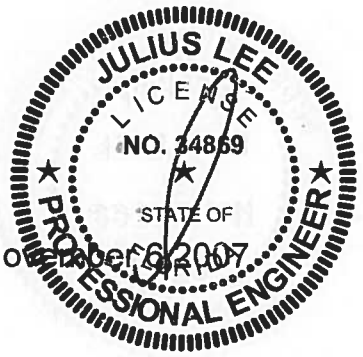
Builders FirstSource



November 6,2007

Project Information for: L258990

Builder: Lipscomb & Eagle Development Inc.
Lot : 69
Subdivision: Emerald Cove
County: Columbia
Truss Count: 34
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): 55.0 Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James M. Lipscomb Florida License No. CBC1253543
Address: 2806 US Highway West Suite 101 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1907381	EJ3	11/6/07	29	J1907409	T17	11/6/07
2	J1907382	HJ4	11/6/07	30	J1907410	T18	11/6/07
3	J1907383	PB1	11/6/07	31	J1907411	T19	11/6/07
4	J1907384	PB1A	11/6/07	32	J1907412	T20	11/6/07
5	J1907385	PB1G	11/6/07	33	J1907413	T21	11/6/07
6	J1907386	PB2	11/6/07	34	J1907414	T21G	11/6/07
7	J1907387	PB2G	11/6/07				
8	J1907388	T01	11/6/07				
9	J1907389	T01G	11/6/07				
10	J1907390	T02	11/6/07				
11	J1907391	T03	11/6/07				
12	J1907392	T04	11/6/07				
13	J1907393	T05	11/6/07				
14	J1907394	T06	11/6/07				
15	J1907395	T07G	11/6/07				
16	J1907396	T08	11/6/07				
17	J1907397	T08G	11/6/07				
18	J1907398	T09	11/6/07				
19	J1907399	T09G	11/6/07				
20	J1907400	T10	11/6/07				
21	J1907401	T11	11/6/07				
22	J1907402	T12	11/6/07				
23	J1907403	T12G	11/6/07				
24	J1907404	T13	11/6/07				
25	J1907405	T13G	11/6/07				
26	J1907406	T14	11/6/07				
27	J1907407	T15	11/6/07				
28	J1907408	T16	11/6/07				

Project Information for: L258990

Builder: Lipscomb & Eagle Development Inc.
Lot : 69
Subdivision: Emerald Cove
County: Columbia
Truss Count: 34
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

November 6,2007

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): 55.0	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
James M. Lipscomb Florida License No. CBC1253543
Address: 2806 US Highway West Suite 101 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

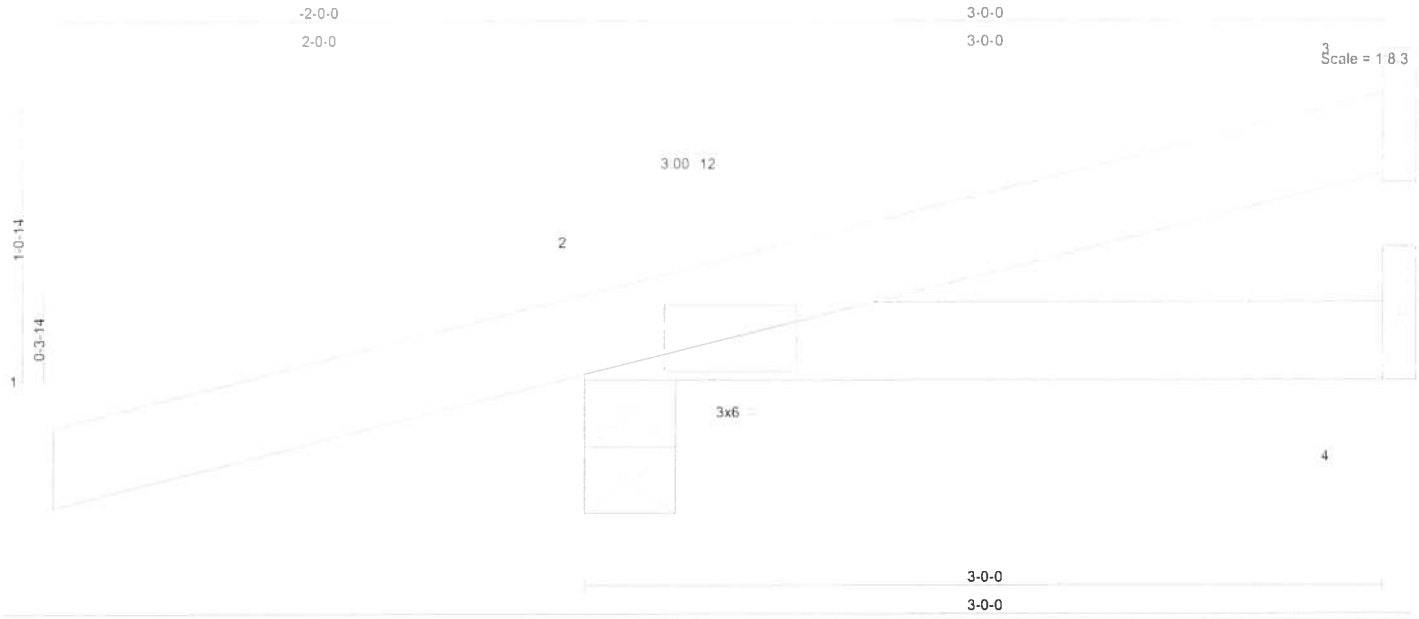
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1907381	EJ3	11/6/07	29	J1907409	T17	11/6/07
2	J1907382	HJ4	11/6/07	30	J1907410	T18	11/6/07
3	J1907383	PB1	11/6/07	31	J1907411	T19	11/6/07
4	J1907384	PB1A	11/6/07	32	J1907412	T20	11/6/07
5	J1907385	PB1G	11/6/07	33	J1907413	T21	11/6/07
6	J1907386	PB2	11/6/07	34	J1907414	T21G	11/6/07
7	J1907387	PB2G	11/6/07				
8	J1907388	T01	11/6/07				
9	J1907389	T01G	11/6/07				
10	J1907390	T02	11/6/07				
11	J1907391	T03	11/6/07				
12	J1907392	T04	11/6/07				
13	J1907393	T05	11/6/07				
14	J1907394	T06	11/6/07				
15	J1907395	T07G	11/6/07				
16	J1907396	T08	11/6/07				
17	J1907397	T08G	11/6/07				
18	J1907398	T09	11/6/07				
19	J1907399	T09G	11/6/07				
20	J1907400	T10	11/6/07				
21	J1907401	T11	11/6/07				
22	J1907402	T12	11/6/07				
23	J1907403	T12G	11/6/07				
24	J1907404	T13	11/6/07				
25	J1907405	T13G	11/6/07				
26	J1907406	T14	11/6/07				
27	J1907407	T15	11/6/07				
28	J1907408	T16	11/6/07				

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907381
L258990	EJ3	MONO TRUSS	4	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:25 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical
Max Horz 2=66(load case 4)
Max Uplift 3=-21(load case 7), 2=-246(load case 4), 4=-26(load case 4)
Max Grav 3=29(load case 1), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-32/4
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 3, 246 lb uplift at joint 2 and 26 lb uplift at joint 4.

Continued on page 2

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Builders FirstSource

LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907382
L258990	HJ4	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:25 2007 Page 1

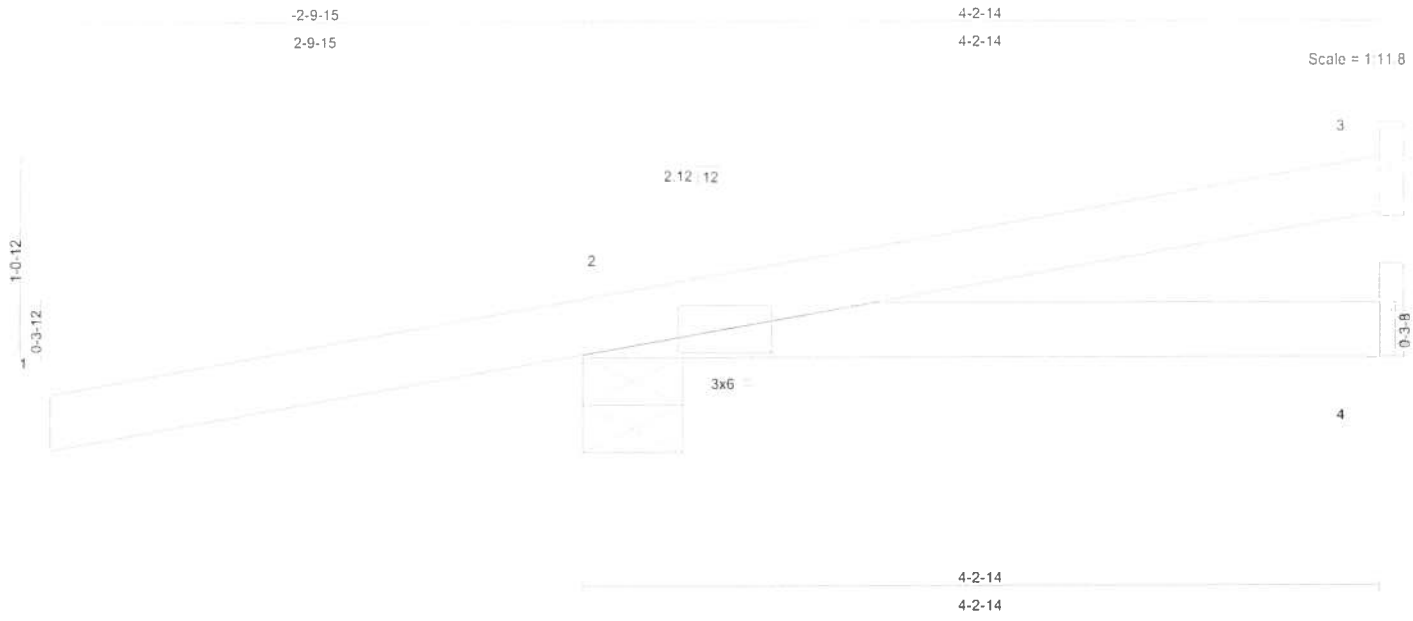


Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.02	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=11/Mechanical, 2=282/0-6-6, 4=14/Mechanical
Max Horz 2=50(load case 3)
Max Uplift 3=-5(load case 6), 2=-315(load case 3), 4=-41(load case 3)
Max Grav 3=31(load case 7), 2=282(load case 1), 4=53(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-21/5
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 3, 315 lb uplift at joint 2 and 41 lb uplift at joint 4.

Continued on page 2

November 6,2007

Builders FirstSource

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

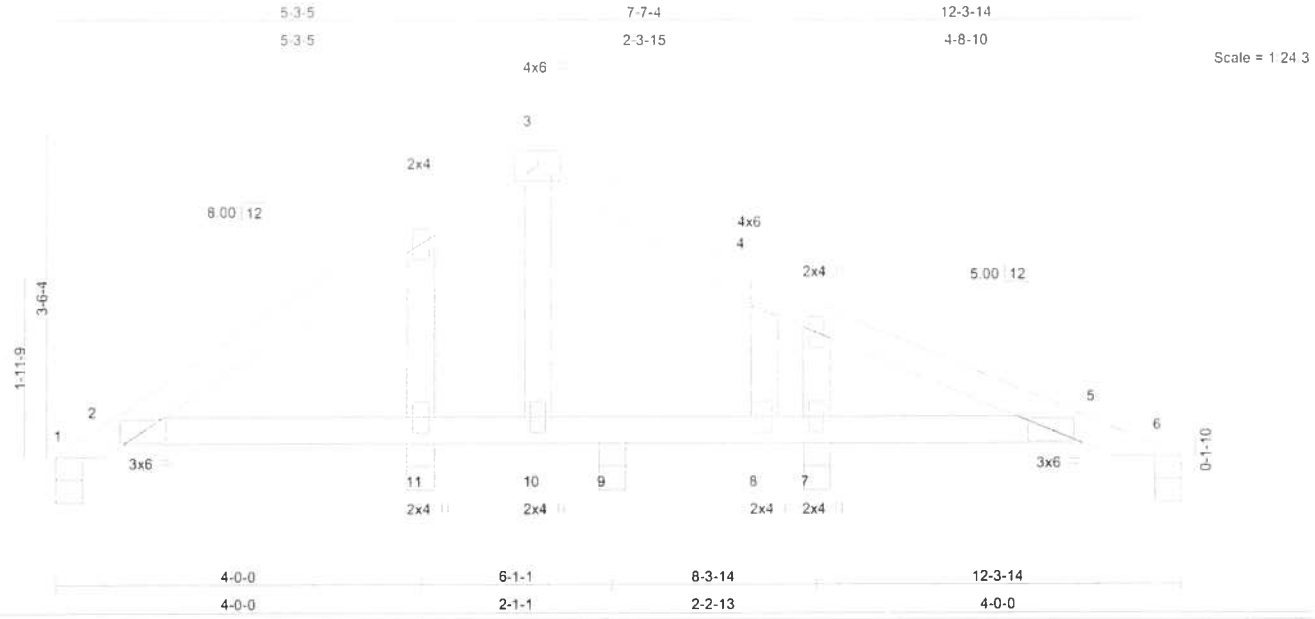
Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=-0(F=5, B=5)-to-4=-11(F=-0, B=-0)

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907383
L258990	PB1	GABLE	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:26 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	2-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.02	2-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=74/0-3-8, 7=185/0-3-8, 11=205/0-3-8, 6=84/0-3-8, 9=225/0-3-8
Max Horz 1=-99(load case 4)
Max Uplift 1=-15(load case 7), 7=-83(load case 4), 11=-60(load case 6), 6=-16(load case 5), 9=-56(load case 6)
Max Grav 1=88(load case 10), 7=223(load case 11), 11=205(load case 1), 6=85(load case 11), 9=225(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-44/93, 2-3=-68/158, 3-4=-18/126, 4-5=-116/138, 5-6=-30/22
BOT CHORD	2-11=-74/153, 10-11=-74/153, 9-10=-74/153, 8-9=-74/153, 7-8=-88/169, 5-7=-88/169
WEBS	3-10=-289/174, 4-8=-189/164

JOINT STRESS INDEX
2 = 0.68, 3 = 0.31, 4 = 0.61, 5 = 0.44, 7 = 0.00, 8 = 0.09, 10 = 0.10, 11 = 0.00, 12 = 0.00 and 13 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

November 6,2007

Builders FirstSource

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 83 lb uplift at joint 7, 60 lb uplift at joint 11, 16 lb uplift at joint 6 and 56 lb uplift at joint 9.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

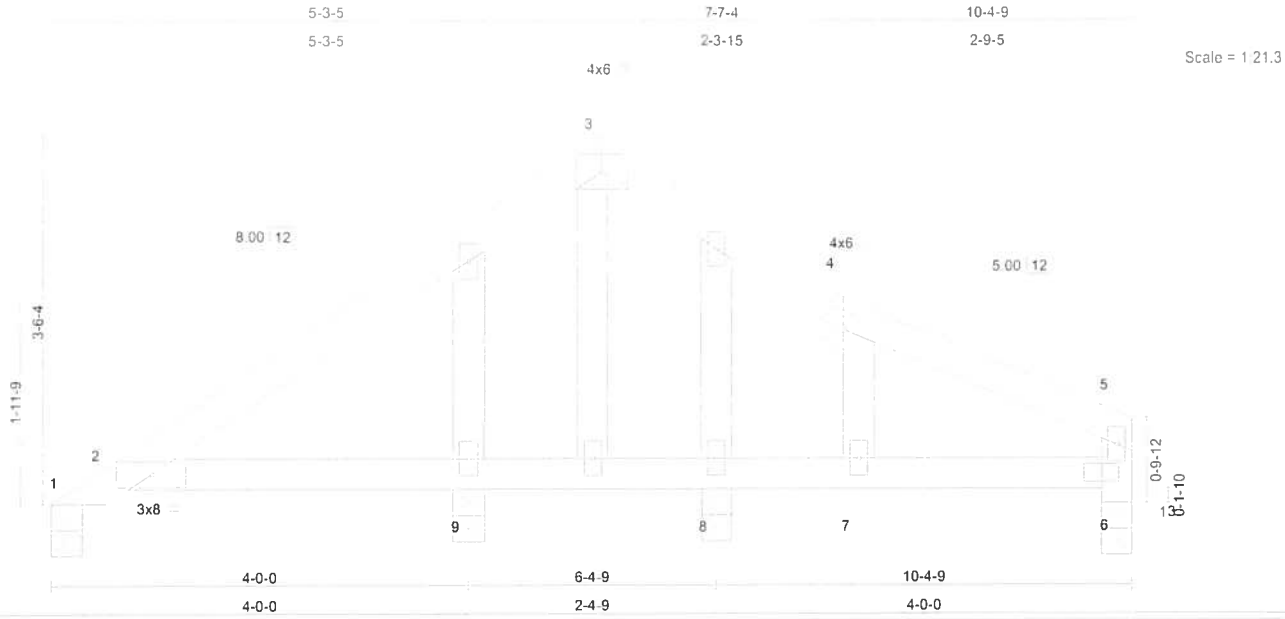
LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907384
L258990	PB1A	GABLE	6	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:27 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.03	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.03	2-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.02	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=226/0-3-8, 8=103/0-3-8, 9=94/0-3-8, 13=223/0-3-8
Max Horz 1=-76(load case 4)
Max Uplift 1=-54(load case 6), 8=-63(load case 4), 9=-27(load case 6), 13=-49(load case 6)
Max Grav 1=226(load case 1), 8=145(load case 11), 9=119(load case 10), 13=223(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-117/62, 2-3=-287/161, 3-4=-227/177, 4-5=-227/102, 6-13=-223/138, 5-6=-165/117
BOT CHORD	2-9=-65/189, 8-9=-65/189, 7-8=-65/189, 6-7=-53/178
WEBS	4-7=-127/124

JOINT STRESS INDEX

2 = 0.61, 3 = 0.44, 4 = 0.12, 5 = 0.47, 6 = 0.47, 7 = 0.07, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

November 6,2007

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 1, 63 lb uplift at joint 8, 27 lb uplift at joint 9 and 49 lb uplift at joint 13.
8) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907385
L258990	PB1G	GABLE	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 08:50:34 2007 Page 1

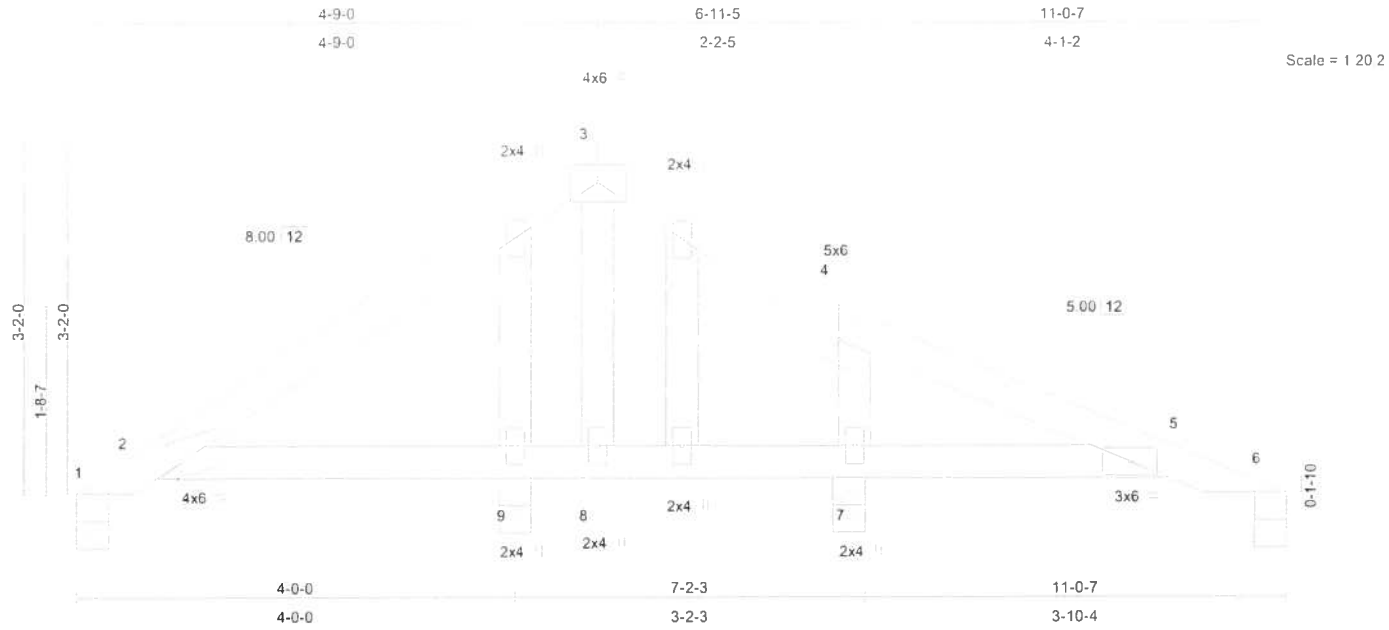


Plate Offsets (X,Y): [2:0-2-0,Edge]

LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.02	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.03	2-9	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=172/0-3-8, 6=163/0-3-8, 9=493/0-3-8, 7=506/0-3-8
Max Horz 1=-116(load case 4)
Max Uplift 1=-77(load case 7), 6=-71(load case 7), 9=-246(load case 6), 7=-272(load case 7)
Max Grav 1=180(load case 10), 6=164(load case 11), 9=493(load case 1), 7=516(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-91/117, 2-3=-93/203, 3-4=-38/146, 4-5=-147/183, 5-6=-57/39
BOT CHORD 2-9=-61/141, 8-9=-61/141, 7-8=-61/141, 5-7=-97/173
WEBS 3-8=-446/299, 4-7=-348/282

JOINT STRESS INDEX
2 = 0.74, 3 = 0.53, 4 = 0.72, 5 = 0.70, 7 = 0.16, 8 = 0.17, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Continued on page 2

Builders FirstSource

NOTES

5) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 1, 71 lb uplift at joint 6, 246 lb uplift at joint 9 and 272 lb uplift at joint 7.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-126(F=-60), 2-3=-114(F=-60), 3-4=-114(F=-60), 4-5=-114(F=-60), 5-6=-125(F=-60), 2-5=-10

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907386
L258990	PB2	GABLE	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:29 2007 Page 1

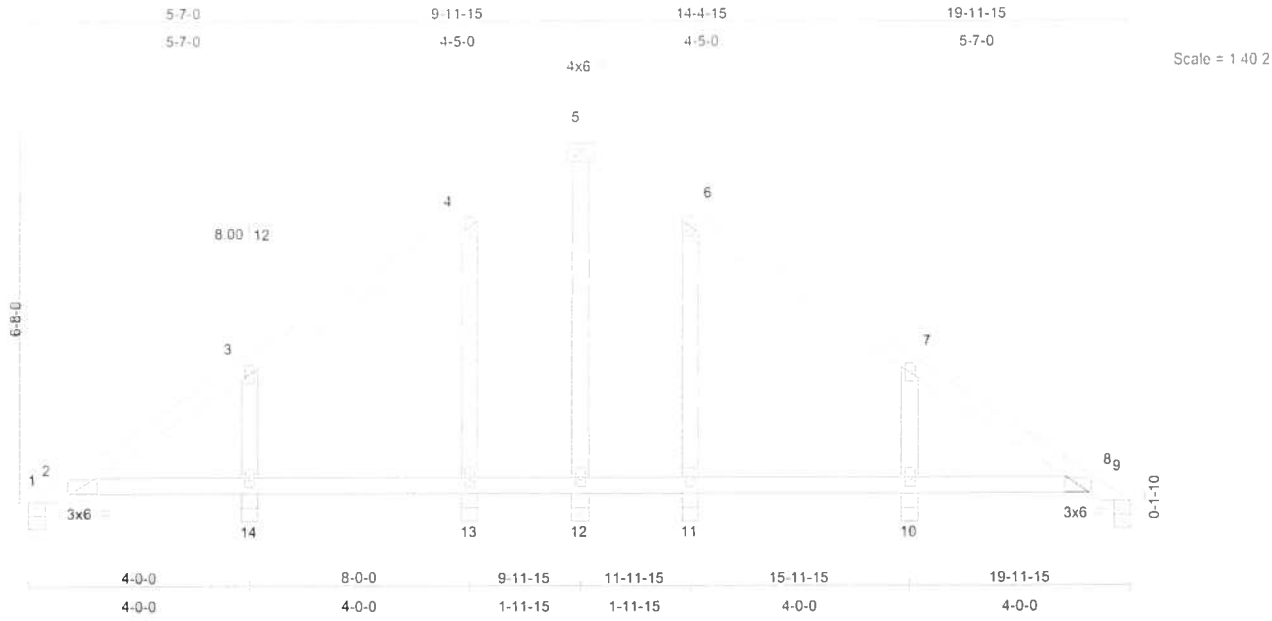

Plate Offsets (X,Y): [3:0-0-0,0-0-0], [4:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=65/0-3-8, 12=145/0-3-8, 10=283/0-3-8, 11=211/0-3-8, 14=283/0-3-8, 13=211/0-3-8, 9=65/0-3-8
Max Horz 1=-182(load case 4)
Max Uplift 1=-52(load case 4), 10=-132(load case 7), 11=-103(load case 7), 14=-138(load case 6), 13=-102(load case 6)
Max Grav 1=76(load case 5), 12=145(load case 1), 10=286(load case 11), 11=211(load case 1), 14=286(load case 10), 13=211(load case 1), 9=74(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-195/187, 2-3=-164/133, 3-4=-80/125, 4-5=-5/157, 5-6=0/157, 6-7=-16/125, 7-8=-107/130, 8-9=-37/2
BOT CHORD	2-14=-57/140, 13-14=-57/140, 12-13=-57/140, 11-12=-57/140, 10-11=-57/140, 8-10=-57/140
WEBS	5-12=-129/0, 7-10=-225/217, 6-11=-184/180, 3-14=-226/217, 4-13=-184/180

JOINT STRESS INDEX
2 = 0.24, 3 = 0.34, 4 = 0.34, 5 = 0.27, 6 = 0.33, 7 = 0.33, 8 = 0.24, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

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NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1, 132 lb uplift at joint 10, 103 lb uplift at joint 11, 138 lb uplift at joint 14 and 102 lb uplift at joint 13.
8) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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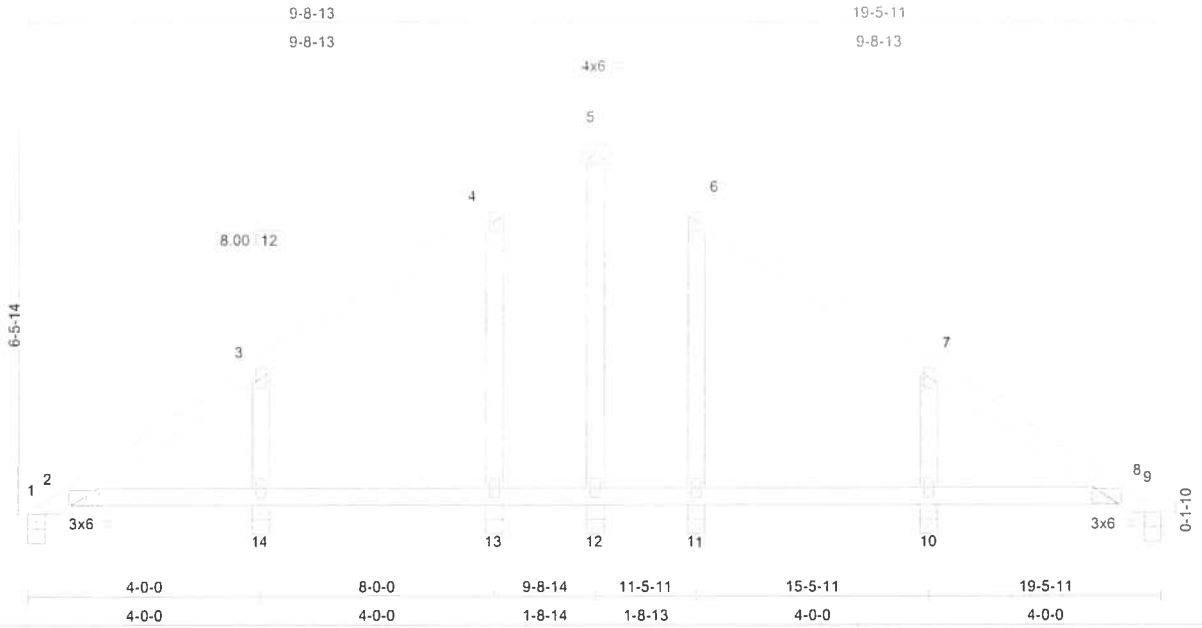
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907387
L258990	PB2G	GABLE	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 08:51:28 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.01	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.02	2-14	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=146/0-3-8, 9=146/0-3-8, 12=280/0-3-8, 10=667/0-3-8, 11=496/0-3-8, 14=667/0-3-8, 13=496/0-3-8
Max Horz 1=-222(load case 4)
Max Uplift 1=-87(load case 4), 9=-37(load case 7), 12=-46(load case 5), 10=-408(load case 7), 11=-306(load case 7), 14=-415(load case 6), 13=-305(load case 6)
Max Grav 1=156(load case 10), 9=156(load case 11), 12=280(load case 1), 10=671(load case 11), 11=496(load case 1), 14=671(load case 10), 13=496(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-226/235, 2-3=-247/333, 3-4=-128/323, 4-5=-31/178, 5-6=-23/178, 6-7=-54/323, 7-8=-197/333, 8-9=-77/18
BOT CHORD	2-14=-145/190, 13-14=-145/190, 12-13=-145/190, 11-12=-145/190, 10-11=-145/190, 8-10=-145/190
WEBS	5-12=-258/43, 7-10=-582/410, 6-11=-480/336, 3-14=-582/412, 4-13=-480/337

JOINT STRESS INDEX

2 = 0.62, 3 = 0.34, 4 = 0.34, 5 = 0.28, 6 = 0.34, 7 = 0.34, 8 = 0.62, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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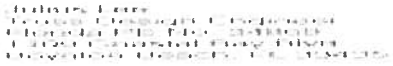
NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 1, 37 lb uplift at joint 9, 46 lb uplift at joint 12, 408 lb uplift at joint 10, 306 lb uplift at joint 11, 415 lb uplift at joint 14 and 305 lb uplift at joint 13.
8) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-153(F=-87), 2-5=-141(F=-87), 5-8=-141(F=-87), 8-9=-153(F=-87), 2-8=-10



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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907388
L258990	T01	ROOF TRUSS	2	1	

Job Reference (optional)

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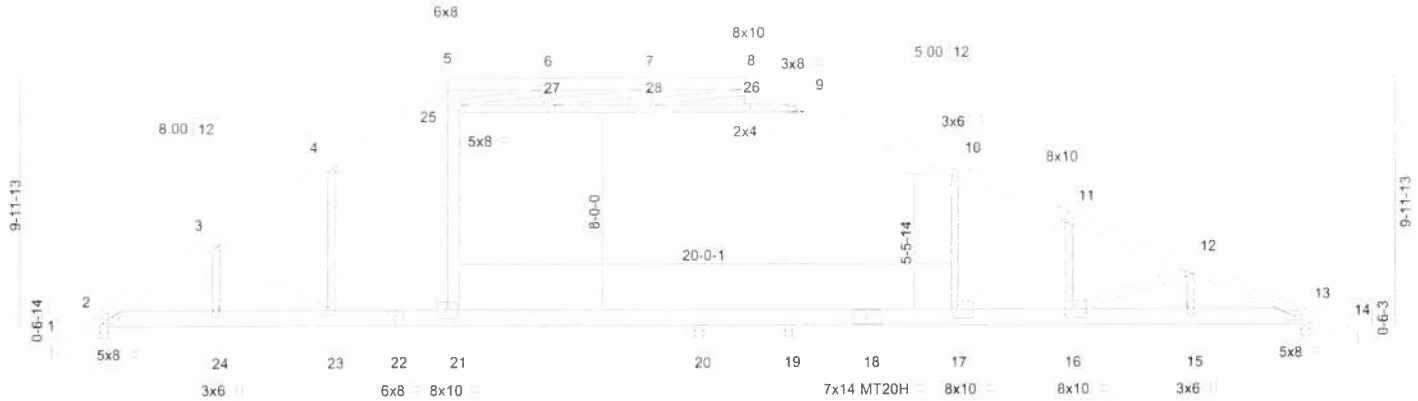
-2-0-0 4-8-7 9-5-0 14-1-7 18-2-3 22-4-1 26-5-5 34-7-0 39-4-12 44-3-12 49-2-0 51-2-0
2-0-0 4-8-7 4-8-9 4-8-7 4-0-13 4-1-13 4-1-4 8-1-11 4-9-12 4-11-0 4-10-4 2-0-0

Scale = 1:90.1



4-8-7 9-5-0 14-1-7 24-4-0 28-1-8 34-7-0 39-4-12 44-3-12 49-2-0
4-8-7 4-8-9 4-8-7 10-2-9 3-9-8 6-5-8 4-9-12 4-11-0 4-10-4

Plate Offsets (X,Y): [11:0-5-0,0-4-8], [13:0-3-8,0-2-8], [16:0-3-8,0-4-0], [17:0-2-14,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.45	16-17	>566	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.82	16-17	>308	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.08	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 413 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
5-21 2 X 6 SYP No.1D, 9-25 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-8 oc purlins, except
2-0-0 oc purlins (5-0-15 max.): 5-8.
BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.
WEBS 1 Row at midpt 9-25

REACTIONS (lb/size) 2=1979/0-4-0, 13=1701/0-4-0, 20=153/0-4-0, 19=1912/0-3-8
Max Horz 2=-267(load case 4)
Max Uplift 2=-215(load case 6), 13=-224(load case 7), 20=-99(load case 2), 19=-300(load case 4)
Max Grav 2=1979(load case 1), 13=1701(load case 1), 20=677(load case 11), 19=2052(load case 12)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-3026/801, 3-4=-2796/788, 4-5=-2642/787, 5-6=-2388/437, 6-7=-2609/727, 7-8=-2227/980, 8-9=-1316/754, 9-10=-2409/773, 10-11=-2487/611, 11-12=-3464/1021, 12-13=-3447/999, 13-14=0/44

BOT CHORD 2-24=-443/2434, 23-24=-443/2434, 22-23=-291/2256, 21-22=-291/2256, 20-21=-241/2212, 19-20=-241/2212, 18-19=-241/2212, 17-18=-241/2212, 16-17=-711/3174, 15-16=-788/3121, 13-15=-788/3121

WEBS 3-24=-0/118, 21-25=-103/917, 5-25=0/1071, 10-17=-225/281, 11-17=-1289/622, 11-16=-383/877, 25-27=-72/680, 27-28=-393/254, 26-28=-1079/36, 9-26=-1096/34, 8-26=0/108, 6-27=-91/34, 7-28=-296/89, 7-27=0/437, 12-15=-225/169, 12-16=-52/130, 4-23=-132/204, 4-21=-265/207, 3-23=-214/182, 6-25=-283/455, 8-28=-230/1316

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JOINT STRESS INDEX

2 = 0.59, 3 = 0.31, 4 = 0.31, 5 = 0.70, 6 = 0.26, 7 = 0.26, 8 = 0.51, 9 = 0.27, 10 = 0.15, 11 = 0.21, 12 = 0.28, 13 = 0.64, 15 = 0.15, 16 = 0.18, 17 = 0.23, 18 = 0.30, 21 = 0.12, 22 = 0.47, 23 = 0.24, 24 = 0.15, 25 = 0.16, 26 = 0.33, 27 = 0.25 and 28 = 0.62

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 4x6 MT20 unless otherwise indicated.
7) Ceiling dead load (5.0 psf) on member(s). 9-10, 25-27, 27-28, 26-28, 9-26; Wall dead load (5.0psf) on member(s).21-25, 10-17
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 20-21, 19-20, 17-19
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 2, 224 lb uplift at joint 13, 99 lb uplift at joint 20 and 300 lb uplift at joint 19.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907389
L258990	T01G	GABLE	1	1	

Job Reference (optional)

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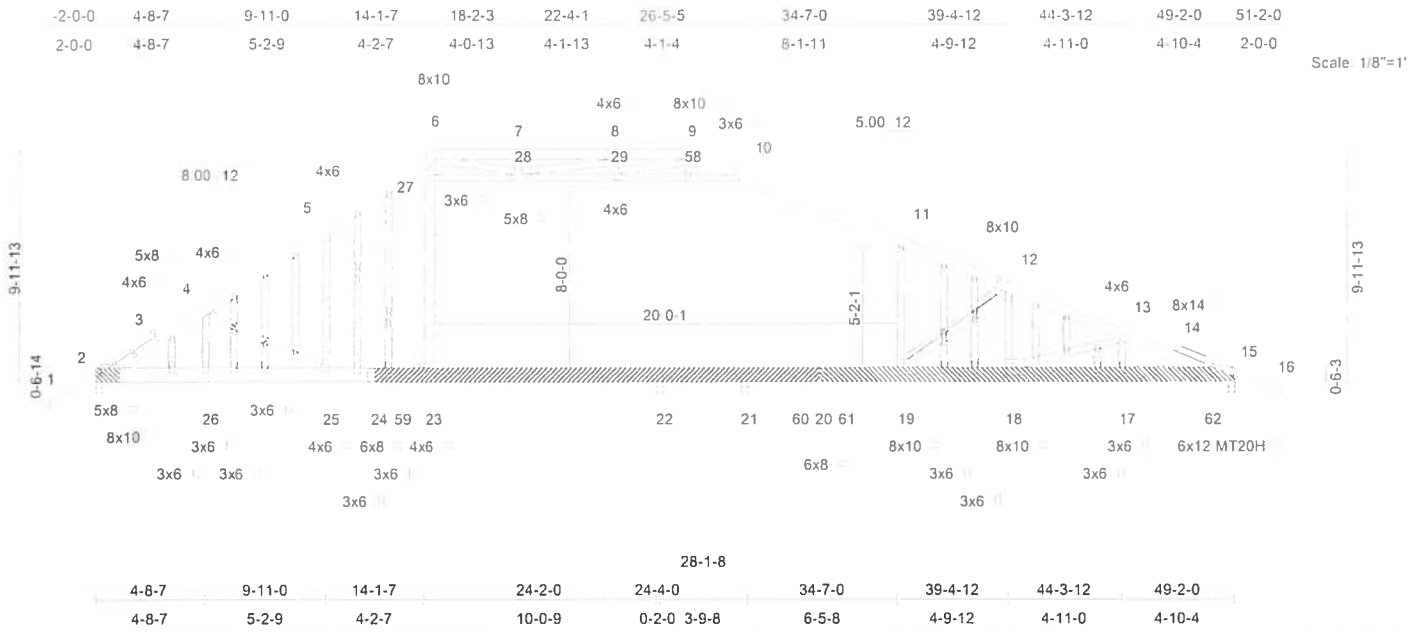

Plate Offsets (X,Y): [2:0-2-13,0-1-2], [2:1-0-8,0-1-12], [3:0-0-4,0-2-0], [12:0-5-0,0-4-8], [14:0-4-8,0-3-4], [15:0-0-12,Edge], [18:0-3-8,0-4-0], [19:0-3-8,0-4-0], [38:0-1-12,0-1-0], [41:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.47	18-19	>534	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.83	18-19	>305	240	MT20H	187/143
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.78	Horz(TL)	0.14	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 583 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.1D, 14-16 2 X 4 SYP No.1D
BOT CHORD	2 X 8 SYP 2400F 2.0E
WEBS	2 X 4 SYP No.3 *Except* 6-23 2 X 6 SYP No.1D, 10-27 2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3
LBR SCAB	20-24 2 X 8 SYP 2400F 2.0E one side 15-20 2 X 8 SYP 2400F 2.0E one side

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-11-10 oc purlins, except 2-0-0 oc purlins (4-1-10 max.): 6-9.
BOT CHORD	Rigid ceiling directly applied or 7-10-12 oc bracing.
WEBS	2 Rows at 1/3 pts 10-27 T-Brace: 2 X 4 SYP No.3 - 12-19 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=3293/0-3-14 (0-3-8 + bearing block), 15=2828/0-3-8, 22=-388/0-3-8, 21=3199/0-4-0 (0-3-8 + bearing block)
Max Horz 2=-374(load case 4)
Max Uplift 2=-914(load case 6), 15=-844(load case 7), 22=-598(load case 12), 21=-1059(load case 4)
Max Grav 2=3293(load case 1), 15=2828(load case 1), 22=982(load case 4), 21=3375(load case 12)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-28/122, 2-3=-5338/2138, 3-4=-5192/2104, 4-5=-4717/1900, 5-6=-4175/1773, 6-7=-3354/1397, 7-8=-3348/1395, 8-9=-3383/1790, 9-10=-2262/1334, 10-11=-3878/1671, 11-12=-3822/1470, 12-13=-5864/2431, 13-14=-6001/2520, 14-15=-6082/2538, 15-16=-19/84

BOT CHORD 2-26=-1606/4440, 25-26=-1606/4440, 24-25=-1210/3772, 24-59=-1210/3772, 23-59=-1210/3772, 22-23=-1063/3439, 21-22=-1063/3439, 21-60=-1063/3439, 20-60=-1063/3439, 20-61=-1063/3439, 19-61=-1063/3439, 18-19=-2015/5312, 17-18=-2268/5643, 17-62=-2268/5643, 15-62=-2268/5643

WEBS 4-26=-35/116, 23-27=-382/1231, 6-27=-446/1442, 11-19=-810/588, 12-19=-2395/1217, 12-18=-847/1783, 27-28=-852/540, 28-29=-583/163, 29-58=-1468/316, 10-58=-1487/318, 7-28=-140/56, 8-29=-469/199, 8-28=-334/496, 13-17=-572/347, 13-18=-351/268, 5-25=-131/478, 5-23=-808/390, 4-25=-758/449, 9-58=-10/124, 9-29=-520/1635, 6-28=-624/713

JOINT STRESS INDEX

2 = 0.65, 2 = 0.00, 2 = 0.83, 3 = 0.00, 3 = 0.51, 3 = 0.78, 4 = 0.32, 5 = 0.32, 6 = 0.79, 7 = 0.34, 8 = 0.27, 9 = 0.67, 10 = 0.39, 11 = 0.35, 12 = 0.44, 13 = 0.29, 14 = 0.00, 14 = 0.00, 14 = 0.00, 14 = 0.75, 14 = 0.00, 15 = 0.85, 15 = 0.00, 15 = 0.00, 17 = 0.16, 17 = 0.00, 18 = 0.37, 18 = 0.00, 19 = 0.40, 19 = 0.00, 20 = 0.66, 20 = 0.00, 20 = 0.00, 23 = 0.52, 23 = 0.00, 24 = 0.65, 24 = 0.00, 25 = 0.25, 26 = 0.16, 27 = 0.19, 28 = 0.33, 29 = 0.80, 30 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.16, 32 = 0.00, 33 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.16, 36 = 0.34, 37 = 0.34, 38 = 0.40, 38 = 0.34, 39 = 0.34, 40 = 0.16, 41 = 0.40, 41 = 0.34, 42 = 0.34, 43 = 0.16, 44 = 0.16, 45 = 0.34, 46 = 0.34, 47 = 0.16, 47 = 0.00, 48 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.16, 50 = 0.00, 51 = 0.34, 51 = 0.34, 52 = 0.34, 53 = 0.34, 54 = 0.34, 55 = 0.34, 56 = 0.16, 56 = 0.00, 57 = 0.34 and 58 = 0.34

NOTES

1) Attached 19-2-0 scab 20 to 24, back face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 0-0-0 from end at joint 24, nail 2 row(s) at 7 o.c. for 3-4-3; starting at 14-11-12 from end at joint 24, nail 2 row(s) at 3 o.c. for 2-0-0; starting at 17-2-0 from end at joint 24, nail 2 row(s) at 2 o.c. for 2-0-0; starting at 19-2-0 from end at joint 24, nail 2 row(s) at 2 o.c. for 2-0-0; starting at 21-8-12 from end at joint 24, nail 2 row(s) at 4 o.c. for 2-0-0; starting at 26-4-12 from end at joint 24, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 35-0-4 from end at joint 24, nail 2 row(s) at 7 o.c. for 2-0-0.
2) Attached 18-0-0 scab 15 to 20, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 15-9-13 from end at joint 20, nail 2 row(s) at 7 o.c. for 3-4-3; starting at 2-2-4 from end at joint 20, nail 2 row(s) at 3 o.c. for 2-0-0; starting at 0-0-0 from end at joint 20, nail 2 row(s) at 2 o.c. for 2-0-0; starting at 0-0-0 from end at joint 20, nail 2 row(s) at 2 o.c. for 2-0-0; starting at 2-6-12 from end at joint 20, nail 2 row(s) at 4 o.c. for 2-0-0; starting at 7-2-12 from end at joint 20, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 15-10-4 from end at joint 20, nail 2 row(s) at 7 o.c. for 2-0-0.
3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 2 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
4) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 22 attached to back face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
5) Unbalanced roof live loads have been considered for this design.
6) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
7) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
8) Provide adequate drainage to prevent water ponding.
9) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) All plates are MT20 plates unless otherwise indicated.
11) All plates are 2x4 MT20 unless otherwise indicated.
12) Gable studs spaced at 1-4-0 oc.
13) Ceiling dead load (5.0 psf) on member(s). 10-11, 27-28, 28-29, 29-58, 10-58; Wall dead load (5.0psf) on member(s).23-27, 11-19
14) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 22-23, 21-22, 19-21
15) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
16) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 2, 844 lb uplift at joint 15, 598 lb uplift at joint 22 and 1059 lb uplift at joint 21.
17) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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Continued on page 3

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-6=-114(F=-60), 6-9=-114(F=-60), 9-10=-114(F=-60), 10-11=-124(F=-60), 11-16=-114(F=-60), 2-23=-10, 19-23=-110, 15-19=-10, 10-27=-10

Drag: 23-27=-10, 11-19=-10

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907390
L258990	T02	ROOF TRUSS	2	1	Job Reference (optional)

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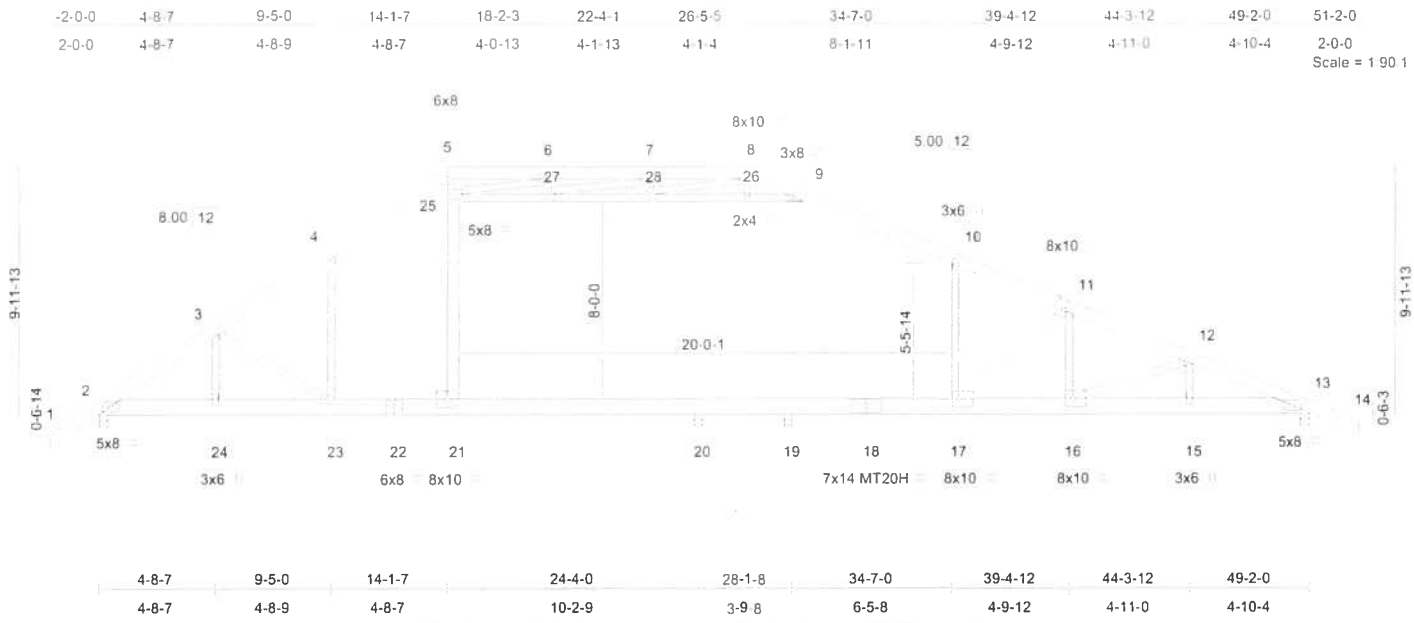

4-8-7	9-5-0	14-1-7	24-4-0	28-1-8	34-7-0	39-4-12	44-3-12	49-2-0
4-8-7	4-8-9	4-8-7	10-2-9	3-9-8	6-5-8	4-9-12	4-11-0	4-10-4

Plate Offsets (X,Y): [11:0-5-0,0-4-8], [13:0-3-8,0-2-8], [16:0-3-8,0-4-0], [17:0-2-14,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.45	16-17	>566	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.82	16-17	>308	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.08	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 413 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
5-21 2 X 6 SYP No.1D, 9-25 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-8 oc purlins, except
2-0-0 oc purlins (5-0-15 max.): 5-8.
BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.
WEBS 1 Row at midpt 9-25

REACTIONS (lb/size) 2=1979/0-4-0, 13=1701/0-4-0, 20=153/0-4-0, 19=1912/0-3-8
Max Horz 2=-267(load case 4)
Max Uplift 2=-215(load case 6), 13=-224(load case 7), 20=-99(load case 2), 19=-300(load case 4)
Max Grav 2=1979(load case 1), 13=1701(load case 1), 20=677(load case 11), 19=2052(load case 12)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-3026/801, 3-4=-2796/788, 4-5=-2642/787, 5-6=-2388/437, 6-7=-2609/727, 7-8=-2227/980, 8-9=-1316/754, 9-10=-2409/773, 10-11=-2487/611, 11-12=-3464/1021, 12-13=-3447/999, 13-14=0/44

BOT CHORD 2-24=-443/2434, 23-24=-443/2434, 22-23=-291/2256, 21-22=-291/2256, 20-21=-241/2212, 19-20=-241/2212, 18-19=-241/2212, 17-18=-241/2212, 16-17=-711/3174, 15-16=-788/3121, 13-15=-788/3121

WEBS 3-24=-0/118, 21-25=-103/917, 5-25=0/1071, 10-17=-225/281, 11-17=-1289/622, 11-16=-383/877, 25-27=-72/680, 27-28=-393/254, 26-28=-1079/36, 9-26=-1096/34, 8-26=0/108, 6-27=-91/34, 7-28=-296/89, 7-27=0/437, 12-15=-225/169, 12-16=-52/130, 4-23=-132/204, 4-21=-265/207, 3-23=-214/182, 6-25=-283/455, 8-28=-230/1316

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JOINT STRESS INDEX

2 = 0.59, 3 = 0.31, 4 = 0.31, 5 = 0.70, 6 = 0.26, 7 = 0.26, 8 = 0.51, 9 = 0.27, 10 = 0.15, 11 = 0.21, 12 = 0.28, 13 = 0.64, 15 = 0.15, 16 = 0.18, 17 = 0.23, 18 = 0.30, 21 = 0.12, 22 = 0.47, 23 = 0.24, 24 = 0.15, 25 = 0.16, 26 = 0.33, 27 = 0.25 and 28 = 0.62

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 4x6 MT20 unless otherwise indicated.
7) Ceiling dead load (5.0 psf) on member(s). 9-10, 25-27, 27-28, 26-28, 9-26; Wall dead load (5.0psf) on member(s).21-25, 10-17
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 20-21, 19-20, 17-19
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 2, 224 lb uplift at joint 13, 99 lb uplift at joint 20 and 300 lb uplift at joint 19.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907391
L258990	T03	MONO HIP	3	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:36 2007 Page 1

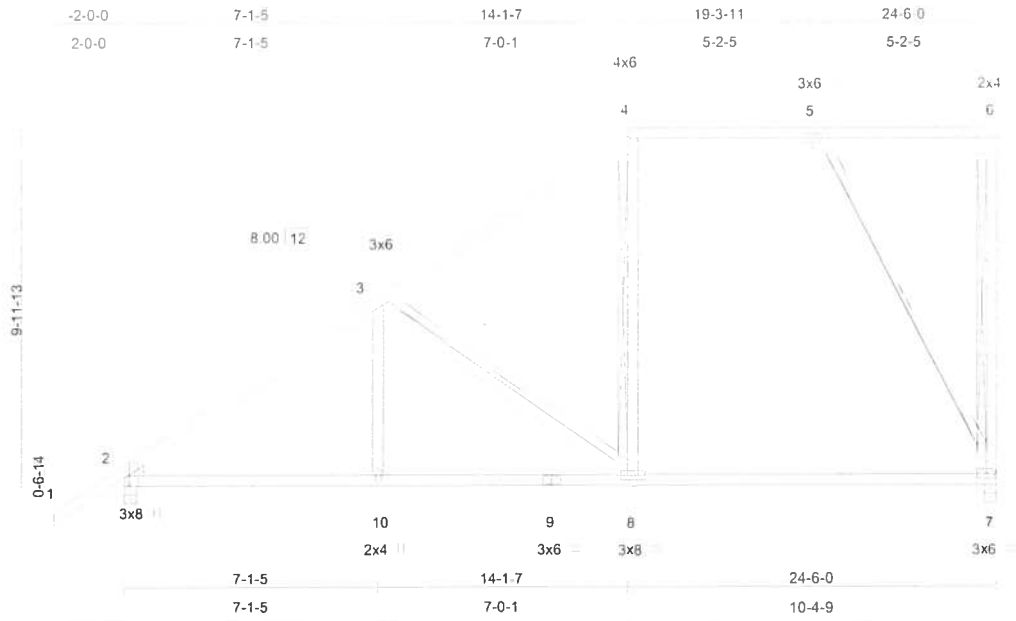

Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.23	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.40	7-8	>723	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 8-2-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-7, 3-8, 4-8, 5-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 7=769/0-4-0, 2=896/0-4-0
Max Horz 2=366(load case 6)
Max Uplift 7=-218(load case 5), 2=-223(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-1095/338, 3-4=-698/281, 4-5=-489/316, 5-6=-21/0, 6-7=-125/87
BOT CHORD 2-10=-576/806, 9-10=-576/806, 8-9=-576/806, 7-8=-208/322
WEBS 3-10=0/202, 3-8=-387/315, 4-8=-7/146, 5-8=-234/361, 5-7=-670/450

JOINT STRESS INDEX
2 = 0.63, 2 = 0.00, 3 = 0.41, 4 = 0.69, 5 = 0.44, 6 = 0.60, 7 = 0.66, 8 = 0.60, 9 = 0.27 and 10 = 0.33

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NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 7 and 223 lb uplift at joint 2.

LOAD CASE(S) Standard

[illegible]

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907392
L258990	T04	MONO HIP	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:37 2007 Page 1

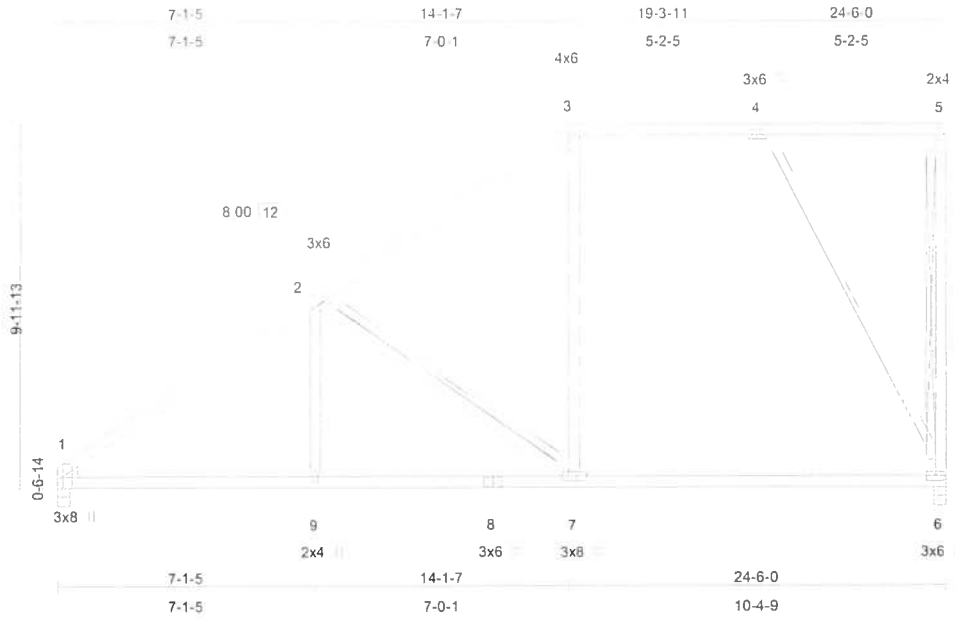

Plate Offsets (X,Y): [1:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.23	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.40	6-7	>720	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-11 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-5.
BOT CHORD Rigid ceiling directly applied or 7-8-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6, 2-7, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.
JOINTS 1 Brace at Jt(s): 5

REACTIONS (lb/size) 6=774/0-4-0, 1=774/0-4-0
Max Horz 1=311(load case 6)
Max Uplift 6=-219(load case 5), 1=-126(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1119/372, 2-3=-705/291, 3-4=-493/322, 4-5=-21/1, 5-6=-125/86
BOT CHORD 1-9=-610/831, 8-9=-610/831, 7-8=-610/831, 6-7=-212/324
WEBS 2-9=0/206, 2-7=-412/349, 3-7=0/148, 4-7=-239/365, 4-6=-676/458

JOINT STRESS INDEX
1 = 0.64, 1 = 0.00, 2 = 0.41, 3 = 0.67, 4 = 0.44, 5 = 0.60, 6 = 0.66, 7 = 0.60, 8 = 0.29 and 9 = 0.33

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907392
L258990	T04	MONO HIP	3	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 6 and 126 lb uplift at joint 1.

LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907393
L258990	T05	CAL.	1	1	

Job Reference (optional)

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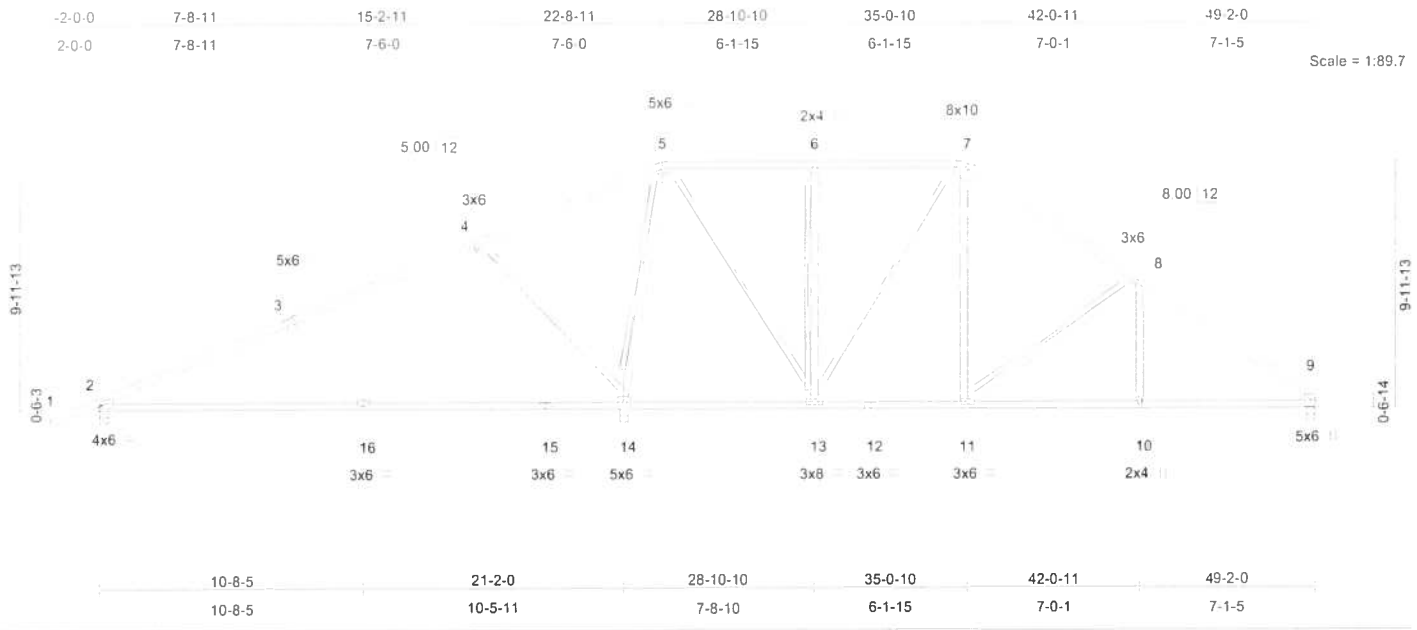

Plate Offsets (X,Y): [2:0-0-1,0-0-15], [3:0-3-0,0-3-0], [7:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.21	2-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.41	2-16	>619	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-8 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 5-14, 5-13, 6-13, 7-13, 8-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=570/0-4-0, 14=1951/0-4-0, 9=721/0-4-0
Max Horz 2=297(load case 5)
Max Uplift 2=-209(load case 6), 14=-420(load case 5), 9=-179(load case 7)
Max Grav 2=598(load case 10), 14=1951(load case 1), 9=795(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-696/319, 3-4=-406/229, 4-5=-164/636, 5-6=-336/282, 6-7=-336/282, 7-8=-737/384, 8-9=-1157/458
BOT CHORD 2-16=-279/565, 15-16=-121/144, 14-15=-121/144, 13-14=-300/371, 12-13=-19/526, 11-12=-19/526, 10-11=-258/861, 9-10=-258/861
WEBS 3-16=-400/368, 4-16=-255/529, 4-14=-692/535, 5-14=-1362/720, 5-13=-430/891, 6-13=-338/206, 7-13=-448/230, 7-11=-148/357, 8-11=-419/338, 8-10=0/237

[redacted]

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JOINT STRESS INDEX

2 = 0.59, 3 = 0.71, 4 = 0.40, 5 = 0.77, 6 = 0.33, 7 = 0.58, 8 = 0.41, 9 = 0.48, 9 = 0.00, 10 = 0.33, 11 = 0.34, 12 = 0.20, 13 = 0.91, 14 = 0.38, 15 = 0.34 and 16 = 0.40

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 2, 420 lb uplift at joint 14 and 179 lb uplift at joint 9.

LOAD CASE(S) Standard

November 6,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907394
L258990	T06	MONO TRUSS	6	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:39 2007 Page 1

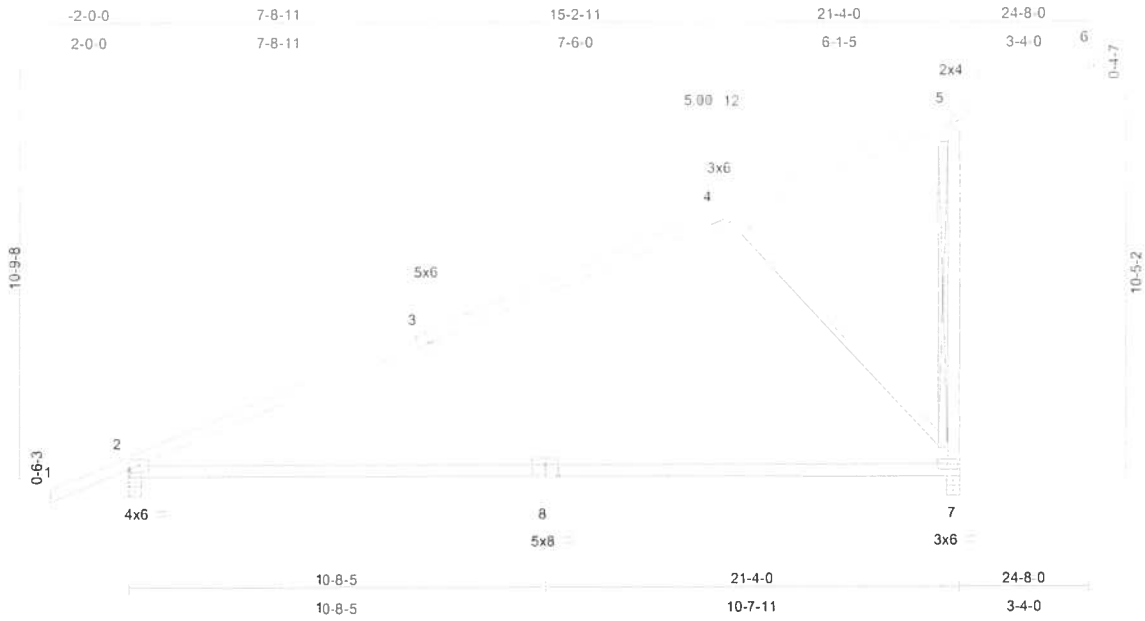

Plate Offsets (X,Y): [2:0-0-1,0-0-15], [3:0-3-0,0-3-0], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.21	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.40	2-8	>638	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.30	Horz(TL)	-0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 119 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-4-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 7-4-6 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-7, 4-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=62/Mechanical, 7=794/0-4-0, 2=791/0-4-0
Max Horz 2=367(load case 6)
Max Uplift 6=-31(load case 6), 7=-316(load case 6), 2=-197(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/32, 2-3=-1166/326, 3-4=-877/233, 4-5=-153/48, 5-6=-47/17, 5-7=-267/257
BOT CHORD	2-8=-731/993, 7-8=-334/447
WEBS	3-8=-390/376, 4-8=-259/535, 4-7=-651/501

JOINT STRESS INDEX
2 = 0.59, 3 = 0.73, 4 = 0.40, 5 = 0.59, 7 = 0.64 and 8 = 0.86

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE
					J1907394
L258990	T06	MONO TRUSS	6	1	
					Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 6, 316 lb uplift at joint 7 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907395
L258990	T07G	GABLE	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:40 2007 Page 1

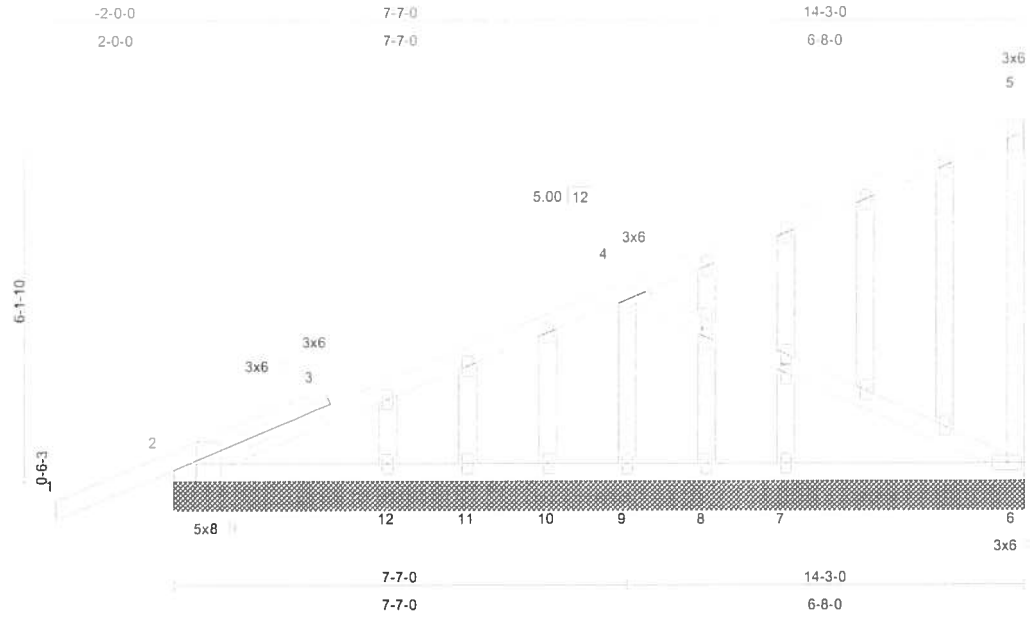

Plate Offsets (X,Y): [2:0-3-8,Edge], [18:0-1-12,0-1-0], [20:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.05	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.20	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 103 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=539/14-3-0, 6=294/14-3-0, 9=943/14-3-0, 7=-8/14-3-0, 8=32/14-3-0, 10=54/14-3-0, 11=-133/14-3-0, 12=255/14-3-0

Max Horz 2=306(load case 6)

Max Uplift 2=-221(load case 6), 6=-168(load case 6), 9=-444(load case 6), 7=-8(load case 1), 8=-18(load case 6), 10=-9(load case 6), 11=-133(load case 1), 12=-83(load case 6)

Max Grav 2=539(load case 1), 6=294(load case 1), 9=943(load case 1), 7=94(load case 2), 8=32(load case 1), 10=64(load case 2), 11=65(load case 6), 12=255(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-17/68, 2-3=-200/11, 3-4=-270/225, 4-5=-212/74, 5-6=-301/246
BOT CHORD	2-12=-54/0, 11-12=-54/0, 10-11=-54/0, 9-10=-54/0, 8-9=-54/0, 7-8=-54/0, 6-7=-54/0
WEBS	4-9=-945/698, 4-6=-19/110

JOINT STRESS INDEX

2 = 0.88, 3 = 0.00, 3 = 0.65, 3 = 0.65, 4 = 0.39, 5 = 0.59, 6 = 0.43, 7 = 0.00, 8 = 0.00, 9 = 0.39, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00 and 23 = 0.00

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NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2, 168 lb uplift at joint 6, 444 lb uplift at joint 9, 8 lb uplift at joint 7, 18 lb uplift at joint 8, 9 lb uplift at joint 10, 133 lb uplift at joint 11 and 83 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 2-6=-10

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907396
L258990	T08	COMMON	5	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 08:55:25 2007 Page 1

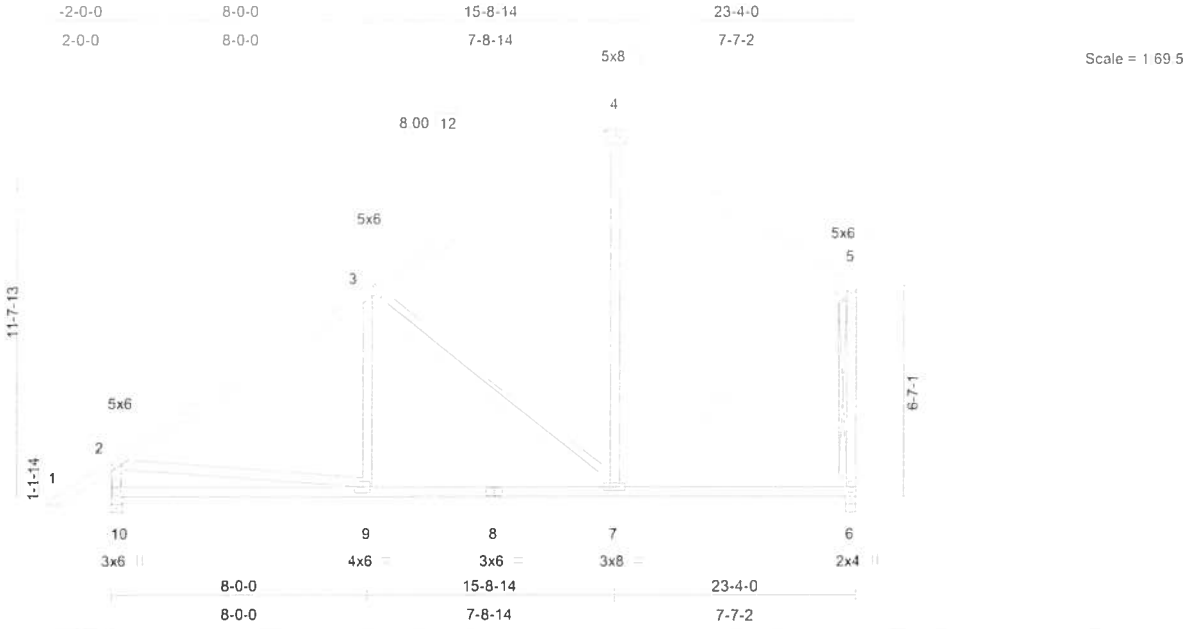

Plate Offsets (X,Y): [2:0-2-12,0-2-0], [3:0-3-0,0-3-0], [5:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.06	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.10	9-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-9-6 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 3-7, 5-6 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 10=847/0-4-0, 6=733/0-4-0
Max Horz 10=328(load case 5)
Max Uplift 10=-224(load case 6), 6=-178(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/58, 2-3=-908/357, 3-4=-539/318, 4-5=-527/312, 2-10=-805/438, 5-6=-693/383
BOT CHORD	9-10=-418/255, 8-9=-404/656, 7-8=-404/656, 6-7=-25/38
WEBS	3-9=0/204, 3-7=-412/332, 4-7=-49/218, 2-9=-25/404, 5-7=-160/423

JOINT STRESS INDEX
2 = 0.83, 3 = 0.82, 4 = 0.94, 5 = 0.78, 6 = 0.83, 7 = 0.57, 8 = 0.31, 9 = 0.26 and 10 = 0.45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 10 and 178 lb uplift at joint 6.

LOAD CASE(S) Standard

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907397
L258990	T08G	GABLE	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 08:55:52 2007 Page 1

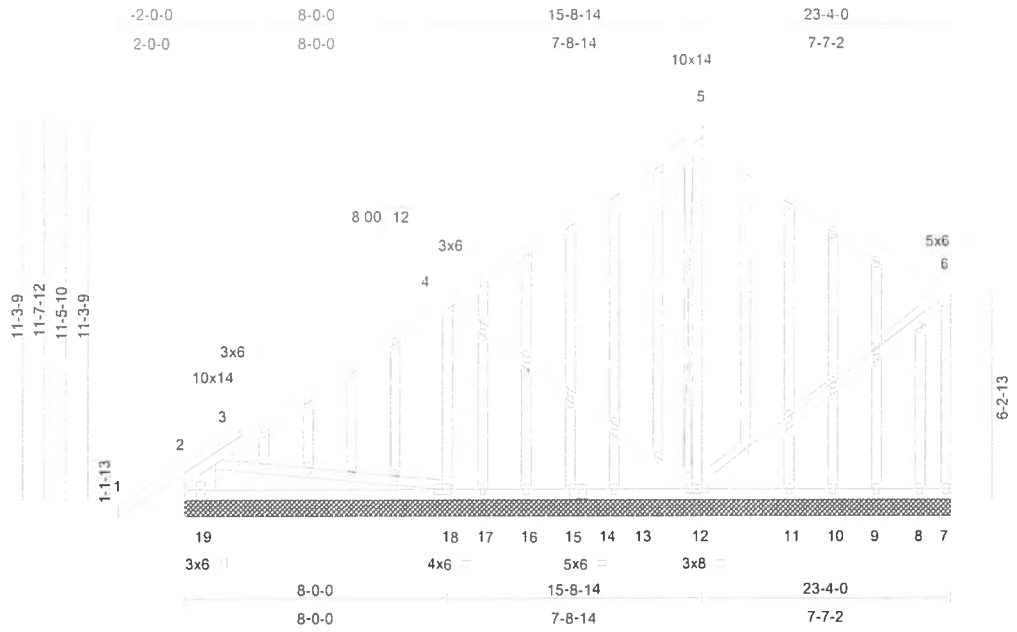

Plate Offsets (X,Y): [5:0-2-5,Edge], [6:Edge,0-1-12], [14:0-2-12,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	0.07	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	0.15	1	n/r	90		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.62	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 294 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2 *Except* 5-6 2 X 6 SYP No.1D
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 5-12 2 X 4 SYP No.1D, 2-19 2 X 12 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 9-6-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-12, 6-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 19=602/23-4-0, 18=1181/23-4-0, 7=402/23-4-0, 12=970/23-4-0, 13=21/23-4-0, 15=6/23-4-0, 16=32/23-4-0, 17=-74/23-4-0, 11=24/23-4-0, 10=5/23-4-0, 9=31/23-4-0, 8=-102/23-4-0
Max Horz 19=393(load case 6)
Max Uplift 19=-299(load case 6), 18=-551(load case 6), 7=-224(load case 7), 12=-517(load case 6), 17=-222(load case 2), 9=-4(load case 7), 8=-119(load case 11)
Max Grav 19=603(load case 10), 18=1181(load case 1), 7=466(load case 11), 12=970(load case 1), 13=63(load case 2), 15=18(load case 2), 16=97(load case 2), 11=71(load case 2), 10=25(load case 2), 9=52(load case 2), 8=100(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-27/144, 2-3=-226/159, 3-4=-242/419, 4-5=-196/296, 5-6=-209/303, 6-7=-348/180
BOT CHORD	18-19=-393/167, 17-18=-137/112, 16-17=-137/112, 15-16=-137/112, 14-15=-137/112, 13-14=-137/112, 12-13=-137/112, 11-12=-30/41, 10-11=-30/41, 9-10=-30/41, 8-9=-30/41, 7-8=-30/41
WEBS	4-18=-1041/580, 4-12=-21/123, 5-12=-968/453, 6-12=-79/151, 2-19=-574/336, 2-18=-138/351

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JOINT STRESS INDEX

2 = 0.27, 3 = 0.00, 3 = 0.56, 4 = 0.43, 5 = 0.57, 6 = 0.71, 7 = 0.77, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.57, 13 = 0.34, 14 = 0.43, 15 = 0.00, 16 = 0.34, 17 = 0.34, 18 = 0.27, 19 = 0.15, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 25 = 0.34, 26 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 45 = 0.34 and 46 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 19, 551 lb uplift at joint 18 , 224 lb uplift at joint 7, 517 lb uplift at joint 12, 222 lb uplift at joint 17, 4 lb uplift at joint 9 and 119 lb uplift at joint 8.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

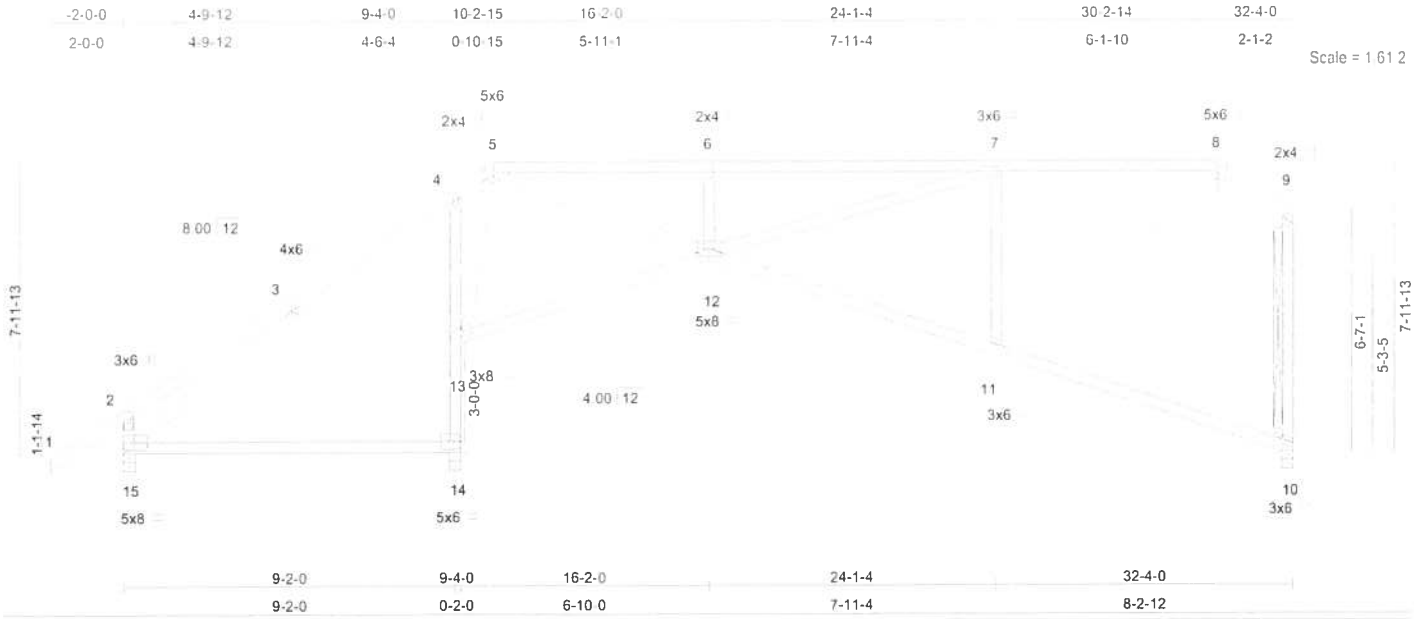
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-6=-114(F=-60), 7-19=-10

November 6,2007

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907398
L258990	T09	SPECIAL	3	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 06 08:18:43 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.27	14-15	>401	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.22	14-15	>498	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 4-14 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-7 max.): 5-8.
BOT CHORD Rigid ceiling directly applied or 5-6-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 9-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 14=1069/0-4-0, 15=378/0-4-0, 10=720/0-4-0
Max Horz 15=284(load case 6)
Max Uplift 14=-511(load case 5), 15=-224(load case 6), 10=-253(load case 4)
Max Grav 14=1069(load case 1), 15=378(load case 10), 10=722(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/64, 2-3=-291/438, 3-4=-37/115, 4-5=-48/121, 5-6=-1491/900, 6-7=-1491/900, 7-8=-806/482, 8-9=-39/40, 2-15=-295/445, 9-10=-41/20
BOT CHORD 14-15=-252/109, 13-14=-906/432, 4-13=-140/154, 12-13=-141/151, 11-12=-455/857, 10-11=-115/216
WEBS 3-14=-192/253, 5-13=-792/345, 5-12=-770/1471, 6-12=-392/283, 7-12=-437/715, 7-11=-619/421, 8-11=-416/795, 3-15=-359/208, 8-10=-750/432

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JOINT STRESS INDEX

2 = 0.29, 3 = 0.26, 4 = 0.35, 5 = 0.46, 6 = 0.33, 7 = 0.39, 8 = 0.32, 9 = 0.38, 10 = 0.48, 11 = 0.53, 12 = 0.73, 13 = 0.70, 14 = 0.69 and 15 = 0.69

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 511 lb uplift at joint 14, 224 lb uplift at joint 15 and 253 lb uplift at joint 10.

LOAD CASE(S) Standard

November 6,2007

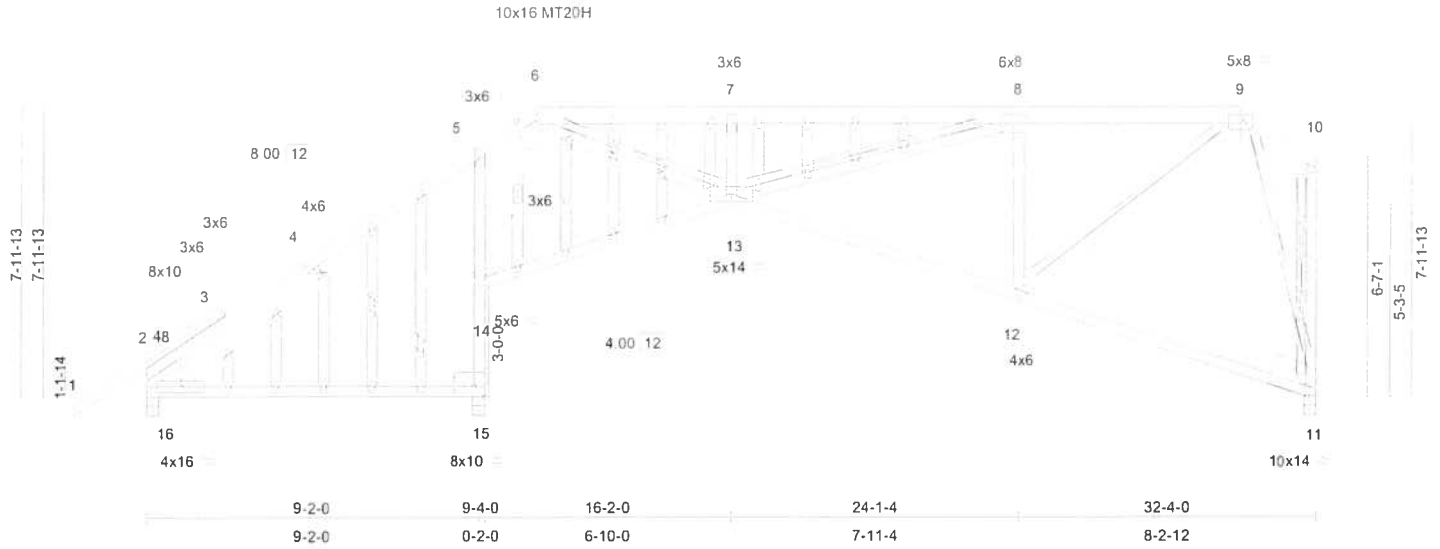
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907399
L258990	T09G	GABLE	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 08:59:23 2007 Page 1

-2-0-0	4-9-12	9-4-0	10-9-3	16-2-0	24-1-4	30-2-14	32-4-0
2-0-0	4-9-12	4-6-4	1-5-3	5-4-13	7-11-4	6-1-10	2-1-2

Scale = 1:61.3



9-2-0	9-4-0	16-2-0	24-1-4	32-4-0
9-2-0	0-2-0	6-10-0	7-11-4	8-2-12

Plate Offsets (X,Y): [2:0-4-4,0-5-4], [4:0-2-0,0-0-8], [4:0-3-0,0-2-4], [6:0-6-12,Edge], [8:0-3-8,0-3-0], [9:0-4-0,0-1-8], [11:0-1-7,Edge], [15:Edge,0-3-8], [25:0-0-13,0-1-12], [27:0-1-12,0-1-0], [27:0-1-12,0-1-0], [30:0-1-12,0-1-0], [30:0-1-12,0-1-0], [34:0-1-11,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.31	12-13	>900	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.34	12-13	>814	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.13	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2 *Except* 6-9 2 X 6 SYP No.1D
BOT CHORD	2 X 4 SYP No.2 *Except* 5-15 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3 *Except* 6-13 2 X 4 SYP No.2, 2-16 2 X 12 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-0-7 max.): 6-9.
BOT CHORD	Rigid ceiling directly applied or 2-10-11 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-13, 8-13, 9-12, 10-11, 9-11 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 15=2354/0-4-0, 16=764/0-4-0, 11=1705/0-4-0
Max Horz 16=379(load case 6)
Max Uplift 15=-2135(load case 5), 16=-616(load case 6), 11=-1478(load case 4)
Max Grav 15=2354(load case 1), 16=764(load case 10), 11=1706(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-48=-70/124, 2-48=-81/137, 2-3=-283/364, 3-4=-226/242, 4-5=-215/249, 5-6=-124/173, 6-7=-3603/3408, 7-8=-3603/3408, 8-9=-1941/1838, 9-10=-75/82, 2-16=-561/699, 10-11=-106/99
BOT CHORD	15-16=-363/204, 14-15=-2100/1902, 5-14=-425/459, 13-14=-523/519, 12-13=-1883/2051, 11-12=-442/485
WEBS	4-15=-362/473, 6-14=-1882/1701, 6-13=-3111/3389, 7-13=-972/1000, 8-13=-1639/1734, 8-12=-1621/1614, 9-12=-1763/1925, 4-16=-219/71, 9-11=-1765/1678

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Builders FirstSource

JOINT STRESS INDEX

2 = 0.36, 3 = 0.00, 3 = 0.42, 3 = 0.42, 4 = 0.32, 4 = 0.63, 5 = 0.32, 6 = 0.97, 7 = 0.33, 8 = 0.36, 9 = 0.63, 10 = 0.52, 11 = 0.39, 12 = 0.87 , 13 = 0.80, 14 = 0.64, 15 = 0.39, 16 = 0.40, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.39, 26 = 0.34, 27 = 0.40, 27 = 0.40, 28 = 0.34, 29 = 0.34, 30 = 0.40, 30 = 0.40, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.52, 35 = 0.34, 36 = 0.16, 37 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.34 and 47 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable studs spaced at 1-4-0 oc.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2135 lb uplift at joint 15, 616 lb uplift at joint 16 and 1478 lb uplift at joint 11.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-48=-114(F=-60), 2-48=-74(F=-60), 2-5=-114(F=-60), 5-6=-141(F=-87), 6-9=-141(F=-87), 9-10=-141(F=-87), 15-16=-10, 13-14=-10, 11-13=-10

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November 6,2007

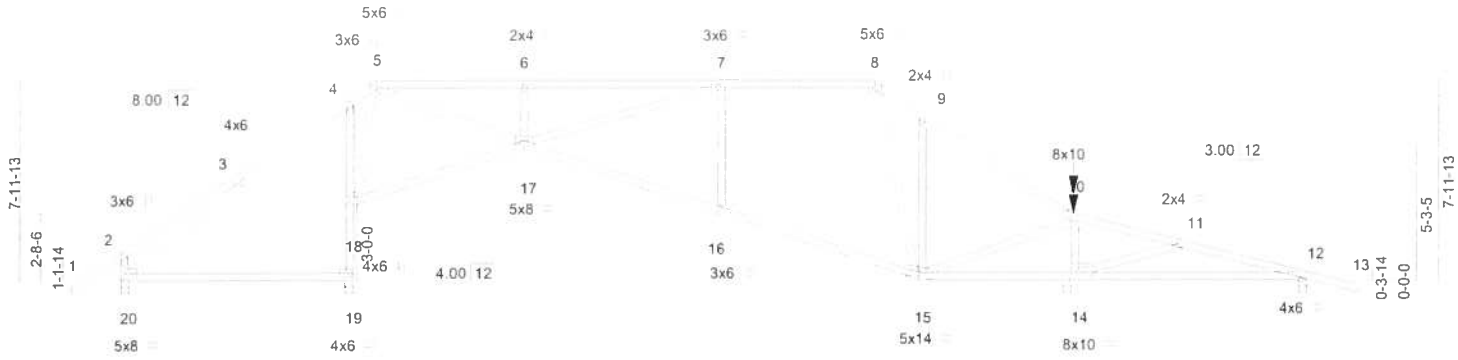
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907400
L258990	T10	SPECIAL	4	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 09:17:00 2007 Page 1

-2-0-0	4-9-12	9-4-0	10-2-15	16-2-0	24-1-0	30-2-14	32-0-0	38-2-1	42-4-8	47-8-0	49-8-0
2-0-0	4-9-12	4-6-4	0-10-15	5-11-1	7-11-0	6-1-14	1-9-2	6-2-1	4-2-8	5-3-8	2-0-0

Scale = 1:89.0



9-2-0	9-4-0	16-2-0	24-1-0	32-0-0	38-0-0	38-2-1	47-8-0
9-2-0	0-2-0	6-10-0	7-11-0	7-11-0	6-0-0	0-2-1	9-5-15

Plate Offsets (X,Y): [10:0-5-0,0-3-5], [14:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.28	12-14	>393	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.32	16-17	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.06	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 274 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 8-10 2 X 4 SYP No.1D, 10-13 2 X 4 SYP No.1D

BOT CHORD 2 X 4 SYP No.2 *Except* 4-19 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-13 oc purlins, except end verticals, and 2-0-0 oc purlins (3-9-11 max.): 5-8.

BOT CHORD Rigid ceiling directly applied or 4-2-4 oc bracing.

REACTIONS (lb/size) 12=192/0-4-0, 19=1376/0-4-0, 14=2861/0-4-0, 20=352/0-4-0
Max Horz 20=-208(load case 4)
Max Uplift 12=-268(load case 5), 19=-513(load case 5), 14=-1332(load case 4), 20=-320(load case 6)
Max Grav 12=204(load case 11), 19=1376(load case 1), 14=2861(load case 1), 20=357(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/64, 2-3=-302/446, 3-4=-3/388, 4-5=-18/456, 5-6=-2184/1453, 6-7=-2184/1453, 7-8=-1591/1165, 8-9=-1470/1280, 9-10=-1634/1167, 10-11=-423/746, 11-12=-187/519, 12-13=0/25, 2-20=-306/453

BOT CHORD 19-20=-173/81, 18-19=-1218/391, 4-18=-153/149, 17-18=-84/163, 16-17=-821/1685, 15-16=-501/1056, 14-15=-545/406, 12-14=-495/244

WEBS 3-19=-191/199, 5-18=-1086/296, 5-17=-1081/2208, 6-17=-388/277, 7-17=-301/639, 7-16=-604/389, 8-16=-399/783, 8-15=-338/159, 9-15=-1032/949, 10-15=-1039/1665, 10-14=-2701/1842, 11-14=-334/286, 3-20=-208/246

JOINT STRESS INDEX

2 = 0.26, 3 = 0.27, 4 = 0.19, 5 = 0.70, 6 = 0.34, 7 = 0.36, 8 = 0.39, 9 = 0.50, 10 = 0.71, 11 = 0.34, 12 = 0.43, 14 = 0.38, 15 = 0.56, 16 = 0.53, 17 = 0.80, 18 = 0.71, 19 = 0.74 and 20 = 0.69

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NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 12, 513 lb uplift at joint 19 , 1332 lb uplift at joint 14 and 320 lb uplift at joint 20.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-5=-54, 5-8=-54, 10-13=-54, 19-20=-10, 17-18=-10, 15-17=-10, 12-15=-10
   Concentrated Loads (lb)
   Vert: 10=-320
   Trapezoidal Loads (plf)
   Vert: 8=-160-to-10=-250

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907401
L258990	T11	SPECIAL	2	1	Job Reference (optional)

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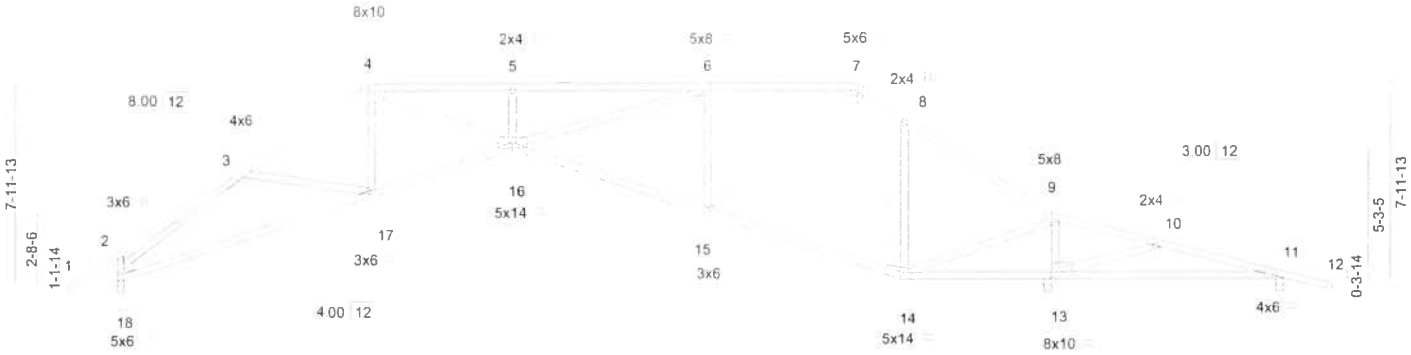
-2-0-0	5-3-3	10-2-15	16-2-0	24-1-0	30-2-14	32-0-0	38-2-1	42-4-8	47-8-0	49-8-0
2-0-0	5-3-3	4-11-11	5-11-1	7-11-0	6-1-14	1-9-2	6-2-1	4-2-8	5-3-8	2-0-0

Scale = 1:90.6



10-2-15	16-2-0	24-1-0	32-0-0	38-0-0	38-2-1	47-8-0
10-2-15	5-11-1	7-11-0	7-11-0	6-0-0	0-2-1	9-5-15

Plate Offsets (X,Y): [4:0-4-0,Edge], [6:0-3-8,0-2-8], [13:0-3-8,Edge], [18:0-2-14,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.42	16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.81	15-16	>562	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.52	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 264 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals, and 2-0-0 oc purlins (2-4-11 max.): 4-7.
BOT CHORD Rigid ceiling directly applied or 4-9-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-18
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 11=-49/0-4-0, 13=2085/0-4-0, 18=1227/0-3-8
Max Horz 18=-209(load case 4)
Max Uplift 11=-269(load case 10), 13=-610(load case 4), 18=-282(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/64, 2-3=-436/124, 3-4=-2282/994, 4-5=-3984/1672, 5-6=-3983/1672, 6-7=-1682/893, 7-8=-572/487, 8-9=-624/338, 9-10=-773/1827, 10-11=-385/1409, 11-12=0/25, 2-18=-405/281
BOT CHORD 17-18=-688/1851, 16-17=-536/1943, 15-16=-536/1778, 14-15=-82/668, 13-14=-1630/820, 11-13=-1350/432
WEBS 3-17=-131/243, 4-17=-30/183, 4-16=-838/2310, 5-16=-369/261, 6-16=-814/2406, 6-15=-1123/538, 7-15=-560/1370, 7-14=-838/377, 8-14=-231/252, 9-14=-905/2224, 9-13=-1849/912, 10-13=-521/463, 3-18=-2053/957

[redacted]

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JOINT STRESS INDEX

2 = 0.26, 3 = 0.60, 4 = 0.62, 5 = 0.33, 6 = 0.53, 7 = 0.42, 8 = 0.33, 9 = 0.72, 10 = 0.33, 11 = 0.48, 13 = 0.19, 14 = 0.73, 15 = 0.91, 16 = 0.74, 17 = 0.38 and 18 = 0.62

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 11, 610 lb uplift at joint 13 and 282 lb uplift at joint 18.

LOAD CASE(S) Standard

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Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907402
L258990	T12	SPECIAL	5	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 06 09:19:54 2007 Page 1

-2-0-0	5-3-3	10-2-15	16-2-0	23-2-7	30-2-14	36-7-1	38-2-1	42-4-8	47-8-0	49-8-0
2-0-0	5-3-3	4-11-11	5-11-1	7-0-7	7-0-7	6-4-3	1-7-0	4-2-8	5-3-8	2-0-0

Scale = 1:87.5

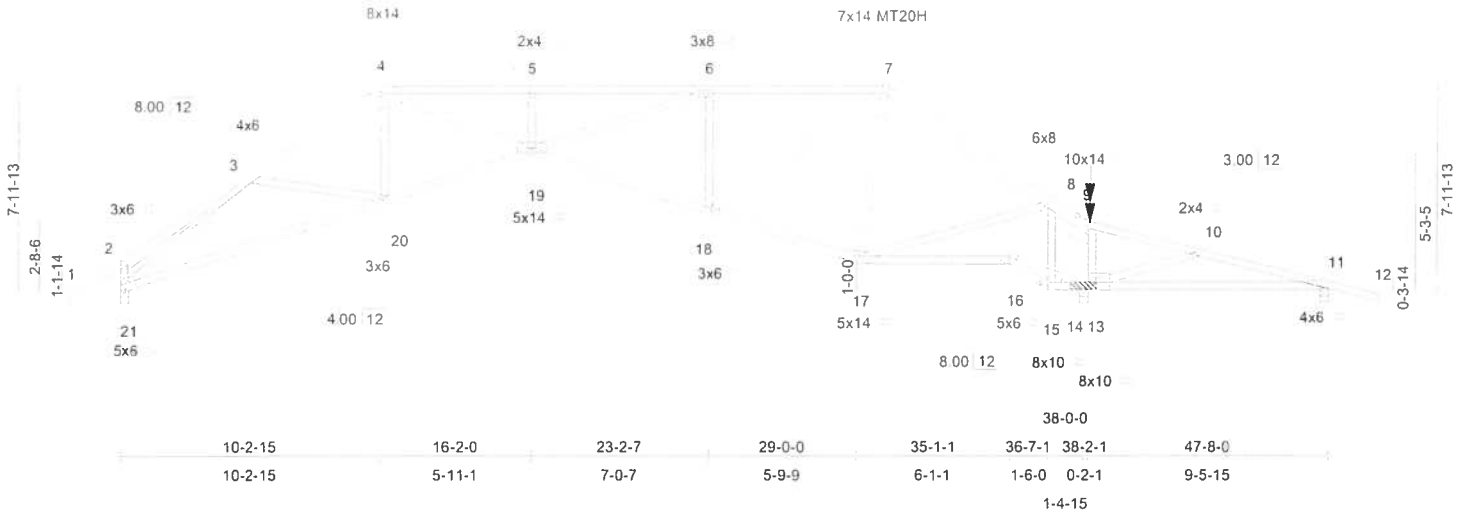

Plate Offsets (X,Y): [4:0-6-0,Edge], [6:0-3-8,0-1-8], [7:0-2-4,Edge], [9:0-7-0,0-3-5], [13:0-3-8,Edge], [15:0-4-0,Edge], [16:0-3-0,0-1-11], [21:0-2-14,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	0.52	18-19	>886	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.91	18-19	>502	240	MT20H	187/143
BCLL	10.0 *	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.60	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 275 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2 *Except* 7-9 2 X 6 SYP No.1D
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 2-21 2 X 4 SYP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-9-8 oc purlins, except end verticals, and 2-0-0 oc purlins (2-4-8 max.): 4-7.
BOT CHORD	Rigid ceiling directly applied or 4-4-7 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 3-21 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 11=-121/0-4-0, 13=3575/0-4-3 (0-4-0 + bearing block), 21=1327/0-4-0
Max Horz 21=-209(load case 4)
Max Uplift 11=-308(load case 10), 13=-1417(load case 4), 21=-348(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/64, 2-3=-446/141, 3-4=-2564/1251, 4-5=-4629/2268, 5-6=-4629/2269, 6-7=-2328/1394, 7-8=-1907/1285, 8-9=-111/461, 9-10=-1079/2155, 10-11=-635/1579, 11-12=0/25, 2-21=-419/293
BOT CHORD	20-21=-866/2046, 19-20=-763/2191, 18-19=-1064/2457, 17-18=-523/1168, 16-17=-407/225, 15-16=-360/172, 14-15=-1871/1065, 13-14=-1870/1062, 11-13=-1491/674
WEBS	3-20=-162/285, 4-20=-45/173, 4-19=-1250/2752, 5-19=-335/237, 6-19=-924/2433, 6-18=-1145/547, 7-18=-617/1477, 7-17=-762/406, 8-17=-709/1559, 8-16=-133/114, 8-15=-2603/1739, 9-15=-1865/3120, 9-13=-3225/1873, 10-13=-603/526, 3-21=-2278/1165

JOINT STRESS INDEX

2 = 0.30, 3 = 0.69, 4 = 0.84, 5 = 0.34, 6 = 0.96, 7 = 0.97, 8 = 0.53, 9 = 0.67, 10 = 0.34, 11 = 0.48, 13 = 0.39, 13 = 0.00, 14 = 0.00, 15 = 0.54, 16 = 0.30, 17 = 0.55, 18 = 0.95, 19 = 0.89, 20 = 0.39 and 21 = 0.71

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NOTES

1) 2 X 4 SYP No.2 bearing block 12" long at jt. 13 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 7.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 11, 1417 lb uplift at joint 13 and 348 lb uplift at joint 21.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-4=-54, 4-7=-54, 9-12=-54, 19-21=-10, 17-19=-10, 16-17=-10, 15-16=-10, 11-15=-10
   Concentrated Loads (lb)
   Vert: 9=-320
   Trapezoidal Loads (plf)
   Vert: 7=-160-to-9=-250

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907403
L258990	T12G	GABLE	1	1	

Job Reference (optional)

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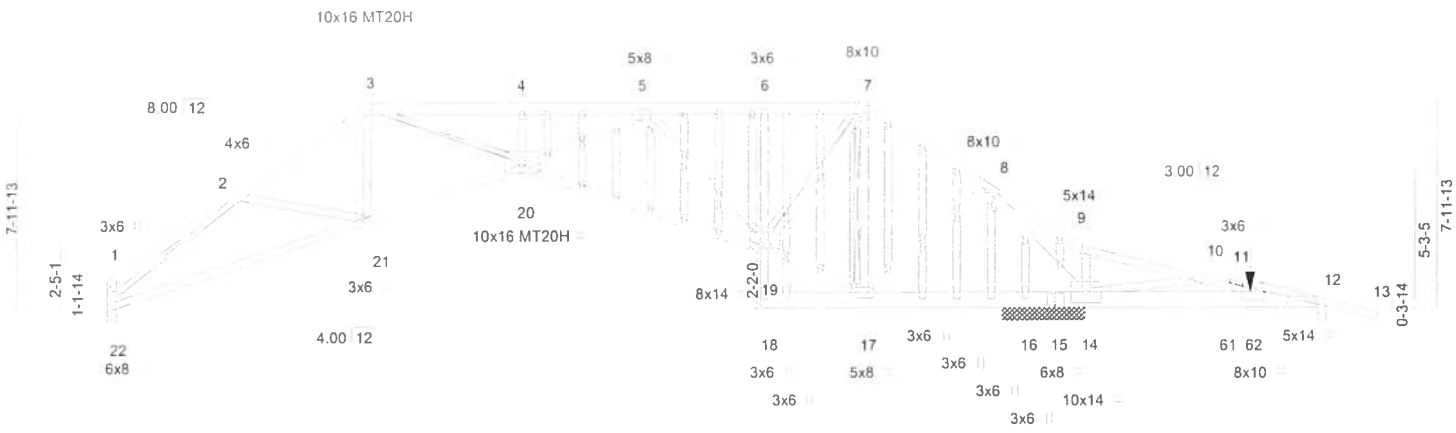
5-3-3	10-2-15	16-2-0	20-10-0	25-6-1	29-8-9	34-2-7	38-0-10	42-4-8	47-8-0	49-8-0
5-3-3	4-11-11	5-11-1	4-8-0	4-8-0	4-2-8	4-5-15	3-10-3	4-3-14	5-3-8	2-0-0

Scale = 1:86.7



10-2-15	16-2-0	25-6-1	29-8-9	36-6-8	38-0-10	47-8-0
10-2-15	5-11-1	9-4-1	4-2-8	6-9-15	1-6-2	9-7-6

Plate Offsets (X,Y): [3:1-0-12,0-2-12], [7:0-3-4,Edge], [12:0-1-8,0-4-0], [12:0-9-7,Edge], [17:0-2-0,0-0-12], [19:0-5-0,Edge], [20:0-8-0,0-3-11], [33:0-1-12,0-1-0], [38:0-1-2,0-1-4], [43:0-1-12,0-1-0], [57:0-0-12,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	0.86	19-20	>496	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-1.12	19-20	>380	240	MT20H	187/143
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.77	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 421 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 3-7 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP No.1D *Except* 20-22 2 X 4 SYP No.2, 19-20 2 X 4 SYP No.1D 6-18 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 *Except* 3-20 2 X 4 SYP No.2, 1-22 2 X 6 SYP No.1D

OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins, except end verticals, and 2-0-0 oc purlins (2-8-13 max.): 3-7.

BOT CHORD Rigid ceiling directly applied or 3-2-14 oc bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 3-20, 5-19, 7-19, 2-22, 7-17
2 X 6 SYP No.1D - 8-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 12=385/0-4-0, 14=3564/3-3-0, 22=1617/0-4-0, 16=738/3-3-0

Max Horz 22=-304(load case 3)

Max Uplift 12=-494(load case 4), 14=-2842(load case 3), 22=-967(load case 4), 16=-378(load case 4)

Max Grav 12=405(load case 10), 14=3564(load case 1), 22=1617(load case 1), 16=738(load case 1)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-538/297, 2-3=-3700/2538, 3-4=-7508/5568, 4-5=-7507/5568, 5-6=-3092/2500, 6-7=-3061/2486, 9-10=-1395/1873, 10-11=-855/1523, 11-12=-848/1502, 12-13=0/33, 1-22=-420/264, 7-8=-2028/1711, 8-9=-1682/2305

BOT CHORD 21-22=-1892/2843, 20-21=-2158/3224, 19-20=-3891/5176, 18-19=-142/211, 6-19=-632/636, 17-18=-94/61, 16-17=-842/1050, 15-16=-842/1050, 14-15=-842/1050, 14-61=-1468/871, 61-62=-1468/871, 12-62=-1468/871

WEBS 2-21=-479/507, 3-21=-182/291, 3-20=-3650/4792, 4-20=-402/416, 5-20=-1951/2902, 5-19=-2742/2146, 17-19=-1145/1642, 7-19=-2036/2579, 9-14=-256/439, 10-14=-608/606, 2-22=-3084/2085, 8-17=-616/718, 8-14=-4323/3188, 7-17=-1692/1319

JOINT STRESS INDEX

1 = 0.75, 2 = 0.97, 3 = 0.92, 4 = 0.34, 5 = 0.98, 6 = 0.30, 7 = 0.83, 8 = 0.96, 9 = 0.90, 10 = 0.34, 11 = 0.00, 11 = 0.35, 12 = 0.52, 12 = 0.14, 14 = 0.46, 15 = 0.83, 16 = 0.16, 17 = 0.75, 17 = 0.37, 18 = 0.36, 19 = 0.99, 20 = 0.95, 21 = 0.39, 22 = 0.73, 23 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.40, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.60, 39 = 0.34, 40 = 0.34, 41 = 0.34, 41 = 0.34, 42 = 0.16, 43 = 0.40, 43 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.16, 52 = 0.34, 52 = 0.34, 53 = 0.34, 54 = 0.16, 55 = 0.34, 55 = 0.34, 56 = 0.16, 57 = 0.69, 58 = 0.34, 59 = 0.34 and 60 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable studs spaced at 1-4-0 oc.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Bearing at joint(s) 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 494 lb uplift at joint 12, 2842 lb uplift at joint 14, 967 lb uplift at joint 22 and 378 lb uplift at joint 16.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-54, 4-7=-141(F=-87), 9-13=-54, 20-22=-10, 19-20=-10, 16-18=-10, 16-61=-125(F=-115), 12-61=-10, 7-9=-141(F=-87)
   Concentrated Loads (lb)
   Vert: 62=-354(F)

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Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE / LOT 69 EMERALD COVE J1907404
L258990	T13	COMMON	2	1	

Job Reference (optional)

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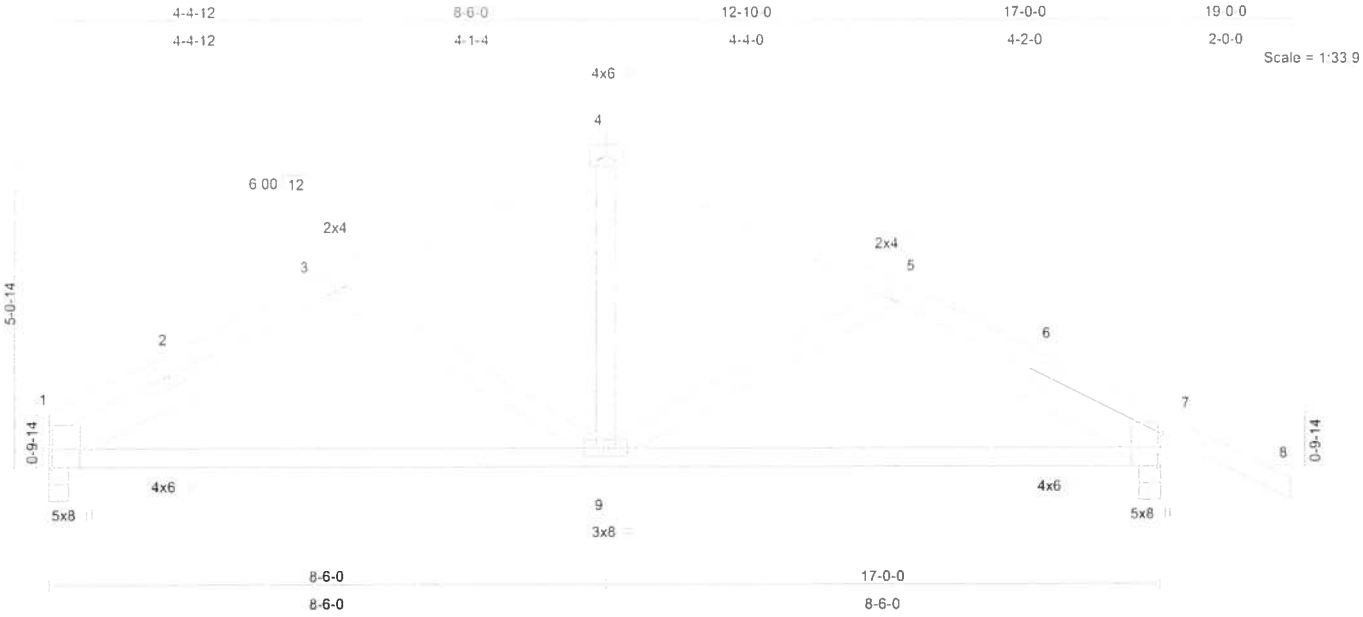

Plate Offsets (X,Y): [1:0-3-8,Edge], [7:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	1-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	1-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D 2-5-9, Right 2 X 6 SYP No.1D 2-4-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=538/0-3-8, 7=658/0-4-0
Max Horz 1=-81(load case 7)
Max Uplift 1=-117(load case 6), 7=-207(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-808/460, 2-3=-750/473, 3-4=-612/387, 4-5=-614/383, 5-6=-740/454, 6-7=-805/443, 7-8=0/26
BOT CHORD	1-9=-281/647, 7-9=-252/627
WEBS	3-9=-199/195, 4-9=-124/305, 5-9=-173/157

JOINT STRESS INDEX

1 = 0.62, 1 = 0.29, 2 = 0.00, 3 = 0.33, 4 = 0.45, 5 = 0.33, 6 = 0.00, 7 = 0.62, 7 = 0.29 and 9 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

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