Columbia County Building Permit Application 494.74

Revised 9-23

For Office Use Only Application # 0601-34 Date Received 1-12-06 By LH Permit # 24234/11

Application Approved by - Zoning Official RLK Date 1.01.06 Plans Examiner OKJTH Date 3-8-06

Flood Zone X Per PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res Low Den

Comments Alternative termite treatment received

Applicants Name SUSAN FAIR Phone 386 752-1711

Address 224 SW VERNON WAY, LAKE CITY, FL 32024

Owners Name Cornerstone Developers, LLC. Phone 752.1711

911 Address 1169 SW Timberland CT Lake City, FL 32025

Contractors Name Bryan Zecher Construction Phone 752-8653

Address PO Box 815 Lake City FL 32056

Fee Simple Owner Name & Address Cornerstone Developers, LLC.

Bonding Co. Name & Address NA

Architect/Engineer Name & Address MARK DISOSWAY PO BOX 868 LAKE CITY, FL 32056

Mortgage Lenders Name & Address NA

Circle the correct power company - (FL Power & Light) - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-16-02438-172 Estimated Cost of Construction _____

Subdivision Name Emerald Cove Lot 72 Block ____ Unit ____ Phase 1

Driving Directions Take Hwy 90 W - go two miles past I-75 - subdivision is on the left. TURN LEFT & GO TO FIRST RIGHT. TURN RIGHT & LOT IS ON LEFT.

Type of Construction Frame + Brick Number of Existing Dwellings on Property Ø

Total Acreage ½ acre Lot Size .50 acres Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 32' Side 40' Side 34' Rear 94'

Total Building Height 18'9" Number of Stories 1 Heated Floor Area 1500 sq ft Roof Pitch 6/12

PORCH 44 GARAGE 430 TOTAL 1974

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CBC054575

Competency Card Number _____

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

SUSAN ANN FAIR
MY COMMISSION # DD 3428
EXPIRES: August 14, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 17 day of Nov 2005.

Personally known ✓ or Produced Identification _____

Notary Signature

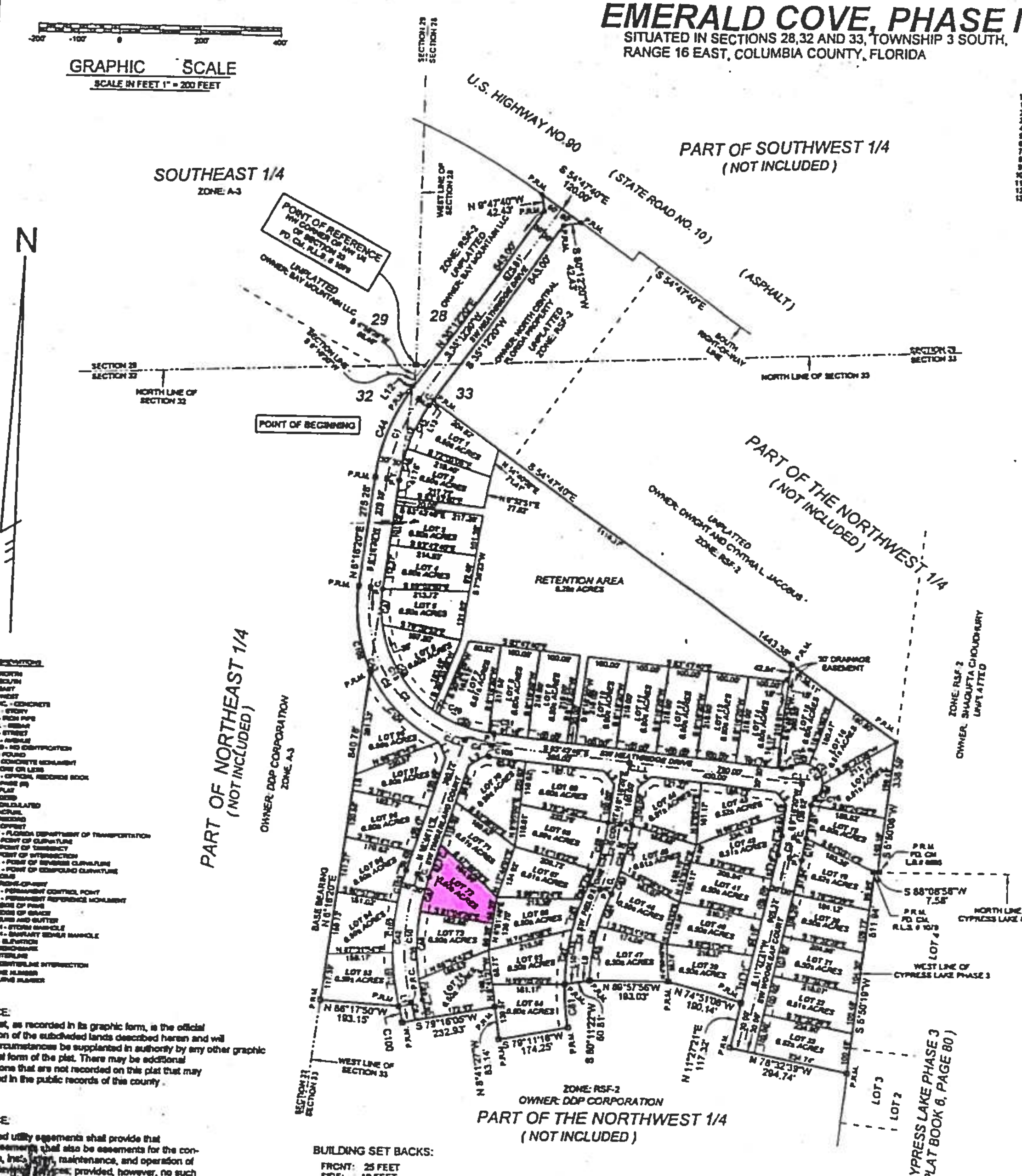
BOOK PAGE
1056/211

EMERALD COVE, PHASE I

SITUATED IN SECTIONS 28,32 AND 33, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

PLAT BOOK 8, PAGE 36

SHEET 2 OF 2



NOTICE:

This plat, as recorded in its graphic form, is the official depiction of the subdivided lands described herein and will in no circumstances be supplanted in authority by any other graphic or digital form of the plat. There may be additional restrictions that are not recorded on this plat that may be found in the public records of this county.

NOTICE:

All platted utility [redacted] easements shall provide that such easements shall also be easements for the construction, installation, maintenance, and operation of cable television [redacted]; provided, however, no such construction, [redacted]tion, maintenance, and operation of cable [redacted] services shall interfere with the facilities [redacted] an electric, telephone, gas, or other public [redacted] event a cable television company damage[redacted] facilities of a public utility, it shall be solely respo[redacted] for the damages.

BUILDING SET BACKS:

FRONT: 25 FEET
SIDE: 10 FEET
REAR: 15 FEET

ZONING:

RSF-2 - RESIDENTIAL SINGLE FAMILY 2

LINE TABLE

Line	Bearing	Distance
1	S 24°48'07" E	44.86'
2	N 58°24'28" E	44.86'
3	N 38°43'40" W	42.43'
4	S 51°16'20" W	42.43'
5	N 38°43'40" W	42.43'
6	N 08°16'20" E	31.28'
7	S 79°16'05" W	30.00'
8	N 08°16'20" E	40.31'
9	S 80°11'22" W	30.47'
10	S 80°11'22" W	30.36'
11	S 79°16'05" W	30.00'
12	N 36°12'20" E	6.07'
13	S 36°12'20" W	2.81

CURVE DATA

Curve	Delta Angle	Radius	Arc	Tangent	Chord	Chord Bearing
1	28°36'00"	450.00	227.24	116.10	224.84	S 20°44'20"W
2	90°00'00"	350.00	549.78	350.00	494.97	S 34°43'40"E
3	79°25'09"	350.00	485.45	290.83	447.46	S 33°27'45"E
4	10°31'51"	350.00	64.33	32.26	64.24	S 78°27'46"E
5	6°11'01"	500.00	54.28	27.18	54.27	S 6°31'31"W
6	4°16'36"	600.00	44.79	22.40	44.77	S 12°32'12"E
7	16°37'03"	600.00	174.02	87.62	173.41	S 14°34'32"W
8	23°21'07"	600.00	244.54	123.98	242.85	S 11°12'30"W
10	31°48'42"	600.00	333.13	170.96	328.87	S 6°32'30"W
12	28°36'00"	420.00	212.00	108.36	209.85	S 20°44'20"W
13	90°00'00"	320.00	502.65	320.00	452.55	S 34°43'40"E
14	41°24'35"	30.00	21.68	11.34	21.21	N 73°34'03"E
15	172°49'05"	60.00	180.91	795.80	119.77	S 36°43'40"E
16	41°24'35"	30.00	21.68	11.34	21.21	S 29°53'37"W
17	6°11'01"	630.00	67.00	33.52	66.96	S 6°31'31"W
24	6°11'01"	570.00	61.57	30.80	61.54	N 6°31'31"E
25	16°37'03"	630.00	182.73	92.01	182.08	S 14°34'32"W
28	22°51'16"	570.00	227.37	115.22	225.87	S 11°27'44"W
33	23°48'03"	630.00	261.70	132.77	259.83	S 10°39'22"W
34	16°37'03"	570.00	165.32	83.24	164.74	N 14°34'32"E
35	31°48'42"	570.00	316.47	162.43	312.43	S 6°32'30"W
42	31°48'42"	630.00	349.78	179.53	345.31	N 6°32'30"E
43	79°25'09"	350.00	487.02	288.14	426.18	N 23°36'19"W
44	28°40'11"	480.00	240.18	122.68	237.68	N 20°38'23"E
45	17°38'35"	420.00	131.41	66.24	130.87	S 20°14'33"W
46	11°00'25"	420.00	80.69	40.47	80.56	S 11°45'33"W
47	16°05'48"	320.00	89.90	45.25	89.61	S 1°45'25"E
48	45°31'25"	320.00	250.57	132.69	244.17	S 27°15'08"E
49	3°46'27"	320.00	20.57	10.29	20.52	S 56°31'08"E
50	23°47'23"	320.00	132.87	67.40	131.91	S 70°16'58"E
51	1°35'01"	320.00	8.84	4.42	8.84	S 67°36'16"E
52	53°22'47"	50.00	46.58	25.18	44.89	N 81°38'39"E
53	73°44'23"	50.00	64.35	37.50	60.00	S 34°48'16"E
54	45°36'58"	50.00	39.81	21.03	38.76	S 24°53'25"W
[illegible]	11°04'38"	630.00	121.77	61.08	121.58	S 11°48'34"W
[illegible]	9°35'25"	630.00	[illegible]	30.90	[illegible]	S 20°07'08"W
[illegible]	6°48'03"	570.00	67.66	43.61	66.57	S 10°30'58"W
[illegible]	14°06'15"	570.00	140.31	70.51	139.96	S 7°08'12"W
[illegible]	9°54'05"	630.00	108.86	54.57	108.73	S 5°?1'41"E
[illegible]	12°53'04"	630.00	141.67	71.14	141.37	N 5°31'52"E
[illegible]	7°16'35"	630.00	80.05	40.08	80.00	N 15°36'49"E
[illegible]	3°36'08"	630.00	39.66	23.00	39.37	N 21°04'10"E
[illegible]	4°01'25"	570.00	40.04	20.03	40.03	N 20°32'30"E
[illegible]	12°35'36"	570.00	125.28	62.89	125.03	N 12°34'07"E
87	8°53'27"	570.00	88.46	44.31	88.36	S 12°11'10"W
[illegible]	17°29'56"	570.00	174.08	87.73	173.41	S 0°?0'14"E
[illegible]	9°35'20"	570.00	53.94	26.99	53.92	S 12°17'51"E
[illegible]	4°16'36"	630.00	47.03	23.52	47.01	S 12°32'12"E
100	4°16'36"	570.00	42.56	21.29	42.54	N 12°32'12"W
101	12°32'29"	630.00	137.89	69.22	137.61	N 8°44'19"W
102	11°41'54"	630.00	128.64	64.54	128.41	N 3°23'32"E
103	7°34'21"	630.00	83.28	41.69	83.20	N 13°01'48"E
104	37°48'47"	380.00	250.78	130.09	246.20	N 45°16'16"W
105	32°38'15"	380.00	216.48	111.25	213.54	N 10°02'48" W

FLOOD ZONE INFORMATION:

THE PROPERTY AS SURVEYED FALLS WITHIN ZONE "X" AS PER THE FLOOD INSURANCE RATE MAP OF THE FEDERAL EMERGENCY MANAGEMENT AGENCY PANEL NO. 120070 0175B

UTILITY EASEMENT DETAIL:

ROAD RIGHT-OF-WAY

DENOTES 20' UTILITY EASEMENT ALONG ROAD RIGHTS-OF-WAY. EASEMENTS FOR UTILITIES SHALL ALSO BE EASEMENTS FOR THE CONSTRUCTION, INSTALLATION, MAINTENANCE AND OPERATION OF CABLE TELEVISION SERVICES.

CERTIFICATE OF SUBDIVIDER'S ENGINEER

THIS IS TO CERTIFY THAT ON THIS 29 OF JULY, 2005, GARY GILL, REGISTERED FLORIDA ENGINEER, AS SPECIFIED WITHIN CHAPTER 471, FLORIDA STATUTES, LICENSE NO. 51942, DOES HEREBY CERTIFY THAT ALL REQUIRED IMPROVEMENTS HAVE BEEN INSTALLED IN COMPLIANCE WITH THE APPROVED CONSTRUCTION PLANS AND IFF APPLICABLE, ANY SUBMITTED "AS-BUILT" BLUEPRINTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE BOARD OF COUNTY COMMISSIONERS OF COLUMBIA COUNTY, FLORIDA.

REGISTERED FLORIDA ENGINEER

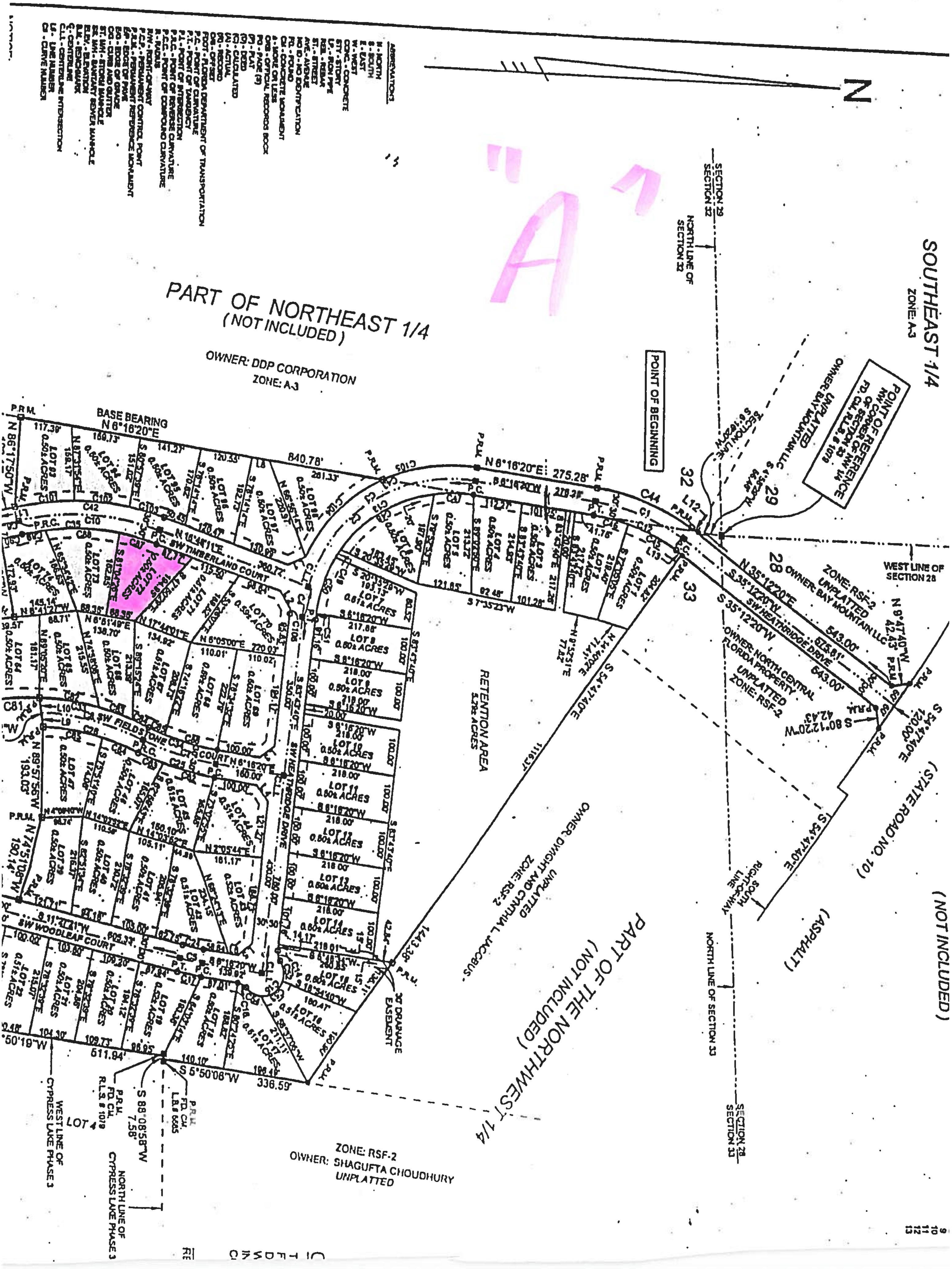
CERTIFICATE OF SURVEYOR

I, THE UNDERSIGNED PROFESSIONAL SURVEYOR AND MAPPER, HEREBY CERTIFY THAT THIS PLAT IS A TRUE AND CORRECT REPRESENTATION OF THE LANDS SURVEYED, THAT THE SURVEY DATA COMPLIES WITH ALL REQUIREMENTS OF CHAPTER 177, FLORIDA STATUTES AND THE COLUMBIA COUNTY LAND DEVELOPMENT CODE AND THAT THE PERMANENT REFERENCE MONUMENTS WERE INSTALLED AS OF THE 29 DAY OF JULY, 2005.

TIMOTHY B. ALCORN
REGISTERED LAND SURVEYOR
FLA. CERT. NO. 5332
DATE: JULY 29, 2005
JOB. NO. 387.2003RP

J. SHERMAN FRIER & ASSOCIATES, INC.
LAND SURVEYORS
CERTIFICATE OF AUTHORIZATION - LB # 7170
130 WEST HOWARD STREET, P.O. BOX 580, LIVE OAK, FLORIDA 32064
PHONE: 386 - 362 - 4629 FAX: 386 - 362 - 5270

BOOK 1056 PAGE 211



PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2214

Property Appraiser's 02435-000
Identification Number 02421-000

TM File No: 05-652

WARRANTY DEED

This Warranty Deed, made this 18th day of October, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor, and CORNERSTONE DEVELOPERS, LLC, a Florida Limited Liability Company, whose post office address is P.O. Box 815, Lake City, Florida 32056, grantee.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 65,66,67,68,71,72,73,74,93,94,95,96,97 & 98, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First Witness)

DeEtte F. Brown

(Typed Name of First Witness)

(Signature of Second Witness)

Karen M. Wright

(Typed Name of Second Witness)

D D P CORPORATION

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 18th day of October, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is/are personally known to me or who has/have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

KAREN M. WRIGHT
Notary Public - State of Florida
My Commission Expires Jan 23, 2008
Commission # DD069234
Bonded By National Notary Assn.

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2215

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-162(c)N

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.

See attached

Notes: _____

Site Plan submitted by: _____ MASTER CONTRACTOR

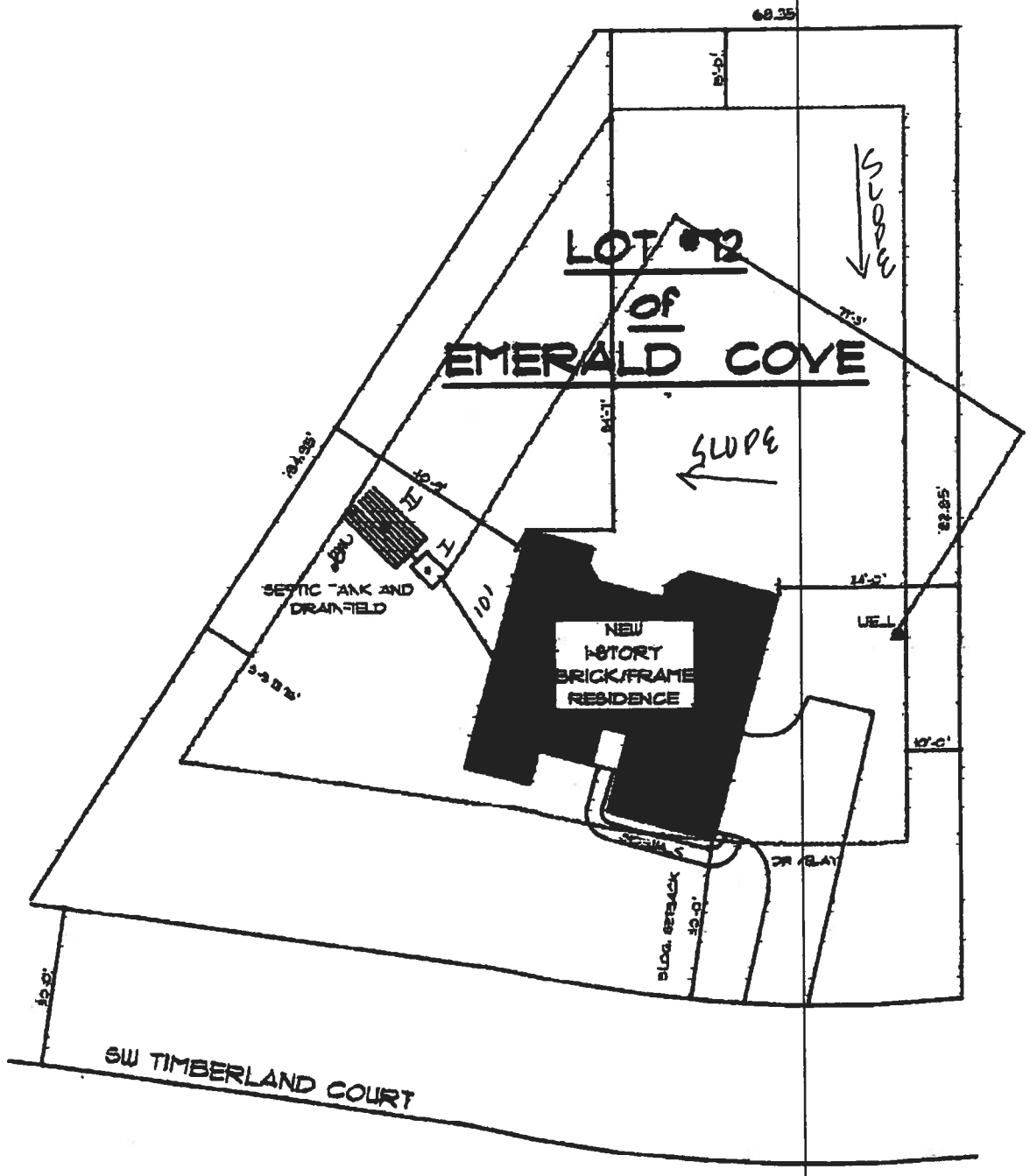
Plan Approved X Not Approved ____ Date OCT 24 2005

By: _____ COLUMBIA County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

05-1126N

o-t-c pay



• SITE PLAN •

SCALE : 1" = 20'-0'

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 3205
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**TheSamuelModel**	Builder:	**Bryan Zecher Constructio**
Address:	**Lot: 72, Sub: Emerald cove, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	24234
Owner:	**Model Home**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	
2. Single family or multi-family	Single family	
3. Number of units, if multi-family	1	
4. Number of Bedrooms	3	
5. Is this a worst case?	Yes	
6. Conditioned floor area (ft²)	1500 ft²	
7. Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)		
a. U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 122.0 ft²	
b. SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 122.0 ft²	
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 196.0(p) ft	
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 172.0 ft²	
b. Frame, Wood, Exterior	R=13.0, 1051.0 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 1754.0 ft²	
b. N/A		
c. N/A		
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 150.0 ft	
b. N/A		

12. Cooling systems		
a. Central Unit	Cap: 34.0 kBtu/hr SEER: 12.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 34.0 kBtu/hr HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons EF: 0.93	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.16

Total as-built points: 23591
Total base points: 23606

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ben Granda

DATE: 11-7-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Chris W. Cr

DATE: 1-10-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	**1500.0**	**20.04**	**5410.8**	Double, Clear	SE	2.0	5.5	45.0	42.75	0.77	1488.9	
				Double, Clear	S	14.5	7.0	10.0	35.87	0.45	161.1	
				Double, Clear	SE	12.0	5.3	45.0	42.75	0.39	759.0	
				Double, Clear	E	8.5	7.0	10.0	42.06	0.47	196.6	
				Double, Clear	SW	2.0	1.5	3.0	40.16	0.44	53.0	
				Double, Clear	NW	9.5	7.5	6.0	25.97	0.60	93.2	
				Double, Clear	NW	2.0	5.5	30.0	25.97	0.86	666.7	
				Double, Clear	NW	2.0	8.0	30.0	25.97	0.93	721.1	
				Double, Clear	NE	1.5	3.5	3.0	29.56	0.80	71.0	
				Double, Clear	SE	1.5	5.5	30.0	42.75	0.86	1104.3	
				Double, Clear	SW	1.5	5.5	30.0	40.16	0.86	1039.8	
				As-Built Total:				**242.0**			**6354.8**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	172.0	0.70	120.4	Frame, Wood, Adjacent	13.0	172.0	0.60	103.2
Exterior	1051.0	1.70	1786.7	Frame, Wood, Exterior	13.0	1051.0	1.50	1576.5
Base Total:	**1223.0**		**1907.1**	**As-Built Total:**		**1223.0**		**1679.7**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	20.0	2.40	48.0	Exterior Insulated	20.0	4.10	82.0
Exterior	40.0	6.10	244.0	Exterior Insulated	20.0	4.10	82.0
				Adjacent Insulated	20.0	1.60	32.0
Base Total:	**60.0**		**292.0**	**As-Built Total:**	**60.0**		**196.0**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM =	Points
Under Attic	1500.0	1.73	2595.0	Under Attic	30.0	1754.0	1.73 X 1.00	3034.4
Base Total:	**1500.0**		**2595.0**	**As-Built Total:**		**1754.0**		**3034.4**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	196.0(p)	-37.0	-7252.0	Slab-On-Grade Edge Insulation	0.0	196.0(p	-41.20	-8075.2
Raised	0.0	0.00	0.0					
Base Total:			**-7252.0**	**As-Built Total:**		**196.0**		**-8075.2**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points			Area X	SPM	=	Points	
	1500.0	10.21	15315.0			1500.0	10.21		15315.0	
Summer Base Points: 18267.9				**Summer As-Built Points:**					**18504.7**	
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
				(sys 1: Central Unit 34000 btuh ,SEER/EFF(12.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)						
				18505	1.00	(1.09 x 1.147 x 1.00)	0.284	1.000	6580.0	
18267.9	**0.4266**	**7793.1**		**18504.7**	**1.00**	**1.250**	**0.284**	**1.000**	**6580.0**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1500.0	12.74	3439.8	Double, Clear	SE	2.0	5.5	45.0	14.71	1.21	798.3	
				Double, Clear	S	14.5	7.0	10.0	13.30	3.52	468.1	
				Double, Clear	SE	12.0	5.3	45.0	14.71	2.54	1682.8	
				Double, Clear	E	8.5	7.0	10.0	18.79	1.34	252.1	
				Double, Clear	SW	2.0	1.5	3.0	16.74	1.76	88.2	
				Double, Clear	NW	9.5	7.5	6.0	24.30	1.03	149.9	
				Double, Clear	NW	2.0	5.5	30.0	24.30	1.01	734.7	
				Double, Clear	NW	2.0	8.0	30.0	24.30	1.00	731.2	
				Double, Clear	NE	1.5	3.5	3.0	23.57	1.02	72.1	
				Double, Clear	SE	1.5	5.5	30.0	14.71	1.11	491.5	
				Double, Clear	SW	1.5	5.5	30.0	16.74	1.07	538.4	
				As-Built Total:				**242.0**			**6007.3**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	172.0	3.60	619.2	Frame, Wood, Adjacent	13.0	172.0	3.30	567.6
Exterior	1051.0	3.70	3888.7	Frame, Wood, Exterior	13.0	1051.0	3.40	3573.4
Base Total:	**1223.0**		**4507.9**	**As-Built Total:**		**1223.0**		**4141.0**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	20.0	11.50	230.0	Exterior Insulated	20.0	8.40	168.0
Exterior	40.0	12.30	492.0	Exterior Insulated	20.0	8.40	168.0
				Adjacent Insulated	20.0	8.00	160.0
Base Total:	**60.0**		**722.0**	**As-Built Total:**	**60.0**		**496.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM	= Points
Under Attic	1500.0	2.05	3075.0	Under Attic	30.0	1754.0	2.05 X 1.00	3595.7
Base Total:	**1500.0**		**3075.0**	**As-Built Total:**		**1754.0**		**3595.7**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	196.0(p)	8.9	1744.4	Slab-On-Grade Edge Insulation	0.0	196.0(p	18.80	3684.8
Raised	0.0	0.00	0.0					
Base Total:			**1744.4**	**As-Built Total:**		**196.0**		**3684.8**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points			Area X	WPM	=	Points	
	1500.0	-0.59	-885.0			1500.0	-0.59		-885.0	
Winter Base Points:			**12604.1**	**Winter As-Built Points:**						**17039.8**
Total Winter Points	X	System Multiplier	= Heating Points	Total Component (System - Points)	X	Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
				(sys 1: Electric Heat Pump 34000 btuh ,EFF(7.9) Ducts:Unc(S),Unc(R),Gar(AH),R6.0						
				17039.8	1.000	(1.069 x 1.169 x 1.00)	0.432	1.000		9191.5
12604.1		**0.6274**	**7907.8**	**17039.8**	**1.00**	**1.250**	**0.432**	**1.000**		**9191.5**

FORM 600A-2004 EnergyGauge® 4.0

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0
				As-Built Total:							**7820.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7793	**7908**	**7905**	**23606**	**6580**	**9191**	**7820**	**23591**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 82.8

The higher the score, the more efficient the home.

Model Home, Lot: 72, Sub: Emerald cove, Plat: , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 34.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 12.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1500 ft²		c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 122.0 ft²		a. Electric Heat Pump	Cap: 34.0 kBtu/hr
	b. SHGC:				HSPF: 7.90
	(or Clear or Tint DEFAULT) 7b.	(Clear) 122.0 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 196.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Adjacent	R=13.0, 172.0 ft²			EF: 0.93
	b. Frame, Wood, Exterior	R=13.0, 1051.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1754.0 ft²	15.	HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 150.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Chris W. Cr Date: 1-10-06

Address of New Home: 169 SW TIMBERLAND CT. City/FL Zip: LAKE CITY FL.

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

BUILDING INPUT SUMMARY REPORT

PROJECT

Title:	TheSamuelModel	**Family Type:**	Single	**Address Type:**	Lot Information
Owner:	Model Home	**New/Existing:**	New	**Lot #:**	72
# of Units:	1	**Bedrooms:**	3	**Subdivision:**	Emerald cove
Builder Name:	Bryan Zecher Construction	**Conditioned Area:**	1500	**Platbook:**	(blank)
Climate:	North	**Total Stories:**	1	**Street:**	N/A
Permit Office:	(blank)	**Worst Case:**	Yes	**County:**	(blank)
Jurisdiction #:	(blank)	**Rotate Angle:**	(blank)	**City, St, Zip:**	Lake City, FL,

FLOORS

#	Floor Type	R-Val	Area/Perimeter	Units
1	Slab-On-Grade Edge Insulation	0.0	196.0(p) ft	1

DOORS

#	Door Type	Orientation	Area	Units
1	Insulated	Exterior	10.0 ft²	2
2	Insulated	Exterior	20.0 ft²	1
3	Insulated	Adjacent	20.0 ft²	1

CEILINGS

#	Ceiling Type	R-Val	Area	Base Area	Units
1	Under Attic	30.0	1754.0 ft²	1500.0 ft²	1

Credit Multipliers: None

COOLING

#	System Type	Efficiency	Capacity
1	Central Unit	SEER: 12.00	34.0 kBtu/hr

Credit Multipliers: None

WALLS

#	Wall Type	Location	R-Val	Area	Units
1	Frame - Wood	Adjacent	13.0	172.0 ft²	1
2	Frame - Wood	Exterior	13.0	1051.0 ft²	1

HEATING

#	System Type	Efficiency	Capacity
1	Electric Heat Pump	COP: 7.90	34.0 kBtu/hr

Credit Multipliers: None

WINDOWS

#	Panes	Tint	Ornt	Area	OH Length	OH Hght	Units
1	Double	Clear	N	15.0 ft²	2.0 ft	5.5 ft	3
2	Double	Clear	NE	10.0 ft²	14.5 ft	7.0 ft	1
3	Double	Clear	N	15.0 ft²	12.0 ft	5.3 ft	3
4	Double	Clear	NW	10.0 ft²	8.5 ft	7.0 ft	1
5	Double	Clear	E	3.0 ft²	2.0 ft	1.5 ft	1
6	Double	Clear	S	6.0 ft²	9.5 ft	7.5 ft	1
7	Double	Clear	S	15.0 ft²	2.0 ft	5.5 ft	2
8	Double	Clear	S	15.0 ft²	2.0 ft	8.0 ft	2
9	Double	Clear	W	3.0 ft²	1.5 ft	3.5 ft	1
10	Double	Clear	N	15.0 ft²	1.5 ft	5.5 ft	2
11	Double	Clear	E	15.0 ft²	1.5 ft	5.5 ft	2

DUCTS

#	Supply Location	Return Location	Air Handler Location	Supply R-Val	Supply Length
1	Uncond.	Uncond.	Garage	6.0	150.0 ft

Credit Multipliers: None

WATER

#	System Type	EF	Cap.	Conservation Type	Con. EF
1	Electric Resistance	0.93	40.0	None	0.00

REFR.

#	Use Default?	Annual Operating Cost	Electric Rate
1	Yes	N/A	N/A

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code 104.2.6)

Date: _____

_____ _____

(Address of Treatment or Lot/Block of Treatment) City

Florida Pest Control & Chemical Co.

www.flapest.com

Product to be used: Bora-Care Termiticide (Wood Treatment)

Chemical to be used: 23% Disodium Octaborate Tetrahydrate

Application will be performed onto structural wood at dried-in stage of construction. Bora-Care Termiticide application shall be applied according to EPA registered label directions as stated in the Florida Building Code Section 1861.1.8

(Information to be provided to local building code offices prior to concrete foundation installation.)

6/04

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001005

DATE 03/14/2006 PARCEL ID # 33-3S-16-02438-172

APPLICANT SUSAN FAIR PHONE 752-1711

ADDRESS 224 SW VERNON WAY LAKE CITY FL 32024

OWNER CORNERSTONE DEVELOPERS PHONE 752-1711

ADDRESS 169 SW TIMBERLAND COURT LAKE CITY FL 32025YAN Z

CONTRACTOR BRYAN ZECHER PHONE 752-8653

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, TR ON TIMBERLAND COURT, 3RD ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 72 1

SIGNATURE Susan Fair

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY FLORIDA

TMD

Rec. 35.5t
Cert. Copy 5.5t

Prepared by and after
recording return to:

William L. Joel
Stonebumer Berry & Simmons, P.A.
841 Prudential Drive, Suite 1400
Jacksonville, FL 32207

24234

Permit No. _____
Tax Folio No. _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with section 713 Florida Statutes, the following information is provided in this notice of commencement.

1. Description of property (legal description and address, if available):

   See Exhibit "A" attached hereto.

2. General description of improvements: Construction of residential dwellings

3. Owner Information:

   (a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
   180 NW Amenity Court
   Lake City, Florida 32025

   (b) Owner's interest in the site of the improvements (if other than fee simple title holder):

   (c) Name and Address of fee simple title holder (if other than owner):

4. Contractor:

   Name and Address: Bryan Zecher Construction Inc.
   465 NW Orange Street
   Lake City, FL 32055

   (b) Phone No. 386-752-8653 Fax No. _____ (Optional, if service by fax is acceptable)

5. Surety on any payment bond: N/A

   (a) Name and Address:
   (b) Phone No. _____ Fax No. _____ (Optional, if service by fax is acceptable)

Inst:2006006012 Date:03/13/2006 Time:09:12
D.C.,P.DeWitt Cason,Columbia County B:1076 P:2469

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By: Karen Teagle
Deputy Clerk
Date 3-13-06

COLUMBIA COUNTY CLERK CIRCUIT COURT FLORIDA

(c) Amount of bond $_____

6. Lender making loan for the construction of the improvements:

(a) Name and Address: First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256
Attn: James J. O'Connor, Jr.

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

7. Persons within the State of Florida designated by owner upon whom notices may be served as provided by Section 713.13(1)(a)7, Florida Statutes:

(a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Phone No. (386) 752-1711 Fax No. _____ (Optional, if service by fax is acceptable)

8. In addition to himself, Owner designates the following person to receive a copy of the lienor's notice as provided in Section 713.13(1)(b), Florida Statute:

(a) Name and Address: James J. O'Connor, Jr.
First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

9. Expiration date of notice of commencement (the expiration date is one (1) year from the date of recording unless a different date is specified):

CORNERSTONE DEVELOPERS, LLC

By:_____
Frank Soucinek, its sole Manager

(SEAL)

(OWNER)

Inst:2006006012 Date:03/13/2006 Time:09:12
_____DC,P.DeWitt Cason,Columbia County B:1076 P:2470

Sworn to and Subscribed before me this 8th day of March, 2006, by Frank Soucinek, the Manager of Cornerstone Developers, LLC. He is personally known or has produced _____ as identification.

_____

Notary Public, State and County aforesaid
Print Name:
My Commission Expires:
My Commission No.:

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

(NOTARIAL SEAL)

POST A CERTIFIED COPY OF THE RECORDED NOTICE ON CONSTRUCTION SITE

Inst:2006006012 Date:03/13/2006 Time:09:12
_____DC,P.DeWitt Cason,Columbia County B:1076 P:2471

EXHIBIT A

Lot 72, Emerald Cove, Phase 1, according to the map or plat thereof as recorded in Plat Book 8, Page(s) 35 and 36, Public Records of Columbia County, Florida.

Inst:2006006012 Date:03/13/2006 Time:09:12
DC,P.DeWitt Cason,Columbia County B:1076 P:2472

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0601-34

Bryan Zecher/Cornerstone Developers Lot 72 of Emerald Cove

On the date of January 18, 2006 application 0601-34 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0601-34 when making reference to this application.

1. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department.

2. Please have Mr. Mark Disosway supply the following information, show all required connectors with uplift rating and required number and size of fasteners for continuous tie from the roof truss system to foundation. These connection points shall be designed by a Windload engineer using the engineered roof truss plans.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☑ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☑ ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- ☑ ☐ c) Any special support required by soil analysis such as piling
- ☑ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☐ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

NA

Wall Sections including:

- ☐ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termiticide or alternative method)
  10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

NA

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) ~~Metal frame wall and roof (designed~~, signed and sealed by Florida Prof Engineer ~~or Architect~~)

N/A

Floor Framing System:

☐ ☐ ~~a) Floor truss package including~~ layout and details, signed and sealed by Florida ~~Registered Professional~~ Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

N/A

☐ ☐ d) Attachment ~~of joist to~~ girder

☐ ☐ ~~e) Wind~~ load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment N/A

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water — Private water system

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

Residential System Sizing Calculation

Summary

Model Home

Lake City, FL

Project Title:
TheSamuelModel

Class 3 Rating
Registration No. 0
Climate: North

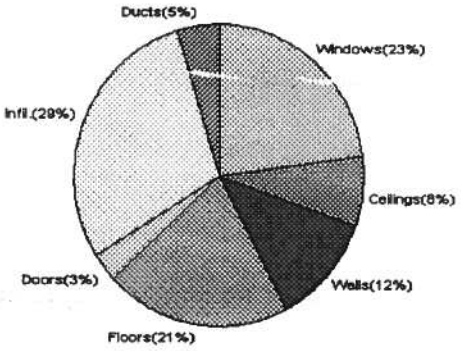
11/7/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**29793**	**Btuh**	**Total cooling load calculation**	**28117**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	114.1	34000	Sensible (SHR = 0.75)	123.5	25500
Heat Pump + Auxiliary(0.0kW)	114.1	34000	Latent	113.9	8500
			Total (Electric Heat Pump)	120.9	34000

WINTER CALCULATIONS

Winter Heating Load (for 1500 sqft)

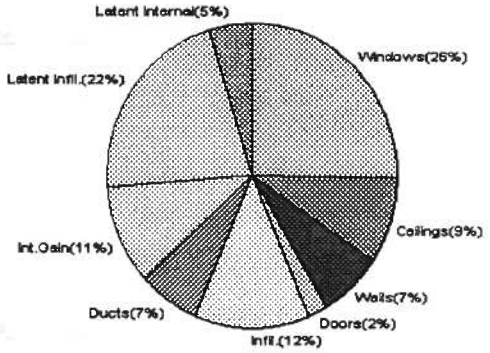
Load component			Load	
Window total	242	sqft	6849	Btuh
Wall total	1223	sqft	3533	Btuh
Door total	60	sqft	921	Btuh
Ceiling total	1754	sqft	2280	Btuh
Floor total	196	ft	6194	Btuh
Infiltration	200	cfm	8597	Btuh
Subtotal			**28374**	**Btuh**
Duct loss			1419	Btuh
TOTAL HEAT LOSS			**29793**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1500 sqft)

Load component			Load	
Window total	242	sqft	7199	Btuh
Wall total	1223	sqft	2008	Btuh
Door total	60	sqft	608	Btuh
Ceiling total	1754	sqft	2491	Btuh
Floor total			0	Btuh
Infiltration	175	cfm	3472	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**18778**	**Btuh**
Duct gain			1878	Btuh
Total sensible gain			**20656**	**Btuh**
Latent gain(infiltration)			6081	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**7461**	**Btuh**
TOTAL HEAT GAIN			**28117**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: Ben Leonard

DATE: 11-7-05

System Sizing Calculations - Winter

Residential Load - Component Details

Model Home

Lake City, FL

Project Title:
TheSamuelModel

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

11/7/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	45.0	28.3	1274 Btuh
2	2, Clear, Metal, DEF	NE	10.0	28.3	283 Btuh
3	2, Clear, Metal, DEF	N	45.0	28.3	1274 Btuh
4	2, Clear, Metal, DEF	NW	10.0	28.3	283 Btuh
5	2, Clear, Metal, DEF	E	3.0	28.3	85 Btuh
6	2, Clear, Metal, DEF	S	6.0	28.3	170 Btuh
7	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
8	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
9	2, Clear, Metal, DEF	W	3.0	28.3	85 Btuh
10	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
11	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
	Window Total		242		6849 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	172	1.6	275 Btuh
2	Frame - Exterior	13.0	1051	3.1	3258 Btuh
	Wall Total		1223		3533 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		20	18.3	367 Btuh
2	Insulated - Exter		20	18.3	367 Btuh
3	Insulated - Adjac		20	9.4	188 Btuh
	Door Total		60		921Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1754	1.3	2280 Btuh
	Ceiling Total		1754		2280Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	196.0 ft(p)	31.6	6194 Btuh
	Floor Total		196		6194 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	15000(sqft)	200	8597 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			200	8597 Btuh

Totals for Heating	**Subtotal**	**28374 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1419 Btuh**
	Total Btuh Loss	**29793 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Model Home	Project Title: TheSamuelModel	Class 3 Rating
		Registration No. 0
Lake City, FL		Climate: North
		11/7/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Model Home
Lake City, FL

Project Title:
TheSamuelModel

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 11/7/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	2	5.5	45.0	0.0	45.0	22	22	990	Btuh
2	2, Clear, DEF, N, N	NE	14.5	7	10.0	0.0	10.0	22	50	500	Btuh
3	2, Clear, DEF, N, N	N	12	5.25	45.0	0.0	45.0	22	22	990	Btuh
4	2, Clear, DEF, N, N	NW	8.5	7	10.0	0.0	10.0	22	50	500	Btuh
5	2, Clear, DEF, N, N	E	2	1.5	3.0	3.0	0.0	22	72	66	Btuh
6	2, Clear, DEF, N, N	S	9.5	7.5	6.0	6.0	0.0	22	37	132	Btuh
7	2, Clear, DEF, N, N	S	2	5.5	30.0	30.0	0.0	22	37	660	Btuh
8	2, Clear, DEF, N, N	S	2	8	30.0	30.0	0.0	22	37	660	Btuh
9	2, Clear, DEF, N, N	W	1.5	3.5	3.0	0.0	3.0	22	72	216	Btuh
10	2, Clear, DEF, N, N	N	1.5	5.5	30.0	0.0	30.0	22	22	660	Btuh
11	2, Clear, DEF, N, N	E	1.5	5.5	30.0	6.7	23.3	22	72	1825	Btuh
	Window Total				242					7199	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Adjacent	13.0	172.0	1.0	179	Btuh
2	Frame - Exterior	13.0	1051.0	1.7	1829	Btuh
	Wall Total		1223.0		2008	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	20.0	10.1	203	Btuh
2	Insulated - Exter	20.0	10.1	203	Btuh
3	Insulated - Adjac	20.0	10.1	203	Btuh
	Door Total	60.0		608	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1754.0	1.4	2491	Btuh
	Ceiling Total		1754.0		2491	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	196.0 ft(p)	0.0	0	Btuh
	Floor Total		196.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.70	15000	175.4	3472	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			175	3472	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	1200	3000	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL

Project Title:
TheSamuelModel

Class 3 Rating
Registration No. 0
Climate: North

11/7/2005

Totals for Cooling	**Subtotal**	**18778 Btuh**
	Duct gain(using duct multiplier of 0.10)	**1878 Btuh**
	Total sensible gain	**20656 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**6081 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**28117 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corp.
1687 Woodlands Drive
Maumee, OH 43537

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: "Construction Series" 6'8 Outswing Opaque Steel Door w & wo sidelites

APPROVAL DOCUMENT: Drawing No. **S-2110**, titled ""Construction Series" 6-8 Single & Double Out-swing Steel Door", sheets 1 through 8, prepared by RW Building Consultants, Inc., dated 03/28/01 with revision #1 dated 3/12/03, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P. E.**

PRODUCT CONTROL ACCEPTED

NOA No 01-0828.10
Expiration Date: May 1, 2008
Approval Date: May 1, 2003

Therma-Tru Corporation

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED
(For File ONLY. Not part of NOA)

A. DRAWINGS

1. Manufacturer's die drawings and sections.
2. Drawing No **S-2110**, titled ""Construction Series" 6-8 Single & Double Out-swing Steel Door", sheets 1 through 8, dated 6/12/02, with revision #1 dated 3/12/03, prepared by RW Building Consultants, Inc.

B. TESTS

1. Test reports on 1) Air Infiltration Test, per SFBC, PA 202-94
   2) Uniform Static Air Pressure Test, Loading per SFBC PA 202-94
   3) Water Resistance Test, per SFBC, PA 202-94
   4) Forced Entry Test, per SFBC 3603.2 (b) and PA 202-94
   5) Large Missile Impact Test per SFBC, PA 201-94
   6) Cyclic Wind Pressure Loading per SFBC, PA 203-94

   along with marked-up drawings and installation diagram of a double opaque door with sidelites, prepared by ETC Laboratories, Test Report No. **ETC-01-741-11004.0**, dated 7/6/01, signed and sealed by Joseph Dolden, P.E.
2. Test reports on 1) Air Infiltration Test, per SFBC, PA 202-94
   2) Uniform Static Air Pressure Test, Loading per SFBC PA 202-94
   3) Water Resistance Test, per SFBC, PA 202-94
   4) Forced Entry Test, per SFBC 3603.2 (b) and PA 202-94
   5) Large Missile Impact Test per SFBC, PA 201-94
   6) Cyclic Wind Pressure Loading per SFBC, PA 203-94

   along with marked-up drawings and installation diagram of a double opaque door with sidelites, prepared by ETC Laboratories, Test Report No. **ETC-01-741-10622.0**, dated 3/23/01, signed and sealed by Joseph Dolden, P.E.

C. CALCULATIONS

1. Anchor Calculations and structural analysis, prepared by Lyndon Schmidt, P.E., dated 8/18/01, signed and sealed by Lyndon Schmidt, P.E.

D. MATERIAL CERTIFICATIONS

1. Notice of Acceptance No. **01-1120.08** issued to Therma-Tru Corporation for "Therma-Tru Series "BTS, TCM, PVC, SMC" Lite Frame" dated 1/18/02, expiring on 1/18/07.
2. Tensile Test of Constructon Series Steel Door Skin, prepared by ETC Laboratories, Test Report No. ETC-01-741-10622.0, dated 3/28/01, signed and sealed by Joseph Dolden, P.E., Tensile Test of Non Foam Plastic, prepared by ETC Laboratories, Test Report No. ETC-01-741-11075.0, dated 3/2/01, signed and sealed by Joseph Dolden, P.E., Surface Burning Characteristics of Building Materials, **ASTM E84-00a,** prepared by Omega Point Laboratories, Test Report No. 15427-107362, dated 8/28/00, signed and sealed by John Dougherty, P.E.

Manuel Perez, P.E.
Product Control Examiner
NOA No 01-0828.10
Expiration Date: May 1, 2008
Approval Date: May 1, 2003

Therma-Tru Corporation

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED
(For File ONLY. Not part of NOA)

E. STATEMENTS

1. Statement letter of conformance, dated 8/23/01, signed and sealed by Lyndon Schmidt, P.E.
2. Statement letter of no financial interest, dated 8/18/01, signed and sealed by Lyndon Schmidt, P.E.
3. Statement letter naming Mr. Rick Wright as their representative and contact person, signed by Steve Kepler

F. OTHER

1. Letter for San Martin Associates, Inc.

Manuel Perez, P. E.
Product Control Examiner
NOA No 01-0828.10
Expiration Date: May 1, 2008
Approval Date: May 1, 2003

THERMA-TRU®

"CONSTRUCTION SERIES" OUTSWING 6-8 SINGLE AND DOUBLE W/& W/OUT SIDELITES. INSULATED STEEL DOOR WITH WOOD FRAMES.

GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO COMPLY WITH THE FLORIDA BUILDING CODE
2. WOOD BUCKS BY OTHERS, MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. DESIGNED PRESSURE RATING SEE TABLE PAGE 1.
5. THIS PRODUCT MEETS THE WATER REQUIREMENTS FOR "HIGH VELOCITY HURRICANE ZONES" AREA WITH THE USE OF THE HIGH DAM BUMP THRESHOLD.
6. WHEN THIS PRODUCT IS USED IN AREAS REQUIRING WINDBORNE DEBRIS PROTECTION, FLORIDA BUILDING CODE APPROVED IMPACT RESISTANT SHUTTERS ARE REQUIRED FOR THE SIDELITES ONLY.
7. SIDELITES ARE AN OPTION AND CAN BE USED IN A SINGLE OR DOUBLE CONFIGURATION.

INSULATED STEEL DOOR

(Common to all frame conditions)

Door & Sidelite Panel Construction:

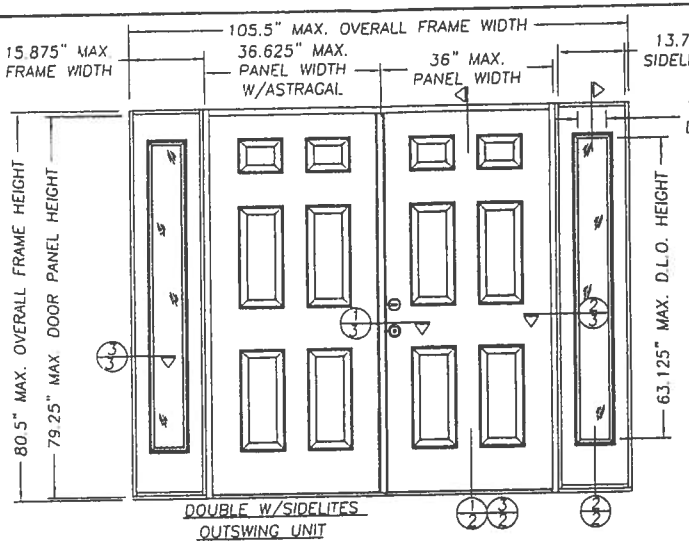
Face sheets: Door Panel is 25 ga.(0.018") minimum thickness Sidelite Panel face sheets are 24 ga.(0.021") minimum thickness Galvanized steel A-525 commercial quality - AKDQ per ASTM 620 with yield strength Fy(min.)=27,830 psi

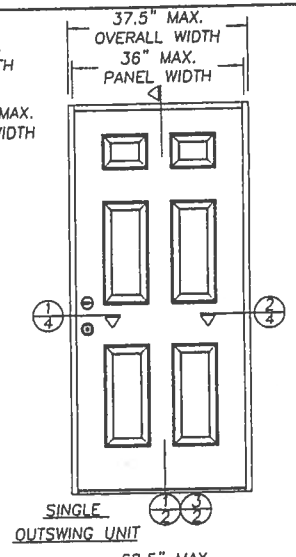
Core design: Polyurethane foam core, with 1.9 lbs density by BASF.

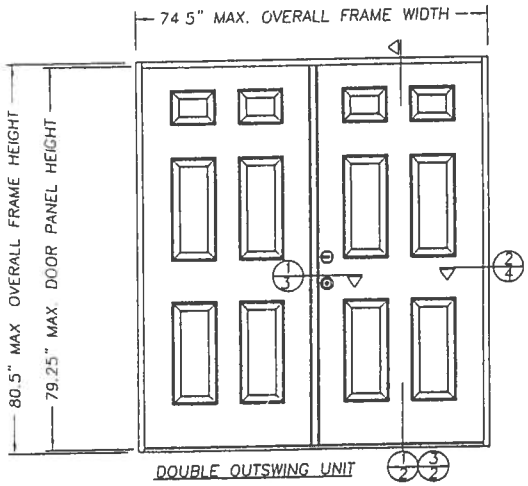
Door Panel Construction: Flush or embossed type. The vertical edges of the skin, rolled formed to provide a mechanical interlock with finger jointed pine stiles. Wood end rails are butt jointed and pressure fitted with contact cement to the wood stiles at the corners.

Sidelite Panel Construction and Glazing: The vertical edges of the skin are rolled formed to provide a mechanical interlock with finger jointed pine stiles. The end rails are butt joined to the wood stiles at the corners. The sidelite panels are sandwich glazed using a two piece lite frame.

Frame Construction: The frame is constructed from finger jointed Ponderosa Pine measuring 4.656" wide x 1.25" thick. The header is joined to the side jambs with (3) 16ga. 1/2" crown x 2" long staples at each side. The threshold is joined to the side jambs with (2) 16ga. 1/2" crown x 2.5" long staples at each side. The mullions are secured together in a sidelite application using #8 x 2 1/2" long PFH Wood Screws (6) screws per each mullion. The unit uses an Outswing Bumpface threshold, either Low Profile or High Water Dam







ALL MODELS ARE VIEWED FROM INTERIOR

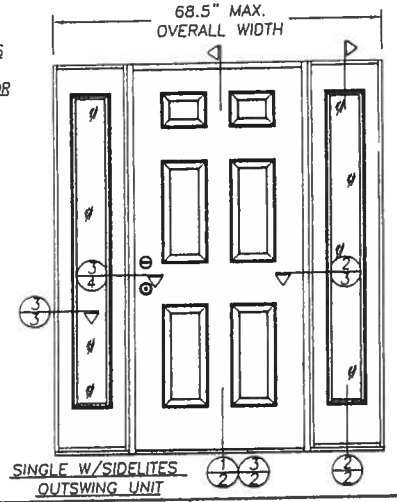


TABLE OF CONTENTS

DESIGN PRESSURE RATING

UNIT TYPE	W/ HIGH DAM THRESHOLD	W/ LOW PROFILE THRESHOLD
SINGLE	+ 67.0 PSF - 67.0 PSF	+ 55.0 PSF - 67.0 PSF
SINGLE W/SIDELITES	+ 60.0 PSF - 60.0 PSF	+ 55.0 PSF - 60.0 PSF
DOUBLE	+ 60.0 PSF - 60.0 PSF	NOT APPROVED
DOUBLE W/SIDELITES	+ 60.0 PSF - 60.0 PSF	NOT APPROVED

Approved as complying with the Florida Building Code
Date MAY 1, 2003
NOA# 01-0828.10
Miami Dade Product Control Division
By Manuel Perez

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO 43537
PHONE 800.537.8827

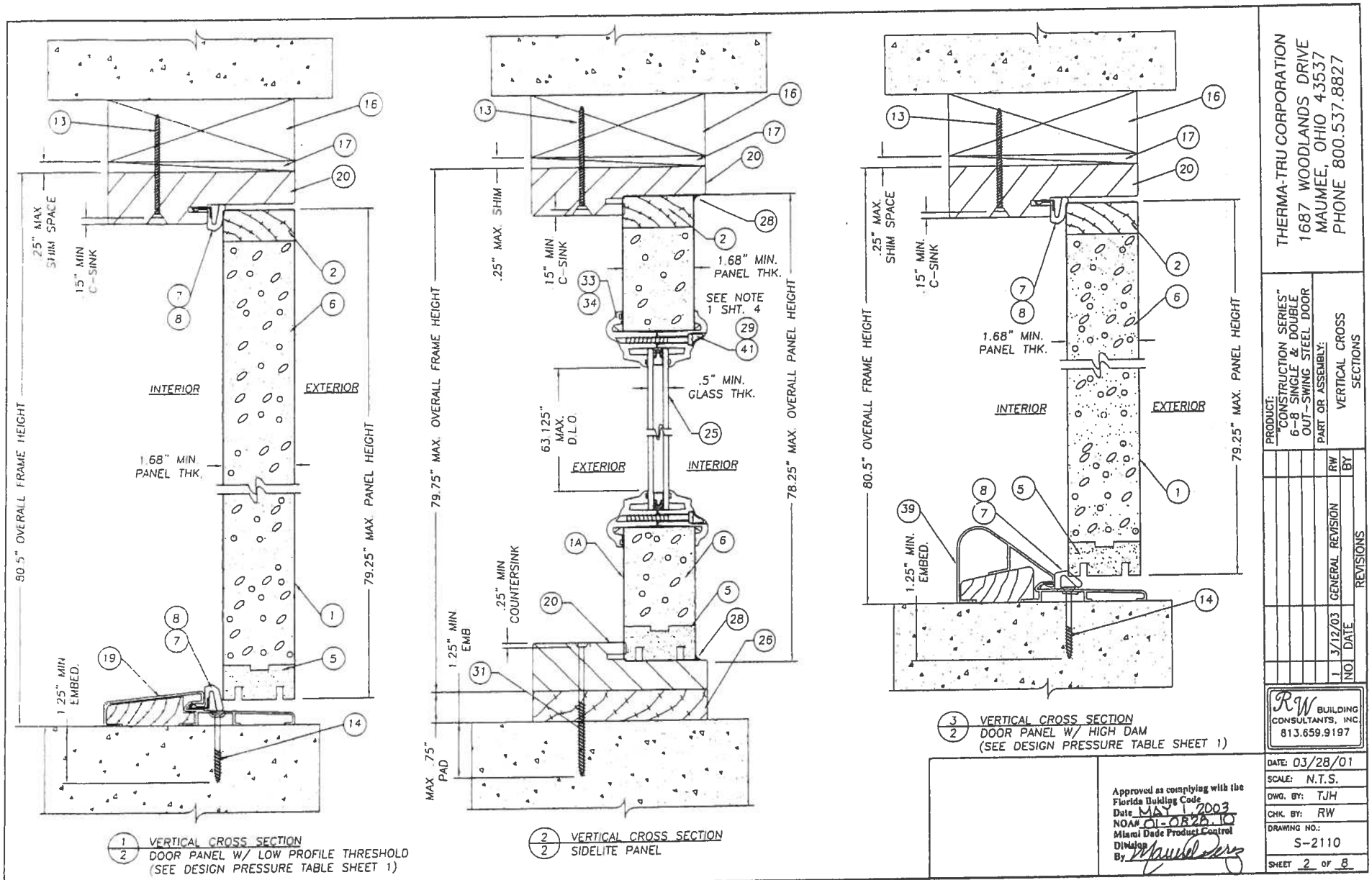
PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR

PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

NO.	DATE	REVISIONS	BY
1	3/12/03	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC.
813.659.9197

DATE: 03/28/01
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2110
SHEET 1 OF 8



1/2 VERTICAL CROSS SECTION
DOOR PANEL W/ LOW PROFILE THRESHOLD
(SEE DESIGN PRESSURE TABLE SHEET 1)

2/2 VERTICAL CROSS SECTION
SIDELITE PANEL

3/2 VERTICAL CROSS SECTION
DOOR PANEL W/ HIGH DAM
(SEE DESIGN PRESSURE TABLE SHEET 1)

Approved as complying with the Florida Building Code
Date MAY 1, 2003
NOA# 01-0828.10
Miami Dade Product Control Division
By

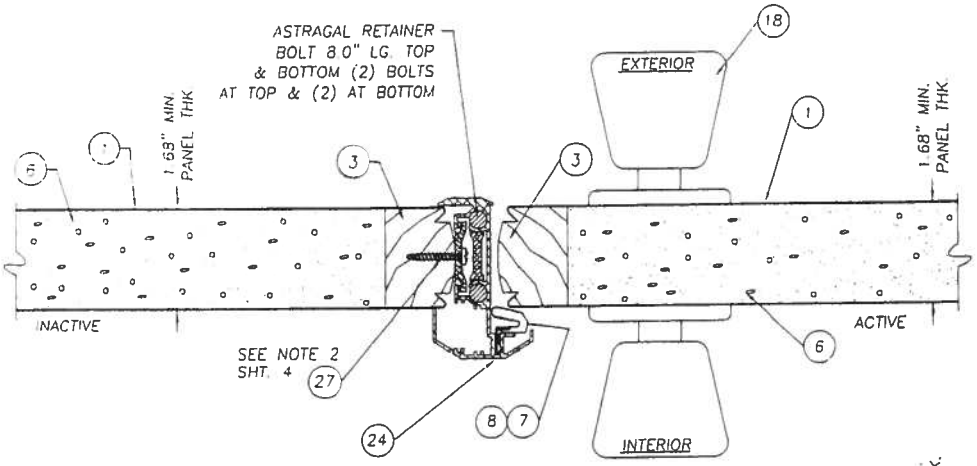
THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO 43537
PHONE 800.537.8827

PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR
PART OR ASSEMBLY: VERTICAL CROSS SECTIONS

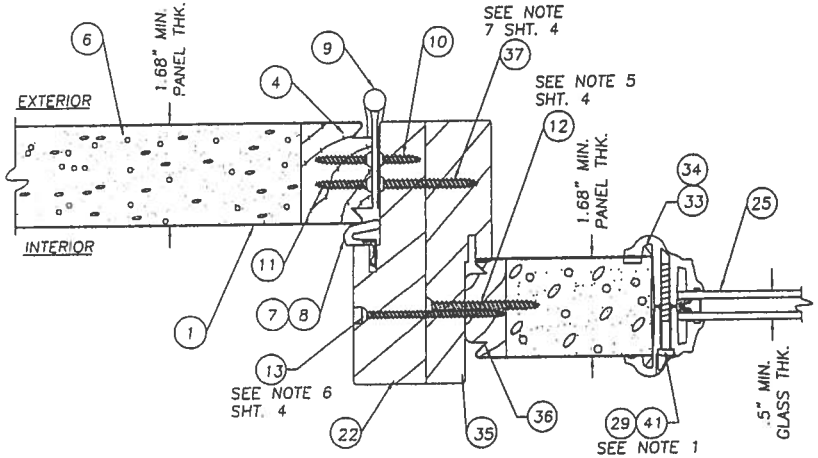
NO.	DATE	REVISIONS	BY
1	3/12/03	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC
813.659.9197

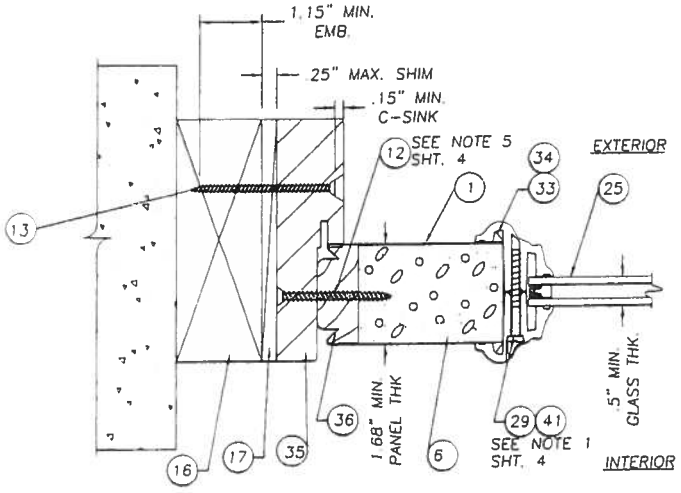
DATE: 03/28/01
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2110
SHEET 2 OF 8



1/3 HORIZONTAL CROSS SECTION @ ASTRAGAL



2/3 HORIZONTAL CROSS SECTION @ HINGE JAMB TO SIDELITE



3/3 HORIZONTAL CROSS SECTION @ SIDELITE TO BUCK

Approved as complying with the Florida Building Code
Date MAY 1, 2003
NOA# 01-0828.10
Miami Dade Product Control Division
By:

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO 43537
PHONE 800.537.8827

PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR

PART OR ASSEMBLY: HORIZONTAL CROSS SECTIONS

NO	DATE	REVISIONS	BY
1	3/12/03	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC.
813.659.9197

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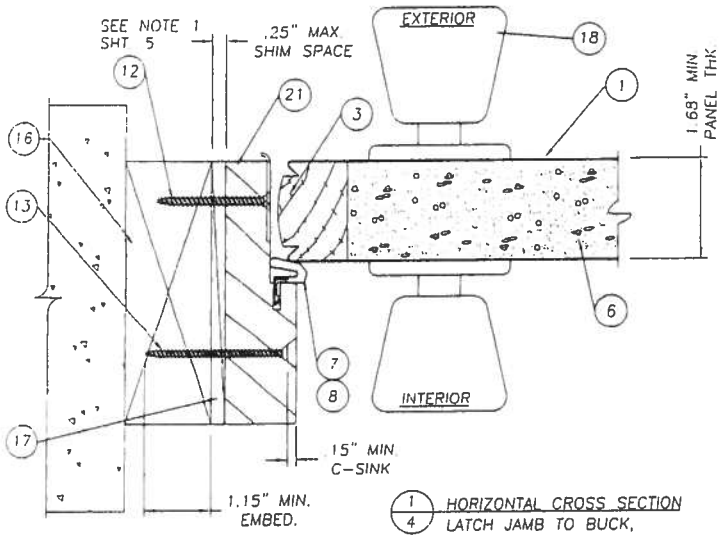
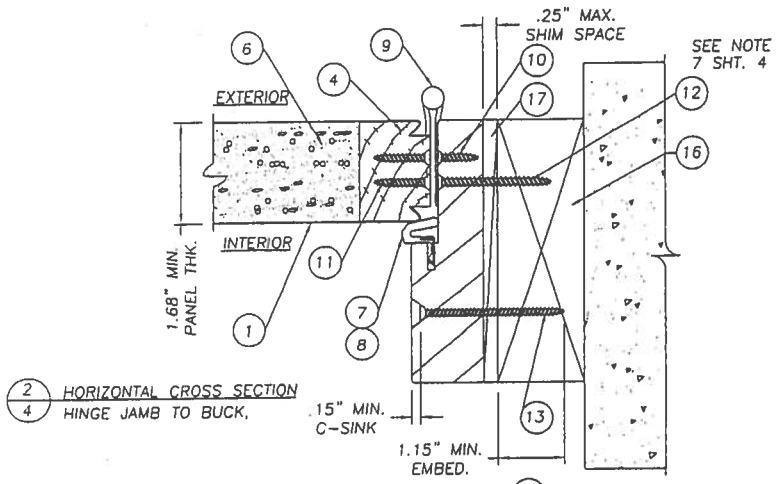
SHEET 3 OF 8



1/4 HORIZONTAL CROSS SECTION
LATCH JAMB TO BUCK,



2/4 HORIZONTAL CROSS SECTION
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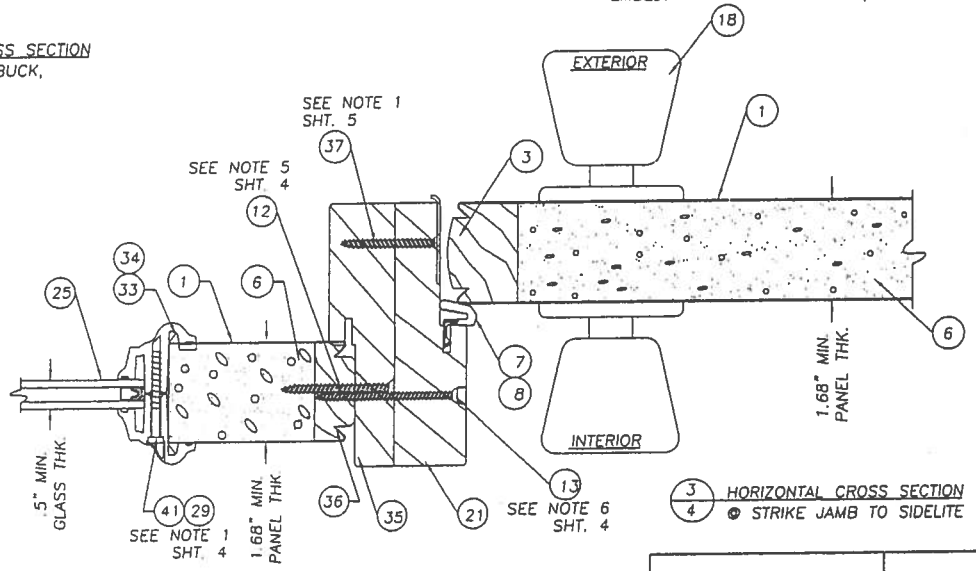

3/4 HORIZONTAL CROSS SECTION
© STRIKE JAMB TO SIDELITE

NOTES:

1. SPACING FOR ITEM #29 & #41 (LITE FRAME SCREWS) IS AS FOLLOWS: FROM THE TOP DOWN ON SIDES, 3", 14.75", 26.5", 38.25", 50.5" & 62.25". THERE IS (1) SCREW BOTH TOP AND BOTTOM AT 4.25" IN FROM CORNER.
2. SPACING FOR ITEM #27 #8 x 1" PANHEAD SCREW ATTACHING THE ASTRAGAL TO THE INACTIVE DOOR IS AS FOLLOWS: FROM THE TOP DOWN 1", 3", 5", 18.25", 40.5", 59.25", 74.25", 76.25" AND 78.25".
3. THE HEAD JAMB IS ATTACHED TO THE SIDE JAMBS WITH (3) 16GA. x 1/2" CROWN x 2" STAPLES AT BOTH SIDE.
4. THE THRESHOLD IS ATTACHED TO THE SIDE JAMBS WITH (2) 16GA. x 1/2" CROWN x 2.5" STAPLES AT BOTH SIDE.
5. THE SIDELITE IS DIRECT SET INTO THE JAMB WITH ITEM #12 #10 x 2" PH.F.H. WOOD SCREW AT 6" DOWN FROM EACH CORNER AND A MAX. OF 12" O.C. ON THE SIDE JAMBS ONLY.
6. SPACING FOR ITEM #13, A #8 x 2 1/2" SCREW SECURING THE MULLIONS TOGETHER IS THE SAME AS THE PERIMETER ANCHORING SCREWS. 6" DOWN FROM THE TOP AND UP FROM THE BOTTOM WITH (4) MORE SPACED AT 13.7" O.C.
7. WHEN ATTACHING THE HINGE TO THE JAMB AND THE BUCK USE ITEM #12 A #10 x 2" SCREW. WHEN ATTACHING THE HINGE TO THE JAMB AND THE SIDELITE AT THE MULLION USE ITEM #37 A #10 x 1 3/4" SCREW.

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PHONE 800.537.8827

PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR

PART OR ASSEMBLY: HORIZONTAL CROSS SECTIONS & NOTES

REVISIONS

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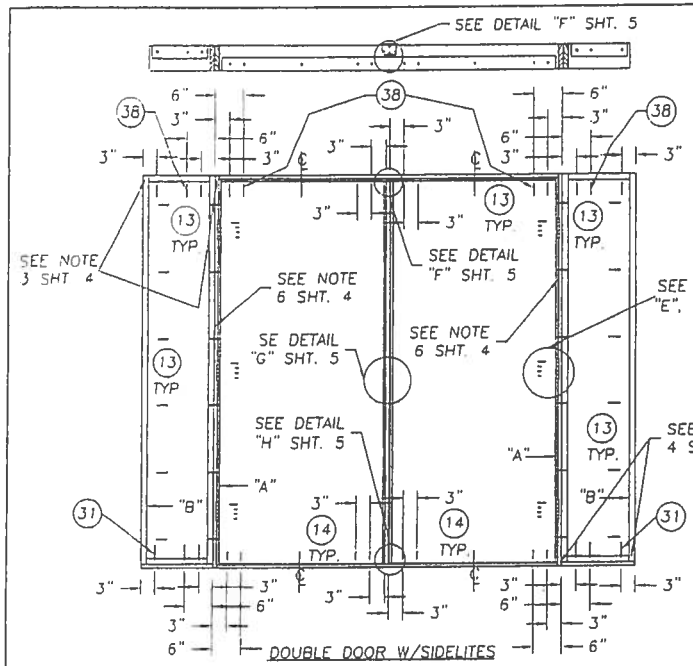
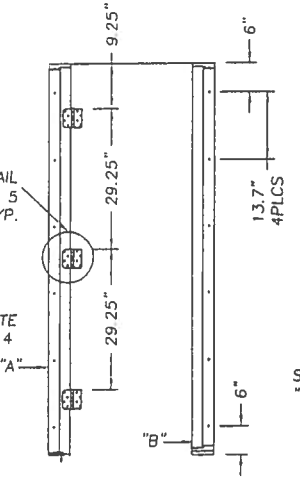
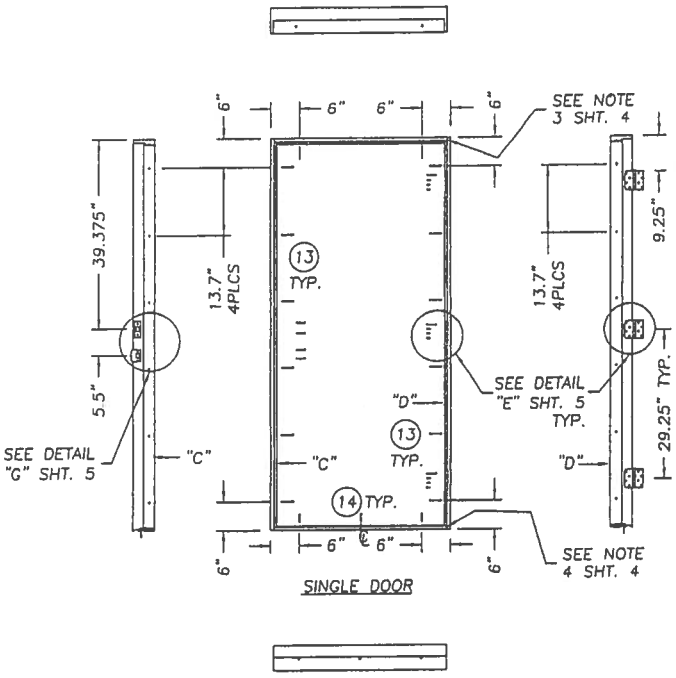
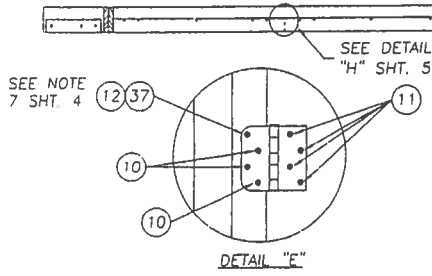
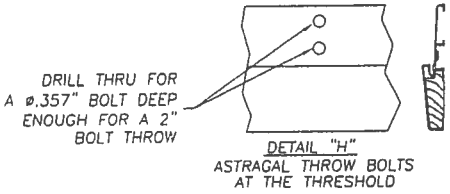
SHEET 4 OF 8

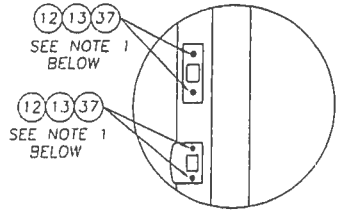


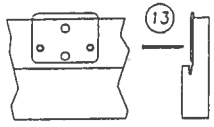












NOTE:
1. WHEN ATTACHING THE STRIKE & DEADBOLT PLATES TO THE STRIKE JAMB AND BUCK USE A #10 x 2" SCREW. WHEN ATTACHING THE STRIKE & DEADBOLT PLATES TO THE ASTRAGAL USE A #8 x 2 1/2' SCREW. WHEN ATTACHING THE STRIKE & DEADBOLT PLATES TO THE STRIKE JAMB AND SIDELITE JAMB USE A #10 x 1 3/4" SCREW.

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PHONE 800.537.8827

PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR
PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

NO	DATE	REVISIONS	BY
1	3/12/03	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC.
813.659.9197

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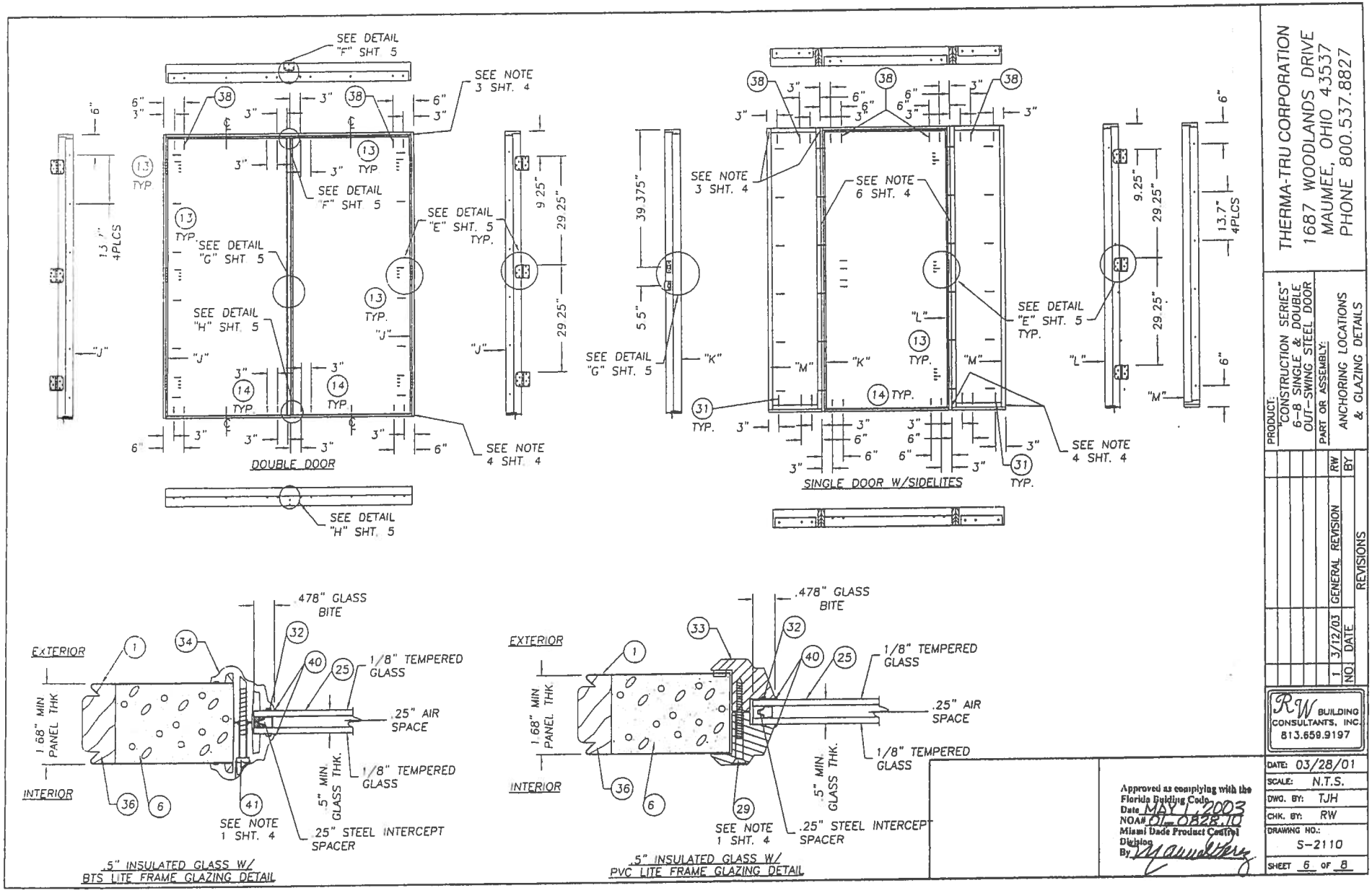
SHEET 5 OF 8



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PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR

PART OR ASSEMBLY: ANCHORING LOCATIONS & GLAZING DETAILS

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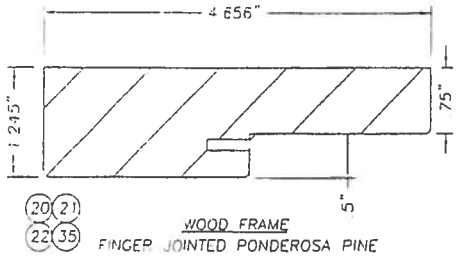
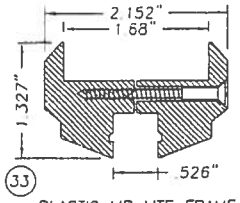
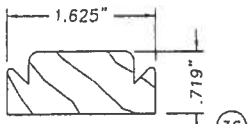


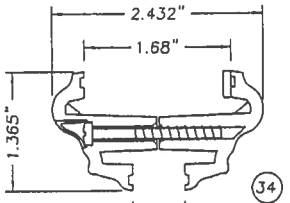
(20) (21) (22) (35) WOOD FRAME
FINGER JOINTED PONDEROSA PINE



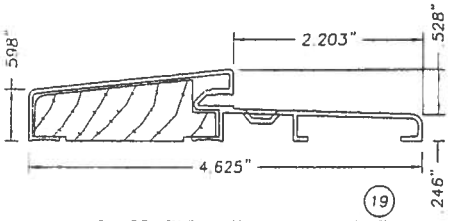
(33) PLASTIC LIP LITE FRAME
EXTRUDED PVC



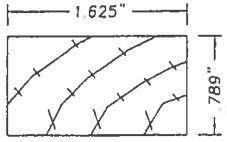
SIDELITE BLANK SIDE STILE (36)
FINGER JOINTED PONDEROSA PINE



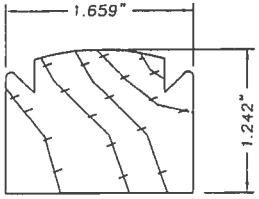
(34) BTS PLASTIC LIP LITE FRAME
EXTRUDED BTS



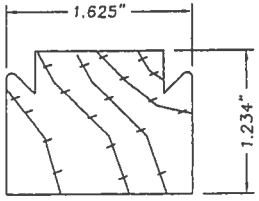
(19) LOW PROFILE OUTSWING BUMP FACE
ALUMINUM/WOOD THRESHOLD
.050" WALL TYP.



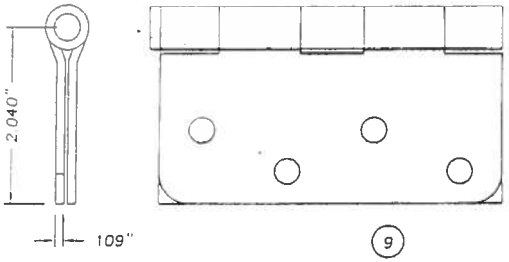
(2) TOP RAIL
FINGER JOINTED PONDEROSA PINE



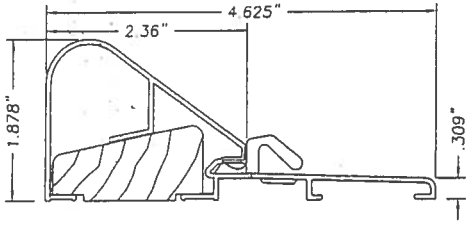
(3) WOOD LATCH SIDE STILE



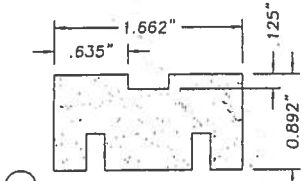
WOOD HINGE SIDE STILE (4)



(9) 4 x 4 STEEL DOOR HINGE



(39) HIGH DAM THRESHOLD
MIN. ALUMINUM WALL THICKNESS 0.050"



(5) BOTTOM RAIL, WOOD COMPOSITE

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MAUMEE, OHIO 43537
PHONE 800.537.8827

PRODUCT: "CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUTS-WING STEEL DOOR
PART OR ASSEMBLY: UNIT COMPONENTS

NO	DATE	REVISIONS	BY
1	3/12/03	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC.
813.659.9197

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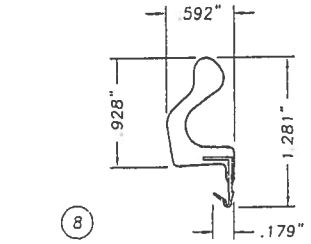
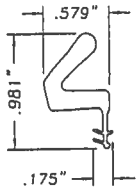
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2110
SHEET 7 OF 8

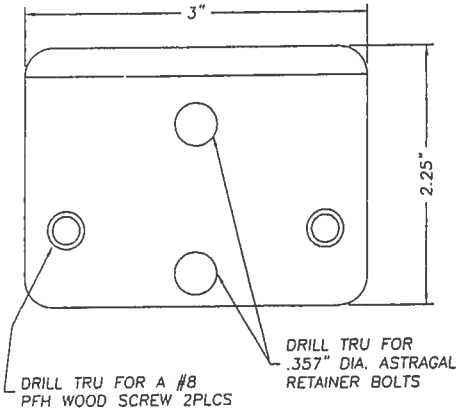
Item	DESCRIPTION	Material
1	DOOR SKIN, CONSTRUCTION SERIES 25GA (.018" MIN.)	STEEL
1A	SIDELITE SKIN, 24 GA. (.021" MIN.)	STEEL
2	TOP RAIL (1.628" x .851" THERMA-TRU PONDEROSA PINE)	WOOD
3	LATCH STILE (THERMA-TRU, PONDEROSA PINE 1.659" x 1.242")	WOOD
4	HINGE STILE (THERMA-TRU, PONDEROSA PINE 1.625" x 1.234")	WOOD
5	BOTTOM RAIL (1.662" x .892" THERMA-TRU WOOD COMPOSITE)	WOOD COMPOSITE
6	POLYURETHANE FOAM (BASF, 1.9lbs. DENSITY)	FOAM
7	SHORT REACH COMPRESSION WEATHERSTRIP (THREMA-TRU)	FOAM
8	LONG REACH COMPRESSION WEATHERSTRIP (THREMA-TRU)	FOAM
9	4" x 4" HINGE .097" THK. (THERMA-TRU)	STEEL
10	#10 x 3/4" lg. PFH WOOD SCREW (Hinge to Frame)	STEEL
11	#10 x 1" LG. PFH WOOD SCREW	STEEL
12	#10 x 2" LG. PFH WOOD SCREW	STEEL
13	#8 x 2 1/2" LG. PFH WOOD SCREW	STEEL
14	3/16" TAPCON ANCHOR (ELCO, 1.75" MIN. LG.)	STEEL
15	NOT USED	–
16	2x WOOD BUCK	WOOD
17	MAX. 1/4" SHIM MATERIAL	WOOD
18	KWIKSET TITAN 700 SERIES PASSAGE LOCK	–
19	ONE PIECE BUMP FACE THRESHOLD LOW PROFILE (THERMA-TRU)	ALUM./WOOD
20	4.656" HEADER (THERMA-TRU, PONDEROSA PINE)	WOOD
21	4.656" STRIKE JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
22	4.656" HINGE JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
23	KWIKSET TITAN 700 SERIES DEADBOLT	–
24	ASTRAGAL WINDJAMBER II WR68T (.052" WALL)	ALUM.
25	GLAZING, 1/2" INSULATED TEMPERED GLASS	GLASS
26	3/4" THK. PRESSURE TREATED SIDELITE PAD	WOOD
27	#8 x 1" LG. PANHEAD SHEET METAL SCREW	STEEL
28	CAULKING	LATEX
29	#6-18 x 1 3/4" PHILLIPS FLATHEAD SCREW (FOR ITEM #33)	STEEL
30	NOT USED	–
31	3/16" TAPCON ANCHOR (ELCO, 3.25" MIN. LG.)	STEEL
32	1/8 THK. CELLULAR GLAZING TAPE (STIK-II TAPE)	–
33	PLASTIC LIP LITE FRAME (PVC, THERMA-TRU)	PVC
34	PLASTIC LIP LITE FRAME (BTS, THERMA-TRU)	BTS
35	4.656" BLANK JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
36	SIDELITE SIDE STILE (THERMA-TRU, 1.625" x .719" PONDEROSA PINE)	WOOD
37	#10 x 1 3/4" LG. PFH WOOD SCREW	STEEL
38	#10 X 3.0" LG PFH WOOD SCREW	STEEL
39	HIGH WATER DAM THRESHOLD (THERMA-TRU)	ALUM.
40	SILICONE CAULK	SILICONE
41	#8-10 x 1 1/2" PLASCREW (FOR ITEM #34)	STEEL



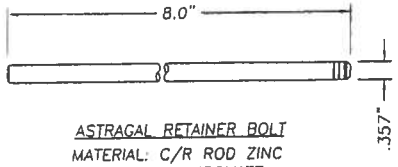
(8) LONG REACH COMPRESSION WEATHERSTRIP
FOAM CELL CORE W/VINYL JACKET



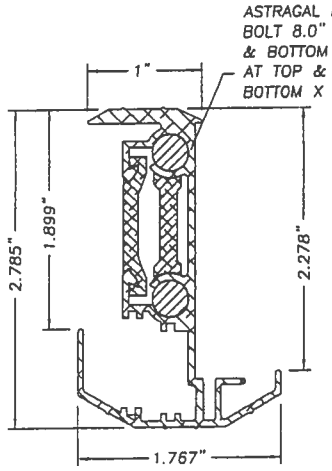
(7) COMPRESSION WEATHERSTRIP
BY THERMA-TRU
FOAM CELL CORE W/VINYL JACKET



WINDJAMBER II WR68T STRIKE PLATE



ASTRAGAL RETAINER BOLT
MATERIAL: C/R ROD ZINC
& YELLOW CHROMATE



(24) WINDJAMBER II WR68T
ASTRAGAL (ALUMINUM .052" WALL TYP.)

Approved as complying with the Florida Building Code
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PHONE 800.537.8827

PRODUCT: CONSTRUCTION SERIES" 6-8 SINGLE & DOUBLE OUT-SWING STEEL DOOR

PART OR ASSEMBLY: BILL OF MATERIALS & UNIT COMPONENTS

NO	DATE	REVISIONS	BY
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SHEET 8 OF 8

MI WINDOWS and DOORS | CAPITOL | BetterBilt | CAPITOL ARCHITECTURAL PRODUCTS

INSTALLATION INSTRUCTIONS FOR ALUMINUM FIN AND FLANGE WINDOWS

FAILURE TO FOLLOW THESE INSTRUCTIONS, AND BUILDING CODES REQUIREMENTS, MAY EFFECT THE REMEDIES AVAILABLE UNDER YOUR WARRANTY

READ THESE INSTRUCTIONS COMPLETELY BEFORE BEGINNING. Please inspect your MI Windows and Doors, Inc. product thoroughly before beginning installation. Inspect the opening and the product, and do not install if there is any observable damage or other irregularity. The product specification sheet and warranty include important information regarding your product and may include product-specific installation requirements (for example, types of fasteners to be used with impact resistant windows and limitations on the height at which the product may be installed); if you did not obtain copies please contact MI Windows and Doors, Inc. Local building codes may impose additional requirements, and those codes supercede these instructions.

1. IF THE HOUSE HAS A WEATHER RESISTANT BARRIER (WRB) I E. HOUSE WRAP, PREPARE THE OPENING ACCORDING TO MANUFACTURER'S INSTRUCTIONS. **BE SURE TO CUT & FOLD BACK THE WRB AT THE TOP AND SIDES SO THAT THE TOP AND SIDE NAILING FINS OF THE UNIT CAN BE INSTALLED UNDERNEATH IT. FLASHING WINDOWS IS RECOMMENDED AND MAY BE REQUIRED BY SOME BUILDING CODES, FLASHING MUST MEET ASTM D-779, 24 HOUR WATER RESISTANT TEST. APPLY THE HORIZONTAL SILL FLASHING BEFORE INSTALLING THE WINDOW AT THE BOTTOM OF THE ROUGH OPENING EXTENDING BEYOND EACH END. (SEE FIGURE 1 BELOW)**

2. MAKE SURE THE ROUGH OPENING IS PLUMB, SQUARE AND THE SILL PLATE IS LEVEL. ROUGH OPENINGS SHOULD BE 1/2" LARGER THAN NET WINDOW SIZE IN WIDTH & HEIGHT. (SEE FIGURE 2 BELOW)

3. CLOSE & LOCK THE SASH THROUGHOUT INSTALLATION, KEEP THE SIDE JAMBS PLUMB & SQUARE WITH HEAD AND SILL. BE CAREFUL NOT TO "CROWN UP" OR "BOW DOWN" THE HEAD OR SILL. CONSTANTLY CHECK WIDTH AT THE MEETING RAILS OF SINGLE AND DOUBLE HUNGS TO AVOID A "BOWED OUT" INSTALLATION. WHEN USING FLASHING APPLY THE BOTTOM PIECE BEFORE INSTALLING THE WINDOWS. FLASHING MUST MEET ASTM D-799, 24 HOUR WATER RESISTANCE TEST.

4. **APPLY A CONTINUOUS 3/8" BEAD OF PREMIUM GRADE, COMPATIBLE EXTERIOR SEALANT TO THE INTERIOR (BACKSIDE) OF THE NAIL FIN OR FLANGE NEAR THE OUTSIDE EDGE ON ALL SIDES PRIOR TO SETTING THE WINDOW INTO THE ROUGH OPENING. (SEE FIGURE 3 BELOW)**

5. SET AND CENTER THE WINDOW INTO THE OPENING. INSERT 1/4" SHIMS UNDER THE BOTTOM CORNERS **(DO NOT PLACE SHIMS OR BLOCKS UNDER THE SILL EXCEPT AT THE CORNERS).** (SEE FIGURE 4 BELOW) MULLED WINDOWS, SLIDERS AND UNITS WITH INTERMEDIATE JAMBS REQUIRE A SHIM AT EACH MULLION, INTERMEDIATE JAMB OR MEETING RAIL TO INSURE A LEVEL SILL CONDITION. **NOTE: REMOVE ALL SHIMS AFTER INSTALLATION IS COMPLETE, EXCEPT AT THE MEETING RAILS OF SLIDERS. IF ADDITIONAL SHIMS ARE REQUIRED TO MAINTAIN A LEVEL SILL, APPLY SHIMS AS NECESSARY.**

6. TEMPORARILY PLACE A FASTENER THROUGH THE NAIL FIN ON EACH TOP CORNER OF FIN UNITS. ON FLANGE WINDOWS INSTALL TEMPORARY FASTENERS INTO THE HOLES PROVIDED IN THE FRAME AT THE TOP OF JAMBS. PLACE SHIMS AT EACH ANCHOR LOCATION AT THE SIDES AND HEAD. FASTENERS NEED TO BE INSTALLED STRAIGHT AND SUFFICIENT LENGTH TO PENETRATE TO FRAMING BY A MINIMUM OF 1 INCH. CHECK THE SILL FOR LEVEL BY RAISING THE SASH SLIGHTLY, THE SPACE SHOULD BE EQUAL, IF NOT ADJUST ACCORDINGLY, RELOCK SASH. CHECK THE JAMBS FOR PLUMB, THEN MEASURE DIAGONALLY ACROSS THE CORNERS, THESE DIMENSIONS MUST BE THE SAME FOR UNIT TO BE SQUARE. NEXT PLACE FASTENERS NEAR THE BOTTOM CORNERS, AGAIN CHECKING WINDOW FOR LEVEL, PLUMB AND SQUARE. CONTINUE PLACING FASTENERS IN THE NAIL FIN, EVERY 16" ON ALL SIDES OF FIN WINDOWS UNTIL SECURE. AVOID DISTORTING THE FIN. FLANGE UNITS REQUIRE FASTENERS IN ALL HOLES PROVIDED IN THE FRAME, SHIMMING AS NEEDED.

7. PLACE SHIMS AT THE MEETING RAILS/CHECK RAILS AT THE SIDE JAMBS OF FIN UNITS TO PREVENT BOWING, THESE SHIMS SHOULD REMAIN AFTER INSTALLATION. CAUTION SHOULD BE TAKEN AS TO NOT OVER SHIM AND CAUSE DEFLECTION OF THE FRAME AND HINDER SASH OPERATION. CHECK THE WIDTH OF THE WINDOW AT THE TOP, MIDDLE AND BOTTOM, IF NOT THE SAME, SHIM ACCORDINGLY. UNLOCK AND OPERATE THE SASH, TILT IT IN AND VISUALLY INSPECT ALL SIGHT LINES.

8. CAULK OVER EXPOSED FASTENER HEADS ON THE NAIL FIN. ALSO CAULK OUTSIDE PERIMETER OF NAIL FIN AND FLANGE, OR IF USING FLASHING APPLY THE SIDE JAMB PIECES OVER LAPPING THE SILL FLASHING. NEXT APPLY THE TOP (HEAD) PIECE OVERLAPPING THE JAMB FLASHING. LASTLY, UNFOLD THE WRB AT THE SIDES AND HEAD COVERING THE FLASHING. TAPE THE SEAMS AND SEAL THE SIDE OF THE WINDOW ONLY, ACCORDING THE THE HOUSE WRAP MANUFACTURERS INSTRUCTIONS. (SEE FIGURE 5 BELOW)

9. INSULATE BETWEEN THE WINDOW FRAME & ROUGH OPENING WITH FIBERGLASS INSULATION OR EQUAL. THE SPACE MAY BE FILLED WITH MEASURED USE OF LOW EXPANSION FOAM BUT ONLY AFTER DETERMINING THAT FOAM WILL NOT EXERT PRESSURE AGAINST THE FRAME, WHICH CAN IMPAIR OPERATION. DISTORTION OF THE FRAME WILL AFFECT THE USER S RIGHTS UNDER THE WARRANTY.

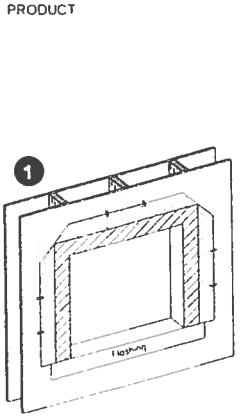
11. ALLOW A 1/4" GAP BETWEEN THE EXTERIOR CLADDING, SIDING BRICK, STUCCO OR STONE AND THE WINDOW FRAME ON ALL SIDES, EXCEPT VINYL J-CHANNEL. THE GAP (EXPANSION JOINT) SHOULD BE FILLED WITH CORRECT SIZE BACKER ROD. THEN SEALED WITH A HIGH GRADE EXTERIOR SEALANT AND WILL NEED TO BE MAINTAINED.

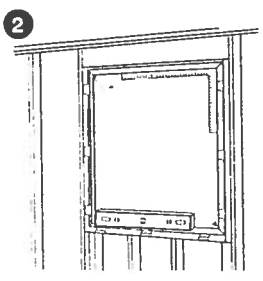
CAUTION:

- USE OF SOLVENTS OR ACIDS WILL DAMAGE COMPONENTS OF THIS PRODUCT AND WILL LIMIT RIGHTS UNDER WARRANTY.
- FIN WINDOWS SHOULD BE FASTENED THROUGH THE FIN ONLY-FLANGE WINDOWS ANCHORED ONLY THROUGH THE PROVIDED HOLES IN THE FRAME. FASTENING IN ANY OTHER PORTION MAY PERMANENTLY DAMAGE UNIT WHICH WILL LIMIT RIGHTS UNDER THE WARRANTY.
- IT IS THE RESPONSIBILITY OF THE OWNER, ARCHITECT, OR BUILDER TO SELECT CORRECT PRODUCTS TO BE IN COMPLIANCE WITH APPLICABLE LAWS AND BUILDING CODES.
- DO NOT STORE IN THE SUN OR LAY FLAT BEFORE OR DURING INSTALLATION.
- ANY PENETRATIONS (e g. ALARM SENSORS) MADE THROUGH ANY PORTION OF ANY M I., BETTERBILT OR CAPITOL PRODUCT MAY AFFECT RIGHTS UNDER THE MANUFACTURER'S WARRANTY.
- SOME LAWS AND BUILDING CODES REQUIRE SAFETY GLASS TO BE USED NEAR DOORS AND/OR FLOORS. UNLESS SPECIFICALLY ORDERED, THE MANUFACTURER'S NEW CONSTRUCTION WINDOWS ARE NOT MADE WITH SAFETY GLASS, AND, IF BROKEN, THE GLASS MAY SHATTER AND CAUSE INJURY.

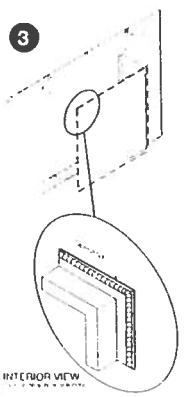
THESE INSTRUCTIONS ARE MINIMUM REQUIREMENTS ONLY. CHECK STATE AND LOCAL CODE RESTRICTIONS FOR ADDITIONAL COMPLIANCE ON INSTALLATION AND OR FASTENING. IF UNIT HAS EXTERIOR TRIM (BRICK MOLD/J CHANNEL, ETC.) THE UNIT MUST BE SEALED BEHIND THE NAIL FIN, THE TRIM IS PROVIDED FOR AESTHETIC PURPOSES ONLY. INSTALLATION INTO MASONRY OR REPLACEMENT OPENINGS MUST BE SEALED TO THE OPENINGS USING AN APPROVED, PROPER METHOD. REFER TO AAMA 2400 AND/OR ASTM E2112 STANDARDS.

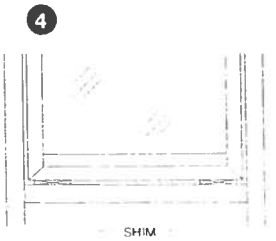
THESE INSTALLATION INSTRUCTIONS ARE PROVIDED FOR INFORMATION ONLY, NO REPRESENTATION AND WARRANTY IS MADE THAT THESE INSTRUCTIONS SET FORTH ALL OF THE INFORMATION NECESSARY FOR PROPER INSTALLATION OF THE PRODUCT. GIVEN THE VARIETY OF FIELD CONDITIONS, PRIMARY RESPONSIBILITY FOR PRODUCT INSTALLATION RESTS WITH THE INSTALLER. DO NOT PROCEED UNLESS YOU HAVE ADDRESSED THE FACTORS NECESSARY TO ACHIEVE WEATHER-TIGHT INSTALLATION OF A PROPERLY FUNCTIONING PRODUCT. MI WINDOWS AND DOORS, INC. ASSUMES NO LIABILITY FOR ANY PERSONAL INJURY OR PROPERTY DAMAGE INCURRED IN INSTALLATION. THESE INSTRUCTIONS, TOGETHER WITH THE PRODUCT SPECIFICATIONS AND WARRANTY SET FORTH THE ENTIRE LIABILITY OF MI WINDOWS AND DOORS, INC. WITH REGARD TO THE PRODUCT.

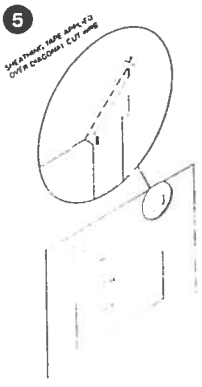




Check for level, plumb and square

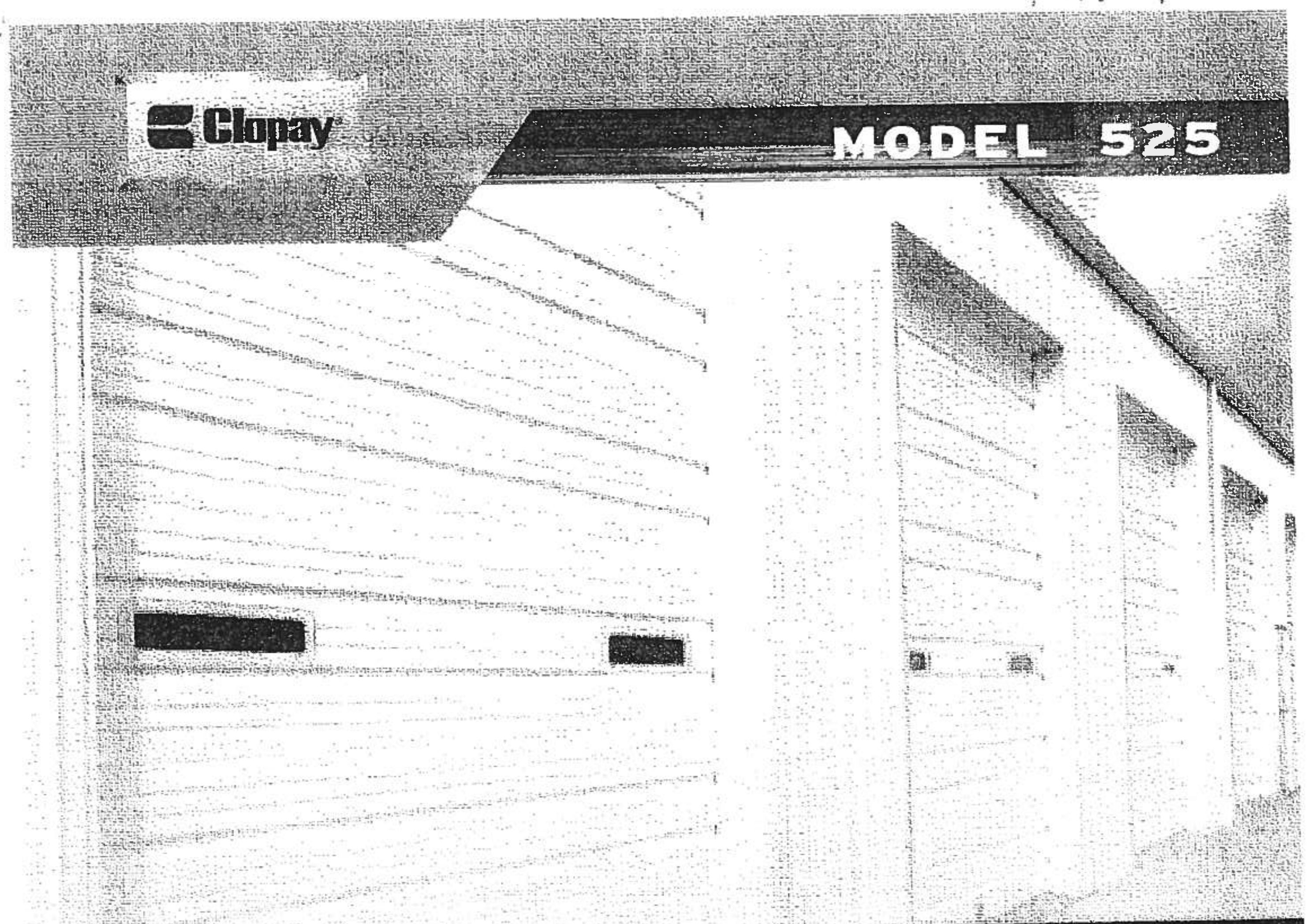






Revised 9 2005

TYPE 1



FEATURES

Model 525-Standard and Model 525S/525V-Insulated

Models 525 and 525S/525V are designed for applications requiring economy and reliability in a commercial door. Steel thickness is .019" min. This is equal to the "nominal" 24 gauge designation used by other door manufacturers.

- 2" thick sectional ribbed door
- Steel skins are hot-dipped galvanized coated with an epoxy primer and finished inside and out with a baked-on white or brown polyester top coat
- Available noninsulated (525) or insulated (525S/525V) with environmentally-safe expanded polystyrene molded to the panel
- Durable rustproof Tog-L-Loc™ construction eliminates rivets and welds
- Many optional window and track configurations available

Rustproof

Panels are protected inside and out to [illegible] rust. Hot-dipped galvanized steel is [illegible] primer and [illegible]

Ribbed Panel Design

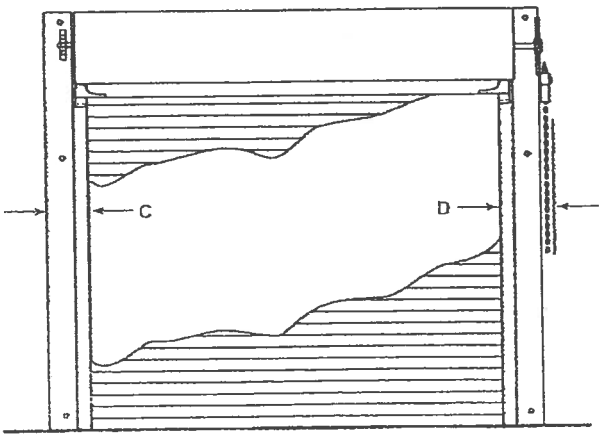
TYPE 2

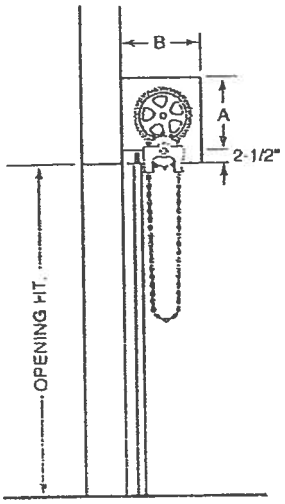
The Cookson Company

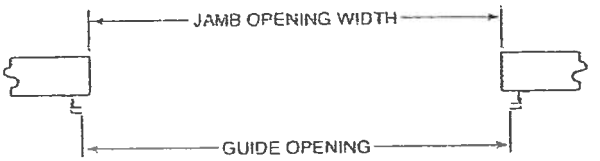
THE PREFERRED ROLLING DOOR

Type FC - Chain Operated Service Door

Gray Prime Finish - Face of Wall Mounted

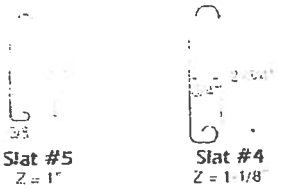






Guide Detail

Slat Selection

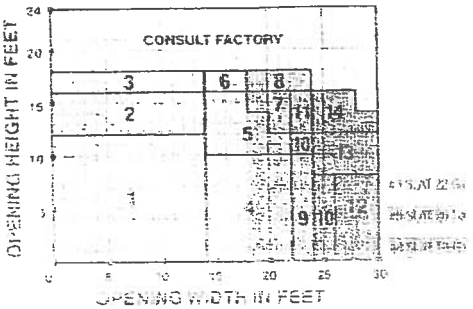


AREA	COMPONENT DIMENSIONS
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	

Bottom Bar Detail

Determine Dimensions For Specific Door Size

1. From chart below select proper Area Number for width and height of door.
2. Refer to Selection Chart for dimensions.



STANDARD DUTY RIBBED STEEL DOORS

[illegible] by Clopay Building Products Company. Each door is provided as one complete unit including sections, brackets, tracks, counterbalance mechanisms, and hardware [illegible] wide x 18'0" high. 320 sq. ft. max.

[illegible] quality (.019" min.)-gauge steel, hot-dipped galvanized per ASTM A-924, A-653, phosphate coated, prepainted with primer and baked-on polyester top coat. Two 1/2" [illegible] section by full 1-3/8". Minimum 18 gauge (.049" min.) end stiles up to 14'2"; over 14'2" [illegible]

Stiles and face panels shall be fastened with a Tog-L-Loc™ joining system. Bottom section reinforced with full length .040" aluminum astragal retainer. Astragal to be U-shaped flexible PVC vinyl. Meeting rails to form weathertight tongue and groove joint. Combination step plate/lift handle provided for the bottom section.

Finish – Exterior of door to be prefinished with a 3-coat process of a baked-on polyester top coat over primer on a phosphate coating. White or brown standard colors. Interior to have baked-on polyester over primer. One full mil exterior and one half mil interior. FINISH GUARANTEED AGAINST RUST-THROUGH FOR A FULL TEN YEARS.

Hardware – All hinges and brackets to be manufactured of hot-dipped galvanized steel, 14 gauge roller hinges, 18 gauge (14 gauge on 525S/525V) center hinges. Ten ball steel rollers to be full floating in case-hardened steel races, mounted to fit the taper of the track.

Tracks – Vertical tracks to be a minimum of 18 gauge galvanized steel tapered and mounted for wedge type closing. Horizontal tracks to be minimum 14 gauge galvanized steel, reinforced with minimum 13 gauge galvanized angles as required. 2" track.

Spring Counterbalance – Door assembly to be operated by a torsion spring counterbalance mechanism, with a helically wound, oil tempered torsion spring mounted on a galvanized steel tube or solid steel shaft as required. Cable drums are die cast aluminum with high strength galvanized aircraft cable with minimum 7 to 1 safety factor.

Locking – Inside spring loaded slide bolt lock on end stile shall engage slot in track.

Wind Loading – Contact factory for specific wind loading requirements.

INSULATION OPTIONS

Insulation Model # with Backer Options	Door Type	Section Backing Material Options	Polystyrene Thickness	Section R-Value	Section U-Value
525V	(.019" min.)	Polyethylene vinyl sheet	1-3/8"	6.66	.15
525S	Gauge Std Ribbed Steel	30 gauge prepainted steel	1-3/8"	6.66	.15

DISTRIBUTED BY:

OPTIONS

A. **Insulation** (See chart below) – Maximize energy efficiency with polystyrene insulation. Polyethylene vinyl (525V) or steel backers (525S) on door interior for [illegible]

B. **Glazing** – 24" X 6" or 24" x 12" window lite with polypropylene screw together [illegible] available for Model 525S/525V. Prepainted full-view section with white or brown finish available with DSB glass and 1/8" Plexiglas. Consult factory for other options.

C. [illegible]

D. **High Cycle Spring** – Available in 25,000, 50,000 and 100,000 cycle

E. [illegible]

G. **Track** – Vertical lift, high lift, follow the roof slope and low headroom track available.

H. **Weatherstripping** – Complete perimeter seals available in various materials.

I. **Installation** – Install door including sections, brackets, guides, tracks, etc. in accordance with final shop drawings (if required) and instructions by Clopay Building Products Company.

PANEL & WINDOW ARRANGEMENT

Door Width	Number of Panels	Max Number of Window Lites
Up to 9'2"	2	2
9'4" to 13'2"	3	3
13'4" to 16'2"	4	4
16'4" to 20' 2"	5	5

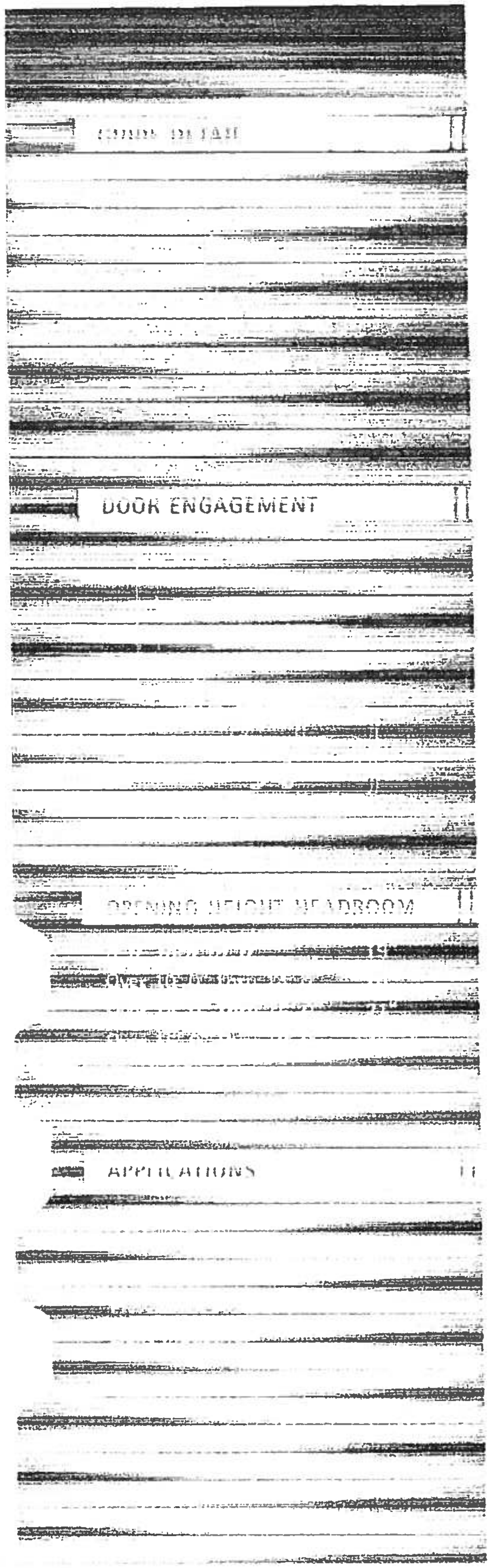
SECTION ARRANGEMENT

Door Height	Number of Sections
Up to 8'0"	4
8'3" to 10'0"	5
10'3" to 12'0"	6
12'3" to 14'0"	7
14'3" to 16'0"	8

For more information on these and other Clopay products, call 1-800-526-4301 or visit our web site at http://www.clopaycommercial.com

Printed in U.S.A. Specifications subject to change without notice.

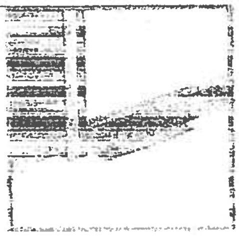
TYPE 3



Guides

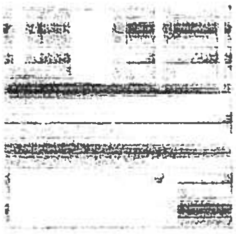
- 12-gauge galvanized steel guide with 12-gauge windlock bar spot welded to guide
- Polyethylene wear strip
- Bolt on head stop 1/4" plate steel
- 3" deep engagement
- Pre punched for lock and attachment screws

Hardware



- 12 gauge saddle clamps
- 10-gauge slide locks 2 per door
- 16 gauge step plate 2 per door
- 1/4" steel angle F bracket

Options



- Chain hoist 2:1 reduction
- Chain hoist 3:1 reduction
- Top draft stop (as pictured)
- Hood top header seal
- Steel mounting plates
- Electric operator

[illegible]

- [illegible]
- 26-gauge grade E steel curtain

2500 (2750 insulated)

- Heavy duty 12-gauge, three-inch deep guides
- [illegible]
- [illegible]

[illegible]

- Aluminum steel angle bottom bar

4000 Series

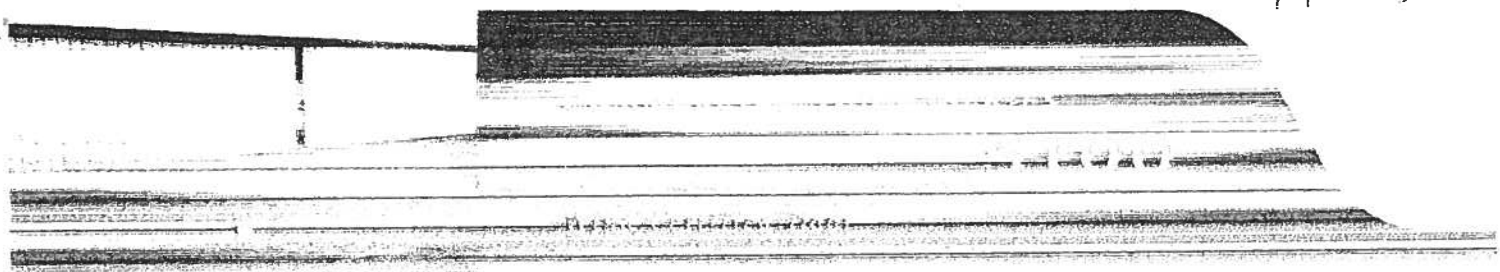
- 24-gauge curtain

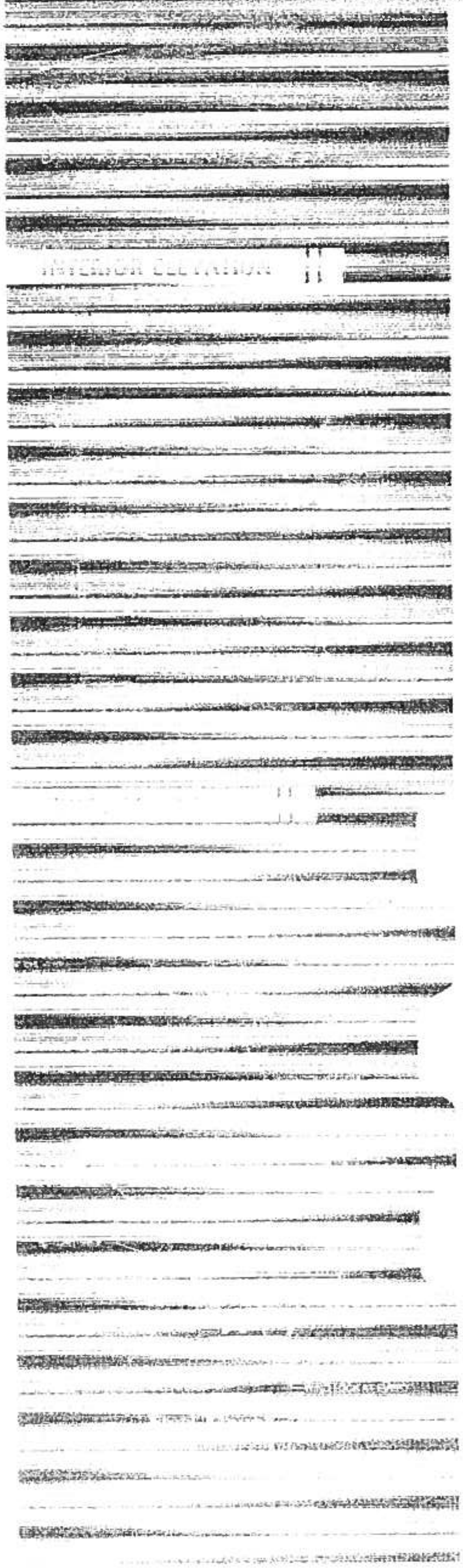
5100 Series

- [illegible]

CONTACT A DBCI SALES REPRESENTATIVE FOR A DETAILED DRAWING.

TYPE 3





General Description

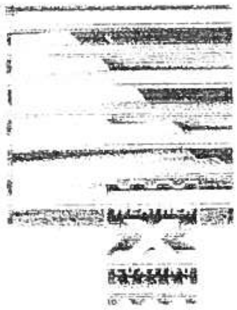
- Designed to meet stringent building codes in Coastal Regions and Florida
- Utilizes 12" drums
- Limited maximum size of 18' x 18'
- Deep engagement guides and windlocks for stronger/better performances

[illegible]

- 10' wide = (150mph)
- 12' wide = (150mph)
- 14' wide = (146mph)
- 16' wide = (140mph)

*For actual test results contact DBCI Commercial Sales at 800 542 0501

Curtain

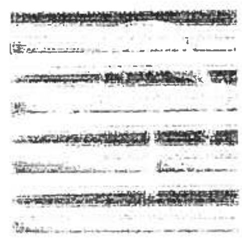


- 26 gauge galvanized
- Grade 5 hard steel
- [illegible]
- [illegible]
- [illegible] polyester [illegible] paint over prime coat
- Wash coat over primer interior side
- [illegible]-year film integrity warranty — up to 25-year chalk and fade
- Improved flexible wear strips

[illegible]

- [illegible] aluminum extrusion
- [illegible] galvanized angles with stiffeners
- [illegible]
- Stainless steel nuts and bolts

[illegible]



- [illegible] steel axle, 14-gauge [illegible]
- [illegible]
- Oil tempered torsion springs
- [illegible]
- [illegible]

TYPE 2

Type FC - Chain Operated Service Door

Gray Prime Finish - Face of Wall Mounted

1.0 GENERAL

1.1 Summary

A. All Rolling Service Doors shall be as manufactured by The Cookson Company, Phoenix, Arizona. Furnished materials shall include curtain, bottom bar, guides, brackets, hoods, operating mechanisms and any special features.

B. Work not to be included by The Cookson Company includes [illegible] not limited to structural or miscellaneous iron work, access [illegible]

1.2 Quality Assurance

A. [illegible] Warlocks shall be installed on doors over 12' wide.

B. All rolling service doors shall be designed to a standard maximum of 25 cycles per day and an overall maximum of 50,000 operating cycles for the life of the door.

2.0 PRODUCTS

2.1 Materials

A. The door curtain shall be constructed of interconnected strip steel slats conforming to ASTM A-653. The proper gauge of steel shall be chosen as follows.

1. 22 gauge with a No. 5 (measuring 2-1/4" high by 5/8" deep) flat slat as designated by The Cookson Company if the door width does not exceed 18'4" and the door height does not exceed 18'4".
2. 20 gauge with a No. 5 flat slat as designated by The Cookson Company if the door width is between 18'5" and 24'4" and the door height does not exceed 18'4".
3. 18 gauge No. 4 (measuring 2-3/4" high by 3/4" deep) flat slat as designated by The Cookson Company if the door width exceeds 24'4" and/or the door height does not exceed 18'4".

B. The finish on the door curtain shall be Cookson FinalCote [illegible]

[illegible] A-653

4. Thermosetting gray polyester top coat with a minimum [illegible] The finish on the curtain slat shall be one (1) coat of bronze rust-inhibiting prime paint.

D. The guides shall consist of 3 steel angles bolted together with 3/8" fasteners to form a channel for the curtain to travel. The wall angle portion shall be continuous and fastened to the surrounding structure with either minimum 1/2" fasteners or welds, both at 36" centers. The finish on the guide angles shall be one (1) coat of bronze rust-inhibiting prime paint.

E. [illegible] fasteners. The finish on the retainers shall be one (1) coat of [illegible]

F. All gears shall be cast iron with teeth cast from machine cut [illegible] equal to not more than six gauges.

G. The barrel shall be steel tubing of not less than 4" in diameter. Oil tempered torsion springs shall be capable of correctly counter balancing the weight of the curtain. The barrel shall be designed to limit the maximum deflection to .03" per foot of opening width. The springs shall be adjusted by means of an exterior wheel. The finish on the barrel shall be one (1) coat of bronze rust-inhibiting prime paint.

H. The hood shall be fabricated from 24 gauge galvanized steel and shall be formed to fit the curvature of the brackets. The finish on the hood shall be the Cookson FinalCote finish as indicated in the curtain section.

2.2 Operation

A. Chain operated doors shall open and close with a maximum of 30 pounds of effort utilizing an endless chain and cast iron reduction gears.

2.3 Locking Mechanisms

A. The chain door shall be secured by means of a chain lock.

3.0 EXECUTION

3.1 Installation

A. [illegible]

3.2 Warranty

[illegible] workmanship and materials.

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 184 Ave. L
City Lake City Phone 752 1703

Site Location: Subdivision Emerald Cove
Lot # 22 Block# _____ Permit # 24234
Address 169 SW TimberLAND CT

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2088	215	4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

5/12/06 — Date
1300 — Time
F254 — Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 33-3S-16-02438-172 **Building permit No.** 000024234

Use Classification SFD,UTILITY **Fire:** 17.76

Permit Holder BRYAN ZECHER **Waste:** 36.75

Owner of Building CORNERSTONE DEVELOPERS **Total:** 54.51

Location: 169 SW TIMBERLAND COURT(EMERALD COVE,LOT 72)

Date: 07/21/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

GARAGE DOORS

REVISIONS		
LETTER	DESCRIPTION	DATE
A	REV PER EN 16130	3/06/98
B	REV PER EN 16141	3/13/98
C	REV PER EN 16152	[illegible]

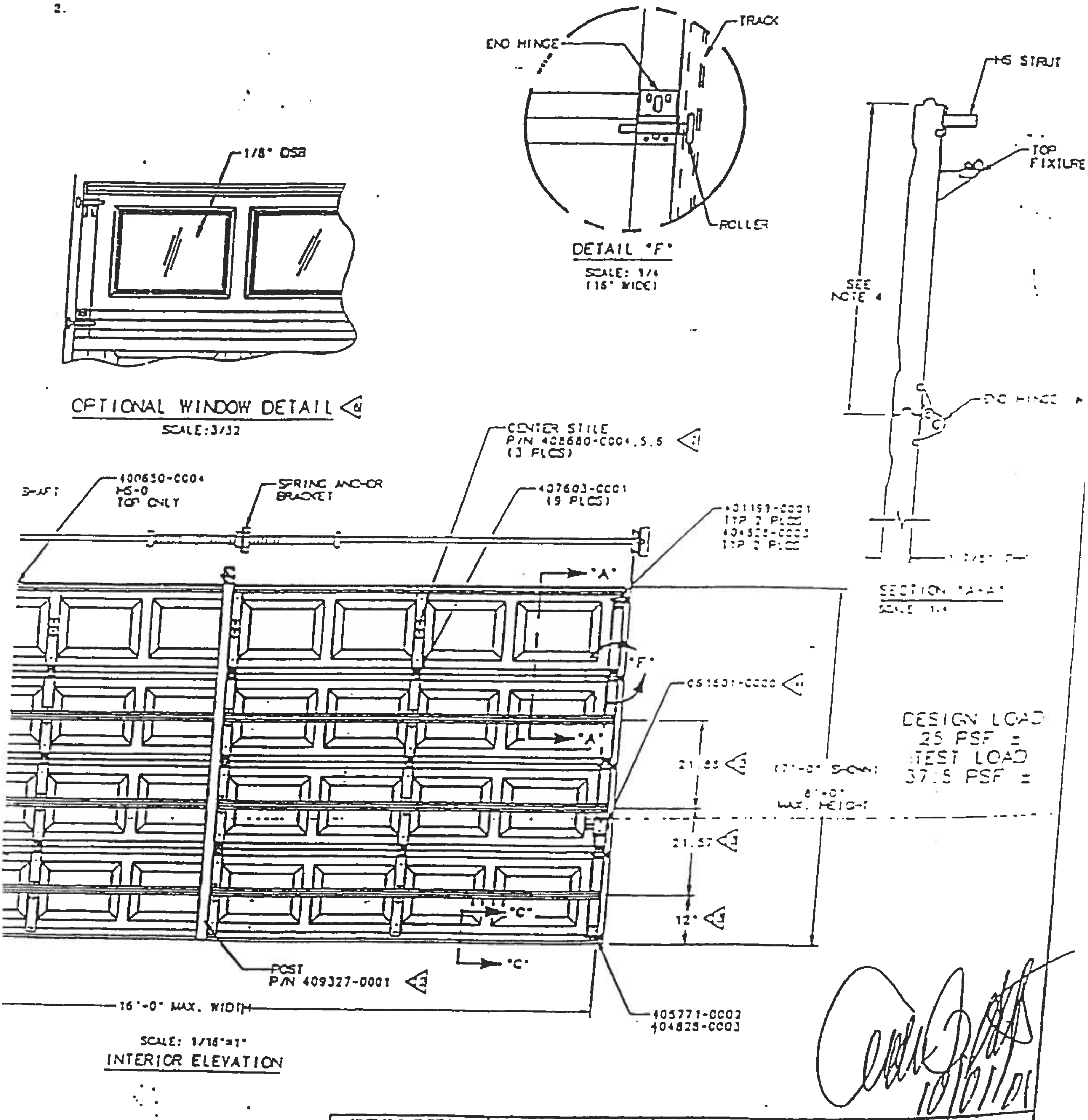
...UCTED WITH 5 OR MORE SECTIONS MUST BE ...STRUTS FOR THE ADDITIONAL INTERMEDIATE ON THE THIRD SECTION.

...L SECTIONS LESS THAN 20.812" MUST BE THE ACTUAL SECTION HEIGHT & 20.812".

... X 5/8" LONG TYPE AB HEX HEAD METAL SCREW

... WIND WARNINGS ISSUED.

2.

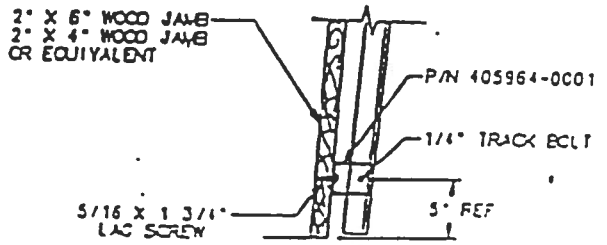


THE DRAWING AND/OR TECHNICAL INFORMATION ON THIS SHEET IS THE PROPERTY OF OVERHEAD DOOR CORPORATION AND IS LOANED IN CONFIDENCE FOR ... AND MAY NOT BE REPRODUCED OR USED TO MANUFACTURE ... WITHOUT THE EXPRESS PERMISSION OF OVERHEAD DOOR CORPORATION ...

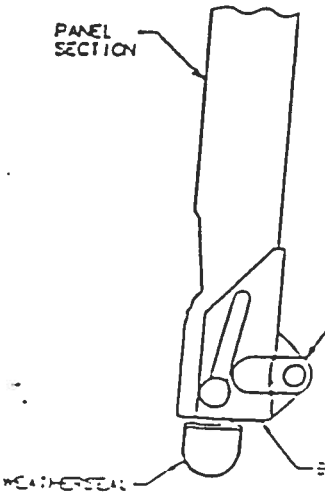
OVERHEAD DOOR	DRAWN BY: M. YOUNIS	01/13/98	SERIES 280 & 180 RESI STL DR, 16'-0" MAX WIDTH, WINDLOAD
SCALE: N/A	CHECKED BY: DAVID FAW	02/19/98	D-409335
NONE / EACH	APPROVED BY: DAVID FAW	02/19/98	

NOTES:

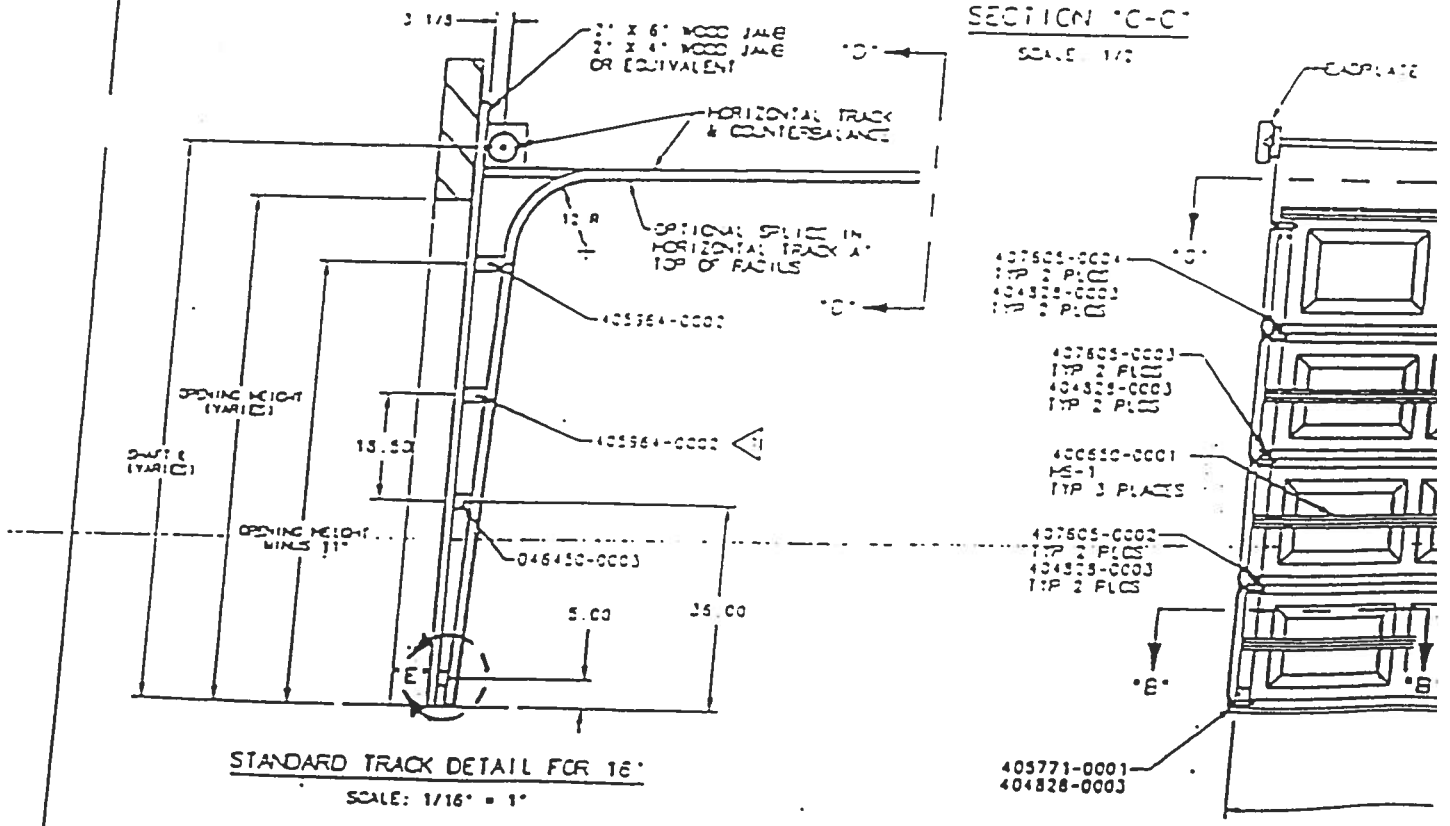
1. TESTED IN ACCORDANCE WITH STANDARD BUILDING CODE, CHAPTER 17, TO A POSITIVE AND NEGATIVE 37.5 PSF.
2. DASH NUMBERS REPRESENT VARIOUS SECTION HEIGHTS.
3. FOUR SECTION 7' HIGH DOOR SHOWN. 8' HIGH DOORS HAVE FIVE SECTIONS.
4. SECTION HEIGHT OF 20.312, 19.00 & 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS HEIGHT DOORS.
5. EMBOSSMENT PATTERN OF 14.50 X 20.375 SHOWN. ALTERNATE PATTERNS OF 12.50 X 43.375 AND 12.50 X 20.375 MAY BE USED.
6. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
7. USE THIS BRACKET, REF. P/N 405964-0002, ON 8' HIGH DOORS ONLY.
8. WINDOW MAY BE INSTALLED IN THE TOP SECTION OR THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
9. THE STRUT PLACEMENT (CONSISTENT WITH THE (SECTIONS ARE TO BE PL
10. THE STRUT PLACEMENT C REDUCED BY THE DIFFER
11. SCREW P/N 605911-0001
12. POST TO BE INSTALLED (
13. STRUT PLACEMENTS CAN
14. QUANTITY FOR LOCKS CAN



DETAIL "E"
SCALE: 1/3



VIEW
2" TR
.049 THK (V



STANDARD TRACK DETAIL FOR 16'
SCALE: 1/16" = 1"

SECTION "C-C"
SCALE: 1/2

SERIES 280 THRU 289 ARE EQUIVALENT CONSTRUCTION 25GA STEEL
SERIES 180 ARE SAME CONSTRUCTION AS SERIES 280 ONLY 24GA STEEL AND END CAPS

DOOR TESTED WAS 281 SERIES.

DOOR WIDTH	CENTER STILE	END STILE	ROLLER SHAFT BRACKET	STRUTS/SECT.	ROLLER	VERTICAL TRACK GAGE	JAMB LOAD (# PER FT-HT)	HARDWARE
16'	3	SINGLE		HS1 HS0	2" X 7/16"	.049"	100	STD.

SHINGLES

ELK Premium Roofing

March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:

ASTM D3462
PA 100 (110 mph uplift and wind driven rain resistance)
PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA)

Raised Profile, Prestique High Definition, Prestique 25, or Prestique 30 –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.04

Prestique I 25 or Prestique I* –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection* –
PA 100 = 4 nails
PA 107 = 4 nails
MD NOA# = 01-1226.03

Capstone*
PA 100 = 4 Nails
PA 107 = 4 Nails
MD NOA# = 01-0523.01

* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty

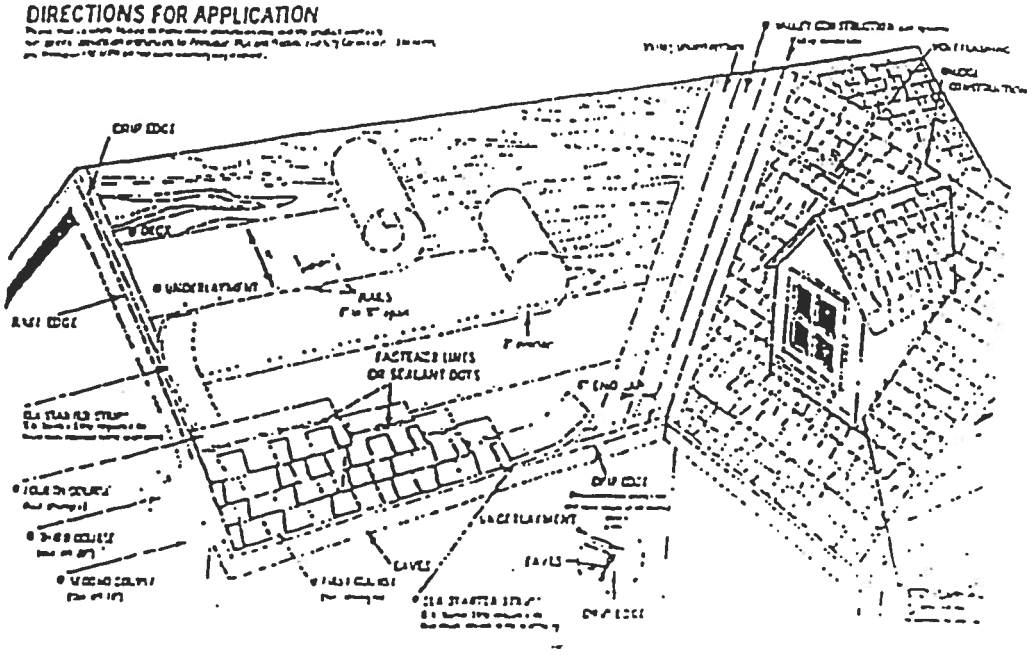
If there are any questions please contact:

Mike Reed – Technical Manager (205) 342-0287

or

Daniel Delemette – QA Engineer (205) 342-0293

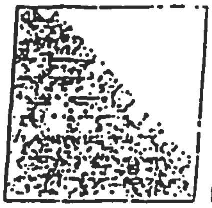
DIRECTIONS FOR APPLICATION



ELK
www.elkcorp.com

ROOFING PRODUCTS SPECIFICATIONS - TUSCALOOSA, AL

PRESTIQUE®
HIGH DEFINITION®



RAISED PROFILE™

HIP AND RIDGE SHINGLES

SOUTHEAST & ATLANTIC OFFICE:
800.945.5551

CORPORATE HEADQUARTERS:
877.354.7732

PLANT LOCATION:
800.945.5545

ELK
www.elkcorp.com

WINDOWS

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

1 APRIL 2002

REGISTERED PROFESSIONAL ENGINEER STATE OF FLORIDA

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

NO. 18354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES, JR.
CERTIFICATE NO. [illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft²	0.3 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE
NO. 1337
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	0.18" max.
@ 70.8 psf (negative)	0.05"	0.8" max.

1 APRIL 2002

LEE H. REEVES II
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:dlb
01-41134.01

ALLEN N. REEVES
CERTIFICATE NO. 1355
STATE OF FLORIDA
PROFESSIONAL ENGINEER

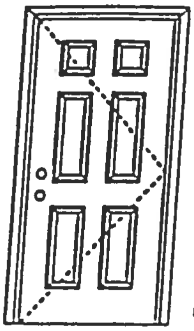
X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+66.0/-66.0

Limited water unless special threshold design is used

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7 or local state or local building codes specify the edition required

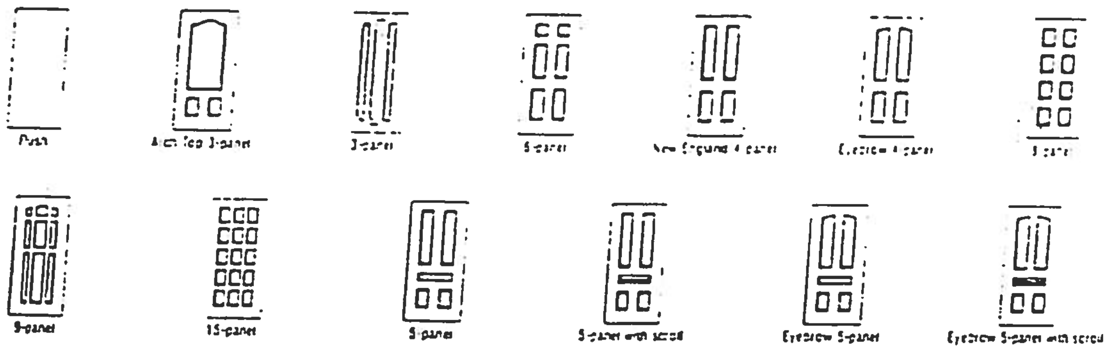
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02

APPROVED DOOR STYLES:



Johnson EntrySystems

June 17, 2002

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice

PREMDOR Premium Quality Doors

Exclusively from Masonite

Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Variation Matrix #3026447A-001 provides additional information - available from the ITS WH website (www.etsiwh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002

PREMDOR Premium Quality Doors

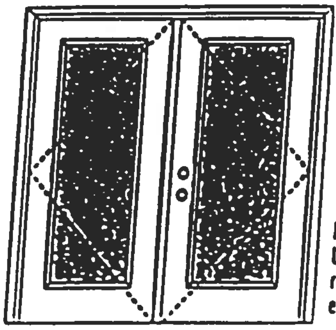
Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED

MINIMUM ASSEMBLY DETAIL:

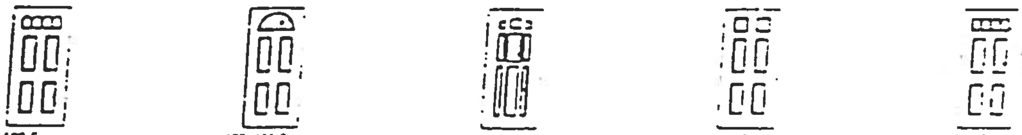
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02

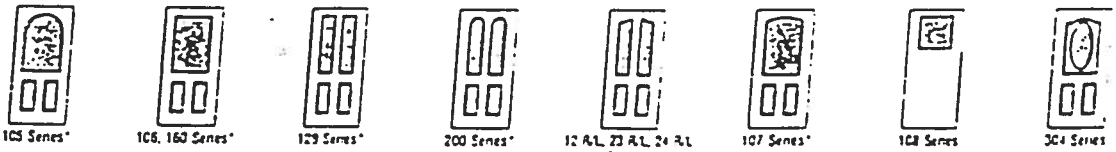
APPROVED DOOR STYLES.

1/4 GLASS:



100 Series — 122, 125 Series — 126 Series — 530 Series — 522 Series

1/2 GLASS:



105 Series* — 106, 160 Series* — 129 Series* — 200 Series* — 12 R.L, 23 R.L, 24 R.L Series* — 107 Series* — 108 Series — 304 Series

*This glass kit may also be used in the following door styles: 5-panel, 5-panel with scroll, Eyebrow 5-panel, Eyebrow 5-panel with scroll

Johnson EntrySystems
June 17, 2002

PREMDOOR — Exclusively from Masonite — Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12, NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Penney, P.E. / 16258

Unit Tested in Accordance with Miami-Dade ECCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from 4.000" Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a single plastic lip lite surround

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE ECCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #30754473 and CCP/Test Report Validation Matrix #30754473-001 provides additional information - available from the ITS/WH website (www.ntsweb.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Johnson EntrySystems

June 17, 2002

PREMDOR

Masonite

Masonite International Corporation

GARAGE HEADER (84) $26F_b - 1.9E$

Anthony Power Header®

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	950 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-1/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
	750 / 490	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	—

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

WOODFORD PLYWOOD, INC.
"Structural Wood Products"

11960 West Beaver Street
Jacksonville, Florida 32220

(904) 695-9080
(800) 447-6568
FAX (904) 695-9160

Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	732	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Project Information for:	L138285		
Builder:	BRYAN ZECHER	Date:	11/2/2005
Lot:	LOT 72 EMERALD COVE	Start Number:	1099
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	33		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
ALACHUA,FL 32615 Designer: 134

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1102051099	11/2/2005				
2	CJ3	1102051100	11/2/2005				
3	CJ3A	1102051101	11/2/2005				
4	CJ5	1102051102	11/2/2005				
5	CJ5A	1102051103	11/2/2005				
6	EJ5	1102051104	11/2/2005				
7	EJ7	1102051105	11/2/2005				
8	EJ7A	1102051106	11/2/2005				
9	HJ5	1102051107	11/2/2005				
10	HJ7	1102051108	11/2/2005				
11	HJ7A	1102051109	11/2/2005				
12	T01	1102051110	11/2/2005				
13	T02	1102051111	11/2/2005				
14	T03	1102051112	11/2/2005				
15	T04	1102051113	11/2/2005				
16	T05	1102051114	11/2/2005				
17	T06	1102051115	11/2/2005				
18	T07	1102051116	11/2/2005				
19	T08	1102051117	11/2/2005				
20	T09	1102051118	11/2/2005				
21	T09	1102051119	11/2/2005				
22	T10	1102051120	11/2/2005				
23	T11	1102051121	11/2/2005				
24	T12	1102051122	11/2/2005				
25	T13	1102051123	11/2/2005				
26	T14	1102051124	11/2/2005				
27	T15	1102051125	11/2/2005				
28	T16	1102051126	11/2/2005				
29	T17	1102051127	11/2/2005				
30	T18	1102051128	11/2/2005				
31	T19H	1102051129	11/2/2005				
32	T20H	1102051130	11/2/2005				
33	T21H	1102051131	11/2/2005				

Dwg.#1102051098

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Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN** (Primary Name) **BRYAN ZECHER CONSTRUCTION INC** (DBA
Main Address:	**P O BOX 815 LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST LAKE CITY, FL 32055 United States Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

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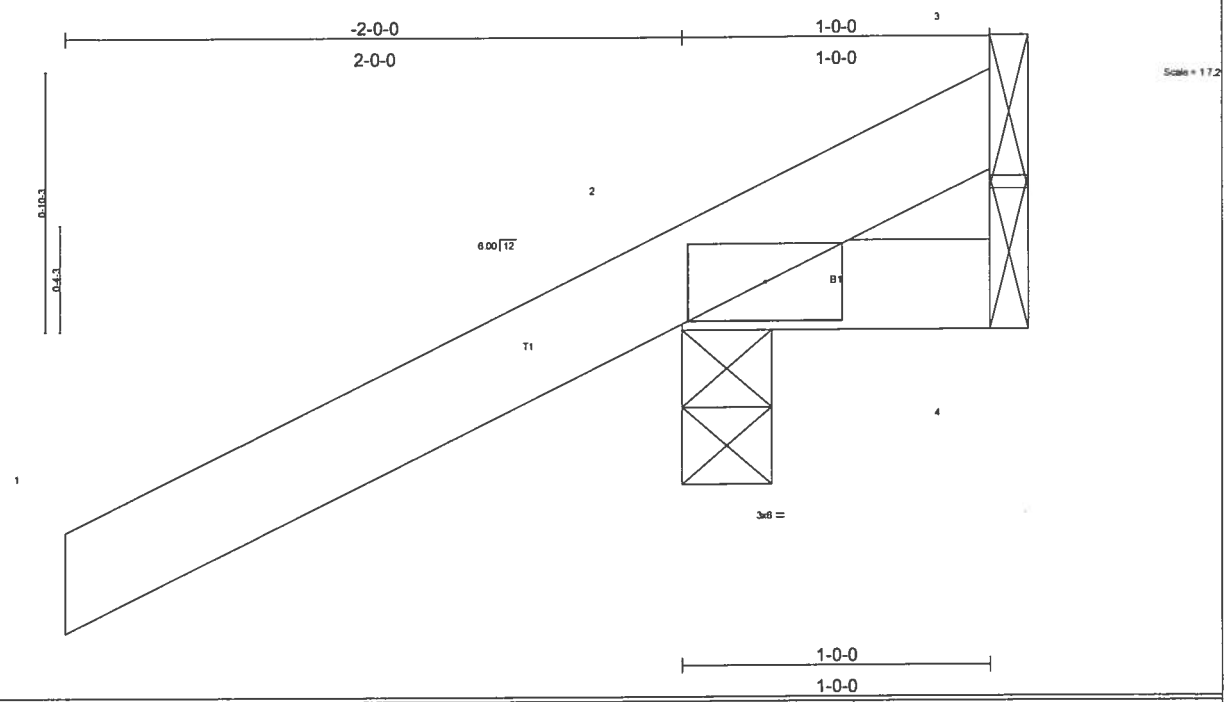
NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	CJ1	MONO TRUSS	10	1	Dwg.#1102051099

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:30 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-274(load case 5), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

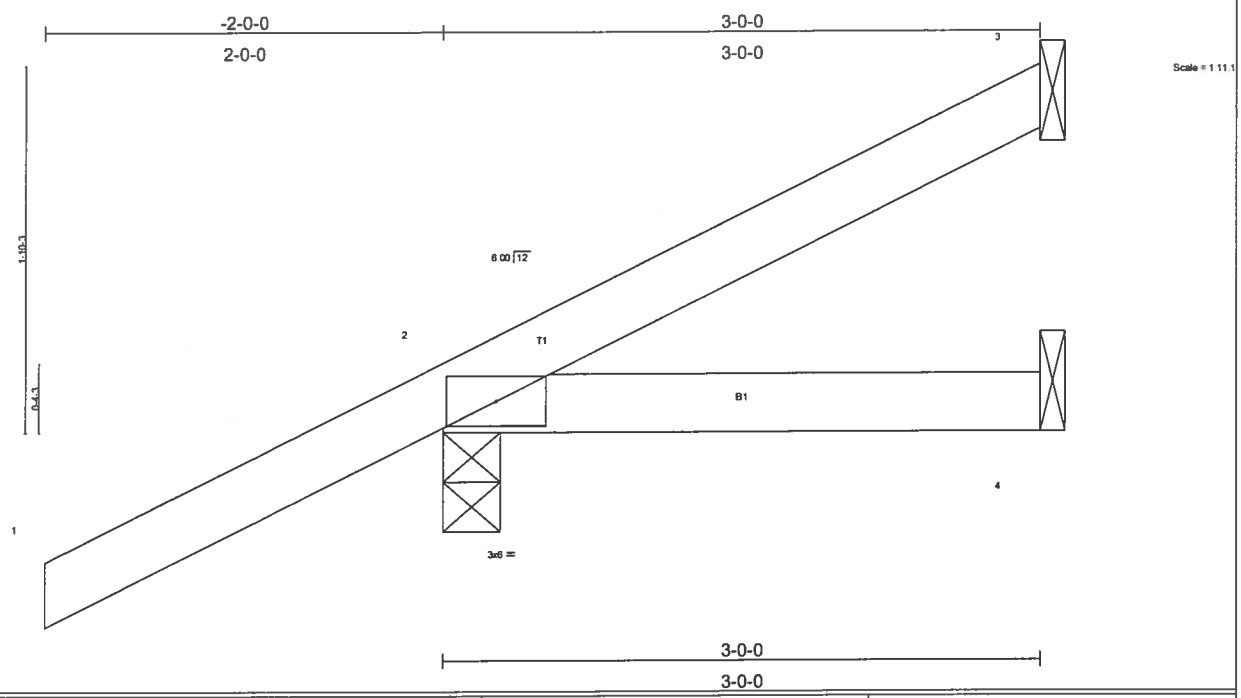
LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	CJ3	MONO TRUSS	10	1	Dwg.#1102051100

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:31 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	CJ3A	MONO SCISSOR	4	1	Dwg.#1102051101

Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:31 2005 Page 1

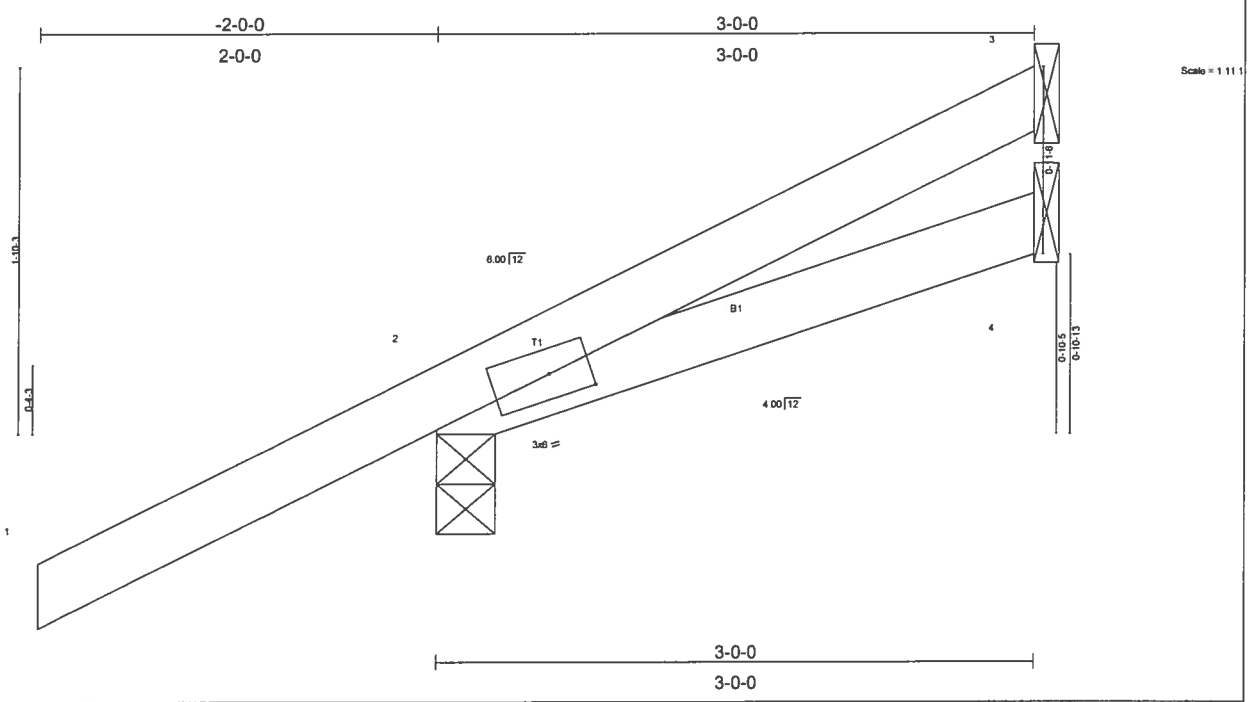


Plate Offsets (X,Y): [2:0-2-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=131(load case 5)
Max Uplift3=-29(load case 6), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-58/8
BOT CHORD 2-4=-11/11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 3 and 201 lb uplift at joint 2.

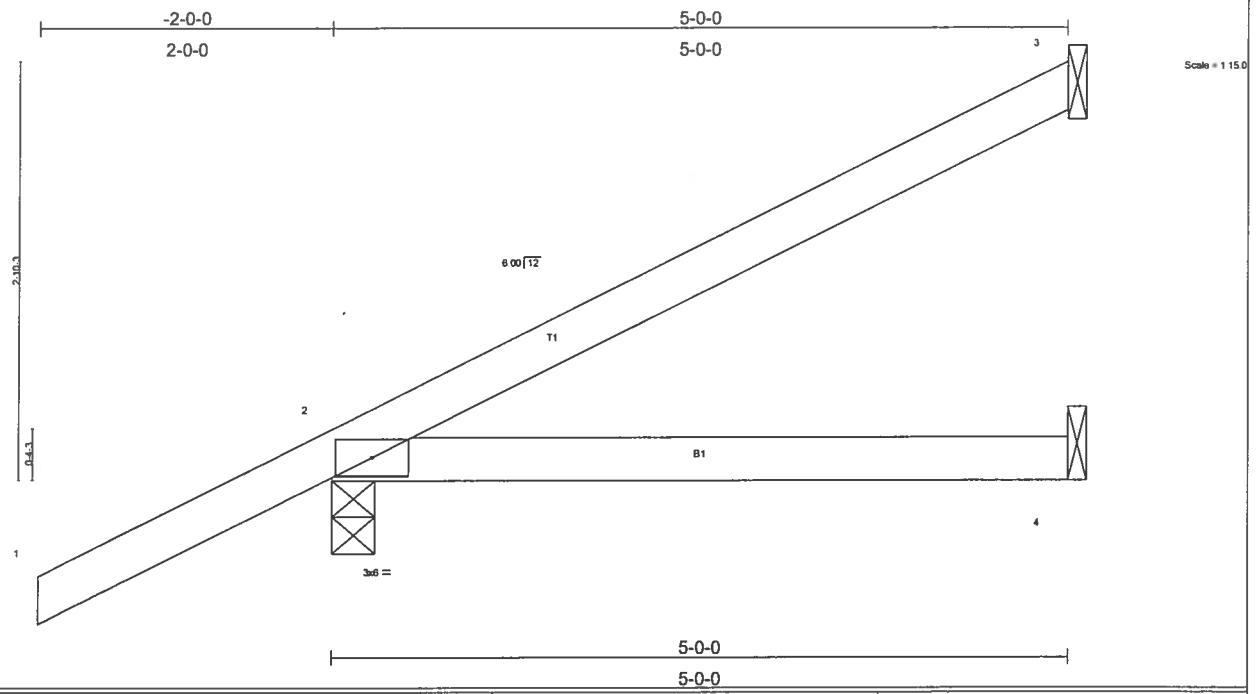
LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	CJ5	MONO TRUSS	6	1	Dwg.#1102051102

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:32 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	CJ5A	MONO SCISSOR	2	1	Dwg.#1102051103
					Job Reference (optional)

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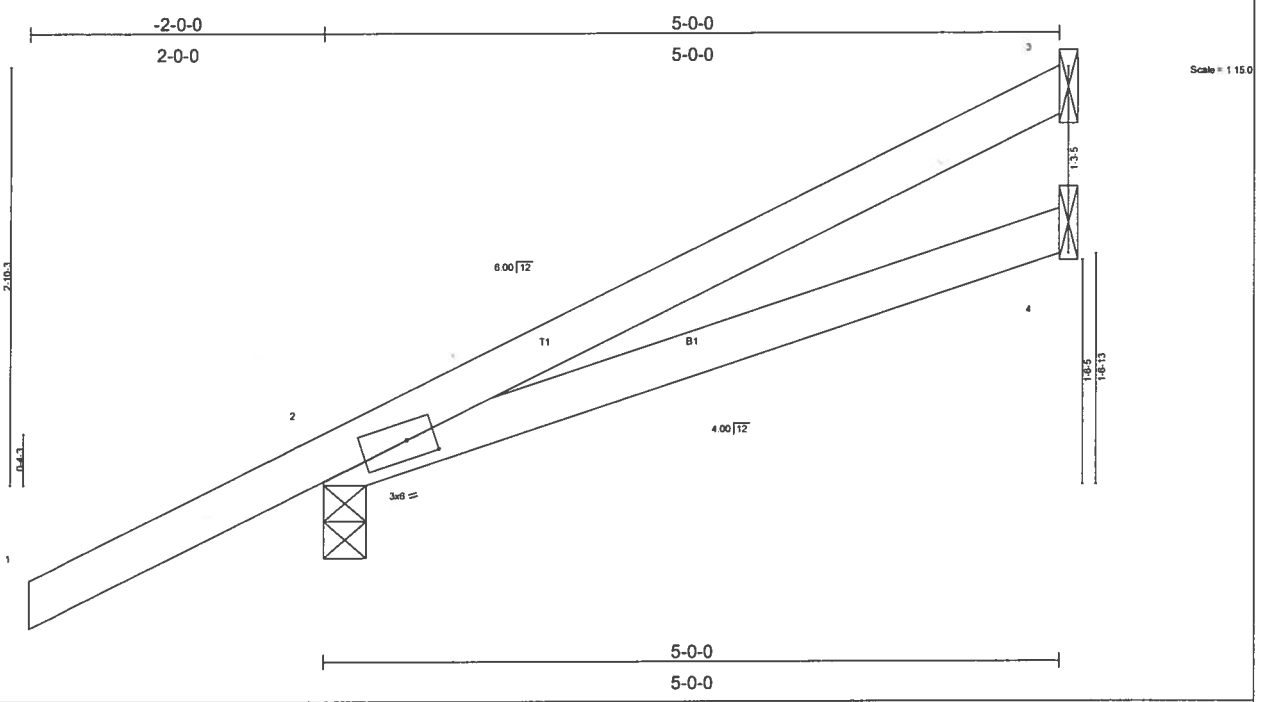


Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=177(load case 5)
Max Uplift3=-89(load case 5), 2=-198(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-90/36
BOT CHORD 2-4=-18/18

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 3 and 198 lb uplift at joint 2.

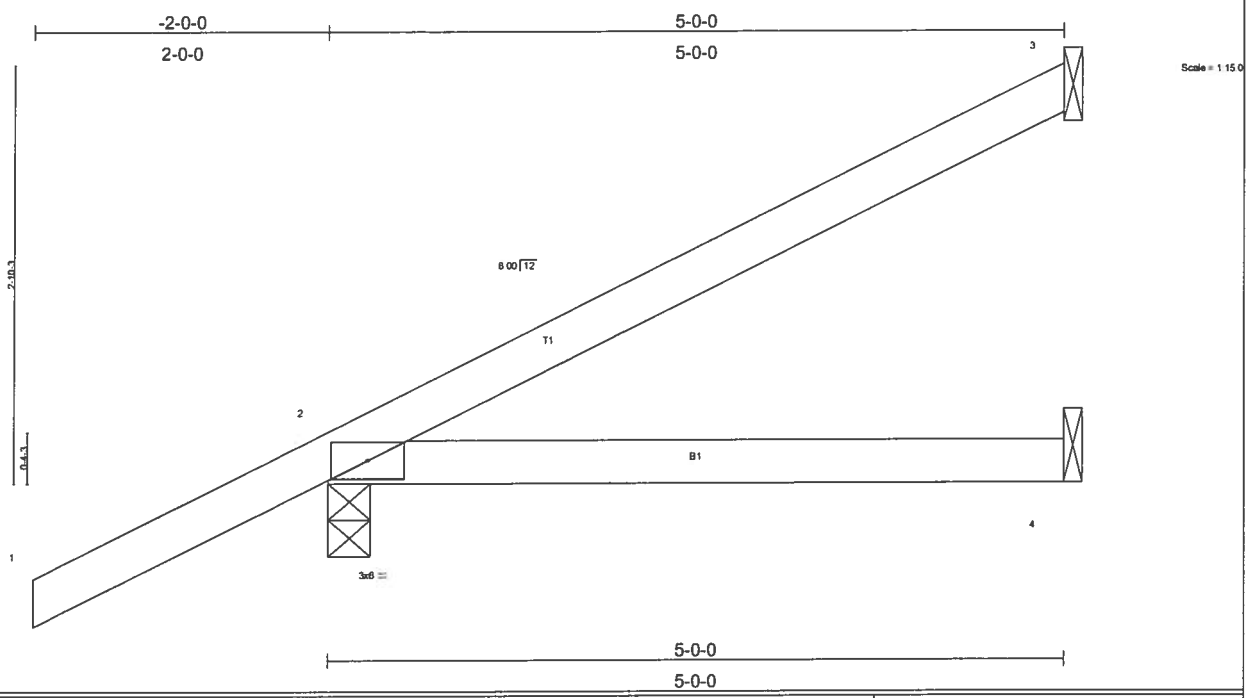
LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	EJ5	MONO TRUSS	2	1	Dwg.#1102051104

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:33 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	EJ7	MONO TRUSS	19	1	Dwg.#1102051105

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:34 2005 Page 1

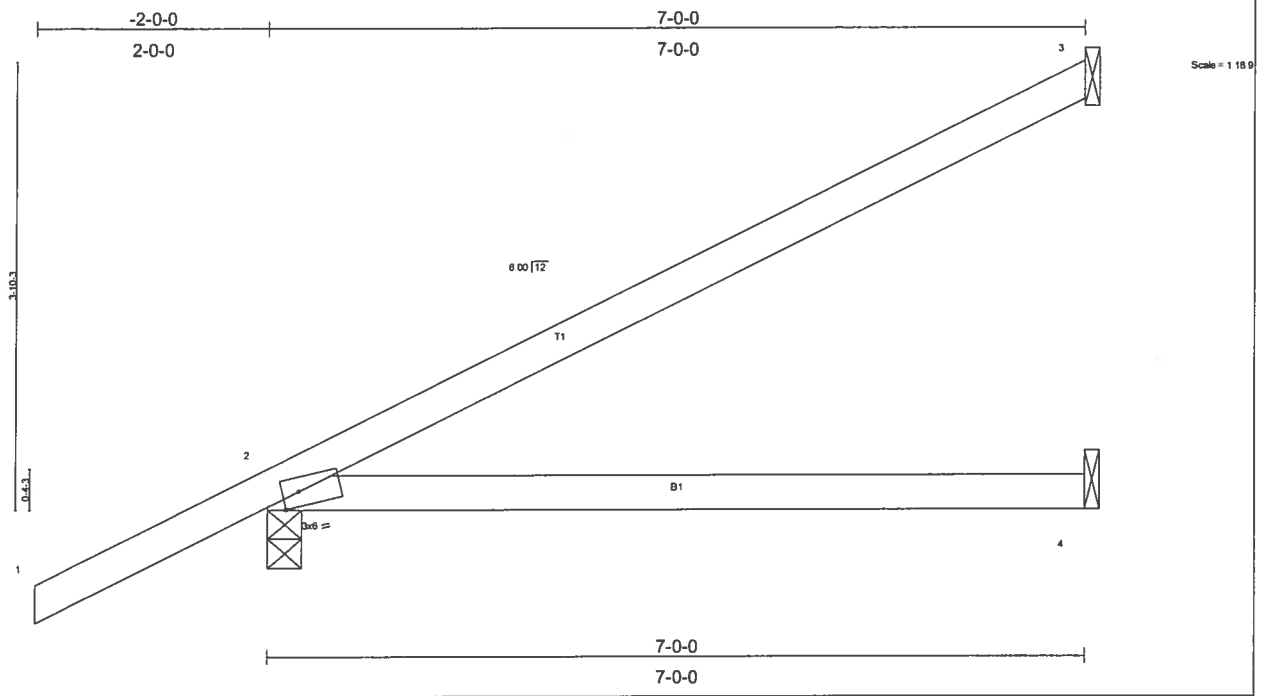


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	EJ7A	SPECIAL	8	1	Dwg.#1102051106

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:34 2005 Page 1

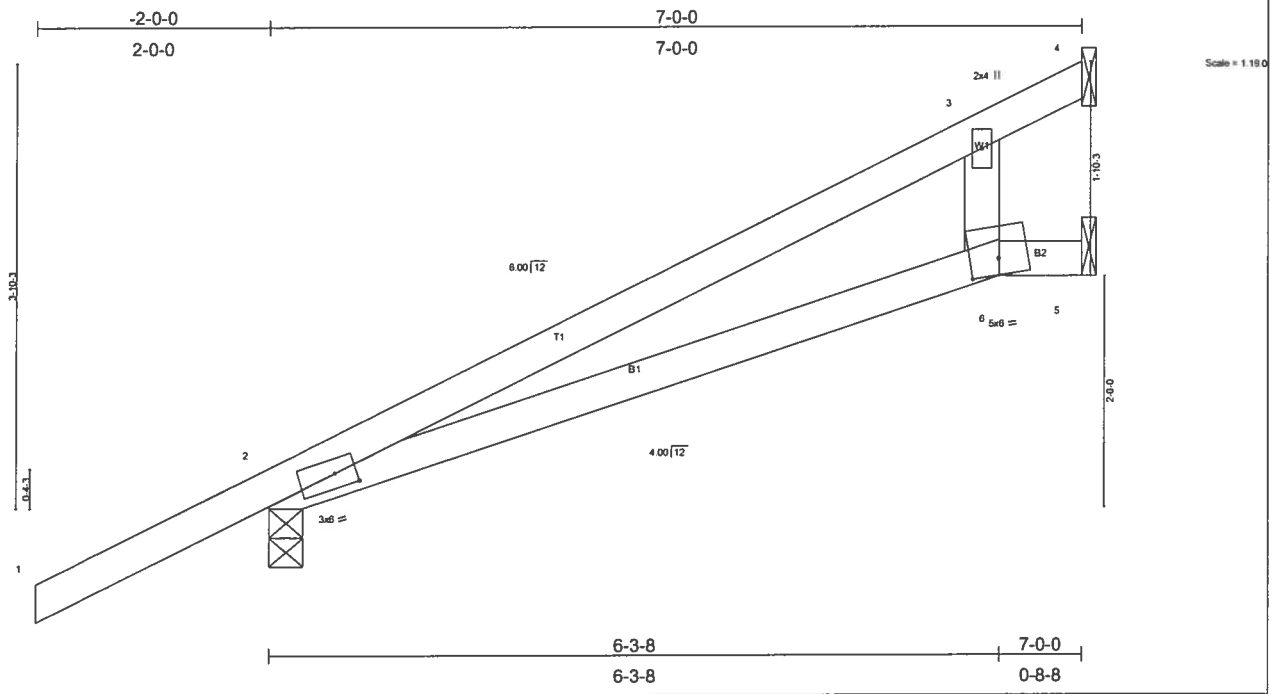


Plate Offsets (X,Y): [2:0-2-4,0-1-8], [6:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.15	2-6	>554	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.24	2-6	>338	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=39/Mechanical, 2=419/0-3-8, 5=228/Mechanical
Max Horz 2=223(load case 5)
Max Uplift2=-209(load case 5), 5=-179(load case 5)
Max Grav 4=52(load case 5), 2=419(load case 1), 5=228(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-104/32, 3-4=-0/31
BOT CHORD 2-6=-39/33, 5-6=-1/2
WEBS 3-6=-141/310

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 2 and 179 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	HJ5	MONO TRUSS	2	1	Dwg.#1102051107

Job Reference (optional)

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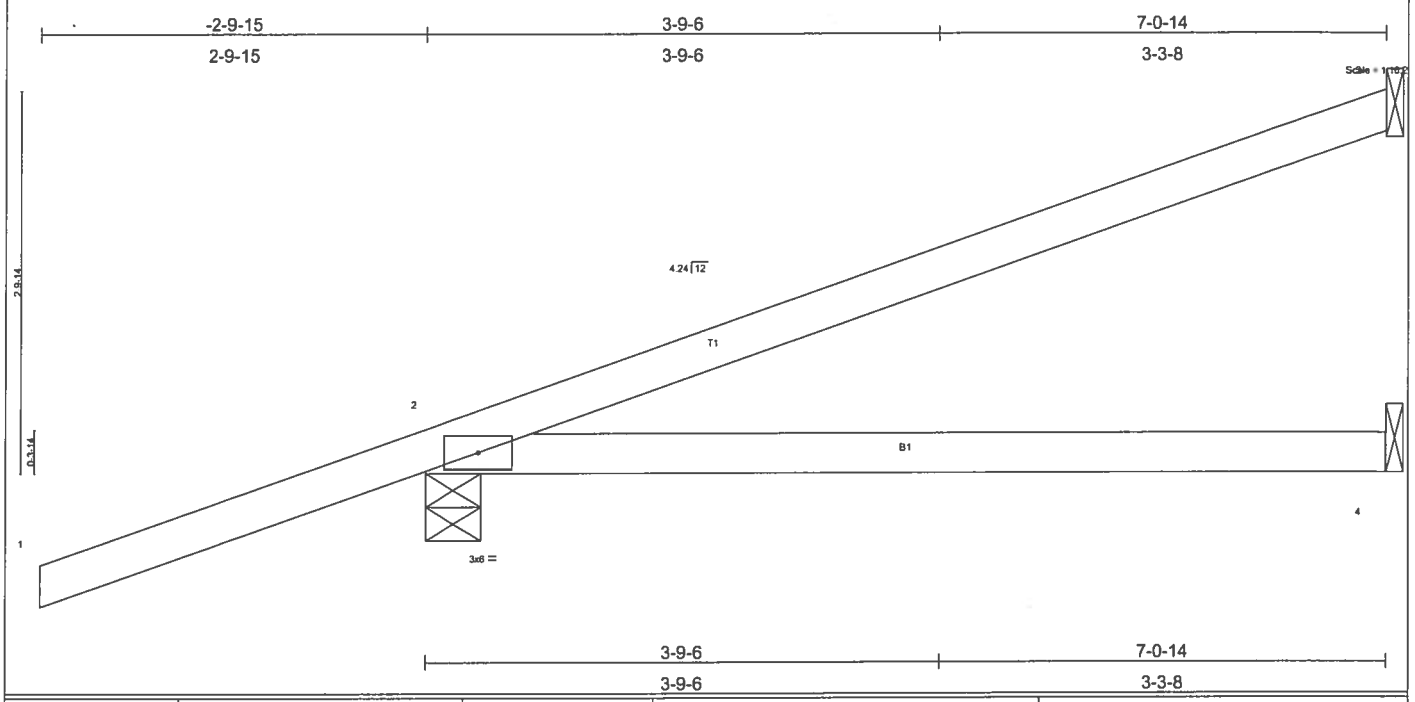

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.08	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.13	2-4	>609	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=192/Mechanical, 2=375/0-4-15, 4=111/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-145(load case 2), 2=-248(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-68/45
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 3 and 248 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	HJ7	MONO TRUSS	3	1	Dwg.#1102051108 Job Reference (optional)

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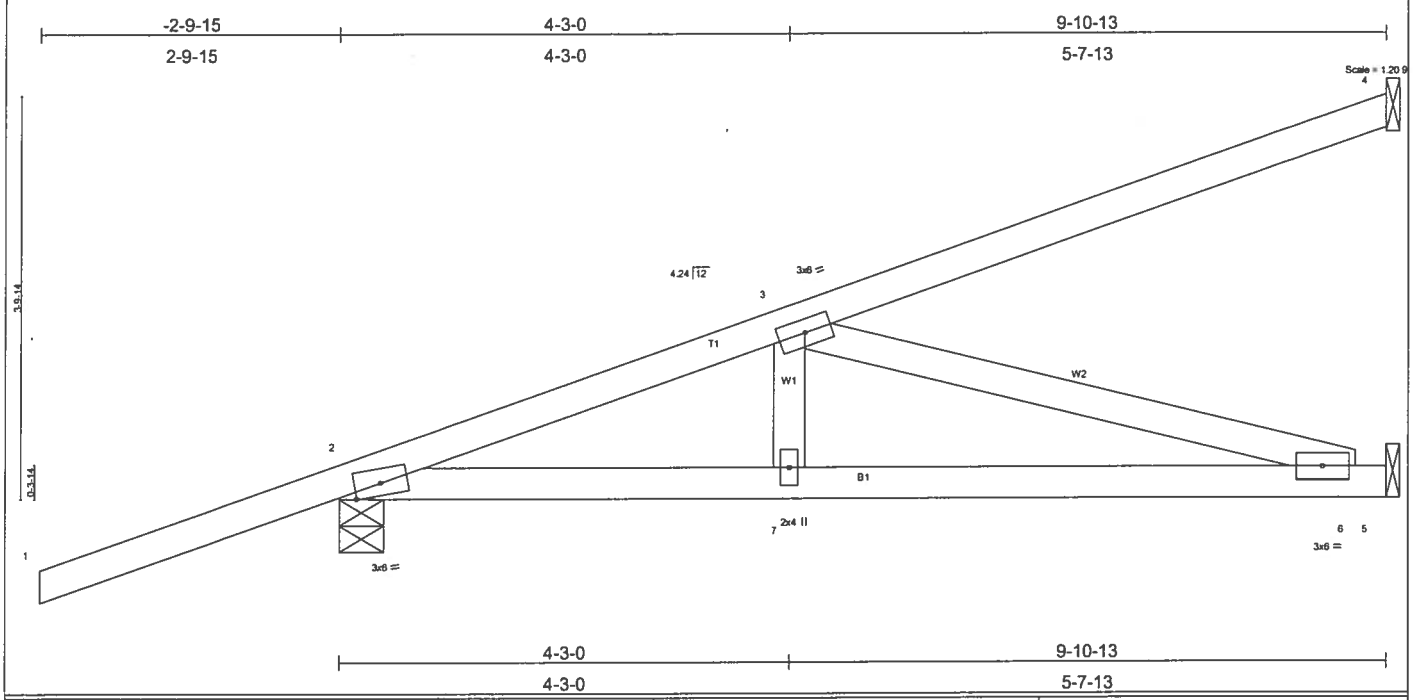

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.17	6-7	>686	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=532/0-4-15, 5=377/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-278(load case 2), 5=-63(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-888/121, 3-4=-105/66
BOT CHORD 2-7=-309/823, 6-7=-309/823, 5-6=0/0
WEBS 3-7=0/180, 3-6=-856/321

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 278 lb uplift at joint 2 and 63 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	HJ7A	SPECIAL	1	1	Dwg.#1102051109

Job Reference (optional)

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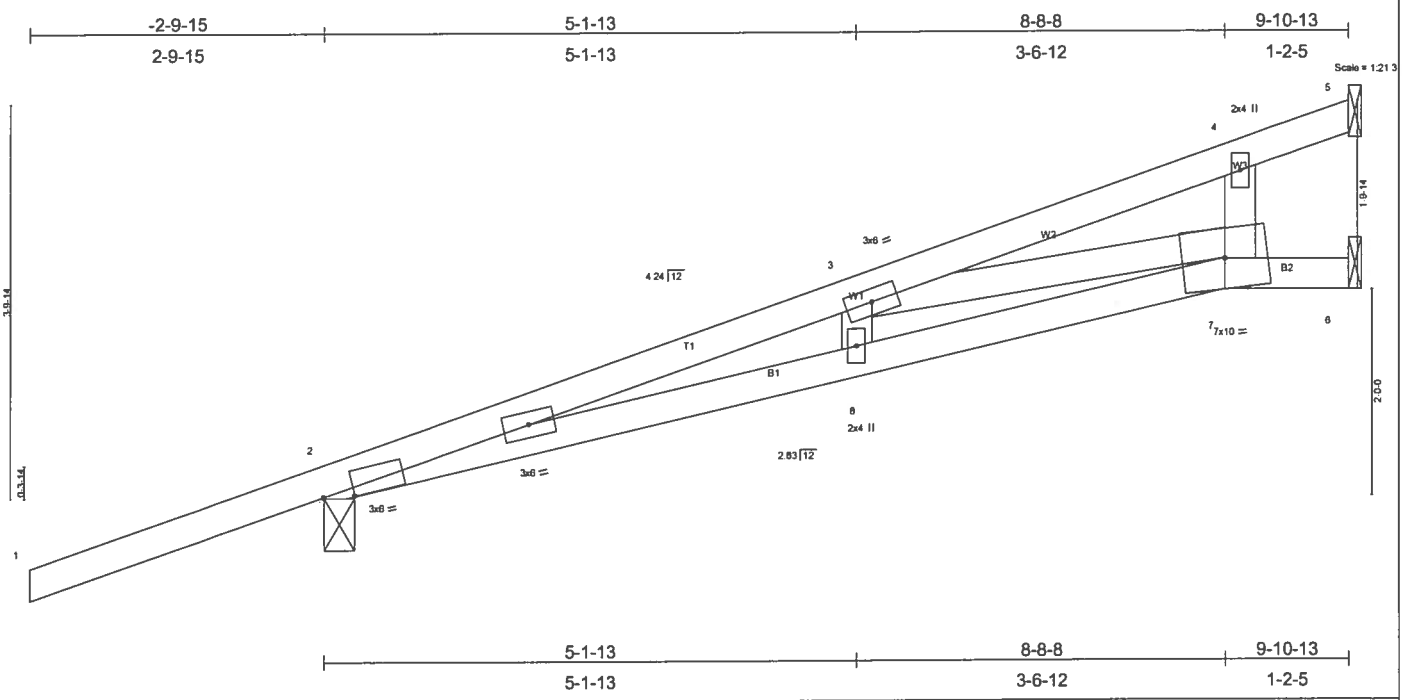

Plate Offsets (X,Y): [2:0-3-8,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.19	7-8	>601	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.31	7-8	>375	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-2 oc bracing.

REACTIONS (lb/size) 5=356/Mechanical, 2=527/0-3-8, 6=294/Mechanical
Max Horz 2=268(load case 2)
Max Uplift5=-157(load case 2), 2=-272(load case 2), 6=-140(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-1850/514, 3-4=-114/0, 4-5=-59/114
BOT CHORD 2-8=-670/1761, 7-8=-672/1757, 6-7=0/0
WEBS 3-8=0/197, 3-7=-1694/669, 4-7=0/138

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 5, 272 lb uplift at joint 2 and 140 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-5=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-7=-66(F=-18, B=-18), 7=-66(F=-18, B=-18)-to-6=-74(F=-22, B=-22)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T01	COMMON	5	1	Dwg.#1102051110

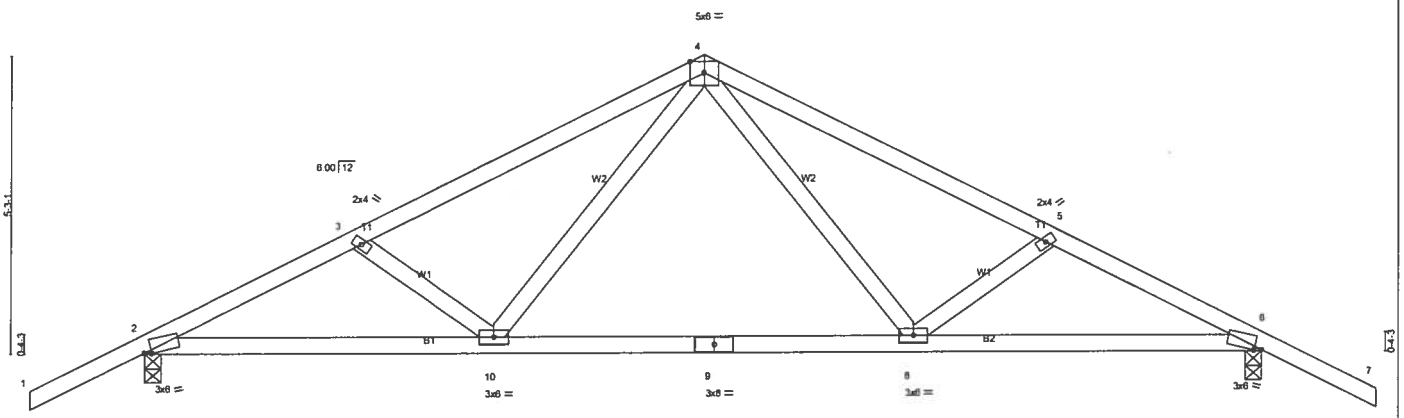
Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:37 2005 Page 1

-2-0-0 | 3-9-13 | 9-9-12 | 15-9-11 | 19-7-8 | 21-7-8

2-0-0 | 3-9-13 | 5-11-15 | 5-11-15 | 3-9-13 | 2-0-0

Scale = 1:38.6



6-1-10 | 13-5-14 | 19-7-8

6-1-10 | 7-4-4 | 6-1-10

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.25	8-10	>930	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.40	8-10	>573	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-5 oc bracing.

REACTIONS (lb/size) 2=1112/0-3-8, 6=1112/0-3-8
Max Horz 2=-107(load case 6)
Max Uplift2=-459(load case 5), 6=-459(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1848/803, 3-4=-1679/740, 4-5=-1679/740, 5-6=-1848/803, 6-7=0/47
BOT CHORD 2-10=-567/1590, 9-10=-275/1046, 8-9=-275/1046, 6-8=-567/1590
WEBS 3-10=-216/201, 4-10=-222/686, 4-8=-222/686, 5-8=-216/201

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 459 lb uplift at joint 2 and 459 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-10=-30, 8-10=-80(F=-50), 6-8=-30, 1-4=-54, 4-7=-54

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

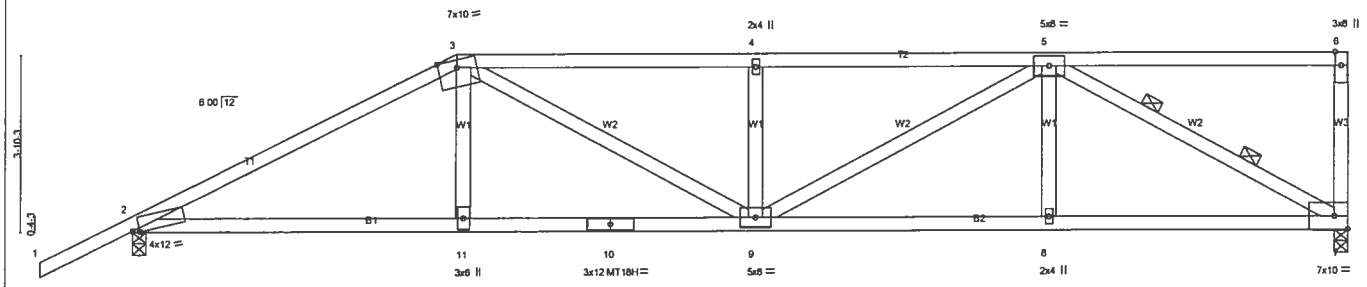
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T02	MONO HIP	1	1	Dwg.#1102051111

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:38 2005 Page 1

-2-0-0	7-0-0	13-5-15	19-10-1	26-4-0
2-0-0	7-0-0	6-5-15	6-4-3	6-5-15

Scale: 1/4"=1'



7-0-0	13-5-15	19-10-1	26-4-0
7-0-0	6-5-15	6-4-3	6-5-15

Plate Offsets (X,Y): [2:0-1-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.32	9-11	>979	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.52	9-11	>604	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 133 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-9 oc bracing.
WEBS 2 Rows at 1/3 pts 5-7

REACTIONS (lb/size) 7=2431/0-3-8, 2=2304/0-3-8
Max Horz 2=227(load case 4)
Max Uplift7=-1094(load case 3), 2=-993(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-4309/1821, 3-4=-4702/2113, 4-5=-4701/2114, 5-6=-154/72, 6-7=-362/287
BOT CHORD 2-11=-1667/3765, 10-11=-1675/3800, 9-10=-1675/3800, 8-9=-1557/3465, 7-8=-1557/3465
WEBS 3-11=-214/863, 3-9=-565/1027, 4-9=-761/645, 5-9=-638/1417, 5-8=0/406, 5-7=-3795/1703

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1094 lb uplift at joint 7 and 993 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-118(F=-64), 2-11=-30, 7-11=-65(F=-35)
Concentrated Loads (lb)
Vert: 11=-539(F)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T03	MONO HIP	1	1	Dwg.#1102051112

Job Reference (optional)

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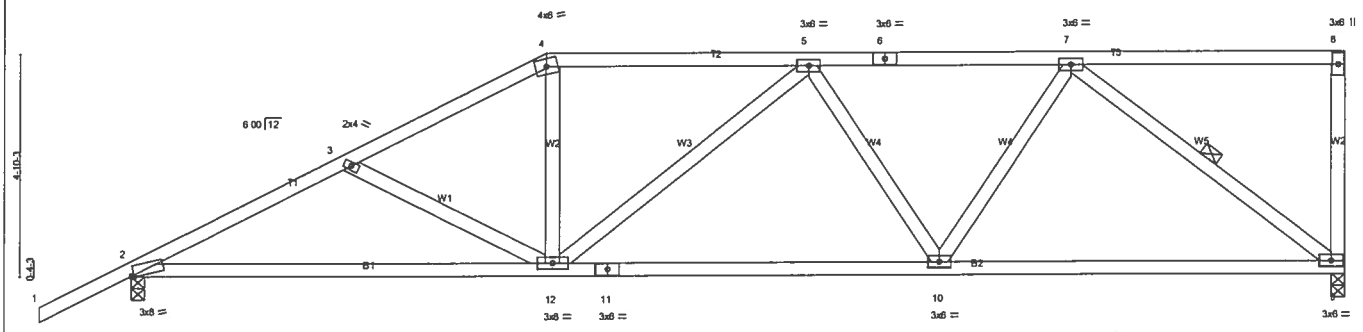
-2-0-0	4-9-4	9-0-0	14-8-3	20-4-5	26-4-0
2-0-0	4-9-4	4-2-12	5-8-3	5-8-3	5-11-11

Scale: 1/4"=1'



9-0-0	17-6-4	26-4-0
9-0-0	8-6-4	8-9-12

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.16	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.28	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-8 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=1089/0-3-8, 2=1214/0-3-8
Max Horz 2=272(load case 5)
Max Uplift9=-396(load case 4), 2=-442(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1908/743, 3-4=-1670/641, 4-5=-1463/629, 5-6=-1403/557, 6-7=-1403/557, 7-8=-59/10, 8-9=-153/102
BOT CHORD 2-12=-809/1658, 11-12=-669/1549, 10-11=-669/1549, 9-10=-477/1095
WEBS 3-12=-236/204, 4-12=-66/449, 5-12=-113/162, 5-10=-278/213, 7-10=-150/581, 7-9=-1315/593

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 9 and 442 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T04	MONO HIP	1	1	Dwg.#1102051113

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:39 2005 Page 1

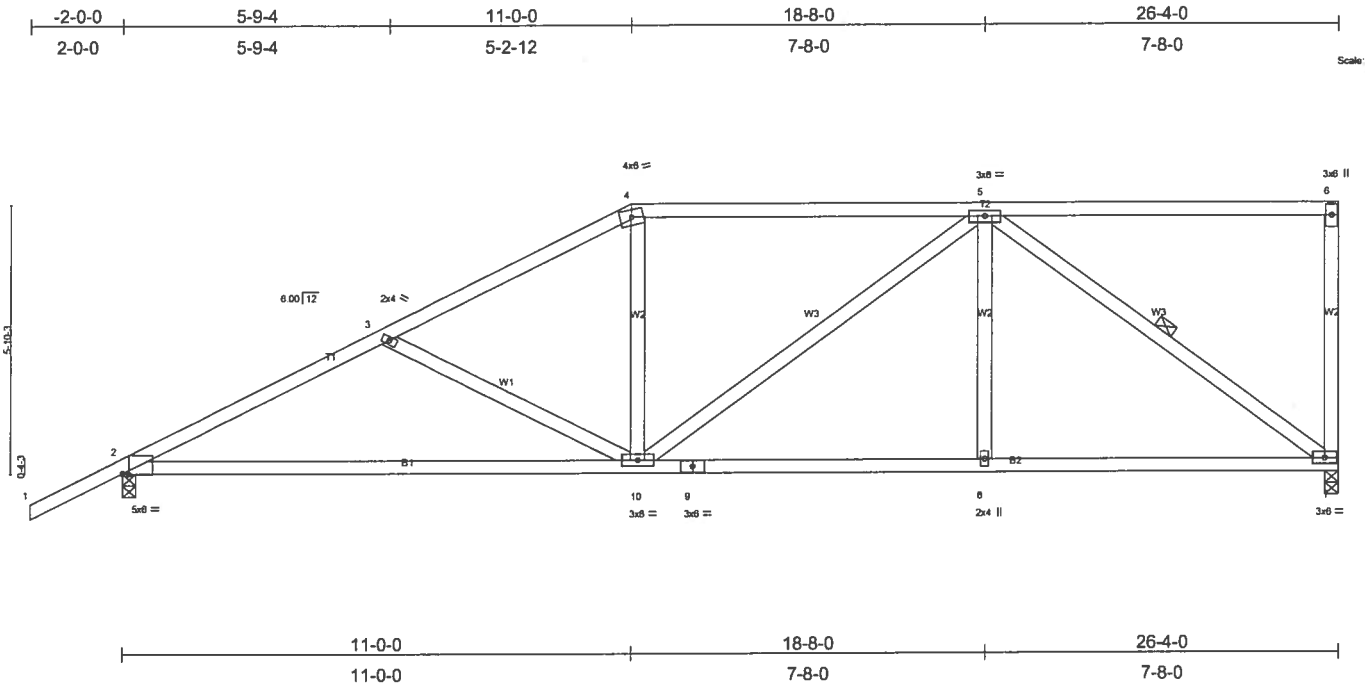


Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.34	2-10	>922	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.58	2-10	>535	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-3 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1089/0-3-8, 2=1214/0-3-8
Max Horz 2=318(load case 5)
Max Uplift7=-388(load case 4), 2=-450(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1841/728, 3-4=-1526/585, 4-5=-1317/588, 5-6=-52/19, 6-7=-185/131
BOT CHORD 2-10=-836/1602, 9-10=-485/1118, 8-9=-485/1118, 7-8=-485/1118
WEBS 3-10=-329/279, 4-10=0/309, 5-10=-131/246, 5-8=0/202, 5-7=-1326/579

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 7 and 450 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481240
L138285	T05	HIP	1	1	Dwg.#1102051114 Job Reference (optional)	

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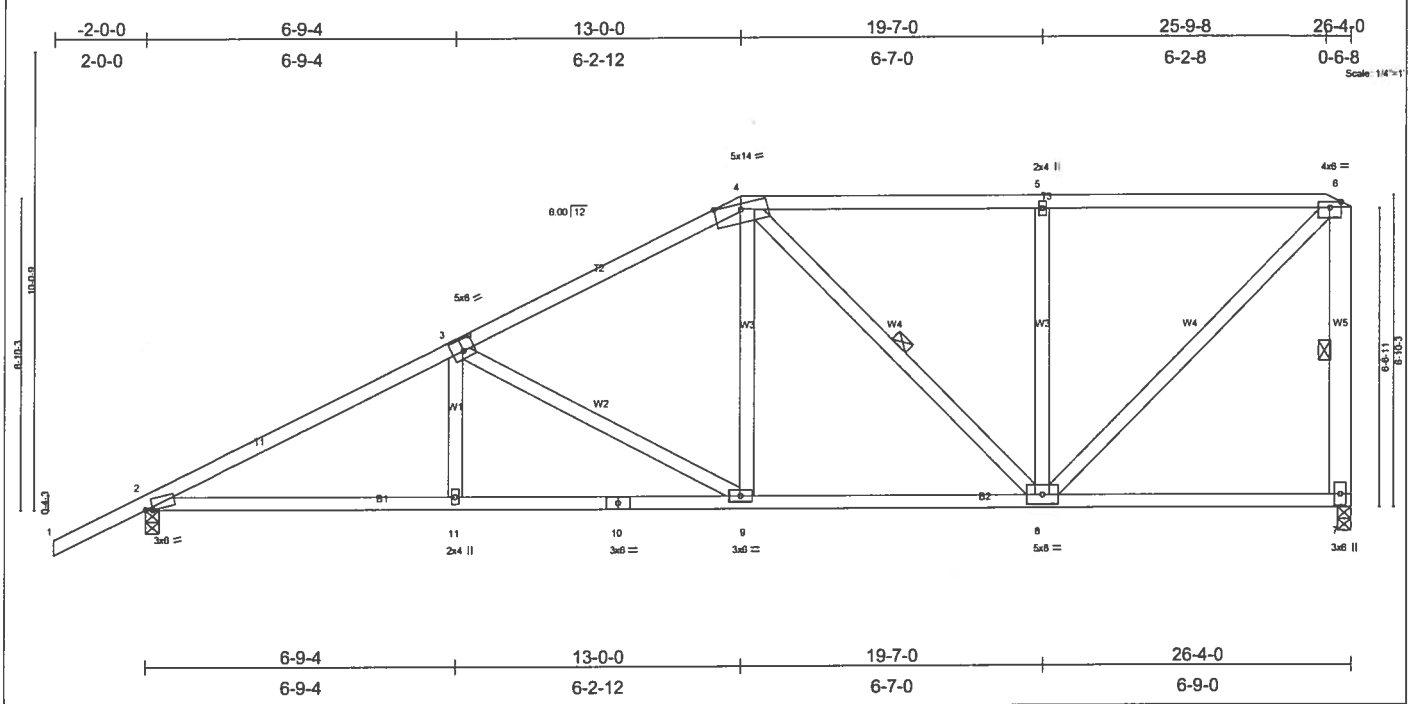

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.10	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.16	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-9 oc bracing.
WEBS 1 Row at midpt 4-8, 6-7

REACTIONS (lb/size) 2=1211/0-3-8, 7=1085/0-3-8
Max Horz 2=364(load case 5)
Max Uplift2=-454(load case 5), 7=-377(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1914/665, 3-4=-1312/520, 4-5=-852/379, 5-6=-852/379, 6-7=-985/487
BOT CHORD 2-11=-824/1632, 10-11=-824/1632, 9-10=-824/1632, 8-9=-535/1115, 7-8=-18/44
WEBS 3-11=0/213, 3-9=-599/331, 4-9=-123/472, 4-8=-371/220, 5-8=-359/265, 6-8=-512/1146

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2 and 377 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T06	HIP	1	1	Dwg.#1102051115

Job Reference (optional)

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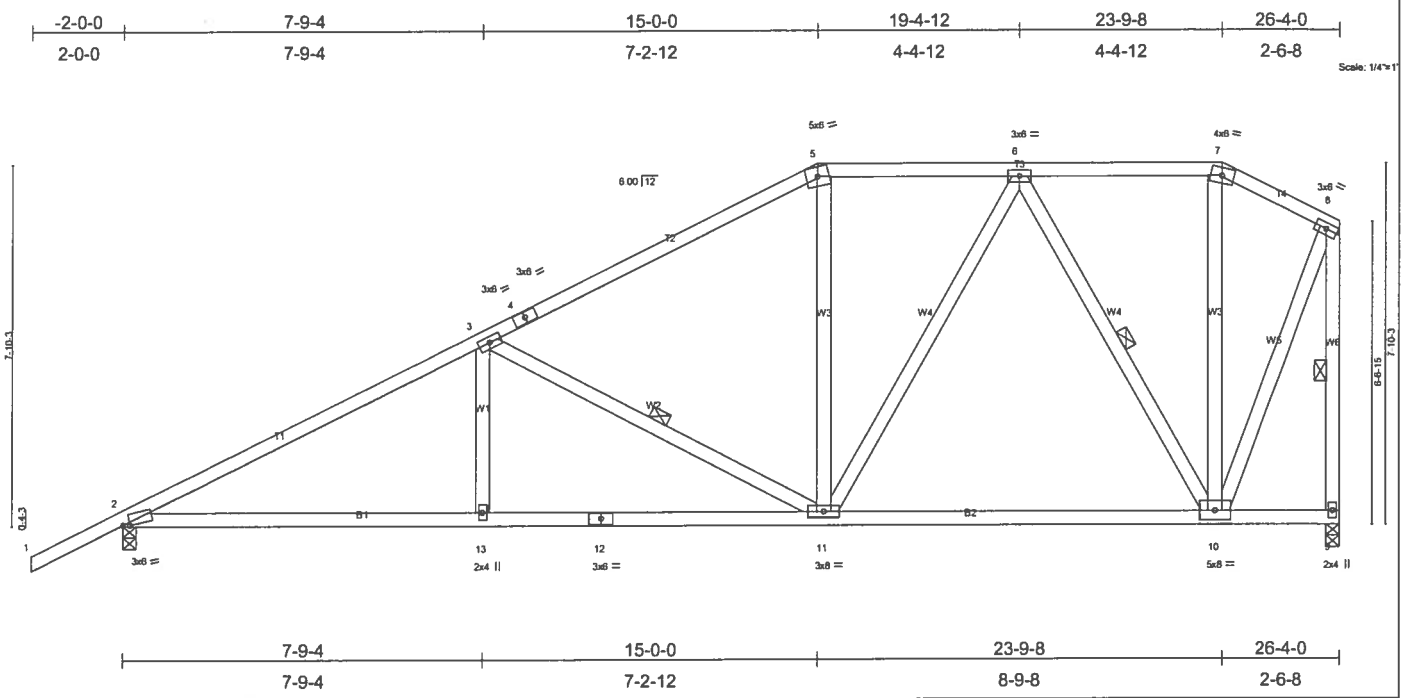

Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.15	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.24	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-0 oc bracing.
WEBS 1 Row at midpt 3-11, 6-10, 8-9

REACTIONS (lb/size) 2=1214/0-3-8, 9=1089/0-3-8
Max Horz 2=371(load case 5)
Max Uplift2=-466(load case 5), 9=-324(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1875/672, 3-4=-1161/455, 4-5=-1066/480, 5-6=-956/503, 6-7=-357/205, 7-8=-418/192, 8-9=-1104/472
BOT CHORD 2-13=-808/1593, 12-13=-808/1593, 11-12=-808/1593, 10-11=-330/716, 9-10=-4/2
WEBS 3-13=0/243, 3-11=-725/414, 5-11=0/188, 6-11=-227/494, 6-10=-740/380, 7-10=-46/74, 8-10=-379/953

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 466 lb uplift at joint 2 and 324 lb uplift at joint 9.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481242
L138285	T07	HIP	1	1	Dwg.#1102051116 Job Reference (optional)	

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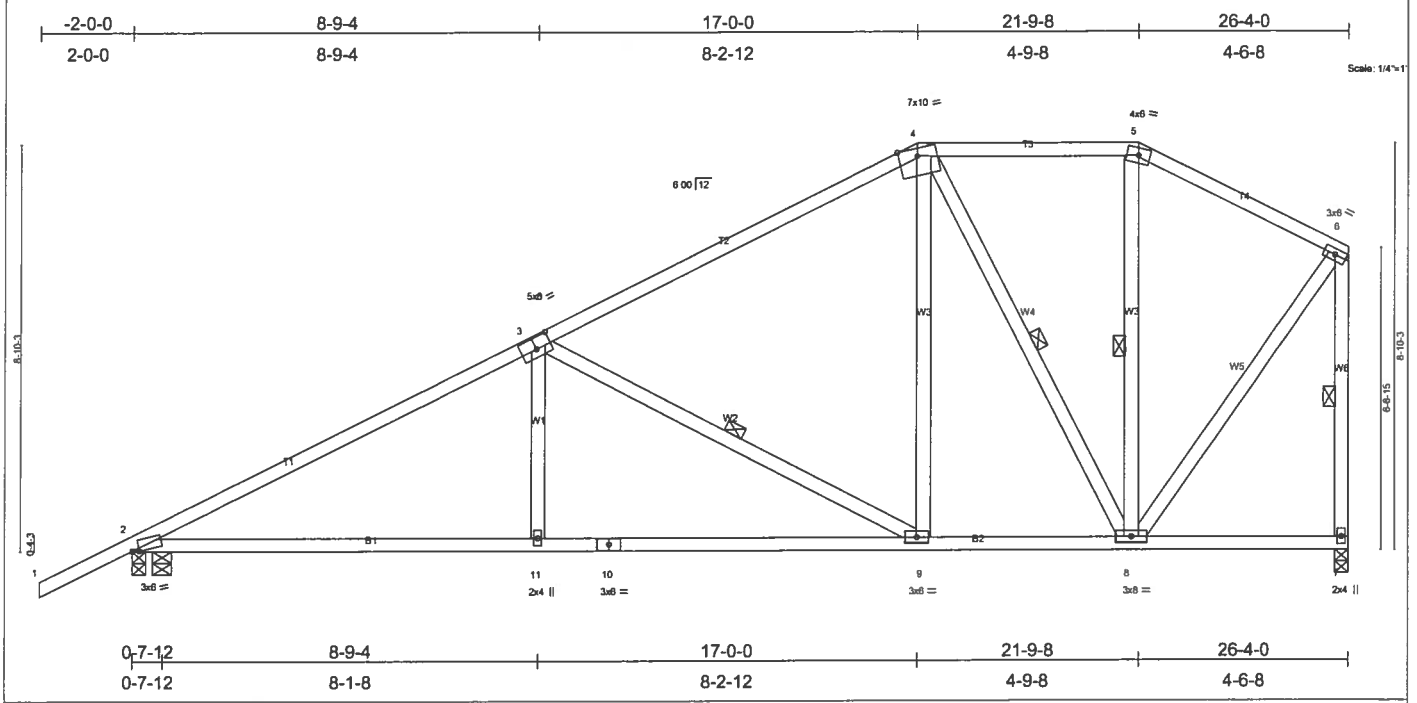

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.21	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.34	2-11	>919	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-8 oc bracing.
WEBS 1 Row at midpt 3-9, 4-8, 5-8, 6-7

REACTIONS (lb/size) 2=1214/0-3-8, 2=1214/0-3-8, 7=1089/0-3-8
Max Horz 2=385(load case 5)
Max Uplift2=-472(load case 5), 7=-345(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1835/666, 3-4=-988/433, 4-5=-489/320, 5-6=-597/303, 6-7=-1019/501
BOT CHORD 2-11=-793/1550, 10-11=-791/1557, 9-10=-791/1557, 8-9=-364/796, 7-8=-10/16
WEBS 3-11=0/299, 3-9=-867/485, 4-9=-181/593, 4-8=-672/327, 5-8=-79/102, 6-8=-340/811

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 472 lb uplift at joint 2 and 345 lb uplift at joint 7.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T08	HIP	1	1	Dwg.#1102051117

Job Reference (optional)

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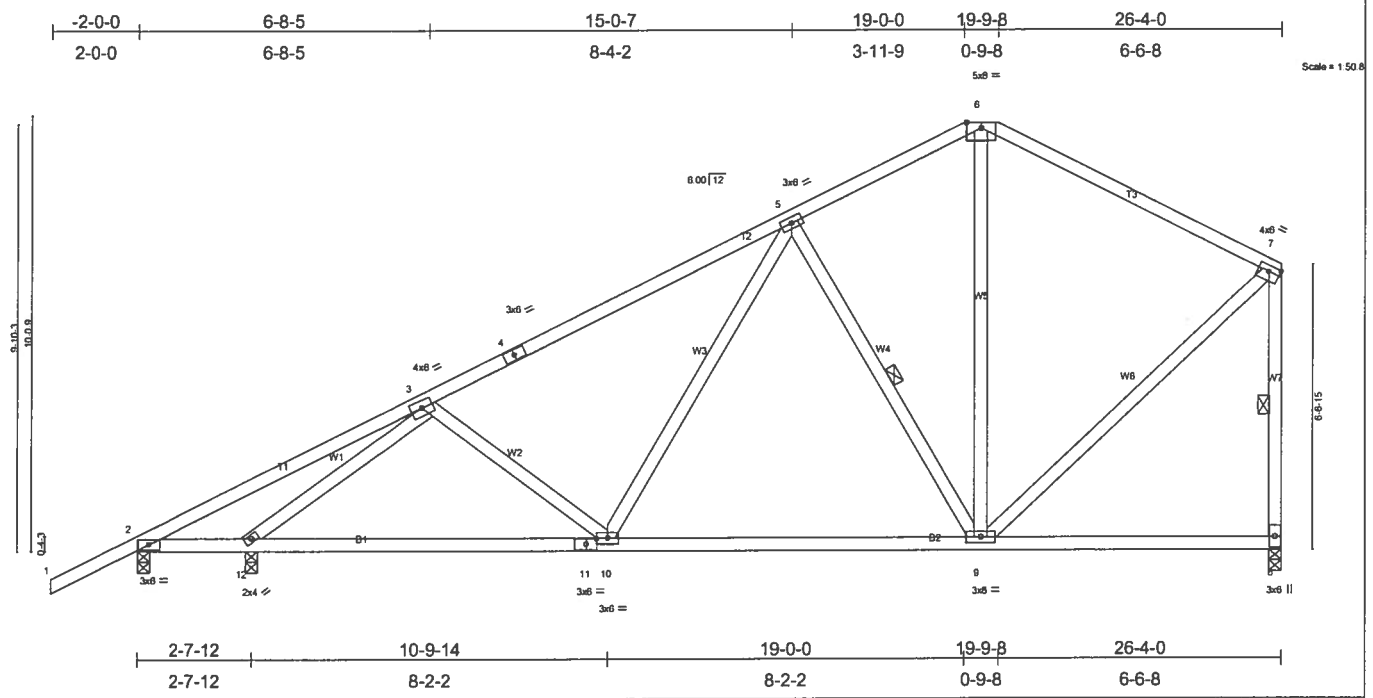

Plate Offsets (X,Y): [7:0-3-0,0-1-8], [11:0-2-14,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.11	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.19	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-3 oc bracing.
WEBS 1 Row at midpt 5-9, 7-8

REACTIONS (lb/size) 2=241/0-3-8, 8=995/0-3-8, 12=1068/0-3-8
Max Horz 2=402(load case 5)
Max Uplift2=-221(load case 5), 8=-347(load case 5), 12=-342(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-113/49, 3-4=-1199/489, 4-5=-1094/518, 5-6=-650/400, 6-7=-670/360, 7-8=-903/484
BOT CHORD 2-12=-182/36, 11-12=-709/1066, 10-11=-709/1066, 9-10=-427/795, 8-9=-25/36
WEBS 3-12=-1323/695, 3-10=-122/276, 5-10=-132/383, 5-9=-571/403, 7-9=-275/671, 6-9=-108/335

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2, 347 lb uplift at joint 8 and 342 lb uplift at joint 12.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481244
L138285	T09	SPECIAL	1	2	Dwg.#1102051118 Job Reference (optional)	

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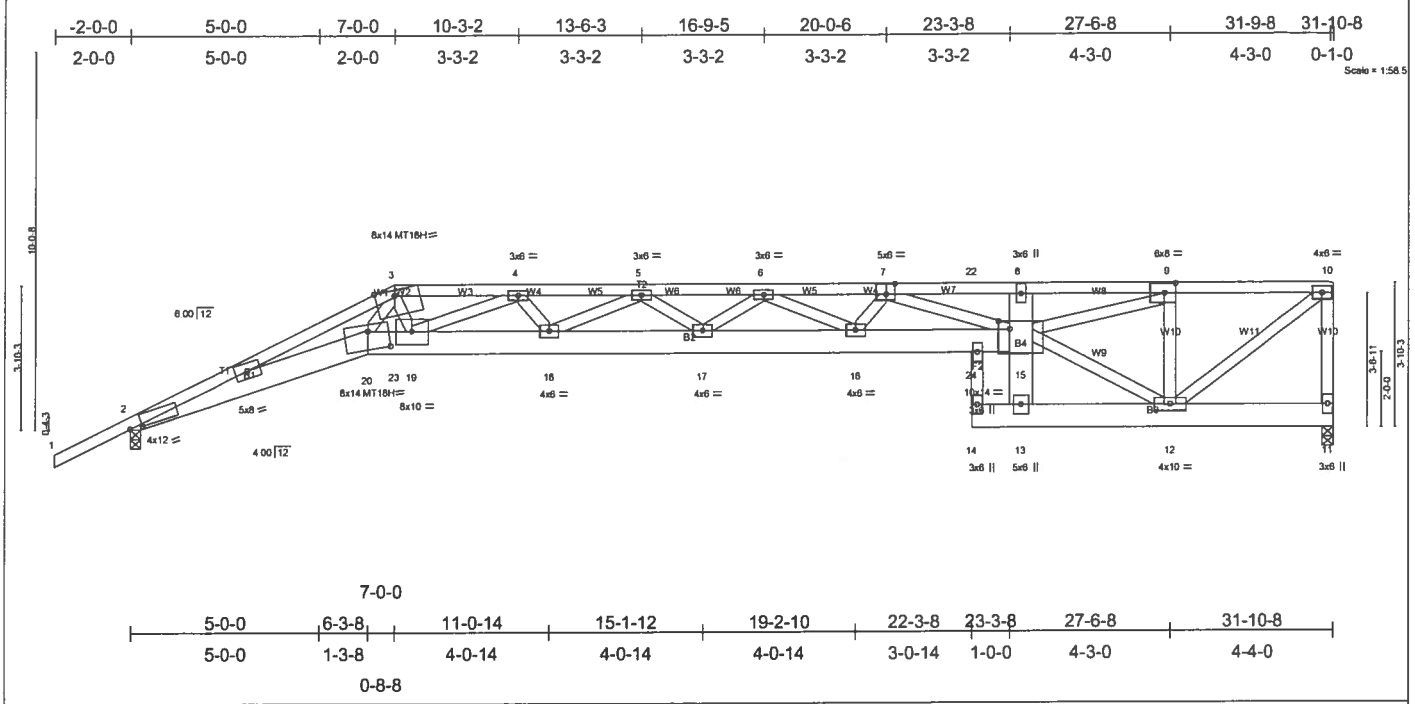

Plate Offsets (X,Y): [2:0-4-1,0-0-5], [3:0-6-3,Edge], [7:0-2-12,0-3-4], [9:0-3-8,0-3-0], [15:0-3-8,0-2-8], [20:0-6-8,0-5-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-1.01	17	>376	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-1.61	17	>235	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.75	Horz(TL)	0.78	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 447 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T3 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E *Except*
B3 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 4 SYP No.1D, W11 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=2863/0-3-8, 11=3063/0-3-8
Max Horz 2=228(load case 4)
Max Uplift2=-1023(load case 4), 11=-1222(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-12552/4510, 3-4=-12183/4495, 4-5=-15102/5614, 5-6=-16741/6264, 6-7=-16132/6078, 7-22=-12452/4729, 8-22=-12452/4729, 8-9=-10131/3872, 9-10=-3283/1285, 10-11=-2823/1187
BOT CHORD 2-20=-4227/11590, 20-23=-3949/10706, 19-23=-3948/10703, 18-19=-5417/14528, 17-18=-6104/16242, 16-17=-6289/16660, 16-24=-5891/15536, 15-24=-5891/15536, 13-15=-85/514, 8-15=-349/300, 13-14=0/0, 12-13=-1043/2690, 11-12=-48/83
WEBS 4-19=-2623/1097, 4-18=-399/1141, 5-18=-1315/586, 5-17=-237/657, 6-17=-23/106, 6-16=-609/243, 7-16=-372/1185, 7-15=-3315/1248, 12-15=-271/664, 9-15=-2745/7267, 9-12=-3036/1370, 10-12=-1593/4122, 3-19=-1726/4669, 3-20=-253/926

NOTES
1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
2) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1023 lb uplift at joint 2 and 1222 lb uplift at joint 11.
9) Girder carries tie-in span(s): 3-0-0 from 7-0-0 to 22-3-8; 7-0-0 from 7-0-0 to 22-3-8
10) Girder carries hip end with 0-0-0 right side setback, 22-3-8 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-22=-69(F=-15), 10-22=-118(F=-64), 2-20=-30, 20-23=-30, 23-24=-129(F=-99), 15-24=-65(F=-35), 13-14=-65(F=-35), 11-13=-65(F=-35)

Continued on page 2

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481244
L138285	T09	SPECIAL	1	2	Dwg.#1102051119 Job Reference (optional)	

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 23=-400(F)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
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Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481245
L138285	T10	SPECIAL	1	1	Dwg.#1102051120 Job Reference (optional)	

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-2-0-0	3-8-9	6-3-8	9-0-0	15-11-12	22-9-12	29-9-8	31-10-8
2-0-0	3-8-9	2-6-15	2-8-8	6-11-12	6-10-0	6-11-12	2-1-0

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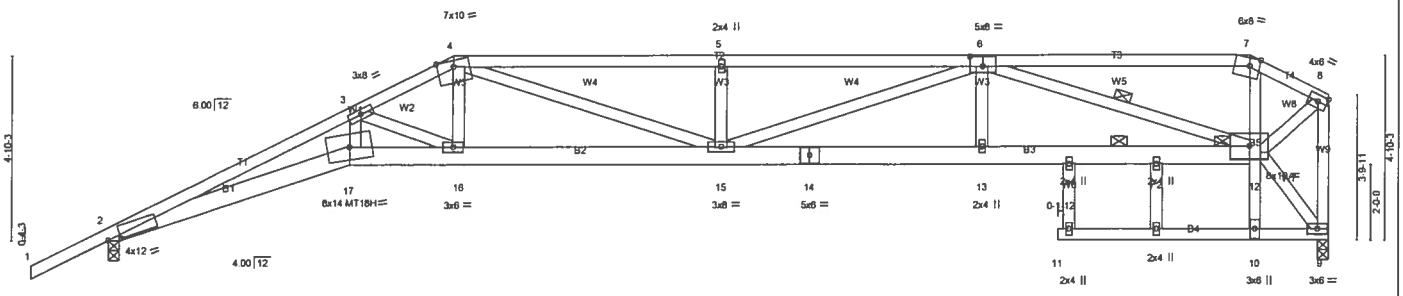


3-8-9	6-3-8	9-0-0	15-11-12	22-9-12	24-9-8	27-2-8	29-9-8	31-10-8
3-8-9	2-6-15	2-8-8	6-11-12	6-10-0	1-11-12	2-5-0	2-7-0	2-1-0

Plate Offsets (X,Y): [2:0-4-1,0-0-11], [6:0-4-0,0-3-0], [7:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	-0.77	11	>494	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.30	11	>292	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.57	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.3, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-7-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
1 Row at midpt 12-13
WEBS 1 Row at midpt 6-12
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 2=1468/0-3-8, 9=1456/0-3-8
Max Horz 2=241(load case 5)
Max Uplift2=-497(load case 5), 9=-399(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-6098/2395, 3-4=-3985/1610, 4-5=-4389/1736, 5-6=-4389/1736, 6-7=-1336/427, 7-8=-1410/381, 8-9=-1685/449
BOT CHORD 2-17=-2317/5634, 16-17=-2144/5162, 15-16=-1452/3636, 14-15=-1357/3705, 13-14=-1357/3705, 12-13=-1357/3703, 10-12=0/319, 7-12=0/347, 10-11=0/0, 9-10=-118/0
WEBS 3-17=-530/1518, 3-16=-1739/782, 4-16=-431/1182, 4-15=-395/948, 5-15=-367/271, 6-15=-350/734, 6-13=0/208, 6-12=-2514/1038, 8-12=-428/1661, 9-12=0/158

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 497 lb uplift at joint 2 and 399 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

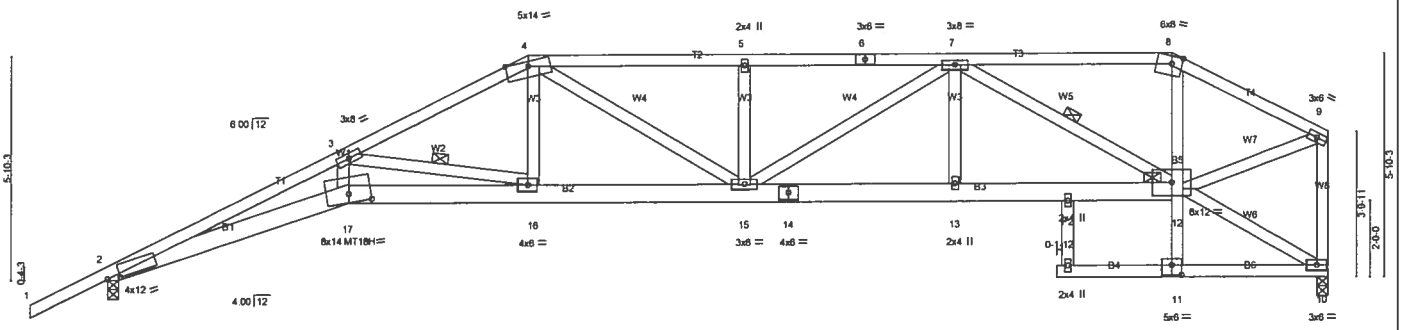
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T11	SPECIAL	1	1	Dwg.#1102051121

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:45 2005 Page 1

-2-0-0	3-8-9	6-3-8	11-0-0	16-7-12	22-1-12	27-9-8	31-10-8
2-0-0	3-8-9	2-6-15	4-8-8	5-7-12	5-6-0	5-7-12	4-1-0

Scale = 1:57.6



3-8-9	6-3-8	11-0-0	16-7-12	22-1-12	24-9-8	27-2-8	27-9-8	31-10-8
3-8-9	2-6-15	4-8-8	5-7-12	5-6-0	2-7-12	2-5-0	0-7-0	4-1-0

Plate Offsets (X,Y): [2:0-4-1,0-0-11], [8:0-3-3,Edge], [11:0-3-0,0-3-0], [17:0-7-0,0-2-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.55	16-17	>694	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.88	16-17	>433	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.50	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 207 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.3, B6 2 X 4 SYP No.2, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-16, 7-12
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 2=1446/0-3-8, 10=1323/0-3-8
Max Horz 2=255(load case 5)
Max Uplift2=-518(load case 5), 10=-371(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-5980/2466, 3-4=-3153/1329, 4-5=-3010/1301, 5-6=-3010/1301, 6-7=-3010/1301, 7-8=-1332/622, 8-9=-1517/638, 9-10=-1264/554
BOT CHORD 2-17=-2387/5525, 16-17=-2360/5478, 15-16=-1160/2833, 14-15=-1022/2595, 13-14=-1022/2595, 12-13=-1022/2595, 11-12=0/57, 8-12=-81/426, 10-11=-6/21
WEBS 3-17=-574/1622, 3-16=-2742/1240, 4-16=-338/956, 4-15=-223/386, 5-15=-297/214, 7-15=-210/509, 7-13=0/194, 7-12=-1495/584, 9-12=-545/1393, 10-12=-13/9

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 518 lb uplift at joint 2 and 371 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

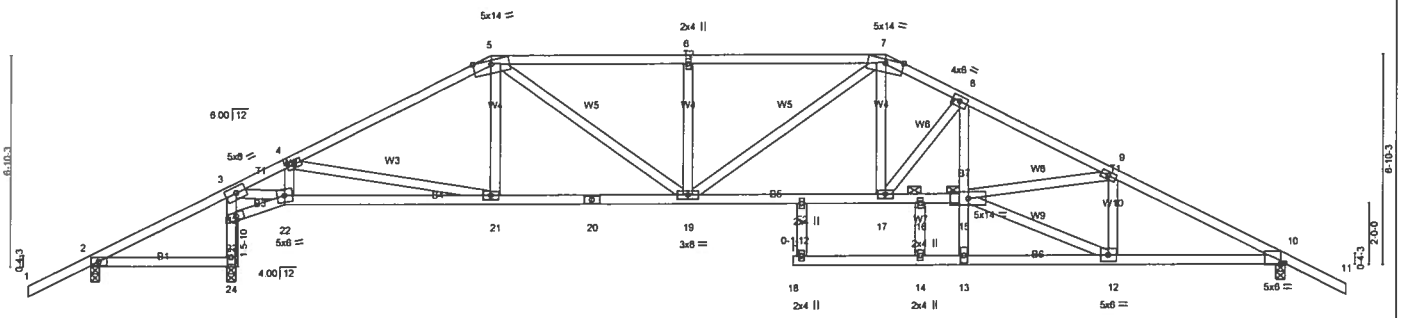
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T12	SPECIAL	1	1	Dwg.#1102051122

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:45 2005 Page 1

-2-0-0	4-8-7	6-3-8	13-0-0	19-4-12	25-9-8	28-2-8	33-2-12	38-9-8	40-9-8
2-0-0	4-8-7	1-7-1	6-8-8	6-4-12	6-4-12	2-5-0	5-0-4	5-6-12	2-0-0

Scale = 1:71.7



4-8-7	6-3-8	13-0-0	19-4-12	22-9-8	25-9-8	26-9-8	33-2-12	38-9-8
4-8-7	1-7-1	6-8-8	6-4-12	3-4-12	3-0-0	1-0-0	5-0-4	5-6-12

28-2-8

1-5-0

Plate Offsets (X,Y): [10:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.51	18	>803	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.86	18	>478	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.19	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B7 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-1-15 oc bracing.
JOINTS 1 Brace at Jt(s): 15, 16

REACTIONS (lb/size) 2=122/0-3-8, 24=1870/0-3-8, 10=1598/0-3-8
Max Horz 2=-129(load case 6)
Max Uplift2=-213(load case 5), 24=-552(load case 5), 10=-556(load case 6)
Max Grav 2=141(load case 9), 24=1870(load case 1), 10=1598(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/455, 3-4=-1632/668, 4-5=-2318/896, 5-6=-2726/1094, 6-7=-2726/1094, 7-8=-3010/1104, 8-9=-3784/1327, 9-10=-2802/1030, 10-11=0/47
BOT CHORD 2-24=-337/121, 23-24=-1842/669, 3-23=-1483/495, 22-23=-884/396, 21-22=-449/1529, 20-21=-463/2010, 19-20=-463/2010, 17-19=-624/2722, 16-17=-871/3325, 15-16=-868/3338, 13-15=-202/13, 8-15=-276/953, 14-18=0/0, 13-14=0/0, 12-13=-53/163, 10-12=-749/2427
WEBS 3-22=-776/2306, 4-22=-628/348, 4-21=-162/601, 5-21=0/141, 5-19=-284/970, 6-19=-359/258, 7-19=-191/174, 7-17=-249/1049, 8-17=-998/399, 12-15=-759/2470, 9-15=-175/907, 9-12=-827/354, 14-16=0/439

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 552 lb uplift at joint 24 and 556 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

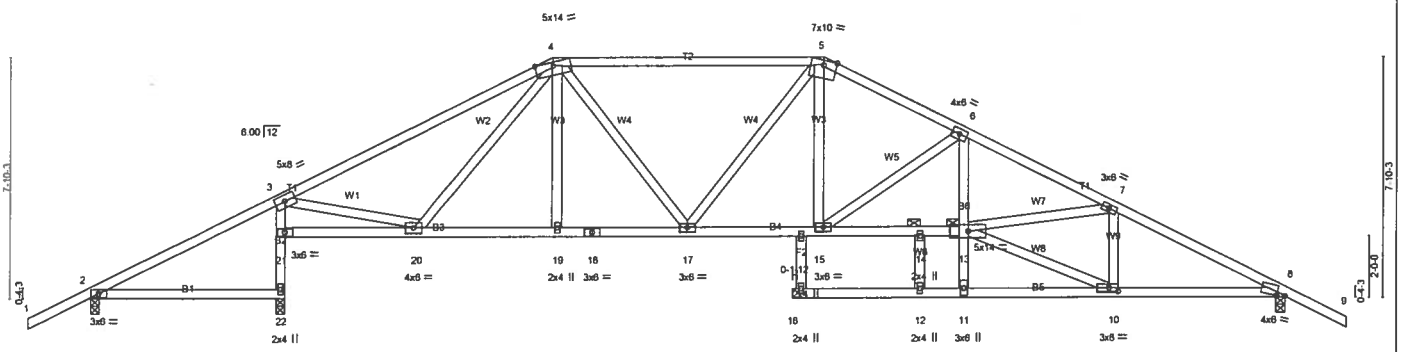
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T13	SPECIAL	1	1	Dwg.#1102051123

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:46 2005 Page 1

-2-0-0	6-3-8	10-7-12	15-0-0	19-4-12	23-9-8	28-2-8	33-2-12	38-9-8	40-9-8
2-0-0	6-3-8	4-4-4	4-4-4	4-4-12	4-4-12	4-5-0	5-0-4	5-6-12	2-0-0

Scale = 1:71.7



6-1-12	6-3-8	10-7-12	15-0-0	19-4-12	22-9-8	23-9-8	27-2-8	28-2-8	33-2-12	38-9-8
6-1-12	0-1-12	4-4-4	4-4-4	4-4-12	3-4-12	1-0-0	3-5-0	1-0-0	5-0-4	5-6-12

Plate Offsets (X,Y): [8:0-3-5,0-0-11], [10:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.52	16	>750	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.88	16	>442	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-0-13 oc bracing.
JOINTS 1 Brace at Jt(s): 13, 14

REACTIONS (lb/size) 2=323/0-3-8, 22=1723/0-3-8, 8=1544/0-3-8
Max Horz 2=-143(load case 6)
Max Uplift2=-282(load case 5), 22=-557(load case 5), 8=-563(load case 6)
Max Grav 2=332(load case 9), 22=1723(load case 1), 8=1544(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-50/262, 3-4=-1761/734, 4-5=-1974/858, 5-6=-2423/951, 6-7=-3642/1276, 7-8=-2683/1005, 8-9=0/47
BOT CHORD 2-22=-31/0, 21-22=-1633/604, 3-21=-1547/608, 20-21=-148/86, 19-20=-355/1686, 18-19=-354/1690, 17-18=-354/1690, 15-17=-455/2172, 14-15=-835/3215, 13-14=-830/3234, 11-13=-135/28, 6-13=-234/1075, 12-16=0/0, 11-12=0/0, 10-11=-68/99, 8-10=-727/2322
WEBS 3-20=-163/1473, 4-20=-329/102, 4-19=0/148, 4-17=-78/539, 5-17=-401/107, 5-15=-226/926, 6-15=-1297/469, 10-13=-719/2426, 7-13=-122/885, 7-10=-815/337, 12-14=0/377

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2, 557 lb uplift at joint 22 and 563 lb uplift at joint 8.

LOAD CASE(S) Standard

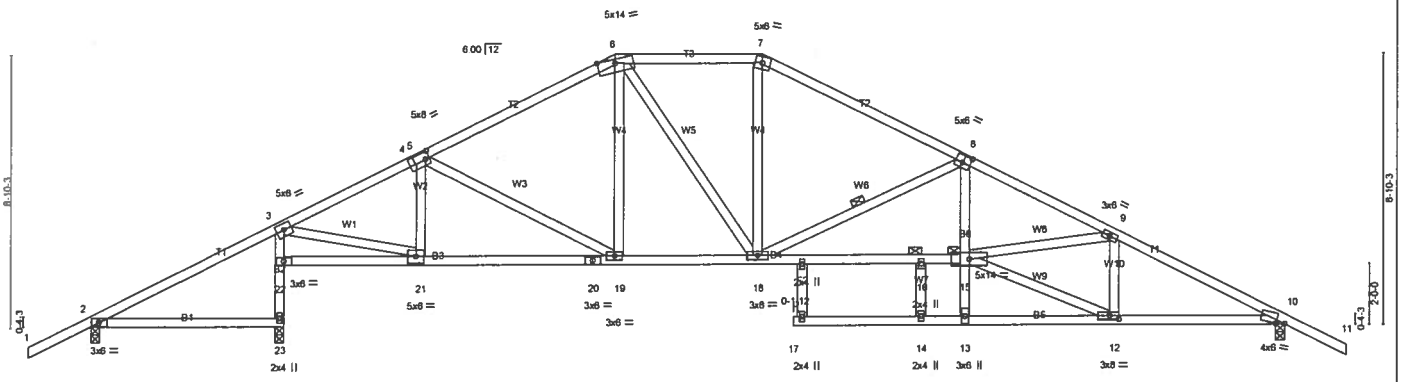
NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE	J1481249
L138285	T14	SPECIAL	1	1	Dwg.#1102051124 Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:47 2005 Page 1

-2-0-0	6-3-8	10-8-6	17-0-0	21-9-8	28-2-8	33-2-12	38-9-8	40-9-8
2-0-0	6-3-8	4-4-13	6-3-10	4-9-8	6-5-0	5-0-4	5-6-12	2-0-0

Scale = 1:71.7



6-1-12	6-3-8	10-8-6	17-0-0	21-9-8	22-9-8	28-1-15	28-2-8	33-2-12	38-9-8
6-1-12	0-1-12	4-4-13	6-3-10	4-9-8	1-0-0	5-4-6	0-0-10	5-0-4	5-6-12

Plate Offsets (X,Y): [4:0-1-12,0-3-0], [8:0-3-0,0-3-0], [10:0-3-5,0-0-11], [12:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.57	17	>686	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.97	17	>403	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.15	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 233 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-0-9 oc bracing.
WEBS 1 Row at midpt 8-18
JOINTS 1 Brace at Jt(s): 15, 16

REACTIONS (lb/size) 2=329/0-3-8, 23=1715/0-3-8, 10=1545/0-3-8
Max Horz 2=157(load case 5)
Max Uplift2=-279(load case 5), 23=-576(load case 5), 10=-576(load case 6)
Max Grav 2=340(load case 9), 23=1715(load case 1), 10=1545(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/277, 3-4=-1855/798, 4-5=-1745/799, 5-6=-1847/800, 6-7=-1815/842, 7-8=-2096/861, 8-9=-3681/1292, 9-10=-2681/1018, 10-11=0/47
BOT CHORD 2-23=-33/0, 22-23=-1623/603, 3-22=-1551/609, 21-22=-126/85, 20-21=-417/1610, 19-20=-417/1610, 18-19=-272/1588, 16-18=-887/3296, 15-16=-883/3317, 13-15=-100/50, 8-15=-211/1137, 14-17=0/0, 13-14=0/0, 12-13=-94/77, 10-12=-737/2319
WEBS 3-21=-517/1787, 5-21=-265/166, 5-19=-99/166, 6-19=-49/203, 6-18=-129/503, 7-18=-154/598, 8-18=-1662/625, 12-15=-701/2446, 9-15=-131/929, 9-12=-826/327, 14-16=-7/345

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 2, 576 lb uplift at joint 23 and 576 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

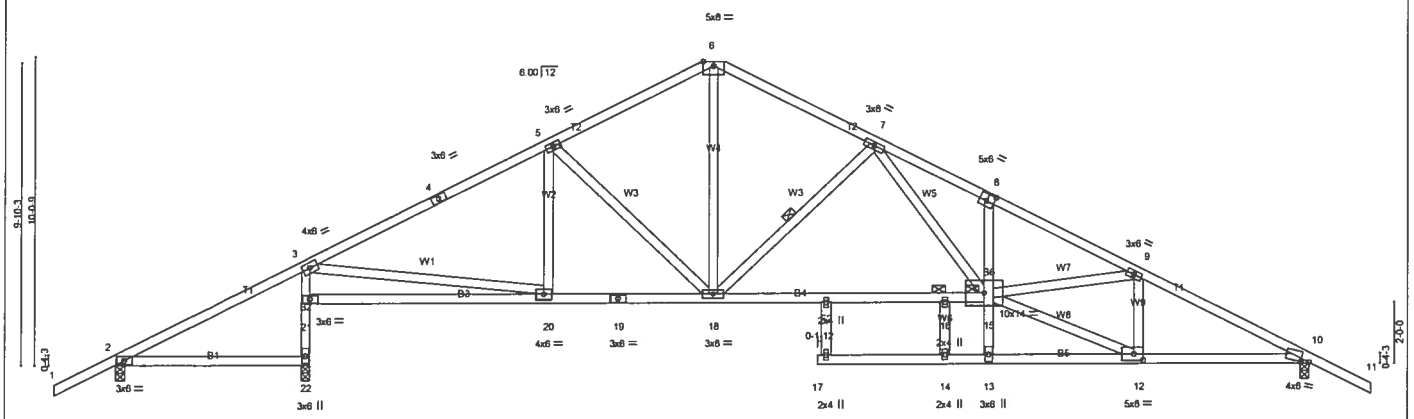
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T15	SPECIAL	1	1	Dwg.#1102051125

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:48 2005 Page 1

-2-0-0	6-3-8	14-0-5	19-0-0	19-9-8	24-7-8	28-2-8	33-2-12	38-9-8	40-9-8
2-0-0	6-3-8	7-8-13	4-11-11	0-9-8	4-10-0	3-7-0	5-0-4	5-6-12	2-0-0

Scale = 1:71.7



6-1-12	6-3-8	14-0-5	19-0-0	19-9-8	22-9-8	27-2-8	28-2-8	33-2-12	38-9-8
6-1-12	0-1-12	7-8-13	4-11-11	0-9-8	3-0-0	4-5-0	1-0-0	5-0-4	5-6-12

Plate Offsets (X,Y) [8:0-3-0,0-3-0], [10:0-3-5,0-0-11], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.76	17	>513	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-1.29	17	>303	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.15	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-15 oc bracing.
WEBS 1 Row at midpt 7-18
JOINTS 1 Brace at Jt(s): 15, 16

REACTIONS (lb/size) 2=332/0-3-8, 22=1712/0-3-8, 10=1546/0-3-8
Max Horz 2=174(load case 5)
Max Uplift2=-280(load case 6), 22=-600(load case 5), 10=-588(load case 6)
Max Grav 2=339(load case 9), 22=1712(load case 1), 10=1546(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-62/306, 3-4=-1991/827, 4-5=-1887/841, 5-6=-1729/805, 6-7=-1719/798, 7-8=-3702/1391, 8-9=-3675/1295, 9-10=-2681/1038, 10-11=0/47
BOT CHORD 2-22=-45/0, 21-22=-1619/612, 3-21=-1481/634, 20-21=-102/216, 19-20=-397/1688, 18-19=-397/1688, 16-18=-551/2257, 15-16=-551/2257, 13-15=0/88, 8-15=-222/209, 14-17=0/0, 13-14=0/0, 12-13=-134/0, 10-12=-756/2321
WEBS 3-20=-298/1500, 5-20=-54/85, 5-18=-350/273, 7-18=-1082/494, 7-15=-509/1734, 12-15=-747/2625, 9-15=-109/918, 9-12=-863/333, 6-18=-495/1203, 14-16=0/178

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 600 lb uplift at joint 22 and 588 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

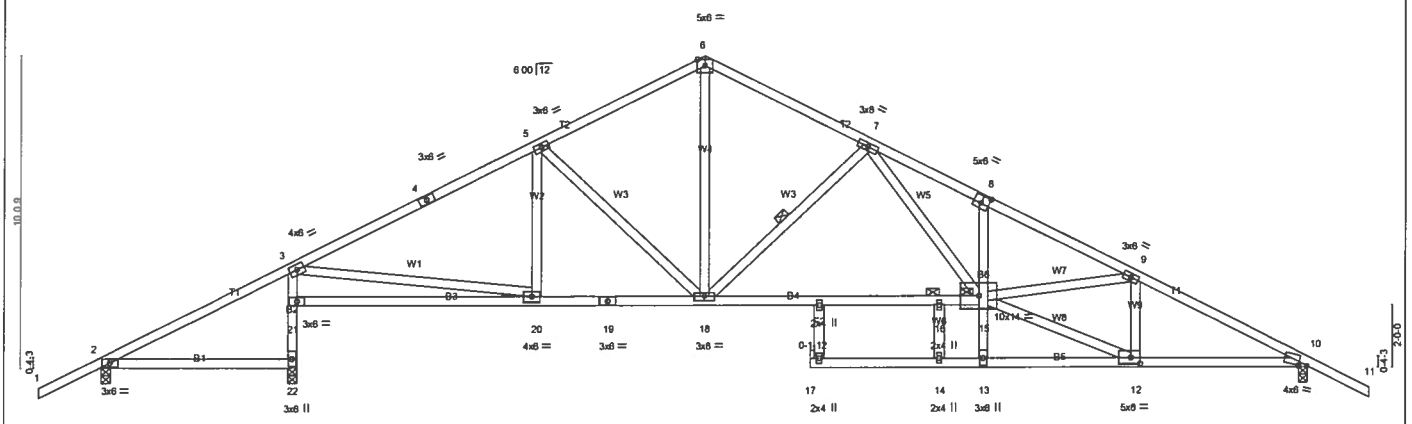
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T16	SPECIAL	1	1	Dwg.#1102051126

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:49 2005 Page 1

-2-0-0	6-3-8	14-0-3	19-4-12	24-7-7	28-2-8	33-2-12	38-9-8	40-9-8
2-0-0	6-3-8	7-8-11	5-4-9	5-2-12	3-7-1	5-0-4	5-6-12	2-0-0

Scale = 1:71.0



6-1-12	6-3-8	14-0-3	19-4-12	22-9-8	27-2-8	28-2-8	33-2-12	38-9-8
6-1-12	0-1-12	7-8-11	5-4-9	3-4-12	4-5-0	1-0-0	5-0-4	5-6-12

Plate Offsets (X,Y): [8:0-3-0,0-3-0], [10:0-3-5,0-0-11], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.76	17	>513	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-1.29	17	>303	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.15	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-15 oc bracing.
WEBS 1 Row at midpt 7-18
JOINTS 1 Brace at Jt(s): 15, 16

REACTIONS (lb/size) 2=332/0-3-8, 22=1712/0-3-8, 10=1546/0-3-8
Max Horz 2=174(load case 5)
Max Uplift2=-280(load case 6), 22=-600(load case 5), 10=-588(load case 6)
Max Grav 2=339(load case 9), 22=1712(load case 1), 10=1546(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-62/306, 3-4=-1991/827, 4-5=-1887/841, 5-6=-1729/805, 6-7=-1719/798, 7-8=-3702/1391, 8-9=-3675/1295, 9-10=-2681/1038, 10-11=0/47
BOT CHORD 2-22=-45/0, 21-22=-1619/612, 3-21=-1481/634, 20-21=-101/215, 19-20=-398/1688, 18-19=-398/1688, 16-18=-551/2256, 15-16=-551/2256, 13-15=0/88, 8-15=-222/210, 14-17=0/0, 13-14=0/0, 12-13=-134/0, 10-12=-756/2321
WEBS 3-20=-299/1501, 5-20=-55/85, 5-18=-350/273, 6-18=-494/1203, 7-18=-1082/493, 7-15=-509/1735, 12-15=-747/2625, 9-15=-109/918, 9-12=-863/333, 14-16=0/178

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2, 600 lb uplift at joint 22 and 588 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T17	SPECIAL	3	1	Dwg.#1102051127

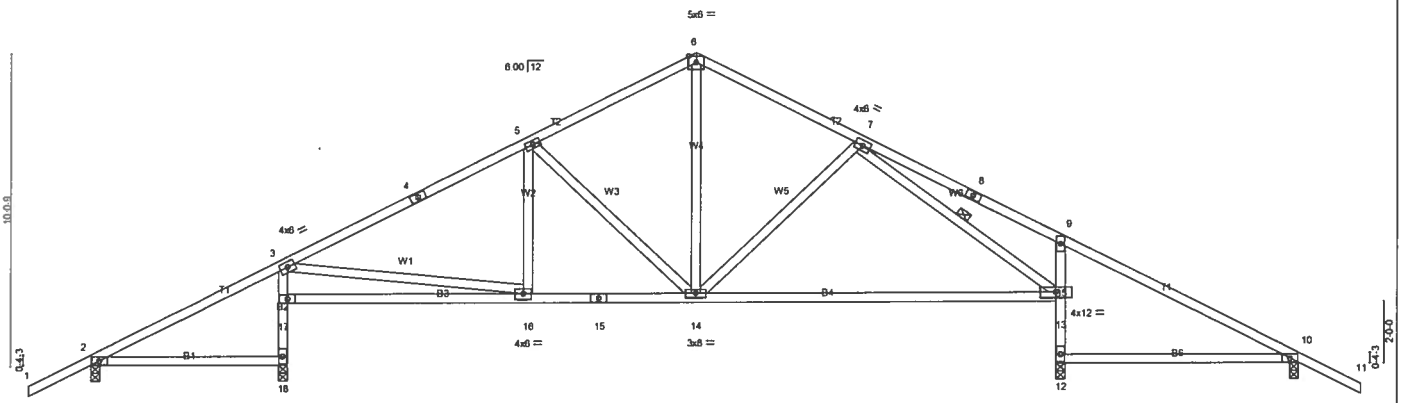
Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:50 2005 Page 1

-2-0-0 | 6-3-8 | 14-0-3 | 19-4-12 | 24-7-8 | 31-0-0 | 38-9-8 | 40-9-8

2-0-0 | 6-3-8 | 7-8-11 | 5-4-9 | 5-2-12 | 6-4-8 | 7-9-8 | 2-0-0

Scale = 1:71.0



6-1-12 | 6-3-8 | 14-0-3 | 19-4-12 | 31-0-0 | 38-9-8

6-1-12 | 0-1-12 | 7-8-11 | 5-4-9 | 11-7-4 | 7-9-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.16	10-12	>559	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.56	13-14	>532	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-1 oc bracing.
WEBS 1 Row at midpt 7-13

REACTIONS (lb/size) 2=355/0-3-8, 18=1306/0-3-8, 12=1414/0-3-8, 10=391/0-3-8
Max Horz 2=174(load case 5)
Max Uplift2=-287(load case 6), 18=-536(load case 5), 12=-464(load case 6), 10=-413(load case 6)
Max Grav 2=355(load case 1), 18=1306(load case 1), 12=1414(load case 1), 10=391(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-96/320, 3-4=-1354/663, 4-5=-1250/677, 5-6=-1023/613, 6-7=-1017/608, 7-8=-123/432, 8-9=-206/410, 9-10=-85/306, 10-11=0/47
BOT CHORD 2-18=-54/27, 17-18=-1209/499, 3-17=-1073/523, 16-17=-133/286, 15-16=-273/1120, 14-15=-273/1120, 13-14=-210/899, 12-13=-1283/492, 9-13=-472/376, 10-12=-92/2
WEBS 3-16=-156/842, 5-16=0/71, 5-14=-402/310, 6-14=-333/580, 7-14=-123/254, 7-13=-1010/248

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 3x6 MT20 unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 536 lb uplift at joint 18, 464 lb uplift at joint 12 and 413 lb uplift at joint 10.

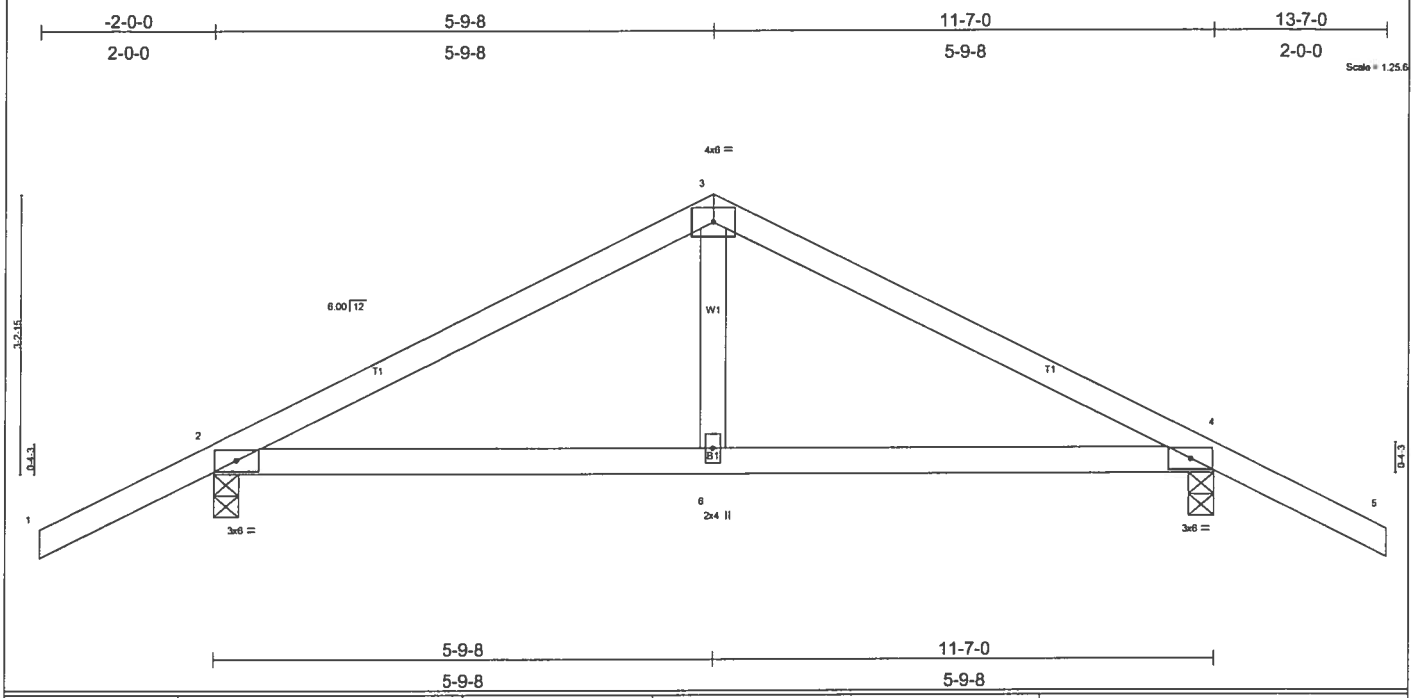
LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T18	COMMON	4	1	Dwg.#1102051128

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:51 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=590/0-3-8, 4=590/0-3-8
Max Horz 2=-79(load case 6)
Max Uplift2=-284(load case 5), 4=-284(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-630/247, 3-4=-630/247, 4-5=0/47
BOT CHORD 2-6=-66/501, 4-6=-66/501
WEBS 3-6=0/193

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 2 and 284 lb uplift at joint 4.

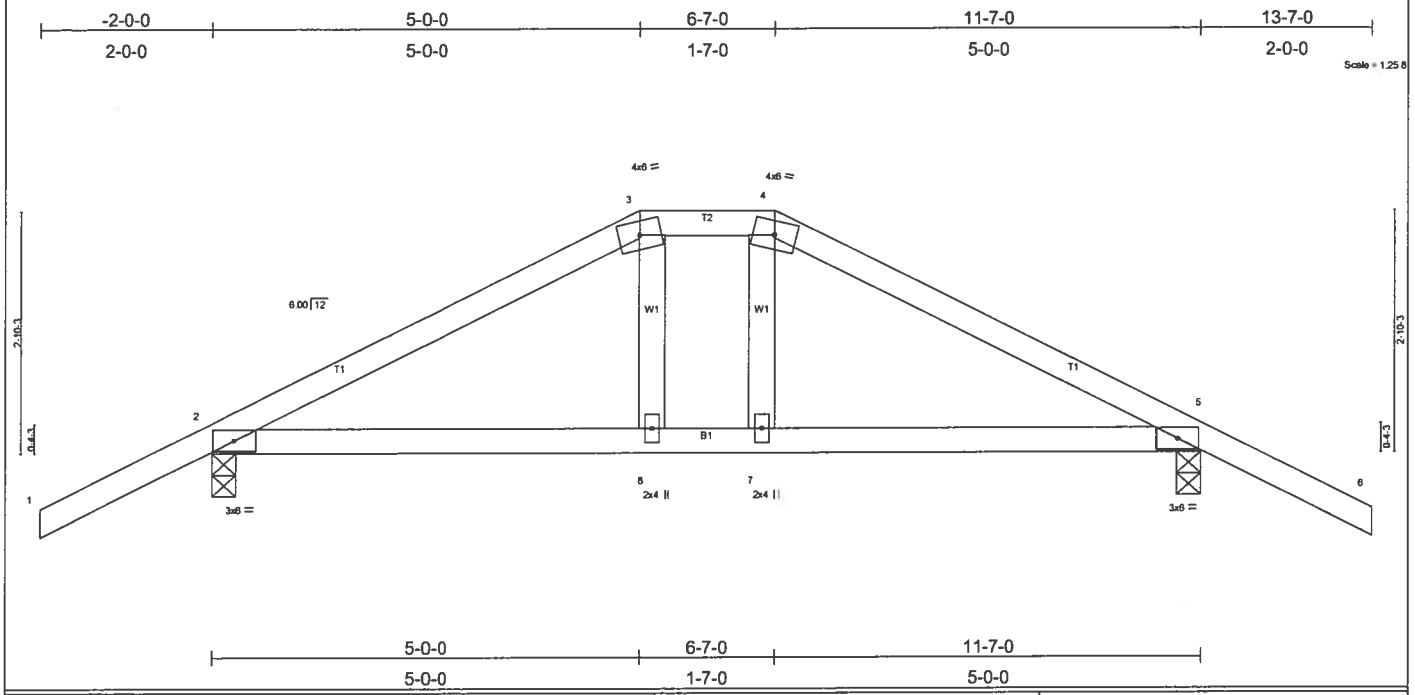
LOAD CASE(S) Standard

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T19H	HIP	1	1	Dwg.#1102051129

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:51 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.04	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.07	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=877/0-3-8, 5=877/0-3-8
Max Horz 2=73(load case 4)
Max Uplift2=-429(load case 4), 5=-429(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1252/470, 3-4=-1074/453, 4-5=-1252/469, 5-6=0/47
BOT CHORD 2-8=-342/1053, 7-8=-347/1074, 5-7=-340/1053
WEBS 3-8=-128/407, 4-7=-128/407

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 5.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 6-7-0, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T20H	HIP	1	1	Dwg.#1102051130

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:52 2005 Page 1

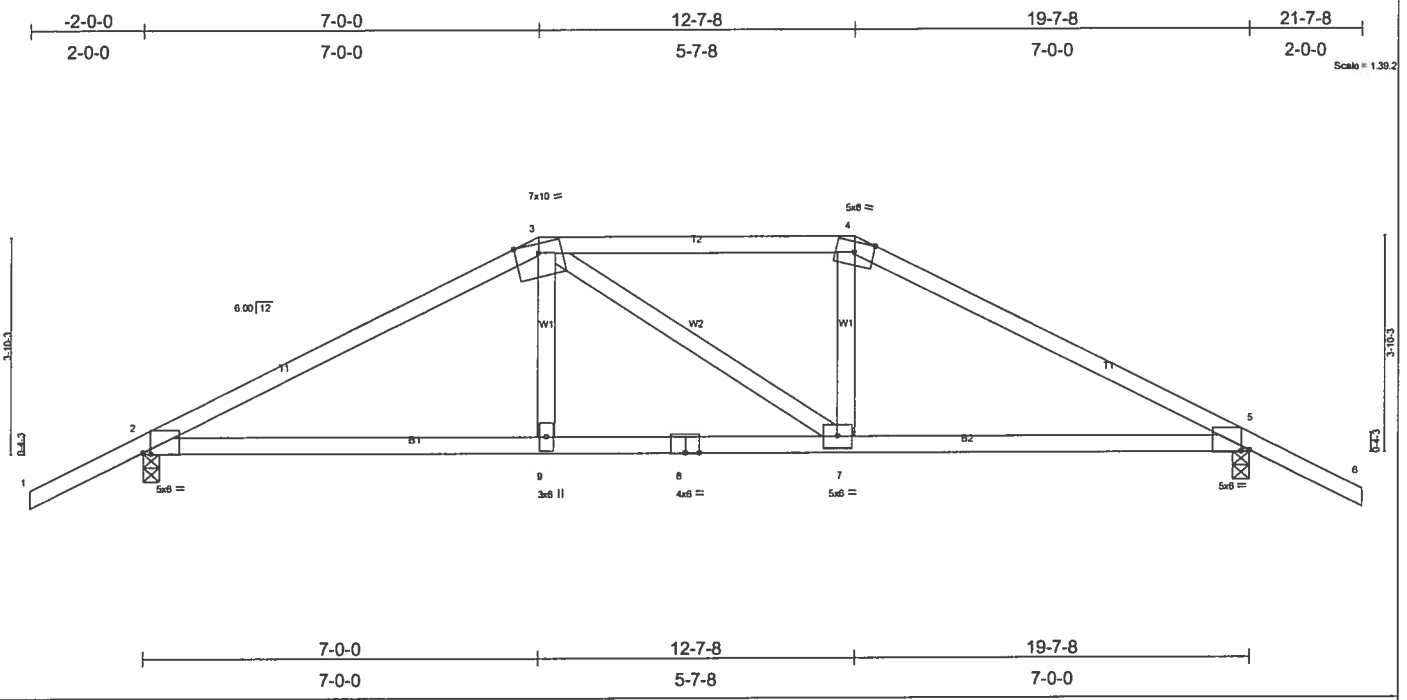


Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.15	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.24	7-9	>968	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.08	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 87 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-12 oc bracing.

REACTIONS (lb/size) 2=1740/0-3-8, 5=1740/0-3-8
Max Horz 2=-87(load case 5)
Max Uplift2=-793(load case 4), 5=-793(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3065/1243, 3-4=-2692/1184, 4-5=-3066/1243, 5-6=0/47
BOT CHORD 2-9=-1039/2656, 8-9=-1048/2691, 7-8=-1048/2691, 5-7=-1002/2658
WEBS 3-9=-224/836, 3-7=-123/126, 4-7=-246/890

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 793 lb uplift at joint 2 and 793 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 12-7-8, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-30, 7-9=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 9=-539(F) 7=-539(F)

NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE-LOT 72 EMERALD COVE
L138285	T21H	HIP	1	1	Dwg.#1102051131

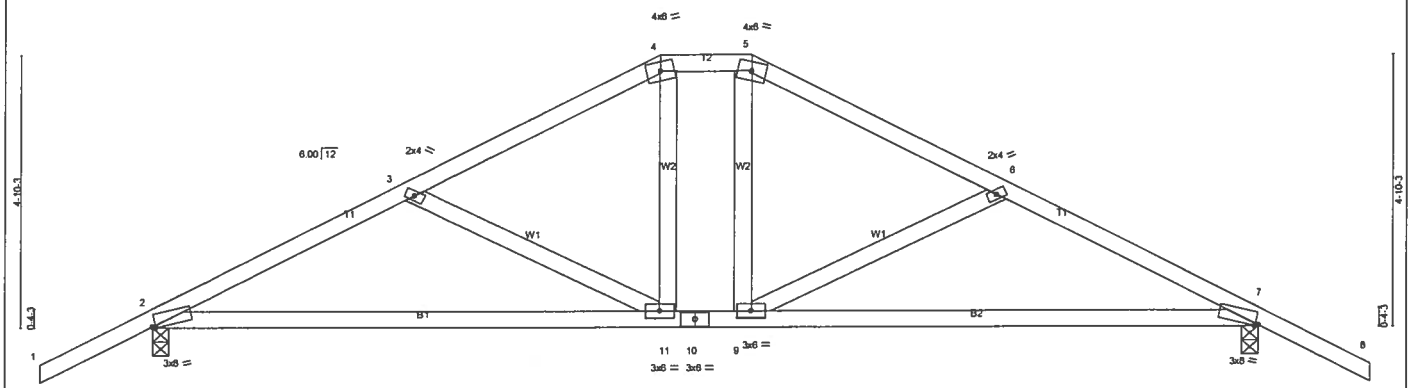
Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 01 11:23:53 2005 Page 1

-2-0-0 | 4-7-15 | 9-0-0 | 10-7-8 | 14-11-9 | 19-7-8 | 21-7-8

2-0-0 | 4-7-15 | 4-4-1 | 1-7-8 | 4-4-1 | 4-7-15 | 2-0-0

Scale = 1:39.2



9-0-0 | 10-7-8 | 19-7-8

9-0-0 | 1-7-8 | 9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.17	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.27	2-11	>855	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=928/0-3-8, 7=928/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-385(load case 5), 7=-385(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1304/553, 3-4=-1037/434, 4-5=-883/441, 5-6=-1037/434, 6-7=-1304/553, 7-8=0/47
BOT CHORD 2-11=-334/1129, 10-11=-138/883, 9-10=-138/883, 7-9=-334/1129
WEBS 3-11=-309/228, 4-11=-61/283, 5-9=-61/283, 6-9=-309/228

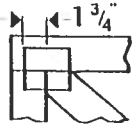
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 2 and 385 lb uplift at joint 7.

LOAD CASE(S) Standard

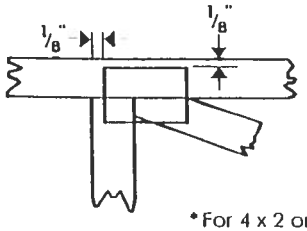
NOVEMBER 02,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

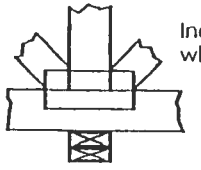
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

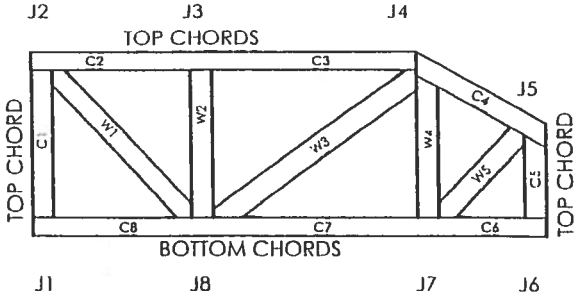
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.