556 | | 15735 Btuh |

| Walls | Type | R-Value | Area X | HTM= | Load |
|---|---|---|---|---|---|
| 1 | Frame - Exterior | 13.0 | 2798 | 3.1 | 8674 Btuh |
| 2 | Frame - Adjacent | 13.0 | 190 | 1.6 | 304 Btuh |
| | Wall Total | | 2988 | | 8978 Btuh |

| Doors | Type | Area X | HTM= | Load |
|---|---|---|---|---|
| 1 | Insulated - Exter | 190 | 18.3 | 3483 Btuh |
| 2 | Insulated - Adjac | 20 | 9.4 | 188 Btuh |
| | Door Total | 210 | | 3671Btuh |

# Manual J Winter Calculations

## Residential Load - Component Details (continued)

Griffin Debra & Thomas
Scenic Lake Drive
Lake City, FL

Project Title:
508085ReedDonGriffinDebra&Thomas

Class 3 Rating
Registration No. 0
Climate: North

8/10/2005

| | | | | | |
|---|---|---|---|---|---|
| **Ceilings** | Type | R-Value | Area X | HTM= | Load |
| 1 | Under Attic | 30.0 | 3614 | 1.3 | 4698 Btuh |
| | Ceiling Total | | 3614 | | 4698Btuh |
| **Floors** | Type | R-Value | Size X | HTM= | Load |
| 1 | Slab-On-Grade Edge Insul | 0 | 276.0 ft(p) | 31.6 | 8722 Btuh |
| | Floor Total | | 276 | | 8722 Btuh |
| **Infiltration** | Type | ACH X | Building Volume | CFM= | Load |
| | Natural | 0.80 | 30850(sqft) | 412 | 17681 Btuh |
| | Mechanical | | | 0 | 0 Btuh |
| | Infiltration Total | | | 412 | 17681 Btuh |

| | | |
|---|---|---|
| **Totals for Heating** | **Subtotal** | **59485 Btuh** |
| | **Duct Loss(using duct multiplier of 0.05)** | **2974 Btuh** |
| | **Total Btuh Loss** | **62459 Btuh** |

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )

# System Sizing Calculations - Summer

## Residential Load - Component Details

Griffin Debra & Thomas
Scenic Lake Drive
Lake City, FL

Project Title:
508085ReedDonGriffinDebra&Thomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults)  Summer Temperature Difference: 18.0 F  8/10/2005

| Window | Type Panes/SHGC/U/InSh/ExSh | Ornt | Overhang Len | Hgt | Window Area(sqft) Gross | Shaded | Unshaded | HTM Shaded | Unshaded | Load |
|---|---|---|---|---|---|---|---|---|---|---|
| 1 | 2, Clear, DEF, N, N | NW | 2 | 6 | 15.0 | 0.0 | 15.0 | 22 | 50 | 750 Btuh |
| 2 | 2, Clear, DEF, N, N | N | 5 | 6 | 30.0 | 0.0 | 30.0 | 22 | 22 | 660 Btuh |
| 3 | 2, Clear, DEF, N, N | NE | 8 | 6 | 15.0 | 0.0 | 15.0 | 22 | 50 | 750 Btuh |
| 4 | 2, Clear, DEF, N, N | E | 99 | 8 | 10.0 | 10.0 | 0.0 | 22 | 72 | 220 Btuh |
| 5 | 2, Clear, DEF, N, N | N | 12 | 8 | 30.0 | 0.0 | 30.0 | 22 | 22 | 660 Btuh |
| 6 | 2, Clear, DEF, N, N | W | 99 | 8 | 10.0 | 10.0 | 0.0 | 22 | 72 | 220 Btuh |
| 7 | 2, Clear, DEF, N, N | W | 4 | 8 | 30.0 | 4.4 | 25.6 | 22 | 72 | 1942 Btuh |
| 8 | 2, Clear, DEF, N, N | N | 15 | 8 | 30.0 | 0.0 | 30.0 | 22 | 22 | 660 Btuh |
| 9 | 2, Clear, DEF, N, N | W | 10 | 8 | 10.0 | 10.0 | 0.0 | 22 | 72 | 220 Btuh |
| 10 | 2, Clear, DEF, N, N | N | 1.5 | 7 | 30.0 | 0.0 | 30.0 | 22 | 22 | 660 Btuh |
| 11 | 2, Clear, DEF, N, N | N | 1.5 | 7 | 30.0 | 0.0 | 30.0 | 22 | 22 | 660 Btuh |
| 12 | 2, Clear, DEF, N, N | N | 1.5 | 1.5 | 15.0 | 0.0 | 15.0 | 22 | 22 | 330 Btuh |
| 13 | 2, Clear, DEF, N, N | NW | 2 | 1.5 | 4.0 | 0.0 | 4.0 | 22 | 50 | 200 Btuh |
| 14 | 2, Clear, DEF, N, N | N | 5 | 1.5 | 14.0 | 0.0 | 14.0 | 22 | 22 | 308 Btuh |
| 15 | 2, Clear, DEF, N, N | NE | 10 | 1.5 | 4.0 | 0.0 | 4.0 | 22 | 50 | 200 Btuh |
| 16 | 2, Clear, DEF, N, N | E | 1.5 | 1.5 | 7.0 | 5.8 | 1.2 | 22 | 72 | 214 Btuh |
| 17 | 2, Clear, DEF, N, N | E | 1.5 | 4.5 | 12.0 | 0.7 | 11.3 | 22 | 72 | 828 Btuh |
| 18 | 2, Clear, DEF, N, N | E | 1.5 | 2.5 | 14.0 | 2.6 | 11.4 | 22 | 72 | 878 Btuh |
| 19 | 2, Clear, DEF, N, N | S | 1.5 | 6.5 | 18.0 | 18.0 | 0.0 | 22 | 37 | 396 Btuh |
| 20 | 2, Clear, DEF, N, N | S | 1.5 | 8 | 36.0 | 36.0 | 0.0 | 22 | 37 | 792 Btuh |
| 21 | 2, Clear, DEF, N, N | S | 1.5 | 1.5 | 6.0 | 6.0 | 0.0 | 22 | 37 | 132 Btuh |
| 22 | 2, Clear, DEF, N, N | S | 7.5 | 9 | 20.0 | 20.0 | 0.0 | 22 | 37 | 440 Btuh |
| 23 | 2, Clear, DEF, N, N | S | 1.5 | 1.5 | 6.0 | 6.0 | 0.0 | 22 | 37 | 132 Btuh |
| 24 | 2, Clear, DEF, N, N | SE | 1.5 | 2.5 | 7.0 | 7.0 | 0.0 | 22 | 62 | 154 Btuh |
| 25 | 2, Clear, DEF, N, N | S | 1.5 | 2.5 | 14.0 | 14.0 | 0.0 | 22 | 37 | 308 Btuh |
| 26 | 2, Clear, DEF, N, N | SW | 1.5 | 2.5 | 7.0 | 7.0 | 0.0 | 22 | 62 | 154 Btuh |
| 27 | 2, Clear, DEF, N, N | W | 1.5 | 2.5 | 21.0 | 2.6 | 18.4 | 22 | 72 | 1382 Btuh |
| 28 | 2, Clear, DEF, N, N | W | 1.5 | 1.5 | 7.0 | 5.8 | 1.2 | 22 | 72 | 214 Btuh |
| 29 | 2, Clear, DEF, N, N | N | 15 | 8 | 40.0 | 0.0 | 40.0 | 22 | 22 | 880 Btuh |
| 30 | 2, Clear, DEF, N, N | W | 17 | 8 | 10.0 | 10.0 | 0.0 | 22 | 72 | 220 Btuh |
| 31 | 2, Clear, DEF, N, N | N | 1.5 | 5.5 | 20.0 | 0.0 | 20.0 | 22 | 22 | 440 Btuh |
| 32 | 2, Clear, DEF, N, N | E | 1.5 | 1.5 | 4.0 | 3.3 | 0.7 | 22 | 72 | 122 Btuh |
| 33 | 2, Clear, DEF, N, N | W | 1.5 | 6 | 30.0 | 4.0 | 26.0 | 22 | 72 | 1962 Btuh |
| | Window Total | | | | 556 | | | | | 18086 Btuh |

| Walls | Type | R-Value | Area | HTM | Load |
|---|---|---|---|---|---|
| 1 | Frame - Exterior | 13.0 | 2798.0 | 1.7 | 4869 Btuh |
| 2 | Frame - Adjacent | 13.0 | 190.0 | 1.0 | 198 Btuh |
| | Wall Total | | 2988.0 | | 5066 Btuh |

| Doors | Type | Area | HTM | Load |
|---|---|---|---|---|
| 1 | Insulated - Exter | 190.0 | 10.1 | 1927 Btuh |
| 2 | Insulated - Adjac | 20.0 | 10.1 | 203 Btuh |
| | Door Total | 210.0 | | 2129 Btuh |

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

Griffin Debra & Thomas
Scenic Lake Drive
Lake City, FL

Project Title:
508085ReedDonGriffinDebra&Thomas

Class 3 Rating
Registration No. 0
Climate: North

8/10/2005

| | | | | | |
|---|---|---|---|---|---|
| **Ceilings** | Type/Color | R-Value | Area | HTM | Load |
| 1 | Under Attic/Dark | 30.0 | 3614.0 | 1.4 | 5132 Btuh |
| | Ceiling Total | | 3614.0 | | 5132 Btuh |
| **Floors** | Type | R-Value | Size | HTM | Load |
| 1 | Slab-On-Grade Edge Insulation | 0.0 | 276.0 ft(p) | 0.0 | 0 Btuh |
| | Floor Total | | 276.0 | | 0 Btuh |
| **Infiltration** | Type | ACH | Volume | CFM= | Load |
| | Natural | 0.70 | 30850 | 360.6 | 7141 Btuh |
| | Mechanical | | | 0 | 0 Btuh |
| | Infiltration Total | | | 361 | 7141 Btuh |

| | | | | | | | |
|---|---|---|---|---|---|---|---|
| **Internal gain** | | Occupants | | Btuh/occupant | | Appliance | Load |
| | | 6 | X | 300 | + | 1200 | 3000 Btuh |

| | | |
|---|---|---|
| **Totals for Cooling** | **Subtotal** | **40554 Btuh** |
| | **Duct gain(using duct multiplier of 0.10)** | **4055 Btuh** |
| | **Total sensible gain** | **44609 Btuh** |
| | **Latent infiltration gain (for 51 gr. humidity difference)** | **12507 Btuh** |
| | **Latent occupant gain (6 people @ 230 Btuh per person)** | **1380 Btuh** |
| | **Latent other gain** | **0 Btuh** |
| | **TOTAL GAIN** | **58496 Btuh** |

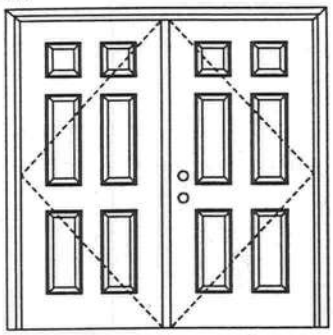
Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

# XX
Opaque Outswing Unit

COP-WL-MA0122-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

**Design Pressure**

+55.0/-55.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

**Note:**
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

## MINIMUM ASSEMBLY DETAIL:

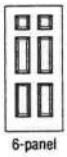
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02.

## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

## APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

Masonite®

**XX**
Opaque Outswing Unit

COP-WL-MA0122-02

# FIBERGLASS DOORS

## CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH®

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors



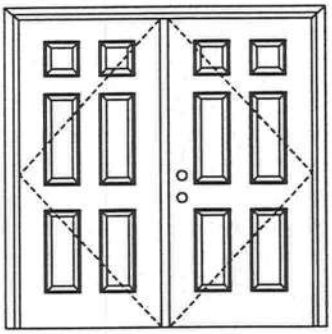
Masonite®

# XX
Opaque Inswing Unit

COP-WL-MA0102-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

**Design Pressure**
+55.0/-55.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

**Note:**
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02.

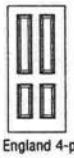
## MINIMUM INSTALLATION DETAIL:

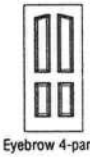
Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

## APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

**XX**
Opaque Inswing Unit

COP-WL-MA0102-02

# FIBERGLASS DOORS

## CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



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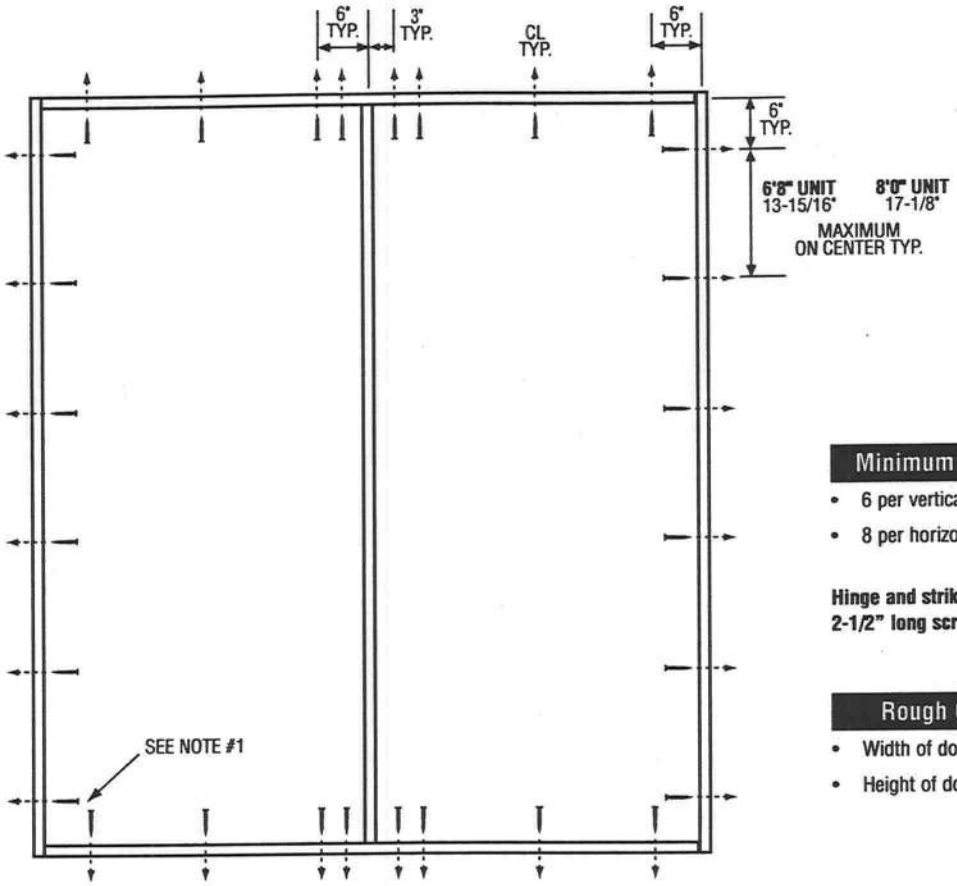
**XX**
Unit

MID-WL-MA0002-02

# DOUBLE DOOR




## Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

**Hinge and strike plates require two 2-1/2" long screws per location.**

## Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

## Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

## Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.



March 10, 2003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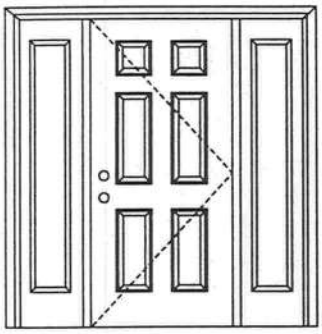
# OXO
Opaque Outswing Unit

COP-WL-MA0124-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

**Note:**
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

**Design Pressure**

+55.0/-55.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0014-02 or MAD-WL-MA0017-02 and MAD-WL-MA0041-02.

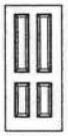
## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

## APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



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ARTEK™ Non-Textured Fiberglass Entry Doors

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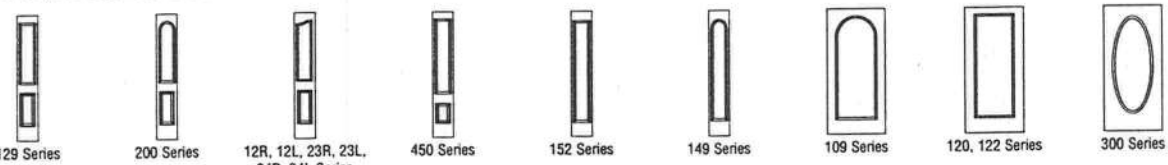
Masonite®

# OXO
Opaque Outswing Unit

COP-WL-MA0124-02

# FIBERGLASS DOORS

## APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

## CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



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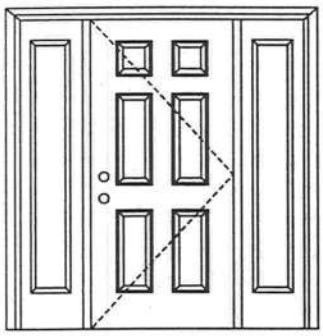
# OXO
Opaque Inswing Unit

COP-WL-MA0104-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

**Design Pressure**
+55.0/-55.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**
Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

**Note:**
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

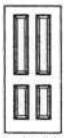
## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

## APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



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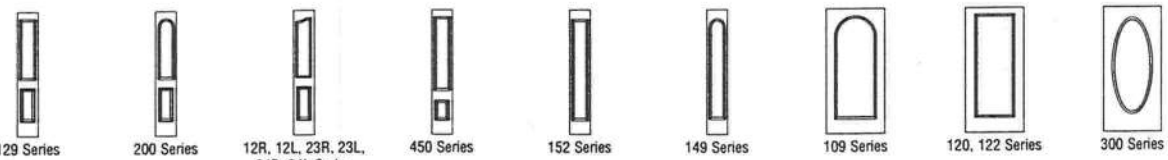
# OXO
Opaque Inswing Unit

COP-WL-MA0104-02

# FIBERGLASS DOORS

## APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

## CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



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March 10, 2003


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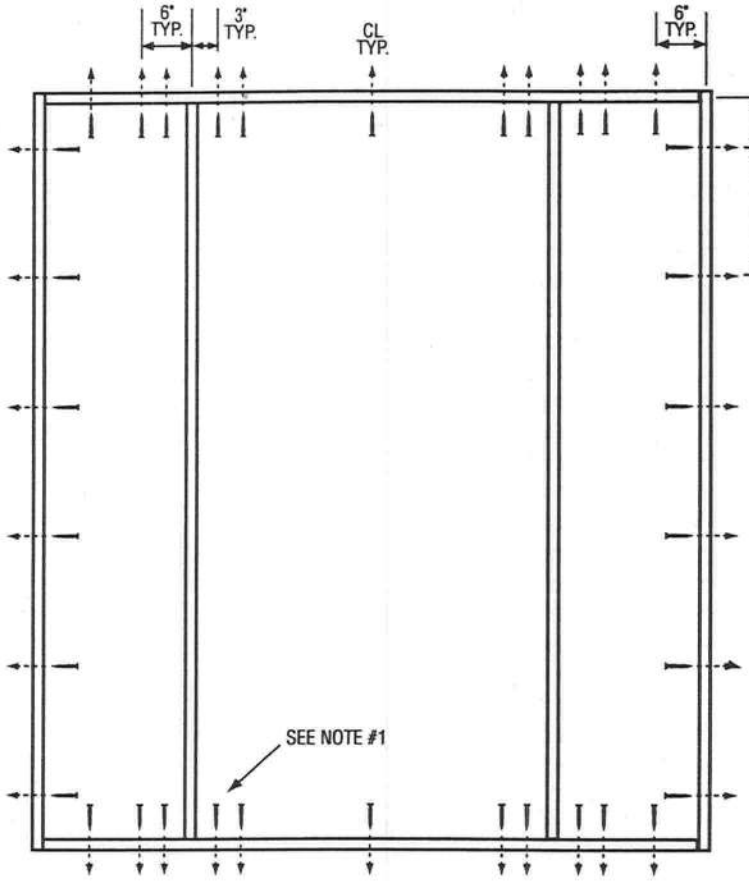
**OXO**
Unit

MID-WL-MA0004-02

# SINGLE DOOR WITH 2 SIDELITES




**Minimum Fastener Count**

- 6 per vertical framing member
- 11 per horizontal framing member

**Hinge and strike plates require two 2-1/2" long screws per location.**

**Rough Opening (RO)**

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

## Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0249*, 0269*, 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

## Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.



Mrach 10, 2003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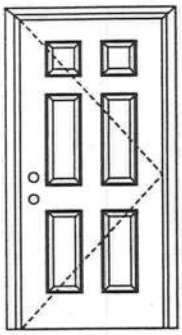
**X**
Opaque Inswing Unit

COP-WL-MA0101-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



Single Door
Maximum unit size = 3'0" x 6'8"

**Note:**
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

**Design Pressure**
+76.0/-76.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

## APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



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ARTEK™ Non-Textured Fiberglass Entry Doors

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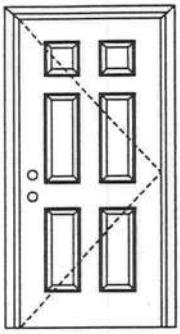
Masonite®

**X**
Opaque Outswing Unit

COP-WL-MA0121-02

# FIBERGLASS DOORS

## APPROVED ARRANGEMENT:



**Note:**
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

**Design Pressure**
+76.0/-76.0
limited water unless special threshold design is used.

**Large Missile Impact Resistance**
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

## MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

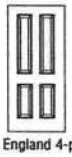
## MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

## APPROVED DOOR STYLES:

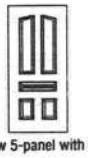

Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll



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Masonite®

**X**
Opaque Outswing Unit

COP-WL-MA0121-02

# FIBERGLASS DOORS

## CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH®

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003


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**X**
Opaque Inswing Unit

COP-WL-MA0101-02

# FIBERGLASS DOORS

## CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

## PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

**COMPANY NAME**
CITY, STATE

**To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).**

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.



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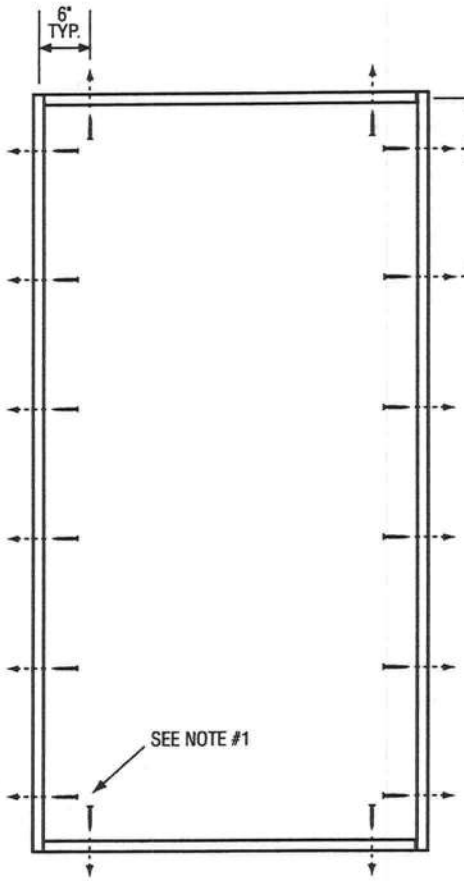
Masonite®

**X**
Unit

MID-WL-MA0001-02

# SINGLE DOOR




## Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

**Hinge and strike plates require two 2-1/2" long screws per location.**

## Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

## Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

## Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.



March 10, 2003


Masonite®

CAPITOL WINDOWS AND DOORS

- Series 3540 Single Hung and Fixed Windows
- Series 8540 Single Hung and Fixed Windows

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

# INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" VINYL FIN WINDOWS

1. Storage: Do not lay windows flat, lean multiple units against poles, or store in the sun before installing.

2. Handle units one at a time in the closed and locked position. Place a continuous bead of adhesive caulk such as silicone or urethane on the back side of "nail fins" (mounting flanges) before placing in opening.

3. Place shims under corners of sill. In the closed and locked position, set unit into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Starting at the center of the longest frame member, place #8 sheet metal or wood screws (with a minimum of 1" penetration into the framing) in every other pre-punched slot which are on 4" to 5" centers (max. spacing 10"). Make sure that screws are driven in straight in order to avoid twisting or bowing. Make sure that the head and sill are straight and level and the jambs are straight and plumb. Check operation of unit frequently as fasteners are set

4. **Note: Adherence to the above screw spacing and caulk requirements will allow this product to be used for design pressures (DP's) up to and including ± 47.2**

5. Caulk entire perimeter of the fin to mounting surface joint. Also caulk over screw heads and unused slots. Note: This step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd, head 3rd.)

6. Fill voids between window frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows. The use of expanding aerosol type insulating foam, which can bow the frame, voids all stated warranties. The use of muriatic acid for brick clean-up may damage the coil spring sash balance system. Windows must be masked off to avoid muriatic acid exposure, which will void the warranty.

7. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

8. For structures finished in brick or stone, allow ¼" gap under the sill, then caulk this joint when complete. Also caulk the head and jamb joints in the same manner.

9. If one or more holes are field drilled (by others) in any area of the window sill for any purpose (such as security systems), the warranty will be void. Adding holes can cause water leaks and interior damage.

## - CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress, structural performance, and energy performance.

Headquarters: M I. Home Products 650 West Market St. Gratz, PA 17030 (717) 365-3300 www.mihp.com

E 7 21

June 26, 2003

Architectural Testing

**ANSI/AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT**

**Rendered to:**

**MI WINDOWS AND DOORS, INC.**

**SERIES/MODEL: 3540**
**PRODUCT TYPE: PVC Triple Single Hung**

| Title | Summary of Results |
|---|---|
| Rating | H-R30* 108 x 74 |
| Operating Force | 17 lbf max. |
| Air Infiltration | 0.11 cfm/ft$^2$ |
| Water Resistance Test Pressure | 4.50 psf |
| Uniform Load Deflection Test Pressure | ±47.2 psf |
| Uniform Load Structural Test Pressure | +52.5 psf, -70.8 psf |
| Forced Entry Resistance | Grade 10 |

Reference should be made to ATI Report No. 50172.01-122-47 for complete test specimen description and data.

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Architectural Testing

# ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

| | |
|---|---|
| Report No.: | 50172.01-122-47 |
| Revision 1: | 08/30/04 |
| Test Dates: | 06/11/04 |
| Through: | 07/07/04 |
| Report Date: | 07/27/04 |
| Expiration Date: | 07/07/08 |

**Project Summary**: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on a Series/Model 3540, triple single hung window at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R30* 108 x 74 rating. Reference should be made to Report No. 01-45617.02 for Gateway Performance results. Test specimen description and results are reported herein.

***General Note:*** *An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

**Test Specification**: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description**:

**Series/Model**: 3540

**Product Type**: PVC Triple Single Hung

**Overall Size**: 8' 11-5/8" wide by 6' 1-3/4" high

**Interior Sash Size (3)**: 2' 9-3/4" wide by 3' 0-1/8" high

**Fixed Daylight Opening Size (3)**: 2' 7-3/4" wide by 2' 9-3/16" high

**Screen Size**: 2' 9" wide by 2' 11-1/4" high

**Overall Area**: 55.1 $ft^2$

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**Test Specimen Description**: (Continued)

**Finish**: All PVC was white.

**Glazing Details**: All glazing consisted of 7/8" thick sealed insulating glass units that were comprised of two sheets of 3/32" thick clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against a double-sided adhesive glazing tape and secured with vinyl glazing beads.

**Weatherstripping**:

| Description | Quantity | Location |
|---|---|---|
| 0.187" backed by 0.250" high polypile | 1 Row | Meeting rail, stiles |
| 0.187" backed by 0.250" high polypile | 1 Row | Sill leg |
| 0.187" backed by 0.310" high polypile | 1 Row | Stiles |
| 0.187" backed, 1/4 foam filled single leaf vinyl bulb gasket | 1 Row | Bottom rail |
| 0.187" backed, 1/8 foam filled vinyl bulb gasket | 1 Row | Fixed meeting rail |

**Frame Construction**: The frame was constructed of extruded PVC members. Corners were mitered and welded. End caps were utilized on the ends of the meeting rail and secured with three #6 by 5/8" screws per cap. The fixed meeting rail was then secured to the frame utilizing three #6 by 5/8" screws.

**Sash Construction**: The sash was constructed of extruded PVC members. Corners were mitered and welded.

**Screen Construction**: The screen was constructed of roll-formed aluminum. Corners were square-cut and secured with vinyl corner keys. The mesh was secured with a flexible vinyl spline.

**Test Specimen Description**: (Continued)

**Hardware**:

| Description | Quantity | Location |
|---|---|---|
| Constant force balances | 6 | One per jamb |
| Metal cam locks with adjacent keepers | 6 | Meeting rail, 7" from each end |
| Plastic tilt latches | 6 | Each end of the interior meeting rail |
| Metal pivot pins | 6 | Each end of the bottom rail |

**Drainage**:

| Description | Quantity | Location |
|---|---|---|
| 3/32" by 1/2" weepslot | 12 | Bottom rail, 2 at each end |
| 1/8" by 1" weepslot | 2 | Sill, 3" from each end |
| 3/16" by 1/2" weepslot | 2 | Screen track, 2-1/2" from each end |

**Reinforcement**: The interior meeting rail and bottom rail utilized a roll-formed "I beam" steel reinforcement (Drawing #GVL-451-020). The fixed meeting rail utilized a steel reinforcement (Drawing #RF-104S-020). The intermediate frame rails utilized a steel reinforcement (Drawing #2.75x.125 steel plate).

**Installation**: The unit was installed into a wood test buck. The nail fin was set against a silicone bedding and fastened to the buck with #6 by 1-5/8" screws, 2" from corners and 8" on center. 3/4" washers were utilized along the entire length of the sill, at midspan of the head and jambs, and at all corners.

**Test Results**: The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.2.6.1.1 | Operating Force | 17 lbf | 30 lbf max. |
| 2.1.2 | Air Infiltration per ASTM E 283<br>1.57 psf (25 mph) | 0.11 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |

***Note #1**: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

**Test Results:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.1.3 | Water Resistance per ASTM E 547<br>(with and without screen) | | See Note #2 |

***Note #2:*** *The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance".*

| | | | |
|---|---|---|---|
| 2.1.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the mullion)<br>(Loads were held for 52 seconds) | | |
| | 35.0 psf (positive) | 0.39" | See Note #3 |
| | 35.0 psf (negative) | 0.54" | See Note #3 |

***Note #3:*** *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

| | | | |
|---|---|---|---|
| 2.1.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the mullion)<br>(Loads were held for 10 seconds) | | |
| | 52.5 psf (positive) | <0.01" | 0.27" max. |
| | 52.5 psf (negative) | 0.07" | 0.27" max. |
| 2.2.6.1.2 | Deglazing Test per ASTM E 987<br>In operating direction - 70 lbs | | |
| | Interior meeting rail | 0.13"/26% | 0.50"/100% |
| | Bottom rail | 0.11"/22% | 0.50"/100% |
| | In remaining direction - 50 lbs | | |
| | Left stile | 0.09"/18% | 0.50"/100% |
| | Right stile | 0.10"/20% | 0.50"/100% |
| 2.1.7 | Welded Corner Test | Meets as stated | Meets as stated |

**Test Results**: (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.1.8 | Forced Entry Resistance per ASTM F 588 | | |
| | Type: A | Grade: 10 | |
| | Lock Manipulation Test | No entry | No entry |
| | Test A1 | No entry | No entry |
| | Test A2 | No entry | No entry |
| | Test A3 | No entry | No entry |
| | Test A4 | No entry | No entry |
| | Test A5 | No entry | No entry |
| | Test A7 | No entry | No entry |
| | Lock Manipulation Test | No entry | No entry |
| Optional Performance | | | |
| 4.3 | Water Resistance per ASTM E 547 (with and without screen) | | |
| | 4.50 psf | No leakage | No leakage |
| 4.4.1 | Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the mullion) (Loads were held for 52 seconds) | | |
| | 47.2 psf (positive) | 0.73" | See Note #3 |
| | 47.2 psf (negative) | 0.92" | See Note #3 |
| 4.4.2 | Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the mullion) (Loads were held for 10 seconds) | | |
| | 52.5 psf (positive) | <0.01" | 0.27" max. |
| | 70.8 psf (negative) | 0.21" | 0.27" max. |

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Jeramie D. Grabosch

Jeramie D. Grabosch
Technician

Digitally Signed by: Steven M. Urich

Steven. M. Urich, P.E.
Senior Project Engineer

JDG:vlm

Architectural Testing

**ANSI/AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 3540 Picture Window**
**with Continuous Head and Sill**
**PRODUCT TYPE: PVC Triple Fixed Window**

| Title | Summary of Results |
|---|---|
| Rating | F-C40 108 x 74 |
| Uniform Load Deflection Test Pressure | ±47.0 psf |
| Uniform Structural Load Test Pressure | ±70.5 psf |

Reference should be made to ATI Report Identification No. 01-51007.02 for complete test specimen description and data.

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# ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030

ATI Report Identification No.: 01-51007.02
Test Date: 04/30/04
Report Date: 06/12/04
Expiration Date: 04/30/08

**Project Summary**: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc to witness testing on a Series/Model 3540 Picture Window with Continuous Head and Sill, PVC triple fixed window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a F-C40 108 x 74 rating. Test specimen description and results are reported herein. Reference should be made to ATI Report Identification No. 01-51007.01 for air infiltration and water penetration test results.

**Test Specification**: The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description**:

**Series/Model**: 3540 Picture Window with Continuous Head and Sill

**Product Type**: PVC Triple Fixed Window

**Overall Size**: 8' 11-9/16" wide by 6' 1-1/2" high

**Daylight Opening Size (3)**: 2' 7-3/4" wide by 5' 9-1/2" high

**Overall Area**: 54.90 $ft^2$

**Finish**: All PVC was white.

**Glazing Details**: The window utilized 7/8" thick, sealed insulating glass units fabricated of two sheets of 1/8" thick, clear annealed glass and metal reinforced butyl spacer system. The glass was interior glazed against a dual-sided adhesive foam tape and secured utilizing extruded snap-in glazing beads.

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**Test Specimen Description**: (Continued)

**Frame Construction**: The frame was constructed of extruded PVC members with mitered and welded corners. An extruded PVC snap-in filler piece was utilized around the entire interior perimeter of the each opening. The mullions were coped, butted, sealed with single-sided adhesive foam pads and secured to the head and sill utilizing two #8 by 2-1/2" long screws and two #8 by 1-1/4" screws. Sealant was utilized at the exterior face of the mullion to jamb joinery.

**Drainage**:

| Description | Quantity | Location |
|---|---|---|
| 5/8" wide by 1/4" high weepslot | 2 Per window | Corners of the sill draining the interior sill hollow |
| 5/8" wide by 1/4" high weepslot | 2 Per window | Corners of the sill draining the intermediate sill hollow |
| 1" long by 1/8" high weepslot | 2 Per window | Exterior sill face draining the exterior sill hollow |
| 1/2" long by 3/16" high weepslot | 2 Per window | 2" from corners draining the glazing pocket |
| 1/2" long by 1/8" high weepslot | 2 Per window | 1" from corners draining the glazing channel |

**Reinforcement**: A 1/8" by 2-1/2" strip of steel was utilized in the mullions.

**Installation**: The window was installed into a Spruce-Pine-Fir wood test buck utilizing an integral nailing fin. The nailing fin was bedded in silicone and secured to the test buck utilizing 1-5/8" wood screws 3" from the corners and 8" on center. Washers were utilized at the corners and midspan of the head, jambs and entire sill. A 3/4" by 3/4" wood blind stop was secured to the sill utilizing 1-5/8" wood screws 2" from the end and 8" on center.

**Test Results:**

The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| | Optional Performance | | |
| 4.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the mullion)<br>(Loads were held for 52 seconds) | | |
| | 47.0 psf (positive) | 0.63" | See Note #2 |
| | 47.0 psf (negative) | 0.69" | See Note #2 |
| 4.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the mullion)<br>(Loads were held for 10 seconds) | | |
| | 70.5 psf (positive) | 0.03" | 0.28" max. |
| | 70.5 psf (negative) | 0.11" | 0.28" max. |

***Note #2:** The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

EW:nlb
01-51007.02

Architectural Testing

**ANSI/AAMA/NWWDA 101/I.S.2-97**
**TEST REPORT**

**Rendered to:**

**MI WINDOWS AND DOORS, INC**

**SERIES/MODEL: 420/430/440**
**PRODUCT TYPE: Aluminum Sliding Glass Door**

| | Summary of Results | | |
|---|---|---|---|
| **Title** | **Test Specimen #1** | **Test Specimen #2** | **Test Specimen #3** |
| Rating | SGD-R25 182 x 96 | SGD-R35 182 x 80 | SGD-R40 144 x 96 |
| Operating Force | 17 lbf max. | 17 lbf max. | N/A |
| Air Infiltration | 0.23 cfm/ft$^2$ | 0.27 cfm/ft$^2$ | N/A |
| Water Resistance Test Pressure | 3.75/6.0/9.0 psf | 6.0 psf | N/A |
| Uniform Load Deflection Test Pressure | ±35.0 psf | ±35.0 psf | +40.0 psf/-40.1 psf |
| Uniform Load Structural Test Pressure | ±37.5 psf | ±52.5 psf | +60.0 psf/-60.2 psf |
| Forced Entry Resistance | Grade 10 | Grade 10 | N/A |

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

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Architectural Testing

# ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

| | |
|---|---|
| Report No.: | 52112.01-122-47 |
| Revision 1: | 09/13/04 |
| Test Dates: | 06/30/04 |
| Through: | 08/12/04 |
| Report Date: | 08/30/04 |
| Expiration Date: | 07/02/08 |

**Project Summary**: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

**Test Specification**: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description**:

**Series/Model**: 420/430/440

**Product Type**: Aluminum Sliding Glass Door

**Test Specimen #1**: SGD-R25 182 x 96 (XXO)

**Overall Size**: 15' 1-3/4" wide by 8' 0" high

**Active Door Panel Size (2)**: 5' 0-1/2" wide by 7' 11" high

**Fixed Door Panel Size**: 5' 1" wide by 7' 11" high

**Screen Size**: 5' 0-3/8" wide by 7' 11" high

**Overall Area**: 121.2 $ft^2$

**Reinforcement**: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

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**Test Specimen Description**: (Continued)

**Test Specimen #2**: SGD-R35 182 x 80 (OXX)

**Overall Size**: 15' 1-3/4" wide by 6' 8" high

**Active Door Panel Size (2)**: 5' 0-1/2" wide by 6' 7" high

**Fixed Door Panel Size**: 4' 8-7/8" wide by 6' 2-5/8" high

**Screen Size**: 5' 0-3/8" wide by 6' 7" high

**Overall Area**: 101 $ft^2$

**Reinforcement**: No reinforcement was utilized.

**Test Specimen #3**: SGD-R40 144 x 96 (XOX)

**Overall Size**: 12' 0" wide by 8' 0" high

**Active Door Panel Size**: 3' 8-1/4" wide by 7' 10-1/2" high

**Fixed Door Panel Size (2)**: 3' 8-3/4" wide by 7' 6-1/2" high

**Screen Size**: 3' 11-1/2" wide by 7' 11-3/8" high

**Overall Area**: 96 $ft^2$

**Reinforcement**: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

***The following descriptions apply to all specimens.***

**Finish**: All aluminum was white.

**Glazing Details**: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

**Test Specimen Description**: (Continued)

**Weatherstripping**:

| Description | Quantity | Location |
|---|---|---|
| 0.187" backed by 0.270" high polypile with center fin | 2 Rows | Stiles |

**Frame Construction**: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 by 5/8" screws.

**Door Panel Construction**: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" screw at the bottom and two #8 by 3/4" screws at the top.

**Screen Construction**: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" and one #8 by 1" screw at the bottom and one #8 by 1" screw at the top.

**Hardware**:

| Description | Quantity | Location |
|---|---|---|
| Locking handle | 1 | 44" from active panel bottom |
| Roller assembly | 2 | 3" from bottom rail ends |
| Screen locking handle | 1 | 46" from screen bottom rail |

**Drainage**:

| Description | Quantity | Location |
|---|---|---|
| Sloped sill | 1 | Sill |

**Installation**: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 by 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

**Test Results:**

The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| **Test Specimen #1**: SGD-R25 182 x 96 (XXO) | | | |
| 2.2.1.6.1 | Operating Force | 17 lbf | 20 lbf max. |
| | Breakaway force | 24 lbf | 30 lbf max. |
| 2.1.2 | Air Infiltration per ASTM E 283 | | |
| | 1.57 psf (25 mph) | 0.23 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |

***Note #1:*** *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

| | | | |
|---|---|---|---|
| 2.1.3 | Water Resistance per ASTM E 547 | | |
| | (with and without screen) | | |
| | 2.86 psf | No leakage | No leakage |
| 2.1.4.1 | Uniform Load Deflection per ASTM E 330 | | |
| | (Deflections reported were taken on the meeting rail) | | |
| | (Loads were held for 52 seconds) | | |
| | 15.0 psf (positive) | 0.56" | See Note #2 |
| | 15.0 psf (negative) | 0.57" | See Note #2 |

***Note #2:*** *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

| | | | |
|---|---|---|---|
| 2.1.4.2 | Uniform Load Structural per ASTM E 330 | | |
| | (Permanent sets reported were taken on the meeting stile) | | |
| | (Loads were held for 10 seconds) | | |
| | 22.5 psf (positive) | 0.02" | 0.30" max. |
| | 22.5 psf (negative) | 0.03" | 0.30" max. |
| 2.2.1.6.2 | Deglazing Test per ASTM E 987 | | |
| | In operating direction - 70 lbs | | |
| | Locking stile | 0.12"/24% | 0.50"/100% |
| | Interlock stile | 0.12"/24% | 0.50"/100% |

**Test Results**: (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| **Test Specimen #1**: SGD-R25 182 x 96 (XXO) (Continued) | | | |
| 2.2.1.6.2 | Deglazing Test per ASTM E 987<br>In remaining direction - 50 lbs | | |
| | Top rail | 0.06"/12% | 0.50"/100% |
| | Bottom rail | 0.06"/12% | 0.50"/100% |
| 2.1.8 | Forced Entry Resistance per ASTM F 842 | | |
| | Type: A | Grade: 10 | |
| | Lock Manipulation Test | No entry | No entry |
| | Test A1 through A6 | No entry | No entry |
| | Lock Manipulation Test | No entry | No entry |
| Optional Performance | | | |
| 4.3 | Water Resistance per ASTM E 547<br>(with and without screen)<br>3.75 psf | No leakage | No leakage |
| 4.3 | Water Resistance per ASTM E 547<br>(with and without screen)<br>(with sill riser)<br>6.0 psf | No leakage | No leakage |
| 4.3 | Water Resistance per ASTM E 547<br>(with and without screen)<br>(with 2-5/8" Dade County sill extension)<br>9.0 psf | No leakage | No leakage |
| 4.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the meeting stile)<br>(Loads were held for 10 seconds) | | |
| | 35.0 psf (positive) | 2.98" | See Note #2 |
| | 35.0 psf (negative) | 2.52" | See Note #2 |

**Test Results**: (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| **Test Specimen #1**: SGD-R25 182 x 96 (XXO) (Continued) | | | |
| 4.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the meeting stile)<br>(Loads were held for 10 seconds) | | |
| | 37.5 psf (positive) | 0.20" | 0.36" max. |
| | 37.5 psf (negative) | 0.19" | 0.36" max. |
| **Test Specimen #2**: SGD-R35 182 x 80 (OXX) | | | |
| 2.2.1.6.1 | Operating Force | 17 lbf | 20 lbf max. |
| | Breakaway force | 21 lbf | 30 lbf max. |
| 2.1.2 | Air Infiltration per ASTM E 283 | | |
| | 1.57 psf (25 mph) | 0.27 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |
| | ***Note #1:*** *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.* | | |
| 2.1.3 | Water Resistance per ASTM E 547<br>(with and without screen) | | |
| | 2.86 psf | No leakage | No leakage |
| 2.2.1.6.2 | Deglazing Test per ASTM E 987<br>In operating direction - 70 lbs | | |
| | Locking stile | 0.12"/24% | 0.50"/100% |
| | Interlock stile | 0.12"/24% | 0.50"/100% |
| | In remaining direction - 50 lbs | | |
| | Top rail | 0.06"/12% | 0.50"/100% |
| | Bottom rail | 0.06"/12% | 0.50"/100% |
| 2.1.8 | Forced Entry Resistance per ASTM F 842 | | |
| | Type: A | Grade: 10 | |
| | Lock Manipulation Test | No entry | No entry |
| | Test A1 through A6 | No entry | No entry |
| | Lock Manipulation Test | No entry | No entry |

**Test Results**: (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| **Test Specimen #2**: SGD-R35 182 x 80 (OXX) (Continued) | | | |
| Optional Performance | | | |
| 4.3 | Water Resistance per ASTM E 547<br>(with and without screen)<br>(with sill riser)<br>6.0 psf | No leakage | No leakage |
| 4.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the meeting stile)<br>(Loads were held for 52 seconds) | | |
| | 35.0 psf (positive) | 1.28" | See Note #2 |
| | 35.0 psf (negative) | 1.33" | See Note #2 |
| 4.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the meeting stile)<br>(Loads were held for 10 seconds) | | |
| | 52.5 psf (positive) | 0.13" | 0.30" max. |
| | 52.5 psf (negative) | 0.15" | 0.30" max. |
| **Test Specimen #3**: SGD-R40 144 x 96 (XOX) | | | |
| Optional Performance | | | |
| 4.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the meeting stile)<br>(Loads were held for 52 seconds) | | |
| | 40.0 psf (positive) | 1.42" | See Note #2 |
| | 40.1 psf (negative) | 1.28" | See Note #2 |
| 4.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the meeting stile)<br>(Loads were held for 10 seconds) | | |
| | 60.0 psf (positive) | 0.27" | 0.37" max. |
| | 60.2 psf (negative) | 0.30" | 0.37" max. |

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

***MI WINDOWS AND DOORS, INC.***

**420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR**

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

STEEL AND ALUMINUM REINFORCING

09/08/2004

SGD ALUM & STL REINF

| PANEL WIDTH >> | 24 | 30 | 36 | 48 |
|---|---|---|---|---|
| PANEL HEIGHT 80 | 85 | 71 | 62 | 51 |
| 96 | 69 | 57 | 49 | 40 |

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 40.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL TEST SIZE: 12'-0" X 8'-0" NOMINAL

OVERALL PANEL SIZE: 48 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES AND INTERMEDIATE JAMB. ADDITIONAL ALUM. REINFORCING ON EXTERIOR OF OPERATING INTERLOCK STILE.
CONFIGURATION: XOX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622.6335
www.ptc-corp.com

## *MI WINDOWS AND DOORS, INC.*

**420 / 430 / 440 SERIES ALUMNUM SLIDING GLASS DOOR**

09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

SGD non-Reinf

| PANEL WIDTH >> | 24 | 30 | 36 | 48 | 60 |
|---|---|---|---|---|---|
| PANEL HEIGHT<br>80 | 64 | 54 | 47 | 39 | 35 |

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 35.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 6'-8" NOMINAL
OVERALL PANEL SIZE TESTED: 5'-0" X 6'-8" NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN THICK TEMP. GLASS
REINFORCING: NONE
CONFIGURATION TESTED: XXO

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 25.0 PSF
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 40.0PSF
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 60.0 PSF

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
*PRODUCT TECHNOLOGY CORPORATION*
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622-6335
www.ptc-corp.com

## *MI WINDOWS AND DOORS, INC.*

**420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR** | STEEL REINFORCED | 09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE | SGD STL REINF

| PANEL WIDTH >> | 24 | 30 | 36 | 48 | 60 |
|---|---|---|---|---|---|
| PANEL HEIGHT<br>80 | 61 | 51 | 44 | 37 | 33 |
| 96 | 49 | 41 | 35 | 29 | 25 |

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 25.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 8'-0" NOMINAL
OVERALL PANEL SIZE TESTED: 60 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES, AND FIXED INTERMEDIATE JAMB
CONFIGURATION TESTED: OXX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

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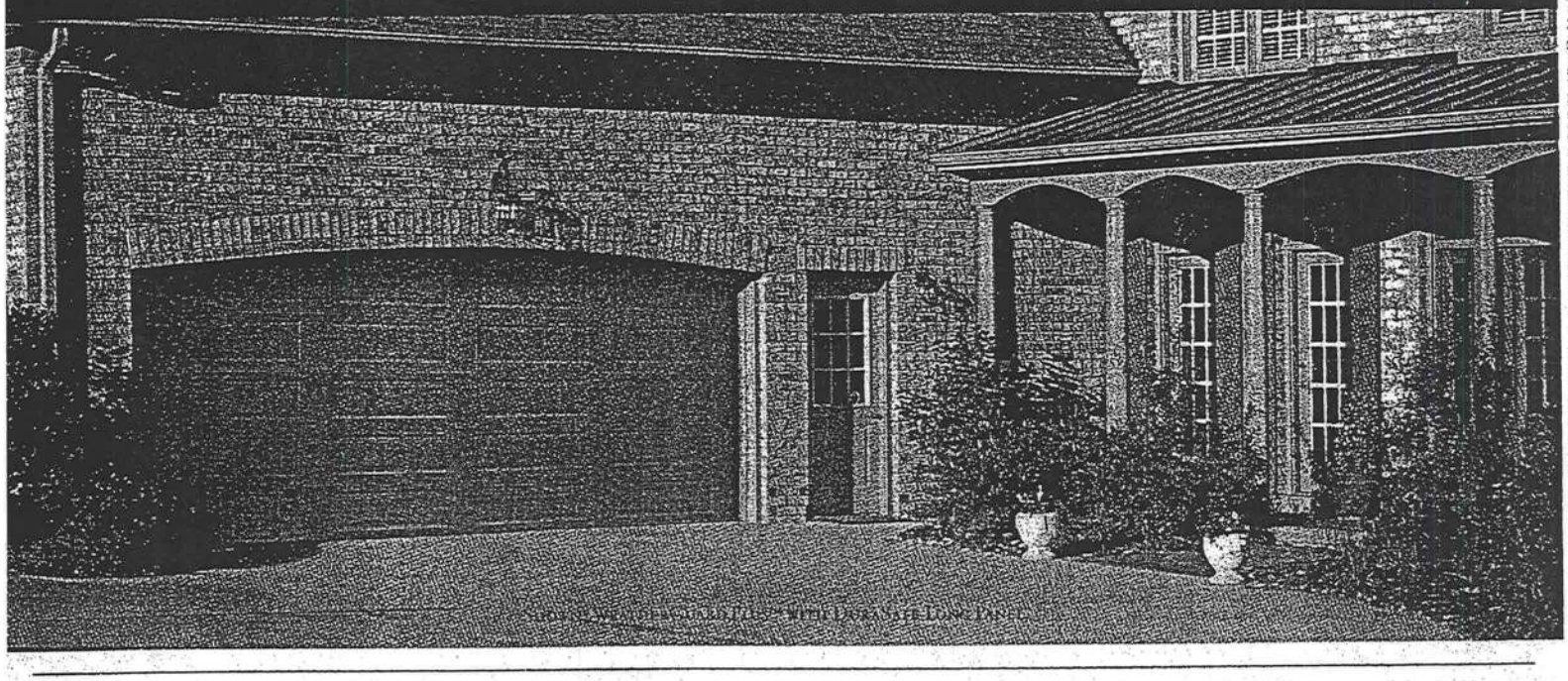
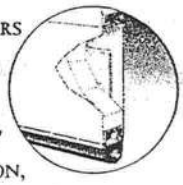
# Amarr GARAGE DOORS

BEST | WEATHERGUARD™ SERIES
FEATURING OUR **DuraSafe System**



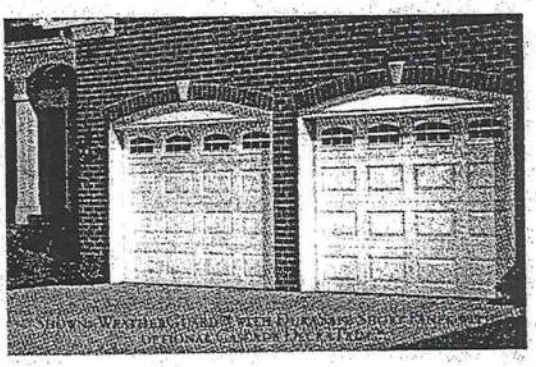
## WEATHERGUARD PLUS™ WITH **DuraSafe**



THE WEATHERGUARD PLUS OFFERS DISCERNING HOMEOWNERS A MASTERFUL COMBINATION OF PREMIUM FEATURES. SUPERIOR TRIPLE-LAYER CONSTRUCTION, 2" (5.1 CM) POLYSTYRENE INSULATION, AN R-VALUE OF 8.34, AND UNMATCHED BEAUTY PUT THE WEATHERGUARD PLUS AT THE TOP OF ITS CLASS.

## WEATHERGUARD™ WITH **DuraSafe**

TOP-QUALITY TRIPLE-LAYER CONSTRUCTION AND 1 3/8" (3.5 CM) POLYSTYRENE INSULATION MAKE OUR WEATHERGUARD STEEL DOOR STRONG, QUIET, AND ENERGY EFFICIENT. FEATURING AN R-VALUE OF 5.73, THE WEATHERGUARD IS THE PERFECT ADDITION TO YOUR HOME FOR YEARS OF TROUBLE FREE SERVICE AND GREAT LOOKS.



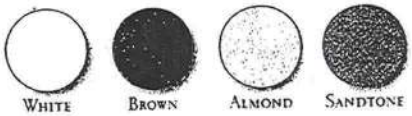
## DESIGN ELEMENTS

THE WEATHERGUARD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT, RAISED LONG, OR FLUSH PANEL DESIGN IN YOUR CHOICE OF FOUR COLORS.*

RAISED SHORT PANEL

RAISED LONG PANEL

FLUSH PANEL



WHITE BROWN ALMOND SANDTONE

* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

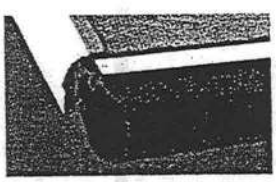
**Bottom Seal**
NEW ALUMINUM BOTTOM SEAL MEANS EASY AND FAST INSTALLATION AND MAINTENANCE... AS WELL AS A BETTER SEAL AGAINST THE ELEMENTS.

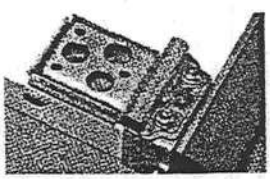
**Bottom Bracket**
NEW TAMPER RESISTANT BOTTOM BRACKET HELPS PREVENT ACCIDENTS, YET ALLOWS FOR ROLLER MAINTENANCE/CHANGE WITHOUT DISASSEMBLY. FULL LENGTH ROLLER TUBE PREVENTS SLIP-OUTS.

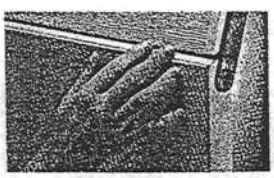
**Door Sections**
THE SECTION JOINT OF THE FUTURE: TODAY. NEW SECTION PROFILE ASSURES PINCH RESISTANCE BOTH INSIDE AND OUT, EXCEEDING INDUSTRY STANDARDS - NEITHER FINGERS NOR WEATHER GETS IN.

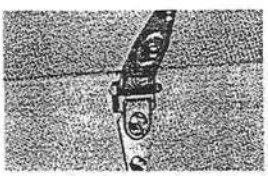
**Center Hinge**
FLUSH MOUNT INBOARD DESIGN CENTER HINGES PROVIDE PINCH RESISTANT PROTECTION AND A LOW PROFILE CLEAN LOOK ON THE INSIDE OF THE DOOR.

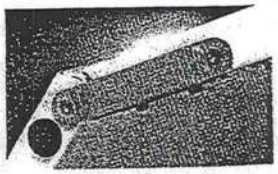
**End Hinge**
WITH MOST OF ITS ACTION HIDDEN INSIDE THE DOOR, OUR RE-ENGINEERED END HINGES LEAVE NO ROOM FOR EVEN THE SMALLEST FINGERS.









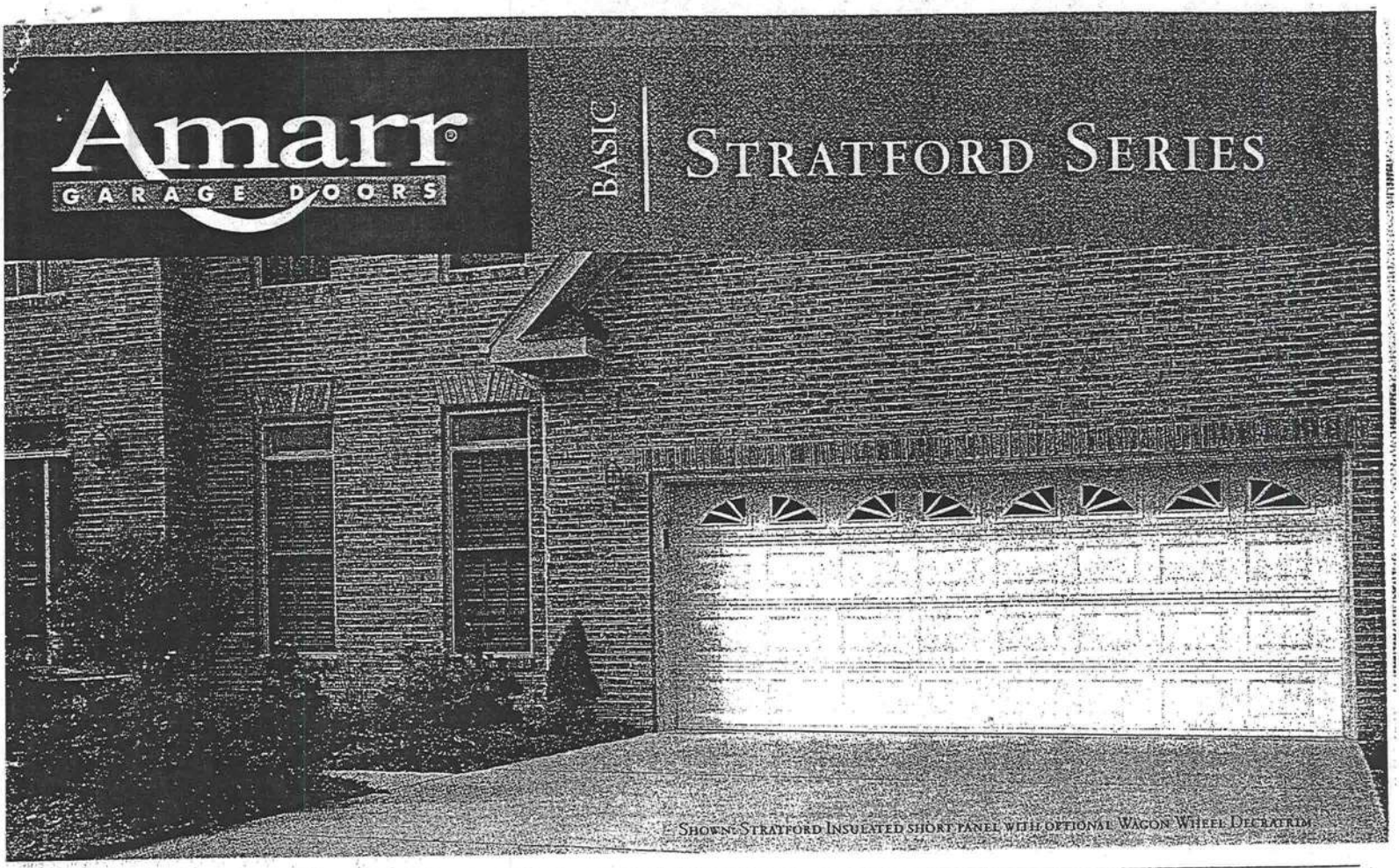


AMARR DURASAFE DOORS UNDER 8'9" WILL BE SUPPLIED WITH DURASAFE HARDWARE. DASMA STANDARDS FOR PINCH-RESISTANCE DO NOT APPLY TO DOORS OVER 8' HIGH SINCE THE POTENTIAL PINCH POINTS ARE ABOVE TYPICAL GRASPING HEIGHTS; AMARR DOORS OVER 8'9" ARE SUPPLIED WITH CONVENTIONAL HARDWARE. THE BOTTOM BRACKET, DOOR SECTIONS, CENTER HINGE AND END HINGE SHOWN ABOVE ARE PATENTED. DOORS SHOWN ARE ELECTRICALLY OPERATED. NON-ELECTRICALLY OPERATED DOORS SHOULD HAVE EXTERIOR AND INTERIOR LIFT HANDLES ATTACHED TO THE DOOR.

Amarr
GARAGE DOORS

BASIC

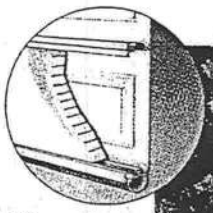
# STRATFORD SERIES



SHOWN: STRATFORD INSULATED SHORT PANEL WITH OPTIONAL WAGON WHEEL DECRATRIM

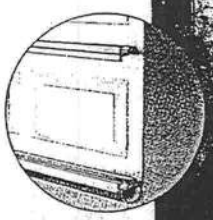
## STRATFORD INSULATED

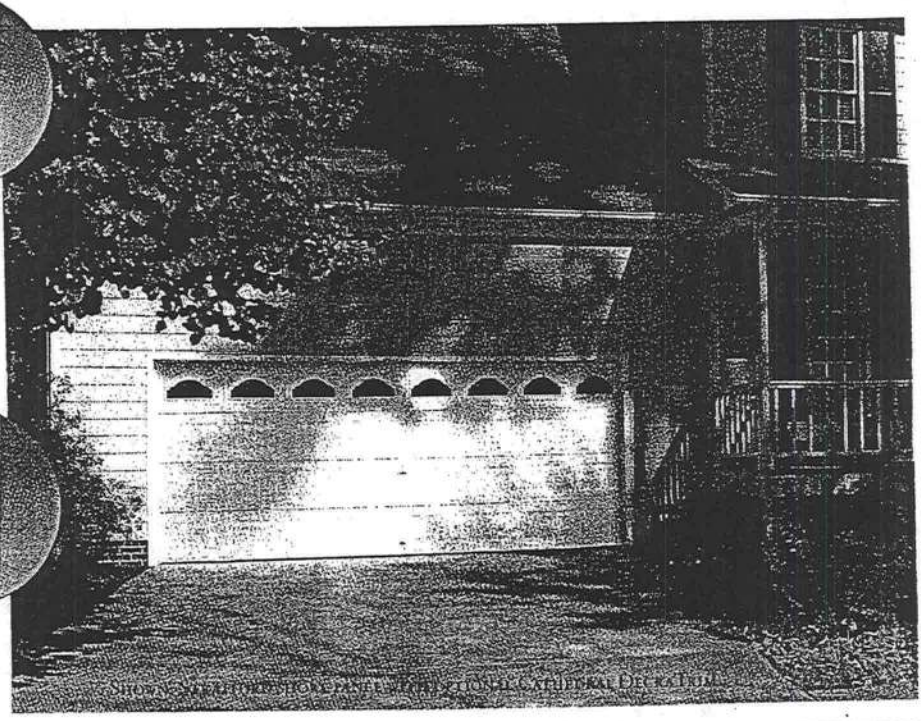
THE 2" (5.1 CM) THICK STRATFORD INSULATED PROVIDES HOMEOWNERS EXCELLENT THERMAL PROTECTION AND HANDSOME GOOD LOOKS. FEATURES INCLUDE DOUBLE-LAYER CONSTRUCTION OF STURDY 25-GAUGE STEEL, AND 1 7/16" (3.7 CM) POLYSTYRENE INSULATION WITH LAMINATED BACKING AND AN $R^2$-VALUE OF 5.65.



## STRATFORD

A SUPERLATIVE ADDITION TO ANY HOME, THE STRATFORD'S DURABLE SINGLE-LAYER CONSTRUCTION, 25-GAUGE STEEL, AND ATTRACTIVE DESIGN PROVIDE HOMEOWNERS WITH EXCEPTIONAL VALUE.





SHOWN: STRATFORD SHORT PANEL WITH OPTIONAL CATHEDRAL DECRATRIM

## DESIGN ELEMENTS

THE STRATFORD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT PANEL DESIGN IN YOUR CHOICE OF THREE COLORS.*

RAISED SHORT PANEL

WHITE ALMOND SANDTONE

* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

# The Florida Department of Community Affairs Building Code Information System

SITE NAVIGATION

Home | Course Accreditation | Florida Building Code | Manufact. Buildings | Prototype Building | Surcharges | Training | Product Approval | License Search | Mailing List | Florida Building Commission

## PRODUCT APPROVAL

Product Type Detail

Overview | Product Search | Organization Search | Product Application

User: Public User - Not Associated with Organization -

Need Help?

| | |
|---|---|
| Application #: | FL5042 |
| Date Submitted: | 08/04/2005 |
| Code Version: | 2004 |
| Product Manufacturer: | Windoor Incorporated |
| Address/Phone/email: | 1978 Stanhome Way<br>Orlando, FL 32804<br>(407) 481-8400 |
| Technical Representative: | Bruce Jasewic |
| Technical Representative Address/Phone/email: | 1978 Stanhome Way<br>Orlando, FL 32804<br>bjasewic@windoorinc.com |
| Category: | Exterior Doors |
| Subcategory: | Sliding |
| Evaluation Method: | Certification Mark or Listing |

Referenced Standards from the Florida Building Code:

| Section | Standard | Year |
|---|---|---|
| 1609.1.4 | TAS 202-94 | 1994 |
| 1609.1.4 | TAS 201-94 | 1994 |
| 1609.1.4 | TAS 203-94 | 1994 |
| 1609.1.4 | TAS 202-94 | 1994 |
| 1609.1.4 | TAS 201-94 | 1994 |
| 1609.1.4 | TAS 203-94 | 1994 |
| 1609.1.4 | TAS 202-94 | 1994 |
| 1609.1.4 | TAS 201-94 | 1994 |
| 1609.1.4 | TAS 203-94 | 1994 |

| | |
|---|---|
| Certification Agency: | Miami-Dade BCCO - CER |
| Quality Assurance Entity: | |
| Validation Entity: | |

| | |
|---|---|
| Authorized Signature: | Bruce Jasewic<br>bjasewic@windoorinc.com |
| Evaluation/Test Reports Uploaded: | |
| Installation Documents Uploaded: | PTID_5042_1_Dade N.O.A. for 8000 SGD.pdf<br>PTID_5042_1_Dade N.O.A. for WIND0077.pdf<br>PTID_5042_1_WIND0026.pdf<br>PTID_5042_1_WIND0076.pdf<br>PTID_5042_1_WIND0077.pdf<br>PTID_5042_1_WIND0081.pdf<br>PTID_5042_1_WIND0082.pdf |
| Product Approval Method: | Method 1 Option A |
| Application Status: | Approved |
| Date Validated: | 08/04/2005 |
| Date Approved: | 08/24/2005 |
| Date Certified to the 2004 Code: | |

Page:
Go

Page 1 / 1

| App/Seq # | Product Model # or Name | Model Description | Limits of Use |
|---|---|---|---|
| 5042.1 | Series 8000 SGD | Series 8000 SGD Impact | Max size: 297 1/8" X 120" Design pressure: see uploaded drawings Max number of configurations: see uploaded drawings For us in HVHZ |

Next

Florida Product Approval #

MIAMI-DADE COUNTY

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

# NOTICE OF ACCEPTANCE (NOA)

**Windoor, Inc.**
**1978 Stanhome Way**
**Orlando, FL 32804**

**SCOPE:**

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.

**DESCRIPTION: Series 8000 Aluminum Sliding Glass Door-S.M.I.**

**APPROVAL DOCUMENT:** Drawing No. **WIND8077**, titled "Series 8000 SGD Small Missile Elevation and General Notes", sheets 1 through 8 of 8, prepared by Product Technology Corporation, dated by 07/22/04 with revision "A" on 12/21/04, signed and sealed by Eric S. Nielsen, P.E., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

**MISSILE IMPACT RATING: Small Missile Impact**

**LABELING:** Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

**RENEWAL** of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

**TERMINATION** of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

**ADVERTISEMENT:** The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

**INSPECTION:** A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consists of this page 1 and evidence pages E-1, as well as approval document mentioned above
The submitted documentation was reviewed by **Herminio F. Gonzalez, P.E., Director, BCCO**

4/18/2005

**NOA No 04-1019.01**
**Expiration Date: May 12, 2010**
**Approval Date: May 12, 2005**
**Page 1**

Windoor, Inc.

## NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

**A. DRAWINGS**

1. Manufacturer's die drawings and sections.
2. Drawing No.WIND0077 titled "Series 8000 SGD Small Missile Elevation and General Notes", Sheets 1 through 8 of 8, prepared by Product Technology Corporation dated 07/22/04 with revision "A" on 12/21/04, signed and sealed by Eric S. Nielsen, P.E.

**B. TESTS**

1. Test reports on 1) Air Infiltration Test, per FBC, TAS 202-94
   2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
   3) Water Resistance Test, per FBC, TAS 202-94
   4) Small Missile Impact Test per FBC, TAS 201-94(aggregate gravels)
   5) Cyclic Wind Pressure Loading per FBC, TAS 203-94
   6) Forced Entry Test, per FBC 2411 3.2.1 (b) and TAS 202-94

   along with marked-up drawings and installation diagram of an aluminum sliding glass door, prepared by National Certified Testing Laboratories, Test Report No. NCTL-210-3018-1, dated 05/03/04, signed and sealed by Gerald J. Ferrara, P.E.

**C. CALCULATIONS**

1. Anchor Calculations, ASTM-E1300, and structural analysis, prepared by Product Technology Corporation, dated 08/03/04, signed and sealed by Eric S. Nielsen, P.E.

**D. QUALITY ASSURANCE**

1. Miami Dade Building Code Compliance Office (BCCO).

**E. MATERIAL CERTIFICATIONS**

1. Notice of Acceptance No.03-0421.01 for" Saflex HP a polyvinyl butryal interlayer for lamination of glass" issued to Solutia Inc., approved on 05/22/03 with expiration date of 04/14/08.

**F. STATEMENTS**

1. Statement letter of compliance and no financial interest, dated October 05, 2004, signed and sealed by Eric S. Nielsen, P.E.

**G. OTHER**

1. Letter from the consultant stating that the product is in compliance with the Florida Building Code.

for

Herminio F. Gonzalez, P.E.
Director, Building Code Compliance Office
NOA No 04-1019.01
Expiration Date: May 12, 2010
Approval Date: May 12, 2005

Architectural Testing

# ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

| | |
|---|---|
| Report No: | 47496.02-122-47 |
| Test Date: | 10/07/03 |
| Report Date: | 01/27/05 |
| Expiration Date: | 10/07/07 |

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness tests on a Series/Model 450/650/680, aluminum picture window with sill insert at their test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a F-C40 71 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

**Test Specimen Description:**

**Series/Model:** 450/650/680

**Type:** Aluminum Picture Window with Sill Insert

**Overall Size:** 5' 11-3/16" wide by 5' 11-11/16" high

**Daylight Opening Size:** 5' 6" wide by 5' 8-1/8" high

**Finish:** All aluminum was white.

**Glazing** [illegible] The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 3/16" thick clear tempered glass and a metal reinforced butyl spacer system. The window was interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

**Test Specimen Description:** (Continued)

**Frame Construction:** The frame was constructed of thermally broken extruded aluminum. The corners were coped, butted, sealed, and fastened with two #8 x 1" screws per corner. The jambs utilized an aluminum snap-in jamb cover.

**Installation:** The window was installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured utilizing #8 x 1-5/8" drywall screws located 3" from corners and midspan of all members.

**Test Results:** The results are tabulated as follows:

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| 2.1.2 | Air Infiltration per ASTM E 283<br>1.57 psf (25 mph) | 0.02 cfm/ft$^2$ | 0.3 cfm/ft$^2$ max. |

***Note #1:*** *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

| | | | |
|---|---|---|---|
| 2.1.3 | Water Resistance per ASTM E 547<br>(with and without screen)<br>4.50 psf | No leakage | No leakage |
| 2.1.4.1 | Uniform Load Deflection per ASTM E 330<br>(Deflections reported were taken on the jamb)<br>(Loads were held for 52 seconds)<br>30.0 psf (positive)<br>30.0 psf (negative) | <br><br><br>0.03"<br>0.05" | <br><br><br>See Note #2<br>See Note #2 |

***Note #2:*** *The Uniform Load Deflection test is not an ANSI/AAMA/NWWDA 101/I.S.2-97 requirement for this product designation. The data is recorded in this report for information only.*

| | | | |
|---|---|---|---|
| 2.1.4.2 | Uniform Load Structural per ASTM E 330<br>(Permanent sets reported were taken on the jamb)<br>(Loads were held for 10 seconds)<br>45.0 psf (positive)<br>45.0 psf (negative) | <br><br><br>0.01"<br>0.01" | <br><br><br>0.27" max.<br>0.27" max. |
| 2.1.8 | Forced Entry Resistance per ASTM F 588 | | |
| | Type: D | Grade: 10 | |
| | Hand and Tool Manipulation Test | No entry | No entry |

**Test Results:** (Continued)

| Paragraph | Title of Test - Test Method | Results | Allowed |
|---|---|---|---|
| | Optional Performance | | |
| 4.3 | Water Resistance per ASTM E 547 (with and without screen) 7.50 psf | No leakage | No leakage |
| 4.4.1 | Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the jamb) (Loads were held for 52 seconds) | | |
| | 40.0 psf (positive) | 0.03" | See Note #2 |
| | 40.0 psf (negative) | 0.06" | See Note #2 |
| 4.4.2 | Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the jamb) (Loads were held for 10 seconds) | | |
| | 60.0 psf (positive) | 0.01" | 0.27" max. |
| | 60.0 psf (negative) | <0.01" | 0.27" max. |

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MAH:vlm

# TRIM-IT™ SPAN TABLES

## Loading 40 TCLL-10 TCDL-0 BCLL-5 BCDL

TCLL = Top Chord Live Load • TCDL = Top Chord Dead Load
BCLL = Bottom Chord Live Load • BCDL = Bottom Chord Dead Load

3 x 2 SPF 2100F 1.8E • 4 x 2 SYP SS

## Deflection L/360

| Depth | Product Number | Lumber Size | On-Center Spacing | | | |
|---|---|---|---|---|---|---|
| | | | 12" | 16" | 19.2" | 24" |
| 11-1/4" | TI 1223S | 3 x 2 | 20-0-0 | 19-4-0 | 18-3-0 | 16-11-0 |
| | TI 1224P | 4 x 2 | 20-0-0 | 20-0-0 | 20-0-0 | 18-0-0 |
| 14" | TI 1423S | 3 x 2 | 24-0-0 | 23-0-0 | 21-2-0 | 18-0-0 |
| | TI 1424P | 4 x 2 | 24-0-0 | 24-0-0 | 24-0-0 | 22-0-0 |
| 16" | TI 1623S | 3 x 2 | 26-0-0 | 25-1-0 | 23-0-0 | 19-4-0 |
| | TI 1624P | 4 x 2 | 28-0-0 | 28-0-0 | 26-0-0 | 22-0-0 |

## Deflection L/480

| Depth | Product Number | Lumber Size | On-Center Spacing | | | |
|---|---|---|---|---|---|---|
| | | | 12" | 16" | 19.2" | 24" |
| 11-1/4" | TI 1223S | 3 x 2 | 19-4-0 | 17-7-0 | 16-7-0 | 15-0-0 |
| | TI 1224P | 4 x 2 | 20-0-0 | 19-5-0 | 18-0-0 | 16-11-0 |
| 14" | TI 1423S | 3 x 2 | 22-0-0 | 20-8-0 | 19-6-0 | 18-0-0 |
| | TI 1424P | 4 x 2 | 24-0-0 | 23-2-0 | 21-5-0 | 20-0-0 |
| 16" | TI 1623S | 3 x 2 | 25-1-0 | 22-6-0 | 21-0-0 | 19-4-0 |
| | TI 1624P | 4 x 2 | 28-0-0 | 25-4-0 | 23-5-0 | 21-5-0 |

**Notes**

- No composite action is assumed when sheathing is fastened to the top chord.
- For web configurations see specific component designs
- Spans can be applied to non-pitched roofs.
- Span tables indicate the maximum design spans (including a 1-3/4" bottom chord minimum bearing at each end).

David C. Wert, FL Lic. #36197
MiTek Industries, Inc.
14515 N. Outer Forty, Ste 300
Chesterfield, MO 63017
FL Cert. #6634

JUN 0 8 2004

Don Reed Const. Fax 755-7272

# COLUMBIA COUNTY BUILDING DEPARTMENT

## RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS LISTED ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INCLUDE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 SECTION 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA. OR ALTERNATE METHODOLOGIES APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND TWO-FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: U.S. HIGHWAY 41 FROM COLUMBIA COUNTYS NORTHERN BOUNDARY TO THE INTERSECTION OF MYRTIS ROAD, FOLLOW MYRTIS EAST TO THE INTERSECTION OF C.R. 245, FOLLOW C.R. 245 SOUTH TO THE SOUTHERN BOUNDARY OF COLUMBIA COUNTY.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
   ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------- 110 MPH
2. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**GENERAL REQUIREMENTS:** Two (2) complete set of plans containing the following:

| Applicant | Plans Examiner | |
|---|---|---|
| ✓ | ✓ | All drawings must be clear, concise and drawn to scale("Optional"details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans |
| ✓ | ✓ | Designer's name and signature on document(FBC 104.2.1) If licensed architect or engineer, official seal shall be affixed |
| ✓ | ✓ | **Site Plan including:** |
| | | a) Dimensions of lot |
| | | b) Dimensions of building setbacks |
| | | c) Location of all other buildings on lot , well and septic tank if applicable, and all utility easements. |
| | | d) Provide a full legal description of property |
| ✓ | ✓ | **Wind-load Engineering Summary, calculations and any details required** |
| | | a) Plans or specifications must state compliance with FBC Section 1606 |
| | | b) The following information must be shown as per section 1606 1.7 FBC |
| | | a. Basic wind speed (MPH) |
| | | b. Wind importance factor (I) and building category |
| | | c. Wind exposure - if more than one wind expore is used , the wind exposure and applicable wind direction shall be indicated |
| | | d. The applicable internal pressure coefficient |
| | | e. Components and Cladding. The design wind pressure in terms of psf ($kN/m^2$), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional |
| ✓ | ✓ | **Elevations including:** |
| ✓ | ✓ | a) All Sides |
| ✓ | ✓ | b) Roof pitch |
| ✓ | ✓ | c) Overhang dimensions and detail with attic ventilation |

d) Location, size and height above roof of chimneys
e) Location and size of skylights
d) Building height
e) Number of stories

**Floor Plan including:** 1 + BONUS ROOM

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and Doors(including garage doors) showing size, mfg, approval listing and attachmenspecs.(FBC1707)and safety glazing where needed (egress windows in bedrooms to be shown) — See NOTE 2 + 3
d) Fireplaces (gas appliance(vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails — See NOTE 4
f) Must show and identify accessability requirements ( accessible bathroom )

**Foundation Plan including:**

a) Location of all load bearing walls with required footings indicated as standard or monolithic and their dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing — See NOTE 5 + 6
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

**Roof System**

a) Truss package including: — See NOTE 6+7
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating )

b) Conventional Framing Layout including — See NOTE 8
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge Beam sized and valley framing and support details
   4. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

**Wall Sections including:**

a) Masonry wall
   1. All materials making up wall
   2. Block size and motar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinenforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation
   6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction ( if required )
   8. Fireproofing requirements
   9. Show type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6 mil. polyethylene with joints lapped 6 inches and sealed )
      b. Must show control joints, synthetic fiber reinforcement or

welded wire fabric reinfrocement and supports
11. Indicate where pressuretreated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

b) Wood Frame wall
1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fasting requirements and product evaluation with wind resistance rating)
8. Fire resistant construction ( if required )
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder ( 6 mil polyethylene with joints lapped 6 inches and sealed )
    b. Must show control joints, synthetic fiber reinenforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal Frame wall and roof (Designed, signed and sealed by Fl. Reg. Prof. Engineer or Architect)

**Floor Framing System**

a) Floor truss package including layout and details signed and sealed by Fl. Reg. P.E.
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

**Plumbing Fixture layout**

**Electrical layout including:**

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment

**HVAC information**

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathrooms

☑ ☐
☑ ☐

**Energy Calculations** (dimensions shall match plans)
**Gas System** Type (LP or Natural) Location and BTU demand of equipment
**Disclosure Statement for Owner Builders** — only if ~~only~~ owner is building owner
**Notice of Commencement**
**Private Potable Water**
a) Size of pump motor
b) Size of pressure tank
c) Cycle Stop Valve if used

| | | | |
|---|---|---|---|
| **Project Information for:** | L128497 | | |
| Builder: | DON REED | Date: | 9/8/2005 |
| Lot: | LOT 23 WOODBROUGH | Start Number: | 907 |
| Subdivision: | N/A | | |
| County or City: | COLUMBIA COUNTY | | |
| Truss Page Count: | 89 | | |

**Truss Design Load Information (UNO)** Design Program: MiTek 5.2 / 6.2

| Gravity | | Wind | | Building Code: | FBC2001 |
|---|---|---|---|---|---|
| Roof (psf): | 42 | Wind Standard: | ASCE 7-98 | | |
| Floor (psf): | 55 | Wind Speed (mph): | 110 | | |

Note: See individual truss drawings for special loading conditions

**Building Designer, responsible for Structural Engineering: (See attached)**

REED, LARRY DON CGC 036224
Address: 2230 E BAYA AVE. STE 101
GLEN ST MARY FL 32040 Designer: 84

**Truss Design Engineer:** Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

| # | Truss ID | Dwg. # | Seal Date | # | Truss ID | Dwg. # | Seal Date |
|---|---|---|---|---|---|---|---|
| 1 | T53 | 090805907 | 9/9/2005 | 41 | T12 | 090805947 | 9/9/2005 |
| 2 | T52 | 090805908 | 9/9/2005 | 42 | T10 | 090805948 | 9/9/2005 |
| 3 | T51 | 090805909 | 9/9/2005 | 43 | T09 | 090805949 | 9/9/2005 |
| 4 | T50 | 090805910 | 9/9/2005 | 44 | T08 | 090805950 | 9/9/2005 |
| 5 | T49 | 090805911 | 9/9/2005 | 45 | T07G | 090805951 | 9/9/2005 |
| 6 | T48 | 090805912 | 9/9/2005 | 46 | T06 | 090805952 | 9/9/2005 |
| 7 | T47 | 090805913 | 9/9/2005 | 47 | T05 | 090805953 | 9/9/2005 |
| 8 | T46 | 090805914 | 9/9/2005 | 48 | T04 | 090805954 | 9/9/2005 |
| 9 | T45 | 090805915 | 9/9/2005 | 49 | T03 | 090805955 | 9/9/2005 |
| 10 | T44 | 090805916 | 9/9/2005 | 50 | T02 | 090805956 | 9/9/2005 |
| 11 | T43 | 090805917 | 9/9/2005 | 51 | T01 | 090805957 | 9/9/2005 |
| 12 | T42 | 090805918 | 9/9/2005 | 52 | PB3 | 090805958 | 9/9/2005 |
| 13 | T41 | 090805919 | 9/9/2005 | 53 | PB2 | 090805959 | 9/9/2005 |
| 14 | T40 | 090805920 | 9/9/2005 | 54 | PB1 | 090805960 | 9/9/2005 |
| 15 | T39 | 090805921 | 9/9/2005 | 55 | HJ7A | 090805961 | 9/9/2005 |
| 16 | T38 | 090805922 | 9/9/2005 | 56 | HJ7 | 090805962 | 9/9/2005 |
| 17 | T37 | 090805923 | 9/9/2005 | 57 | HJ6C | 090805963 | 9/9/2005 |
| 18 | T36 | 090805924 | 9/9/2005 | 58 | HJ6 | 090805964 | 9/9/2005 |
| 19 | T35 | 090805925 | 9/9/2005 | 59 | HJ5D | 090805965 | 9/9/2005 |
| 20 | T34 | 090805926 | 9/9/2005 | 60 | HJ5C | 090805966 | 9/9/2005 |
| 21 | T33 | 090805927 | 9/9/2005 | 61 | HJ5B | 090805967 | 9/9/2005 |
| 22 | T32 | 090805928 | 9/9/2005 | 62 | HJ5A | 090805968 | 9/9/2005 |
| 23 | T31 | 090805929 | 9/9/2005 | 63 | HJ4D | 090805969 | 9/9/2005 |
| 24 | T30 | 090805930 | 9/9/2005 | 64 | HJ3C | 090805970 | 9/9/2005 |
| 25 | T29 | 090805931 | 9/9/2005 | 65 | EJ7A | 090805971 | 9/9/2005 |
| 26 | T28 | 090805932 | 9/9/2005 | 66 | EJ7 | 090805972 | 9/9/2005 |
| 27 | T27G | 090805933 | 9/9/2005 | 67 | EJ6C | 090805973 | 9/9/2005 |
| 28 | T27 | 090805934 | 9/9/2005 | 68 | EJ6B | 090805974 | 9/9/2005 |
| 29 | T26 | 090805935 | 9/9/2005 | 69 | EJ6A | 090805975 | 9/9/2005 |
| 30 | T25 | 090805936 | 9/9/2005 | 70 | EJ6 | 090805976 | 9/9/2005 |
| 31 | T24 | 090805937 | 9/9/2005 | 71 | EJ5D | 090805977 | 9/9/2005 |
| 32 | T23 | 090805938 | 9/9/2005 | 72 | EJ5C | 090805978 | 9/9/2005 |
| 33 | T22 | 090805939 | 9/9/2005 | 73 | EJ5B | 090805979 | 9/9/2005 |
| 34 | T21 | 090805940 | 9/9/2005 | 74 | EJ5A | 090805980 | 9/9/2005 |
| 35 | T20 | 090805941 | 9/9/2005 | 75 | EJ5 | 090805981 | 9/9/2005 |
| 36 | T19 | 090805942 | 9/9/2005 | 76 | CJ6C | 090805982 | 9/9/2005 |
| 37 | T18 | 090805943 | 9/9/2005 | 77 | CJ5D | 090805983 | 9/9/2005 |
| 38 | T17 | 090805944 | 9/9/2005 | 78 | CJ5A | 090805984 | 9/9/2005 |
| 39 | T16 | 090805945 | 9/9/2005 | 79 | CJ5 | 090805985 | 9/9/2005 |
| 40 | T15 | 090805946 | 9/9/2005 | 80 | CJ3D | 090805986 | 9/9/2005 |

SEP 0 8 2005

| | | | |
|---|---|---|---|
| **Project Information for:** | L128497 | | |
| Builder: | DON REED | Date: | 9/8/2005 |
| Lot: | LOT 23 WOODBROUGH | Start Number: | 907 |
| Subdivision: | N/A | | |
| County or City: | COLUMBIA COUNTY | | |
| Truss Page Count: | 89 | | |

**Truss Design Load Information (UNO)** Design Program: MiTek 5.2 / 6

| Gravity | | Wind | | Building Code: | FBC2001 |
|---|---|---|---|---|---|
| Roof (psf): | 42 | Wind Standard: | ASCE 7-98 | | |
| Floor (psf): | 55 | Wind Speed (mph): | 110 | | |

Note: See individual truss drawings for special loading conditions

**Building Designer, responsible for Structural Engineering: (See attached)**
REED, LARRY DON CGC 036224
Address: 2230 E BAYA AVE. STE 101
GLEN ST MARY FL 32040

**Truss Design Engineer:** Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

| # | Truss ID | Dwg. # | Seal Date | # | Truss ID | Dwg. # | Seal Date |
|---|---|---|---|---|---|---|---|
| 81 | CJ3B | 090805987 | 9/9/2005 | | | | |
| 82 | CJ3A | 090805988 | 9/9/2005 | | | | |
| 83 | CJ3 | 090805989 | 9/9/2005 | | | | |
| 84 | CJ2D | 090805990 | 9/9/2005 | | | | |
| 85 | CJ2C | 090805991 | 9/9/2005 | | | | |
| 86 | CJ1C | 090805992 | 9/9/2005 | | | | |
| 87 | CJ1B | 090805993 | 9/9/2005 | | | | |
| 88 | CJ1A | 090805994 | 9/9/2005 | | | | |
| 89 | CJ1 | 090805995 | 9/9/2005 | | | | |

SEP 0 8 2005



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## Licensee Details

### Licensee Information

| | |
|---|---|
| Name: | **REED, LARRY DON (Primary Name)** |
| | **DON REED CONSTRUCTION INC (DBA Name)** |
| Main Address: | **2230 E BAYA AVE STE 101** |
| | **LAKE CITY Florida 32025** |
| County: | **COLUMBIA** |
| License Mailing: | |
| LicenseLocation: | **2230 E BAYA AVE STE 101** |
| | **LAKE CITY FL 32025** |
| County: | **COLUMBIA** |

### License Information

| | |
|---|---|
| License Type: | **Certified General Contractor** |
| Rank: | **Cert General** |
| License Number: | **CGC036224** |
| Status: | **Current,Active** |
| Licensure Date: | **03/08/1986** |
| Expires: | **08/31/2006** |
| **Special Qualifications** | **Qualification Effective** |
| **Bldg Code Core Course Credit** | |
| **Qualified Business License Required** | **08/13/2004** |

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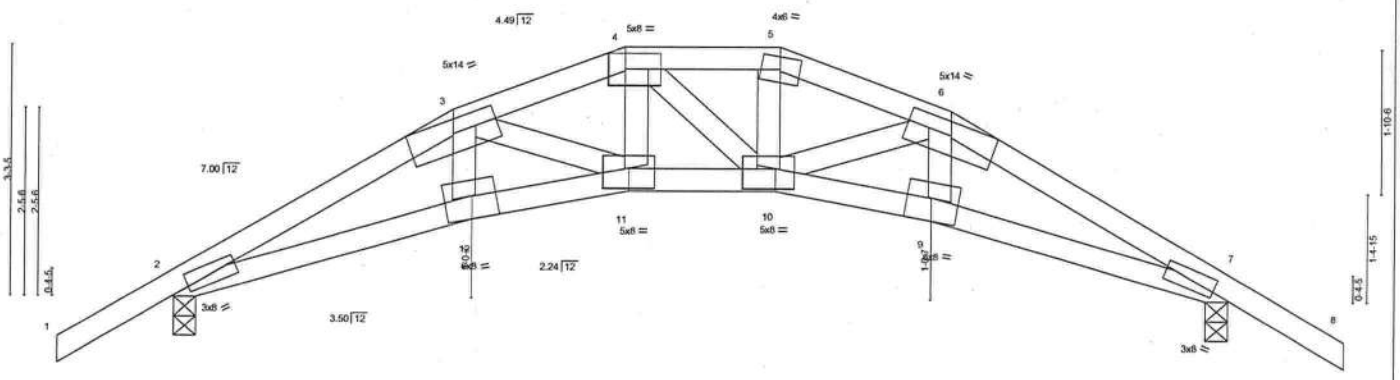
| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T53 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805907

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 11:02:13 2005 Page 1

| -1-6-0 | 3-7-0 | 5-9-8 | 7-9-8 | 10-0-0 | 13-7-0 | 15-1-0 |
|---|---|---|---|---|---|---|
| 1-6-0 | 3-7-0 | 2-2-8 | 2-0-0 | 2-2-8 | 3-7-0 | 1-6-0 |

Scale = 1:28.4




| 3-10-1 | 5-10-3 | 7-8-13 | 9-8-15 | 13-7-0 |
|---|---|---|---|---|
| 3-10-1 | 2-0-2 | 1-10-9 | 2-0-2 | 3-10-1 |

Plate Offsets (X,Y): [2:0-2-11,0-0-11], [3:0-7-0,0-2-1], [4:0-5-8,0-2-8], [6:0-7-0,0-2-1], [7:0-2-11,0-0-11], [10:0-5-0,0-0-8], [11:0-4-0,0-0-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.34 | Vert(LL) | -0.15 | 11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.66 | Vert(TL) | -0.24 | 10-11 | >675 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.29 | Horz(TL) | 0.19 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 65 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-12 oc bracing.

**REACTIONS** (lb/size) 2=1245/0-3-8, 7=1245/0-3-8
Max Horz 2=96(load case 4)
Max Uplift2=-678(load case 4), 7=-677(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3373/1771, 3-4=-3206/1782, 4-5=-3027/1720, 5-6=-3216/1774, 6-7=-3368/1705, 7-8=0/39
BOT CHORD 2-12=-1562/2918, 11-12=-1581/2971, 10-11=-1633/3015, 9-10=-1467/2965, 7-9=-1449/2913
WEBS 3-12=-208/491, 3-11=-180/161, 4-11=-465/918, 4-10=-62/90, 5-10=-449/917, 6-10=-195/179, 6-9=-195/484

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 678 lb uplift at joint 2 and 677 lb uplift at joint 7.
6) Girder carries hip end with 7-9-8 right side setback, 3-7-0 left side setback, and 4-7-0 end setback.
7) Girder carries hip end with 3-7-0 right side setback, 7-9-8 left side setback, and 4-7-0 end setback.
8) Girder carries hip end with 5-9-8 right side setback, 5-9-8 left side setback, and 5-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 155 lb down and 107 lb up at 3-10-1, 155 lb down and 107 lb up at 9-8-15, and 284 lb down and 197 lb up at 7-8-13, and 284 lb down and 197 lb up at 5-10-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

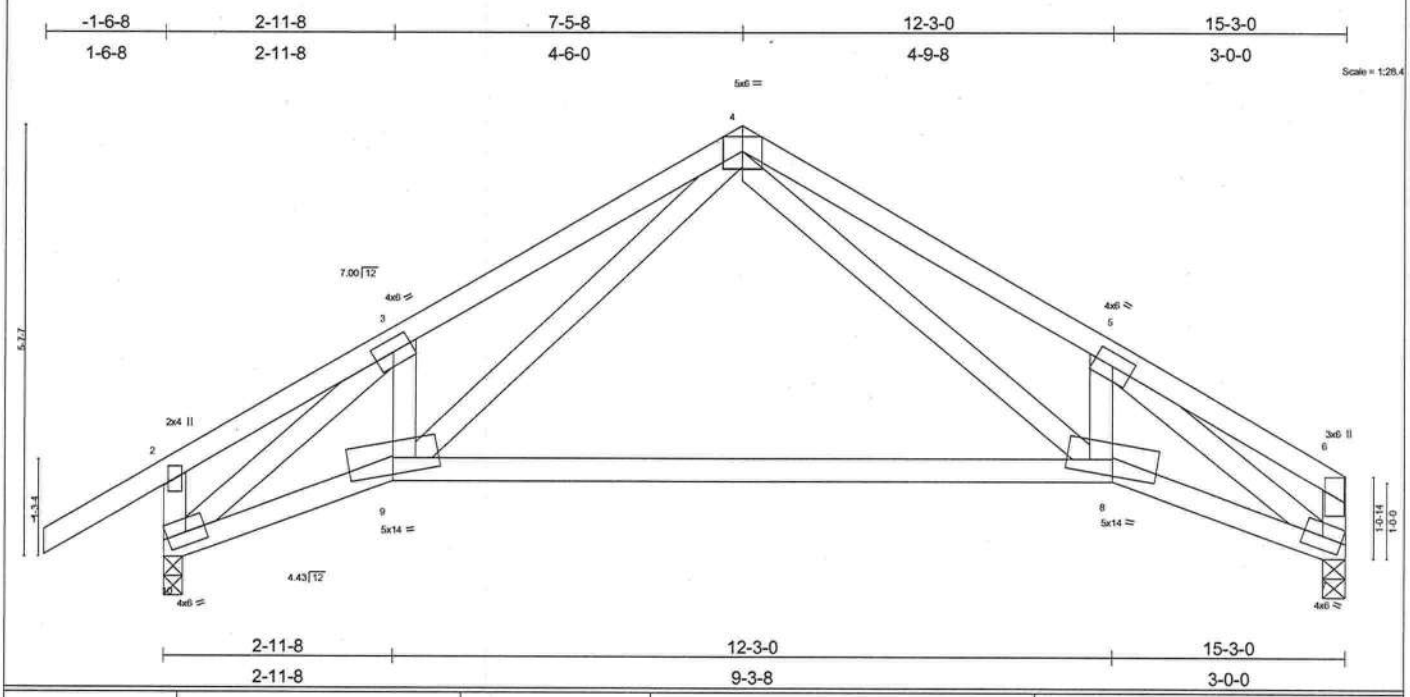
**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-86(F=-32), 4-5=-91(F=-37), 5-6=-85(F=-31), 6-8=-54, 2-12=-30, 11-12=-47(F=-17), 10-11=-50(F=-20), 9-10=-47(F=-17), 7-9=-30
Concentrated Loads (lb)
Vert: 12=-155(F) 9=-155(F) 11=-284(F) 10=-284(F)

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T52 | SPECIAL | 2 | 1 | Job Reference (optional) |

Dwg.#090805908

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 11:00:23 2005 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | 0.48 | 8-9 | >377 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.45 | Vert(TL) | 0.40 | 8-9 | >453 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.67 | Horz(TL) | -0.08 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 88 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W8 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-6-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-5 oc bracing.

**REACTIONS** (lb/size) 10=725/0-3-0, 7=623/0-3-8
Max Horz 10=221(load case 4)
Max Uplift10=-465(load case 5), 7=-370(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-90/162, 3-4=-1105/1359, 4-5=-1275/1531, 5-6=-202/239, 2-10=-221/265, 6-7=-183/199
BOT CHORD 9-10=-1090/1041, 8-9=-328/532, 7-8=-1241/1185
WEBS 3-9=-269/261, 4-9=-817/580, 4-8=-996/741, 5-8=-222/219, 3-10=-1261/1367, 5-7=-1273/1415

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 10, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 465 lb uplift at joint 10 and 370 lb uplift at joint 7.

**LOAD CASE(S)** Standard

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T51 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805909

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 11:00:03 2005 Page 1

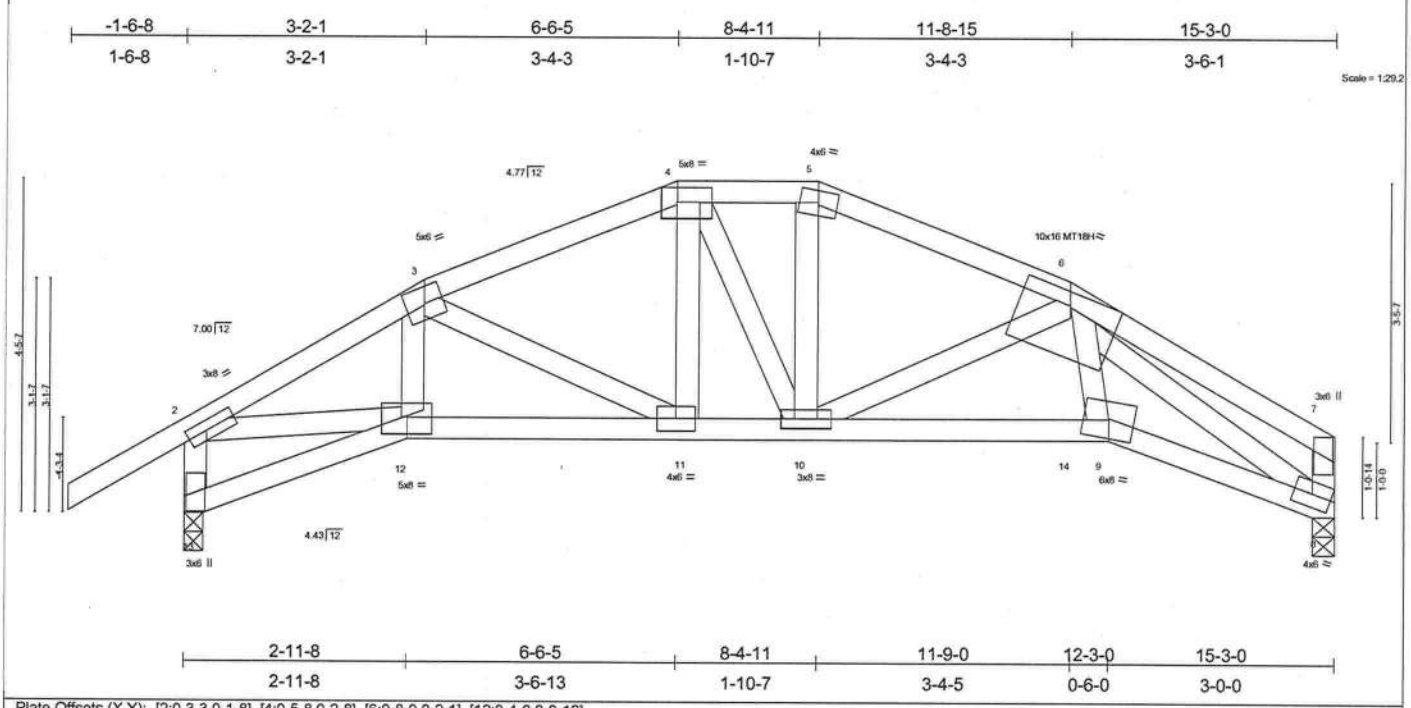



Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-5-8,0-2-8], [6:0-8-0,0-2-1], [12:0-4-0,0-0-12]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | 0.13 | 9-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.82 | Vert(TL) | -0.17 | 9-10 | >999 | 180 | MT18H | 244/190 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.80 | Horz(TL) | 0.13 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 92 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W9 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-11-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-4 oc bracing.

**REACTIONS** (lb/size) 13=1533/0-3-0, 8=1400/0-3-8
Max Horz 13=125(load case 4)
Max Uplift13=-1021(load case 4), 8=-913(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2561/1741, 3-4=-2292/1595, 4-5=-2173/1586, 5-6=-2343/1618, 6-7=-563/401, 2-13=-1488/1010, 7-8=-400/286
BOT CHORD 12-13=-153/84, 11-12=-1486/2135, 10-11=-1419/2112, 10-14=-1448/2233, 9-14=-1449/2233, 8-9=-1750/2688
WEBS 3-12=-188/268, 3-11=-76/128, 4-11=-327/499, 4-10=-130/201, 5-10=-373/577, 6-10=-117/154, 6-9=-732/1137, 2-12=-1375/2103, 6-8=-2784/1885

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 13, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1021 lb uplift at joint 13 and 913 lb uplift at joint 8.
7) Girder carries hip end with 8-8-11 right side setback, 3-2-1 left side setback, and 5-0-0 end setback.
8) Girder carries hip end with 3-6-0 right side setback, 8-4-11 left side setback, and 5-0-0 end setback.
9) Girder carries hip end with 6-10-5 right side setback, 6-6-5 left side setback, and 5-5-8 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 155 lb down and 108 lb up at 2-11-8, 172 lb down and 119 lb up at 11-9-0, and 380 lb down and 264 lb up at 8-4-11, and 362 lb down and 251 lb up at 6-6-5 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-4=-91(F=-37), 4-5=-97(F=-43), 5-6=-91(F=-37), 6-7=-54, 12-13=-30, 11-12=-51(F=-21), 10-11=-54(F=-24), 10-14=-51(F=-21), 9-14=-30, 8-9=-30
Concentrated Loads (lb)
Vert: 12=-155(F) 11=-362(F) 10=-380(F) 14=-172(F)


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T50 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805910

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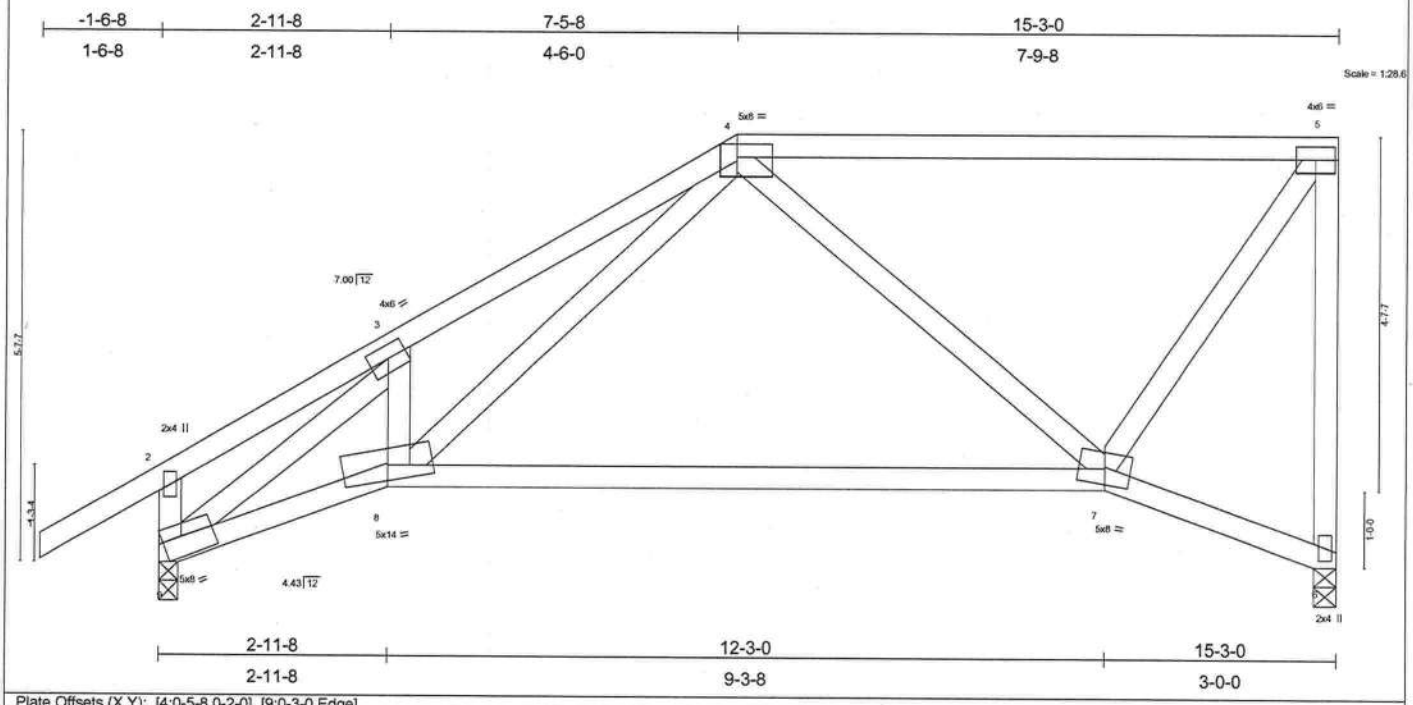


Plate Offsets (X,Y): [4:0-5-8,0-2-0], [9:0-3-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.85 | Vert(LL) | 0.43 | 7-8 | >419 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.44 | Vert(TL) | 0.36 | 7-8 | >499 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.58 | Horz(TL) | -0.05 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 92 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-9 oc bracing.

**REACTIONS** (lb/size) 6=623/0-3-8, 9=725/0-3-0
Max Horz 9=287(load case 5)
Max Uplift6=-414(load case 4), 9=-440(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-112/193, 3-4=-1059/1426, 4-5=-333/456, 5-6=-644/799, 2-9=-242/289
BOT CHORD 8-9=-1410/1012, 7-8=-580/557, 6-7=-35/71
WEBS 3-8=-315/241, 4-8=-885/475, 4-7=-301/282, 5-7=-782/539, 3-9=-1208/1431

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 6, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 414 lb uplift at joint 6 and 440 lb uplift at joint 9.

**LOAD CASE(S)** Standard


**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T49 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805911

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 10:55:43 2005 Page 1

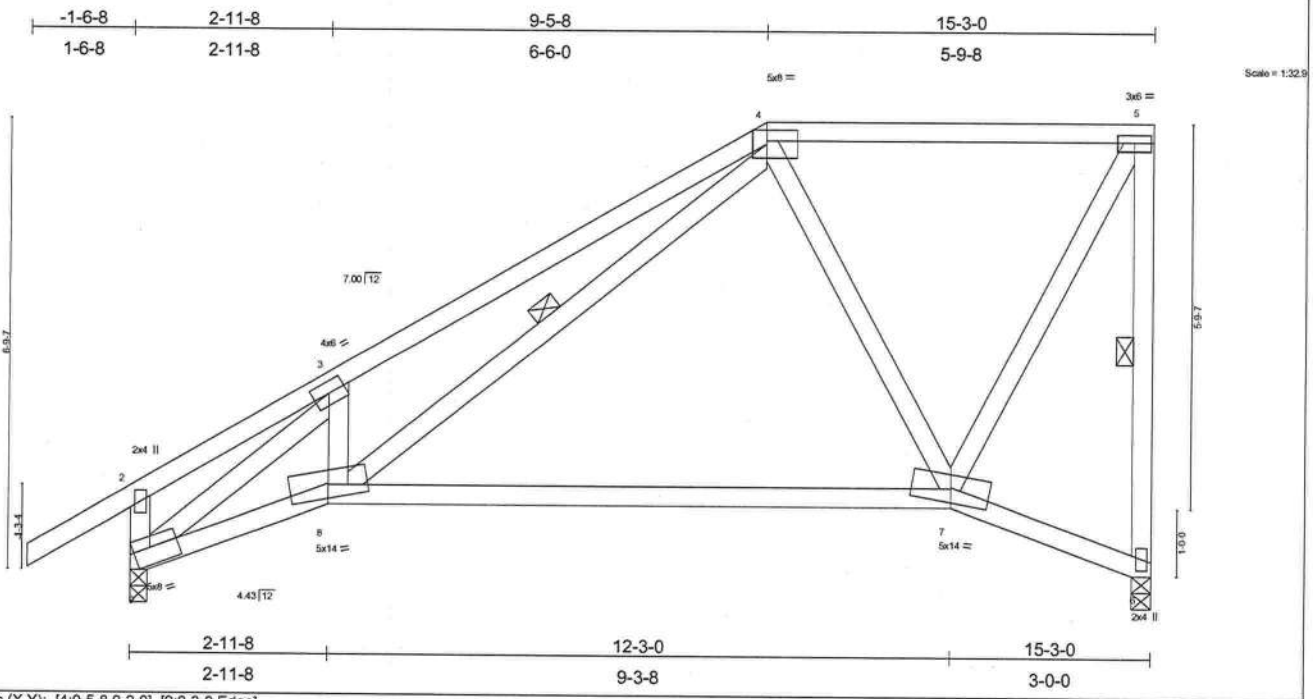


Plate Offsets (X,Y): [4:0-5-8,0-2-0], [9:0-3-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.37 | Vert(LL) | 0.45 | 7-8 | >399 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.45 | Vert(TL) | 0.38 | 7-8 | >476 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.55 | Horz(TL) | -0.06 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 99 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-4 oc bracing.
WEBS 1 Row at midpt 5-6, 4-8

**REACTIONS** (lb/size) 6=623/0-3-8, 9=725/0-3-0
Max Horz 9=341(load case 5)
Max Uplift6=-404(load case 4), 9=-437(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-53/142, 3-4=-1198/1533, 4-5=-281/383, 5-6=-649/818, 2-9=-175/229
BOT CHORD 8-9=-1502/1071, 7-8=-430/419, 6-7=-15/46
WEBS 3-8=-222/118, 4-8=-1162/718, 4-7=-303/291, 5-7=-824/593, 3-9=-1327/1517

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 6, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 6 and 437 lb uplift at joint 9.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T48 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805912

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 10:55:21 2005 Page 1

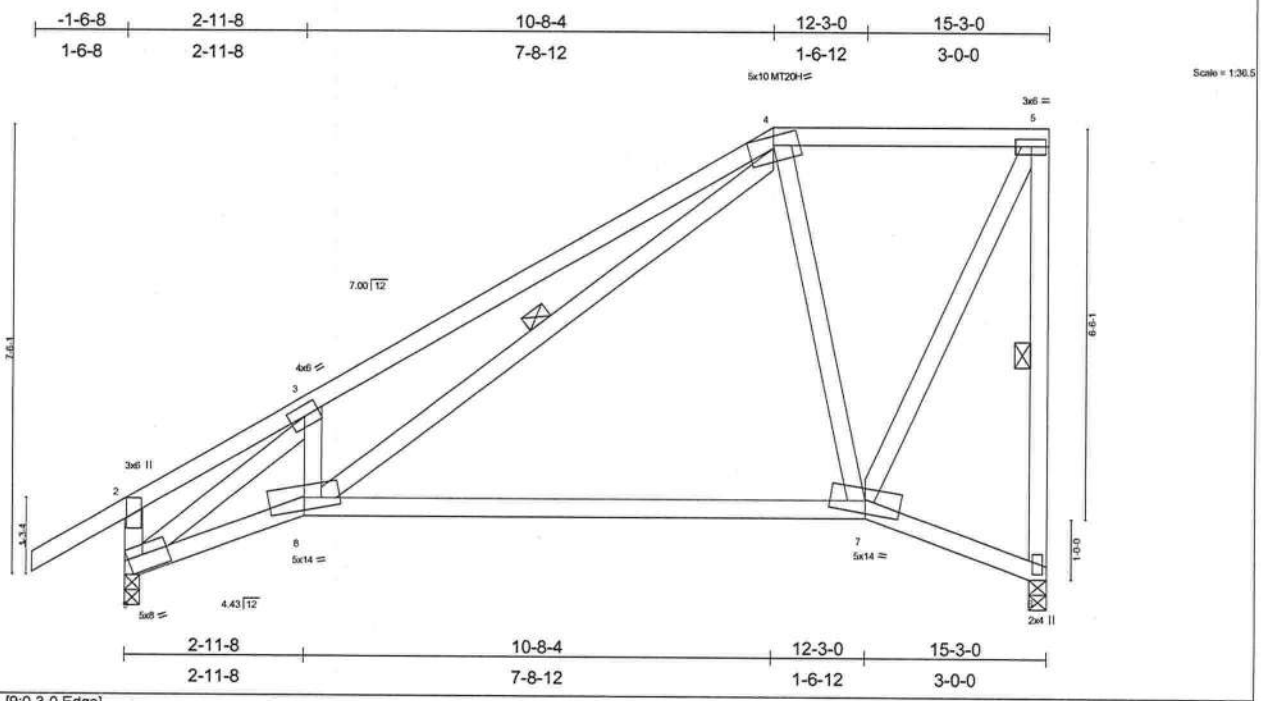


Plate Offsets (X,Y): [9:0-3-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.41 | Vert(LL) | 0.47 | 7-8 | >385 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | 0.39 | 7-8 | >459 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.71 | Horz(TL) | -0.07 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 104 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-3-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-9 oc bracing.
WEBS 1 Row at midpt 5-6, 4-8

**REACTIONS** (lb/size) 6=623/0-3-8, 9=725/0-3-0
Max Horz 9=374(load case 5)
Max Uplift6=-427(load case 5), 9=-430(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-26/105, 3-4=-1270/1591, 4-5=-270/363, 5-6=-653/832, 2-9=-129/187
BOT CHORD 8-9=-1560/1111, 7-8=-358/353, 6-7=-9/42
WEBS 3-8=-206/167, 4-7=-339/329, 5-7=-877/650, 3-9=-1409/1576, 4-8=-1309/844

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 6, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 6 and 430 lb uplift at joint 9.

**LOAD CASE(S)** Standard

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T47 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805913

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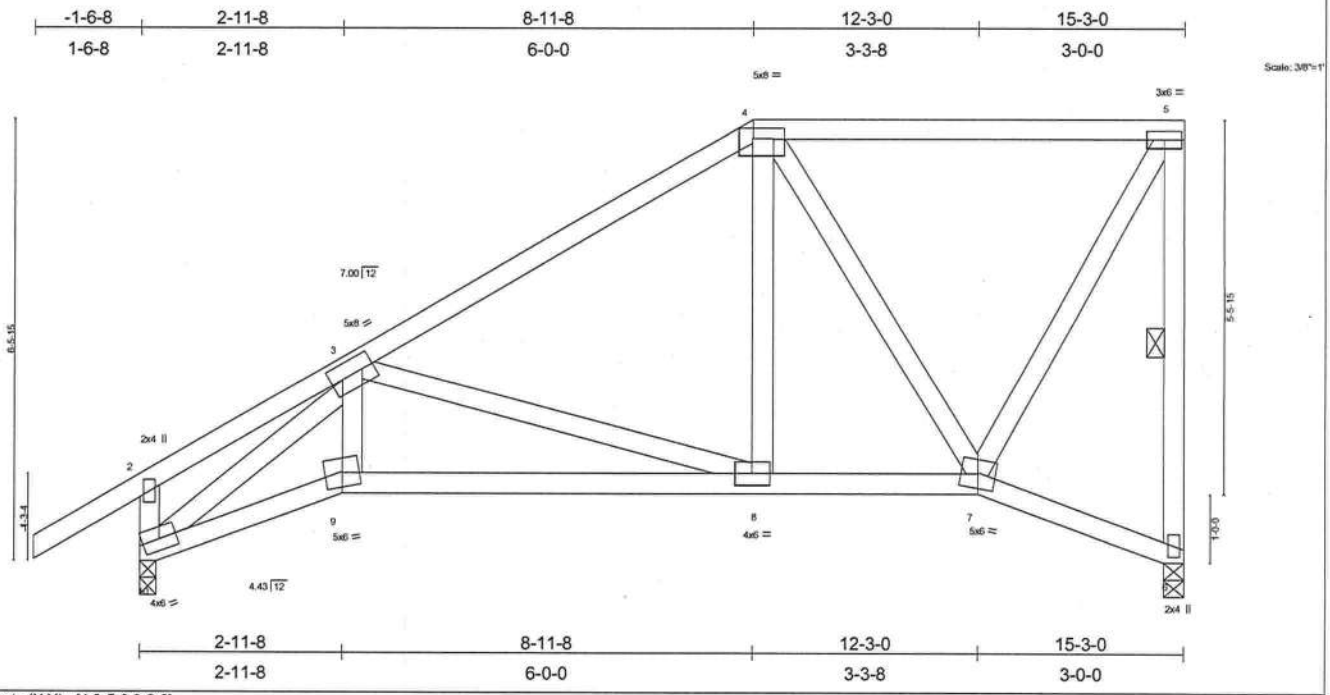


Plate Offsets (X,Y): [4:0-5-8,0-2-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.36 | Vert(LL) | 0.14 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.24 | Vert(TL) | 0.12 | 8-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.40 | Horz(TL) | -0.05 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 102 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-9 oc bracing.
WEBS 1 Row at midpt 5-6

**REACTIONS** (lb/size) 6=623/0-3-8, 10=725/0-3-0
Max Horz 10=327(load case 5)
Max Uplift6=-407(load case 4), 10=-438(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-97/204, 3-4=-627/613, 4-5=-254/335, 5-6=-571/686, 2-10=-202/261
BOT CHORD 9-10=-1342/975, 8-9=-1151/865, 7-8=-532/490, 6-7=-5/32
WEBS 3-9=-737/455, 3-8=-399/659, 4-8=-480/289, 4-7=-442/526, 5-7=-658/485, 3-10=-1176/1292

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 6, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 407 lb uplift at joint 6 and 438 lb uplift at joint 10.

**LOAD CASE(S)** Standard

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T46 | MONO HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805914

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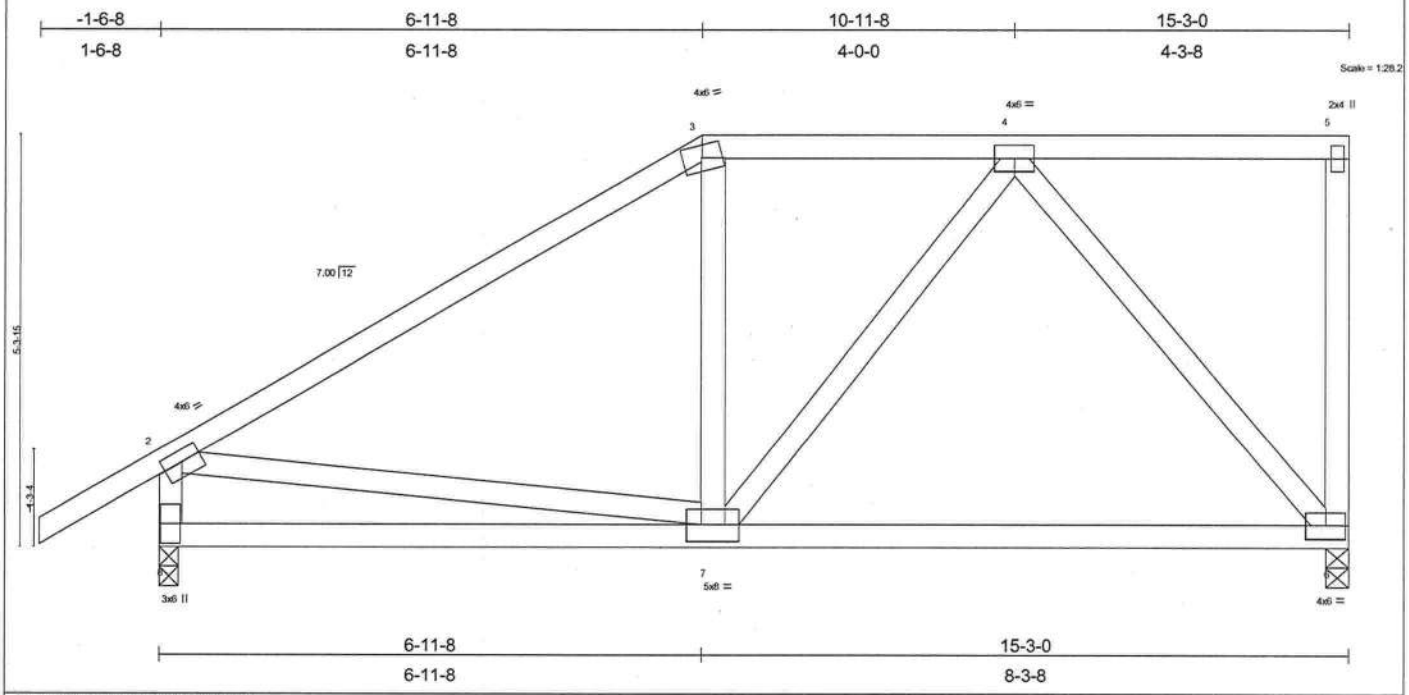


Plate Offsets (X,Y): [2:0-3-0,0-1-12]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.41 | Vert(LL) | -0.07 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.24 | Vert(TL) | -0.13 | 6-7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.38 | Horz(TL) | 0.01 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 93 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 6=623/0-3-8, 8=725/0-3-0
Max Horz 8=273(load case 5)
Max Uplift6=-228(load case 4), 8=-253(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-695/218, 3-4=-516/257, 4-5=-36/3, 5-6=-116/82, 2-8=-628/320
BOT CHORD 7-8=-340/239, 6-7=-193/359
WEBS 3-7=-11/132, 4-7=-131/258, 4-6=-509/299, 2-7=-157/280

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 6 and 253 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T45 | MONO HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805915

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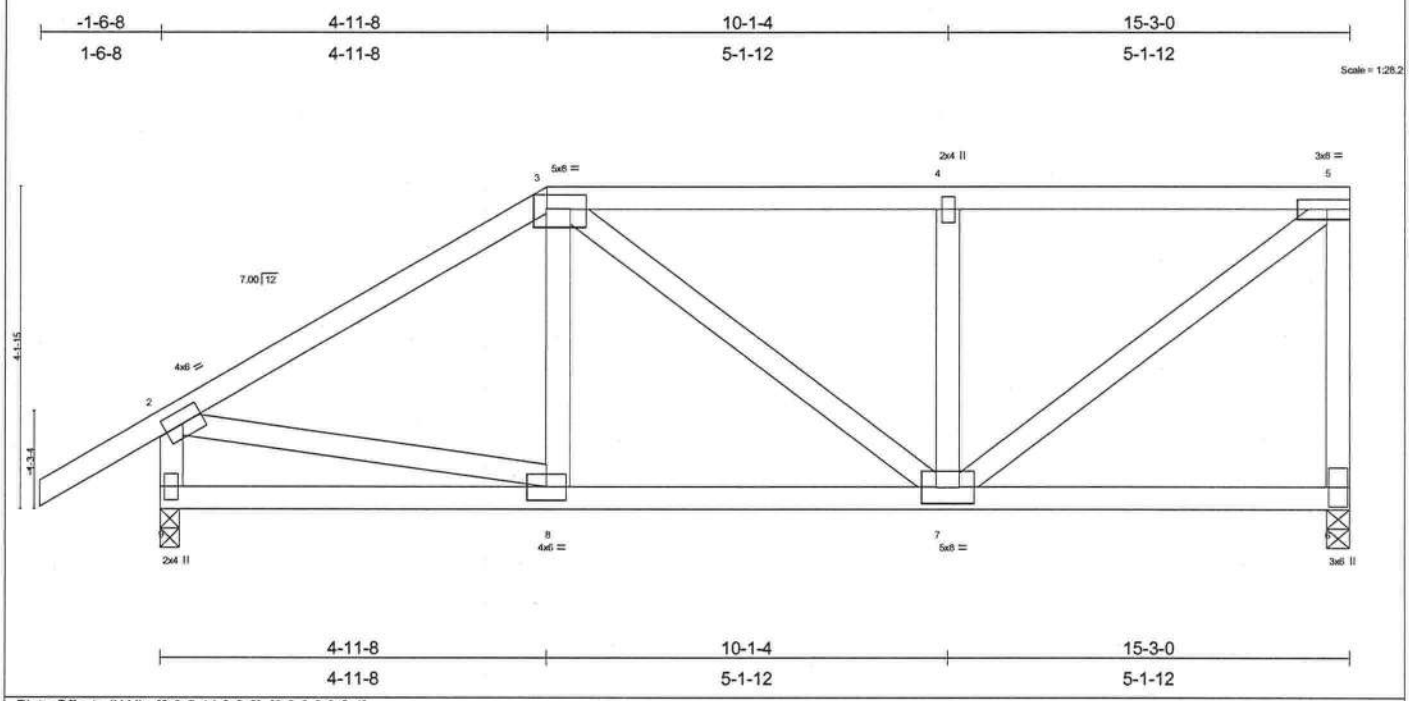


Plate Offsets (X,Y): [2:0-2-14,0-2-0], [3:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.59 | Vert(LL) | -0.05 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.31 | Vert(TL) | -0.08 | 7-8 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.43 | Horz(TL) | 0.01 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 91 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-11-2 oc bracing.

**REACTIONS** (lb/size) 6=1077/0-3-8, 9=1076/0-3-0
Max Horz 9=220(load case 4)
Max Uplift6=-567(load case 2), 9=-490(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-1238/569, 3-4=-1021/528, 4-5=-1021/528, 5-6=-954/580, 2-9=-1002/501
BOT CHORD 8-9=-195/93, 7-8=-529/1024, 6-7=-29/55
WEBS 3-8=-63/240, 3-7=-94/87, 4-7=-486/509, 5-7=-631/1221, 2-8=-538/941

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 567 lb uplift at joint 6 and 490 lb uplift at joint 9.
4) Girder carries hip end with 0-0-0 right side setback, 4-11-8 left side setback, and 4-11-8 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 240 lb down and 167 lb up at 4-11-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-90(F=-36), 8-9=-30, 6-8=-50(F=-20)
Concentrated Loads (lb)
Vert: 8=-240(F)


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T44 | COMMON | 1 | 1 | Job Reference (optional) |

Dwg.#090805916

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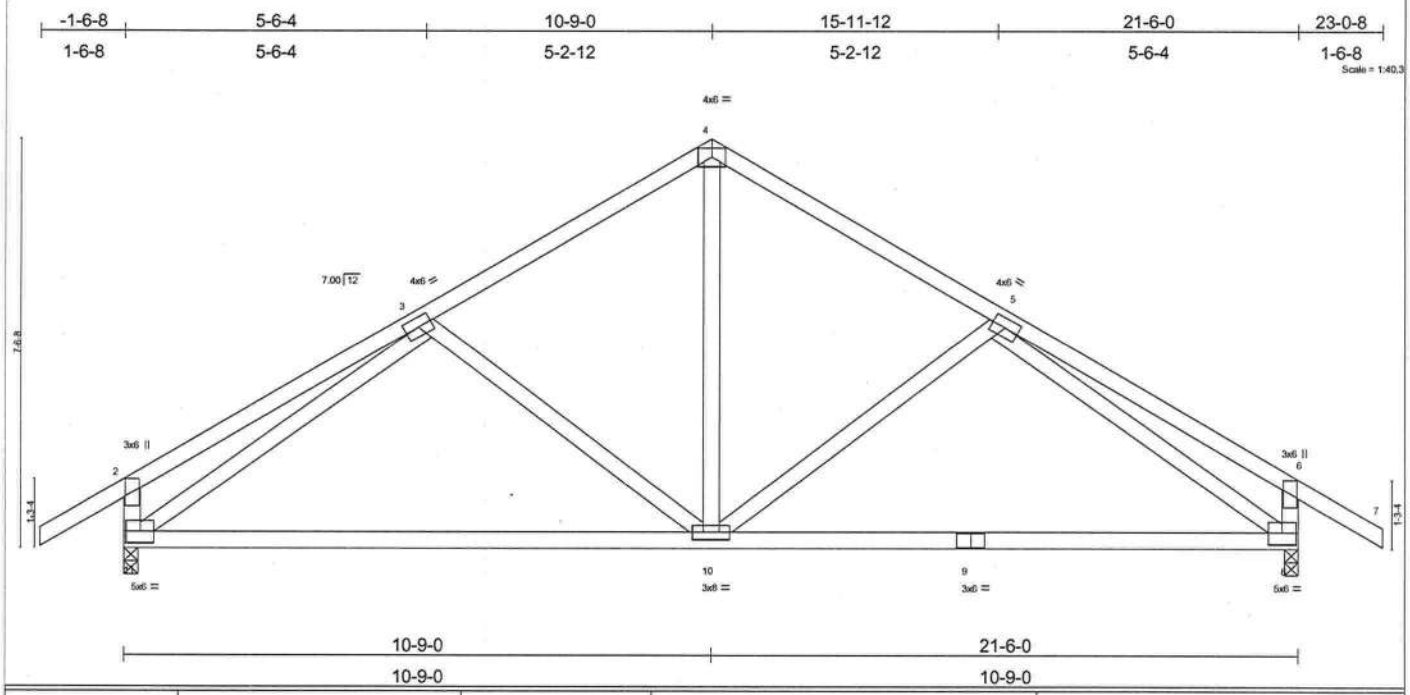


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.44 | Vert(LL) | -0.15 | 10-11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.47 | Vert(TL) | -0.27 | 10-11 | >946 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.51 | Horz(TL) | 0.03 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 126 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 11=982/0-3-0, 8=982/0-3-0
Max Horz 11=273(load case 4)
Max Uplift11=-356(load case 5), 8=-356(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-450/141, 3-4=-882/378, 4-5=-882/378, 5-6=-450/141, 6-7=0/46, 2-11=-430/257, 6-8=-430/257
BOT CHORD 10-11=-285/829, 9-10=-169/829, 8-9=-169/829
WEBS 3-10=-198/240, 4-10=-178/517, 5-10=-198/241, 3-11=-642/293, 5-8=-642/293

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 11 and 356 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T43 | SPECIAL | 3 | 1 | Job Reference (optional) |

Dwg.#090805917

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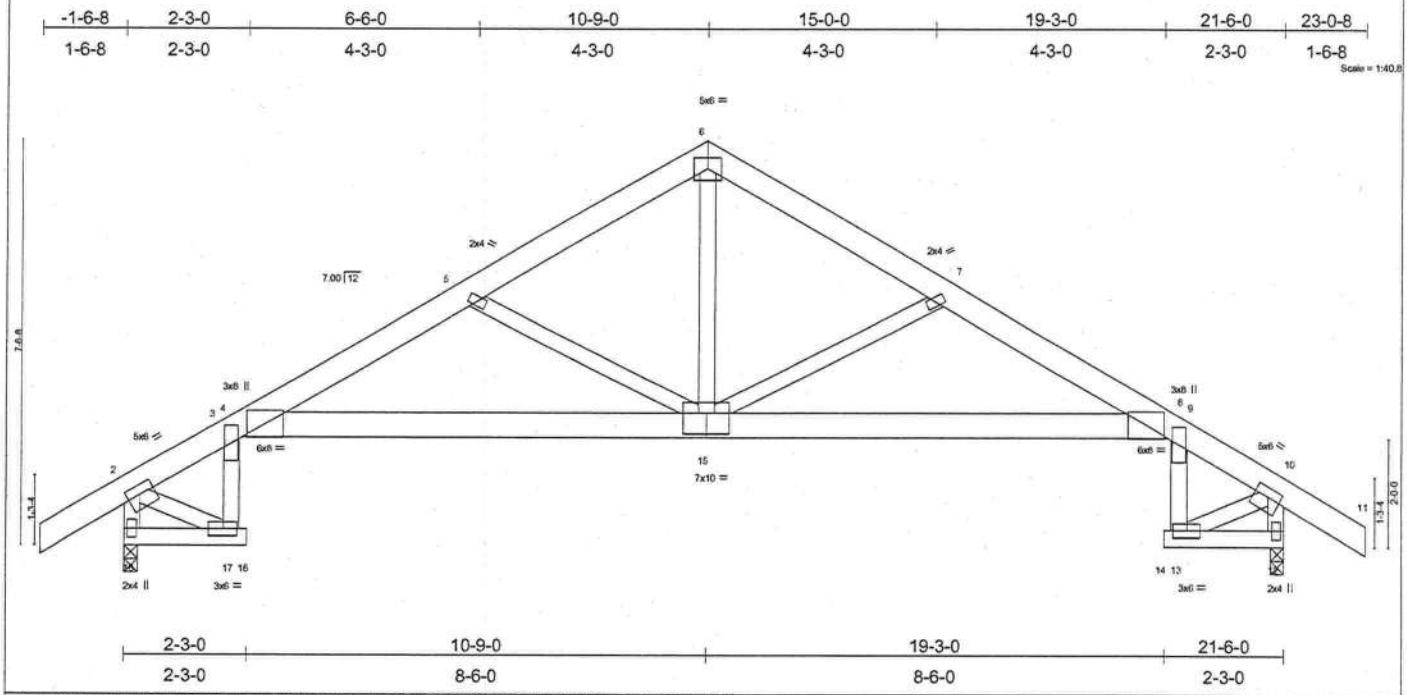


Plate Offsets (X,Y): [2:0-2-12,0-2-0], [4:0-0-15,0-0-0], [8:0-0-15,0-0-0], [10:0-2-12,0-2-0], [15:0-5-0,0-4-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.98 | Vert(LL) | -0.28 | 8-15 | >915 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.88 | Vert(TL) | -0.45 | 8-15 | >566 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.34 | Horz(TL) | 0.64 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 147 lb | |

**LUMBER**
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2D, B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3, B1 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 18=984/0-3-0, 12=984/0-3-0
Max Horz 18=1462(load case 1), 12=-1462(load case 1)
Max Uplift18=-356(load case 5), 12=-356(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-602/267, 3-4=-275/288, 4-5=-1702/529, 5-6=-1278/408, 6-7=-1278/408, 7-8=-1702/529, 8-9=-276/235, 9-10=-602/267, 10-11=0/46, 2-18=-988/417, 10-12=-988/417
BOT CHORD 17-18=-275/322, 16-17=0/0, 3-17=-129/160, 4-15=-465/1554, 8-15=-285/1554, 9-13=0/88, 13-14=0/0, 12-13=-22/202
WEBS 5-15=-602/360, 6-15=-253/1075, 7-15=-602/285, 2-17=-269/241, 10-13=-45/0

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 12.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 18 and 356 lb uplift at joint 12.

**LOAD CASE(S)** Standard

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805918 |
|---|---|---|---|---|---|---|
| L128497 | T42 | SPECIAL | 1 | 1 | Job Reference (optional) | |

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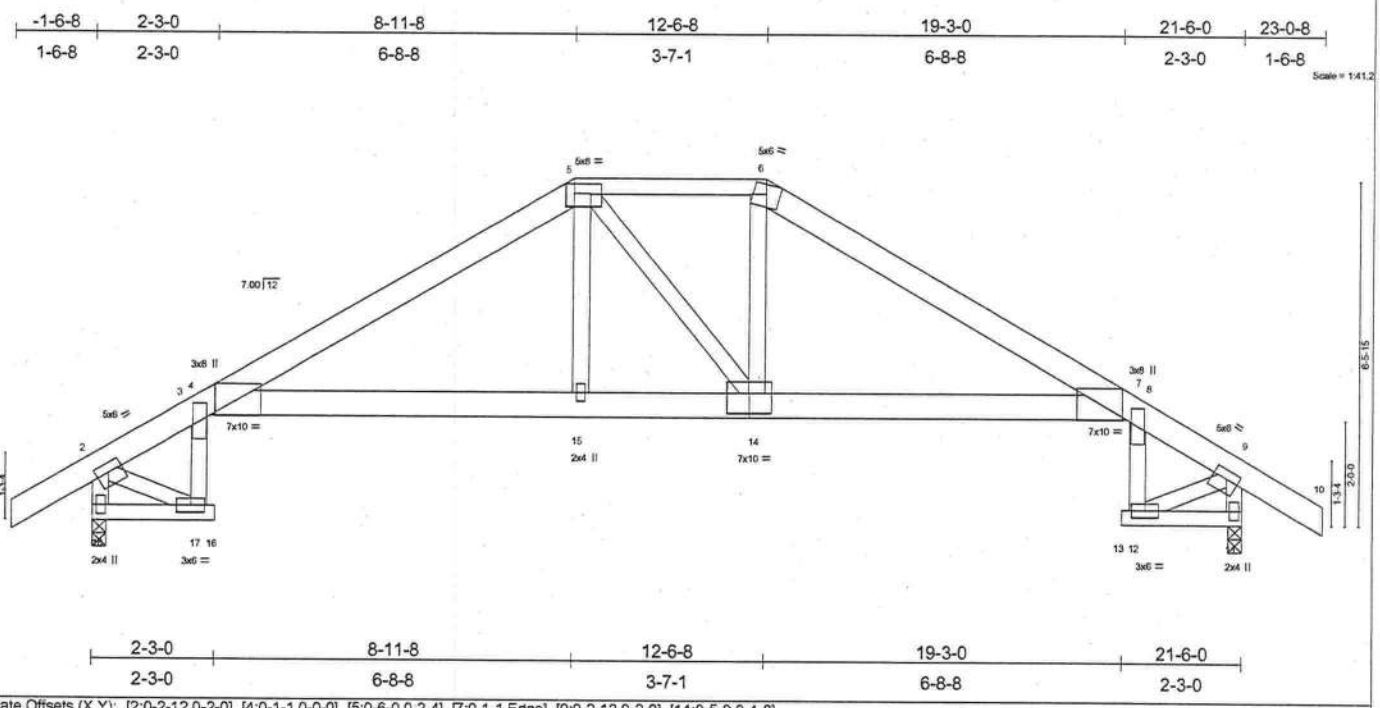


Plate Offsets (X,Y): [2:0-2-12,0-2-0], [4:0-1-1,0-0-0], [5:0-6-0,0-2-4], [7:0-1-1,Edge], [9:0-2-12,0-2-0], [14:0-5-0,0-4-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.98 | Vert(LL) | -0.32 | 4-15 | >797 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.73 | Vert(TL) | -0.51 | 4-15 | >496 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.12 | Horz(TL) | 0.74 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 140 lb | |

**LUMBER**
TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 6 SYP No.1D, B4 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 18=984/0-3-0, 11=984/0-3-0
Max Horz18=1462(load case 1), 11=-1462(load case 1)
Max Uplift18=-343(load case 5), 11=-343(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-615/274, 3-4=-275/261, 4-5=-1456/448, 5-6=-1252/466, 6-7=-1458/447, 7-8=-276/238, 8-9=-615/274, 9-10=0/46, 2-18=-991/424, 9-11=-991/424
BOT CHORD 17-18=-256/262, 16-17=0/0, 3-17=-112/132, 4-15=-342/1238, 14-15=-345/1250, 7-14=-185/1240, 8-12=0/76, 12-13=0/0, 11-12=-15/195
WEBS 5-15=-90/353, 5-14=-241/243, 6-14=-100/378, 2-17=-219/200, 9-12=-45/0

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 18 and 343 lb uplift at joint 11.

**LOAD CASE(S)** Standard


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805919 |
|---|---|---|---|---|---|---|
| L128497 | T41 | SPECIAL | 1 | 2 | Job Reference (optional) | |

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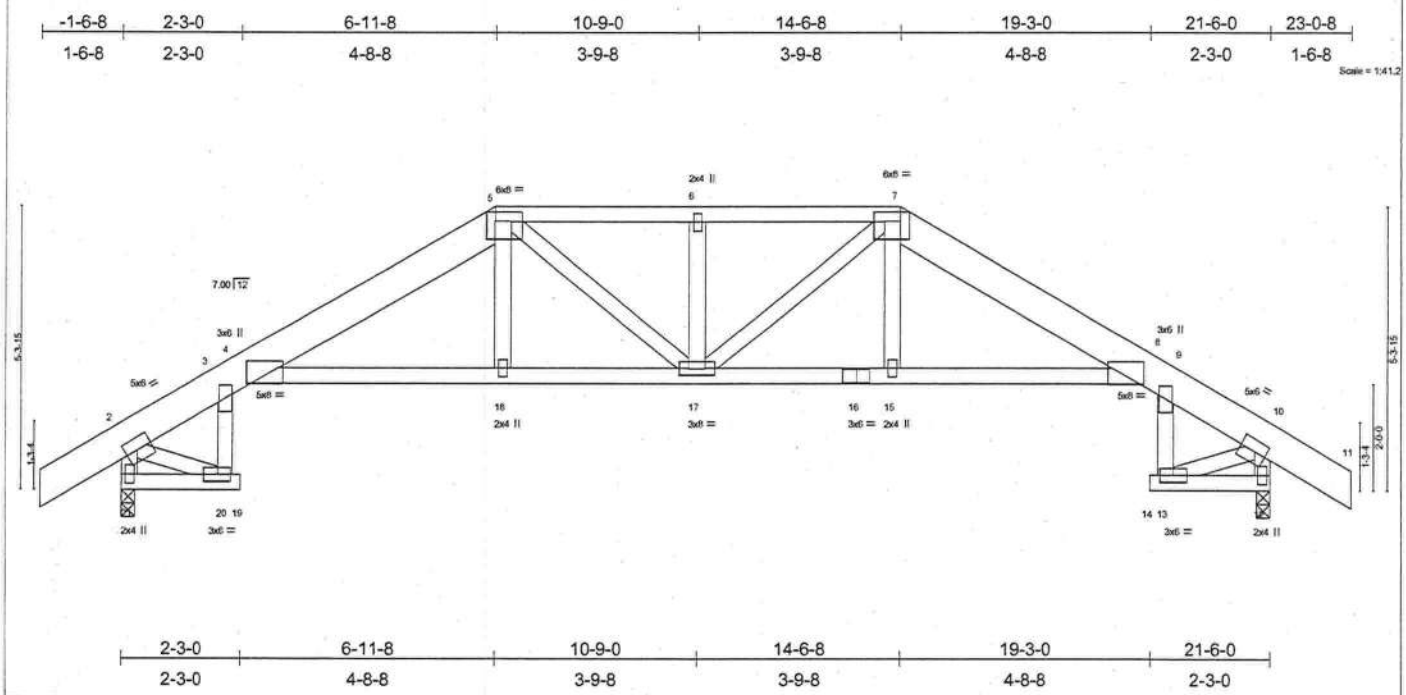





Plate Offsets (X,Y): [2:0-2-9,0-2-8], [5:0-6-0,0-2-4], [7:0-6-0,0-2-4], [10:0-2-9,0-2-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.83 | Vert(LL) | 0.22 | 4-18 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.58 | Vert(TL) | -0.35 | 8-15 | >727 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.14 | Horz(TL) | 0.52 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 282 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.1D, B4 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 21=1842/0-3-0, 12=1842/0-3-0
Max Horz 21=1462(load case 1), 12=-1462(load case 1)
Max Uplift21=-941(load case 4), 12=-941(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-1087/589, 3-4=-265/314, 4-5=-3933/2138, 5-6=-3792/2099, 6-7=-3792/2099, 7-8=-3933/2054, 8-9=-265/262, 9-10=-1087/591, 10-11=0/46, 2-21=-1844/968, 10-12=-1844/958
BOT CHORD 20-21=-307/296, 19-20=0/0, 3-20=-109/98, 4-18=-2060/3615, 17-18=-2081/3659, 16-17=-1918/3659, 15-16=-1918/3659, 8-15=-1898/3615, 9-13=-29/98, 13-14=0/0, 12-13=-133/296
WEBS 5-18=-429/897, 5-17=-264/253, 6-17=-72/288, 7-17=-237/253, 7-15=-412/897, 2-20=-121/199, 10-13=-94/45

**NOTES**
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 12.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 941 lb uplift at joint 21 and 941 lb uplift at joint 12.
8) Girder carries hip end with 6-11-8 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 532 lb down and 369 lb up at 14-6-8, and 532 lb down and 369 lb up at 6-11-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 4-5=-54, 5-7=-118(F=-64), 7-8=-54, 9-10=-54, 10-11=-54, 20-21=-30, 4-18=-30, 15-18=-65(F=-35), 8-15=-30, 12-13=-30
Concentrated Loads (lb)
Vert: 18=-532(F) 15=-532(F)

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. Dwg.#090805920 |
|---|---|---|---|---|---|
| L128497 | T40 | SPECIAL | 1 | 1 | Job Reference (optional) |

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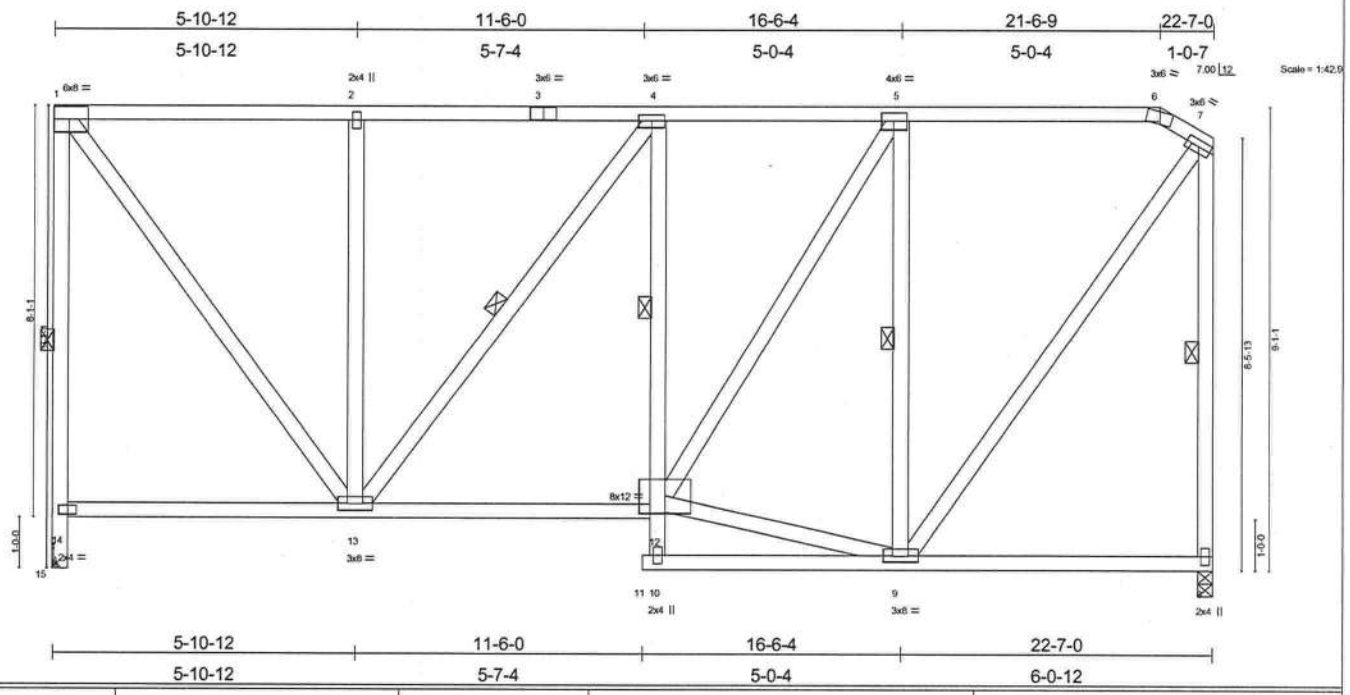


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.39 | Vert(LL) | -0.06 | 12-13 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.22 | Vert(TL) | -0.09 | 12-13 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.70 | Horz(TL) | 0.04 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 191 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
1 Row at midpt 4-12
WEBS 1 Row at midpt 1-15, 4-13, 5-9, 7-8

**REACTIONS** (lb/size) 15=940/Mechanical, 8=941/0-3-8
Max Horz 15=-25(load case 6)
Max Uplift15=-441(load case 4), 8=-393(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 14-15=-940/441, 1-14=-860/452, 1-2=-549/257, 2-3=-549/257, 3-4=-549/257, 4-5=-707/329, 5-6=-486/224, 6-7=-511/246, 7-8=-859/407
BOT CHORD 13-14=-10/24, 12-13=-335/717, 10-12=0/70, 4-12=-68/135, 10-11=0/0, 9-10=-25/14, 8-9=-2/6
WEBS 1-13=-419/897, 2-13=-337/292, 4-13=-274/125, 9-12=-208/487, 5-12=-199/421, 5-9=-610/405, 7-9=-385/823

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
4) Provide adequate drainage to prevent water ponding.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 15 and 393 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T39 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805921

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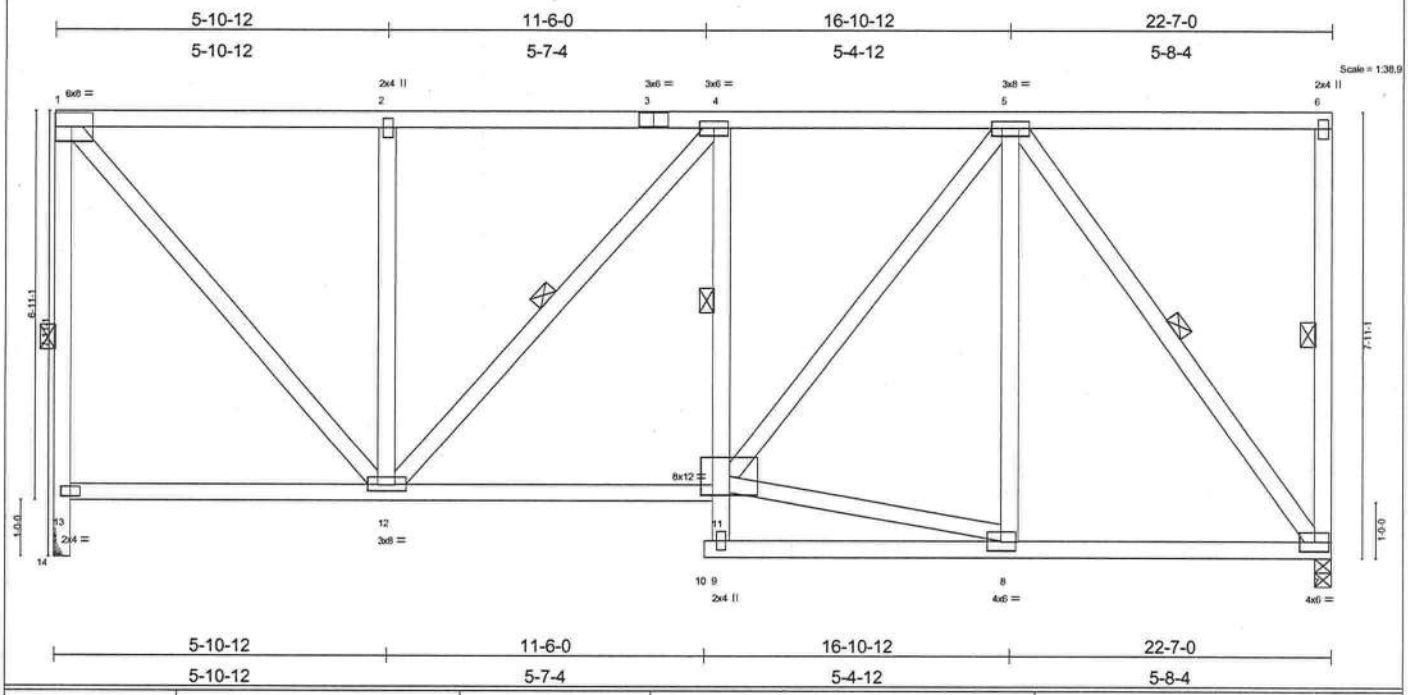


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | -0.05 | 11-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.24 | Vert(TL) | -0.09 | 11-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.49 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 178 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
1 Row at midpt 4-11
WEBS 1 Row at midpt 1-14, 6-7, 4-12, 5-7

**REACTIONS** (lb/size) 14=940/Mechanical, 7=941/0-3-8
Max Uplift14=-353(load case 3), 7=-353(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 13-14=-940/365, 1-13=-859/376, 1-2=-645/250, 2-3=-645/250, 3-4=-645/250, 4-5=-834/323, 5-6=-16/6, 6-7=-136/95
BOT CHORD 12-13=-9/21, 11-12=-326/846, 9-11=0/81, 4-11=-63/118, 9-10=0/0, 8-9=-19/29, 7-8=-210/543
WEBS 1-12=-368/952, 2-12=-335/246, 4-12=-297/114, 8-11=-195/527, 5-11=-182/469, 5-8=0/87, 5-7=-896/346

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 14 and 353 lb uplift at joint 7.

**LOAD CASE(S)** Standard


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T38 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805922

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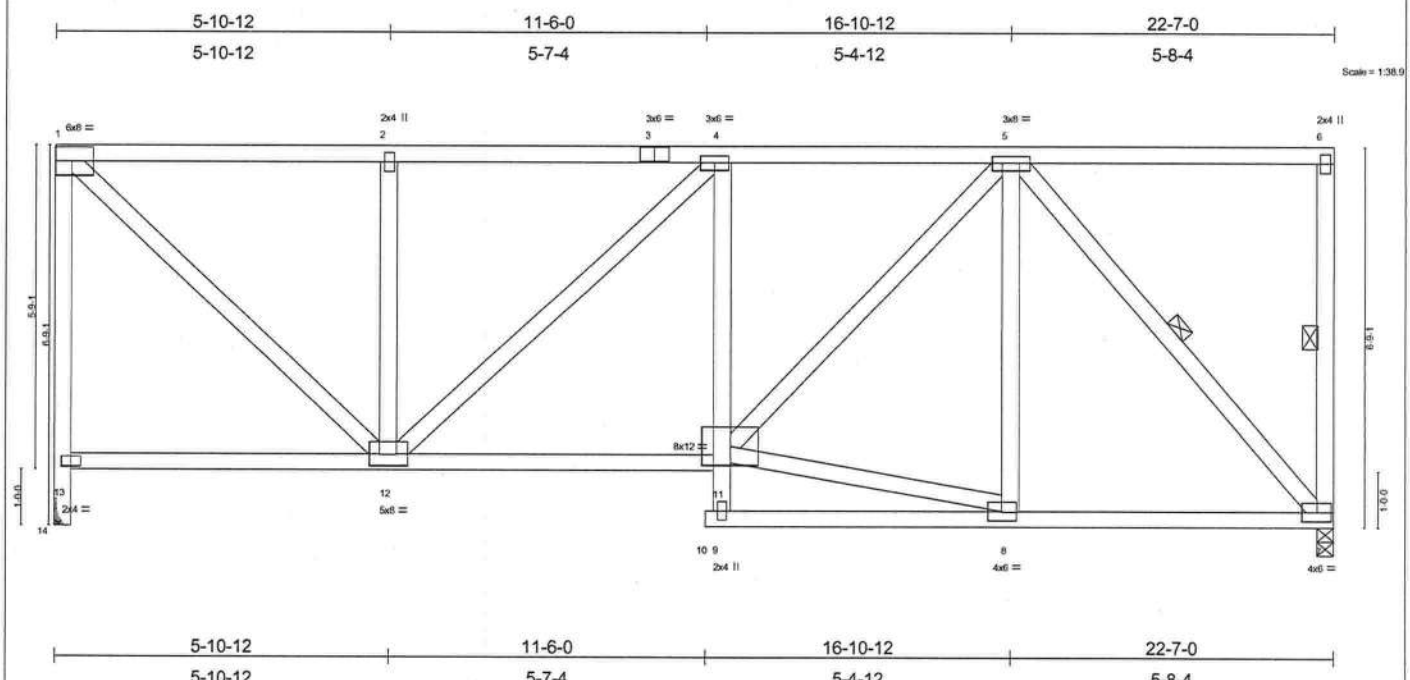


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.82 | Vert(LL) | -0.06 | 11-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.27 | Vert(TL) | -0.10 | 11-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.43 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 164 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

**REACTIONS** (lb/size) 14=940/Mechanical, 7=941/0-3-8
Max Uplift14=-353(load case 3), 7=-353(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 13-14=-940/365, 1-13=-858/376, 1-2=-782/303, 2-3=-782/303, 3-4=-782/303, 4-5=-1015/393, 5-6=-21/8, 6-7=-138/96
BOT CHORD 12-13=-11/28, 11-12=-398/1031, 9-11=0/82, 4-11=-57/116, 9-10=0/0, 8-9=-23/41, 7-8=-247/640
WEBS 1-12=-402/1040, 2-12=-333/244, 4-12=-335/129, 8-11=-230/615, 5-11=-210/540, 5-8=0/95, 5-7=-951/367

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 14 and 353 lb uplift at joint 7.

**LOAD CASE(S)** Standard


**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T37 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805923

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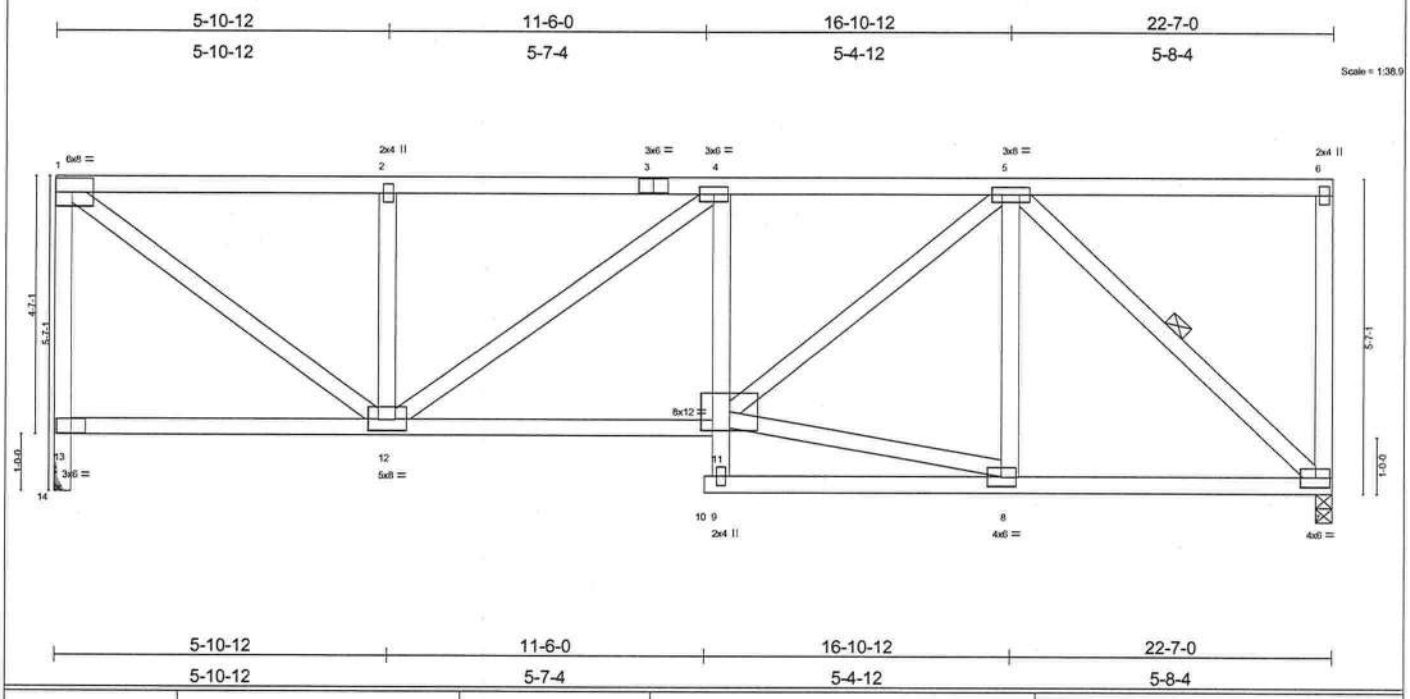


| LOADING (psf) | SPACING 2-0-0 | CSI | DEFL in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plates Increase 1.25 | TC 0.57 | Vert(LL) -0.07 11-12 >999 240 | MT20 244/190 |
| TCDL 7.0 | Lumber Increase 1.25 | BC 0.31 | Vert(TL) -0.12 11-12 >999 180 | |
| BCLL 10.0 | Rep Stress Incr YES | WB 0.40 | Horz(TL) 0.05 7 n/a n/a | |
| BCDL 5.0 | Code FBC2004/TPI2002 | (Matrix) | | Weight: 151 lb |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-1-8 oc bracing.
WEBS 1 Row at midpt 5-7

**REACTIONS** (lb/size) 14=940/Mechanical, 7=941/0-3-8
Max Uplift14=-353(load case 3), 7=-353(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 13-14=-940/365, 1-13=-857/376, 1-2=-993/384, 2-3=-993/384, 3-4=-993/384, 4-5=-1296/502, 5-6=-28/11, 6-7=-140/98
BOT CHORD 12-13=-16/40, 11-12=-510/1321, 9-11=0/84, 4-11=-48/113, 9-10=0/0, 8-9=-29/61, 7-8=-301/780
WEBS 1-12=-460/1190, 2-12=-329/242, 4-12=-402/155, 8-11=-279/738, 5-11=-258/664, 5-8=-4/107, 5-7=-1040/401

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 14 and 353 lb uplift at joint 7.

**LOAD CASE(S)** Standard


**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805924 |
|---|---|---|---|---|---|---|
| L128497 | T36 | HIP | 3 | 1 | Job Reference (optional) | |

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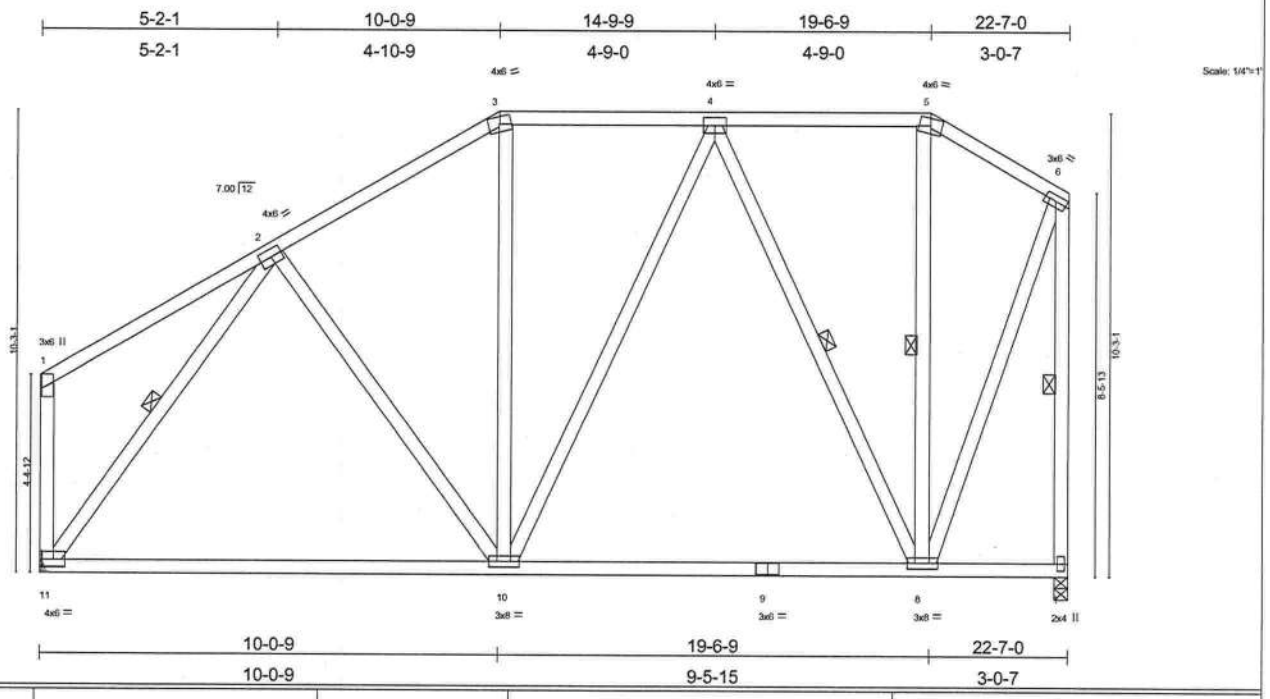


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.37 | Vert(LL) | -0.15 | 10-11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.39 | Vert(TL) | -0.27 | 10-11 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.40 | Horz(TL) | 0.01 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 186 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W7 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-8, 5-8, 2-11, 6-7

**REACTIONS** (lb/size) 11=936/Mechanical, 7=936/0-3-8
Max Horz 11=222(load case 5)
Max Uplift11=-219(load case 5), 7=-295(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-153/86, 2-3=-709/314, 3-4=-555/323, 4-5=-267/177, 5-6=-339/161, 1-11=-177/119, 6-7=-935/392
BOT CHORD 10-11=-325/505, 9-10=-226/471, 8-9=-226/471, 7-8=-3/2
WEBS 2-10=-118/190, 3-10=0/94, 4-10=-129/206, 4-8=-498/304, 5-8=-77/97, 2-11=-751/245, 6-8=-293/769

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 11 and 295 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T35 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805925

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 10:29:37 2005 Page 1

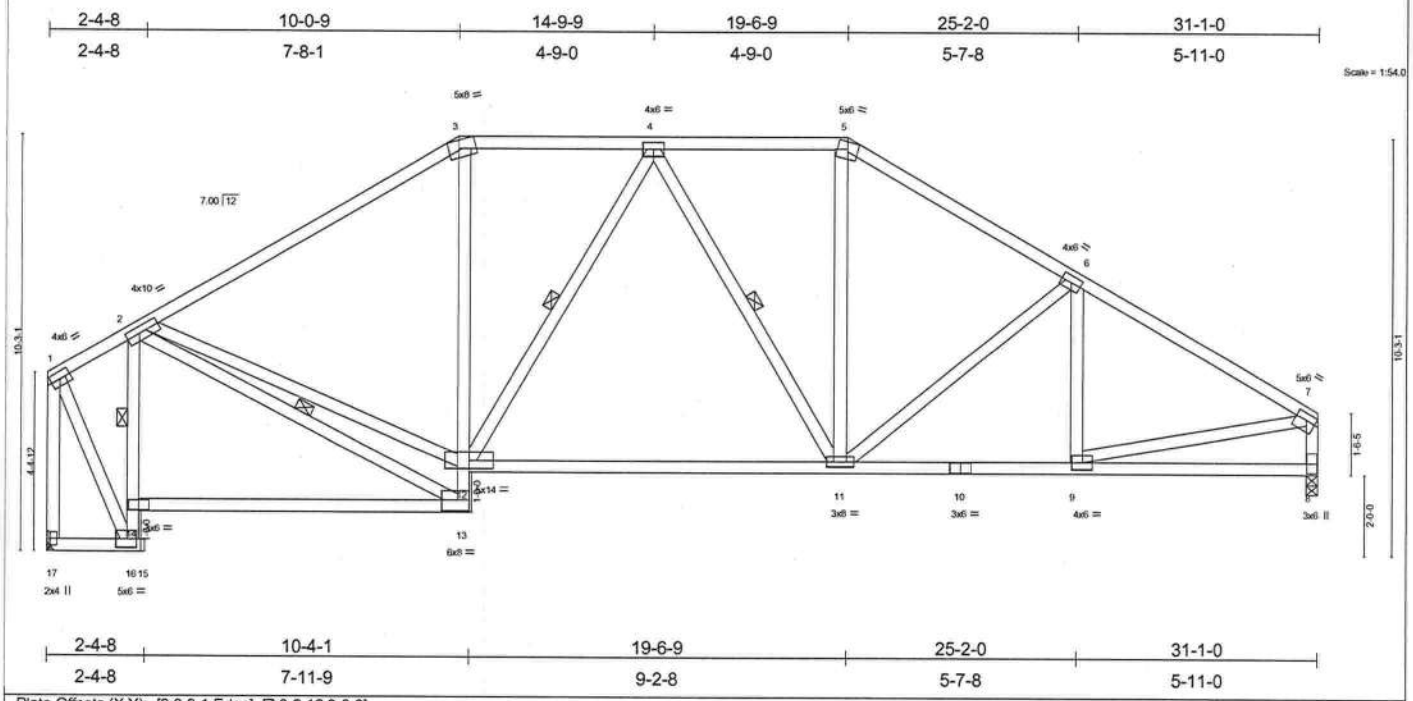





Plate Offsets (X,Y): [3:0-3-1,Edge], [7:0-2-12,0-2-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.41 | Vert(LL) | -0.21 | 11-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.98 | Vert(TL) | -0.35 | 11-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.44 | Horz(TL) | 0.15 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 221 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W10 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
2-2-0 oc bracing: 12-13
9-9-15 oc bracing: 9-11.
1 Row at midpt 2-14
WEBS 1 Row at midpt 2-13, 4-12, 4-11

**REACTIONS** (lb/size) 17=1301/Mechanical, 8=1294/0-3-8
Max Horz 17=-228(load case 3)
Max Uplift17=-322(load case 5), 8=-335(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-604/215, 2-3=-1424/575, 3-4=-1146/574, 4-5=-1213/602, 5-6=-1473/626, 6-7=-1629/602, 1-17=-1414/500, 7-8=-1193/473
BOT CHORD 16-17=-198/220, 15-16=0/0, 14-16=-906/403, 2-14=-812/434, 13-14=-366/615, 12-13=-137/388, 3-12=-74/357, 11-12=-409/1236, 10-11=-438/1338, 9-10=-438/1338, 8-9=-96/211
WEBS 2-13=-509/315, 2-12=-350/1046, 4-12=-285/289, 4-11=-164/254, 5-11=-105/404, 6-11=-191/227, 6-9=-121/116, 1-16=-463/1197, 7-9=-351/1155

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 322 lb uplift at joint 17 and 335 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805926 |
|---|---|---|---|---|---|---|
| L128497 | T34 | SPECIAL | 3 | 1 | Job Reference (optional) | |

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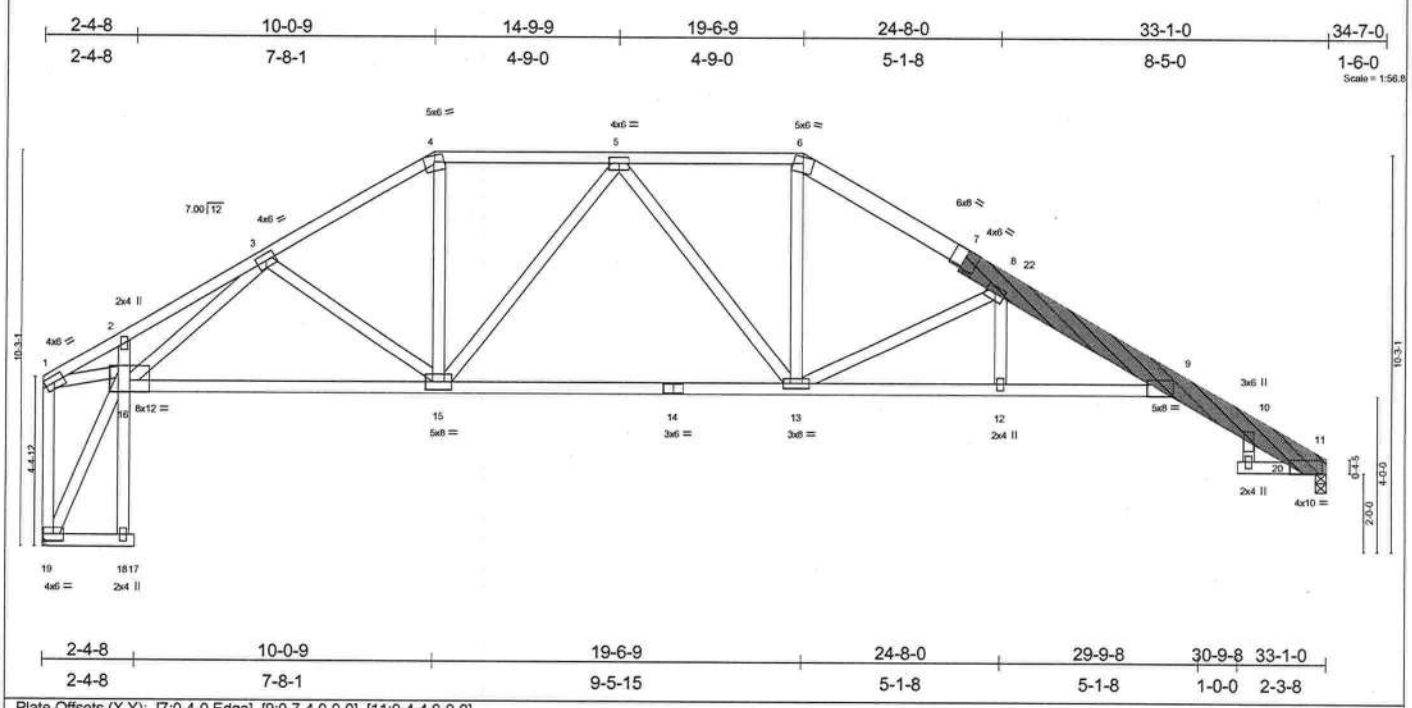





Plate Offsets (X,Y): [7:0-4-0,Edge], [9:0-7-4,0-0-0], [11:0-4-4,0-0-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | -0.44 | 9-12 | >892 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.90 | Vert(TL) | -0.70 | 9-12 | >559 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.85 | Horz(TL) | 0.69 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 245 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D *Except* T3 2 X 6 SYP No.1D, T4 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except* W1 2 X 4 SYP No.2D
LBR SCAB 7-11 2 X 8 SYP 2400F 2.0E one side

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-3 oc bracing.

**REACTIONS** (lb/size) 11=1270/0-3-8, 19=1379/Mechanical
Max Horz 19=-271(load case 3)
Max Uplift11=-394(load case 6), 19=-342(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2229/1113, 2-3=-2359/1155, 3-4=-1953/724, 4-5=-1654/679, 5-6=-1876/783, 6-7=-2225/849, 7-22=-2252/833, 8-22=-2252/831, 8-9=-3184/1138, 9-10=-503/242, 10-11=-635/263, 1-19=-1345/663
BOT CHORD 18-19=-15/9, 17-18=0/0, 16-18=-6/38, 2-16=-173/138, 15-16=-758/1851, 14-15=-566/1851, 13-14=-566/1851, 12-13=-976/3101, 9-12=-976/3100, 10-20=0/0
WEBS 3-15=-279/276, 4-15=-186/701, 5-15=-411/338, 5-13=-111/261, 6-13=-253/856, 8-13=-1409/619, 8-12=-1/181, 1-16=-945/1890, 16-19=-433/595, 3-16=-521/787

**NOTES**
1) Attached 10-10-4 scab 7 to 11, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 0-0-0 from end at joint 7, nail 2 row(s) at 7 o.c. for 2-1-6; starting at 5-3-11 from end at joint 7, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) Refer to girder(s) for truss to truss connections.
6) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 11 and 342 lb uplift at joint 19.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T33 | SPECIAL | 3 | 1 | Job Reference (optional) |

Dwg.#090805927

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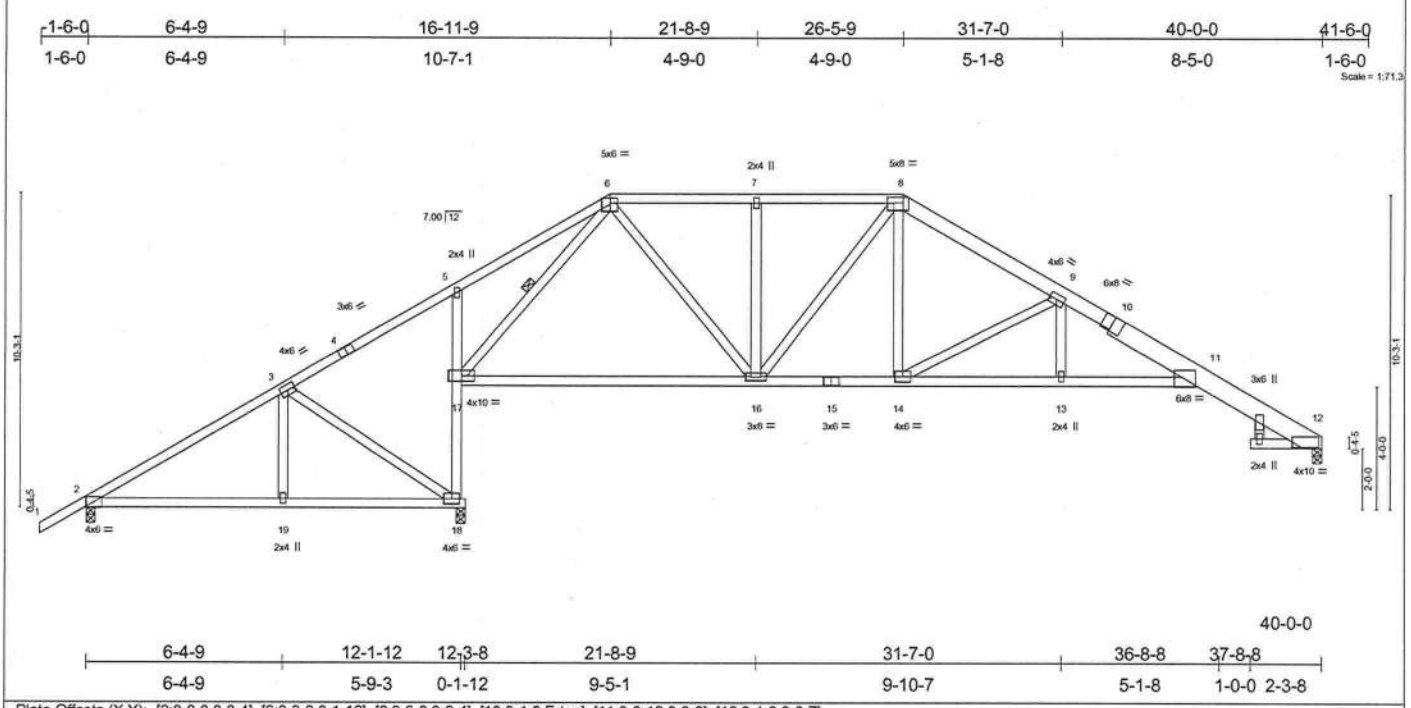





Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-0,0-1-12], [8:0-6-0,0-2-4], [10:0-4-0,Edge], [11:0-0-12,0-0-0], [12:0-4-3,0-0-7]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.98 | Vert(LL) | -0.56 | 11-13 | >599 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.94 | Vert(TL) | -0.91 | 11-13 | >366 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.82 | Horz(TL) | 0.39 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 233 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D *Except*
T4 2 X 6 SYP No.1D, T5 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3, B4 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-6-6 oc bracing. Except:
1 Row at midpt 17-18
WEBS 1 Row at midpt 6-17

**REACTIONS** (lb/size) 2=537/0-3-8, 12=1079/0-3-8, 18=1736/0-3-8
Max Horz 2=354(load case 4)
Max Uplift2=-373(load case 5), 12=-377(load case 6), 18=-662(load case 4)
Max Grav 2=549(load case 7), 12=1079(load case 1), 18=1736(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-539/509, 3-4=-185/290, 4-5=-172/312, 5-6=-84/340, 6-7=-1207/617, 7-8=-1208/616, 8-9=-1629/706, 9-10=-2511/981, 10-11=-2541/942, 11-12=-494/228
BOT CHORD 2-19=-432/392, 18-19=-432/392, 17-18=-1388/430, 5-17=-309/277, 16-17=-235/699, 15-16=-310/1327, 14-15=-310/1327, 13-14=-796/2479, 11-13=-796/2479
WEBS 3-19=-249/243, 3-18=-541/528, 6-17=-1207/352, 6-16=-260/847, 7-16=-263/232, 8-16=-204/114, 9-13=-16/291, 8-14=-238/722, 9-14=-1331/559

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 2, 377 lb uplift at joint 12 and 662 lb uplift at joint 18.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T32 | SPECIAL | 3 | 1 | Job Reference (optional) |

Dwg.#090805928

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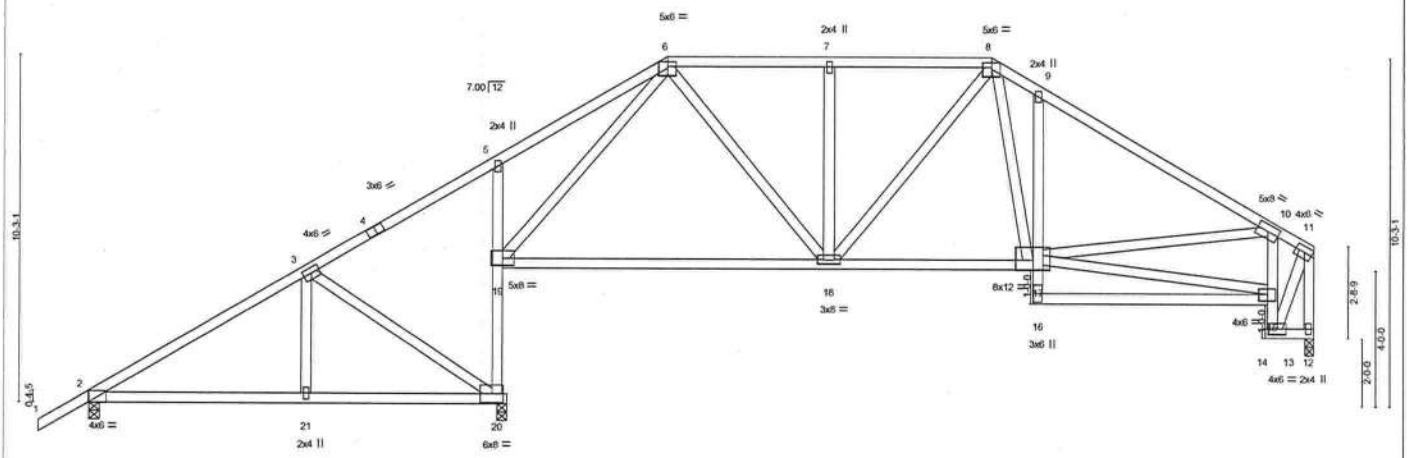
| -1-6-0 | 6-4-9 | 12-3-8 | 16-11-9 | 21-8-9 | 26-5-9 | 27-8-8 | 34-5-8 | 35-11-8 |
|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 6-4-9 | 5-10-15 | 4-8-1 | 4-9-0 | 4-9-0 | 1-2-15 | 6-9-0 | 1-6-0 |

Scale = 1:64.7




| 6-4-9 | 12-1-12 | 12-3-8 | 21-8-9 | 27-8-8 | 34-5-8 | 35-11-8 |
|---|---|---|---|---|---|---|
| 6-4-9 | 5-9-3 | 0-1-12 | 9-5-1 | 5-11-15 | 6-9-0 | 1-6-0 |

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-0,0-1-12], [8:0-3-0,0-1-12]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.24 | Vert(LL) | -0.15 | 18-19 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.98 | Vert(TL) | -0.27 | 18-19 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.96 | Horz(TL) | 0.07 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 231 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3, B7 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W10 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-3-9 oc bracing: 2-21
8-4-14 oc bracing: 20-21.

**REACTIONS** (lb/size) 2=570/0-3-8, 12=997/0-3-0, 20=1526/0-3-8
Max Horz 2=521(load case 4)
Max Uplift2=-343(load case 5), 12=-292(load case 6), 20=-747(load case 4)
Max Grav 2=571(load case 7), 12=997(load case 1), 20=1526(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-579/354, 3-4=-311/229, 4-5=-298/250, 5-6=-219/251, 6-7=-989/476, 7-8=-989/476, 8-9=-1247/618, 9-10=-1311/501, 10-11=-476/199, 11-12=-1114/388
BOT CHORD 2-21=-582/428, 20-21=-582/428, 19-20=-1163/483, 5-19=-316/275, 18-19=-237/617, 17-18=-238/940, 16-17=0/115, 9-17=-263/288, 15-16=0/142, 13-15=-649/303, 10-15=-624/352, 13-14=0/0, 12-13=-30/41
WEBS 3-21=-238/221, 3-20=-540/529, 6-19=-910/460, 6-18=-211/632, 7-18=-275/243, 8-18=-109/169, 8-17=-294/446, 15-17=-181/467, 10-17=-172/451, 11-13=-355/877

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 2, 292 lb uplift at joint 12 and 747 lb uplift at joint 20.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805929 |
|---|---|---|---|---|---|---|
| L128497 | T31 | SPECIAL | 1 | 1 | Job Reference (optional) | |

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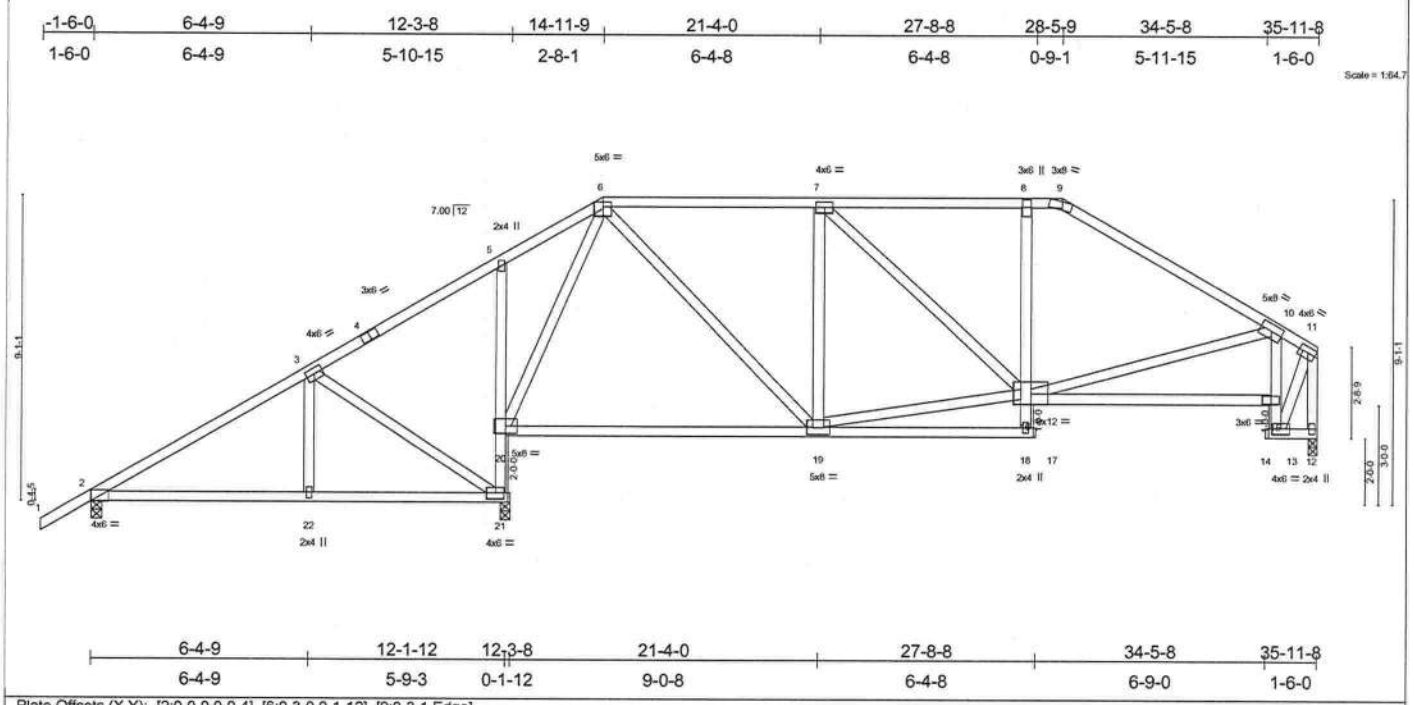


Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-0,0-1-12], [9:0-3-1,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.42 | Vert(LL) | -0.13 | 19-20 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.67 | Vert(TL) | -0.23 | 19-20 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.91 | Horz(TL) | 0.05 | 12 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 227 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W10 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-9-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 2=529/0-3-8, 12=982/0-3-0, 21=1591/0-3-8
Max Horz 2=421(load case 4)
Max Uplift2=-378(load case 5), 12=-327(load case 3), 21=-778(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-500/450, 3-4=-224/223, 4-5=-210/244, 5-6=-159/248, 6-7=-830/488, 7-8=-922/491, 8-9=-906/483, 9-10=-1124/480, 10-11=-478/208, 11-12=-1089/395
BOT CHORD 2-22=-535/361, 21-22=-535/361, 20-21=-1226/516, 5-20=-209/216, 19-20=-171/291, 18-19=-48/0, 17-18=0/0, 16-18=0/81, 8-16=0/184, 15-16=-153/571, 13-15=-575/285, 10-15=-463/306, 13-14=0/0, 12-13=-35/39
WEBS 3-22=-250/225, 3-21=-548/545, 6-20=-952/483, 6-19=-278/790, 7-19=-473/301, 16-19=-335/875, 7-16=-29/129, 10-16=-248/396, 11-13=-329/785

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 378 lb uplift at joint 2, 327 lb uplift at joint 12 and 778 lb uplift at joint 21.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T30 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805930

Builders FirstSource, Lake City, Fl 32055

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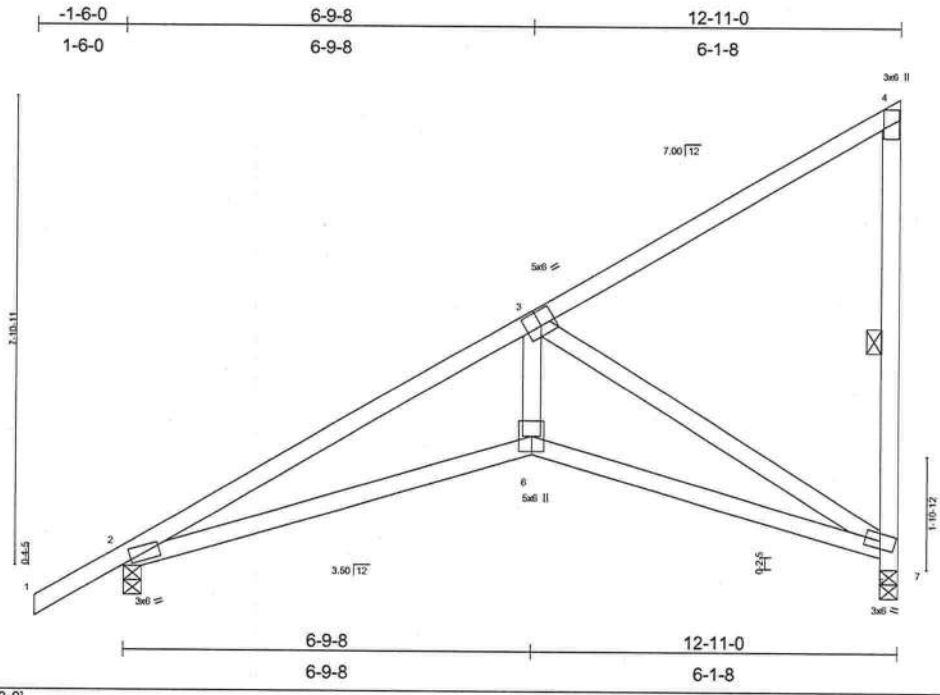


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.28 | Vert(LL) | -0.09 | 2-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.36 | Vert(TL) | -0.15 | 2-6 | >991 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.92 | Horz(TL) | 0.08 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 69 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-8-5 oc bracing.
WEBS 1 Row at midpt 4-7

**REACTIONS** (lb/size) 7=524/0-3-8, 2=625/0-3-8
Max Horz 2=399(load case 5)
Max Uplift7=-296(load case 5), 2=-197(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1166/319, 3-4=-113/48, 5-7=-524/296, 4-5=-126/127
BOT CHORD 2-6=-544/996, 5-6=-530/964
WEBS 3-6=-259/750, 3-5=-1055/595

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Bearing at joint(s) 7, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 7 and 197 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T29 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805931

Builders FirstSource, Lake City, Fl 32055

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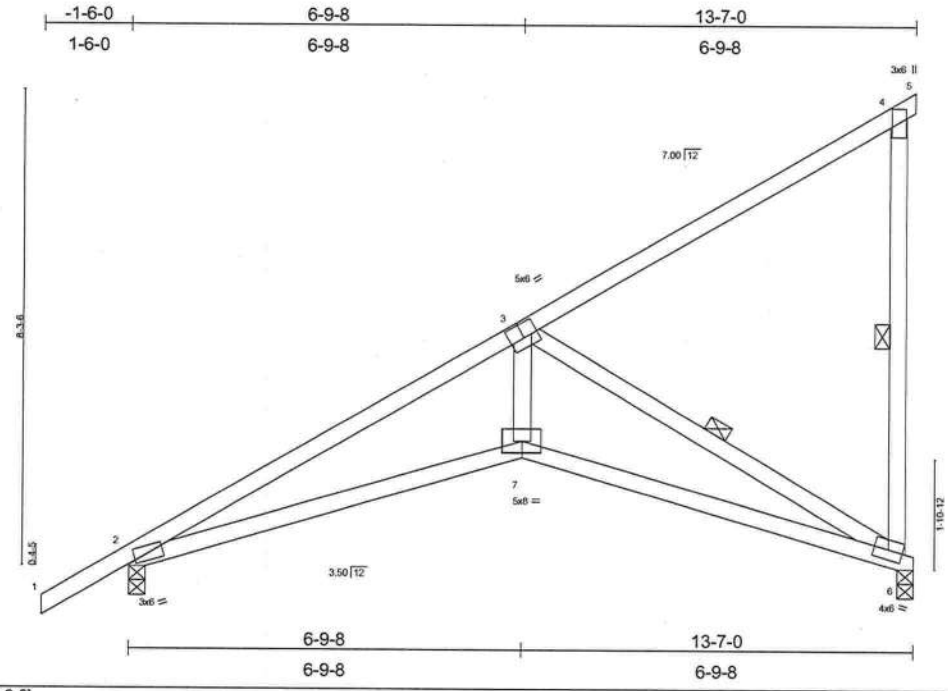


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | -0.10 | 2-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.36 | Vert(TL) | -0.16 | 2-7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.35 | Horz(TL) | 0.08 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 72 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-5-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-3 oc bracing.
WEBS 1 Row at midpt 4-6, 3-6

**REACTIONS** (lb/size) 6=551/0-3-8, 2=647/0-3-8
Max Horz 2=419(load case 5)
Max Uplift6=-322(load case 5), 2=-196(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1261/351, 3-4=-125/54, 4-5=-2/0, 4-6=-149/159
BOT CHORD 2-7=-597/1083, 6-7=-600/1097
WEBS 3-7=-287/814, 3-6=-1173/656

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 322 lb uplift at joint 6 and 196 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. | Dwg.#090805932 |
|---|---|---|---|---|---|---|
| L128497 | T28 | SPECIAL | 2 | 1 | Job Reference (optional) | |

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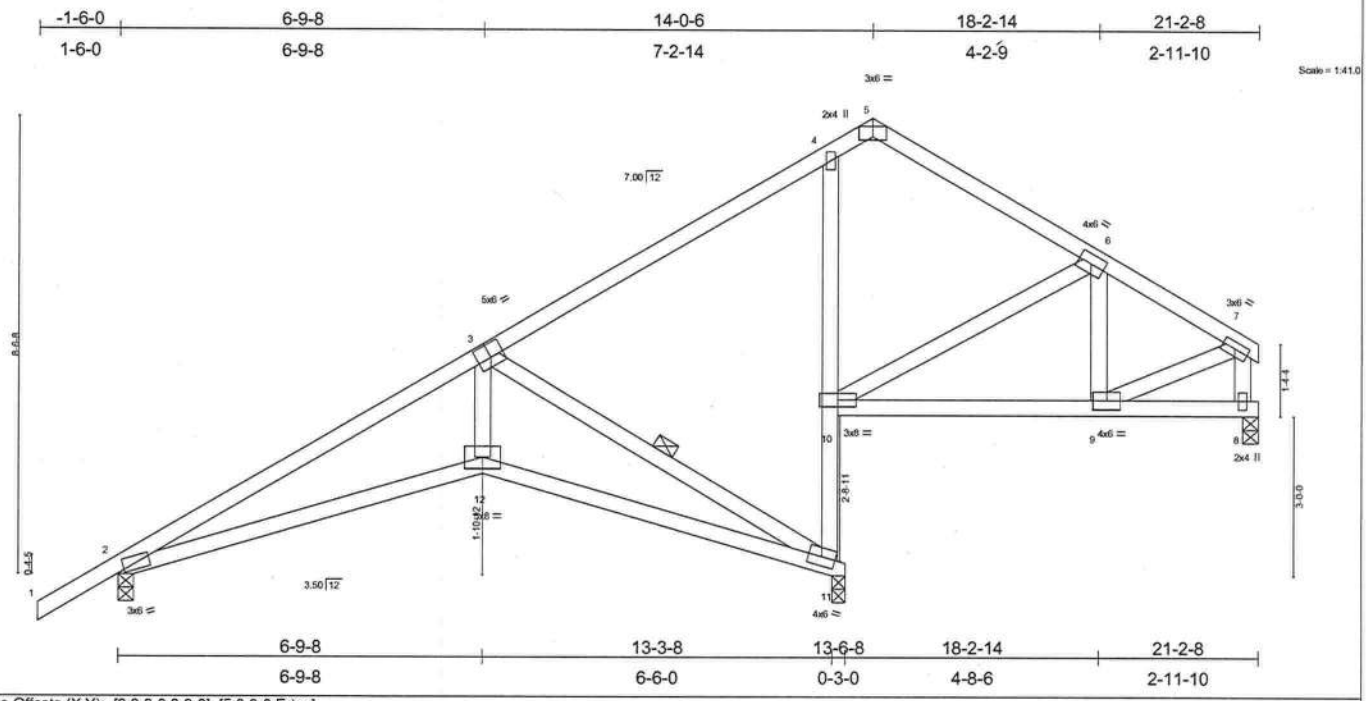


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.26 | Vert(LL) | -0.09 | 2-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.40 | Vert(TL) | -0.15 | 2-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.31 | Horz(TL) | 0.07 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 114 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-9 oc bracing.
WEBS 1 Row at midpt 3-11

**REACTIONS** (lb/size) 2=618/0-3-8, 8=277/0-3-8, 11=938/0-3-0
Max Horz 2=327(load case 5)
Max Uplift2=-220(load case 5), 8=-221(load case 6), 11=-476(load case 5)
Max Grav 2=618(load case 1), 8=310(load case 8), 11=938(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1149/361, 3-4=-143/158, 4-5=-66/144, 5-6=-83/113, 6-7=-295/273, 7-8=-282/254
BOT CHORD 2-12=-481/980, 11-12=-473/951, 10-11=-547/435, 4-10=-342/203, 9-10=-202/235, 8-9=-24/19
WEBS 3-12=-235/758, 3-11=-1065/574, 6-10=-254/300, 6-9=-125/93, 7-9=-196/240

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2, 221 lb uplift at joint 8 and 476 lb uplift at joint 11.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T27G | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805933

Builders FirstSource, Lake City, Fl 32055

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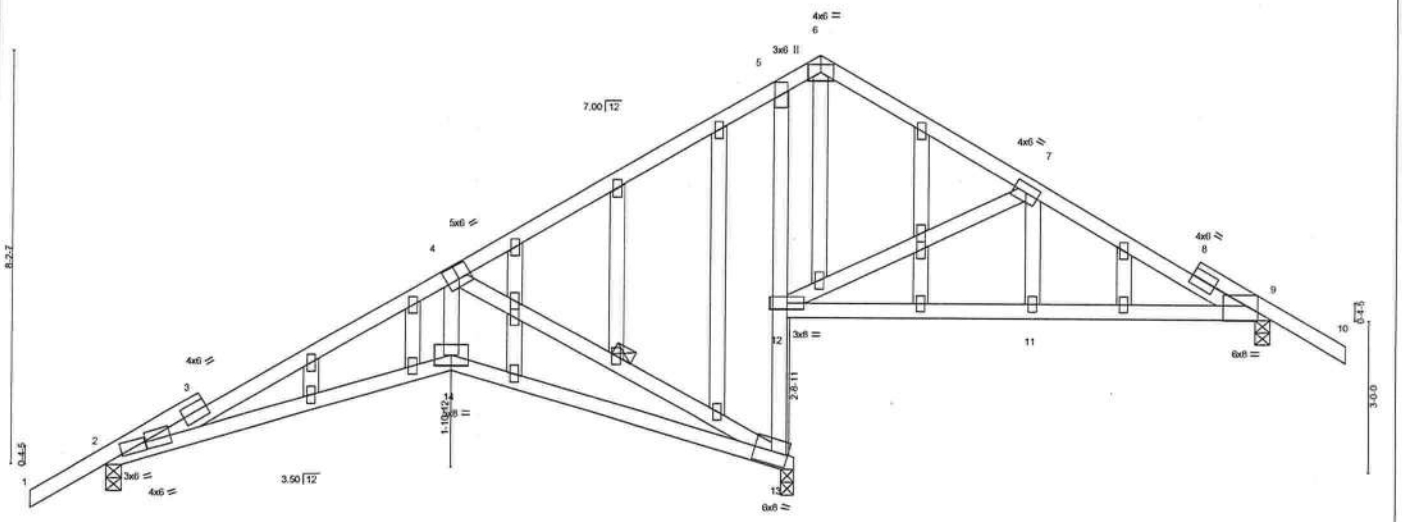
| -1-6-0 | 6-9-8 | 14-0-6 | 18-2-14 | 22-11-0 | 24-5-0 |
|---|---|---|---|---|---|
| 1-6-0 | 6-9-8 | 7-2-14 | 4-2-9 | 4-8-2 | 1-6-0 |

Scale = 1:43.4




| 6-9-8 | 13-3-8 | 13-6-8 | 18-2-14 | 22-11-0 |
|---|---|---|---|---|
| 6-9-8 | 6-6-0 | 0-3-0 | 4-8-6 | 4-8-2 |

Plate Offsets (X,Y): [2:0-10-6,0-0-8], [2:0-4-2,0-0-8], [4:0-3-0,0-3-4], [9:0-2-11,Edge], [29:0-1-14,0-1-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.55 | Vert(LL) | -0.16 | 2-14 | >973 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.74 | Vert(TL) | -0.26 | 2-14 | >602 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.47 | Horz(TL) | 0.14 | 13 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 152 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-2-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-3 oc bracing.
WEBS 1 Row at midpt 4-13

**REACTIONS** (lb/size) 9=630/0-3-8, 2=859/0-3-8, 13=1444/0-3-0
Max Horz 2=273(load case 5)
Max Uplift9=-533(load case 6), 2=-412(load case 5), 13=-753(load case 5)
Max Grav 9=663(load case 8), 2=859(load case 1), 13=1444(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-17/62, 2-3=-1743/932, 3-4=-1639/906, 4-5=-154/218, 5-6=-73/190, 6-7=-87/179, 7-8=-602/625, 8-9=-664/637, 9-10=-18/65
BOT CHORD 2-14=-832/1514, 13-14=-810/1464, 12-13=-940/713, 5-12=-585/331, 11-12=-443/520, 9-11=-443/520
WEBS 4-14=-452/1076, 4-13=-1665/998, 7-12=-619/642, 7-11=-232/147

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Bearing at joint(s) 2, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 13.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 533 lb uplift at joint 9, 412 lb uplift at joint 2 and 753 lb uplift at joint 13.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-87(F=-33), 6-10=-87(F=-33), 2-14=-30, 13-14=-30, 9-12=-30

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T27 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805934

Builders FirstSource, Lake City, Fl 32055

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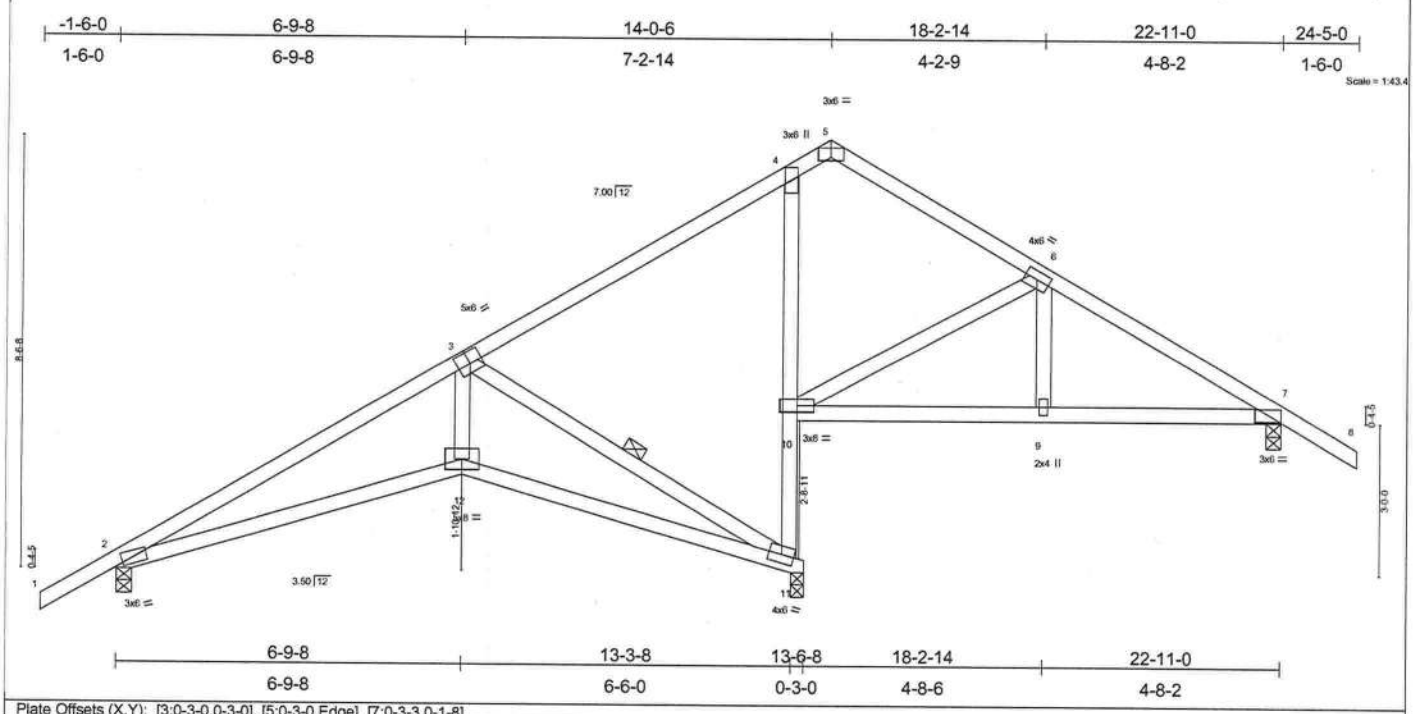


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,Edge], [7:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.25 | Vert(LL) | -0.09 | 2-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.46 | Vert(TL) | -0.15 | 2-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.31 | Horz(TL) | 0.07 | 11 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 117 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-11 oc bracing.
WEBS 1 Row at midpt 3-11

**REACTIONS** (lb/size) 2=617/0-3-8, 7=458/0-3-8, 11=1004/0-3-0
Max Horz 2=279(load case 5)
Max Uplift2=-245(load case 5), 7=-419(load case 6), 11=-458(load case 5)
Max Grav 2=617(load case 1), 7=484(load case 8), 11=1004(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1143/464, 3-4=-132/240, 4-5=-59/193, 5-6=-79/208, 6-7=-489/488, 7-8=0/40
BOT CHORD 2-12=-472/975, 11-12=-463/946, 10-11=-613/389, 4-10=-340/156, 9-10=-293/367, 7-9=-293/367
WEBS 3-12=-231/756, 3-11=-1065/571, 6-10=-408/435, 6-9=-236/151

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2, 419 lb uplift at joint 7 and 458 lb uplift at joint 11.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T26 | SPECIAL | 3 | 1 | Job Reference (optional) |

Dwg.#090805935

Builders FirstSource, Lake City, Fl 32055

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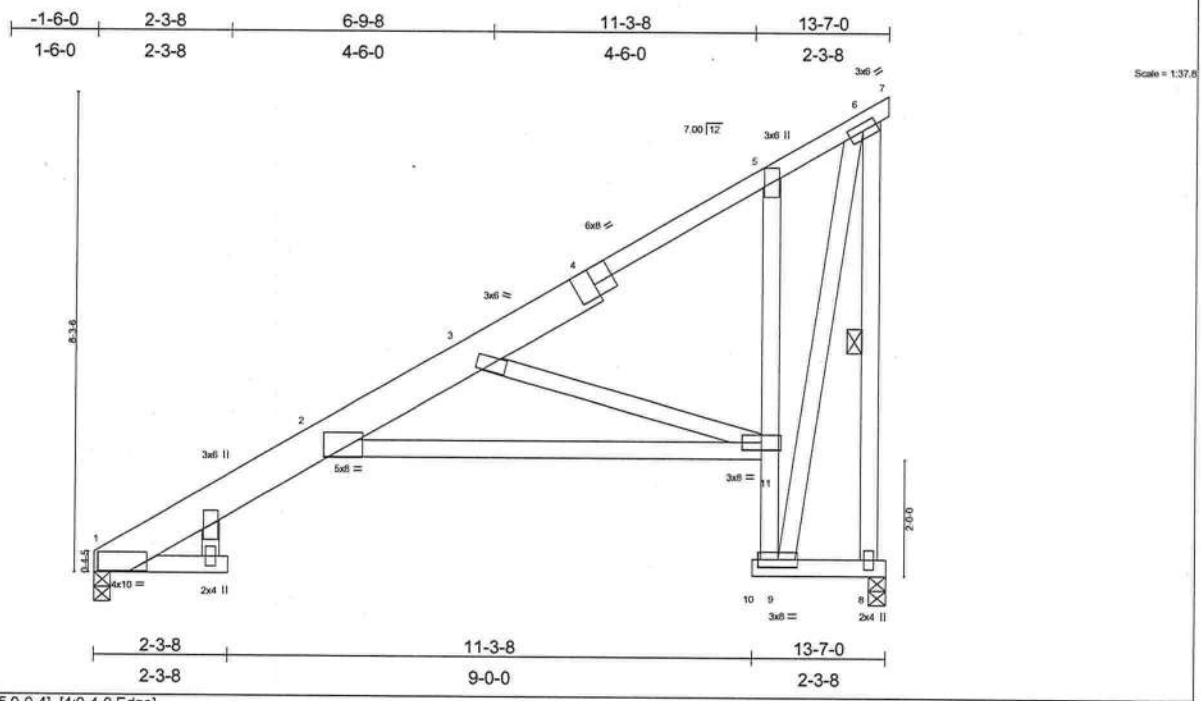


Plate Offsets (X,Y): [1:0-3-15,0-0-4], [4:0-4-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.59 | Vert(LL) | -0.21 | 2-11 | >751 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.74 | Vert(TL) | -0.35 | 2-11 | >445 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.48 | Horz(TL) | 0.35 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 97 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D *Except*
T1 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-12 oc bracing.
WEBS 1 Row at midpt 6-8

**REACTIONS** (lb/size) 8=553/0-3-8, 1=492/0-3-8
Max Horz 1=369(load case 5)
Max Uplift8=-328(load case 5), 1=-93(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-279/4, 2-3=-1022/402, 3-4=-319/0, 4-5=-217/7, 5-6=-120/52, 6-7=-2/0, 6-8=-622/374
BOT CHORD 2-11=-744/1149, 9-11=-422/302, 5-11=-19/123, 9-10=0/0, 8-9=-3/6
WEBS 3-11=-1009/676, 6-9=-349/571

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 8 and 93 lb uplift at joint 1.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T25 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805936

Builders FirstSource, Lake City, Fl 32055

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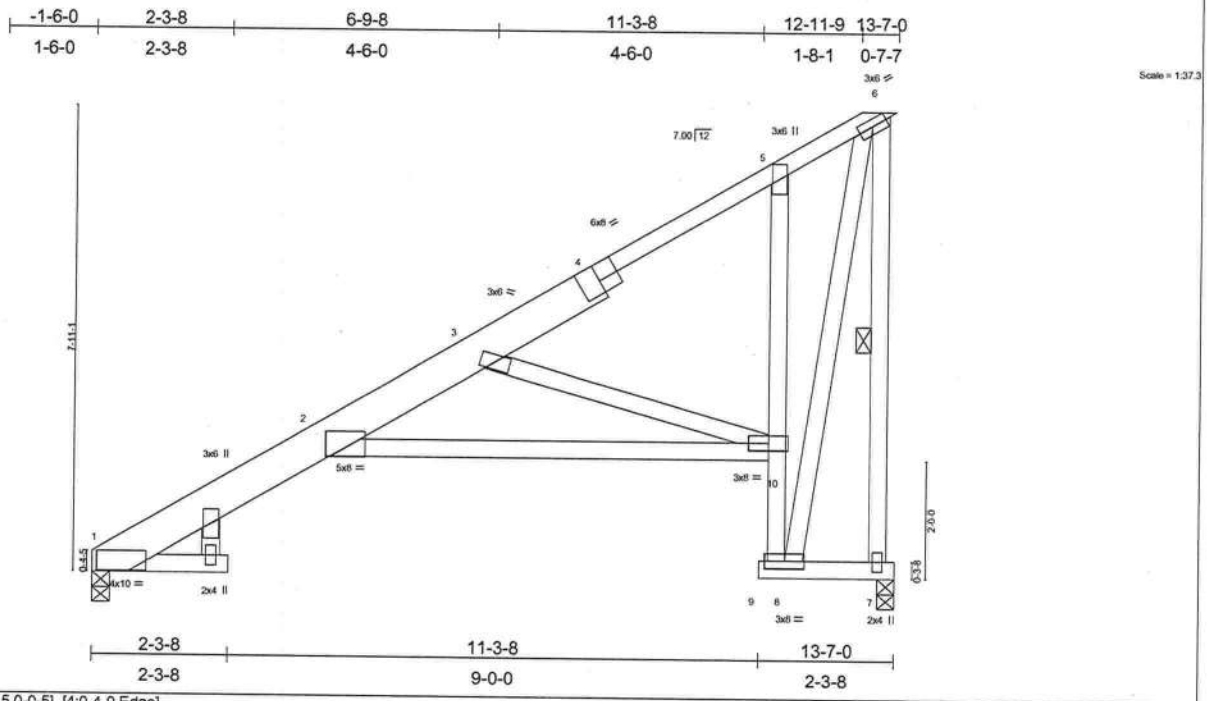



Plate Offsets (X,Y): [1:0-3-15,0-0-5], [4:0-4-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.59 | Vert(LL) | -0.21 | 2-10 | >751 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.74 | Vert(TL) | -0.35 | 2-10 | >445 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.48 | Horz(TL) | 0.35 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 97 lb | |

**LUMBER**

TOP CHORD 2 X 4 SYP No.2D *Except*
T1 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-12 oc bracing.
WEBS 1 Row at midpt 6-7

**REACTIONS** (lb/size) 7=549/0-3-8, 1=492/0-3-8
Max Horz 1=362(load case 5)
Max Uplift7=-315(load case 5), 1=-98(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-271/1, 2-3=-1021/410, 3-4=-319/0, 4-5=-217/8, 5-6=-120/53, 6-7=-618/362
BOT CHORD 2-10=-744/1148, 8-10=-422/303, 5-10=-19/124, 8-9=0/0, 7-8=-3/6
WEBS 3-10=-1008/676, 6-8=-351/572

**NOTES**

1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 7 and 98 lb uplift at joint 1.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T24 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805937

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:57:30 2005 Page 1

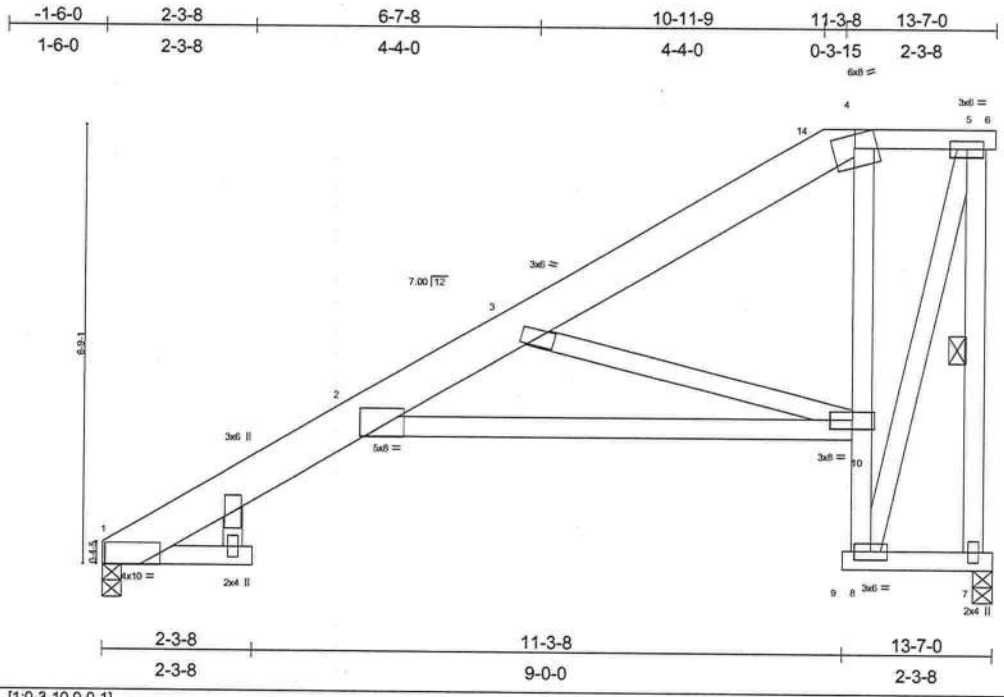



Plate Offsets (X,Y): [1:0-3-10,0-0-1]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.58 | Vert(LL) | -0.20 | 2-10 | >800 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.85 | Vert(TL) | -0.33 | 2-10 | >474 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.49 | Horz(TL) | 0.32 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 97 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-7
JOINTS 1 Brace at Jt(s): 10

**REACTIONS** (lb/size) 1=492/0-3-8, 7=553/0-3-8
Max Horz 1=290(load case 5)
Max Uplift1=-130(load case 5), 7=-229(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-219/0, 2-3=-1022/430, 3-14=-314/56, 4-14=-176/58, 4-5=-137/70, 5-6=0/0, 5-7=-607/300
BOT CHORD 2-10=-669/1149, 8-10=-379/231, 4-10=0/82, 8-9=0/0, 7-8=-2/5
WEBS 3-10=-983/608, 5-8=-261/510

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 1 and 229 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T23 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805938

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:59:06 2005 Page 1

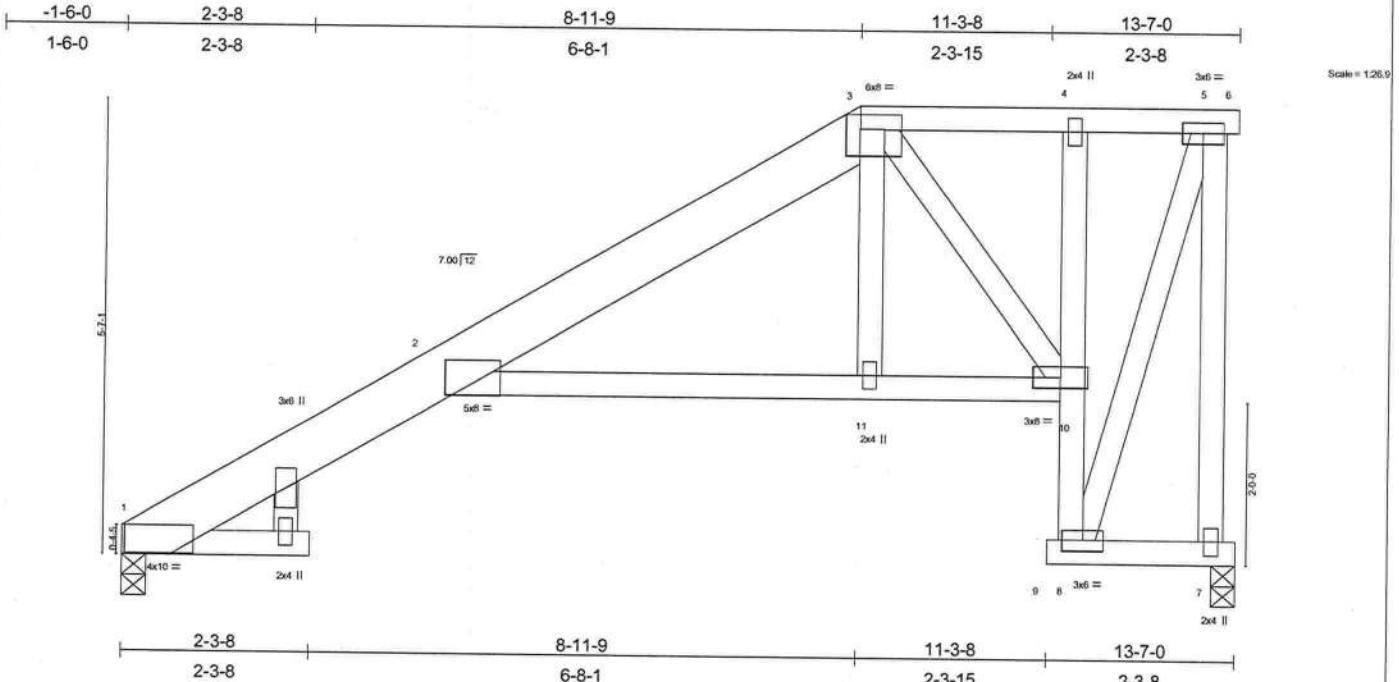



Plate Offsets (X,Y): [1:0-3-6,0-0-4], [3:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | 0.22 | 2-11 | >719 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.66 | Vert(TL) | -0.34 | 2-11 | >459 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.20 | Horz(TL) | 0.32 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 90 lb | |

**LUMBER**

TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.1D, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=492/0-3-8, 7=553/0-3-8
Max Horz 1=245(load case 5)
Max Uplift1=-142(load case 5), 7=-187(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-219/3, 2-3=-608/211, 3-4=-202/101, 4-5=-155/81, 5-6=0/0, 5-7=-590/316
BOT CHORD 2-11=-317/585, 10-11=-320/597, 8-10=-362/214, 4-10=-52/110, 8-9=0/0, 7-8=-4/7
WEBS 3-11=-50/263, 3-10=-634/344, 5-8=-251/482

**NOTES**

1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 1 and 187 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T22 | MONO HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805939

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:50:52 2005 Page 1

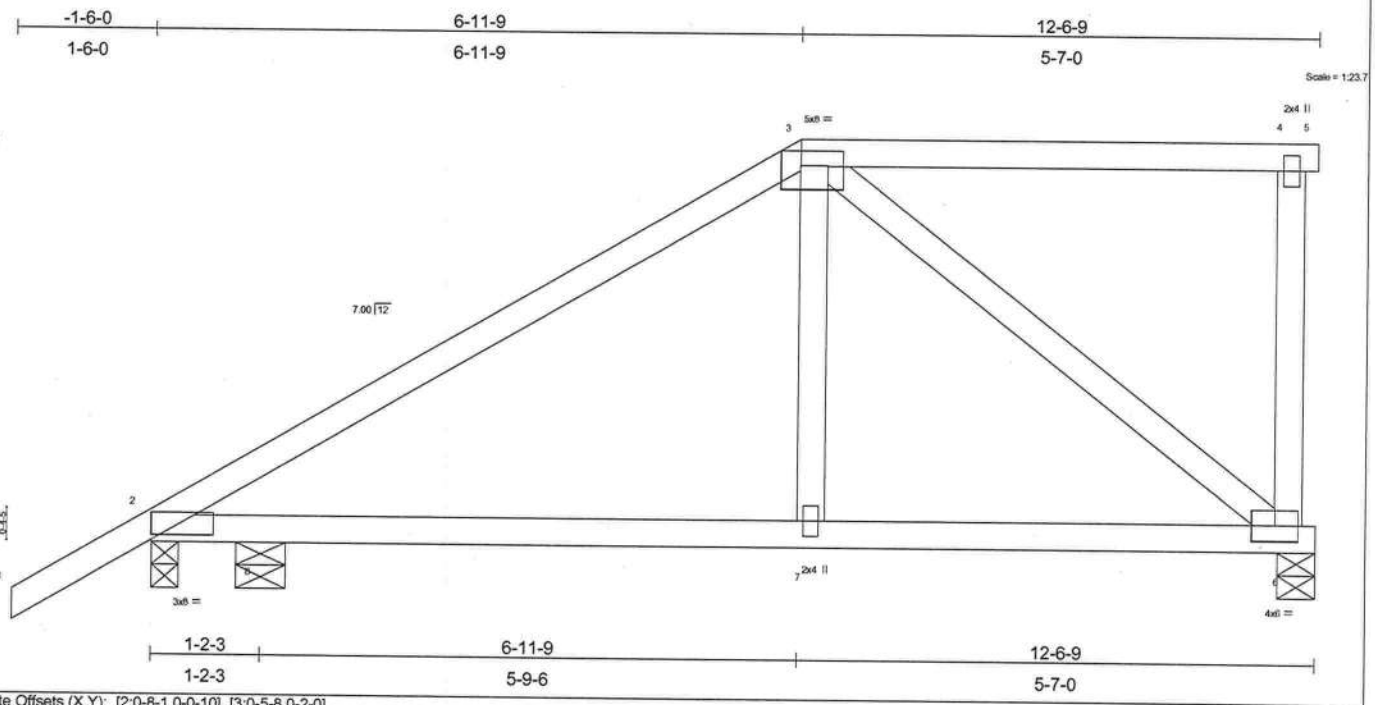



Plate Offsets (X,Y): [2:0-8-1,0-0-10], [3:0-5-8,0-2-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.26 | Vert(LL) | -0.02 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.26 | Vert(TL) | -0.03 | 7-8 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.34 | Horz(TL) | 0.01 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 62 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 6=478/0-4-15, 2=293/0-3-8, 8=340/0-6-7
Max Horz 2=244(load case 5)
Max Uplift6=-176(load case 4), 2=-152(load case 5), 8=-121(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-543/154, 3-4=-28/23, 4-5=0/0, 4-6=-125/136
BOT CHORD 2-8=-212/381, 7-8=-212/381, 6-7=-210/387
WEBS 3-7=0/166, 3-6=-455/251

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 6, 152 lb uplift at joint 2 and 121 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T21 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805940

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:48:29 2005 Page 1

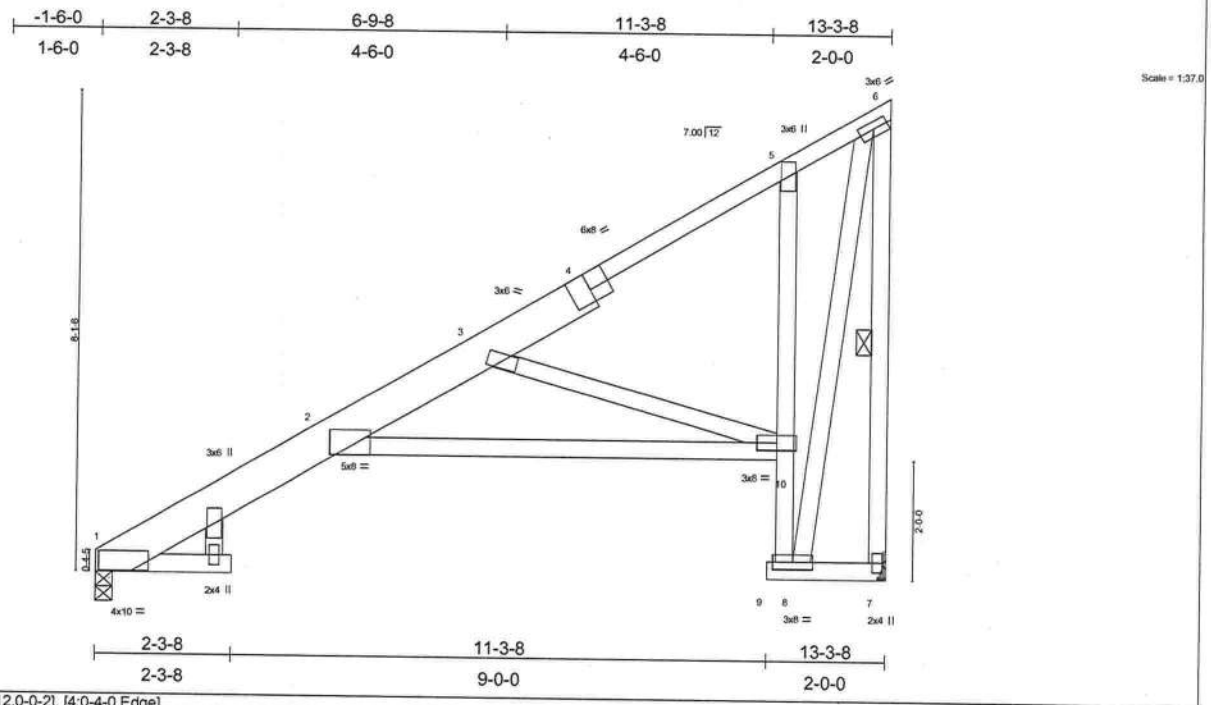



Plate Offsets (X,Y): [1:0-3-12,0-0-2], [4:0-4-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.58 | Vert(LL) | -0.20 | 2-10 | >767 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.71 | Vert(TL) | -0.34 | 2-10 | >454 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.48 | Horz(TL) | 0.34 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 95 lb | |

**LUMBER**

TOP CHORD 2 X 4 SYP No.2D *Except*
T1 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-5 oc bracing.
WEBS 1 Row at midpt 6-7

**REACTIONS** (lb/size) 7=543/Mechanical, 1=486/0-3-8
Max Horz 1=358(load case 5)
Max Uplift7=-311(load case 5), 1=-96(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-268/1, 2-3=-999/404, 3-4=-305/0, 4-5=-202/4, 5-6=-101/47, 6-7=-612/356
BOT CHORD 2-10=-734/1125, 8-10=-411/297, 5-10=-9/118, 8-9=0/0, 7-8=-4/7
WEBS 3-10=-998/674, 6-8=-342/554

**NOTES**

1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 7 and 96 lb uplift at joint 1.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T20 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805941

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:49:48 2005 Page 1

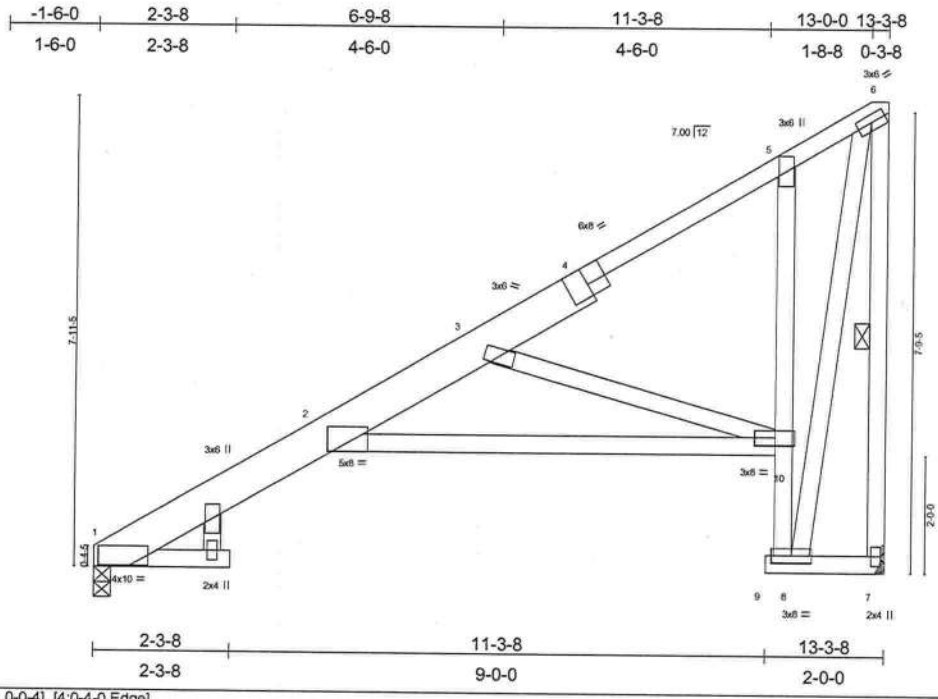



Plate Offsets (X,Y): [1:0-4-1,0-0-4], [4:0-4-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.58 | Vert(LL) | -0.20 | 2-10 | >767 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.71 | Vert(TL) | -0.34 | 2-10 | >454 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.48 | Horz(TL) | 0.34 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 95 lb | |

**LUMBER**

TOP CHORD 2 X 4 SYP No.2D *Except*
T1 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-5 oc bracing.
WEBS 1 Row at midpt 6-7

**REACTIONS** (lb/size) 7=543/Mechanical, 1=486/0-3-8
Max Horz 1=358(load case 5)
Max Uplift7=-311(load case 5), 1=-96(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-268/1, 2-3=-999/404, 3-4=-305/0, 4-5=-202/4, 5-6=-101/47, 6-7=-612/356
BOT CHORD 2-10=-734/1125, 8-10=-411/297, 5-10=-9/118, 8-9=0/0, 7-8=-4/7
WEBS 3-10=-998/674, 6-8=-342/554

**NOTES**

1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 7 and 96 lb uplift at joint 1.

**LOAD CASE(S)** Standard

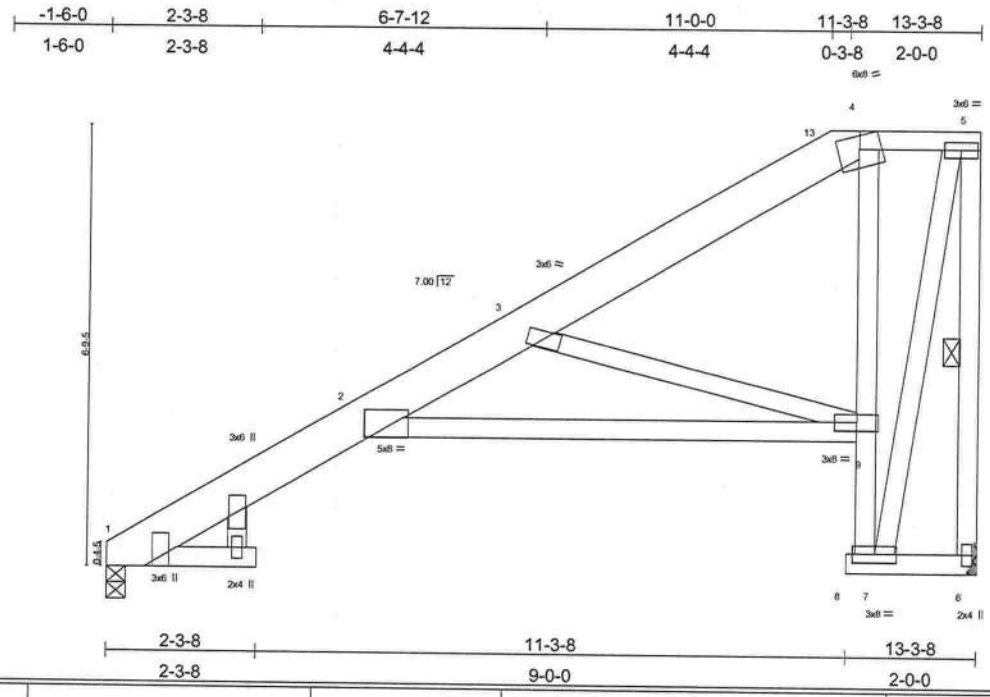
**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T19 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805942

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:44:49 2005 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.57 | Vert(LL) | -0.19 | 2-9 | >801 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.82 | Vert(TL) | -0.33 | 2-9 | >474 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.49 | Horz(TL) | 0.32 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 97 lb | |

**LUMBER**

TOP CHORD 2 X 8 SYP No.1D *Except* T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-6
JOINTS 1 Brace at Jt(s): 9

**REACTIONS** (lb/size) 1=486/0-3-8, 6=543/Mechanical
Max Horz 1=292(load case 5)
Max Uplift1=-127(load case 5), 6=-229(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-216/0, 2-3=-999/419, 3-13=-293/45, 4-13=-166/47, 4-5=-114/59, 5-6=-616/309
BOT CHORD 2-9=-660/1125, 7-9=-344/218, 4-9=0/72, 7-8=0/0, 6-7=-3/7
WEBS 3-9=-982/607, 5-7=-257/493

**NOTES**

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1 and 229 lb uplift at joint 6.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T18 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805943

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:42:38 2005 Page 1

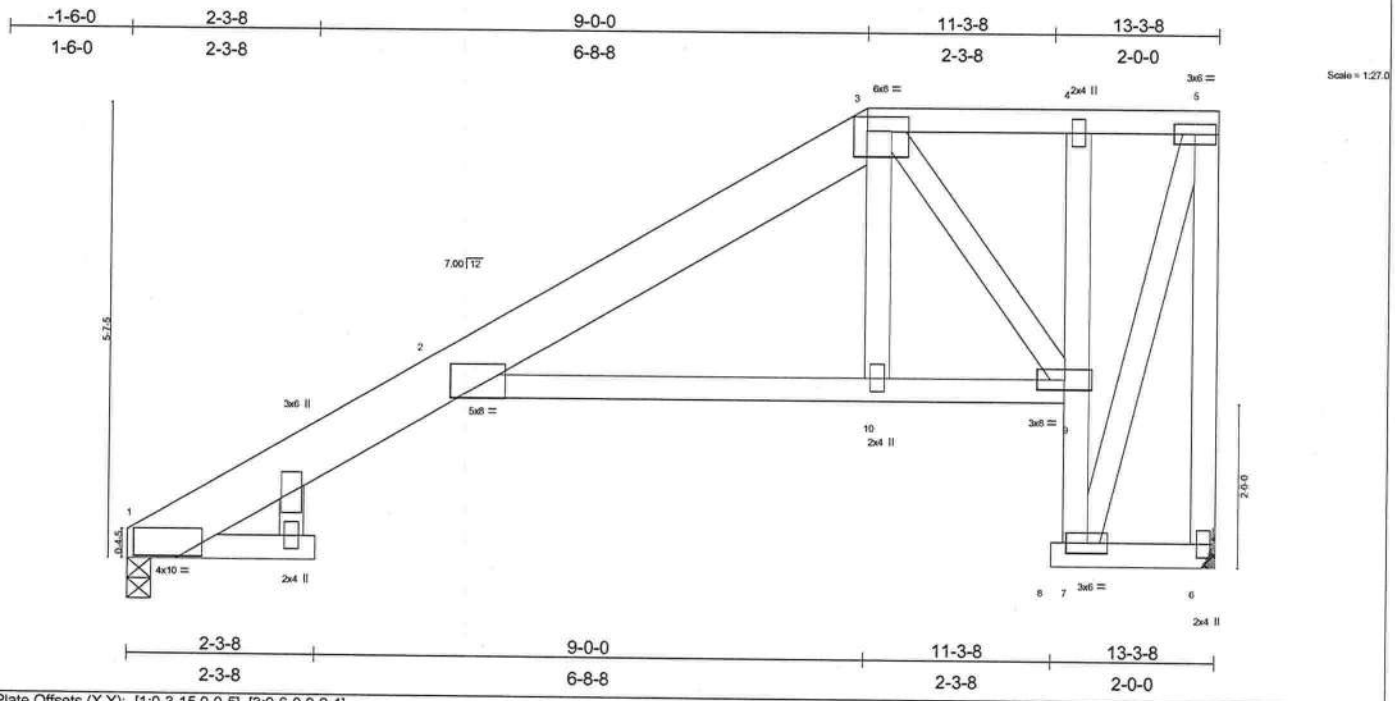



Plate Offsets (X,Y): [1:0-3-15,0-0-5], [3:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | 0.22 | 2-10 | >721 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.59 | Vert(TL) | -0.34 | 2-10 | >463 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.20 | Horz(TL) | 0.31 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 89 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B3 2 X 4 SYP No.1D, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=486/0-3-8, 6=543/Mechanical
Max Horz 1=246(load case 5)
Max Uplift1=-140(load case 5), 6=-185(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-216/3, 2-3=-585/200, 3-4=-182/92, 4-5=-139/73, 5-6=-578/316
BOT CHORD 2-10=-308/562, 9-10=-310/574, 7-9=-357/211, 4-9=-58/107, 7-8=0/0, 6-7=-4/8
WEBS 3-10=-47/258, 3-9=-636/348, 5-7=-245/468

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 1 and 185 lb uplift at joint 6.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T17 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805944

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:39:36 2005 Page 1

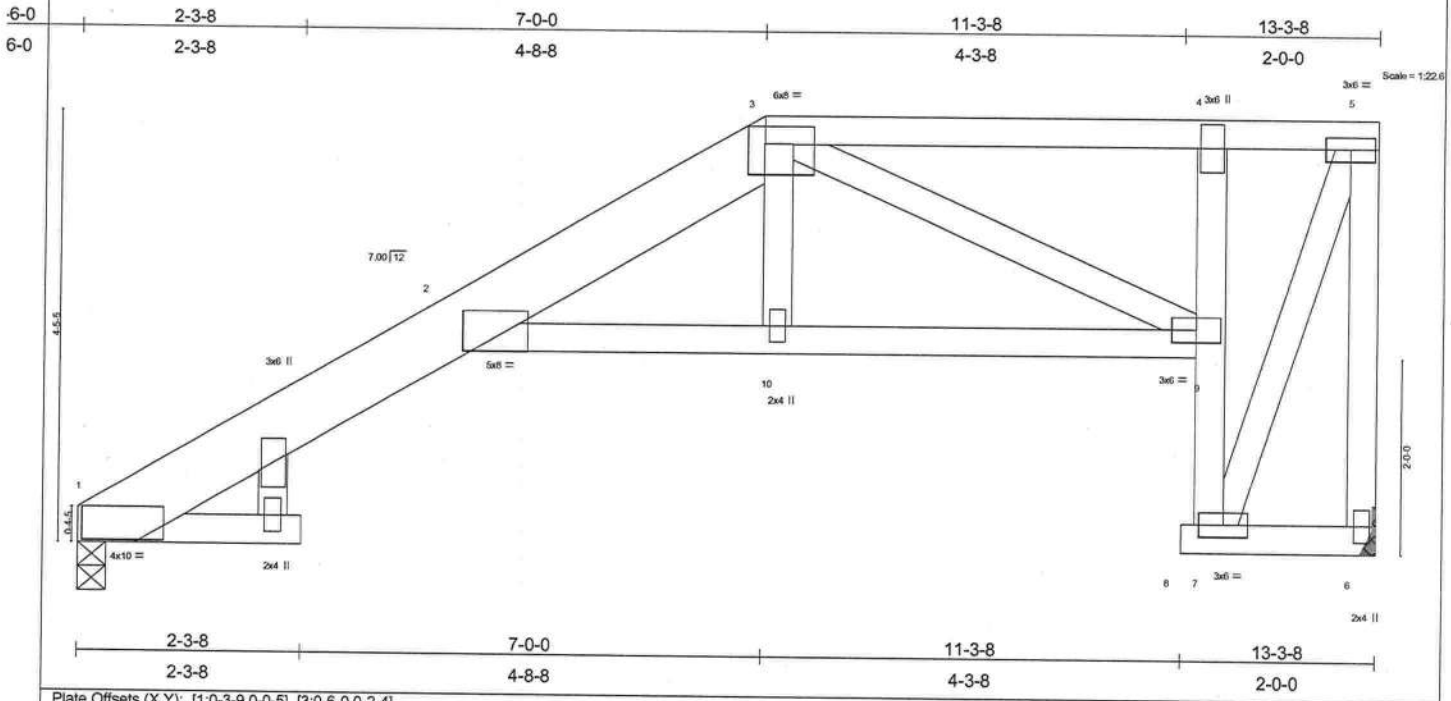



Plate Offsets (X,Y): [1:0-3-9,0-0-5], [3:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.54 | Vert(LL) | 0.17 | 2-10 | >904 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.77 | Vert(TL) | -0.27 | 2-10 | >580 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.28 | Horz(TL) | 0.30 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 79 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-11 oc bracing.

**REACTIONS** (lb/size) 1=486/0-3-8, 6=543/Mechanical
Max Horz 1=192(load case 5)
Max Uplift1=-144(load case 5), 6=-187(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-216/17, 2-3=-916/413, 3-4=-357/175, 4-5=-182/97, 5-6=-603/319
BOT CHORD 2-10=-509/933, 9-10=-511/948, 7-9=-343/212, 4-9=-34/131, 7-8=0/0, 6-7=-4/10
WEBS 3-10=-23/211, 3-9=-651/365, 5-7=-264/490

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 1 and 187 lb uplift at joint 6.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T16 | SPECIAL | 2 | 1 | Job Reference (optional) |

Dwg.#090805945

Builders FirstSource, Lake City, Fl 32055

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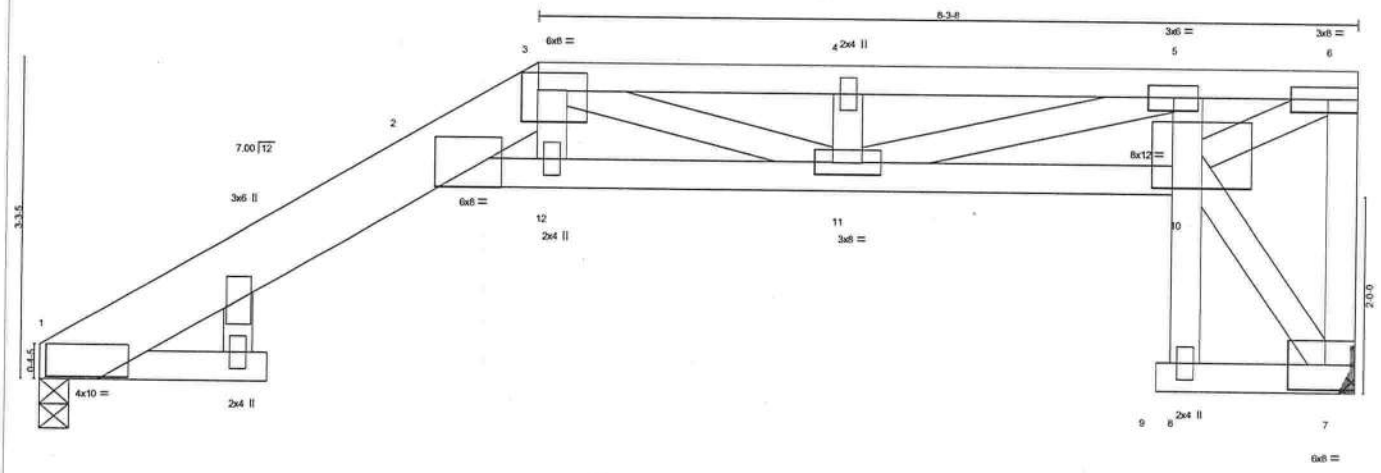
6-0 2-3-8 5-0-0 8-1-12 11-3-8 13-3-8

6-0 2-3-8 2-8-8 3-1-12 3-1-12 2-0-0

Scale = 1:22.3




2-3-8 3-3-8 5-0-0 8-1-12 11-3-8 11-5-4 13-3-8

2-3-8 1-0-0 1-8-8 3-1-12 3-1-12 0-1-12 1-10-4

Plate Offsets (X,Y): [1:0-3-12,0-0-5], [2:0-7-8,0-0-0], [3:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.73 | Vert(LL) | 0.37 | 2-12 | >425 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.79 | Vert(TL) | -0.40 | 2-12 | >394 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.52 | Horz(TL) | 0.39 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code | FBC2004/TPI2002 | (Matrix) | | | | | | | Weight: 73 lb | |

**LUMBER**

TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* T2 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3, B3 2 X 4 SYP No.1D, B5 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-9-2 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 4-0-11 oc bracing.

**REACTIONS** (lb/size) 1=749/0-3-8, 7=904/Mechanical
Max Horz 1=141(load case 4)
Max Uplift1=-514(load case 4), 7=-858(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-339/220, 2-3=-2815/2304, 3-4=-2835/2179, 4-5=-2828/2182, 5-6=-1460/1439, 6-7=-827/822

BOT CHORD 2-12=-2624/3200, 11-12=-2711/3278, 10-11=-1646/1802, 8-10=0/60, 5-10=-512/278, 8-9=0/0, 7-8=-46/44

WEBS 3-12=-534/478, 3-11=-467/667, 4-11=-208/155, 5-11=-571/1075, 6-10=-1593/1614, 7-10=-53/62

**NOTES**

1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 514 lb uplift at joint 1 and 858 lb uplift at joint 7.
6) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
7) Girder carries tie-in span(s): 4-4-2 from 5-0-0 to 11-3-8; 2-0-0 from 5-0-0 to 11-3-8
8) Girder carries hip end with 8-3-8 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
9) Girder carries hip end with 0-0-0 right side setback, 11-3-8 left side setback, and 5-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 452 lb up at 5-0-0, and 384 lb up at 11-3-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-65, 2-3=-54, 3-5=-97(F=-43), 5-6=-91(F=-37), 2-10=-30, 8-9=-50(F=-20), 7-8=-50(F=-20)
Concentrated Loads (lb)
Vert: 12=-245(F)

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T15 | MONO TRUSS | 2 | 1 | Job Reference (optional) |

Dwg.#090805946

Builders FirstSource, Lake City, Fl 32055

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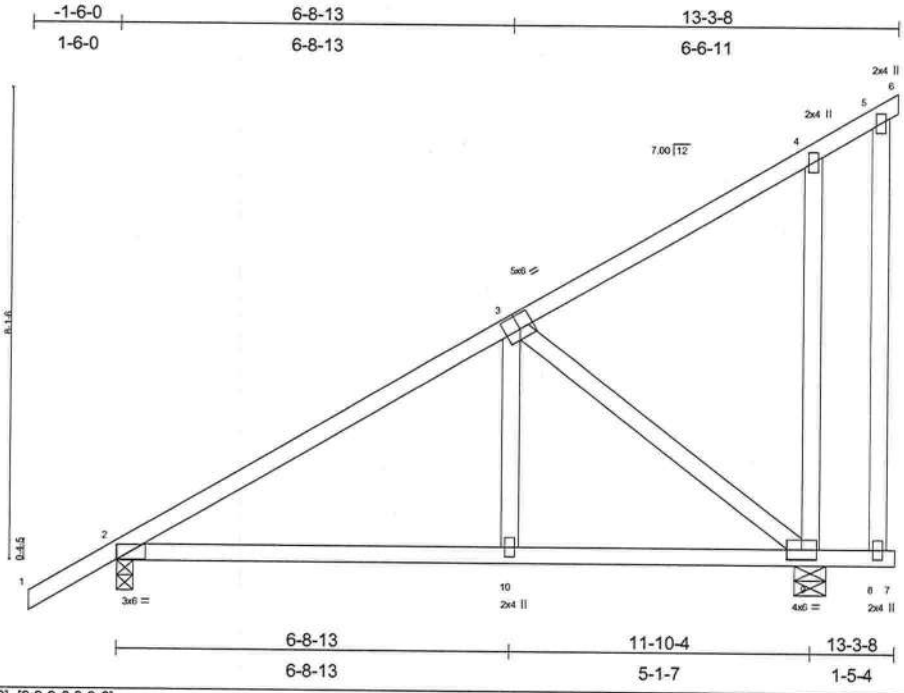


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.32 | Vert(LL) | -0.06 | 2-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.28 | Vert(TL) | -0.11 | 2-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.39 | Horz(TL) | 0.01 | 9 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 81 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=581/0-3-8, 9=601/0-6-7
Max Horz 2=412(load case 5)
Max Uplift2=-165(load case 5), 9=-343(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-573/0, 3-4=-121/53, 4-5=-38/8, 5-6=-2/0, 5-8=-17/7
BOT CHORD 2-10=-230/418, 9-10=-231/415, 8-9=-2/1, 7-8=0/0
WEBS 3-10=0/218, 3-9=-528/294, 4-9=-179/193

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 2 and 343 lb uplift at joint 9.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B. LUTZ, FL 33548**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T12 | MONO HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805947

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:28:36 2005 Page 1

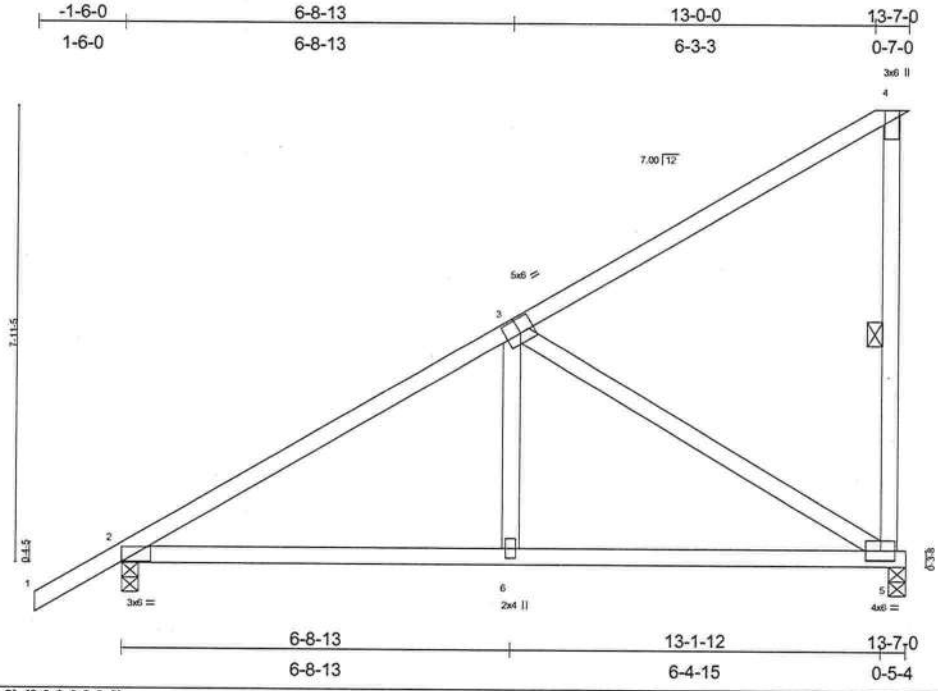


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.34 | Vert(LL) | -0.06 | 2-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.32 | Vert(TL) | -0.10 | 2-6 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.62 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 74 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

**REACTIONS** (lb/size) 2=647/0-3-8, 5=547/0-3-8
Max Horz 2=413(load case 5)
Max Uplift2=-202(load case 5), 5=-308(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-712/47, 3-4=-123/53, 4-5=-140/143
BOT CHORD 2-6=-300/541, 5-6=-300/536
WEBS 3-6=0/232, 3-5=-607/339

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 2 and 308 lb uplift at joint 5.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T10 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805948

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:26:38 2005 Page 1

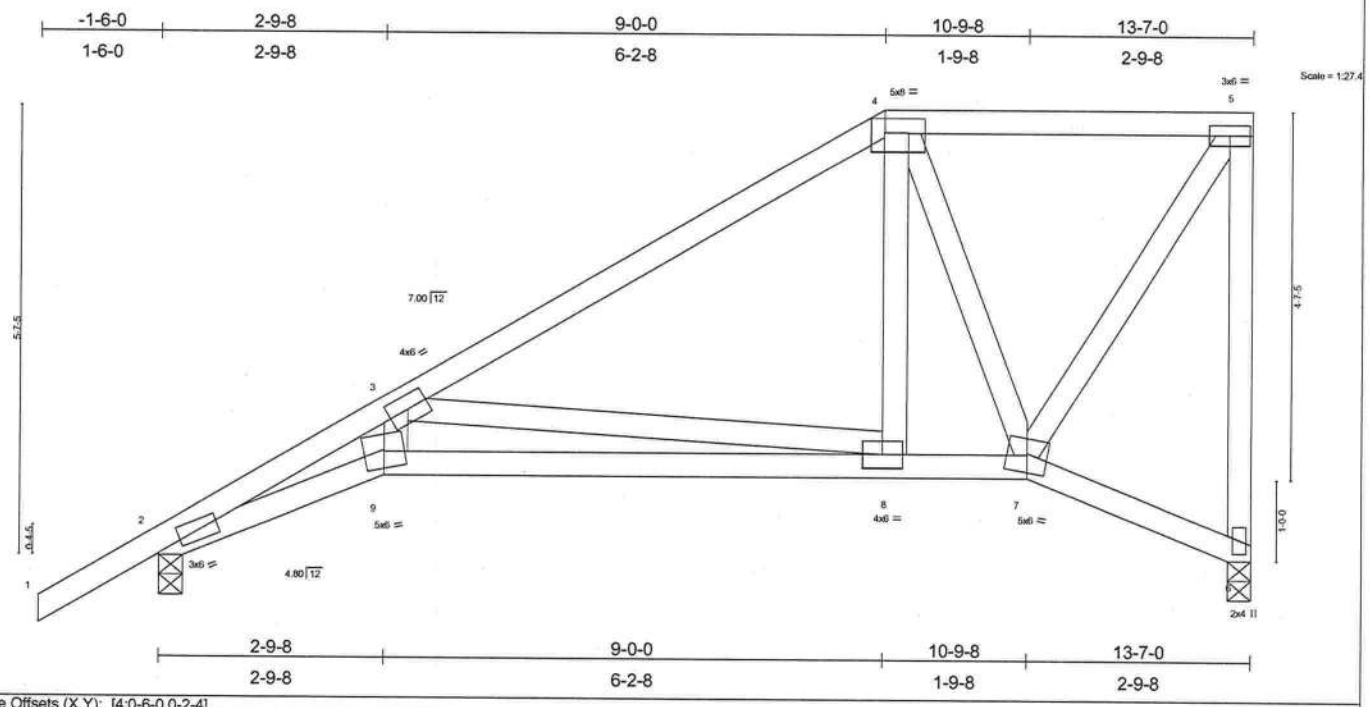


Plate Offsets (X,Y): [4:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.30 | Vert(LL) | -0.13 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.47 | Vert(TL) | -0.21 | 8-9 | >756 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.75 | Horz(TL) | 0.10 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 83 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-15 oc bracing.

**REACTIONS** (lb/size) 6=553/0-3-8, 2=653/0-3-8
Max Horz 2=298(load case 5)
Max Uplift6=-180(load case 5), 2=-246(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-2025/775, 3-4=-567/181, 4-5=-268/134, 5-6=-504/276
BOT CHORD 2-9=-900/1796, 8-9=-807/1526, 7-8=-234/456, 6-7=-13/16
WEBS 3-9=-244/748, 3-8=-1090/596, 4-8=-71/366, 4-7=-491/260, 5-7=-249/495

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 6 and 246 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T09 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805949

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:27:02 2005 Page 1

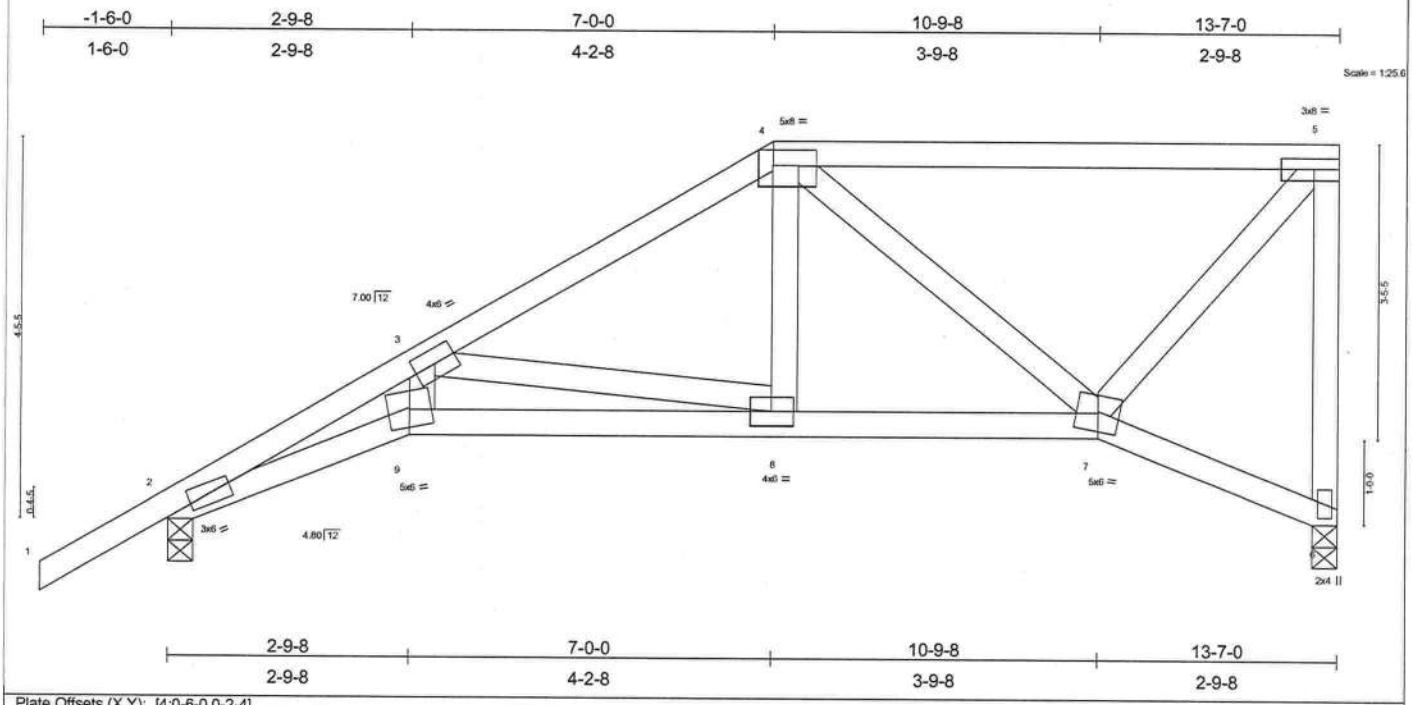


Plate Offsets (X,Y): [4:0-6-0,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | -0.08 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.38 | Vert(TL) | -0.13 | 8-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.25 | Horz(TL) | 0.08 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 75 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-7-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-11 oc bracing.

**REACTIONS** (lb/size) 6=553/0-3-8, 2=653/0-3-8
Max Horz 2=244(load case 5)
Max Uplift6=-191(load case 4), 2=-249(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-1855/714, 3-4=-768/313, 4-5=-333/140, 5-6=-511/264
BOT CHORD 2-9=-777/1623, 8-9=-690/1385, 7-8=-337/664, 6-7=-34/51
WEBS 3-9=-220/645, 3-8=-747/381, 4-8=-62/302, 4-7=-430/256, 5-7=-170/455

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 6 and 249 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T08 | SPECIAL | 1 | 1 | Job Reference (optional) |

Dwg.#090805950

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:26:12 2005 Page 1

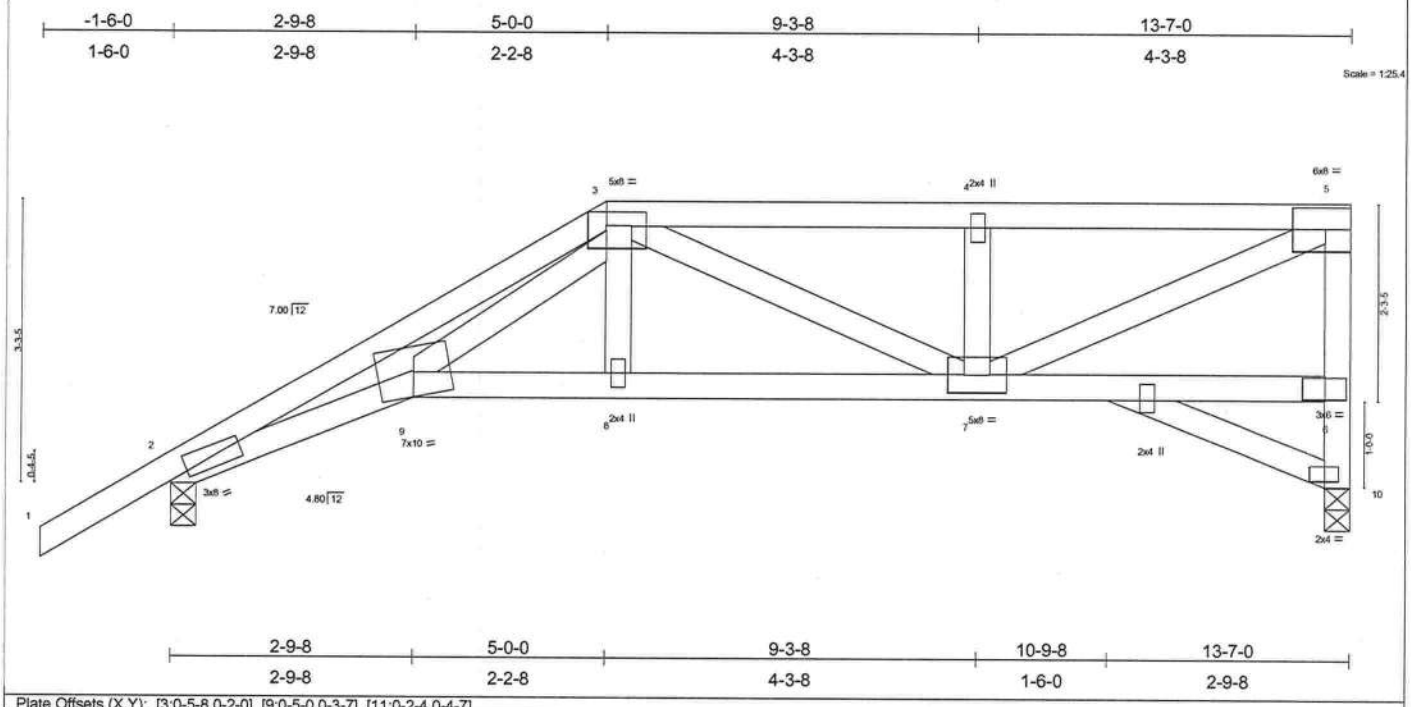


Plate Offsets (X,Y): [3:0-5-8,0-2-0], [9:0-5-0,0-3-7], [11:0-2-4,0-4-7]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | -0.14 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.96 | Vert(TL) | -0.22 | 8-9 | >721 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.49 | Horz(TL) | 0.15 | 10 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 74 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-5-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-14 oc bracing.

**REACTIONS** (lb/size) 10=971/0-3-8, 2=956/0-3-8
Max Horz 2=190(load case 4)
Max Uplift10=-502(load case 2), 2=-453(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-3080/1419, 3-4=-1498/762, 4-5=-1498/762, 6-10=-971/502, 5-6=-852/506
BOT CHORD 2-9=-1317/2737, 8-9=-843/1669, 7-8=-859/1710, 6-7=-56/108
WEBS 3-8=-217/553, 3-7=-233/202, 4-7=-390/424, 5-7=-785/1541, 3-9=-535/1218

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 10 and 453 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 170 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-91(F=-37), 2-9=-30, 8-9=-30, 6-8=-50(F=-20)
Concentrated Loads (lb)
Vert: 8=-245(F)

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T07G | GABLE | 1 | 1 | Job Reference (optional) |

Dwg.#090805951

Builders FirstSource, Lake City, Fl 32055

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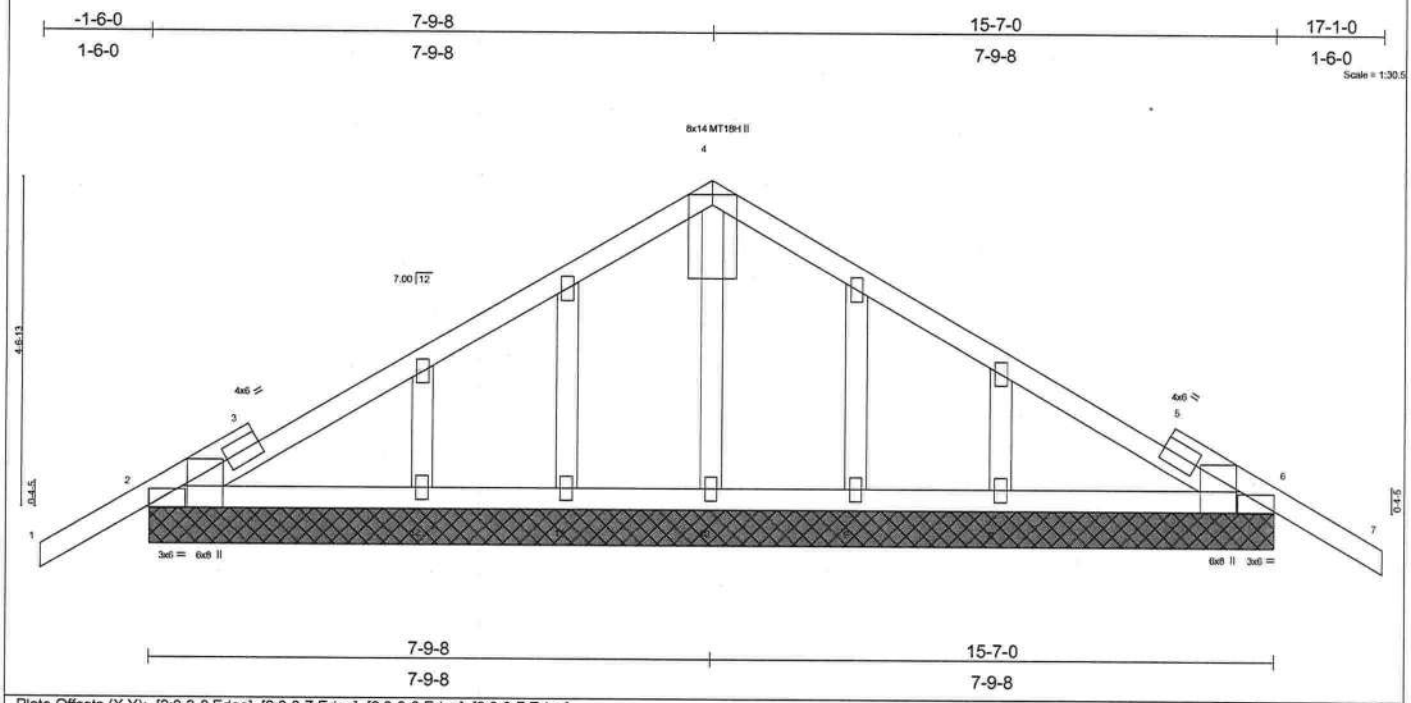


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [6:0-3-8,Edge], [6:0-0-7,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.64 | Vert(LL) | 0.09 | 7 | n/r | 90 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.79 | Vert(TL) | 0.14 | 7 | n/r | 80 | MT18H | 244/190 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.00 | Horz(TL) | 0.01 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 78 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=712/15-7-0, 6=712/15-7-0, 10=132/15-7-0, 11=-84/15-7-0, 12=348/15-7-0, 9=-84/15-7-0, 8=348/15-7-0
Max Horz 2=-152(load case 3)
Max Uplift2=-344(load case 5), 6=-344(load case 6), 10=-19(load case 5), 11=-87(load case 7), 12=-143(load case 5), 9=-87(load case 8), 8=-142(load case 6)
Max Grav 2=712(load case 1), 6=712(load case 1), 10=132(load case 1), 11=90(load case 5), 12=362(load case 7), 9=89(load case 6), 8=362(load case 8)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3/63, 2-3=-826/358, 3-4=-716/374, 4-5=-716/374, 5-6=-826/358, 6-7=-3/63
BOT CHORD 2-12=-232/618, 11-12=-232/618, 10-11=-232/618, 9-10=-232/618, 8-9=-232/618, 6-8=-232/618

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 344 lb uplift at joint 2, 344 lb uplift at joint 6, 19 lb uplift at joint 10, 87 lb uplift at joint 11, 143 lb uplift at joint 12, 87 lb uplift at joint 9 and 142 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-30

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T06 | MONO TRUSS | 2 | 1 | Job Reference (optional) |

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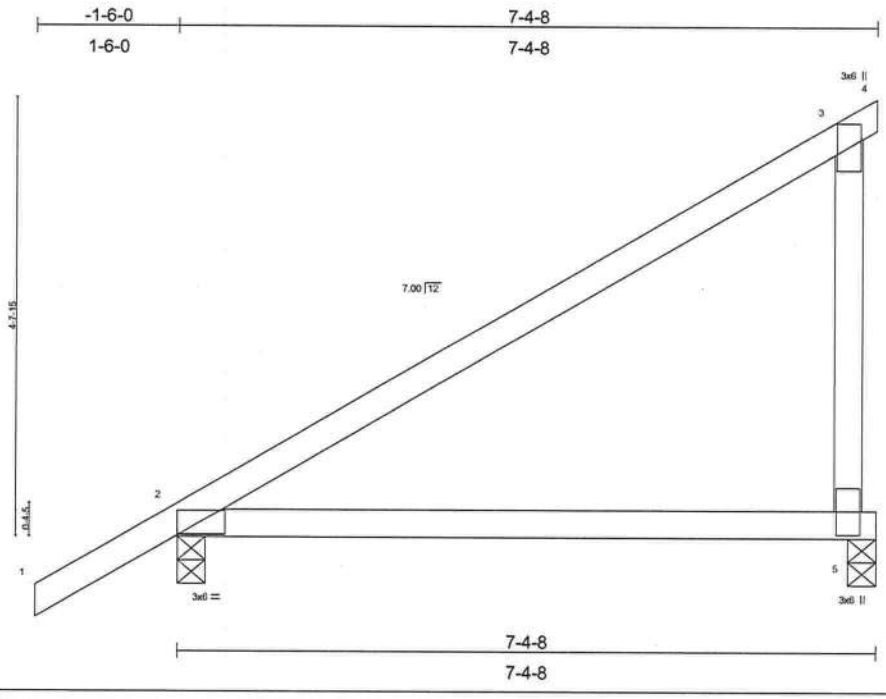


Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.73 | Vert(LL) | -0.07 | 2-5 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.27 | Vert(TL) | -0.12 | 2-5 | >682 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 32 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=391/0-3-8, 5=285/0-3-8
Max Horz 2=253(load case 5)
Max Uplift2=-154(load case 5), 5=-167(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-188/5, 3-4=-2/0, 3-5=-153/166
BOT CHORD 2-5=-48/84

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2 and 167 lb uplift at joint 5.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T05 | COMMON | 4 | 1 | Job Reference (optional) |

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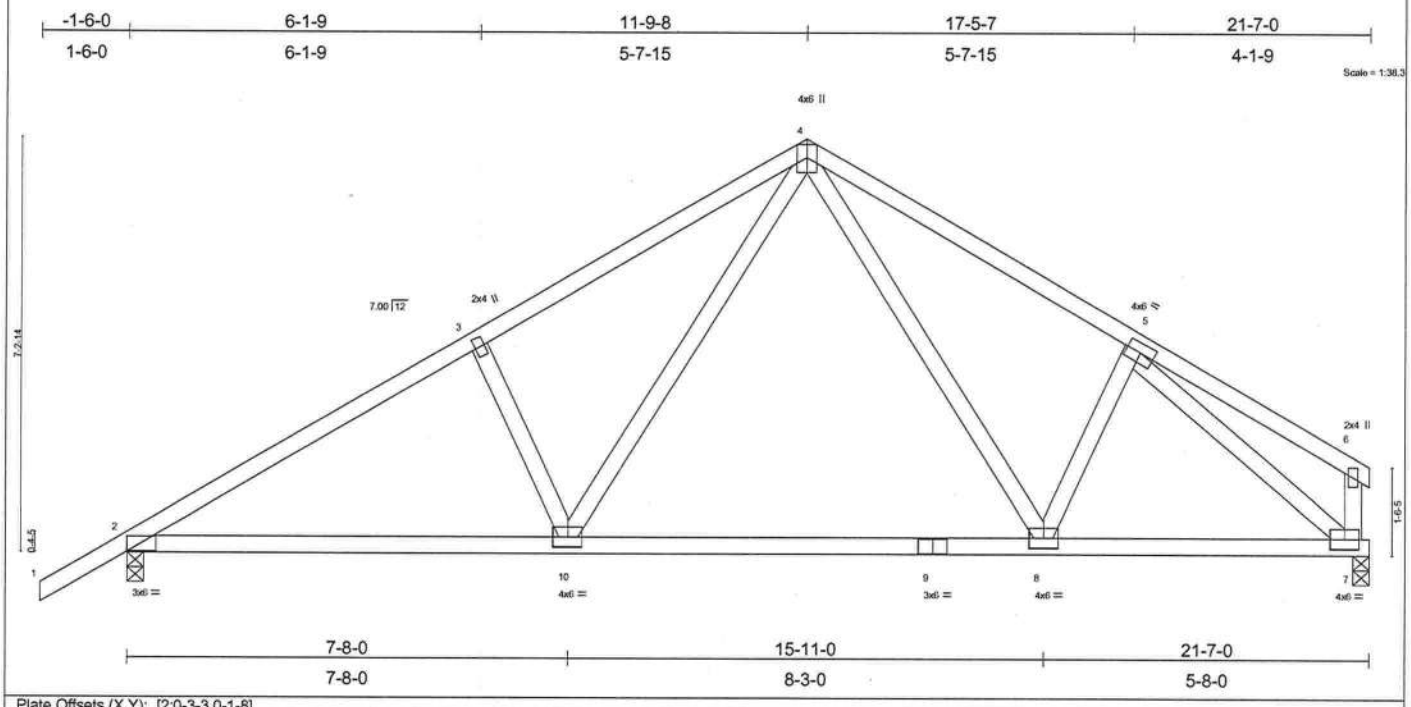


Plate Offsets (X,Y): [2:0-3-3,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.24 | Vert(LL) | -0.30 | 8-10 | >854 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.91 | Vert(TL) | -0.48 | 8-10 | >524 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.66 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 115 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-9-15 oc bracing.

**REACTIONS** (lb/size) 2=1166/0-3-8, 7=1112/0-3-8
Max Horz 2=254(load case 4)
Max Uplift2=-436(load case 5), 7=-336(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1747/595, 3-4=-1610/647, 4-5=-1363/567, 5-6=-90/66, 6-7=-116/83
BOT CHORD 2-10=-531/1427, 9-10=-252/893, 8-9=-252/893, 7-8=-357/1070
WEBS 3-10=-268/268, 4-10=-332/861, 4-8=-215/495, 5-8=-94/225, 5-7=-1425/479

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 436 lb uplift at joint 2 and 336 lb uplift at joint 7.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-10=-30, 8-10=-80(F=-50), 7-8=-30

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T04 | COMMON | 4 | 1 | Job Reference (optional) |

Dwg.#090805954

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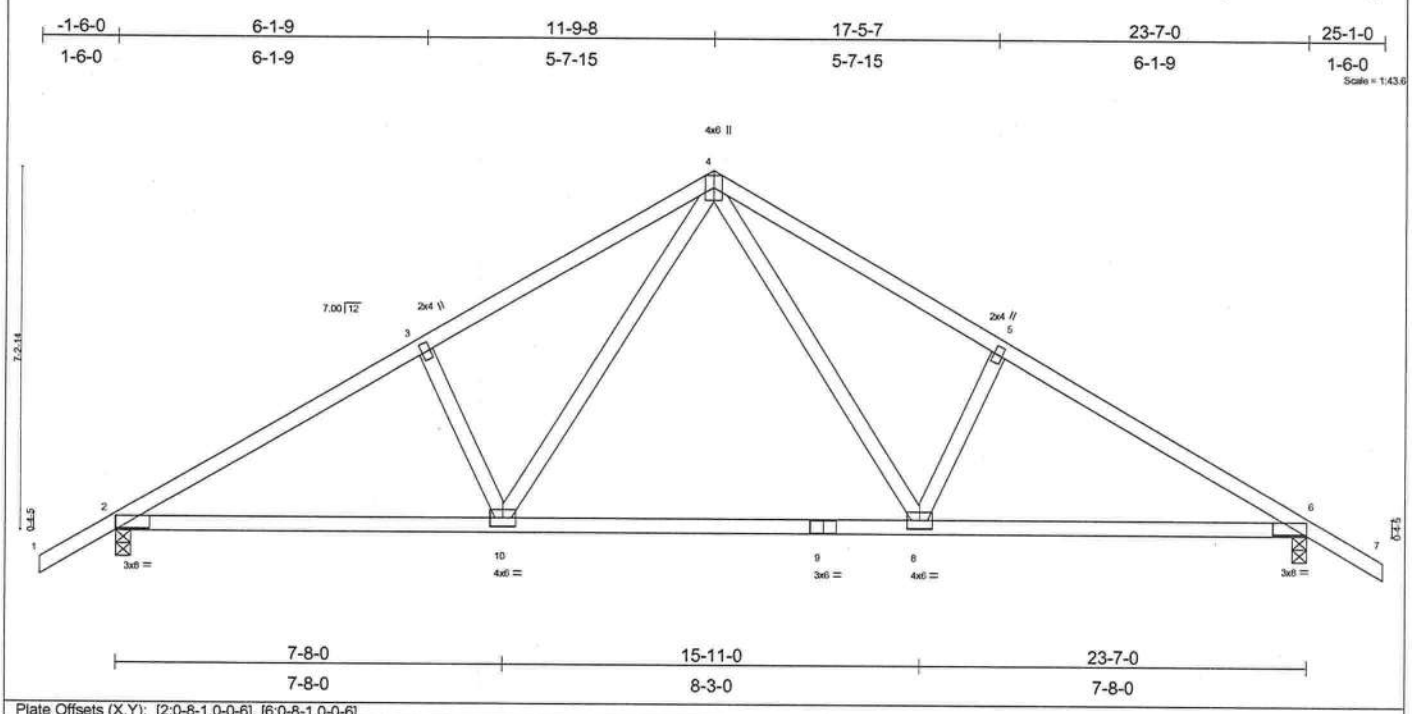


Plate Offsets (X,Y): [2:0-8-1,0-0-6], [6:0-8-1,0-0-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.25 | Vert(LL) | -0.32 | 8-10 | >879 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.94 | Vert(TL) | -0.52 | 8-10 | >541 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.39 | Horz(TL) | 0.05 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 115 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-10-5 oc bracing.

**REACTIONS** (lb/size) 2=1273/0-3-8, 6=1273/0-3-8
Max Horz 2=-245(load case 3)
Max Uplift2=-464(load case 5), 6=-464(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1954/665, 3-4=-1817/717, 4-5=-1817/717, 5-6=-1954/665, 6-7=0/40
BOT CHORD 2-10=-526/1604, 9-10=-246/1073, 8-9=-246/1073, 6-8=-423/1604
WEBS 3-10=-266/268, 4-10=-335/858, 4-8=-335/858, 5-8=-266/268

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 464 lb uplift at joint 2 and 464 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T03 | HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805955

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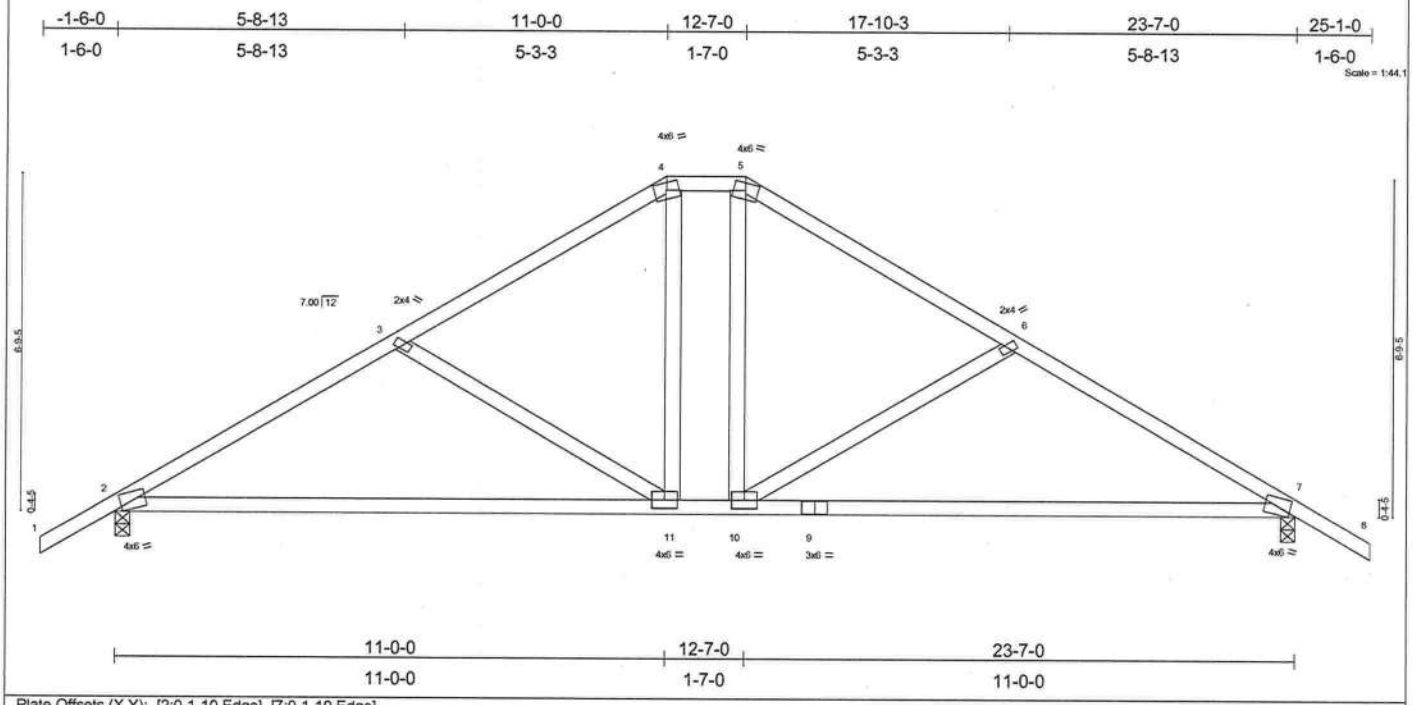


Plate Offsets (X,Y): [2:0-1-10,Edge], [7:0-1-10,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | -0.34 | 7-10 | >830 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.70 | Vert(TL) | -0.55 | 7-10 | >510 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.26 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 118 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=1067/0-3-8, 7=1067/0-3-8
Max Horz 2=229(load case 4)
Max Uplift2=-381(load case 5), 7=-381(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1444/541, 3-4=-1146/440, 4-5=-924/439, 5-6=-1146/440, 6-7=-1444/541, 7-8=0/40
BOT CHORD 2-11=-391/1206, 10-11=-138/924, 9-10=-323/1206, 7-9=-323/1206
WEBS 3-11=-362/296, 4-11=-95/368, 5-10=-95/368, 6-10=-362/296

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 381 lb uplift at joint 2 and 381 lb uplift at joint 7.

**LOAD CASE(S)** Standard


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T02 | HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805956

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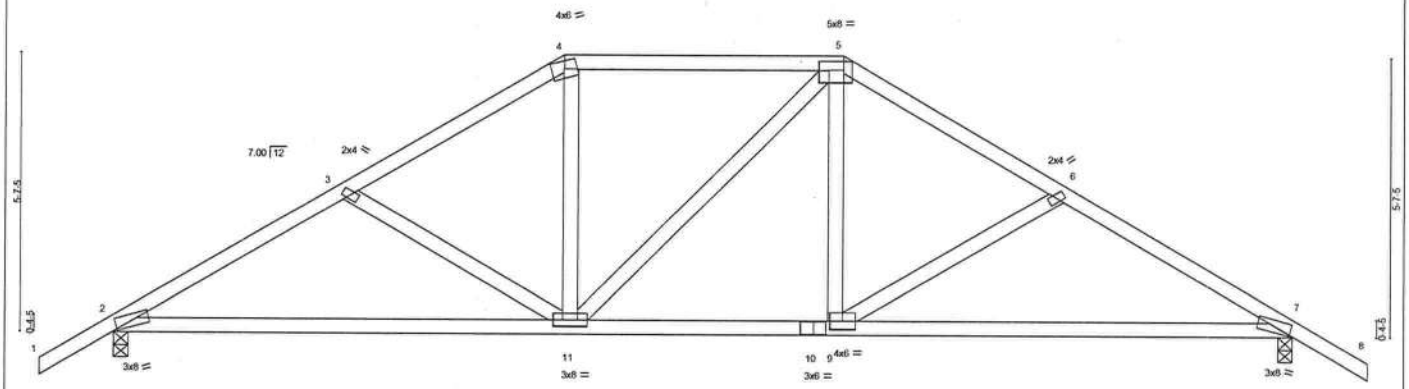
| -1-6-0 | 4-8-13 | 9-0-0 | 14-7-0 | 18-10-3 | 23-7-0 | 25-1-0 |
|---|---|---|---|---|---|---|
| 1-6-0 | 4-8-13 | 4-3-3 | 5-7-0 | 4-3-3 | 4-8-13 | 1-6-0 |

Scale = 1:44.1




| 9-0-0 | 14-7-0 | 23-7-0 |
|---|---|---|
| 9-0-0 | 5-7-0 | 9-0-0 |

Plate Offsets (X,Y): [2:0-0-14,0-0-3], [5:0-6-0,0-2-4], [7:0-0-14,0-0-3]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.23 | Vert(LL) | -0.17 | 7-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.42 | Vert(TL) | -0.29 | 7-9 | >961 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.12 | Horz(TL) | 0.04 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 120 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=1067/0-3-8, 7=1067/0-3-8
Max Horz 2=189(load case 4)
Max Uplift2=-364(load case 5), 7=-364(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1490/559, 3-4=-1266/490, 4-5=-1051/476, 5-6=-1265/490, 6-7=-1490/559, 7-8=0/40
BOT CHORD 2-11=-369/1245, 10-11=-190/1050, 9-10=-190/1050, 7-9=-347/1245
WEBS 3-11=-237/218, 4-11=-60/337, 5-11=-126/126, 5-9=-67/337, 6-9=-238/218

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 364 lb uplift at joint 2 and 364 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | T01 | HIP | 1 | 1 | Job Reference (optional) |

Dwg.#090805957

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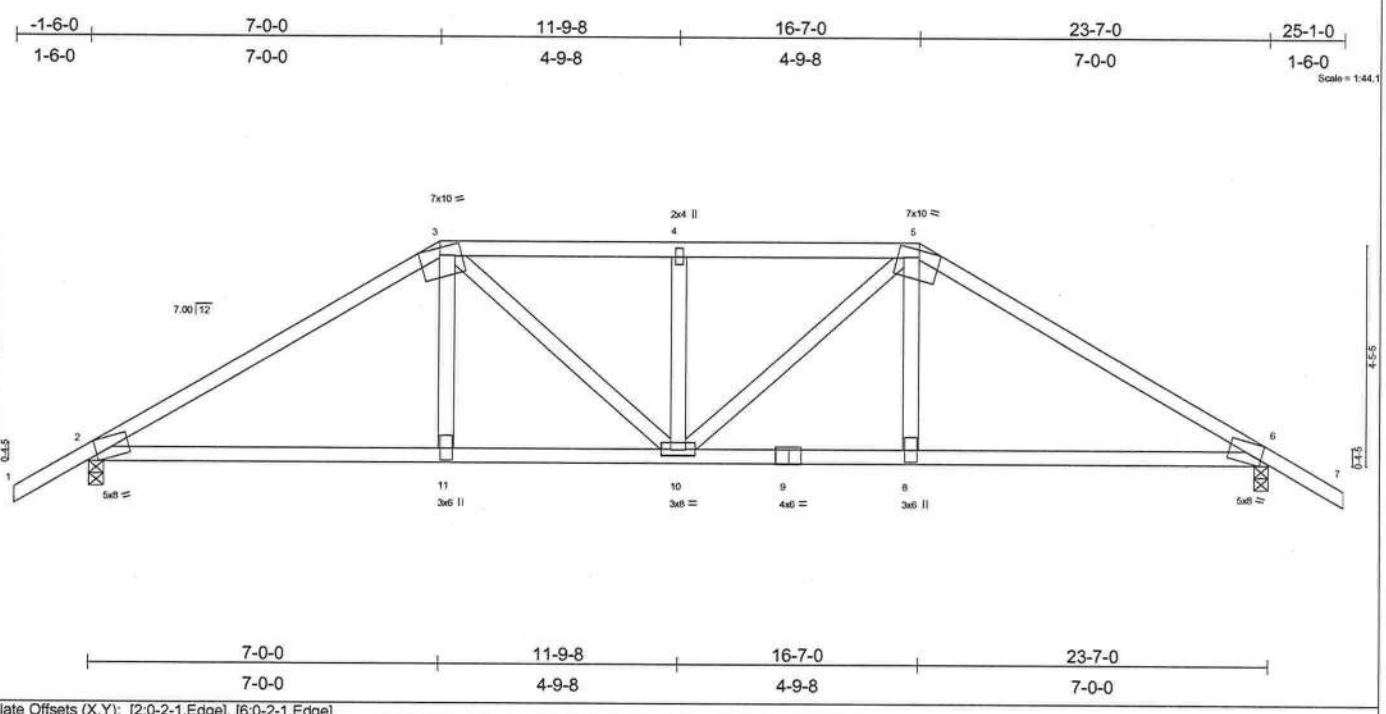


Plate Offsets (X,Y): [2:0-2-1,Edge], [6:0-2-1,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.55 | Vert(LL) | -0.16 | 8-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.76 | Vert(TL) | -0.25 | 8-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.37 | Horz(TL) | 0.10 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 114 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-13 oc bracing.

**REACTIONS** (lb/size) 2=2075/0-3-8, 6=2075/0-3-8
Max Horz 2=-148(load case 2)
Max Uplift2=-1046(load case 4), 6=-1046(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-3450/1789, 3-4=-3379/1871, 4-5=-3379/1871, 5-6=-3450/1789, 6-7=0/40
BOT CHORD 2-11=-1589/2882, 10-11=-1601/2912, 9-10=-1453/2912, 8-9=-1453/2912, 6-8=-1441/2882
WEBS 3-11=-333/828, 3-10=-555/728, 4-10=-561/650, 5-10=-556/728, 5-8=-334/828

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1046 lb uplift at joint 2 and 1046 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 374 lb up at 16-7-0, and 539 lb down and 374 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-11=-30, 8-11=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-539(F)

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | PB3 | HIP PIGGYBACK | 2 | 1 | Job Reference (optional) |

Dwg.#090805958

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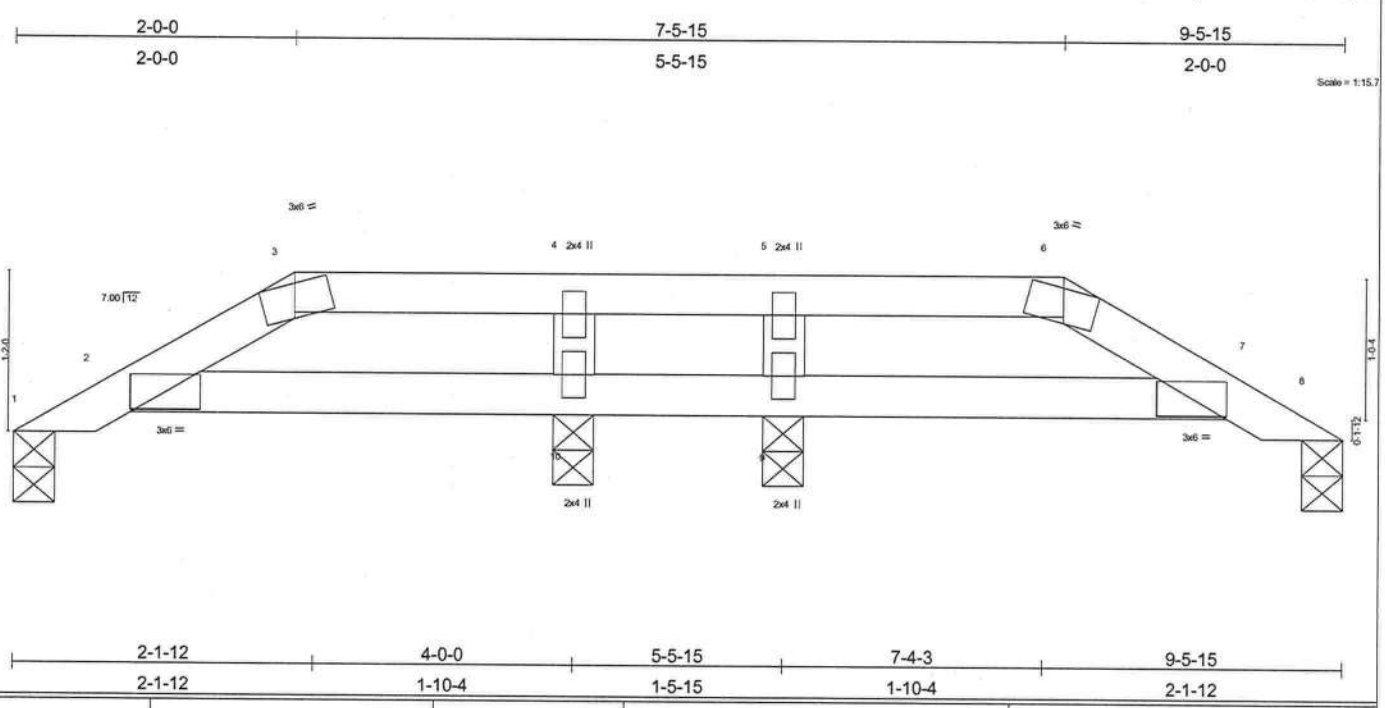


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.14 | Vert(LL) | 0.01 | 2-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.10 | Vert(TL) | -0.01 | 2-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.03 | Horz(TL) | 0.01 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 28 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=154/0-3-8, 8=154/0-3-8, 10=220/0-3-8, 9=220/0-3-8
Max Horz 1=-38(load case 3)
Max Uplift1=-50(load case 5), 8=-53(load case 6), 10=-120(load case 4), 9=-114(load case 3)
Max Grav 1=154(load case 1), 8=154(load case 1), 10=242(load case 7), 9=242(load case 8)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-72/46, 2-3=-161/125, 3-4=-136/124, 4-5=-136/124, 5-6=-136/124, 6-7=-161/125, 7-8=-72/46
BOT CHORD 2-10=-70/136, 9-10=-70/136, 7-9=-70/136
WEBS 4-10=-138/117, 5-9=-138/117

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 53 lb uplift at joint 8, 120 lb uplift at joint 10 and 114 lb uplift at joint 9.

**LOAD CASE(S)** Standard


**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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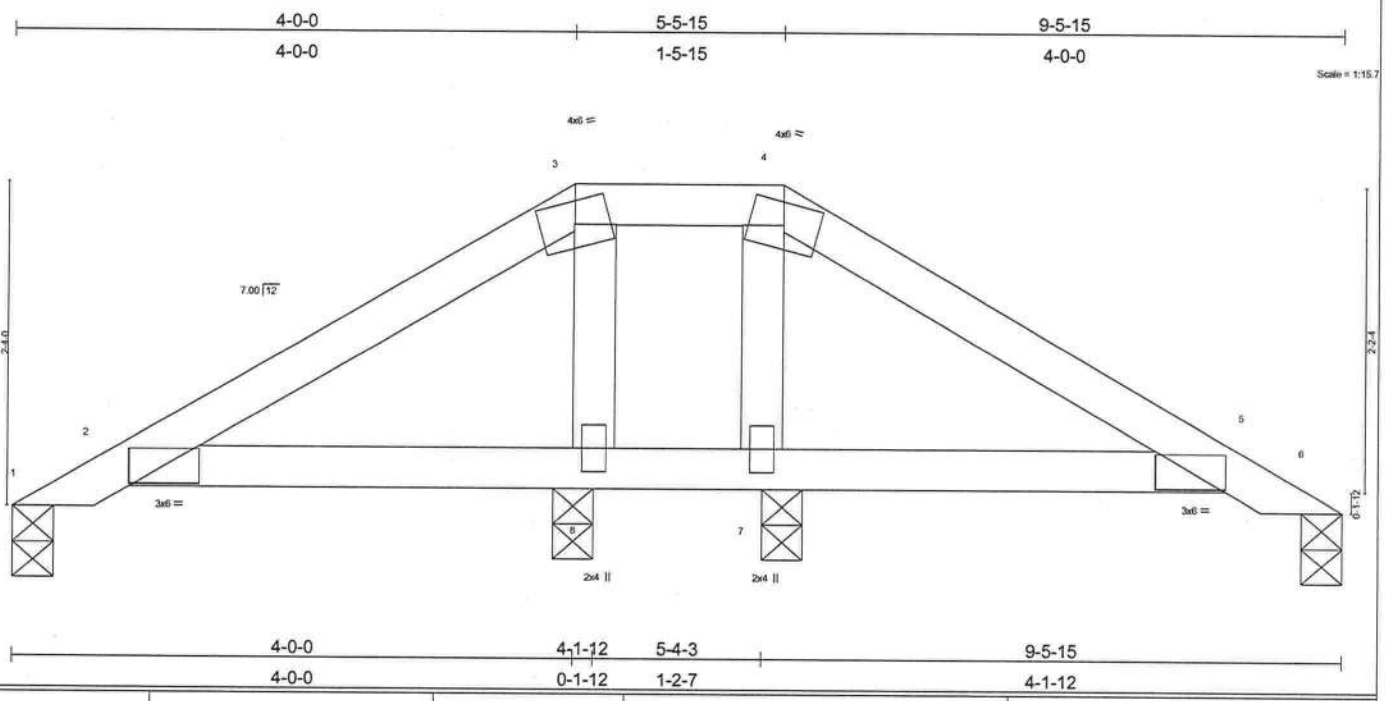
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | PB2 | HIP PIGGYBACK | 2 | 1 | Job Reference (optional) |

Dwg.#090805959

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:16:42 2005 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.11 | Vert(LL) | 0.01 | 2-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.08 | Vert(TL) | -0.01 | 2-8 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.04 | Horz(TL) | 0.00 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 32 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 1=35/0-3-8, 6=35/0-3-8, 8=338/0-3-8, 7=338/0-3-8
Max Horz 1=78(load case 4)
Max Uplift1=-10(load case 6), 6=-32(load case 3), 8=-152(load case 5), 7=-133(load case 6)
Max Grav 1=58(load case 7), 6=58(load case 8), 8=362(load case 7), 7=362(load case 8)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-69/74, 2-3=-96/276, 3-4=-41/214, 4-5=-96/276, 5-6=-26/21
BOT CHORD 2-8=-195/139, 7-8=-214/151, 5-7=-195/139
WEBS 3-8=-255/153, 4-7=-255/153

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1, 32 lb uplift at joint 6, 152 lb uplift at joint 8 and 133 lb uplift at joint 7.

**LOAD CASE(S)** Standard

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | PB1 | PIGGYBACK | 7 | 1 | Job Reference (optional) |

Dwg.#090805960

Builders FirstSource, Lake City, Fl 32055

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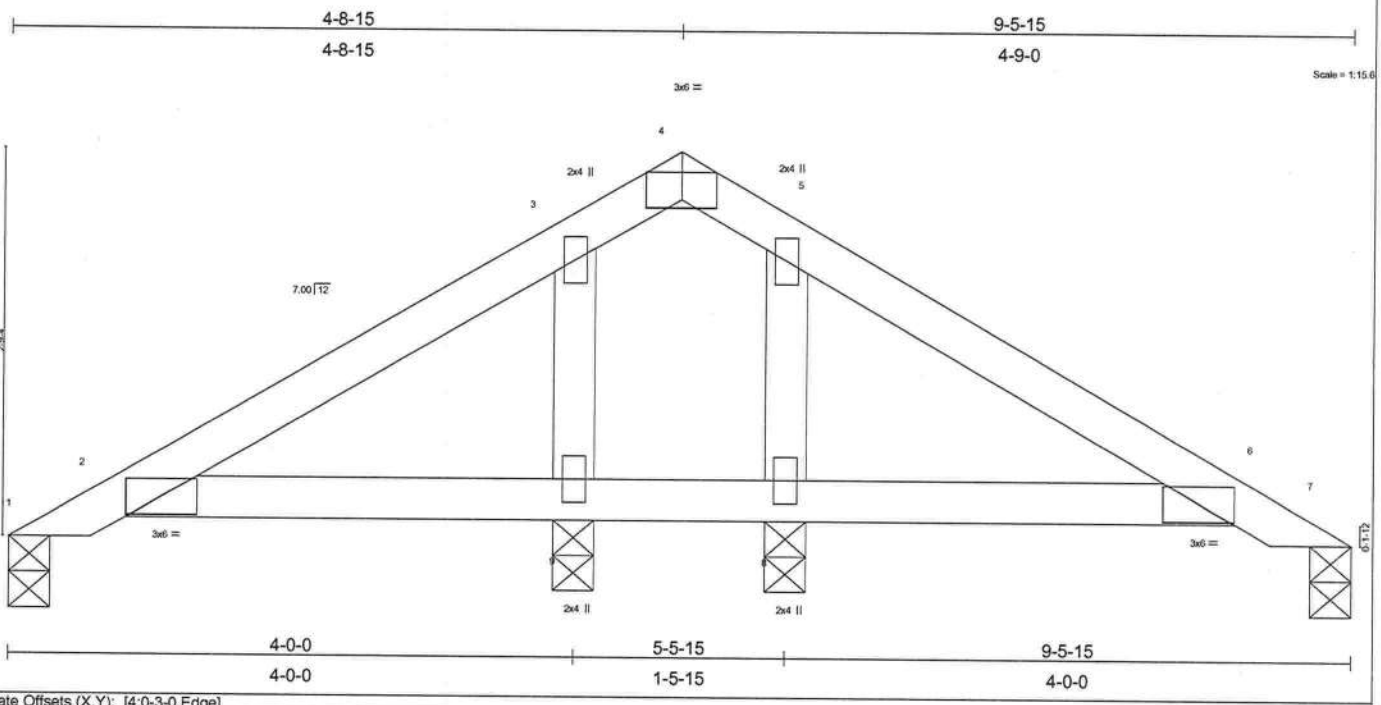


Plate Offsets (X,Y): [4:0-3-0,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.12 | Vert(LL) | 0.01 | 2-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.08 | Vert(TL) | -0.01 | 2-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.04 | Horz(TL) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 33 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=40/0-3-8, 7=40/0-3-8, 9=334/0-3-8, 8=334/0-3-8
Max Horz 1=-93(load case 3)
Max Uplift1=-7(load case 6), 7=-21(load case 3), 9=-170(load case 5), 8=-150(load case 6)
Max Grav 1=60(load case 7), 7=60(load case 8), 9=348(load case 7), 8=348(load case 8)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-88/89, 2-3=-122/257, 3-4=-31/152, 4-5=-35/152, 5-6=-122/257, 6-7=-27/15
BOT CHORD 2-9=-176/163, 8-9=-176/163, 6-8=-176/163
WEBS 3-9=-247/160, 5-8=-247/153

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 21 lb uplift at joint 7, 170 lb uplift at joint 9 and 150 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | HJ7A | SPECIAL | 2 | 1 | Job Reference (optional) |

Dwg.#090805961

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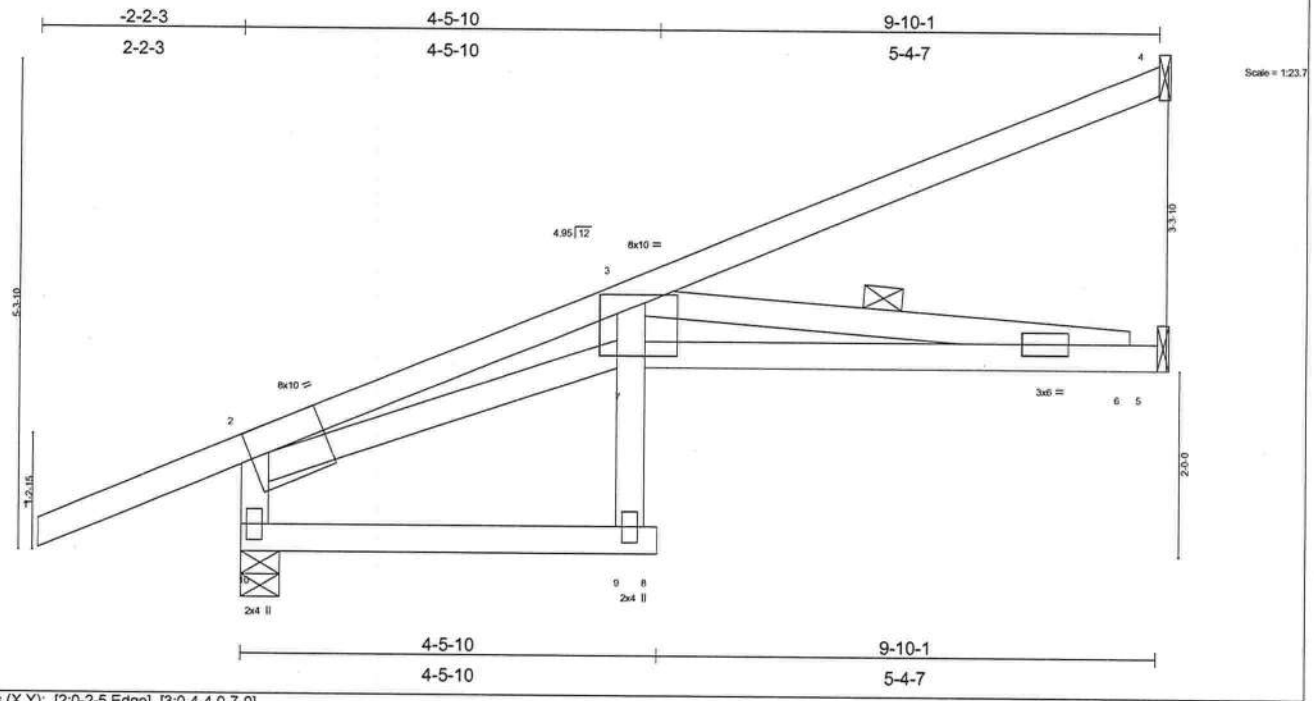


Plate Offsets (X,Y): [2:0-2-5,Edge], [3:0-4-4,0-7-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.52 | Vert(LL) | -0.16 | 6-7 | >740 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.73 | Vert(TL) | -0.26 | 6-7 | >452 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.42 | Horz(TL) | 0.11 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 53 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-3-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
5-11-10 oc bracing: 6-7.
WEBS 1 Row at midpt 3-6

**REACTIONS** (lb/size) 4=264/Mechanical, 10=484/0-5-0, 5=391/Mechanical
Max Horz 10=323(load case 4)
Max Uplift4=-237(load case 4), 10=-173(load case 4), 5=-119(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1378/523, 3-4=-122/73, 2-10=-449/205
BOT CHORD 9-10=-110/81, 8-9=0/0, 7-9=-17/37, 3-7=-127/473, 6-7=-1139/2001, 5-6=0/0
WEBS 3-6=-2017/1147, 2-7=-611/1193

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 4, 173 lb uplift at joint 10 and 119 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-4=-133(F=-39, B=-39), 10=0(F=15, B=15)-to-9=-31(F=-0, B=-0), 9=-31(F=-0, B=-0)-to-8=-33(F=-2, B=-2), 7=-31(F=-0, B=-0)-to-5=-74(F=-22, B=-22)


SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | HJ7 | MONO TRUSS | 2 | 1 | Job Reference (optional) |

Dwg.#090805962

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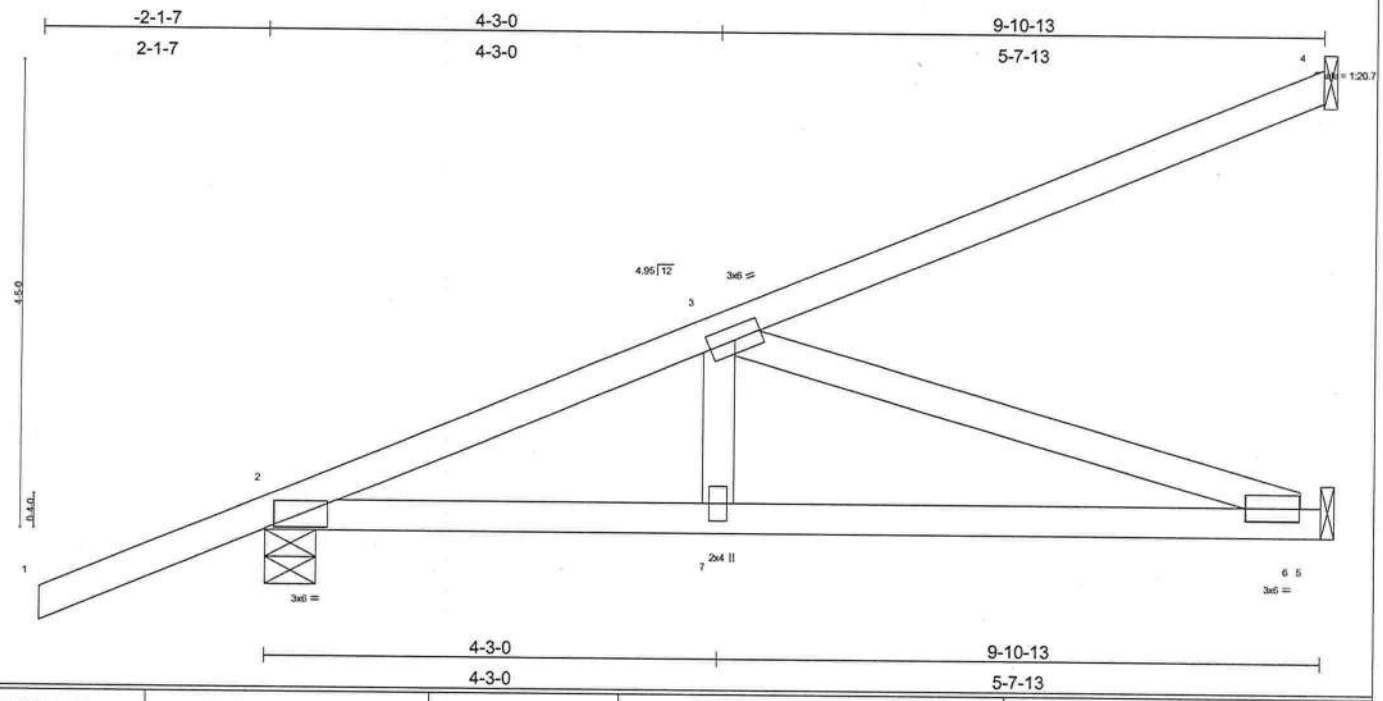


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.53 | Vert(LL) | -0.09 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.48 | Vert(TL) | -0.15 | 6-7 | >787 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.48 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 45 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=266/Mechanical, 2=488/0-5-11, 5=390/Mechanical
Max Horz 2=302(load case 4)
Max Uplift4=-261(load case 4), 2=-202(load case 4), 5=-89(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-828/116, 3-4=-131/74
BOT CHORD 2-7=-355/754, 6-7=-355/754, 5-6=0/0
WEBS 3-7=0/197, 3-6=-793/373

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 4, 202 lb uplift at joint 2 and 89 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)


**SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:**
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**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | HJ6C | SPECIAL | 2 | 1 | Job Reference (optional) |

Dwg.#090805963

Builders FirstSource, Lake City, Fl 32055

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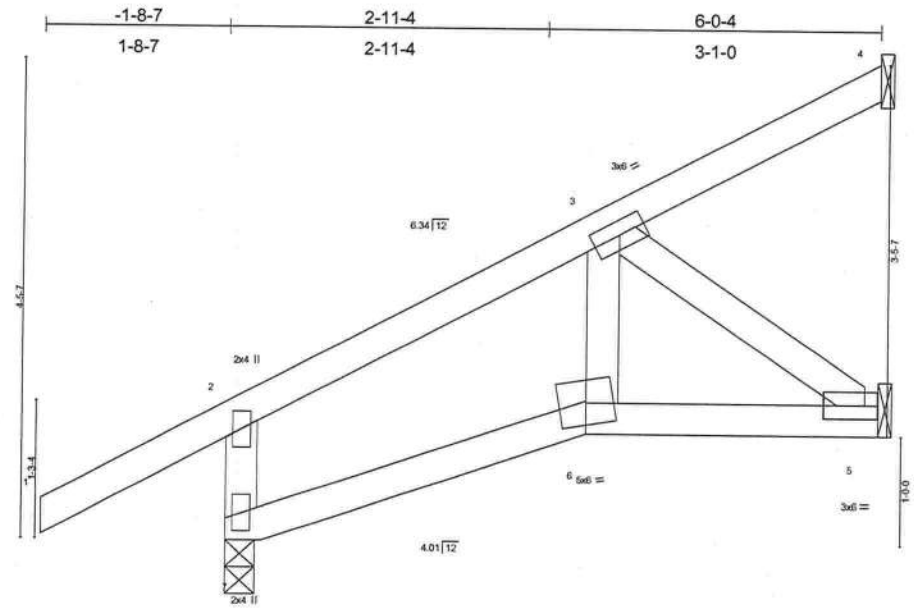


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.45 | Vert(LL) | -0.11 | 6 | >600 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.05 | Vert(TL) | -0.18 | 6 | >373 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.05 | Horz(TL) | -0.16 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 30 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 4=184/Mechanical, 7=242/0-3-5, 5=45/Mechanical
Max Horz 7=216(load case 4)
Max Uplift4=-75(load case 4), 7=-154(load case 4), 5=-145(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-60/109, 3-4=-47/77, 2-7=-229/180
BOT CHORD 6-7=-201/6, 5-6=-166/9
WEBS 3-6=-123/67, 3-5=-12/207

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 4, 154 lb uplift at joint 7 and 145 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-4=-81(F=-14, B=-14), 7=-0(F=15, B=15)-to-6=-25(F=3, B=3), 6=-25(F=3, B=3)-to-5=-45(F=-8, B=-8)

SEPTEMBER 08, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

| Job | Truss | Truss Type | Qty | Ply | GRIFFIN RES. DON REED CONST. |
|---|---|---|---|---|---|
| L128497 | HJ6 | MONO TRUSS | 1 | 1 | Job Reference (optional) |

Dwg.#090805964

Builders FirstSource, Lake City, Fl 32055

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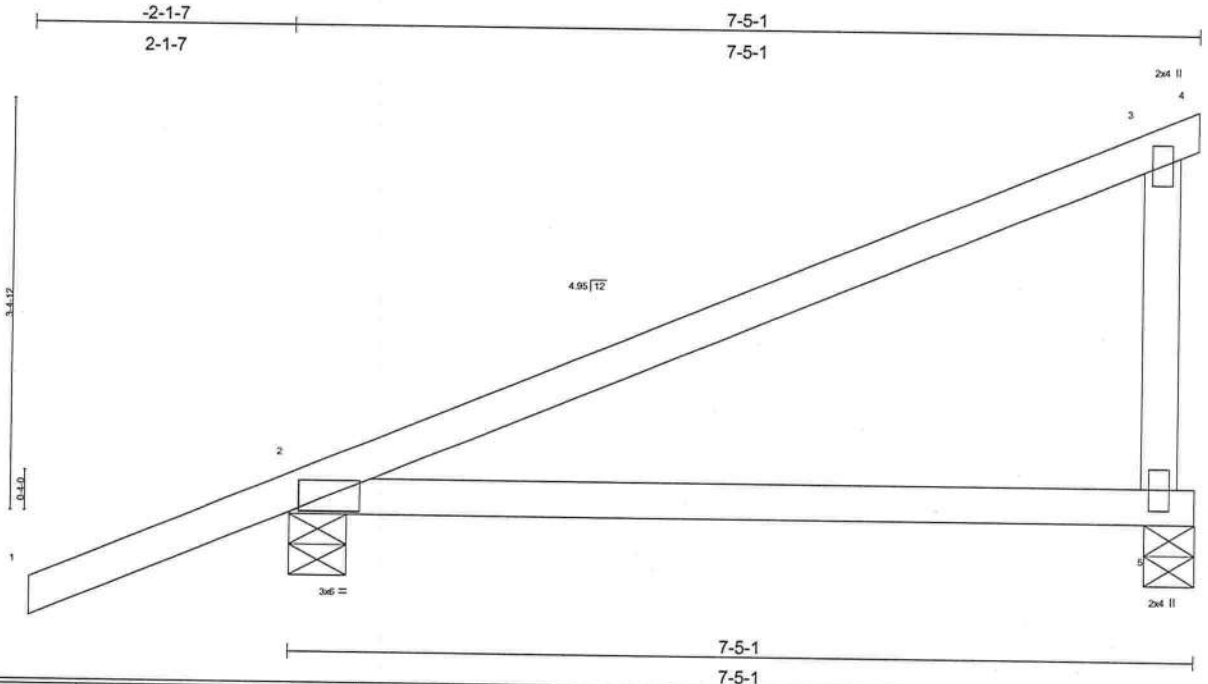


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.45 | Vert(LL) | 0.13 | 2-5 | >659 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.31 | Vert(TL) | -0.17 | 2-5 | >473 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.05 | Horz(TL) | 0.00 | | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 31 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=331/0-5-11, 5=348/0-4-15
Max Horz 2=189(load case 4)
Max Uplift2=-263(load case 4), 5=-259(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-103/56, 3-4=-6/0
BOT CHORD 2-5=0/0
WEBS 3-5=-222/203

**NOTES**
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2 and 259 lb uplift at joint 5.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-96(F=-21, B=-21), 3=-56(F=-21, B=-21)-to-4=-60(F=-23, B=-23), 2=0(F=15, B=15)-to-5=-56(F=-13, B=-13)

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