

DATE 12/28/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023992

APPLICANT SUSAN HOLTON

PHONE 961-1086

ADDRESS 116 NW EGRET LANE

LAKE CITY

FL 32055

OWNER THOMAS EAGLE

PHONE 961-1086

ADDRESS 219 NW HERITAGE DR

LAKE CITY

FL 32055

CONTRACTOR DAVID MANGRUM

PHONE 752-4716

LOCATION OF PROPERTY 90W, TR ON BROWN RD, TL ON WINDING PLACE, TL ON

EMERALD LAKES DR., TR ON HERITAGE DR., TR ON RIGHT

TYPE DEVELOPMENT SFD, UTILITY

ESTIMATED COST OF CONSTRUCTION 110000.00

HEATED FLOOR AREA 2200.00

TOTAL AREA 2967.00

HEIGHT

STORIES 1

FOUNDATION CONC

WALLS FRAMED

ROOF PITCH 8/12

FLOOR SLAB

LAND USE & ZONING RSF-2

MAX. HEIGHT 31

Minimum Set Back Requirements: STREET-FRONT 25.00

REAR 15.00

SIDE 10.00

NO. EX.D.U. 0

FLOOD ZONE X PP

DEVELOPMENT PERMIT NO.

PARCEL ID 28-3S-16-02372-514

SUBDIVISION ARBOR GREEN @ EMERALD LAKES

TOTAL ACRES

000000930

RB29003100

Susan Holton

Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

CULVERT

05-0909-N

BK

LU & Zoning checked by

JH

Approved for Issuance

New Resident Y

COMMENTS: ONE FOOT ABOVE THE ROAD

Check # or Cash 1653

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power

date/app. by

Foundation

date/app. by

Monolithic

date/app. by

Under slab rough-in plumbing

date/app. by

Slab

date/app. by

Sheathing/Nailing

Framing

date/app. by

Heat & Air Duct

date/app. by

Peri. beam (Lintel)

Electrical rough-in

date/app. by

C.O. Final

date/app. by

Culvert

date/app. by

M/H tie downs, blocking, electricity and plumbing

date/app. by

Utility Pole

date/app. by

Pool

Reconnection

date/app. by

Pump pole

date/app. by

Re-roof

date/app. by

M/H Pole

INSPECTORS OFFICE

CLERKS OFFICE

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 679.68

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

BUILDING PERMIT FEE \$ 550.00 CERTIFICATION FEE \$ 14.84 SURCHARGE FEE \$ 14.84

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

COLUMBIA COUNTY INSPECTION SHEET

DATE 03/22/2007

TAKEN BY *JW*

INSPECTION DATE: 3.23.07

BUILDING PERMIT # 000023992 CULVERT / WAIVER PERMIT # 00000930 WAIVER

PARCEL ID # 28-3S-16-02372-514

ZONING RSF-2

TYPE OF DEVELOPMENT SFD, UTILITY

SETBACKS: FRONT 25.00 REAR 15.00 SIDE 10.00 HEIGHT 0.00

FLOOD ZONE X PP SEPTIC 05-0909-N NO. EXISTING D.U. 0

SUBDIVISION ARBOR GREEN @ EMERALD LAKES

Lot 14 Block Unit 0 Phase 1

OWNER THOMAS EAGLE

PHONE 961-1086

ADDRESS 219 NW HERITAGE DR LAKE CITY FL 32055

CONTRACTOR DAVID MANGRUM

PHONE ~~702-4715~~

LOCATION 90W, TR ON BROWN RD, TL ON WINDING PLACE, TL ON EMERALD LAKES DR., TR ON HERITAGE DR., 7TH ON RIGHT

COMMENTS: ONE FOOT ABOVE THE ROAD

WE HAVE NO A/C

INSPECTION(S) REQUESTED:

Temp Power

Foundation 02/17/2006 RJ

Setbacks 02/17/2006 RJ

Mono Slab

Under Slab Rough-in 05/01/2006 HD

Slab 05/03/2006 HD

Sheathing/Nailing 05/24/2006 hd

Framing 08/18/2006 RJ

Other

Above slab Rough-in 07/17/2006 RJ

Electrical Rough-in 07/17/2006 RJ

Heat & A/C 08/18/2006 RJ

Beam (Lintel)

Perm Power 12/13/2006 JK

CO Final

Culvert

Reconnection

Pool

MH Perm Power

Utility Pole

RV Power

Re-Roof

MH Pole

INSPECTORS:

APPROVED

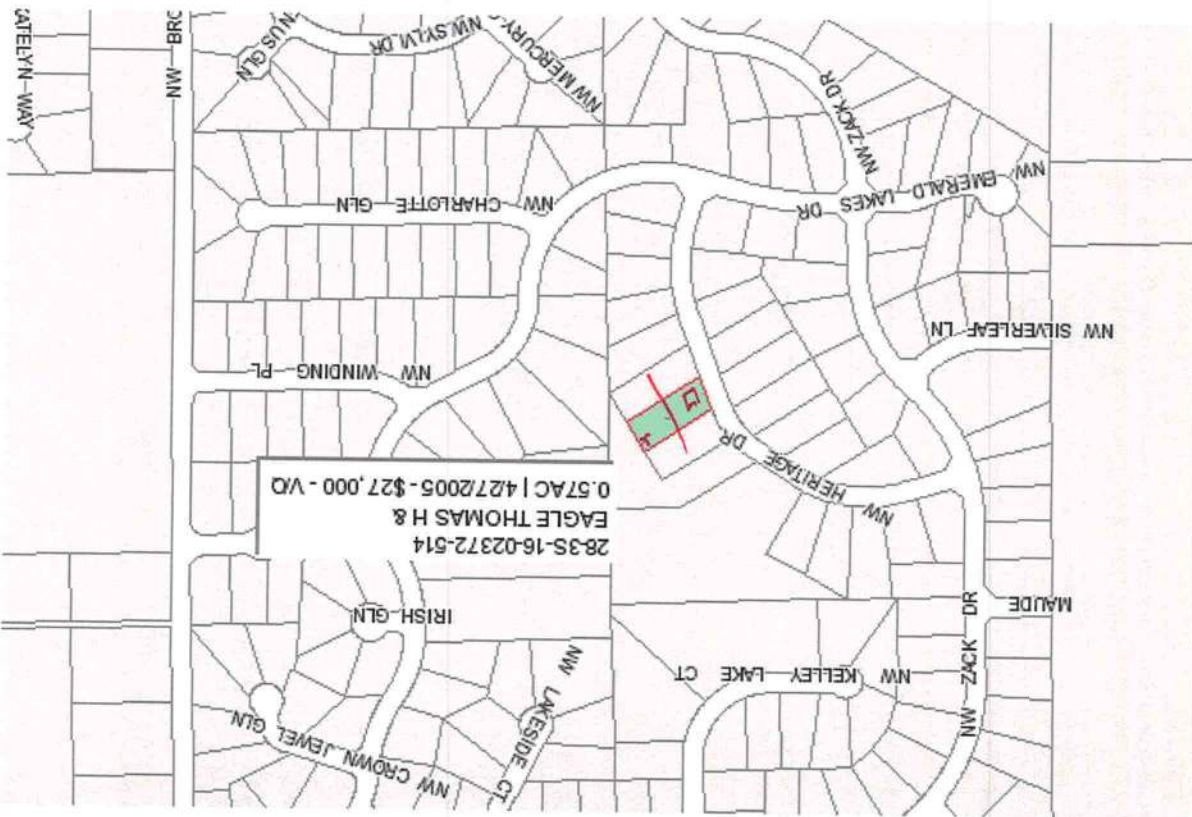
NOT APPROVED

BY

POWER CO. FPL

INSPECTORS COMMENTS:

23992



4/10/07
Thank you,
Gary Gill, P.E. #51942
Project Manager

Joe,
I inspected the culvert for the abovementioned project. As noted from our previous conversation, the culvert length installed does not agree with the length on the design drawings.
However, the installed culvert will still work properly for site conditions.

SUBJECT: Culvert Installation – Emerald Lakes Lot #14

Joe Haltwanger
135 NE Hernando Avenue
P. O. Box 1529
Lake City, Florida 32056-1529

April 10, 2007

GTC DESIGN GROUP



GTC Design Group, LLC
P.O. Box 187
Live Oak, FL 32064
(Phone) 386.362.3678
(Fax) 386.362.6133
ggill@gtcdesigngroup.com

4/10/07
Project Manager
Gary Gill, P.E. #51942

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ggill@gtcdesigngroup.com

Columbia County Property Appraiser

DB Last Updated: 3/8/2007

Parcel: 28-3S-16-02372-514

Tax Record

Property Card

Interactive GIS Map Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	EAGLE THOMAS H &		
Site Address	GREENE AT EMERALD LAKES		
Mailing Address	STEPHEN N KIRALY P O BOX 813 LAKE CITY, FL 32056		
Use Desc. (code)	SINGLE FAM (000100)		
Neighborhood	28316.05	Tax District	2
UD Codes	MKTA06	Market Area	06
Total Land Area	0.570 ACRES		
Description	LOT 14 ARBOR GREENE AT EMERALD LAKES S/D, ORB 964-2321, WD 1044-2552.		



GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (1)	\$32,000.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (1)	\$186,164.00
XFOB Value	cnt: (1)	\$3,732.00
Total Appraised Value		\$221,896.00

Just Value	\$221,896.00
Class Value	\$0.00
Assessed Value	\$221,896.00
Exempt Value	\$0.00
Total Taxable Value	\$221,896.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
4/27/2005	1044/2552	WD	V	Q		\$27,000.00
10/10/2002	964/2321	WD	V	Q		\$19,500.00

Building Characteristics

Bldg Item	Bldg Desc	Year Bilt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	2006	WD FR Stucco (16)	2324	3043	\$186,164.00

Note: All S.F. calculations are based on exterior building dimensions.

Extra Features & Out Buildings

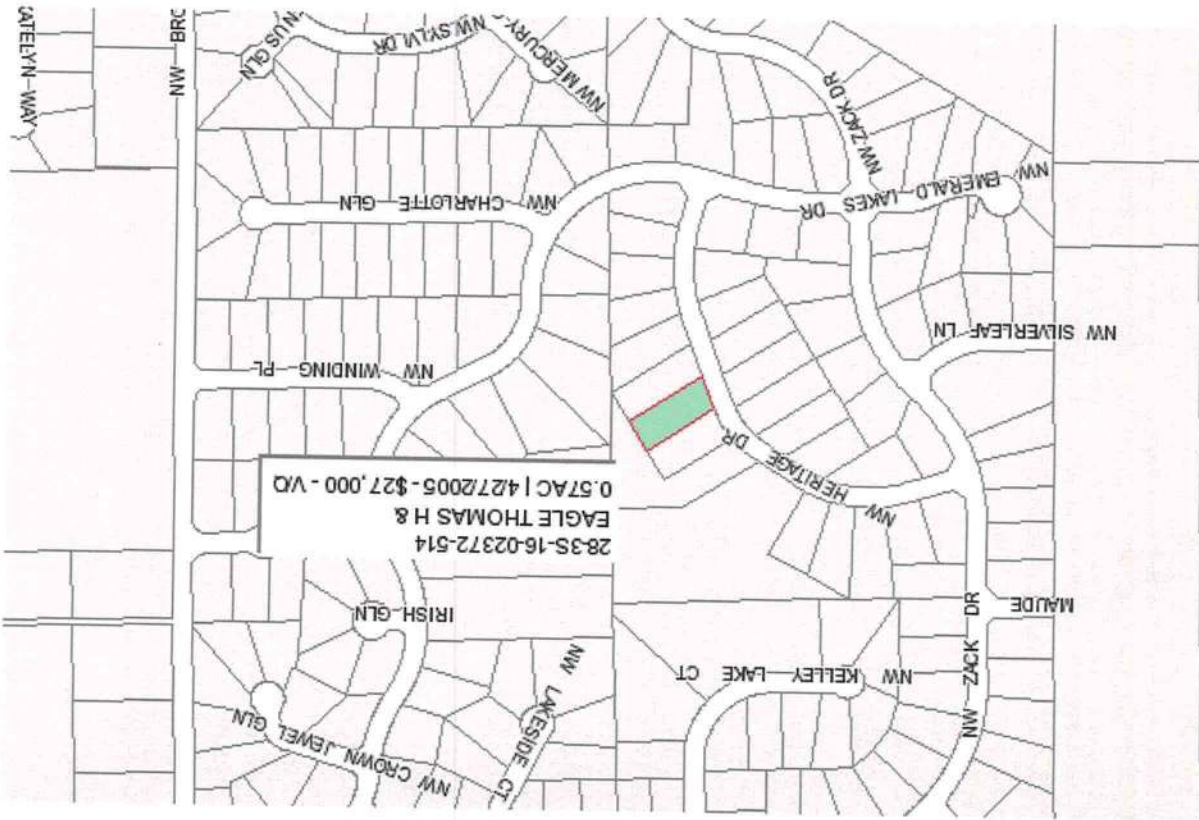
Code	Desc	Year Bilt	Value	Units	Dims	Condition (% Good)
0166	CONC,PAVMT	2006	\$3,732.00	1244.000	0 x 0 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	1.000 LT - (.570AC)	1.00/1.00/1.00/1.00	\$32,000.00	\$32,000.00

Columbia County Property Appraiser

DB Last Updated: 3/8/2007



Columbia County Building Permit Application *App# 1653*

For Office Use Only Application # 0508-111 Date Received 8-30-05 By LH Permit # 23992/929
 Application Approved by - Zoning Official BK Date 24/10/05 Plans Examiner DKSH Date 2-24-05
 Flood Zone Xp Development Permit N/A Zoning R5F-2 Land Use Plan Map Category RES. Low Den
 Comments NOC, ~~not~~ ~~to~~ ~~contact~~ ~~Env. Health~~ ~~10-25-05~~ ~~(LH)~~

Applicants Name Thomas H. Eagle Address 116 NW Egret Lane Lake City, FL 32055 Phone 386-961-1086
 Owners Name Thomas H. Eagle Address 219 NW Heritage Dr Lake City, FL 32055 Phone 386-752-4716
 Contractors Name David Mangrum Address 3809-A South First St Lake City, FL 32055 Phone 386-752-4716
 Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address _____

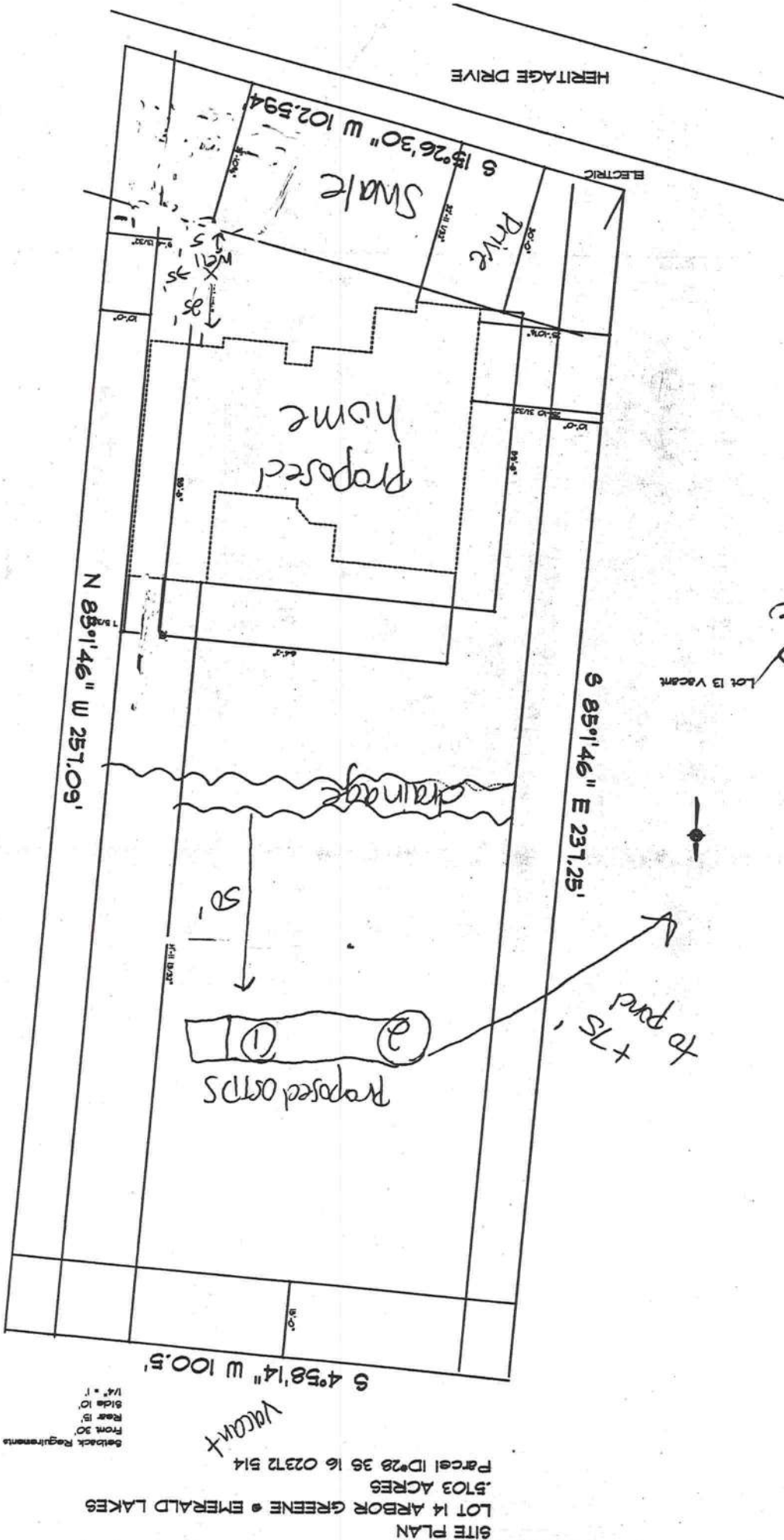
Architect/Engineer Name & Address GTC Design Group, LLC Live Oak, FL
 Mortgage Lenders Name & Address N/A

Circle the correct power company FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
 Property ID Number 28-35-16-62372-514 Estimated Cost of Construction 150,000
 Subdivision Name Arbor Green @ Emerald Lakes Lot 14 Block 1 Unit 1 Phase 1
 Driving Directions 90 west, right on Brown Rd, left on winding place left on Emerald Lakes Drive, right on Heritage Drive, 7th on right

Type of Construction Brick STD Number of Existing Dwellings on Property 0
 Total Acreage 1.2 Lot Size .5 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive
 Actual Distance of Structure from Property Lines - Front 32.11' Side 25.11' Side 10' Rear 151.11'
 Total Building Height 31' Number of Stories 1 Heated Floor Area 2200 Roof Pitch 8/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
 OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
 WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) _____
 STATE OF FLORIDA COUNTY OF COLUMBIA
 Sworn to (or affirmed) and subscribed before me this 27 day of July 2005
 Personally known or Produced Identification _____
 Notary Signature Susan L. Holton
 Commission # DD431203 Expires: MAY 19, 2009 www.AARONNOTARY.com
 Contractor Signature _____
 Contractors License Number RB29003100
 Competency Card Number _____
 NOTARY STAMP/SEAL
 Notary Signature _____



9/1/05
 X 258

REVISED
 12-16-05
 Steve Hader
 ES-COURT/BLK
APPROVED

Retention Pond

Lot 15 Vacant

Lot 13 Vacant

to height
 100' +
 well

to pond
 + 75'

05-0909-V

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 05-378
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number R02372-514

WARRANTY DEED

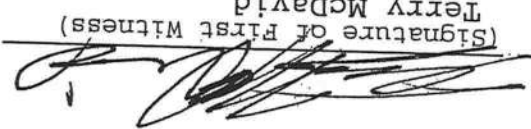
This Warranty Deed, made this 27th day of April, 2005, BETWEEN GARY GILL and GWENDOLYN GILL, Husband and wife whose post office address is 8878 CR 417, Live Oak, FL 32060, of the County of Suwannee, State of Florida, grantor*, and THOMAS H. EAGLE and STEPHEN N. KIRALY, each as to an undivided 1/2 interest, whose post office address is 114 NW Egret Lane, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

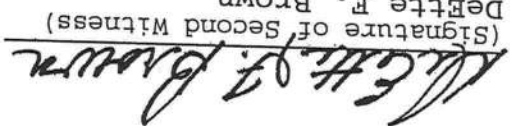
Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 14, ARBOR GREENE AT EMERALD LAKES, a subdivision according

Signed, sealed and delivered
in our presence:


Terry McDavid
(Signature of First Witness)

(Typed Name of First Witness)


DeEtte F. Brown
(Signature of Second Witness)

(Typed Name of Second Witness)

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 27th day of April, 2005, by GARY GILL and GWENDOLYN GILL, Husband and Wife who are personally known to me or who have produced as identification and who did not take an oath.

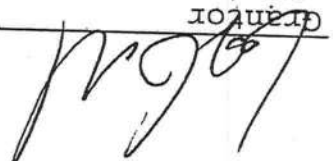
My Commission Expires:



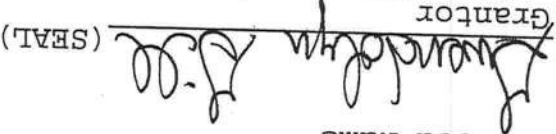
Notary Public
Printed, typed, or stamped name:



GARY GILL
Printed Name

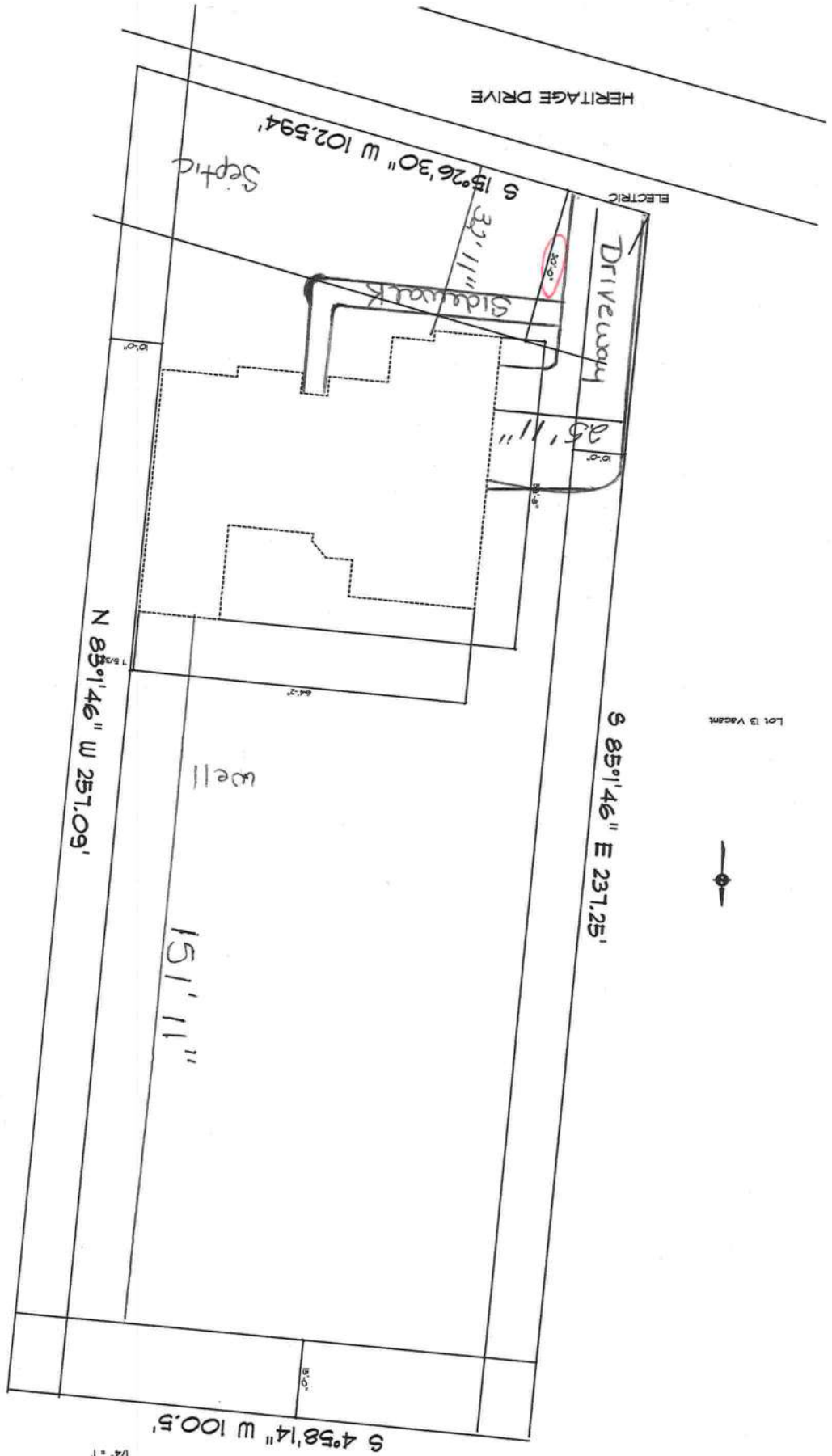

Grantor
(SEAL)

GWENDOLYN GILL
Printed Name


Grantor
(SEAL)

SITE PLAN
 LOT 14 ARBOR GREENE • EMERALD LAKES
 .5103 ACRES

Setback Requirements
 Front 30'
 Rear 10'
 Side 10'
 1/4" = 1'



Lot 15 Vacant

LOT 14 ARBOR GREENE @ EMERALD LAKES

DRIVING DIRECTIONS

HWY 90 WEST, RIGHT ON BROWN ROAD, LEFT INTO EMERALD LAKES
ENTRANCE ON WINDING PLACE. GO TO STOP SIGN AND LEFT ONTO
EMERALD LAKES DRIVE. RIGHT ON HERITAGE DRIVE. PROPERTY ON
RIGHT BEFORE RETENTION POND.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	Mackenzie Residence
Address:	Lot: 14, Sub: Arbor Greene, Plat:
City, State:	Lake City, FL
Owner:	Gateway Development, LLC
Climate Zone:	North
Builder:	(Columba)
Permitting Office:	23992
Permit Number:	221000
Jurisdiction Number:	221000

1. New construction or existing	Single family or multi-family	1	Single family	—
2. Single family or multi-family	Number of units, if multi-family	4	Single family	—
3. Number of Bedrooms	Is this a worst case?	Yes	—	—
4. Is this a worst case?	Conditioned floor area (ft ²)	2200 ft ²	—	—
5. Conditioned floor area (ft ²)	Glass area & type	—	—	—
6. Glass area & type	a. Clear glass, default U-factor	0.0 ft ²	—	—
7. a. Clear glass, default U-factor	b. Default tint, default U-factor	0.0 ft ²	—	—
8. a. Clear glass, default U-factor	c. Labeled U-factor or SHGC	0.0 ft ²	—	—
9. a. Clear glass, default U-factor	b. Labeled U-factor or SHGC	0.0 ft ²	—	—
10. a. Clear glass, default U-factor	c. Labeled U-factor or SHGC	0.0 ft ²	—	—
11. a. Clear glass, default U-factor	8. Floor types	—	—	—
12. a. Clear glass, default U-factor	a. Stab-On-Grade Edge Insulation	R=0.0, 253.0(p) ft	—	—
13. a. Clear glass, default U-factor	b. N/A	—	—	—
14. a. Clear glass, default U-factor	c. N/A	—	—	—
15. a. Clear glass, default U-factor	a. Hot water systems	R=13.0, 1870.0 ft ²	—	—
16. a. Clear glass, default U-factor	b. N/A	—	—	—
17. a. Clear glass, default U-factor	c. N/A	—	—	—
18. a. Clear glass, default U-factor	a. Electric Resistance	R=30.0, 2422.0 ft ²	—	—
19. a. Clear glass, default U-factor	b. N/A	—	—	—
20. a. Clear glass, default U-factor	c. N/A	—	—	—
21. a. Clear glass, default U-factor	a. Conservation credits	—	—	—
22. a. Clear glass, default U-factor	(HR-Heat recovery, Solar	—	—	—
23. a. Clear glass, default U-factor	DHP-Dedicated heat pump)	—	—	—
24. a. Clear glass, default U-factor	(CF-Ceiling fan, CV-Cross ventilation,	—	—	—
25. a. Clear glass, default U-factor	HF-Whole house fan,	—	—	—
26. a. Clear glass, default U-factor	PT-Programmable Thermostat,	—	—	—
27. a. Clear glass, default U-factor	MZ-C-Multizone cooling,	—	—	—
28. a. Clear glass, default U-factor	MZ-H-Multizone heating)	—	—	—

Glass/Floor Area: 0.11
Total as-built points: 29812
Total base points: 34425
PASS

DATE:	7/15/03
PREPARED BY:	Gary Gill
OWNER/AGENT:	—
DATE:	—
BUILDING OFFICIAL:	—
DATE:	—



Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL, PERMIT #:

BASE		AS-BUILT	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		Type/SC Overhang Ornt Len Hgt Area X SPM X SOF = Points	
.18	2200.0	20.04	7935.8
Double, Clear	S 1.5	4.5	27.0
Double, Clear	S 7.0	6.3	20.0
Double, Clear	S 12.0	6.3	15.0
Double, Clear	S 8.0	6.7	47.3
Double, Clear	E 1.5	6.3	15.0
Double, Clear	E 1.5	4.5	9.0
Double, Clear	N 1.5	6.3	50.0
Double, Clear	N 1.5	6.3	19.20
Double, Clear	W 1.5	6.3	45.0
As-Built Total:			248.3
WALL TYPES		Area X BSPM = Points	
Adjacent	0.0	0.0	1870.0
Exterior	0.00	1.70	3179.0
Frame, Wood, Exterior		13.0	1870.0
Area X SPM = Points		1.50	2805.0
As-Built Total:			1870.0
DOOR TYPES		Area X BSPM = Points	
Adjacent	21.0	2.40	50.4
Exterior	36.8	6.10	224.2
Exterior Wood	21.0	6.10	128.1
Adjacent Wood	15.8	6.10	96.1
Adjacent Wood	21.0	2.40	50.4
As-Built Total:			57.8
CEILING TYPES		Area X BSPM = Points	
Under Attic	2200.0	1.73	3806.0
Under Attic	30.0	2422.0	1.73 X 1.00
As-Built Total:			2422.0
FLOOR TYPES		Area X BSPM = Points	
Slab	253.0(p)	-37.0	-9361.0
Raised	0.0	0.00	0.0
Slab-On-Grade Edge Insulation	0.0	253.0(p)	-41.20
As-Built Total:			253.0
INFILTRATION		Area X BSPM = Points	
	2200.0	10.21	22462.0
Area X SPM = Points			10.21
As-Built Total:			22462.0

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL, PERMIT #:

BASE	Summer Base Points: 28296.4					
	Total Summer X System = Cooling Points	Multiplier	X	System X Credit = Cooling Points		
	28296.4			25273.7		
AS-BUILT	Summer As-Built Points: 25273.7					
	Total X Cap X Duct X System X Credit = Cooling Points	Ratio	X	Component Ratio		
	25273.7	1.00	(1.090 x 1.147 x 0.91)	0.284		
	28296.4	0.4266		1.138	0.950	7769.2
	12071.3			1.00	0.950	7769.2
	28296.4			1.00	0.950	7769.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL, PERMIT #:

AS-BUILT		BASE			
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area		.18 2200.0 12.74 5045.0			
Type/SC Overhang Ornt Len Hgt Area X WPM X WOF = Points		Double, Clear S 1.5 4.5 27.0 13.30 1.26 451.8 S 7.0 6.3 20.0 13.30 2.90 772.1 S 12.0 6.3 15.0 13.30 3.48 694.8 S 8.0 6.7 47.3 13.30 3.05 1915.5 E 1.5 6.3 15.0 18.79 1.03 291.0 E 1.5 4.5 9.0 18.79 1.06 179.4 N 1.5 6.3 50.0 24.58 1.00 1231.4 N 1.5 6.3 20.0 24.58 1.00 492.6 W 1.5 6.3 45.0 20.73 1.02 952.2 As-Built Total: 248.3		Adjacent 0.0 0.0 3.70 6919.0 Exterior 1870.0 1870.0 6919.0 Base Total: 1870.0 Area X BWPM = Points	
Type R-Value Area X WPM = Points		Frame, Wood, Exterior 13.0 1870.0 3.40 6358.0 As-Built Total: 1870.0 Area X WPM = Points			
WALL TYPES Area X BWPM = Points		Adjacent 21.0 11.50 241.5 Exterior Wood 21.0 12.30 258.3 Exterior Wood 15.8 12.30 193.7 Adjacent Wood 21.0 11.50 241.5 Base Total: 57.8 Area X BWPM = Points			
CEILING TYPES Area X BWPM = Points		Under Attic 2.05 2200.0 4965.1 Under Attic 30.0 2422.0 2.05 X 1.00 4965.1 As-Built Total: 2422.0 Area X WCM = Points			
FLOOR TYPES Area X BWPM = Points		Slab 253.0(p) 8.9 2251.7 Slab-On-Grade Edge Insulation 0.0 253.0(p) 18.80 4756.4 Raised 0.0 0.00 0.0 Base Total: 2251.7 As-Built Total: 253.0 Area X WPM = Points			
INFILTRATION Area X BWPM = Points		Slab 253.0(p) 8.9 2251.7 Slab-On-Grade Edge Insulation 0.0 253.0(p) 18.80 4756.4 Raised 0.0 0.00 0.0 Base Total: 2251.7 As-Built Total: 253.0 Area X WPM = Points			
Area X WPM = Points -1298.0		Area X WPM = Points -1298.0			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL, PERMIT #:

AS-BUILT	BASE
Winter As-Built Points: 22455.8	Winter Base Points: 18121.3
Total X Cap X Duct X System X Credit = Heating Points Component Ratio Multiplier (DM x DSM x AHU)	Total Winter X System = Heating Points Multiplier
22455.8 1.00 (1.069 x 1.169 x 0.93) 0.487 1.162 0.950 12077.7	18121.3 0.6274 11369.3

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL, PERMIT #:

BASE	AS-BUILT
WATER HEATING Number of X Multiplier = Total Bedrooms	As-Built Total: 4 2746.00 10984.0
Tank EF Number of X Tank X Multiplier X Credit = Total Volume Bedrooms Ratio Multiplier	40.0 0.97 4 1.00 2491.22 1.00 9964.9 9964.9

CODE COMPLIANCE STATUS	
BASE	AS-BUILT
Cooling Points + Heating Points + Hot Water Points = Total 12071 11369 10984 34425	Cooling Points + Heating Points + Hot Water Points = Total 12078 9965 29812

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 14, Sub: Arbor Greene, Plat: , Lake City, FL,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: 3 cfm/sq.ft. window area; 5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Emerald Lakes Sub.
Tom Eagle

Thank you,
Donald D. Hall
Donald D. Hall
DDH/JK

If you have any questions, please feel free to call our office anytime.

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

NOTICE TO ALL CONTRACTORS

June 12, 2002

DONALD AND MARY HALL
OWNERS



SPECIALIZING IN 4"-6" WELLS

HALL'S PUMP & WELL SERVICE, INC.

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXXXXXXXXXXXXXXXXXX~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

April 7, 2006

Mr. John Colson: Consulting engineer for the County of Columbia County Florida
Donald Lee & Associates Inc.
950 Ridgewood Drive
Lake City, FL. 32024

Dear Sir:

This is a follow up of our discussion concerning lot 14 of Arbor Green @ Emerald Lake Subdivision. Thank you for reviewing the two drawings which detailed the culvert and retaining walls that were prepared by CTC Structural/Civil Engineers. Upon the on site inspection we conducted and your review of these drawings, per our phone conversation this date, the consensus is that the proposed retaining walls will protect the integrity of the single family dwelling structural foundations. Please notify me in writing by April 14, 2006 if you should have any additional concerns or corrections in regards to this matter. Again thank you for your professional engineering assessment.

Sincerely,

Joseph T. Haltiwanger

Joseph T. Haltiwanger
Plan reviewer
Columbia County Building Department



THE COLUMBIA COUNTY BUILDING DEPARTMENT
PLANS REVIEW
135 NE HERNANDO AVE.
P.O. BOX 1529
LAKE CITY FLORIDA, 32056-1526

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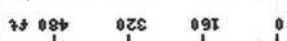
Dear Sir:

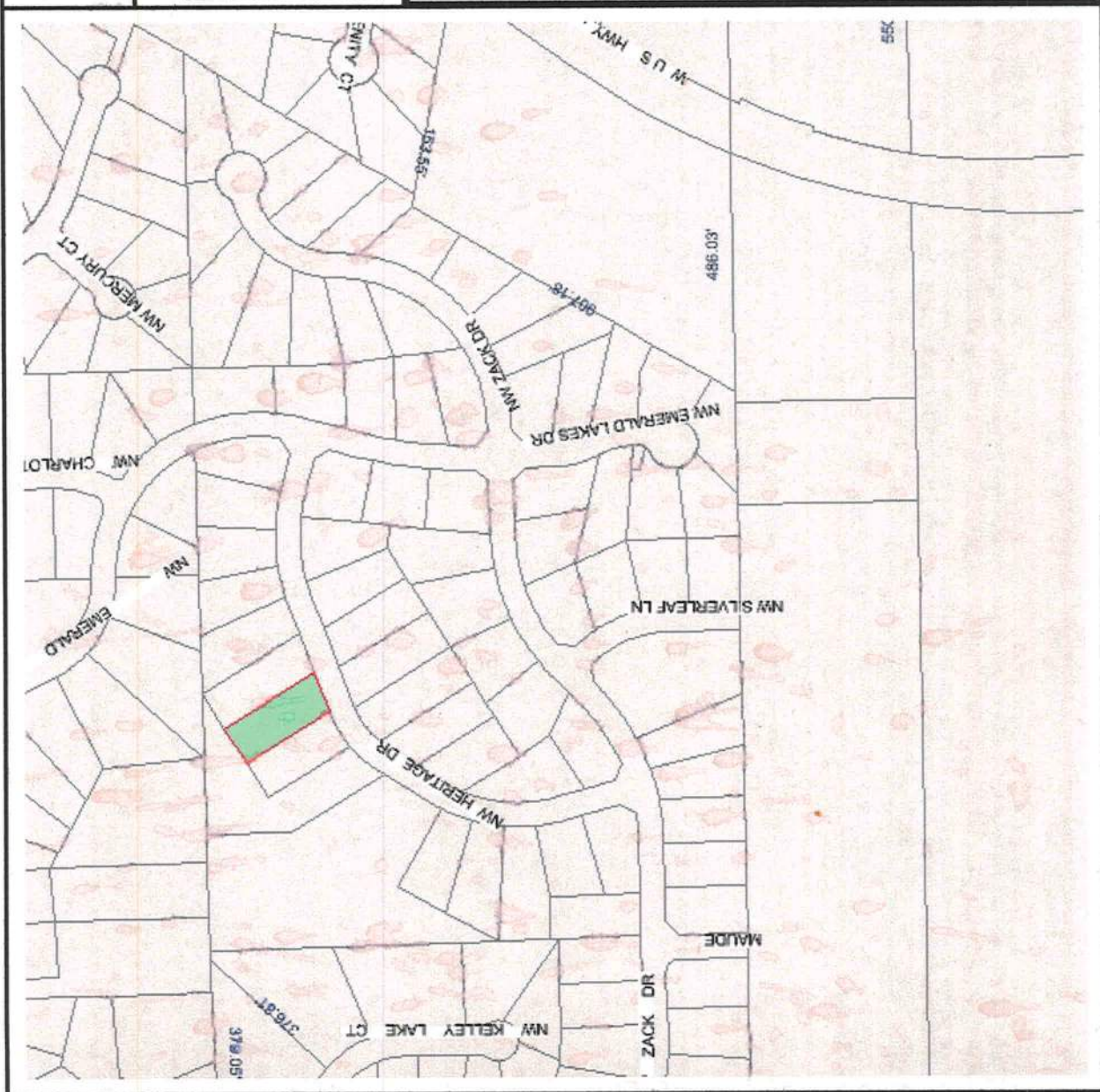
Mr. John Colson: Consulting engineer for the County of Columbia County Florida
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950 Ridgewood Drive
Lake City, FL. 32024

April 7, 2006

2 392

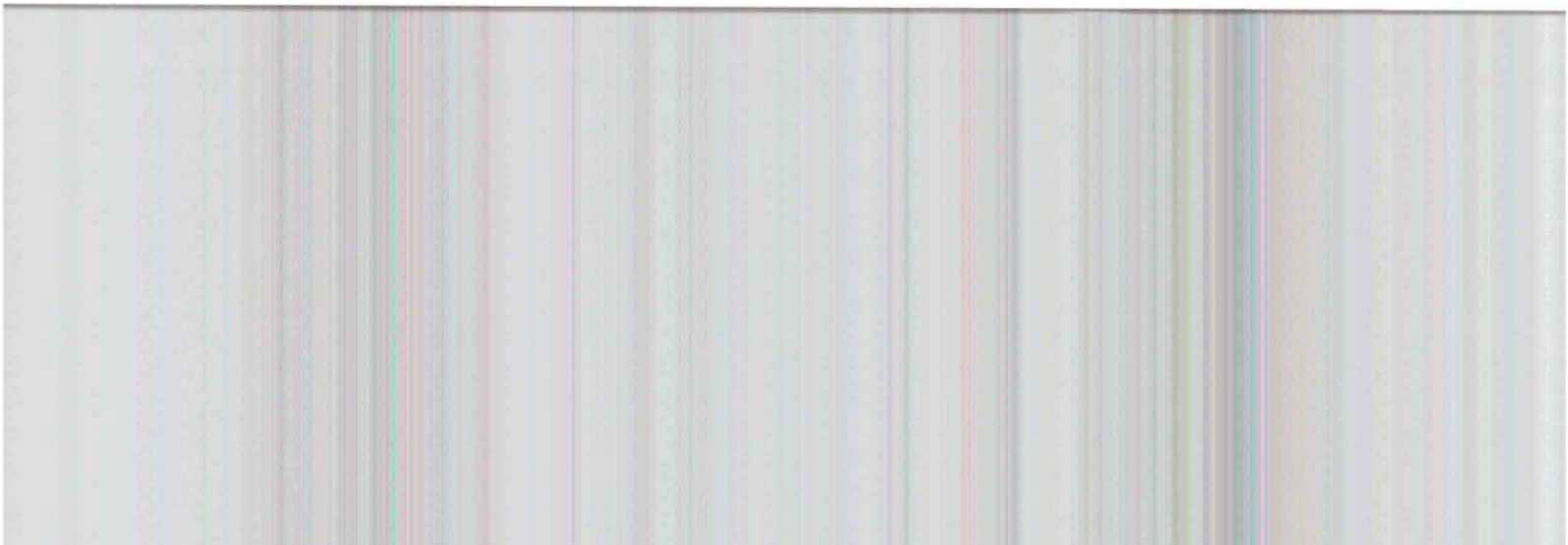
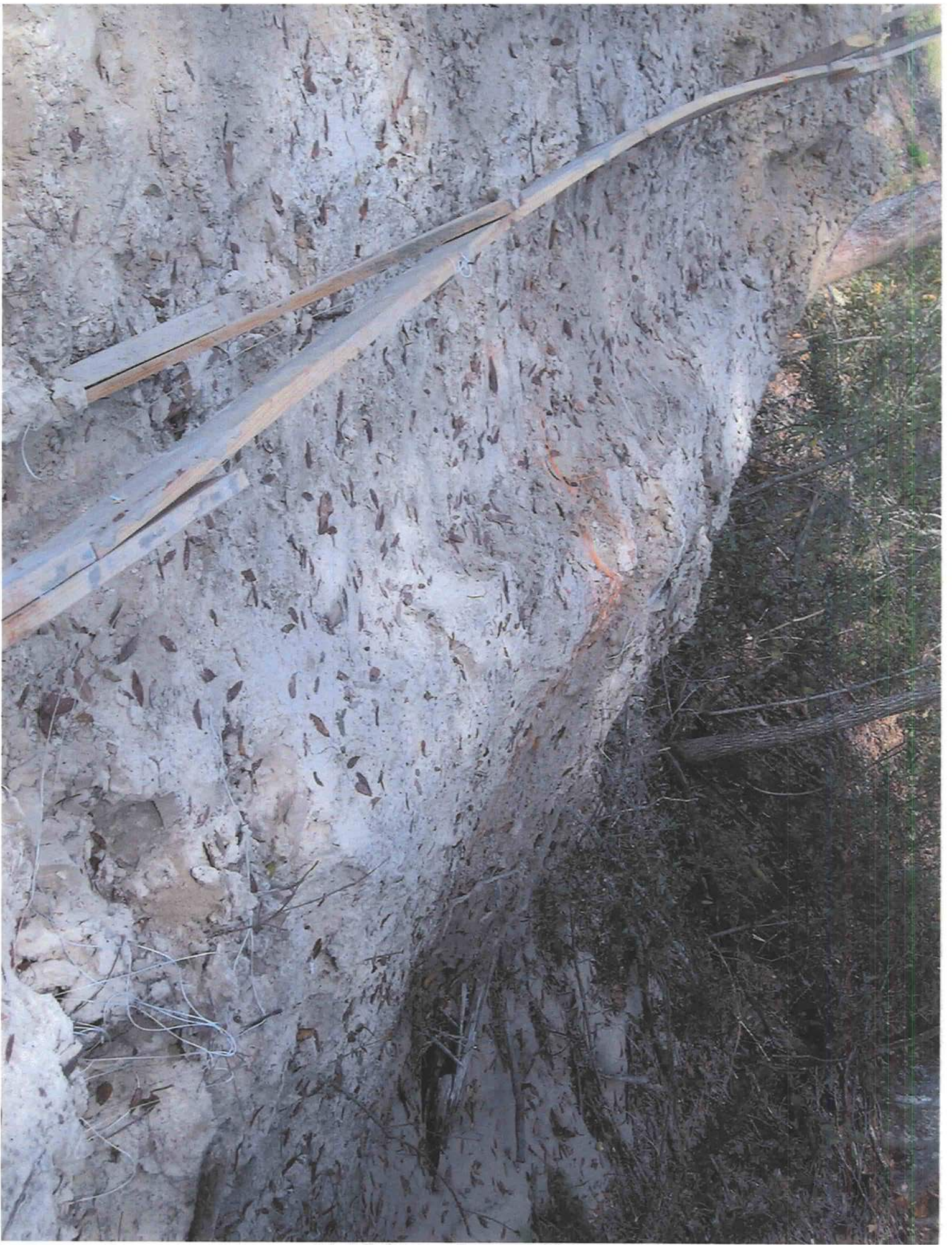
This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

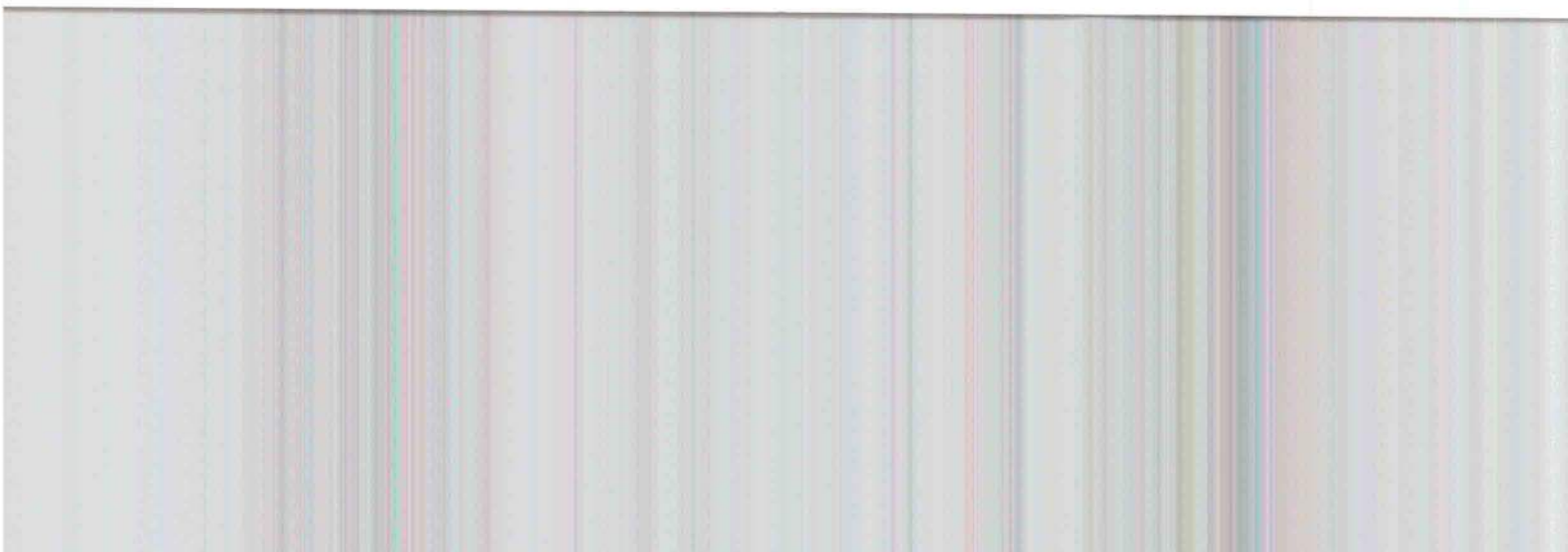
 		Name: EAGLE THOMAS H & Site: GREENE AT EMERALD LAKES Mail: P O BOX 813 LAKE CITY, FL 32056 Sales 4/27/2005 \$27,000.00V/Q Info 10/10/2002 \$19,500.00V/Q Landval \$18,275.00 BidgVal \$0.00 Appval \$18,275.00 JustVal \$18,275.00 Assd \$18,275.00 Exmpt \$0.00 Taxable \$18,275.00
 		PARCEL: 28-3S-16-02372-514 - VACANT (000000) LOT 14 ARBOR GREENE AT EMERALD LAKES S/D, ORB 964-2321, WD 1044-2552 J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083 Columbia County Property Appraiser

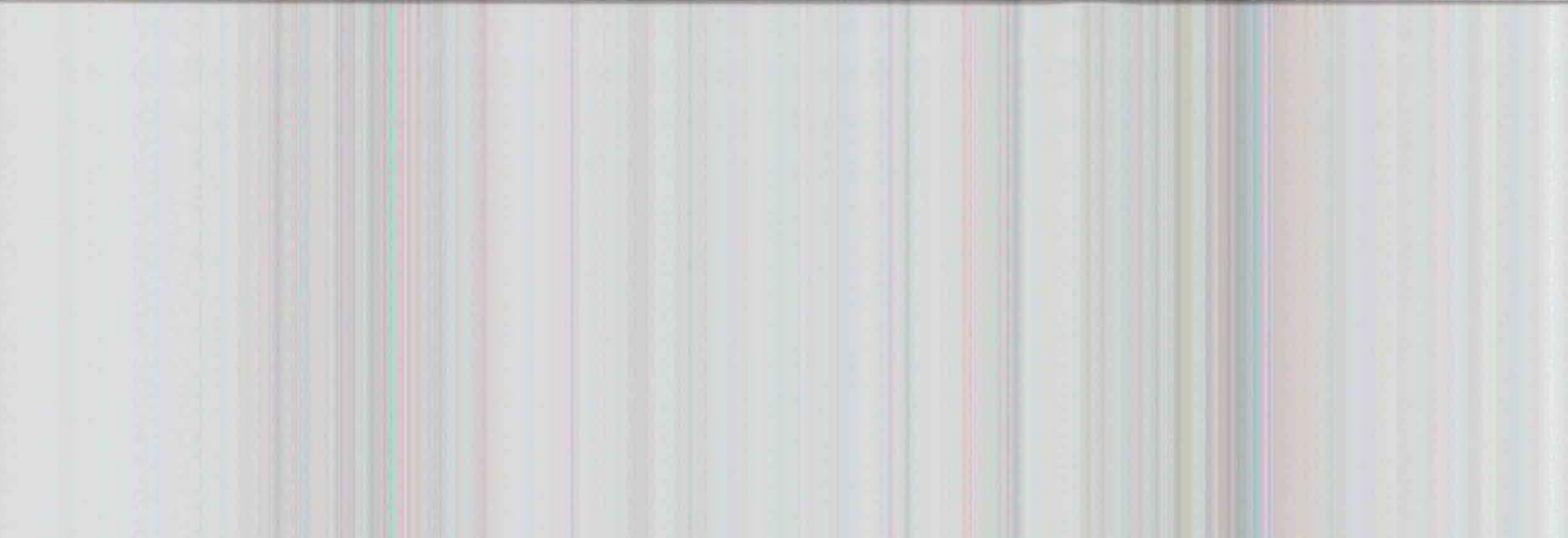


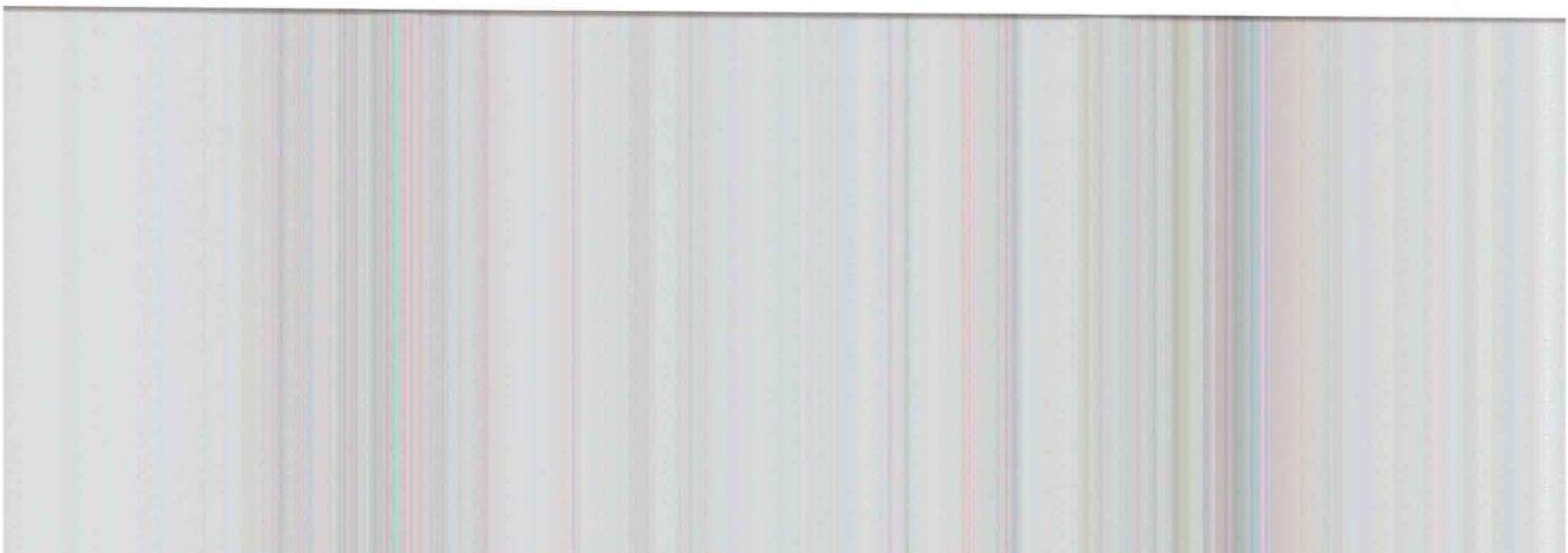
Amber Greene at Emerald Lakes

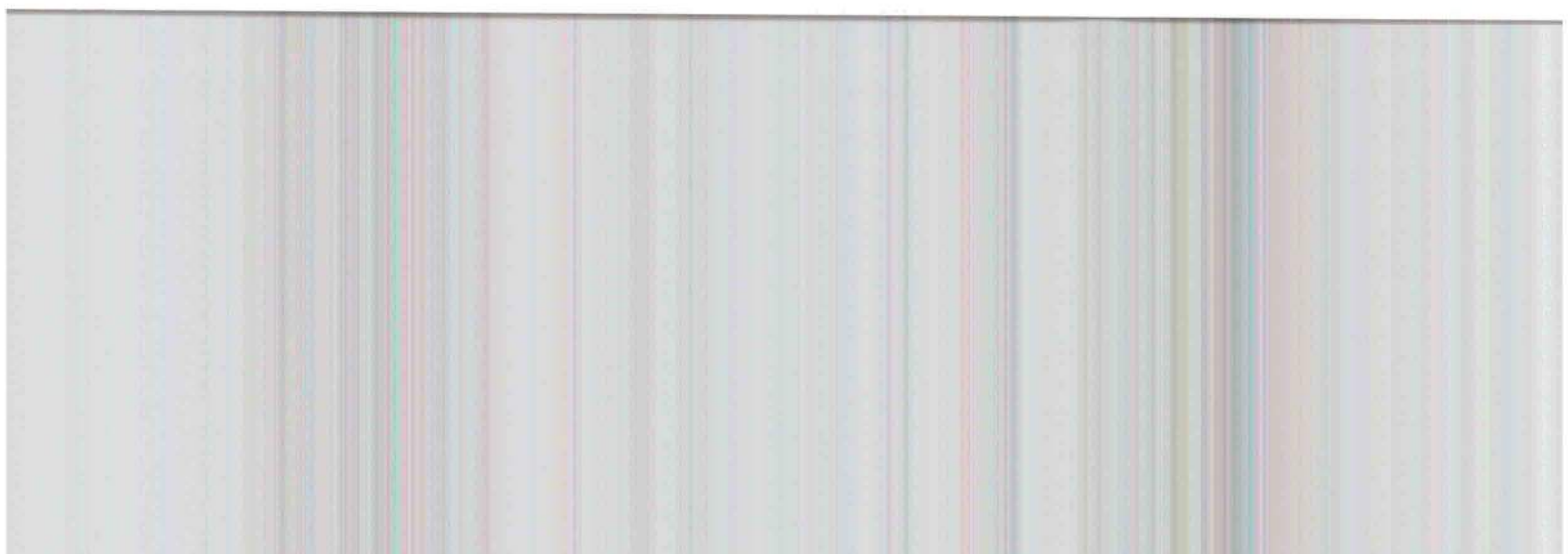
The County Engineer stated that the Building & Zoning Department should require the Owner and Builder to have their Engineer design a drainage plan to stabilize the ground, in order to protect and to keep it from eroding away for the proposed foundation and house.

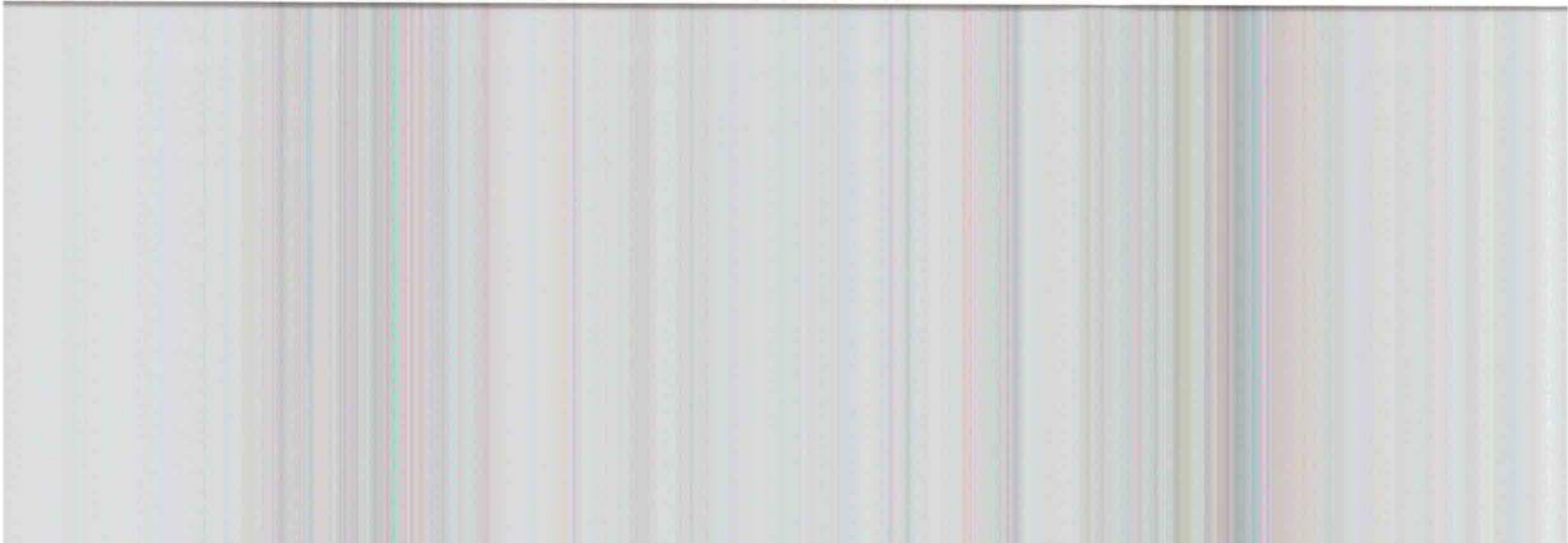


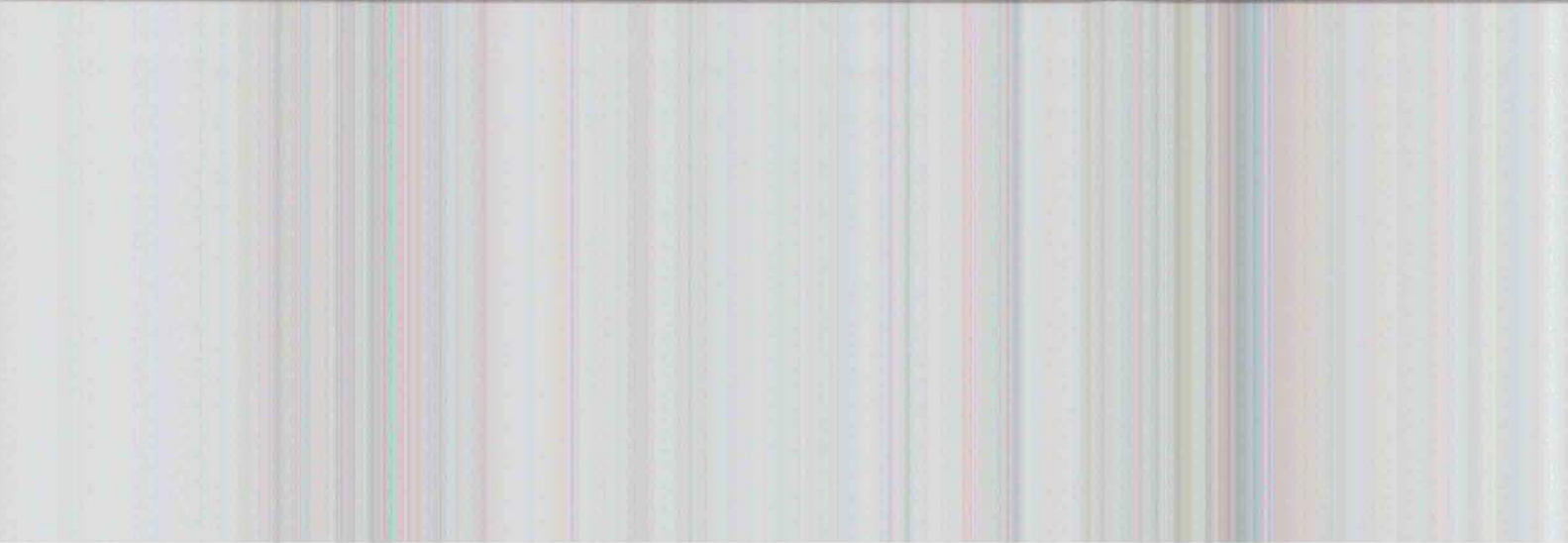


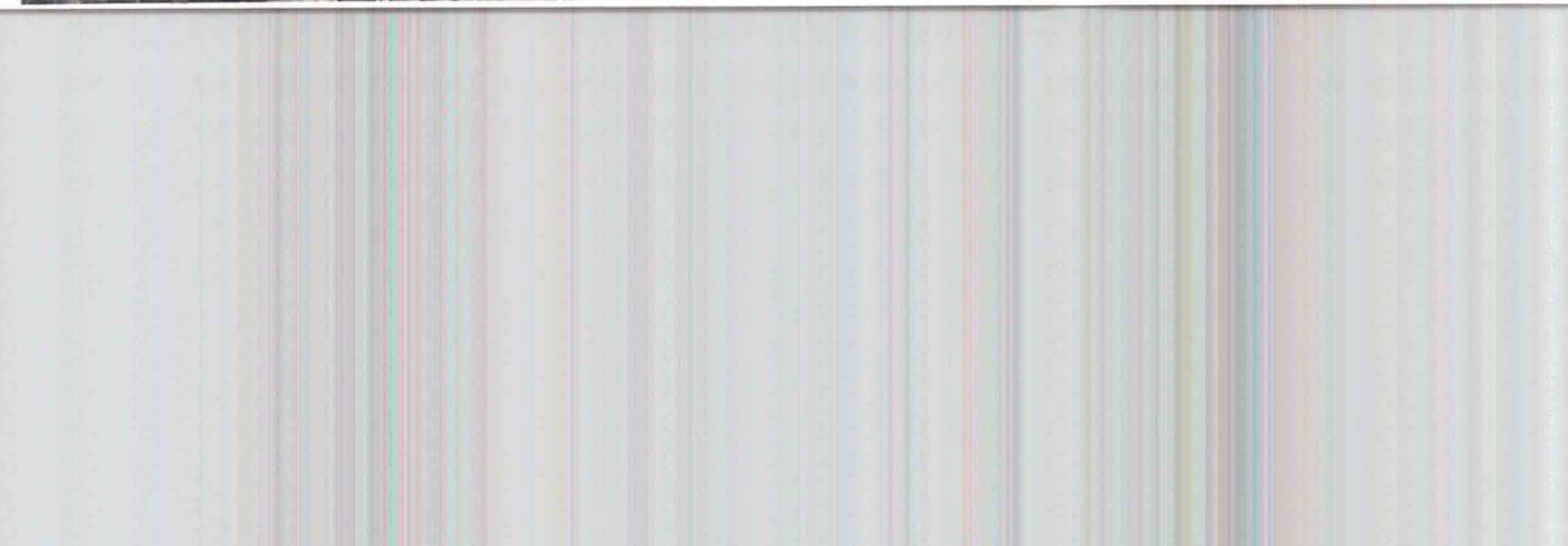




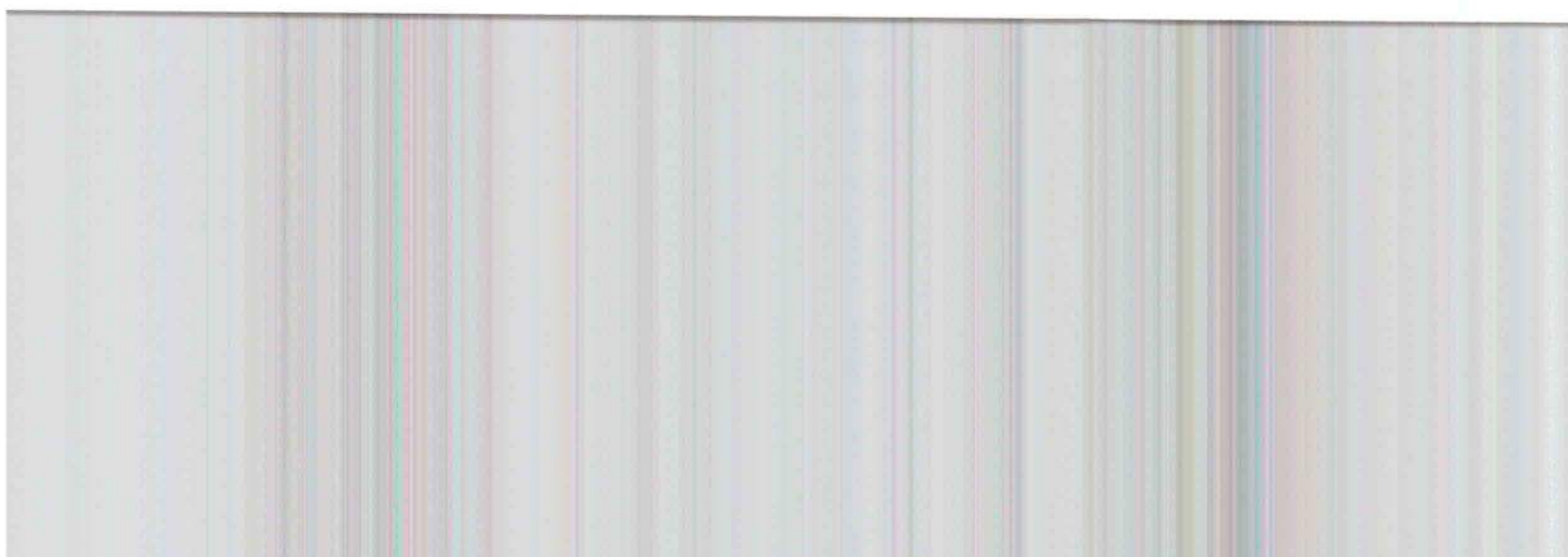
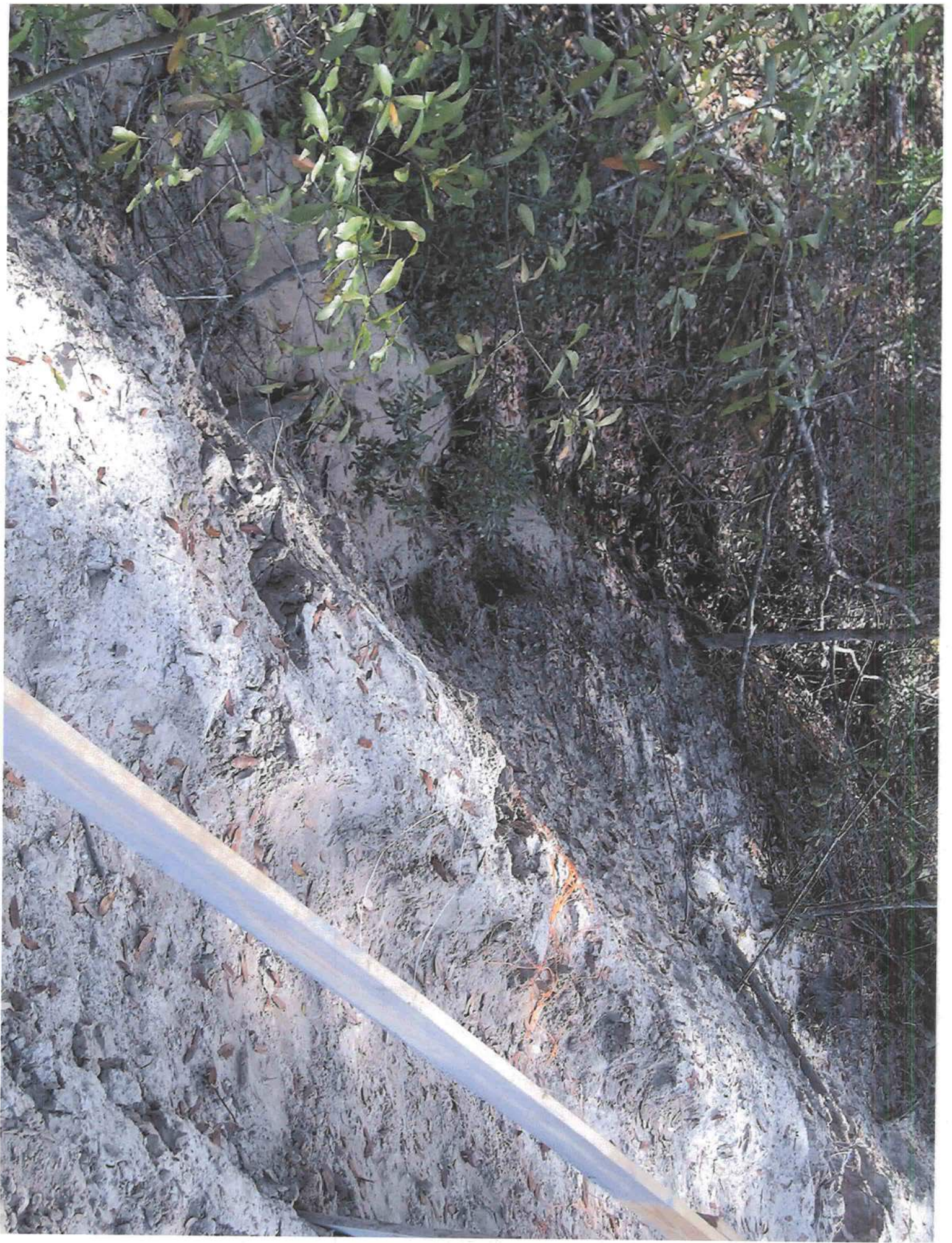


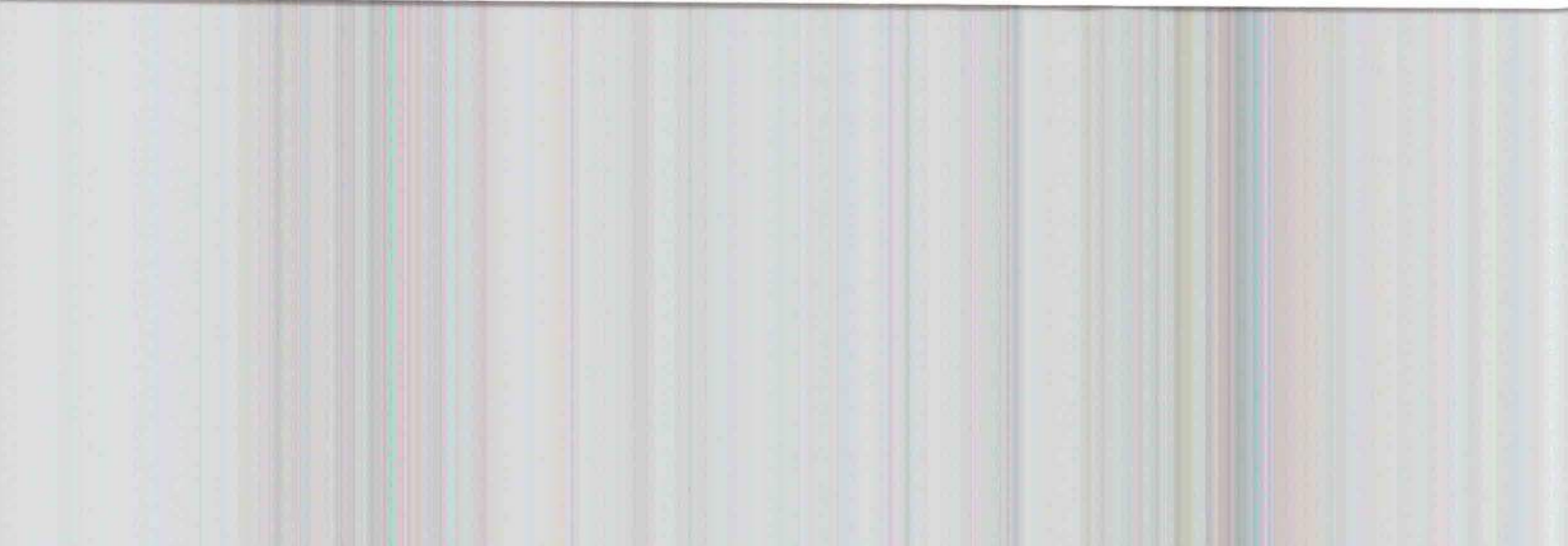














Phone: 386-758-1008 Fax: 386-758-2160

Lake City, FL 32055

135 NE Hernando Ave., Suite B-21

Amount Paid

25.00

DURING THE INSTALLATION OF THE CULVERT.

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED

Other

Department of Transportation Permit installation approved standards.

Culvert installation shall conform to the approved site plan standards.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION REQUIREMENTS

SIGNATURE

Susan Holt

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ARBOR GREEN @ EMERALD 14 1

EMERALD LAKES DR., TR ON HERITAGE DR., 7TH ON RIGHT

LOCATION OF PROPERTY 90W, TR ON BROWN RD, TL ON WINDING PLACE, TL ON

CONTRACTOR DAVID MANGRUM

PHONE 752-4716

ADDRESS 219 NW HERITAGE DR

LAKE CITY FL 32055

OWNER THOMAS EAGLE

PHONE 961-1086

ADDRESS 11 NW EGRET LANE

LAKE CITY FL 32055

APPLICANT SUSAN HOLT

PHONE 961-1086

DATE 12/28/2005

PARCEL ID # 28-3S-16-02372-514

Culvert Permit

Columbia County Building Department

Culvert Permit No. 000000930

COLUMBIA COUNTY BUILDING DEPARTMENT

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR
FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS
ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

- WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.
1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL
GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant Plans Examiner

All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. Designer's name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.

Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC.

- a. Basic wind speed (MPH)
- b. Wind importance factor (I) and building category
- c. Wind exposure - if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- d. The applicable internal pressure coefficient
- e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional

Elevations including:

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

Floor Plan Including:	a) Rooms labeled and dimensioned	<input type="checkbox"/>
	b) Shear walls	<input type="checkbox"/>
	c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)	<input type="checkbox"/>
	d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth	<input checked="" type="checkbox"/>
	e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails	<input checked="" type="checkbox"/>
	f) Must show and identify accessibility requirements (accessible bathroom)	<input type="checkbox"/>
Foundation Plan including:	a) Location of all load-bearing wall with required footings indicated as standard	<input type="checkbox"/>
	Or monolithic and dimensions and reinforcing	
	b) All posts and/or column footing including size and reinforcing	<input type="checkbox"/>
	c) Any special support required by soil analysis such as piling	<input type="checkbox"/>
	d) Location of any vertical steel	<input type="checkbox"/>
Roof System:	a) Truss package including:	<input type="checkbox"/>
	1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.	
	2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)	<input type="checkbox"/>
	b) Conventional Framing Layout including:	
	1. Rafter size, species and spacing	
	2. Attachment to wall and uplift	
	3. Ridge beam sized and valley framing and support details	
	4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)	<input type="checkbox"/>
Wall Sections including:	a) Masonry wall	<input type="checkbox"/>
	1. All materials making up wall	
	2. Block size and mortar type with size and spacing of reinforcement	
	3. Lintel, tie-beam sizes and reinforcement	
	4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details	
	5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation	
	6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)	
	7. Fire resistant construction (if required)	
	8. Fireproofing requirements	
	9. Shoe type of termite treatment (termicide or alternative method)	
	10. Slab on grade	
	a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)	
	b. Must show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and supports	
	11. Indicate where pressure treated wood will be placed	
	12. Provide insulation R value for the following:	
	a. Attic space	
	b. Exterior wall cavity	
	c. Crawl space (if applicable)	

b) Wood frame wall	<input type="checkbox"/>	<input type="checkbox"/>
1. All materials making up wall		
2. Size and species of studs		
3. Sheathing size, type and nailing schedule		
4. Headers sized		
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail		
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)		
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)		
8. Fire resistant construction (if applicable)		
9. Fireproofing requirements		
10. Show type of termite treatment (termicide or alternative method)		
11. Slab on grade		
a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed		
b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports		
12. Indicate where pressure treated wood will be placed		
13. Provide insulation R value for the following:		
a. Attic space		
b. Exterior wall cavity		
c. Crawl space (if applicable)		
c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)	<input type="checkbox"/>	<input type="checkbox"/>
Floor Framing System:		
a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer	<input type="checkbox"/>	<input type="checkbox"/>
b) Floor joist size and spacing		
c) Girder size and spacing		
d) Attachment of joist to girder		
e) Wind load requirements where applicable		
Plumbing Fixture layout		
Electrical layout including:		
a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified		
b) Ceiling fans		
c) Smoke detectors		
d) Service panel and sub-panel size and location(s)		
e) Meter location with type of service entrance (overhead or underground)		
f) Appliances and HVAC equipment		
g) Arc Fault Circuits (AFCI) in bedrooms		
HVAC information		
a) Manual J sizing equipment or equivalent computation		
b) Exhaust fans in bathroom		
Energy Calculations (dimensions shall match plans)		
Gas System Type (LP or Natural) Location and BTU demand of equipment		
Disclosure Statement for Owner Builders		
***Notice Of Commencement Required Before Any Inspections Will Be Done		
Private Potable Water	<input type="checkbox"/>	<input type="checkbox"/>
a) Size of pump motor		
b) Size of pressure tank		
c) Cycle stop valve if used		

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is \$50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE - TIME WILL NOT ALLOW THIS - PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE

COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR

AN APPOINTMENT TIME AND DATE:

YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE

TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY

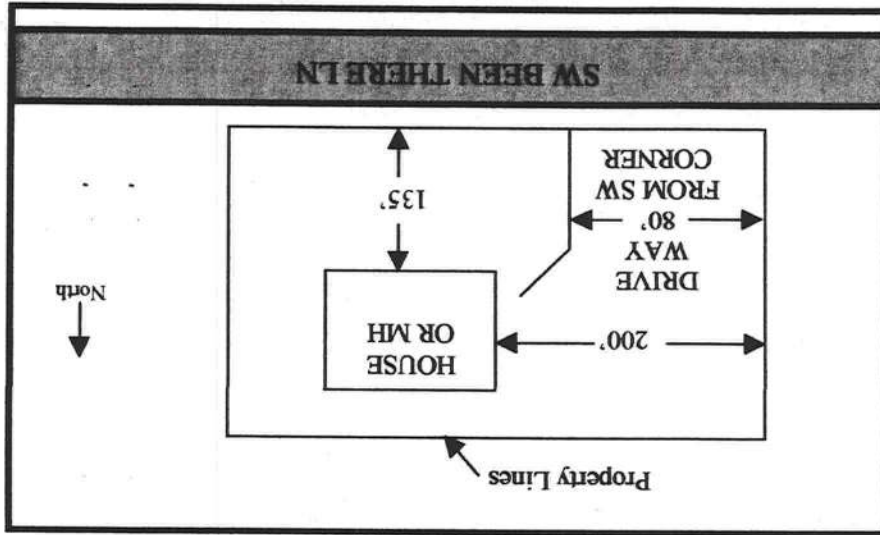
EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123") FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.

- a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
- b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
- c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	062705810	6/27/2005	41	T20	062705850	6/27/2005
2	CJ3	062705811	6/27/2005	42	T21	062705851	6/27/2005
3	CJ3A	062705812	6/27/2005	43	T22	062705852	6/27/2005
4	CJ5	062705813	6/27/2005	44	T23	062705853	6/27/2005
5	CJ5A	062705814	6/27/2005	45	T26	062705854	6/27/2005
6	EJ5	062705815	6/27/2005	46	T27	062705855	6/27/2005
7	EJ7	062705816	6/27/2005	47	T28	062705856	6/27/2005
8	EJ7A	062705817	6/27/2005	48	T29	062705857	6/27/2005
9	HJ5	062705818	6/27/2005	49	T31	062705858	6/27/2005
10	HJ7	062705819	6/27/2005	50	T32	062705859	6/27/2005
11	HJ7A	062705820	6/27/2005	51	T33	062705860	6/27/2005
12	PB01	062705821	6/27/2005	52	T34	062705861	6/27/2005
13	PB02	062705822	6/27/2005				
14	PB03	062705823	6/27/2005				
15	PB04	062705824	6/27/2005				
16	PB05	062705825	6/27/2005				
17	PB06	062705826	6/27/2005				
18	PB07	062705827	6/27/2005				
19	PB08	062705828	6/27/2005				
20	PB09	062705829	6/27/2005				
21	PB10	062705830	6/27/2005				
22	PB11	062705831	6/27/2005				
23	T01	062705832	6/27/2005				
24	T02	062705833	6/27/2005				
25	T03	062705834	6/27/2005				
26	T04	062705835	6/27/2005				
27	T05	062705836	6/27/2005				
28	T06	062705837	6/27/2005				
29	T07	062705838	6/27/2005				
30	T08	062705839	6/27/2005				
31	T09	062705840	6/27/2005				
32	T10	062705841	6/27/2005				
33	T11	062705842	6/27/2005				
34	T12	062705843	6/27/2005				
35	T13	062705844	6/27/2005				
36	T15	062705845	6/27/2005				
37	T16	062705846	6/27/2005				
38	T17	062705847	6/27/2005				
39	T18	062705848	6/27/2005				
40	T19	062705849	6/27/2005				

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Building Designer, responsible for Structural Engineering: (See attached)
MANGRUM, DAVID E RB29003100
Address: 634 SE MAYHALL TERRACE
LAKE CITY, FL 32025
Designer: 60

Project Information for:
L117440
Builder: Lipscomb Eagle
Lot: Lot 14
Subdivision: Arbor Green
County or City: Columbia County
Truss Page Count: 52
Design Program: MitTek 5.2 / 6.2
Building Code: FBC2001
Wind
Roof (psf): 42
Wind Standard: ASCE 7-98
Floor (psf): 55
Wind Speed (mph): 120
Note: See individual truss drawings for special loading conditions



LOAD CASES) Standard

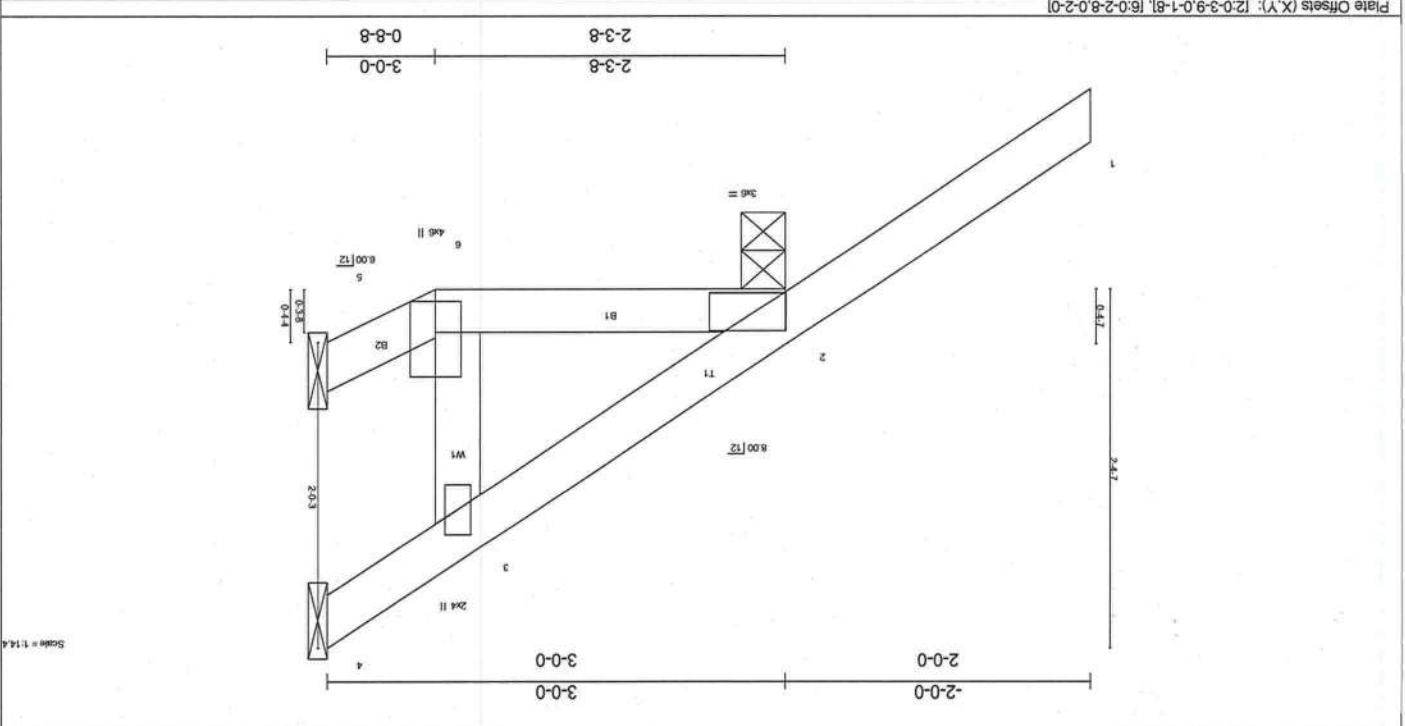
NOTES

1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TC_{DL}=4.2psf; BC_{DL}=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Extent(2) zone: Lumber DCL=1.60 plate gfp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 4 and 243 lb uplift at joint 2.

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-74/17, 3-4=-20/33
 BOT CHORD 2-6=-4/0, 5-6=4/4
 WEBS 3-6=0/42

REACTIONS (lb/size) 4=63/Mechanical, 2=278/0-3-6, 5=10/Mechanical
 Max Uplift=29(load case 4), 2=-243(load case 5)
 Max Horiz 2=209(load case 5)

LUMBER		BRACING		LOADING (psf)		SPACING		PLATES		GRIP	
TOP CHORD	2 X 4 SYP No.2D	TOP CHORD	Sheathed or 3-0 oc purlins.	DEFL	In (loc)	Vert(L)	2-0-0	PLATES	MT20	Weight:	16 lb
BOT CHORD	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0 oc bracing.	Horiz(TL)	5	Vert(TL)	5.0	GRIP	244/190		
BCDL	5.0	CSI		in (loc)	6	Vert(L)	1.25				
BCLL	10.0	WB		in (loc)	6	Horiz(TL)	1.25				
TCDL	7.0	BC		in (loc)	6	Vert(TL)	1.25				
TCLL	20.0	TC		in (loc)	6	Horiz(TL)	1.25				
Rep Stress Incr	YES	WB		in (loc)	6	Vert(TL)	1.25				
Lumber Increase	1.25	BC		in (loc)	6	Horiz(TL)	1.25				
Plates Increase	1.25	TC		in (loc)	6	Vert(TL)	1.25				
Code FBC2001/ANSI95		WB		in (loc)	6	Horiz(TL)	1.25				



Job	L117440	Truss	C3A	SPECIAL	1	Qty	2	Plat	1	Job Reference (optional)	6/2005 Dec 15 2004 Milltek Industries, Inc. Fri Jun 24 12:36:49 2005 Page 1
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Dwg.#062705812

LOAD CASE(S) Standard

1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone; Lumber DOL=1.60; This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girders for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 3 and 226 lb uplift at joint 2.

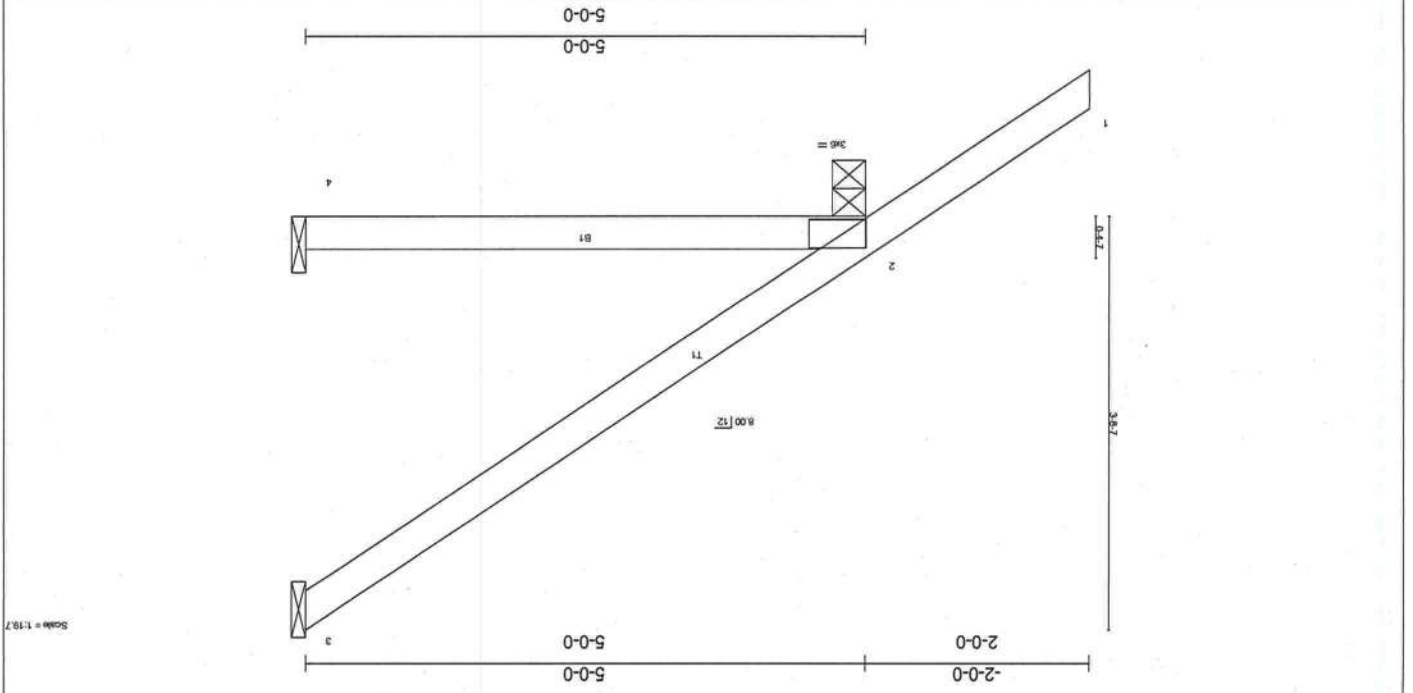
NOTES

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-104/44
 BOT CHORD 2-4=0/0
REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
 Max Horiz Z=282(load case 5)
 Max Uplift3=-129(load case 5), 2=-226(load case 5)

LUMBER TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
BRACING TOP CHORD Sheathed or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP STRESS INC	Code FBC2001/ANSI95	CS1	DEFL	VERT(L)	VERT(T)	Horz(TL)	Weight: 20 lb
20.0	2-0-0	1.25	1.25	YES		TC 0.32	in (loc)	-0.03	-0.04	-0.00	PLATES MT20 244/190
10.0						WB 0.00	in (del)	>999	2-4	3	GRIP
5.0							L/d	240	180	n/a	

Plate Offsets (X,Y): [2:0-3-9-0-1-8]



Job	Truss	CJS	MONO TRUSS	8	Qty	1	Job Reference (optional)
L117440							

Dwg. #062705813

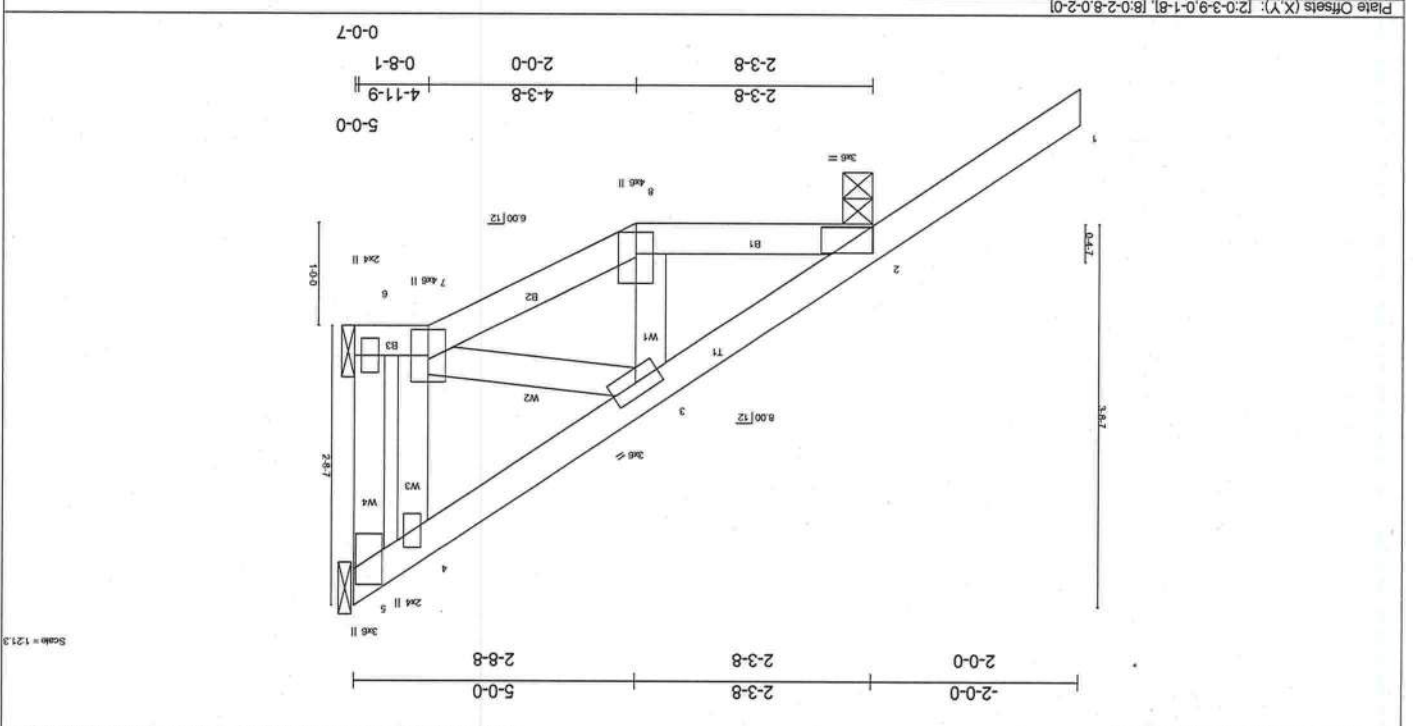
LOAD CASE(S) Standard

- NOTES
 1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC=4; p=3; BCDL=3; 0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Extent(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 1.13 lb uplift at joint 2.
 4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

- REACTIONS (lb/size) 5=163/Mechanical, 6=68/Mechanical, 2=340/0-3-8
 Max Uplift=1.13(load case 5), 2=226(load case 5)
 FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=205/0, 3-4=68/0, 4-5=-72/89, 5-6=0/0
 BOT CHORD 2-8=52/113, 7-8=-56/138, 6-7=-0/0
 WEBS 3-8=23/37, 4-7=-27/119, 3-7=107/47

LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3
 BRACING TOP CHORD Sheathed or 5-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf)	2.0-0	SPACING	Plates increase 1.25	Lumber increase 1.25	Rep Stress Incr YES	Code FBC2001/ANSI95	CS1	TC 0.32	BC 0.05	WB 0.04	DEFL	In (loc)	L/d	L/d	PLATES	GRIP	Weight: 31 lb
TCLL	20.0	Plates increase	1.25	Lumber increase	1.25	Code FBC2001/ANSI95	CS1	TC 0.32	BC 0.05	WB 0.04	DEFL	In (loc)	L/d	L/d	MT20	244/190	
TCCL	7.0	Rep Stress Incr	YES	Code FBC2001/ANSI95							Horz(TL)	-0.00	6	n/a			
BCLL	10.0										Vert(TL)	0.01	7	>999			
BCLD	5.0										Horz(TL)	-0.00	5	n/a			



Job	L117440	Truss	CJ5A	SPECIAL	Qty	1	Job Reference (optional)	6:200 s Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:50 2005 Page 1
Truss Type					Ply	1		

Dwg.#062705814

LOAD CASE(S) Standard

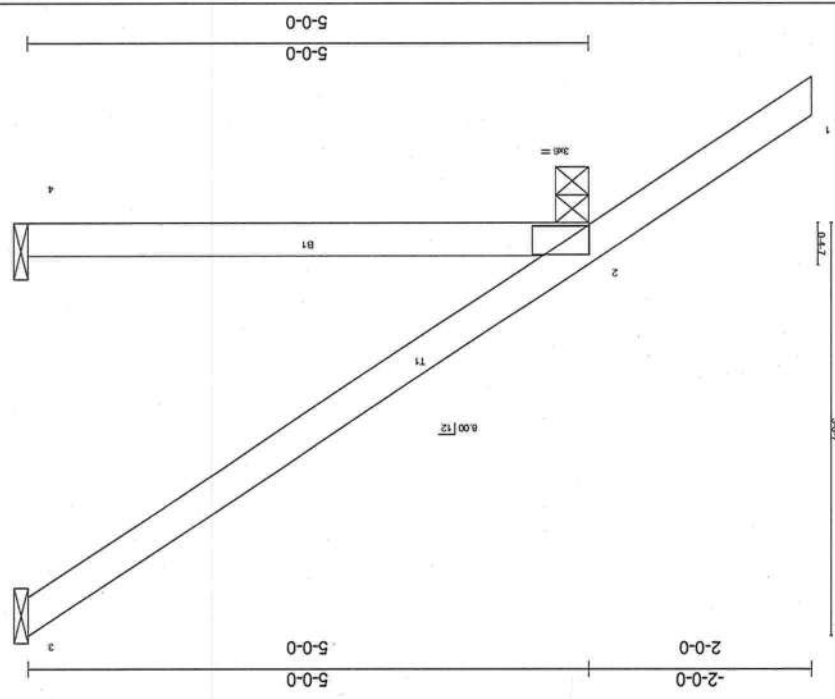
NOTES
 (1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC DL=4.2psf; BC DL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior (2) zone; Lumber DOL=1.60 plate gip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (2) Refer to girder(s) for truss to truss connections.
 (3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 3 and 226 lb uplift at joint 2.

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-104/44
 BOT CHORD 2-4=0/0
REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
 Max Horiz 2=282(load case 5), 2=226(load case 5)
 Max Uplift 3=-129(load case 5), 2=-226(load case 5)

LUMBER TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
BRACING TOP CHORD Sheathed or 5-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf)	SPACING	Plates Increase	TC	BC	WB	CSI	DEFL	Vert(LL)	Vert(TL)	Horz(TL)	L/D	l/defl	PLATES	GRIP	Weight: 20 lb
20.0	2-0-0	1.25	0.32	0.14	0.00	0.00	0.03	-0.04	-0.00	0.00	2-4	>999	MT20	244/190	
7.0		1.25					2-4	>999	180	n/a	n/a				
10.0		YES					3	n/a	180	n/a					
5.0		Code FBC2001/ANSI95													

Plate Offsets (X,Y): [2-0-3-9-0-1-8]



Scale = 1/16"

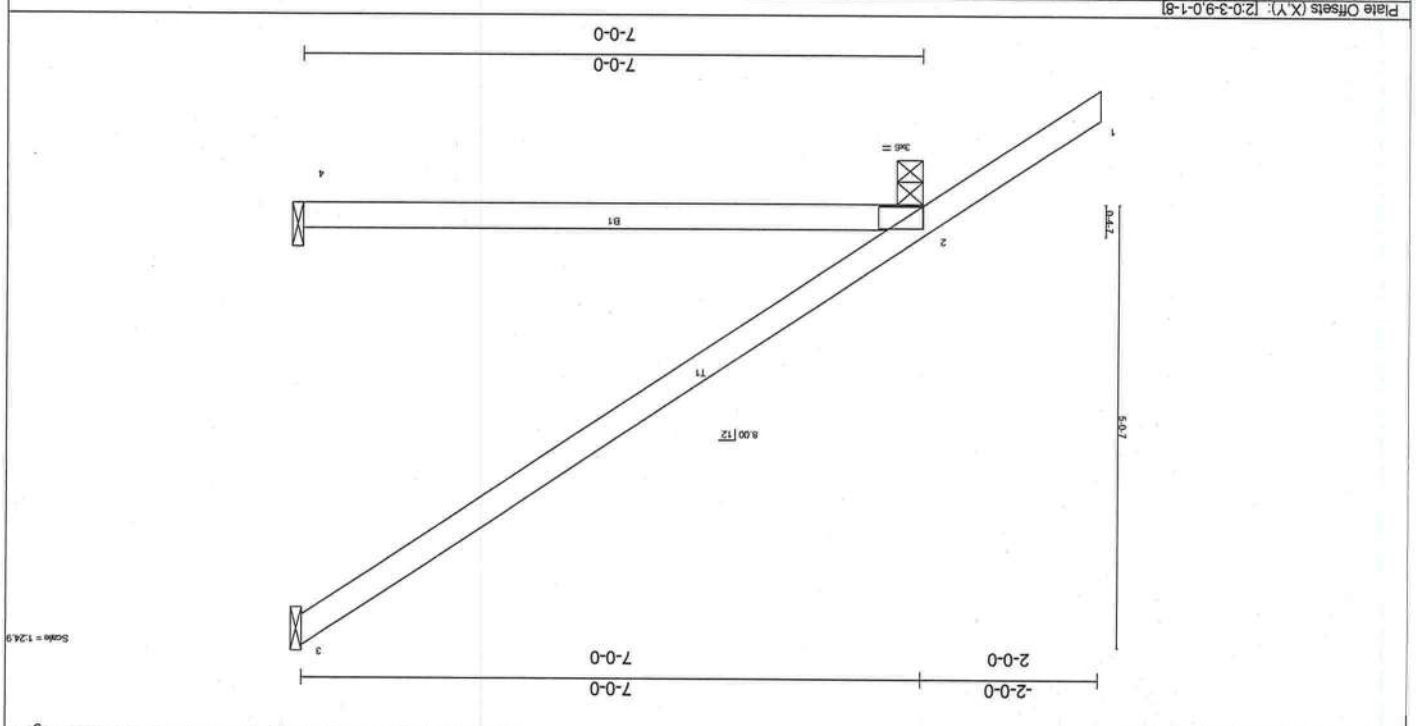
Builders FirstSource, Lake City, FL 32055

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Job	L117440	Truss	EJS	MONO TRUSS	Qty	4	Job Reference (optional)	1
Dwg.#	062705815							

Job	L17440	EJ7	MONO TRUSS	27	1
Truss				Qty	Ply
Job Reference (optional)					
6,200 s Dec 15 2004 Milltek Industries, Inc. Fri Jun 24 12:36:51 2005 Page 1					

Dwg.#062705816



LOADING (psf)	TOLL 20.0	SPACING	2'-0"	Plates Increase 1.25	TC 0.43	DEFL	in (loc)	l/def	L/D
BCLL	10.0	Rep Stress Incr	YES	Lumber Increase 1.25	BC 0.28	Vert(TL)	-0.11	2.4	>776
BDDL	5.0	Code FBC2001/ANSI95			WB 0.00	Horz(TL)	-0.00	3	n/a

LUMBER TOP CHORD 2 X 4 SYP No.2D
BRACING TOP CHORD Sheathed or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=419/0-3-8, 4=102/Mechanical
 Max Horz 2=355(load case 5)
 Max Uplift 3=-214(load case 5), 2=-229(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-162/73
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 3 and 229 lb uplift at joint 2.
LOAD CASE(S) Standard

LOAD CASE(S) Standard

- (1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extent(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (2) Refer to girders) for truss to truss connections.
 (3) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 126 lb uplift at joint 6, 228 lb uplift at joint 2 and 64 lb uplift at joint 5.
 (4) Gap between inside of top chord bearing and first diagonal or vertical bearing and first diagonal or vertical web shall not exceed 0.500in.

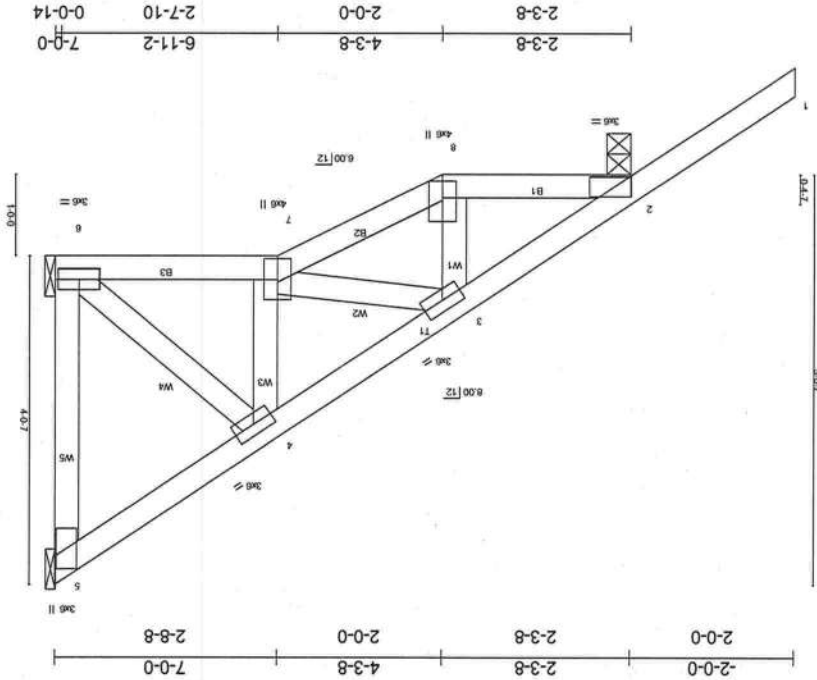
NOTES

- FORCES** (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-319/0, 3-4=-283/27, 4-5=-49/21, 5-6=0/0
 BOT CHORD 2-8=-106/197, 7-8=-114/228, 6-7=-174/211
 WEBS 3-8=-36/62, 3-7=-77/129, 4-7=-25/160, 4-6=-274/228
- REACTIONS** (lb/size) 6=2'13"/Mechanical, 2=4'6"-3'-8, 5=50"/Mechanical
 Max Uplift=126(load case 5), 2=228(load case 5), 5=64(load case 5)
 Max Horiz 2=352(load case 5)

LUMBER 2 X 4 SYP No.2D
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
BRACING TOP CHORD Sheathed or 6-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing.

LOADING (psf)	20.0	SPACING	2-0-0	CSI	TC 0.31	DEFL	in (loc)	PLATES	GRIP
TCLL	7.0	Plates Increase	1.25	WB	0.06	in (loc)	l/d	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.08	Vert(L)	7 > 999	Weight: 44 lb	
BCLL	5.0	Rep Stress Incr	YES	Horz(TL)	0.00	6-7	> 999		
BCLD	5.0	Code FBC2001/ANSI95		7	n/a	n/a	n/a		

Plate Offsets (X,Y): 12-0-3-9-0-1-81, 18-0-2-8-0-2-0



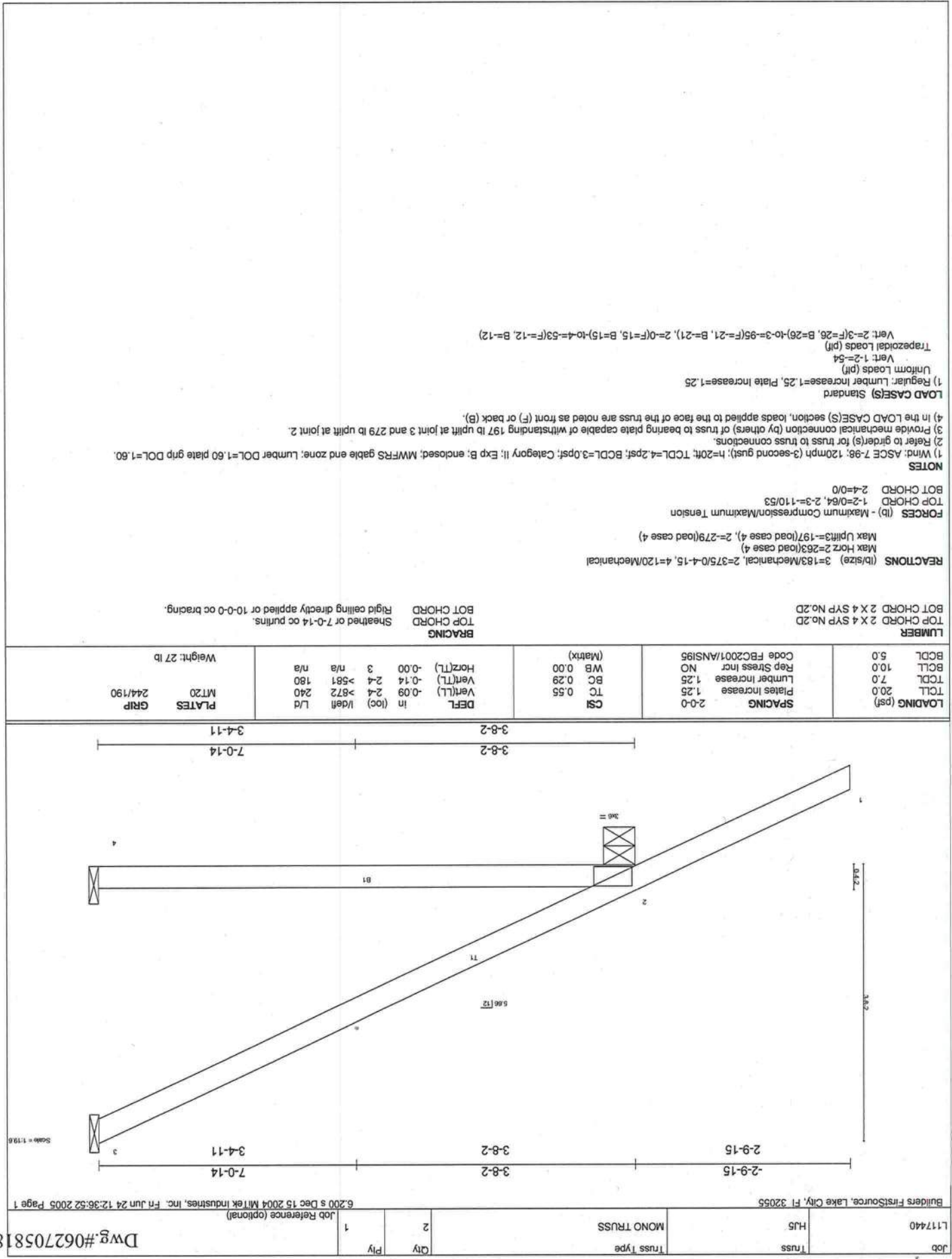
Scale = 1/288

Builders FirstSource, Lake City, FL 32055

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Dwg.#062705817

Job	L117440	Truss	EJ7A	Truss Type	SPECIAL	Qty	3	Ply	1
									Job Reference (optional)



Job	L117440	Truss	HJ5	MONO TRUSS	1	Job Reference (optional)
Truss Type		Qty	2			
		Ply	1			

Builders FirstSource, Lake City, FL 32055
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Dwg #062705818

LOADING (psf)	SPACING	CSI	DEFL	IN (in)	I/defl	L/d
20.0	2-0-0	0.55	0.09	2.4	240	240
7.0	1.25	0.29	0.14	2.4	180	180
10.0	NO	0.00	0.00	3	n/a	n/a
5.0	Code FBC2001/ANSI95					

BRACING	TOP CHORD	BOT CHORD
TOP CHORD 2 X 4 SYP No.2D	Sheathed or 7-0-14 oc purlins	Rigid ceiling directly applied or 10-0-0 oc bracing.
BOT CHORD 2-4-0/0		

REACTIONS (lb/size) 3=183/Mechanical, 2=375/0-4-15, 4=120/Mechanical
 Max Horiz 2=263(load case 4)
 Max Uplift 3=197(load case 4), 2=279(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/64, 2-3=110/53
 BOT CHORD 2-4=0/0

NOTES
 1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 2) Refer to girder(s) for truss connections.
 3) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 197 lb uplift at joint 3 and 279 lb uplift at joint 2.
 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pl)
 Vert: 1-2=-54
 Trapezoidal Loads (pl)
 Vert: 2=-3(F)=26, B=26(-10-3=-95(F)=-21, B=21), 2=0(F)=15, B=15(-10-4=-53(F)=-12, B=12)

Job	L117440	H17	MONO TRUSS	3	Qty	1	Job Reference (optional)
Truss							
Truss Type							

Builders FirstSource, Lake City, FL 32055
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LOADING (psf)		SPACING		CSI		DEFL		BRACING	
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(L)	-0.07	In (loc)	L/D
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(R)	-0.10	5-6	>999
BCLL	10.0	Rep Stress Incr	NO	WB	0.42	Horz(TL)	0.01	5	n/a
BCDL	5.0	Code FBC2001/ANSI95						n/a	

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3

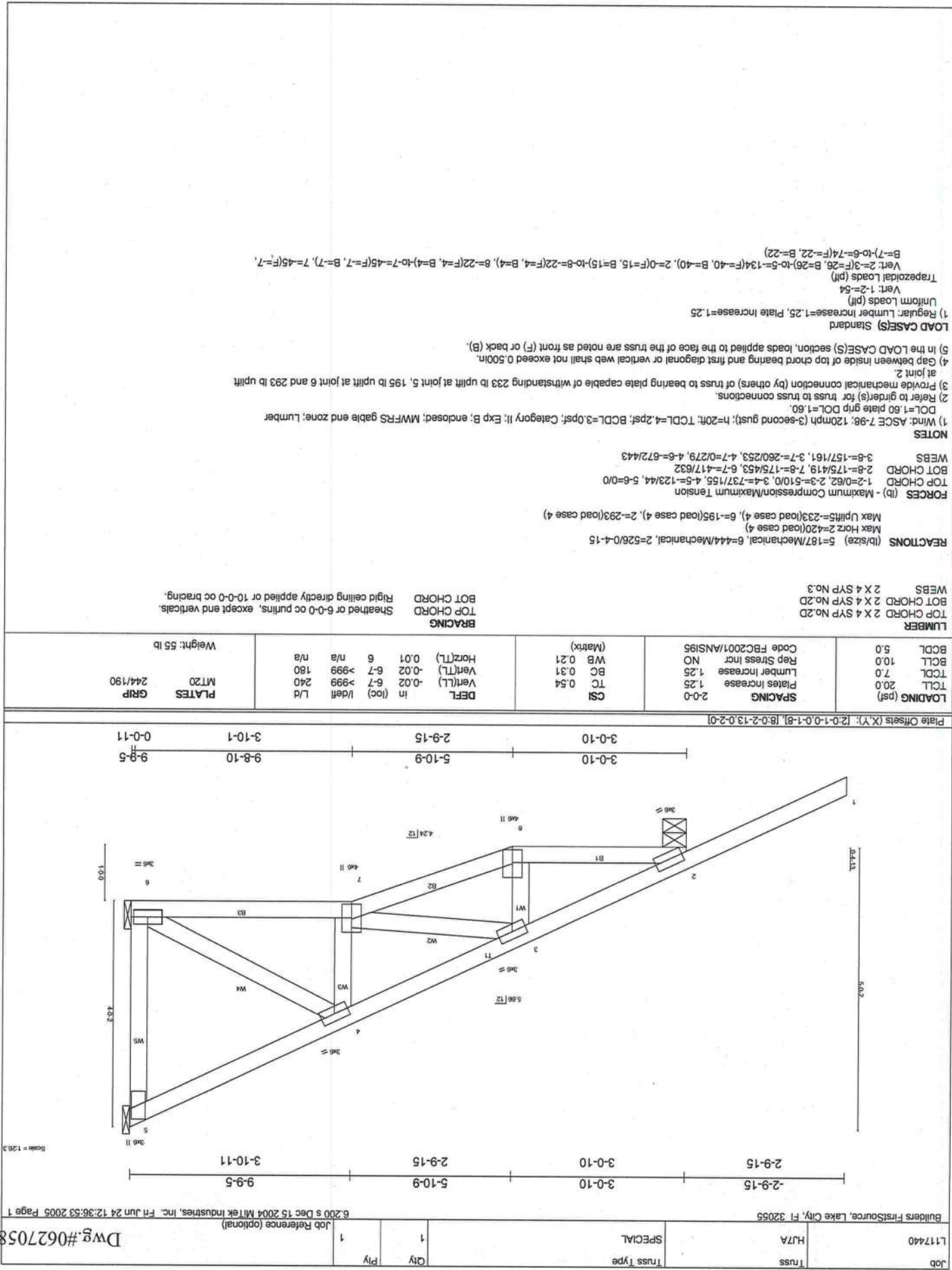
REACTIONS (lb/size) 4=266/Mechanical, 2=531/0-4-15, 5=379/Mechanical
 Max Uplift=-327(load case 4), 2=-295(load case 4), 5=-111(load case 4)
 Max Horiz Z=432(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/64, 2-3=700/26, 3-4=-175/83
 BOT CHORD 2-6=-349/618, 5-6=-349/618
 WEBS 3-6=0/165, 3-5=-659/372

NOTES
 1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft, TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber
 DOL=1.60 plate grip DOL=1.60
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 327 lb uplift at joint 4, 295 lb uplift at joint 2 and 111 lb uplift at joint 5.
 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pl)
 Vert: 1-2=54
 Trapezoidal Loads (pl)
 Vert: 2-3(F)=26, B=26/10-4=-134(F)=40, B=40), 2=0(F)=15, B=15)-10-5=-74(F)=22, B=22)
 Vert: 2-3(F)=26, B=26/10-4=-134(F)=40, B=40), 2=0(F)=15, B=15)-10-5=-74(F)=22, B=22)

BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins
 BOT CHORD Rigid ceiling directly applied or 9-7-6 oc bracing.

PLATES MT20 244/190
GRIP 244/190
 Weight: 47 lb



LOADING (psf)	SPACING	CS1	DEFL	BRACING	LUMBER	REACTIONS (lb/size)	FORCES (lb)	TOP CHORD	BOT CHORD	WEBS
20.0	2'-0"	TC 0.54	in (loc)	TOP CHORD Sheathed or 6'-0" oc purlins, except end verticals.	TOP CHORD 2 X 4 SYP No.2D	5=187Mechanical, 6=444Mechanical, 2=526/0-4-15	Max Uplift=233(load case 4), 6=-195(load case 4), 2=-293(load case 4)	1-2=0/62, 2-3=51/0, 3-4=737/155, 4-5=-123/44, 5-6=0/0	2-8=-175/419, 7-8=-175/453, 6-7=-417/632	3-8=-157/161, 3-7=-260/253, 4-7=0/279, 4-6=-672/443
TCLL 7.0	Rep Stress Incr NO	BC 0.31	Vert(TL) -0.02	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.	BOT CHORD 2 X 4 SYP No.3					
TCDL 10.0	Lumber Increase 1.25	WB 0.21	Horiz(TL) 0.01							
BCLL 5.0	Code FBC2001/ANSI95									
BCLD 5.0										

REACTONS (lb/size) 5=187Mechanical, 6=444Mechanical, 2=526/0-4-15
 Max Uplift=233(load case 4), 6=-195(load case 4), 2=-293(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/62, 2-3=51/0, 3-4=737/155, 4-5=-123/44, 5-6=0/0
 BOT CHORD 2-8=-175/419, 7-8=-175/453, 6-7=-417/632
 WEBS 3-8=-157/161, 3-7=-260/253, 4-7=0/279, 4-6=-672/443

NOTES
 1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4; 2psf; BCDL=3; 0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber at joint 2.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 5, 195 lb uplift at joint 6 and 293 lb uplift at joint 2.
 4) Gap between inside of top chord bearing and first diagonal or vertical shall not exceed 0.500in.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pl)
 Vert: 1-2=54
 Trapezoidal Loads (pl)
 Vert: 2-3(F)=26, B=26/0-5=-134(F)=22, B=-22
 B=7/0-6=-74(F)=22, B=-22

16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549
 THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); n=20ft; TC DL=4.2psf; BC DL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1, 60 lb uplift at joint 9, 186 lb uplift at joint 13, 143 lb uplift at joint 12 and 176 lb uplift at joint 11.

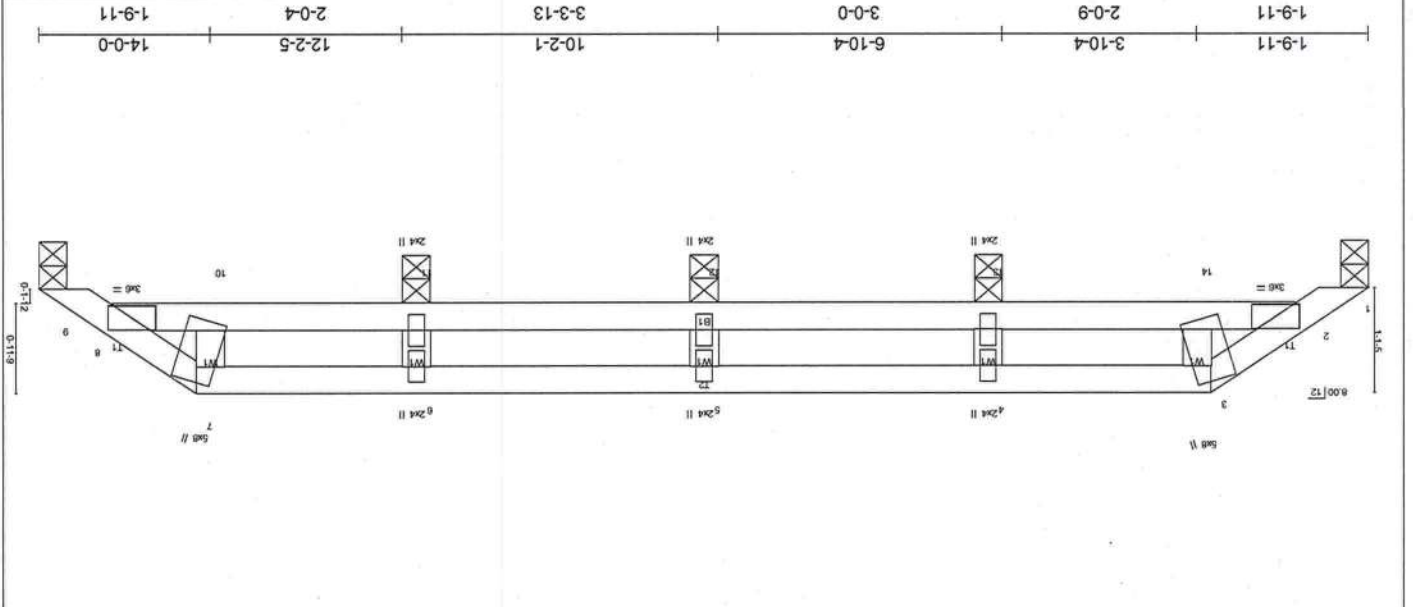
NOTES

- FORCES (lb)
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3
 REACTIONS (lb/size) 1=145/0-3-8, 9=144/0-3-8, 13=313/0-3-8, 12=213/0-3-8, 11=313/0-3-8
 Max Horiz 1=42(load case 4)
 Max Uplift 1=57(load case 5), 9=60(load case 6), 13=186(load case 4), 12=143(load case 3), 11=176(load case 3)
 Max Grav 1=145(load case 1), 9=144(load case 1), 13=315(load case 7), 12=218(load case 7), 11=314(load case 8)

LUMBER	TOP CHORD 2 X 4 SYP No.2D	BRACING	TOP CHORD	Sheathed or 6-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.3	WEBS	2 X 4 SYP No.3	Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf)	20.0	SPACING	2-0-0	DEFL	In (loc)	0.01	14	>999	240	PLATES	GRIP	244/190
TCLL	7.0	Plates Increase	1.25	Vert(LL)	0.01	14	>999	180	MT20	Weight:	43 lb	
TC DL	1.0	Lumber Increase	1.25	Vert(TL)	0.01	9	n/a	n/a				
BCLL	10.0	Rep Stress Incr	YES	Horz(TL)	0.01	9	n/a	n/a				
BC DL	5.0	Code FBC2001/ANSI95		(Matrix)								
		TC	0.13	WB	0.04							
		BC	0.10									

Plate Offsets (X, Y): [3:0-2-0-0-2-4], [7:0-2-0-0-2-4]



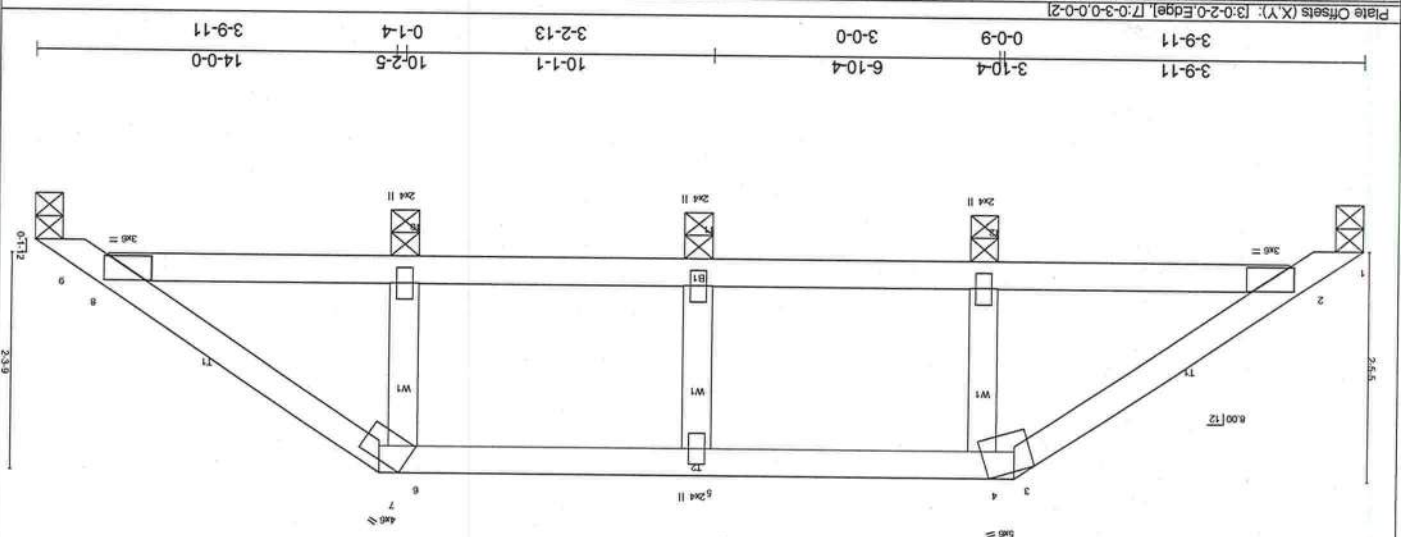
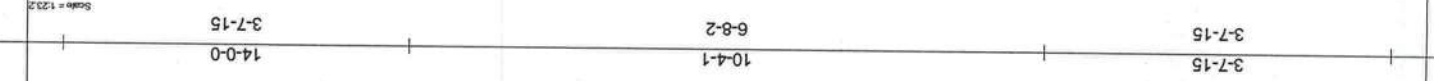
1-7-15	1-7-15	7-0-0	5-4-1	5-4-1	12-4-1	14-0-0	1-7-15
1-9-11	1-9-11	3-10-4	2-0-9	3-0-0	10-2-1	12-2-5	1-9-11

Job	L117440	Truss	PB01	HIP CAP	Qty	1	Job Reference (optional)	6,200 s Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:54 2005 Page 1
					Ply	1		

Dwg.#062705821

Scale = 1/200

Job	L117440	Truss	PB02	HIP CAP	Qty	1	Ply	1
Builders FirstSource, Lake City, FL 32055								
Job Reference (optional)								
6/200 8 Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:55 2005 Page 1								



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	Vert(LL) 0.01	MT20	244/190
TCDL 7.0	Rep Stress Incr YES	BC 0.08	Vert(TL) 0.01	Weight: 49 lb	
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Horz(TL) 0.00		
BCLD 5.0	Code FBC2001/ANSI95	(Matrix)	in (loc)		

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=66/0-3-8, 9=62/0-3-8, 12=400/0-3-8, 11=203/0-3-8, 10=398/0-3-8
 Max Horiz 1=98(load case 4)
 Max Uplift=30(load case 6), 9=47(load case 6), 12=178(load case 4), 11=176(load case 3), 10=146(load case 6)
 Max Grav 1=79(load case 7), 9=73(load case 6), 12=400(load case 5), 11=226(load case 8), 10=398(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=86/90, 2-3=88/173, 7-8=54/175, 8-9=35/29, 3-4=6/103, 4-5=6/103, 5-6=6/103, 6-7=6/103
 BOT CHORD 2-12=103/112, 11-12=103/112, 10-11=103/112, 8-10=103/112
 WEBS 4-12=269/206, 5-11=151/192, 6-10=269/207

NOTES:
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-99, 120mph (3-second gust), n=20ft, TCCL=4.2psf, TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSITF1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1, 47 lb uplift at joint 9, 178 lb uplift at joint 12, 176 lb uplift at joint 11 and 146 lb uplift at joint 10.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSIT/F1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 27 lb uplift at joint 9, 102 lb uplift at joint 11, 237 lb uplift at joint 12 and 220 lb uplift at joint 10.

NOTES

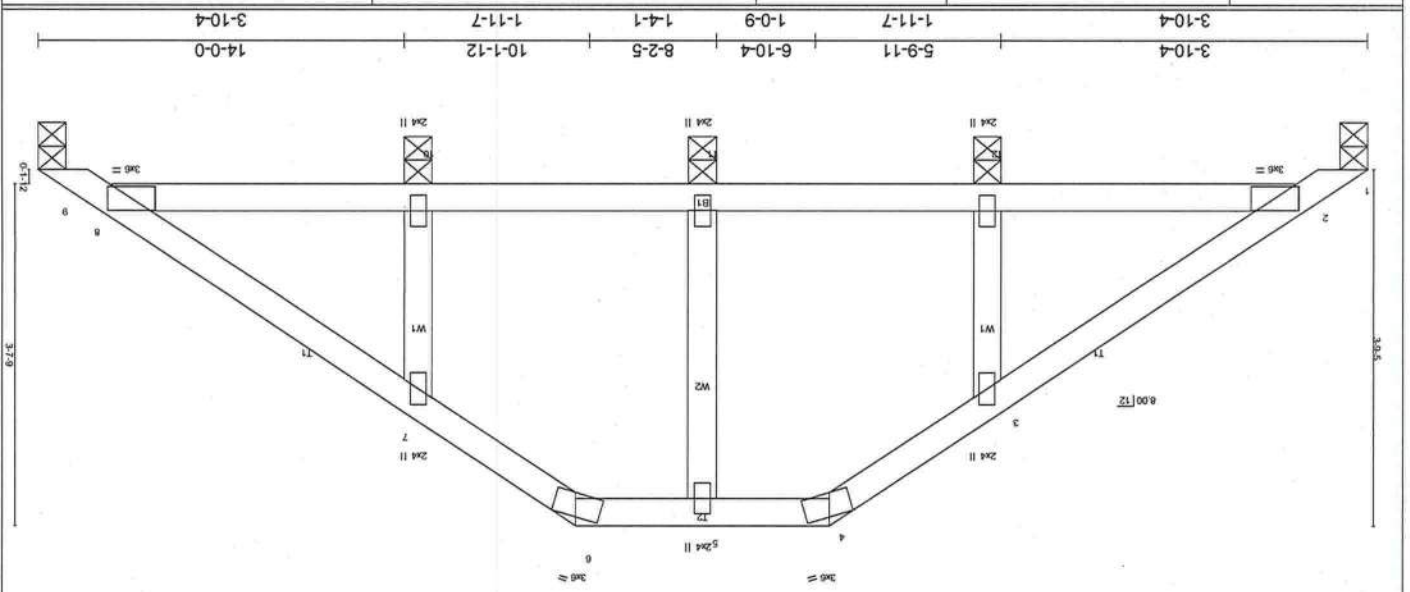
- FORCES** (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-144/154, 2-3=-134/149, 3-4=0/84, 6-7=0/80, 7-8=-90/149, 8-9=-44/18, 4-5=0/73, 5-6=0/73
 BOT CHORD 2-12=-72/146, 11-11=-72/146, 10-11=-72/146, 8-10=-72/146
 WEBS 5-11=-207/124, 3-12=-217/229, 7-10=-217/220

REACTIONS (lb/size)

- 1=80/0-3-8, 9=80/0-3-8, 11=272/0-3-8, 12=349/0-3-8, 10=349/0-3-8
 Max Horiz =153(load case 4)
 Max Uplift=-34(load case 3), 9=27(load case 3), 11=-102(load case 4), 12=-237(load case 5), 10=-220(load case 6)
 Max Grav =92(load case 7), 9=92(load case 8), 11=272(load case 1), 12=349(load case 1), 10=349(load case 1)

LUMBER 2 X 4 SYP No.2D
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

LOADING (psf)	SPACING	CSI	DEFL	VERT (L)	HORIZ (TL)	PLATES	GRIP	MT20	Weight: 53 lb
BCDL 5.0	2-0-0	TC 0.14	in (loc)	0.01	0.01	L/D	244/190	240	
BCLL 10.0	Plates Increase 1.25	BC 0.08	in (loc)	2-12	2-12	L/D		180	
TCCL 7.0	Rep Stress Incr YES	WB 0.06	in (loc)	2-12	2-12	L/D		180	
TCDL 20.0	Code FBC2001/ANSI95			9	9			n/a	

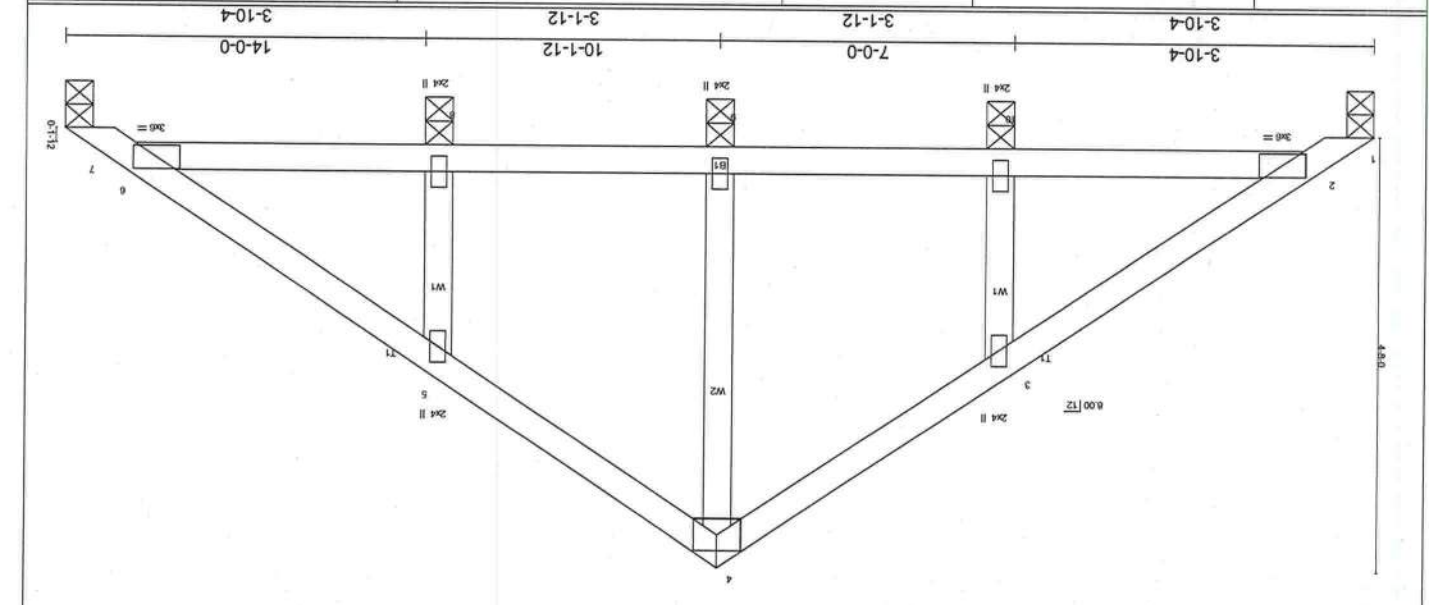


Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
L117440	PB03	HIP CAP	1		6.2005 Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:55 2005 Page 1

DWG.#062705823

Job	L117440	Truss	PB04	PIGgyBACK	4	1	Job Reference (optional)	6,200 S Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:56 2005 Page 1
Truss Type					Qty	Ply		

Builders FirstSource, Lake City, FL 32055
 Scale = 1/2" = 1'-0"



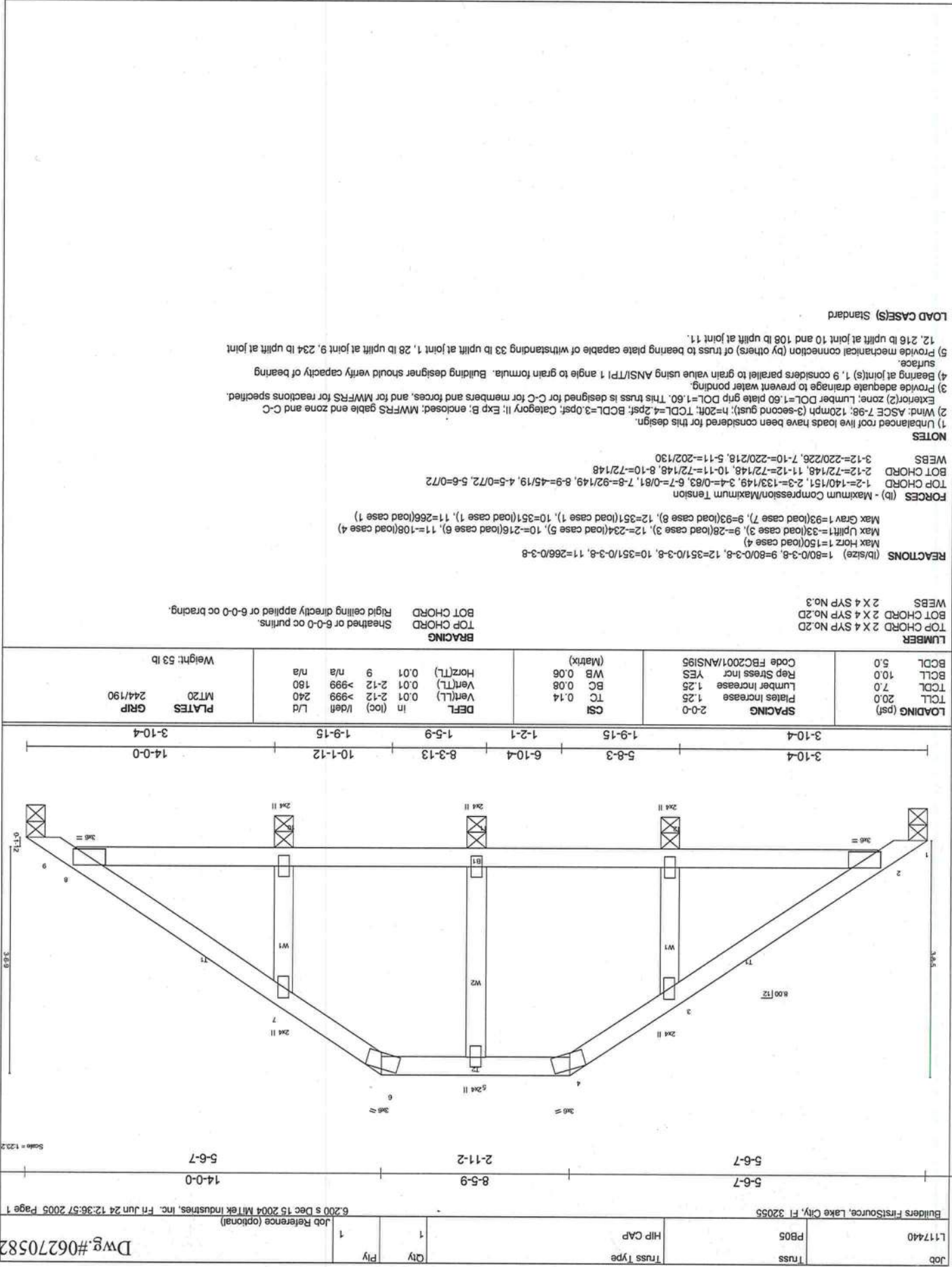
LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER
TOLL 20.0	2'-0-0	TC 0.13	in (loc)	TOP CHORD Sheathed or 6'-0-0 oc purlins.	TOP CHORD 2 X 4 SYP No.2D
TCDL 7.0	Plates Increase 1.25	BC 0.07	Vert(L) 0.01	BOT CHORD Rigid ceiling directly applied or 6'-0-0 oc bracing.	BOT CHORD 2 X 4 SYP No.3
BCLL 10.0	Rep Stress Incr YES	WB 0.09	Horz(TL) 0.00		WEBS
BCDL 5.0	Code FBC2001/ANSI95	(Maxk)			

REACTIONS (lb/size) 1=62/0-3-8, 7=62/0-3-8, 9=349/0-3-8, 10=328/0-3-8
 Max Horz 1=189(load case 4)
 Max Uplift 1=50(load case 3), 7=21(load case 6), 9=54(load case 5), 10=254(load case 5), 8=243(load case 6)
 Max Grav 1=75(load case 7), 7=75(load case 7), 9=349(load case 8), 10=336(load case 7), 8=336(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-192/193, 2-3=-158/189, 3-4=-49/168, 4-5=-13/168, 5-6=-103/189, 6-7=-36/15
 BOT CHORD 2-10=-107/155, 8-9=-107/155, 6-8=-107/155
 WEBS 4-9=-282/82, 3-10=-207/248, 5-8=-207/243

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft, TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate gfp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 21 lb uplift at joint 7, 54 lb uplift at joint 9, 254 lb uplift at joint 10 and 243 lb uplift at joint 8.

LOAD CASE(S) Standard



Job	L117440	Truss	PB05	HIP CAP	Qty	1	Job Reference (optional)	1
Truss Type					Ply			

Builders FirstSource, Lake City, FL 32055
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Dwg.#062705825

Scale = 1/32

Weight: 53 lb

PLATES	MT20	244/190
GRIP		

LOADING (psf)	TCLL	20.0
TCLD	7.0	
BCLL	10.0	
BCDL	5.0	

SPACING	2-0-0
Plates Increase	1.25
Lumber Increase	1.25
Rep Stress Incr	YES
Code	FBC2001/ANSI95

CSI	TC	0.14
BC	0.08	
WB	0.06	

DEFL	in (loc)	l/d	L/D
Vert(L)	0.01	2-12	>999
Vert(R)	0.01	2-12	>999
Horz(TL)	0.01	9	n/a
Horz(TR)	0.01	9	n/a

BRACING	TOP CHORD	Sheathed or 6-0 oc purlins
	BOT CHORD	Rigid ceiling directly applied or 6-0 oc bracing

LUMBER	TOP CHORD	2 X 4 SYP No.2D
	BOT CHORD	2 X 4 SYP No.2D
	WEBS	2 X 4 SYP No.3

REACTIONS (lb/size) 1=80/0-3-8, 9=80/0-3-8, 12=351/0-3-8, 10=351/0-3-8, 11=266/0-3-8
 Max Horiz =150(load case 4)
 Max Uplift =33(load case 3), 9=28(load case 3), 12=234(load case 5), 10=216(load case 5), 11=108(load case 4)
 Max Grav =93(load case 7), 9=93(load case 8), 12=351(load case 8), 10=351(load case 1), 11=266(load case 1)

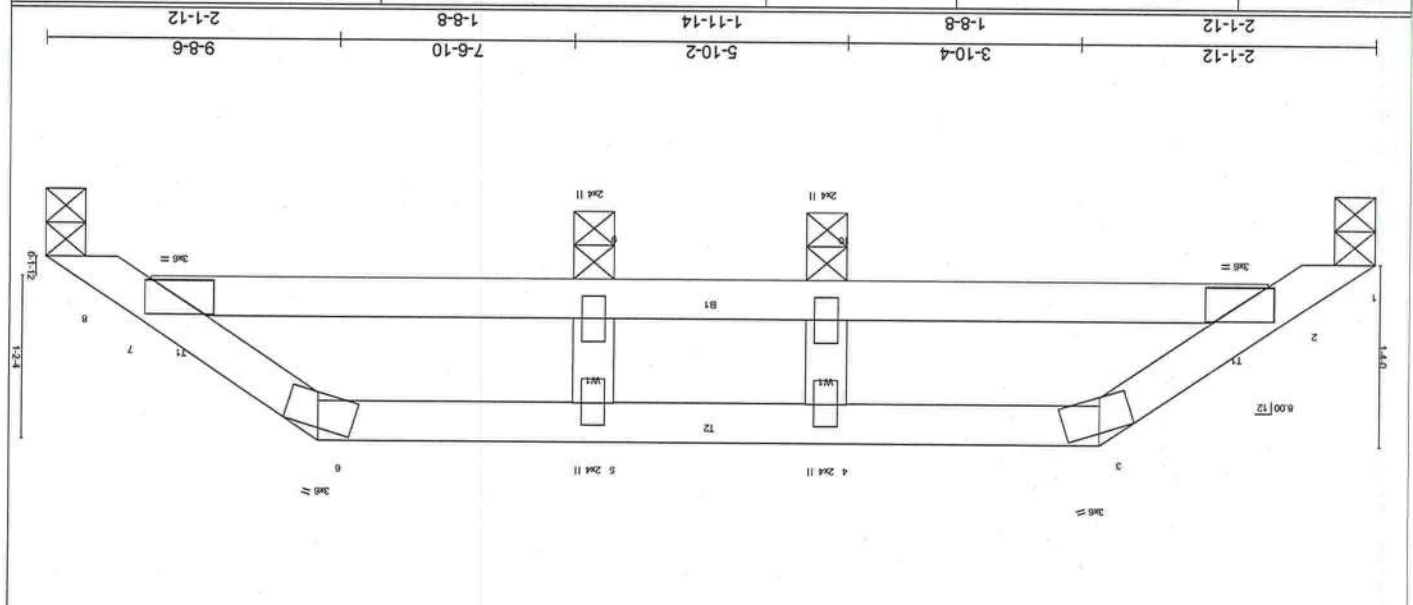
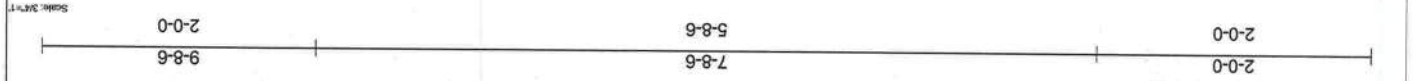
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=140/151, 2-3=133/149, 3-4=0/83, 6-7=0/81, 7-8=-92/149, 8-9=-45/19, 4-5=0/72, 5-6=0/72
 BOT CHORD 2-12=72/148, 10-11=72/148, 8-10=72/148
 WEBS 3-12=220/226, 7-10=220/218, 5-11=202/130

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior zone; Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1, 28 lb uplift at joint 9, 234 lb uplift at joint 11.
 12, 216 lb uplift at joint 10 and 108 lb uplift at joint 11.

LOAD CASE(S) Standard

Job	L17440	Truss	PB06	HIP PIGGYBACK	1	1	1	Job Reference (optional)
Dwg.#	062705826							

Builders FirstSource, Lake City, FL 32055 6:200 s Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:57 2005 Page 1

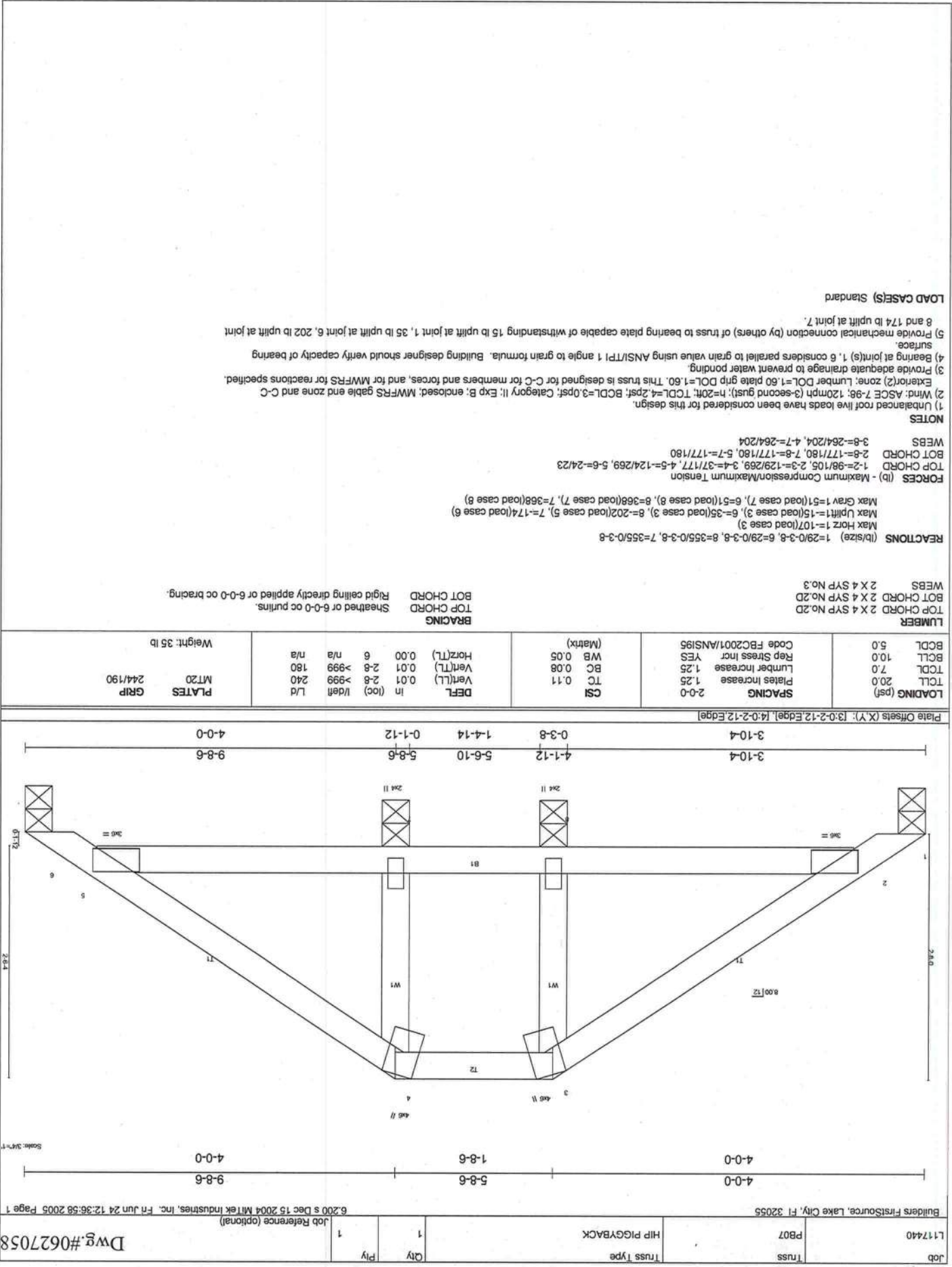


LOADING (psf)	SPACING		DEFL		BRACING		Weight: 30 lb
	PLATES INCREASE	Rep Stress Incr	Code FBC2001/ANSI95	CS	TC	BC	
20.0	1.25	YES	FBC2001/ANSI95	0.14	0.10	0.04	244/190
7.0	1.25	YES		0.01	0.01	0.01	MT20
10.0	1.25	YES		0.01	0.01	0.01	GRIP
5.0	2.0-0			8	8	8	

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.3
WEBS
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.3
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=162/0-3-8, 8=162/0-3-8, 10=221/0-3-8, 9=221/0-3-8
 Max Horiz 1=52(load case 3)
 Max Uplift 1=67(load case 5), 8=71(load case 6), 10=-152(load case 4), 9=-145(load case 3)
 Max Grav 1=162(load case 1), 8=162(load case 1), 10=242(load case 7), 9=242(load case 8)
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-80/61, 2-3=-166/151, 3-4=-136/151, 4-5=-136/151, 5-6=-136/151, 6-7=-166/151, 7-8=-80/61
 BOT CHORD 2-10=-86/136, 9-10=-86/136, 7-9=-86/136
 WEBS 4-10=-137/144, 5-9=-137/144

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft, TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANS/TF1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1, 71 lb uplift at joint 8, 152 lb uplift at joint 10 and 145 lb uplift at joint 9.
LOAD CASE(S) Standard



LOADING (psf)		SPACING		CSI		DEFL		BRACING	
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	In (loc)	L/d
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	6	>999
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	n/a	180
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)				n/a	n/a
LUMBER		TOP CHORD 2 X 4 SYP No.2D		TOP CHORD		Sheathed or 6-0-0 oc purlins		Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS		2 X 4 SYP No.3		BOT CHORD					

REACTIONS (lb/size) 1=29/0-3-8, 6=29/0-3-8, 8=355/0-3-8, 7=355/0-3-8
 Max Horiz 1=-107(load case 3)
 Max Uplift 1=-15(load case 3), 6=-55(load case 3), 8=-202(load case 5), 7=-174(load case 6)
 Max Grav 1=51(load case 7), 6=51(load case 8), 8=366(load case 7), 7=366(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-98/105, 2-3=-129/269, 3-4=-37/177, 4-5=-124/269, 5-6=-24/23
 BOT CHORD 2-8=-177/180, 7-8=-177/180, 5-7=-177/180
 WEBS 3-8=-264/204, 4-7=-264/204

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98: 120mph (3-second gust); n=20ft; TC DL=4; 2psf; BC DL=3; 0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 35 lb uplift at joint 6, 202 lb uplift at joint 8 and 174 lb uplift at joint 7.
 LOAD CASE(S) Standard

LOAD CASES (Standard)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extent(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 19 lb uplift at joint 7, 230 lb uplift at joint 9 and 203 lb uplift at joint 8.

NOTES

- FORCES** (lb) - Maximum Compression/Maximum Tension
- TOP CHORD 1-2=-126/123, 2-3=-163/240, 3-4=-31/124, 4-5=-38/124, 5-6=-163/240, 6-7=-28/14
- BOT CHORD 2-9=-149/212, 6-9=-149/212
- WEBS 3-9=-253/217, 5-8=-253/208
- Max Horiz 1=-130(load case 3), 9=-230(load case 5), 8=-203(load case 6)
- Max Uplift=-130(load case 3), 7=-19(load case 3), 9=-230(load case 5), 8=-203(load case 6)
- Max Grav 1=60(load case 7), 7=60(load case 7), 9=350(load case 8), 8=350(load case 8)

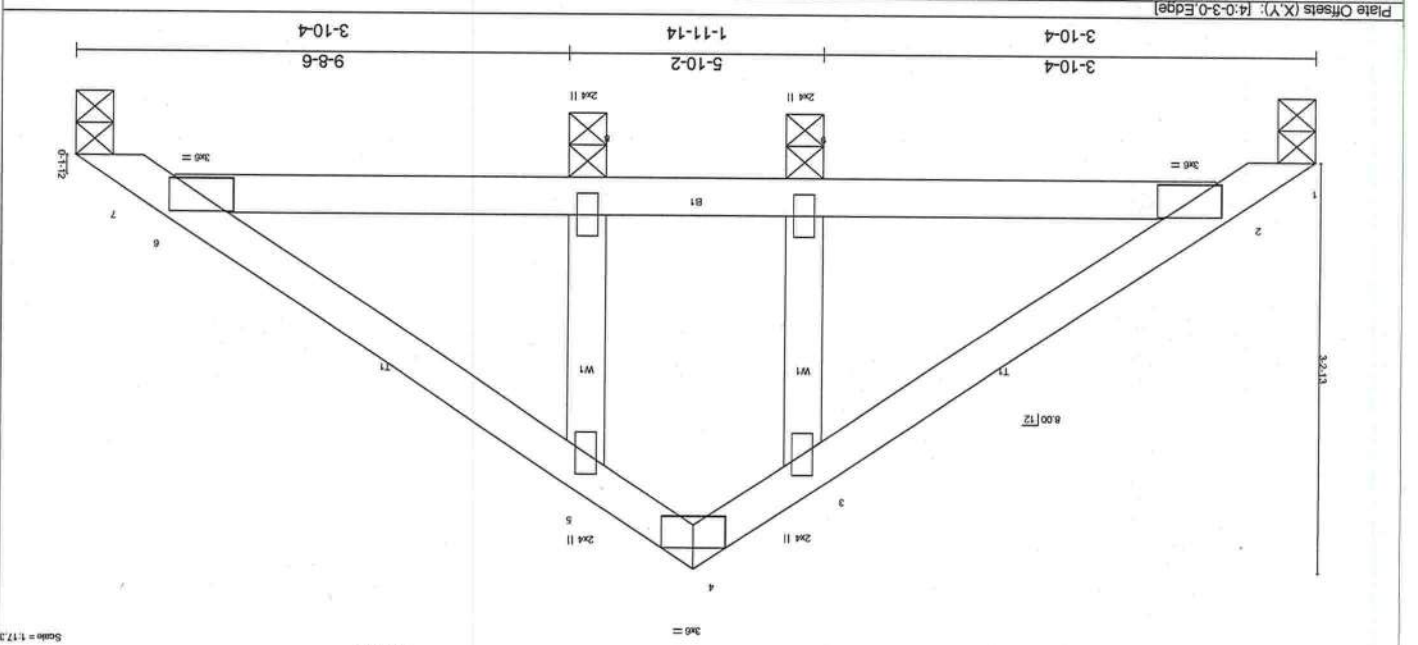
REACTIONS (lb/size)

- 1=40/0-3-8, 7=40/0-3-8, 9=343/0-3-8, 8=343/0-3-8

LUMBER 2 X 4 SYP No.2D
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

LOADING (psf)	20.0	TCDL	7.0	SPACING	2-0-0
TCDL	7.0	Plates Increase	1.25	CSI	TC 0.12
TOLL	20.0	Lumber Increase	1.25	WB	0.05
BCLL	10.0	Rep Stress Incr	YES	BC	0.08
BCLD	5.0	Code FBC2001/ANSI95		Matrix	



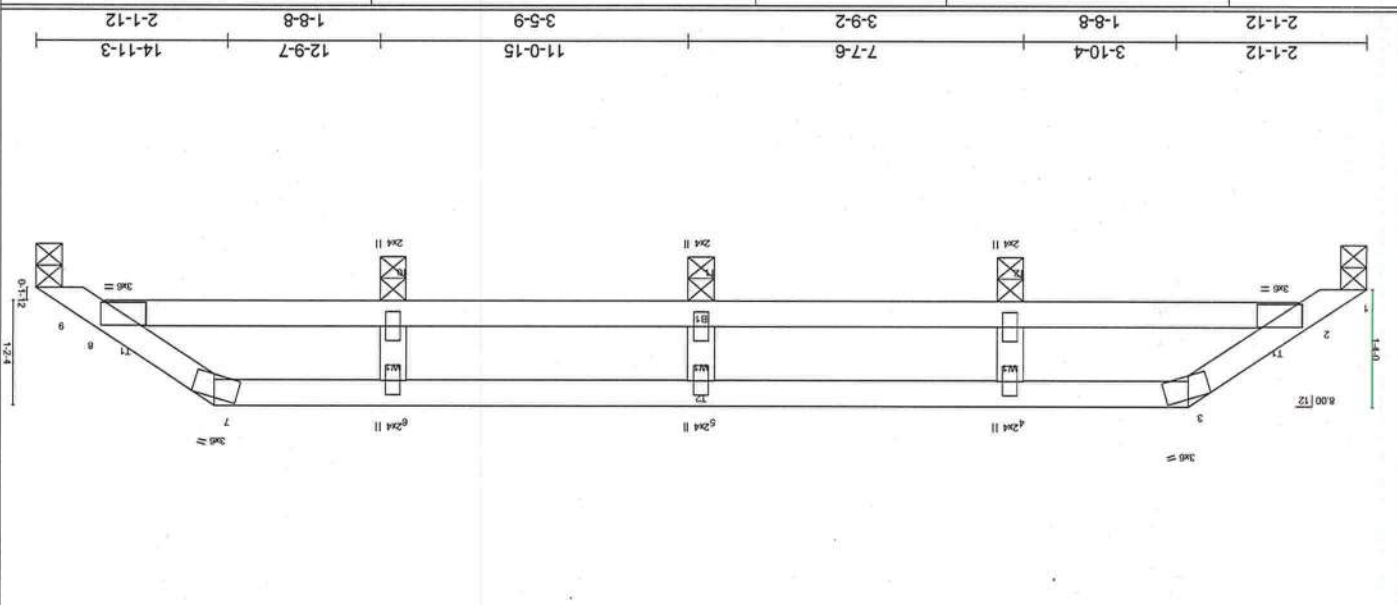
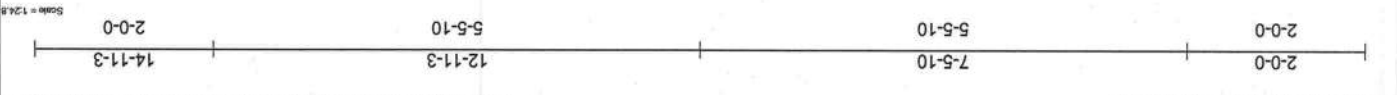
0-0	4-10-3	9-8-6	4-10-3	3-10-4	5-10-2	1-1-14	3-10-4
3-0	4-10-3	9-8-6	4-10-3	3-10-4	5-10-2	1-1-14	3-10-4

Job	L117440	Truss	PB08	Truss Type	PIGYPACK	Qty	3	Job Reference (optional)	6:200 s Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:58 2005 Page 1
Dwg.#	#062705828								

Builders FirstSource, Lake City, FL 32055

Scale = 1/17.3

Job	Truss	Truss type	HIP PIGGYBACK	Qty	1	Job Reference (optional)	Dwg.#062705830
L117440	PB10			Ply	1	6/2005 Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:36:59 2005 Page 1	



LOADING (psf)	SPACING	CSJ	DEFL	in (loc)	W/delt	L/d	PLATES	GRIP
20.0	2-0-0	0.12	0.01	2-12	>999	240	MT20	244/190
7.0	Lumber increase	1.25	0.01	2-12	>999	180		
10.0	Rep Stress Incr	YES	0.01	2-12	>999	180		
5.0	Code FBC2001/ANSI95	WB 0.05	0.01	9	n/a	n/a		

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=143/0-3-8, 9=143/0-3-8, 12=327/0-3-8, 11=266/0-3-8, 10=327/0-3-8
 Max Horiz 1=52(load case 4)
 Max Uplift 1=62(load case 5), 9=67(load case 6), 12=189(load case 6), 11=180(load case 3), 10=177(load case 3)
 Max Grav 1=143(load case 1), 9=143(load case 1), 12=330(load case 7), 11=272(load case 7), 10=330(load case 8)

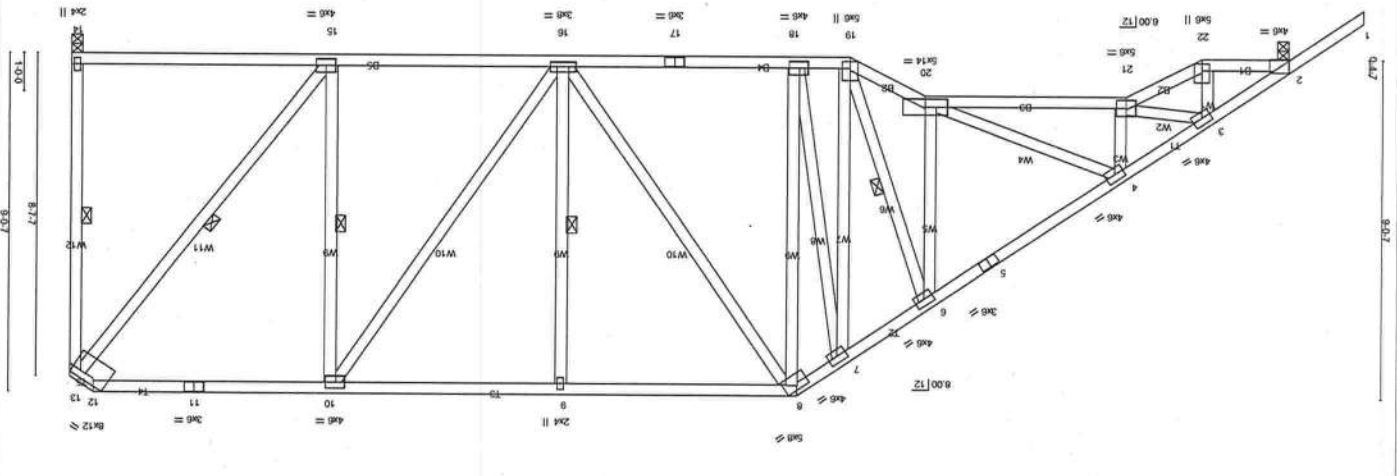
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-70/54, 2-3=-116/89, 3-4=-91/94, 4-5=-91/94, 5-6=-91/94, 6-7=-91/94, 7-8=-116/89, 8-9=-70/47
 BOT CHORD 2-12=-51/91, 11-12=-51/91, 10-11=-51/91, 8-10=-51/91
 WEBS 4-12=-194/192, 5-11=-183/203, 6-10=-194/185

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60; This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide adequate drainage to prevent water ponding.
 (4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSIT/P1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 11 and 177 lb uplift at joint 10.
 (6) Bearing at joint 11, 67 lb uplift at joint 9, 189 lb uplift at joint 10.
 (7) 12, 180 lb uplift at joint 11 and 177 lb uplift at joint 10.
 Standard

Job	L177440	Truss	T04	SPECIAL	1	Qty	1	Ply	1
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2-0-0	2-3-8	2-3-8	4-3-8	9-7-8	11-7-8	13-0-0	1-4-8	6-3-5	6-1-9	25-4-13	31-8-2	6-3-5	32-3-10	0-7-8
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2-3-8	4-3-8	9-7-8	11-7-8	13-0-0	1-4-8	6-3-5	6-1-9	25-4-13	32-3-10	6-10-13
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LOADING (psf)	TCLL	TCDL	BCLL	BCDL	SPACING	CS	DEFL	VERT(L)	HORIZ(TL)	WEBS	TOP CHORD	BOT CHORD	PLATES	GRP
2.0-0	20.0	1.25	1.25	1.25	2.0-0	0.41	-0.16	20-21	>999	240	L/D	>999	MT20	244/190
2.0-0	20.0	1.25	1.25	1.25	2.0-0	0.41	-0.23	20-21	>999	180	L/D	>999	MT20	244/190
5.0	5.0	10.0	10.0	5.0	Code FBC2001/ANSI95	WB 0.72	0.10	14	n/a	n/a	n/a	n/a		

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3 Except
 W12 2 X 4 SYP No.2D

BRACING
 TOP CHORD Sheathed or 3-8-3 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-3-11 oc bracing.
 WEBS 1 Row at midpt
 6-19, 9-16, 10-15, 13-14, 13-15

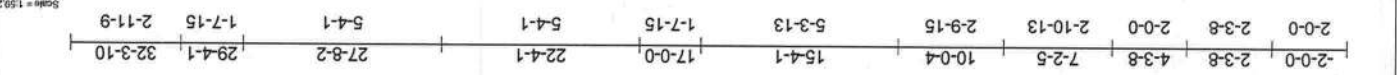
REACTIONS
 (lb/size) 2=1464/0-3-8, 14=1341/0-3-8
 Max Horiz Z=567(load case 5)
 Max Uplift Z=603(load case 5), 14=740(load case 4)

FORCES
 (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=2037/724, 3-4=3044/1440, 4-5=2058/949, 5-6=1954/961, 6-7=1575/839, 7-8=1374/747, 8-9=1207/678, 9-10=1207/678,
 10-11=847/483, 11-12=847/483, 13-14=1248/756
 BOT CHORD 2-22=935/1586, 21-22=991/1715, 20-21=1520/2483, 19-20=1111/1902, 18-19=726/1249, 17-18=689/1206, 16-17=689/1206,
 15-16=485/847, 14-15=7/9
 WEBS 3-22=759/513, 3-21=671/1044, 4-21=277/670, 4-20=890/575, 6-20=659/1280, 6-19=1344/843, 7-19=354/436, 7-18=290/302,
 8-18=262/443, 8-16=234/149, 9-16=337/349, 10-16=361/626, 10-15=837/641, 13-15=766/1340

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98; (3-second gust): h=20ft, TCCL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone: Lumber DOL=1.60 plate grp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide adequate drainage to prevent water ponding.
 (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 603 lb uplift at joint 2 and 740 lb uplift at joint 14.
 LOAD CASE(S) Standard

Job	L117440	Truss	T06	SPECIAL	Qty	1	Job Reference (optional)	1
Dwg.#	062705837							

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LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2001/ANSI95	CSI	DEFL	VERT(L)	HORIZ(TL)	WEBS	TOP CHORD	BRACING
2.0	2.0-0	1.25	1.25	YES	2.0-0	TC 0.75	in (loc)	0.09	0.09	WEBS	TOP CHORD	Sheathed or 3-8 oc purlins, except end verticals.
2.0	2.0-0	1.25	1.25	YES	2.0-0	BC 0.59	in (loc)	15-17	15-17	WEBS	TOP CHORD	Rigid ceiling directly applied or 5-5-0 oc bracing.
7.0	2.0-0	1.25	1.25	YES	2.0-0	WB 0.90	in (loc)	14	14	WEBS	TOP CHORD	1 Row at midpt
10.0	2.0-0	1.25	1.25	YES	2.0-0	WB 0.90	in (loc)	n/a	n/a	WEBS	TOP CHORD	7-17, 10-15, 11-15, 13-14
5.0	2.0-0	1.25	1.25	YES	2.0-0	WB 0.90	in (loc)	n/a	n/a	WEBS	TOP CHORD	Weight: 245 lb

REACTIONS (lb/size)	FORCES (lb)	TOP CHORD	WEBS
2=146/0-3-8, 14=134/0-3-8	1-2=0/59, 2-3=2055/721, 3-4=2993/1404, 4-5=2313/1089, 5-6=1693/788, 6-7=1617/796, 7-8=1247/659, 12-13=634/353, 13-14=1322/694, 8-9=1031/644, 9-10=1031/644, 10-11=530/357, 11-12=530/357	1-2=0/59, 2-3=2055/721, 3-4=2993/1404, 4-5=2313/1089, 5-6=1693/788, 6-7=1617/796, 7-8=1247/659, 12-13=634/353, 13-14=1322/694, 8-9=1031/644, 9-10=1031/644, 10-11=530/357, 11-12=530/357	1-2=0/59, 2-3=2055/721, 3-4=2993/1404, 4-5=2313/1089, 5-6=1693/788, 6-7=1617/796, 7-8=1247/659, 12-13=634/353, 13-14=1322/694, 8-9=1031/644, 9-10=1031/644, 10-11=530/357, 11-12=530/357

Max Horiz Z=605(load case 5)
 Max Uplift Z=623(load case 5), 14=577(load case 4)

TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3 Except
 W12 2 X 4 SYP No.2D

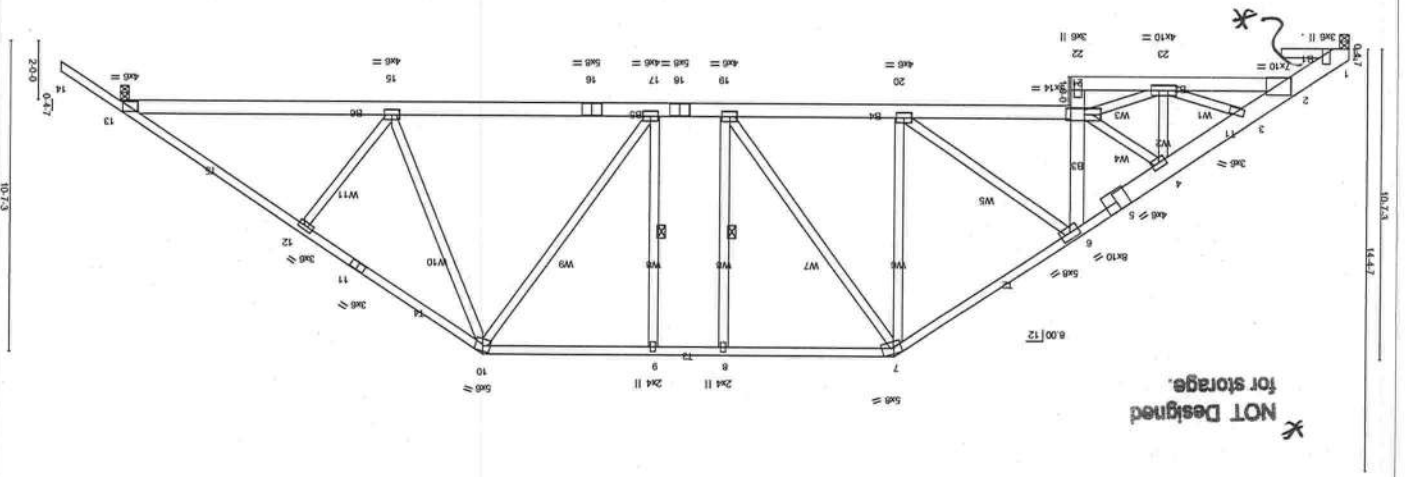
NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98: 120mph (3-second gust); n=20ft, TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate grp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide adequate drainage to prevent water ponding.
 (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 2 and 577 lb uplift at joint 14.

LOAD CASE(S) Standard

Dwg.#062705839

Job	L117440	T08	Tuss	SPECIAL	Qty	1	Job Reference (optional)
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2-0-0	2-3-8	9-5-3	7-1-11	15-2-10	5-9-6	0-1-8	21-0-0	23-8-2	29-4-1	29-6-8	35-4-14	5-10-6	6-3-4	43-8-2	2-0-0
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2-3-8	2-3-8	9-5-3	7-1-11	15-2-10	5-9-6	5-9-6	21-0-0	23-8-2	32-5-11	8-9-9	41-8-2	9-2-7
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LOADING (psf)	TCLL	20.0	SPACING	2.0-0	CSL	0.82	DEFL	in (loc)	L/d	240	PLATES	MT20	GRIP	244/190
TCDL	7.0	Lumber Increase	1.25	TC	0.82	Vert(TL)	-0.27	20-21	>999	240	PLATES	MT20	GRIP	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horiz(TL)	0.22	13	n/a	180	PLATES	MT20	GRIP	244/190
BCDL	5.0	Code FBC2001/ANSI95		BC	0.82				n/a	180	PLATES	MT20	GRIP	244/190

LUMBER
 TOP CHORD 2 X 8 SYP No.22 "except"
 BOT CHORD 2 X 6 SYP No.1D "except"
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 3-4" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-10-5 oc bracing.
 WEBS 1 Row at midpt 8-19, 9-17

REACTIONS (lb/size) 1=170/10-3-8, 13=1856/0-3-8
 Max Horiz=388(load case 4)
 Max Uplift=607(load case 5), 13=713(load case 6)

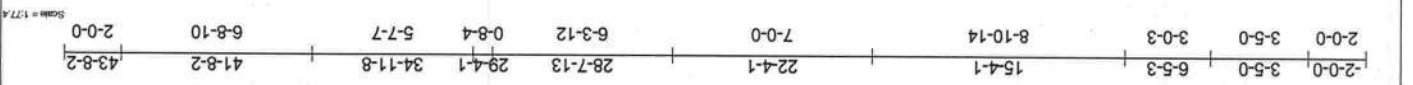
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-864/439, 2-3=-4369/2036, 3-4=-3631/1697, 4-5=-3570/1663, 5-6=-3523/1679, 6-7=-2592/1310, 10-11=-2426/1262, 11-12=-2515/1236,
 12-13=-2710/1248, 13-14=0/62, 7-8=-2177/1225, 8-9=-2177/1225, 9-10=-2177/1225
 BOT CHORD 2-23=-1902/4214, 22-23=-333/658, 21-22=-231/04, 6-21=-396/1023, 20-21=-1325/2979, 19-20=-917/2070, 18-19=-894/2177,
 17-18=-894/2177, 16-17=-601/1823, 15-16=-601/1823, 13-15=-806/2172
 WEBS 6-20=-1084/665, 7-20=-389/882, 7-19=-442/379, 8-19=-233/286, 9-17=-267/303, 10-17=-507/731, 10-15=-246/555, 12-15=-265/365,
 4-23=-123/108, 4-21=-154/189, 21-23=-1062/2471, 3-23=-1307/667

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide adequate drainage to prevent water ponding.
 (4) Beaming at joint(s) 1 considers parallel to grain value using ANSITF1 angle to grain formula. Building designer should verify capacity of beaming surface.
 (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 807 lb uplift at joint 1 and 713 lb uplift at joint 13.
 LOAD CASE(S) Standard

Job	L17440	Truss	T09	SPECIAL	Qty	1	Job Reference (optional)	1
Dwg.#	062705840							

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LOADING (psf)	TOLL	TCDL	BCLL	BCDL	SPACING	CSI	DEFL	BRACING	LUMBER	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS	NOTES
20.0	2.0	7.0	10.0	5.0	2'-0"	TC 0.50	Vert(LL) -0.40 Vert(TL) -0.57 Horz(TL) 0.23	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3	Max Horiz 2=424(load case 4) Max Uplift2=760(load case 5), 11=712(load case 6)	(b) - Maximum Compression/Maximum Tension 1'-2-0/59, 2'-3-2=2758/1177, 3'-4-4=4871/2052, 4'-5-4=872/2116, 5'-6-2=2539/1261, 6'-8-9=2356/1274, 9'-10-2=2509/1250, 10'-11-2=2683/1215, 11'-2-0/59, 6'-7-2=2192/1221, 7'-8-2=2192/1221, 2'-2-2=92/2199, 2'-2-2=61/31, 2'-2-2=0/0, 1'-9-20=0/0, 2'-1-22=0/183, 4'-2-1=79/100, 1'-8-21=1319/2947, 1'-7-18=1319/2947, 1'-6-17=897/2053, 1'-5-16=897/2053, 1'-4-15=599/1800, 1'-3-14=599/1800, 1'-1-13=765/2134, 3'-2-1=739/1814, 5'-17=1082/657, 6'-15=418/378, 7'-15=396/434, 8'-15=473/711, 8'-13=278/588, 10'-13=256/363, 18-20=71/0, 6'-17=319/827, 5'-21=725/1961	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 760 lb uplift at joint 2 and 712 lb uplift at joint 11.			

PLATE OFFSETS (X,Y):	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
[2-0-0-0-0-4], [5-0-3-0-0-3-0], [6-4-0-0-1-9], [11-0-0-0-0-4]	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
MT20	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

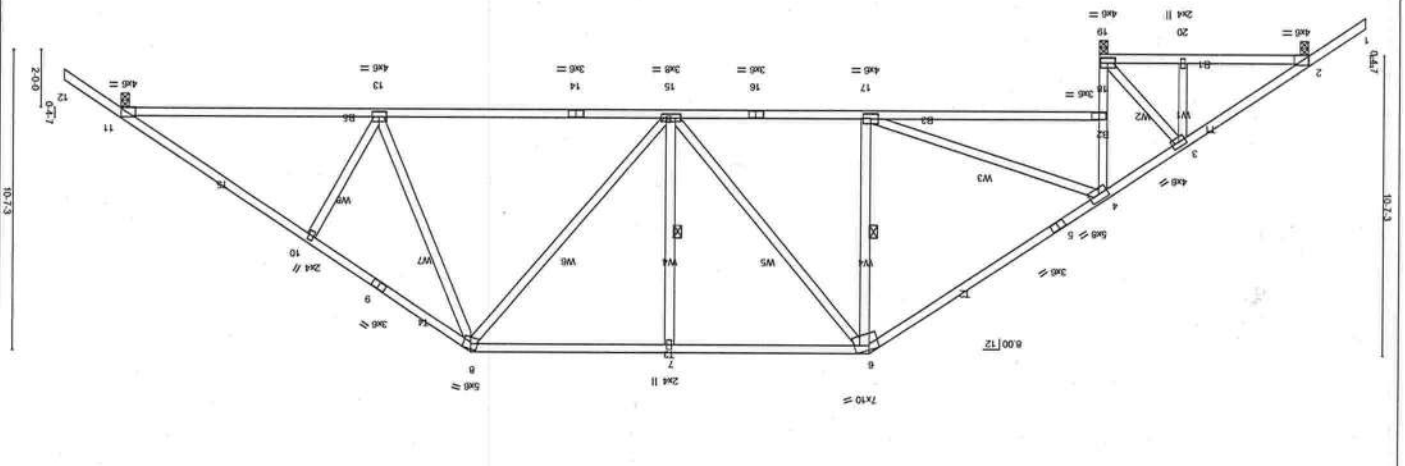
PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

PLATES	WEBS	TOP CHORD	BOT CHORD	WEBS	DEFL	BRACING	LOADING
GRP	2 X 4 SYP No.3	2 X 4 SYP No.2D	2 X 4 SYP No.2D	2 X 4 SYP No.3	TC 0.50	TOP CHORD Rigid ceiling directly applied or 5'-7-13 oc bracing. Sheathed or 2'-9-5 oc purlins.	20.0 7.0 10.0 5.0

Job	L117440	Truss	T10	SPECIAL	2	Qty	1	Ply
Dwg #	062705841							

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2-0-0	7-4-3	15-4-1	22-4-1	7-0-0	6-3-12	0-8-4	5-7-7	6-8-10	2-0-0
2-0-0	7-4-3	15-4-1	22-4-1	7-0-0	28-7-13	29-4-1	34-1-8	41-8-2	43-8-2



2-0-0	7-4-3	15-4-1	22-4-1	7-0-0	10-3-5	41-8-2	9-0-12
2-0-0	7-4-3	15-4-1	22-4-1	7-0-0	32-7-6	41-8-2	9-0-12

LOADING (psf)	20.0	TOLL	7.0	SPACING	2.0-0	DEFL	in (loc)	13.15	VERT(L)	0.25	VERT(R)	-0.36	CSI	WB	0.85	Code	FBC2001/ANSI95	
TOP CHORD	2 X 4 SYP No.2D	TOP CHORD	2 X 4 SYP No.2D Except	TOP CHORD	2 X 4 SYP No.3	WEBS	2 X 4 SYP No.3	BRACING	TOP CHORD	Sheathed or 4-2-14 oc purlins	TOP CHORD	Rigid ceiling directly applied or 4-4-1 oc bracing	WEBS	1 Row at midpt	WEBS	7-15, 6-17	WEBS	2 X 4 SYP No.3

REACTIONS	(lb/size)	2-378/0-3-8, 11=1551/0-3-8, 19=1780/0-3-8	Max Horiz 2=424/0-3-8 (load case 4)	Max Uplift 2=292/0-3-8 (load case 6), 11=679/0-3-8 (load case 7), 19=1780/0-3-8 (load case 1)	FORCES	(lb) - Maximum Compression/Maximum Tension	TOP CHORD	1-2=0/59, 2-3=299/392, 3-4=275/429, 4-5=-1438/769, 5-6=-1315/803, 8-9=-1820/1066, 9-10=-1974/1042, 10-11=-2146/1007, 11-12=0/59	TOP CHORD	6-7=-1487/946, 7-8=-1487/946	TOP CHORD	2-20=95/110, 19-20=95/110, 18-19=-1589/696, 4-18=-1455/720, 17-18=-110/69, 16-17=-493/1091, 15-16=-493/1091, 14-15=-410/1353, 13-14=-410/1353, 11-13=-594/1693	WEBS	4-17=-439/113, 6-15=-423/693, 7-15=-397/441, 8-15=-302/331, 8-13=-279/593, 10-13=-263/363, 3-20=0/135, 3-19=-232/89, 6-17=-156/188	NOTES	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 679 lb uplift at joint 11 and 760 lb uplift at joint 19. LOAD CASES) Standard
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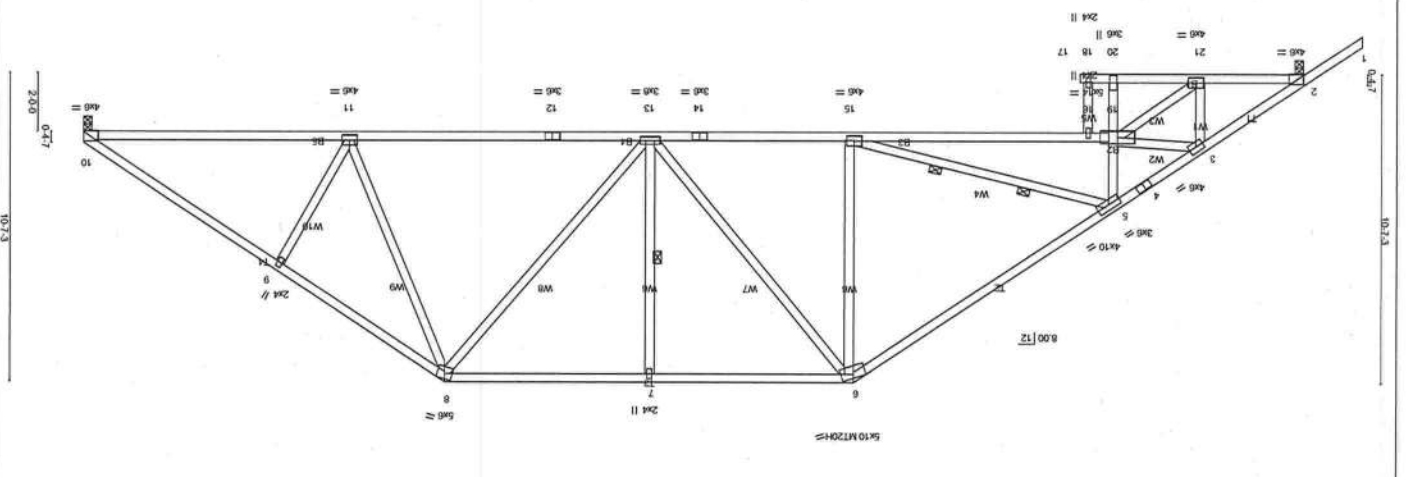
Job	L117440	Truss	T11	SPECIAL	Qty	1	Job Reference (optional)	1
Truss Type								

Dwg.#062705842

Builders FirstSource, Lake City, FL 32055

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2-0-0	3-6-4	6-7-10	3-1-6	8-8-7	15-4-1	7-0-0	22-4-1	28-7-13	6-3-12	0-8-4	5-7-7	41-8-2	6-8-10
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3-6-4	6-7-10	3-1-6	8-8-7	15-4-1	22-4-1	7-0-0	32-7-6	10-3-5	41-8-2	9-0-12
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LOADING (psf)	2.0-0	SPACING	2.0-0	Code FBC2001/ANSI95	Rep Stress Incr	YES	TC	0.70	DEFL	in (loc)	Vert(UL)	-0.38	15-16	>999	240	MT20	244/190	GRIP	187/143
TCLL	20.0	Plates Increase	1.25	Lumber Increase	1.25	BC	0.94	Horz(TL)	0.26	10	15-16	>917	180	MT20H	187/143	Weight:	247 lb		
TCDL	7.0	Plates Increase	1.25	BC	0.94	WB	0.96	Vert(UL)	-0.54	15-16	>917	180	MT20H	187/143	Weight:	247 lb			
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	BC	0.94	Horz(TL)	0.26	10	n/a	n/a	MT20	244/190	Weight:	247 lb			
BCLD	5.0	Code FBC2001/ANSI95	2.0-0	WB	0.96	BC	0.94	Horz(TL)	0.26	10	n/a	n/a	MT20	244/190	Weight:	247 lb			

LUMBER	TOP CHORD	2 X 4 SYP No.2D	BRACING	TOP CHORD	Sheathed or 2-10-9 oc purlins
BOT CHORD	2 X 4 SYP No.2D	BOT CHORD	TOP CHORD	Rigid ceiling directly applied or 4-9-13 oc bracing	WEBS
WEBS	2 X 4 SYP No.3	WEBS	BOT CHORD	1 Row at midpt	7-13
			WEBS	2 Rows at 1/3 pts	5-15

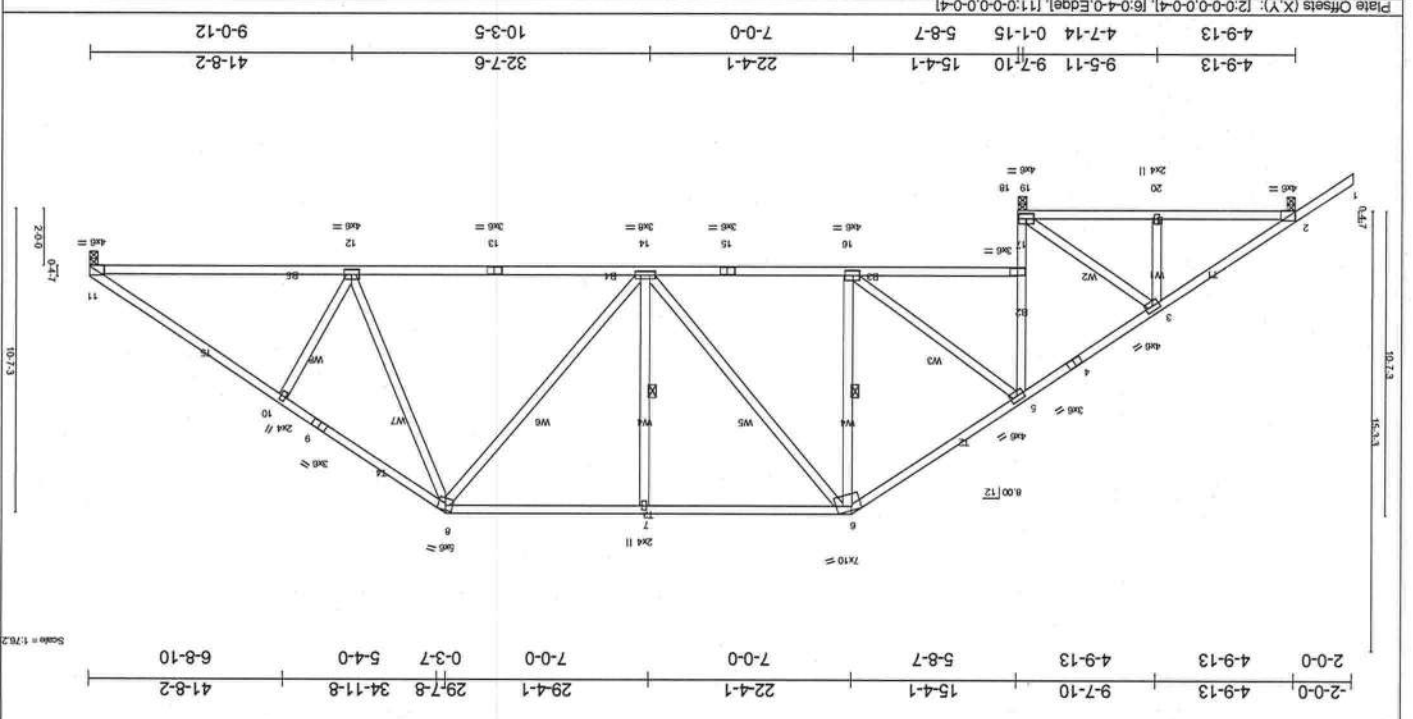
REACTIONS (lb/size)	10=1737/0-3-8, 2=1864/0-3-8	FORCES (lb)	Maximum Compression/Maximum Tension
Max Horiz 2=451 (load case 4)	Max Uplift 10=-564 (load case 6), 2=-760 (load case 5)	TOP CHORD	1-2=0/59, 2-3=-2765/1174, 3-4=-4783/2117, 4-5=-4712/2122, 5-6=-2604/1271, 6-9=-2534/1329, 9-10=-2687/1268, 6-7=-2197/1242, 7-8=-2197/1242
		BOT CHORD	2-21=-1011/2198, 20-21=-178/234, 18-20=0/0, 17-18=0/0, 19-20=-180/245, 5-19=-499/1463, 16-19=-1867/4121, 15-16=-1857/4112, 14-15=-940/2070, 13-14=-940/2070, 12-13=-658/1812, 11-12=-658/1812, 10-11=-921/2160
		WEBS	16-18=-233/269, 6-15=-254/768, 6-13=-429/379, 3-21=-1346/630, 19-21=-1050/2420, 3-19=-757/1776, 5-15=-2126/1150, 7-13=-391/446, 8-13=-468/703, 8-11=-319/610, 9-11=-269/366

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grp DOL=1.60.
 (3) Provide adequate drainage to prevent water ponding.
 (4) All plates are MT20 plates unless otherwise indicated.
 (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 564 lb uplift at joint 10 and 760 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	L117440	Truss	T12	SPECIAL	Qty	1	Job Reference (optional)	1
Dwg.#	062705843							

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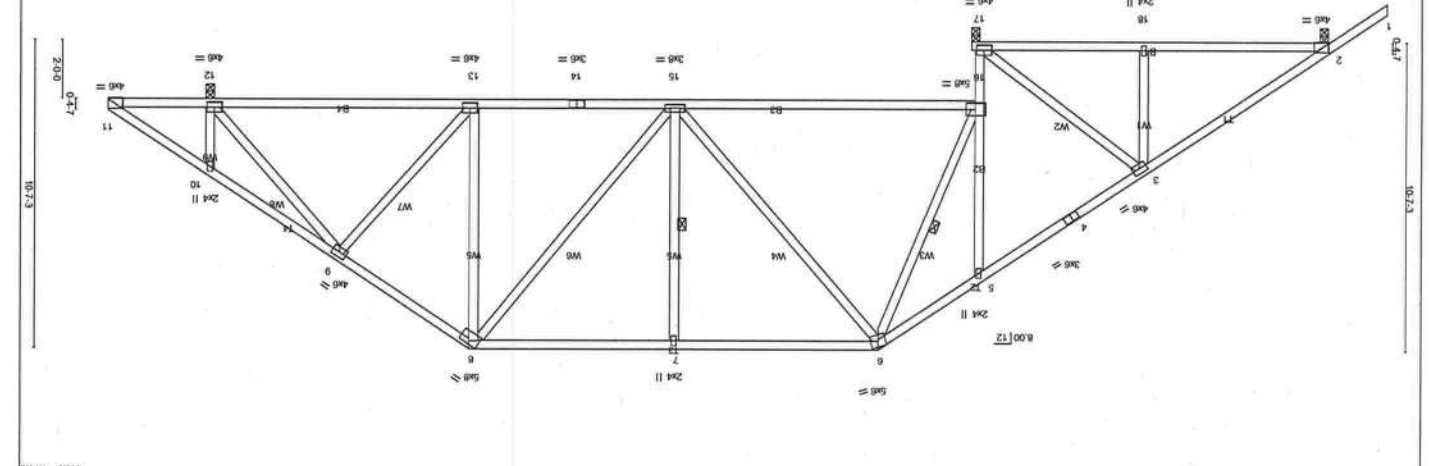
LOADING (psf)	SPACING	CSI	DEFL	VERT(L)	HORIZ(TL)	BRACING	WEBS	LUMBER	REACTIONS	FORCES	NOTES	
5.0	2.0-0	TC 0.35	in (loc)	-0.21	0.03	TOP CHORD	1 Row at midpt	TOP CHORD 2 X 4 SYP No.2D	(b/size) 2=493/0-3-8, 18=1763/0-3-8, 11=1337/0-3-8	Max Compression/Maximum Tension	Unbalanced roof live loads have been considered for this design.	
10.0	2.0-0	BC 0.76	in (loc)	12.14	11	BOT CHORD	Rigid ceiling directly applied or 4-6-3 oc bracing	BOT CHORD 2 X 4 SYP No.2D "except"	Max Grv 2=496(load case 7), 18=1763(load case 7), 11=1337(load case 1)		2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.	
20.0	2.0-0	WB 0.87	in (loc)	12.14	n/a	WEBS	Shathed or 4-4-5 oc purlins	WEBS 2 X 4 SYP No.3	Max Horiz 2=451(load case 4)		3) Provide adequate drainage to prevent water ponding.	
7.0	1.25								Max Uplift 2=-258(load case 5), 18=-767(load case 4), 11=-507(load case 5)		4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2, 787 lb uplift at joint 18 and 507 lb uplift at joint 11.	
5.0	5.0											

LOADING (psf)	SPACING	CSI	DEFL	VERT(L)	HORIZ(TL)	BRACING	WEBS	LUMBER	REACTIONS	FORCES	NOTES	
5.0	2.0-0	TC 0.35	in (loc)	-0.21	0.03	TOP CHORD	1 Row at midpt	TOP CHORD 2 X 4 SYP No.2D	(b/size) 2=493/0-3-8, 18=1763/0-3-8, 11=1337/0-3-8	Max Compression/Maximum Tension	Unbalanced roof live loads have been considered for this design.	
10.0	2.0-0	BC 0.76	in (loc)	12.14	11	BOT CHORD	Rigid ceiling directly applied or 4-6-3 oc bracing	BOT CHORD 2 X 4 SYP No.2D "except"	Max Grv 2=496(load case 7), 18=1763(load case 7), 11=1337(load case 1)		2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.	
20.0	2.0-0	WB 0.87	in (loc)	12.14	n/a	WEBS	Shathed or 4-4-5 oc purlins	WEBS 2 X 4 SYP No.3	Max Horiz 2=451(load case 4)		3) Provide adequate drainage to prevent water ponding.	
7.0	1.25								Max Uplift 2=-258(load case 5), 18=-767(load case 4), 11=-507(load case 5)		4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2, 787 lb uplift at joint 18 and 507 lb uplift at joint 11.	
5.0	5.0											

LOAD CASE(S) Standard

THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

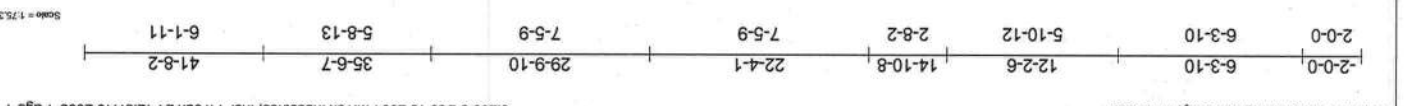
JUNE 27, 2005 TRUSS DESIGN ENGINEER:



2-0-0	6-3-10	12-2-6	15-4-1	22-4-1	29-4-1	33-9-4	38-2-6	41-8-2	45-2	49-7	53-2	56-7	60-2	63-7	67-2	70-7	74-2	77-7	81-2	84-7	88-2	91-7	95-2	98-7	102-2	105-7	109-2	112-7	116-2	119-7	123-2	126-7	130-2	133-7	137-2	140-7	144-2	147-7	151-2	154-7	158-2	161-7	165-2	168-7	172-2	175-7	179-2	182-7	186-2	189-7	193-2	196-7	200-2	203-7	207-2	210-7	214-2	217-7	221-2	224-7	228-2	231-7	235-2	238-7	242-2	245-7	249-2	252-7	256-2	259-7	263-2	266-7	270-2	273-7	277-2	280-7	284-2	287-7	291-2	294-7	298-2	301-7	305-2	308-7	312-2	315-7	319-2	322-7	326-2	329-7	333-2	336-7	340-2	343-7	347-2	350-7	354-2	357-7	361-2	364-7	368-2	371-7	375-2	378-7	382-2	385-7	389-2	392-7	396-2	399-7	403-2	406-7	410-2	413-7	417-2	420-7	424-2	427-7	431-2	434-7	438-2	441-7	445-2	448-7	452-2	455-7	459-2	462-7	466-2	469-7	473-2	476-7	480-2	483-7	487-2	490-7	494-2	497-7	501-2	504-7	508-2	511-7	515-2	518-7	522-2	525-7	529-2	532-7	536-2	539-7	543-2	546-7	550-2	553-7	557-2	560-7	564-2	567-7	571-2	574-7	578-2	581-7	585-2	588-7	592-2	595-7	599-2	602-7	606-2	609-7	613-2	616-7	620-2	623-7	627-2	630-7	634-2	637-7	641-2	644-7	648-2	651-7	655-2	658-7	662-2	665-7	669-2	672-7	676-2	679-7	683-2	686-7	690-2	693-7	697-2	700-7	704-2	707-7	711-2	714-7	718-2	721-7	725-2	728-7	732-2	735-7	739-2	742-7	746-2	749-7	753-2	756-7	760-2	763-7	767-2	770-7	774-2	777-7	781-2	784-7	788-2	791-7	795-2	798-7	802-2	805-7	809-2	812-7	816-2	819-7	823-2	826-7	830-2	833-7	837-2	840-7	844-2	847-7	851-2	854-7	858-2	861-7	865-2	868-7	872-2	875-7	879-2	882-7	886-2	889-7	893-2	896-7	900-2	903-7	907-2	910-7	914-2	917-7	921-2	924-7	928-2	931-7	935-2	938-7	942-2	945-7	949-2	952-7	956-2	959-7	963-2	966-7	970-2	973-7	977-2	980-7	984-2	987-7	991-2	994-7	998-2	1001-7	1005-2	1008-7	1012-2	1015-7	1019-2	1022-7	1026-2	1029-7	1033-2	1036-7	1040-2	1043-7	1047-2	1050-7	1054-2	1057-7	1061-2	1064-7	1068-2	1071-7	1075-2	1078-7	1082-2	1085-7	1089-2	1092-7	1096-2	1099-7	1103-2	1106-7	1110-2	1113-7	1117-2	1120-7	1124-2	1127-7	1131-2	1134-7	1138-2	1141-7	1145-2	1148-7	1152-2	1155-7	1159-2	1162-7	1166-2	1169-7	1173-2	1176-7	1180-2	1183-7	1187-2	1190-7	1194-2	1197-7	1201-2	1204-7	1208-2	1211-7	1215-2	1218-7	1222-2	1225-7	1229-2	1232-7	1236-2	1239-7	1243-2	1246-7	1250-2	1253-7	1257-2	1260-7	1264-2	1267-7	1271-2	1274-7	1278-2	1281-7	1285-2	1288-7	1292-2	1295-7	1299-2	1302-7	1306-2	1309-7	1313-2	1316-7	1320-2	1323-7	1327-2	1330-7	1334-2	1337-7	1341-2	1344-7	1348-2	1351-7	1355-2	1358-7	1362-2	1365-7	1369-2	1372-7	1376-2	1379-7	1383-2	1386-7	1390-2	1393-7	1397-2	1400-7	1404-2	1407-7	1411-2	1414-7	1418-2	1421-7	1425-2	1428-7	1432-2	1435-7	1439-2	1442-7	1446-2	1449-7	1453-2	1456-7	1460-2	1463-7	1467-2	1470-7	1474-2	1477-7	1481-2	1484-7	1488-2	1491-7	1495-2	1498-7	1502-2	1505-7	1509-2	1512-7	1516-2	1519-7	1523-2	1526-7	1530-2	1533-7	1537-2	1540-7	1544-2	1547-7	1551-2	1554-7	1558-2	1561-7	1565-2	1568-7	1572-2	1575-7	1579-2	1582-7	1586-2	1589-7	1593-2	1596-7	1600-2	1603-7	1607-2	1610-7	1614-2	1617-7	1621-2	1624-7	1628-2	1631-7	1635-2	1638-7	1642-2	1645-7	1649-2	1652-7	1656-2	1659-7	1663-2	1666-7	1670-2	1673-7	1677-2	1680-7	1684-2	1687-7	1691-2	1694-7	1698-2	1701-7	1705-2	1708-7	1712-2	1715-7	1719-2	1722-7	1726-2	1729-7	1733-2	1736-7	1740-2	1743-7	1747-2	1750-7	1754-2	1757-7	1761-2	1764-7	1768-2	1771-7	1775-2	1778-7	1782-2	1785-7	1789-2	1792-7	1796-2	1799-7	1803-2	1806-7	1810-2	1813-7	1817-2	1820-7	1824-2	1827-7	1831-2	1834-7	1838-2	1841-7	1845-2	1848-7	1852-2	1855-7	1859-2	1862-7	1866-2	1869-7	1873-2	1876-7	1880-2	1883-7	1887-2	1890-7	1894-2	1897-7	1901-2	1904-7	1908-2	1911-7	1915-2	1918-7	1922-2	1925-7	1929-2	1932-7	1936-2	1939-7	1943-2	1946-7	1950-2	1953-7	1957-2	1960-7	1964-2	1967-7	1971-2	1974-7	1978-2	1981-7	1985-2	1988-7	1992-2	1995-7	1999-2	2002-7	2006-2	2009-7	2013-2	2016-7	2020-2	2023-7	2027-2	2030-7	2034-2	2037-7	2041-2	2044-7	2048-2	2051-7	2055-2	2058-7	2062-2	2065-7	2069-2	2072-7	2076-2	2079-7	2083-2	2086-7	2090-2	2093-7	2097-2	2100-7	2104-2	2107-7	2111-2	2114-7	2118-2	2121-7	2125-2	2128-7	2132-2	2135-7	2139-2	2142-7	2146-2	2149-7	2153-2	2156-7	2160-2	2163-7	2167-2	2170-7	2174-2	2177-7	2181-2	2184-7	2188-2	2191-7	2195-2	2198-7	2202-2	2205-7	2209-2	2212-7	2216-2	2219-7	2223-2	2226-7	2230-2	2233-7	2237-2	2240-7	2244-2	2247-7	2251-2	2254-7	2258-2	2261-7	2265-2	2268-7	2272-2	2275-7	2279-2	2282-7	2286-2	2289-7	2293-2	2296-7	2300-2	2303-7	2307-2	2310-7	2314-2	2317-7	2321-2	2324-7	2328-2	2331-7	2335-2	2338-7	2342-2	2345-7	2349-2	2352-7	2356-2	2359-7	2363-2	2366-7	2370-2	2373-7	2377-2	2380-7	2384-2	2387-7	2391-2	2394-7	2398-2	2401-7	2405-2	2408-7	2412-2	2415-7	2419-2	2422-7	2426-2	2429-7	2433-2	2436-7	2440-2	2443-7	2447-2	2450-7	2454-2	2457-7	2461-2	2464-7	2468-2	2471-7	2475-2	2478-7	2482-2	2485-7	2489-2	2492-7	2496-2	2499-7	2503-2	2506-7	2510-2	2513-7	2517-2	2520-7	2524-2	2527-7	2531-2	2534-7	2538-2	2541-7	2545-2	2548-7	2552-2	2555-7	2559-2	2562-7	2566-2	2569-7	2573-2	2576-7	2580-2	2583-7	2587-2	2590-7	2594-2	2597-7	2601-2	2604-7	2608-2	2611-7	2615-2	2618-7	2622-2	2625-7	2629-2	2632-7	2636-2	2639-7	2643-2	2646-7	2650-2	2653-7	2657-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Dwg.#062705845

Job	L117440	Truss	T15	SPECIAL	2	City	1
Job Reference (optional)	6,200 s Dec 15 2004 Mitek Industries, Inc. Fri Jun 24 12:37:10 2005 Page 1						



LOADING (psf)	SPACING	PLATES INCREASE	PLATES INCREASE	REP STRESS INC	CODE FBC2001/ANSI95	CSI	TC	BC	WB	(Matrx)	DEFL	IN (loc)	W/DEFL	L/D	PLATES	GRP	WEIGHT: 247 lb
20.0	2.0-0	1.25	1.25	YES		0.36	0.70	0.81			0.02	11	n/a	n/a	MT20	244/190	

LUMBER	BRACING	REACTIONS	FORCES	NOTES
TOP CHORD 2 X 4 SYP No.2D	Sheathed or 6-0 oc purlins.	(b/size) 2=548/0-3-8, 16=1679/0-3-8, 11=1378/0-3-8	(b) - Maximum Compression/Maximum Tension	(1) Unbalanced roof live loads have been considered for this design.
BOT CHORD 2 X 4 SYP No.2D	Rigid ceiling directly applied or 6-0-0 oc bracing.	Max Horiz 2=509(load case 4)		(2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
WEBS 2 X 4 SYP No.3	1 Row at midpt	Max Uplift2=502(load case 5), 16=1000(load case 6)		(3) Provide adequate drainage to prevent water ponding.
	6-15	11=670(load case 6)		(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2, 1000 lb uplift at joint 16 and 670 lb uplift at joint 11.
		11=1378(load case 1)		

TOP CHORD	BOT CHORD	WEBS
1-2=0/60, 2-3=445/550, 3-4=214/347, 4-5=-199/371, 5-6=-128/436, 6-7=-848/665, 7-8=-848/666, 8-9=-1008/599, 9-10=-283/295	2-17=-580/294, 16-17=-580/294, 15-16=-1319/666, 5-15=-192/283, 14-15=-208/274, 13-14=-280/769, 12-13=-280/769, 11-12=-222/542	9-11=1305/895

JUNE 27, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L117440	Truss	T16	SPECIAL	1	Ply	1
Job Reference (optional)							

Builders Firstsource, Lake City, FL 32055
 6/20/05 Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:37:11 2005 Page 1

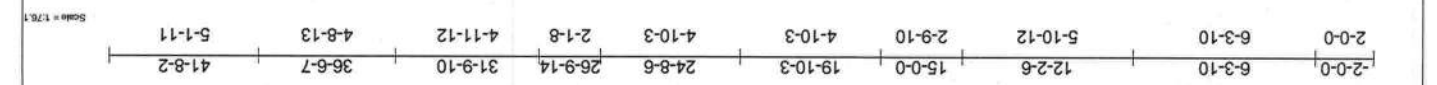


Plate Offsets (X,Y):	[2.0-0.0-0.0-0.4], [6.0-1.1-1.3-0.2-8], [12.0-0.3-Edge]											
LOADING (psf)	TOLL 20.0 TCDL 7.0 Lumber Increase 1.25 Plates Increase 1.25 CSI 2.0-0											
SPACING	2.0-0											
REPS	Code FBC2001/ANSI95 Rep Stress Incr YES Lumber Increase 1.25 Plates Increase 1.25											
CS	WB 0.76 BC 0.59 TC 0.32											
DEFL	in (loc) l/d Vert(L) -0.25 12-13 >999 240 Vert(T) -0.37 12-13 >956 180 Horz(TL) 0.02 12 n/a n/a											
BRACING	TOP CHORD Sheathed or 4-5-9 oc purlins. BOT CHORD Rigid ceiling directly applied or 7-8-13 oc bracing. WEBS 1 Row at midpt 6-17, 8-16, 5-18											
LUMBER	TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3											
REACTIONS	(lb/size) 12=1237/0-3-8, 2=601/0-3-8, 18=1755/0-3-8 Max Grav 12=1237(load case 1), 2=604(load case 7), 18=1755(load case 4) Max Uplift 12=559(load case 6), 2=332(load case 5), 18=784(load case 4) Max Horiz 2=442(load case 4)											
FORCES	(lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/59, 2-3=542/392, 3-4=225/361, 4-5=211/385, 5-6=96/484, 6-7=914/710, 7-8=914/710, 8-9=-1692/1117, 9-10=-1268/804, 10-11=-1587/872, 11-12=-1815/953 BOT CHORD 2-19=-265/374, 18-19=-265/374, 16-17=-190/392, 15-16=-389/1108, 14-15=-389/1108, 13-14=-543/1378, 12-13=-689/1466 2-19=0/215, 3-18=-483/341, 6-17=-149/531, 6-16=-402/1018, 7-16=-273/299, 8-16=-381/207, 8-14=-624/1070, 9-14=-916/631, 9-13=-212/216, 10-13=-233/560, 11-13=-254/308, 17-18=-1401/663, 5-17=-234/325											
NOTES	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate grip DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 12, 332 lb uplift at joint 2 and 784 lb uplift at joint 18. LOAD CASE(S) Standard											

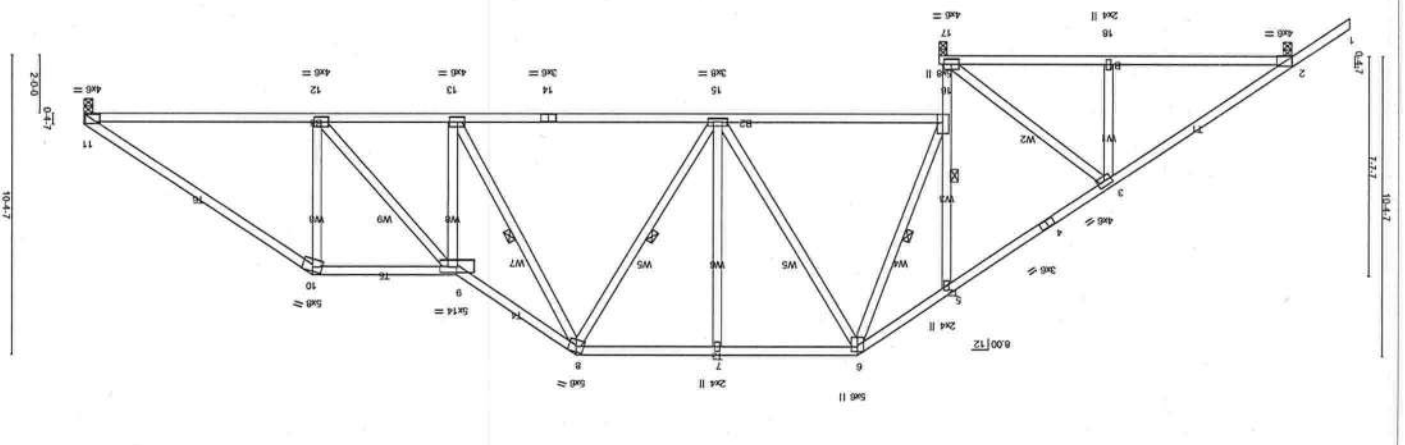
JUNE 27, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#062705846

Job	L171440	Truss	117	SPECIAL	1	Ply	1
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2-0-0	6-3-10	12-2-6	15-0-0	2-9-10	4-10-3	19-10-3	24-8-6	28-9-14	4-1-8	33-9-10	4-1-1-12	7-10-8
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6-3-10	12-2-6	19-10-3	28-9-14	33-9-10	4-1-1-12	7-10-8
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LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2001/ANSI95	CSI	DEFL	Vert(UL)	Vert(TL)	Horz(TL)	WEBS	TOP CHORD	BOT CHORD	BRACING
20.0	2-0-0	1.25	1.25	YES	2-0-0	0.51	0.20	-0.29	0.02	0.02	11	11	11	11
TCLL	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2001/ANSI95	CSI	DEFL	Vert(UL)	Vert(TL)	Horz(TL)	WEBS	TOP CHORD	BOT CHORD	BRACING	
7.0	1.25	1.25	YES	2-0-0	0.51	0.20	-0.29	0.02	0.02	11	11	11	11	
BCLL	1.25	1.25	YES	2-0-0	0.51	0.20	-0.29	0.02	0.02	11	11	11	11	
5.0	2-0-0	1.25	1.25	YES	2-0-0	0.51	-0.20	-0.29	0.02	0.02	11	11	11	
BCLL	2-0-0	1.25	1.25	YES	2-0-0	0.51	-0.20	-0.29	0.02	0.02	11	11	11	
5.0	2-0-0	1.25	1.25	YES	2-0-0	0.51	-0.20	-0.29	0.02	0.02	11	11	11	

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3

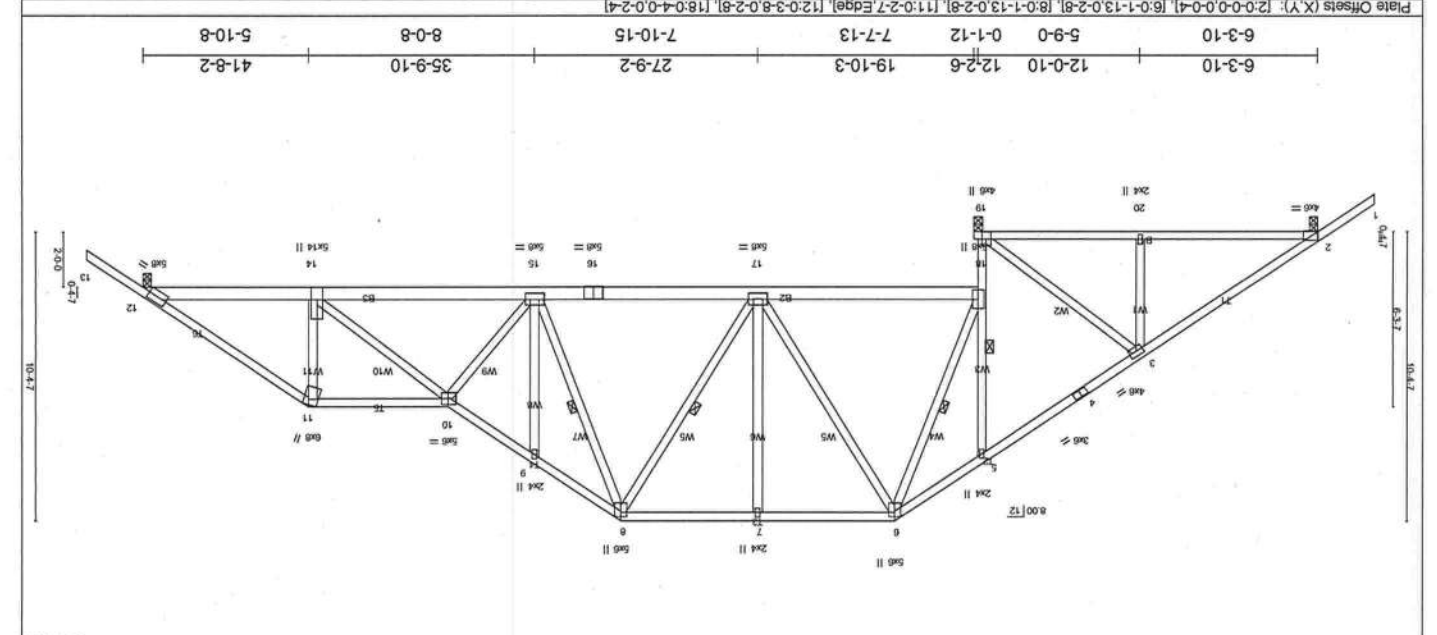
REACTIONS (lb/size) 11=1236/0-3-8, 2=599/0-3-8, 17=1758/0-3-8
 Max Uplift 1=559(load case 6), 2=331(load case 5), 17=762(load case 4)
 Max Horiz 2=442(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=539/391, 3-4=227/360, 4-5=212/383, 5-6=97/482, 6-7=915/706, 7-8=915/706, 8-9=2111/1300, 9-10=-1421/872,
 10-11=-1809/898
 BOT CHORD 2-18=264/371, 17-18=-264/371, 15-16=-179/391, 14-15=-391/1101, 13-14=-391/1101, 12-13=-719/1701, 11-12=-592/1407
 WEBS 3-18=0/215, 3-17=-482/341, 6-16=-1159/505, 6-15=-398/1023, 7-15=-276/312, 8-15=-365/225, 8-13=-742/1341, 9-13=-1004/715,
 9-12=416/228, 10-12=-147/538, 16-17=-1405/641, 5-16=-234/325

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 11, 331 lb uplift at joint 2 and 762 lb uplift at joint 17.

LOAD CASE(S) Standard

2-0-0	6-3-10	12-2-6	5-10-12	2-9-10	4-10-3	4-10-3	27-9-2	3-0-12	3-0-12	30-9-14	35-9-10	41-8-2	5-10-8	2-0-0
2-0-0	6-3-10	12-2-6	5-10-12	2-9-10	4-10-3	4-10-3	27-9-2	3-0-12	3-0-12	30-9-14	35-9-10	41-8-2	5-10-8	2-0-0



LOADING (psf)	SPACING	PLATES INCREASE	TCI	CS	WB	BC	Rep Stress Incr	Code FBC2001/ANSI95	NO	Rep Stress Incr	Code FBC2001/ANSI95	NO	Rep Stress Incr	Code FBC2001/ANSI95
20.0	2.0-0	1.25	0.47	0.94	0.94	0.48	1.25	1.25	1.25	1.25	1.25	1.25	1.25	1.25
TOLL	2.00	1.25	0.47	0.94	0.94	0.48	1.25	1.25	1.25	1.25	1.25	1.25	1.25	1.25
TCDL	7.0	1.25	0.47	0.94	0.94	0.48	1.25	1.25	1.25	1.25	1.25	1.25	1.25	1.25
BCLL	10.0	1.25	0.47	0.94	0.94	0.48	1.25	1.25	1.25	1.25	1.25	1.25	1.25	1.25
BCLD	5.0	1.25	0.47	0.94	0.94	0.48	1.25	1.25	1.25	1.25	1.25	1.25	1.25	1.25

LUMBER	BRACING	WEBS
TOP CHORD 2 X 4 SYP No. 2D	TOP CHORD	2 X 4 SYP No. 3
BOT CHORD 2 X 6 SYP No. 1D "Except"	BOT CHORD	2 X 4 SYP No. 2D
WEBS	WEBS	WEBS
1 Row at midpt.	1 Row at midpt.	1 Row at midpt.
Sheathed or 3-1-10 oc purlins.	Rigid ceiling directly applied or 6-8-14 oc bracing.	6-16, 8-17, 8-15, 5-19

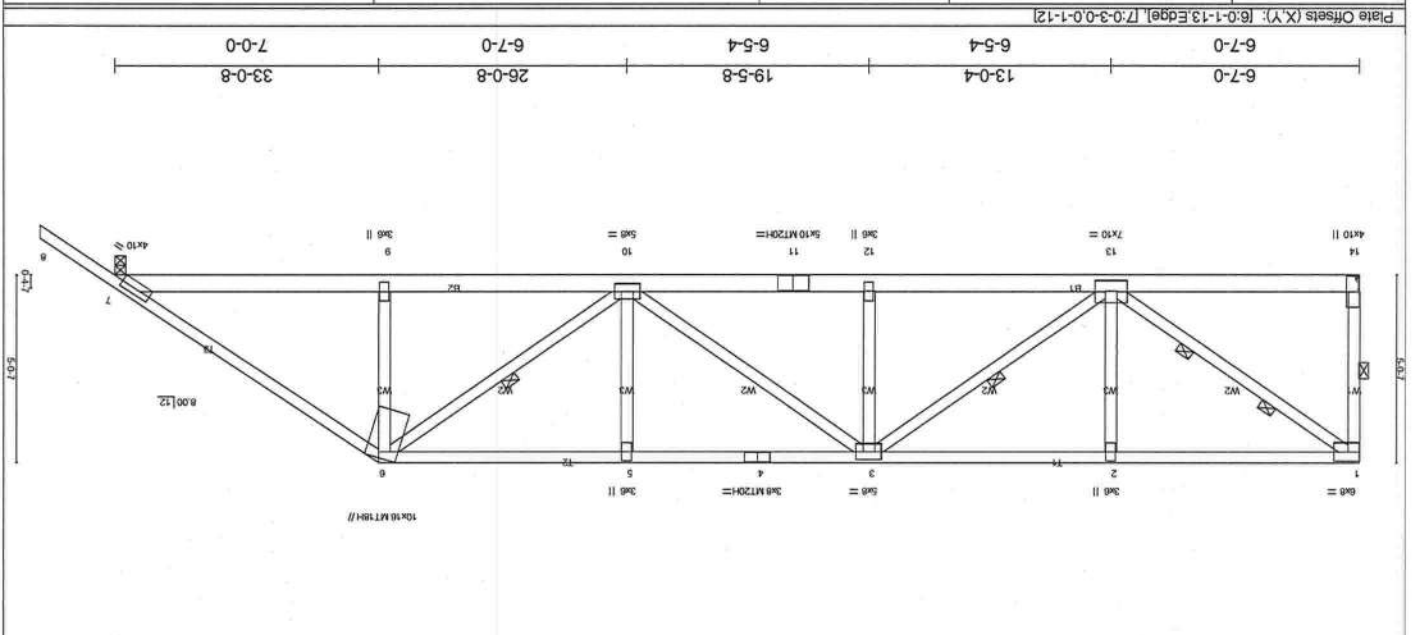
REACTIONS (lb/size) 2=592/0-3-8, 19=2057/0-3-8, 12=2542/0-3-8
 Max Horiz 2=412(load case 3)
 Max Uplift 2=354(load case 5), 19=893(load case 3), 12=1305(load case 5)
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=527/502, 3-4=199/472, 4-5=185/495, 5-6=71/593, 6-7=1201/767, 7-8=1201/767, 8-9=2800/1528, 9-10=2829/1396,
 10-11=3291/1661, 11-12=3980/1898, 12-13=0/62
 BOT CHORD 3-20=257/361, 19-20=224/496, 16-17=591/1577, 15-16=591/1577, 14-15=1556/3534, 12-14=1447/3220
 WEBS 2-20=0/215, 3-19=481/340, 6-18=1472/648, 6-17=576/1363, 7-17=273/295, 8-17=728/343, 8-15=1070/2052, 9-15=129/211,
 10-15=1978/1061, 10-14=312/165, 11-14=899/1884, 18-19=1705/773, 5-18=238/327

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60
 (3) Provide adequate drainage to prevent water ponding.
 (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 893 lb uplift at joint 19 and 1305 lb uplift at joint 12.
 (5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1483 lb down and 866 lb up at 35-8-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 (6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASES Standard
 (1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert 1-6=-54, 6-8=-54, 8-10=-54, 10-11=-54, 11-13=-54, 2-19=-30, 12-18=-30
 Concentrated Loads (lb)
 Vert 14=-1483(F)

Job	L171440	Truss	T19	SPECIAL	1	PLY	1
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Dwg. #062705849

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LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	NO	Code FBC2001/ANSI95	CS	TC	BC	WB	(Matrix)	DEFL	Vert(L)	Horz(TL)	PLATES	GRP	MT20	MT20H	MT18H	Weight: 209 lb
2.00	2.0-0	1.25	1.25			0.98	0.63	0.77	0.10		0.10	0.10	0.10	244/190	244/190	187/143	187/143	244/190	

LUMBER
 TOP CHORD 2 X 4 SYP No. 1D "Except"
 T3 X 4 SYP No. 2D
 BOT CHORD 2 X 6 SYP No. 1D
 WEBS 2 X 4 SYP No. 2D "Except"
 W3 2 X 4 SYP No. 3, W3 2 X 4 SYP No. 3, W3 2 X 4 SYP No. 3, W3 2 X 4 SYP No. 3

BRACING
 TOP CHORD Sheathed or 2-3-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-2-1 oc bracing.
 WEBS 1-14, 3-13, 6-10
 2 Rows at 1/3 pts 1-13

REACTIONS (lb/size) 14=3034/Mechanical, 7=2927/0-3-8
 Max Uplift 14=-362(load case 5)
 Max Horiz 14=-2331(load case 3), 7=-1905(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-14=-2812/2321, 1-2=-3495/2669, 2-3=-3495/2669, 3-4=-5371/4044, 4-5=-5371/4044, 5-6=-5372/4043, 6-7=-4665/3251, 7-8=0/62
 BOT CHORD 1-3=-3162/4181, 2-13=-781/1054, 3-13=-2162/1627, 3-12=0/431, 3-10=-165/155, 5-10=-773/1088, 6-10=-1661/1905, 6-9=-440/873
 WEBS 1-13=-3162/4181, 2-13=-781/1054, 3-13=-2162/1627, 3-12=0/431, 3-10=-165/155, 5-10=-773/1088, 6-10=-1661/1905, 6-9=-440/873

NOTES
 1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC/DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
 2) Provide adequate drainage to prevent water ponding.
 3) All plates are MT20 plates unless otherwise indicated.
 4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2331 lb uplift at joint 14 and 1905 lb uplift at joint 7.
 6) Girder carries hip end with 7'-0" right side setback, and 7'-0" end setback.
 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 506 lb up at 26'-0" on bottom chord. The design/section of such connection device(s) is the responsibility of others.
 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 9) Regular: Lumber Increase=1.25, Plate Increase=1.25
 1) Uniform Loads (psf)
 Vert: 1-6=-118(F=-64), 6-8=-54, 9-14=-65(F=-35), 7-9=-30
 Concentrated Loads (lb)
 Vert 9=-539(F)

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 1) Uniform Loads (psf)
 Vert: 1-6=-118(F=-64), 6-8=-54, 9-14=-65(F=-35), 7-9=-30
 Concentrated Loads (lb)
 Vert 9=-539(F)

JUNE 27, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg. #062705830

Job	L117440
Truss	T20
Truss Type	HIP
Qty	1
Plty	1

Builders FirstSource, Lake City, FL 32055

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Job Reference (optional)



LOADING (lb/ft)	SPACING	CSI	DEFL	VERT(L)	VERT(R)	Horz(TL)	Weight: 192 lb
20.0	2-0-0	0.59	0.25	10-12	>999	240	PLATES MT20 244/190 GRP
12.0	1-25	0.67	-0.36	10-12	>999	180	
10.0	YES	0.82	0.07	8	n/a	n/a	
5.0	Code FBC2001/ANSI95	(Matrix)					

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No. 2D	Shathed or 4-5-3 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No. 2D	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No. 3 "Except"	1 Row at midpt
WEBS W1 2 X 4 SYP No. 2D	3-14, 5-10

REACTIONS (lb/size) 15=1372/Mechanical, 8=1495/0-3-8
 Max Horiz 15=385/load case 6)
 Max Uplift 15=667/load case 4), 8=624/load case 3)

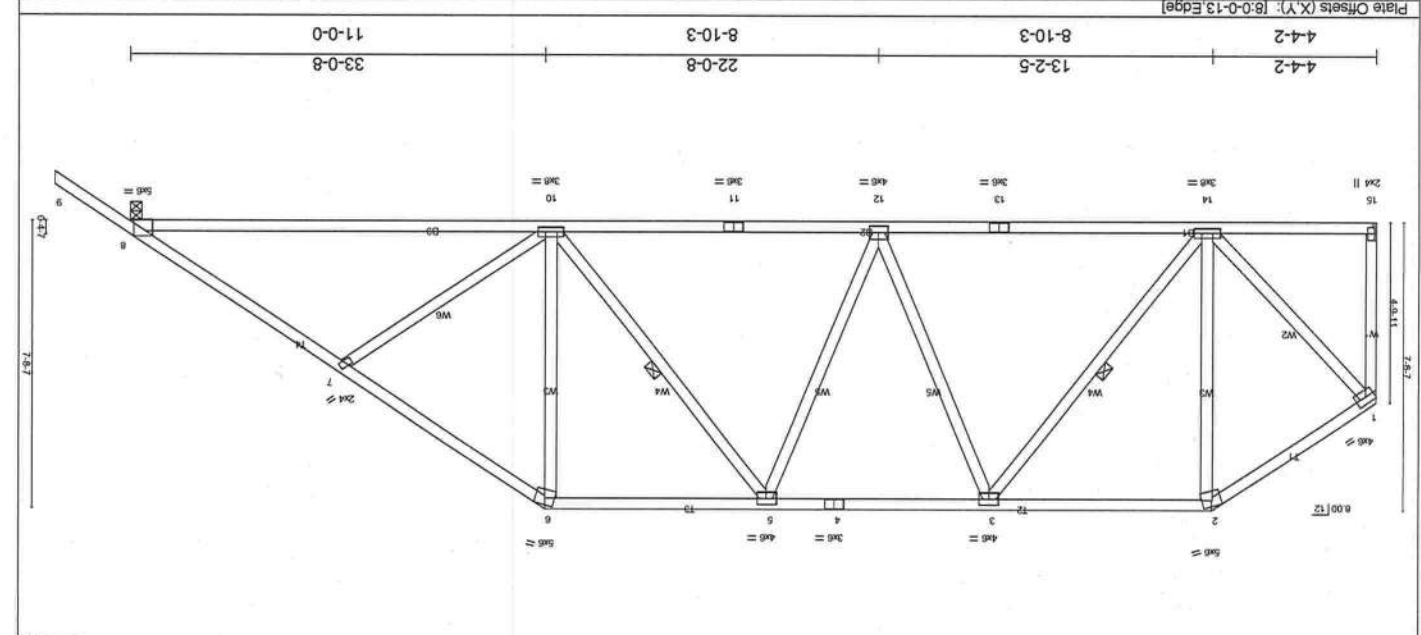
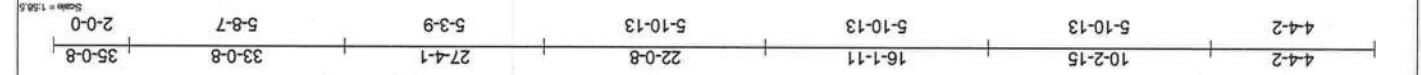
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=651/314, 2-3=555/328, 3-4=1798/980, 4-5=1798/980, 5-6=1532/835, 6-7=1877/922, 7-8=2058/952, 8-9=0/59, 1-15=-1440/658
 BOT CHORD 14-15=55/382, 13-14=844/1535, 12-13=-844/1535, 11-12=-933/1864, 10-11=-933/1864, 8-10=-665/1647
 WEBS 2-14=0/123, 3-14=-1297/831, 3-12=-145/519, 5-12=-132/223, 5-10=515/466, 6-10=-228/681, 7-10=-159/244, 1-14=-631/1233

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 667 lb uplift at joint 15 and 624 lb uplift at joint 8.
 LOAD CASE(S) Standard

Dwg.#062705851

Job	L117440	Truss	T21	HIP	1	Qty	1	Ply	1	Job Reference (optional)
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LOADING (psf)	TCLL 20.0	SPACING 2-0-0	CSI TC 0.54	DEFL in (loc) l/d	VERT(L) -0.31	VERT(TL) -0.47	HORIZ(TL) 0.06	WEBS 8 n/a	PLATES MT20 244/190	Weight: 203 lb
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LUMBER	TOP CHORD 2 X 4 SYP No.2D	BRACING	TOP CHORD Sheathed or 4-2-6 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2D	WEBS 2 X 4 SYP No.3 "Except"	BOT CHORD WEBS 1 Row at midpt.	Rigid ceiling directly applied or 6-0-0 oc bracing.
TOP CHORD 2 X 4 SYP No.2D	WEBS 1 Row at midpt.	WEBS 3-14, 5-10	

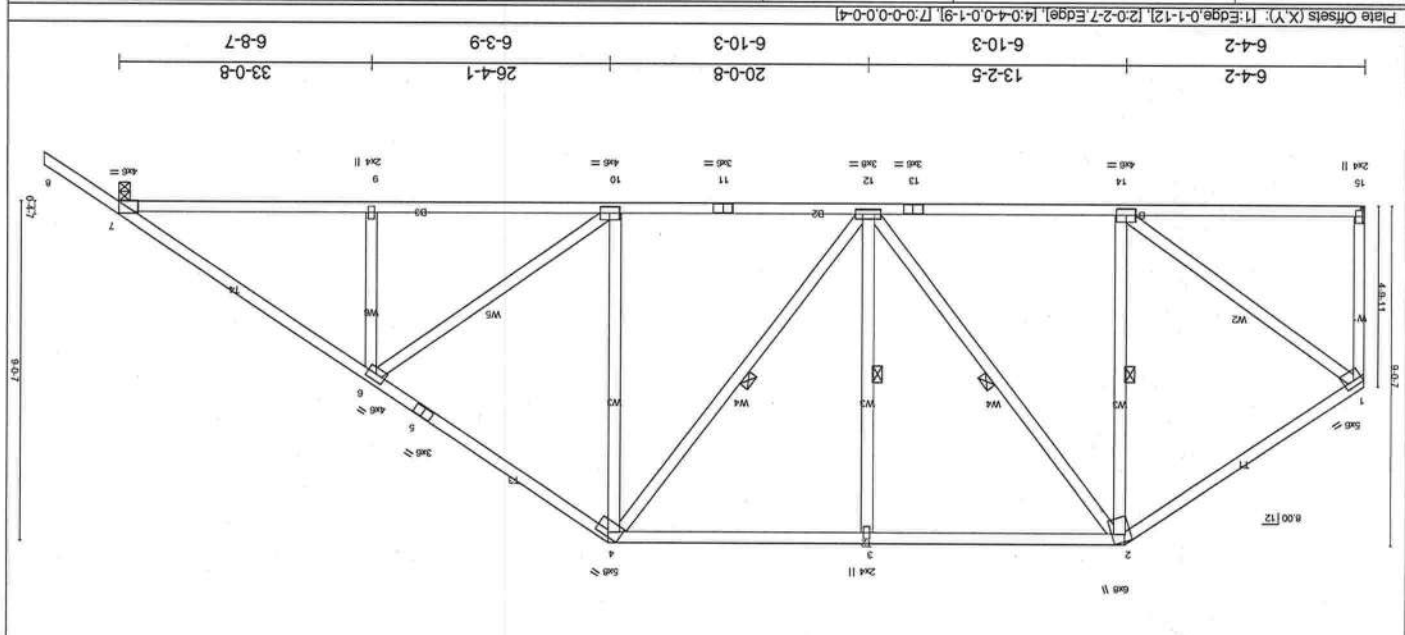
REACTIONS (lb/size) 15=1372/Mechanical, 8=1495/0-3-8
 Max Horiz 15=417(load case 6)
 Max Uplift 15=563(load case 4), 8=613(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=928/480, 2-3=734/474, 3-4=-1462/829, 4-5=-1462/829, 5-6=-1400/808, 6-7=-1754/864, 7-8=-2005/945, 8-9=0/0/59, 1-15=-1337/669
 BOT CHORD 14-15=-118/408, 13-14=-663/1300, 12-13=-663/1300, 11-12=-685/1524, 10-11=-685/1524, 8-10=-573/1613
 WEBS 2-14=75/254, 3-14=-946/617, 3-12=-146/448, 5-12=-178/201, 5-10=-305/381, 6-10=-204/628, 7-10=-272/330, 1-14=-507/1044

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC DL=4.2psf; BCDL=1.60; This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 15 and 613 lb uplift at joint 8.
LOAD CASE(S) Standard

Job	L117440	Truss	T22	HIP	1	Qty	1	Job Reference (optional)	1
Dwg #	062705852								

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 Scale = 1/8" = 1'-0"



LOADING (psf)	20.0	TC	0.62	DEFL	In (in)	Weight: 213 lb
TOP CHORD	2.0	CS	0.82	Vert(LL)	10-12	PLATES
BOT CHORD	2.0	TC	0.62	Vert(UL)	10-12	MT20
SPACING	2.0-0	WB	0.52	Horz(TL)	0.05	GRP
Code FBC2001/ANSI95	2.0-0	Matrix)				
Rep Stress Incr	YES					
Lumber Increase	1.25					
Plates Increase	1.25					
TOP CHORD 2 X 4 SYP No.2D						
BOT CHORD 2 X 4 SYP No.2D						
WEBS 2 X 4 SYP No.3 Except						
WEBS W1 2 X 4 SYP No.2D						

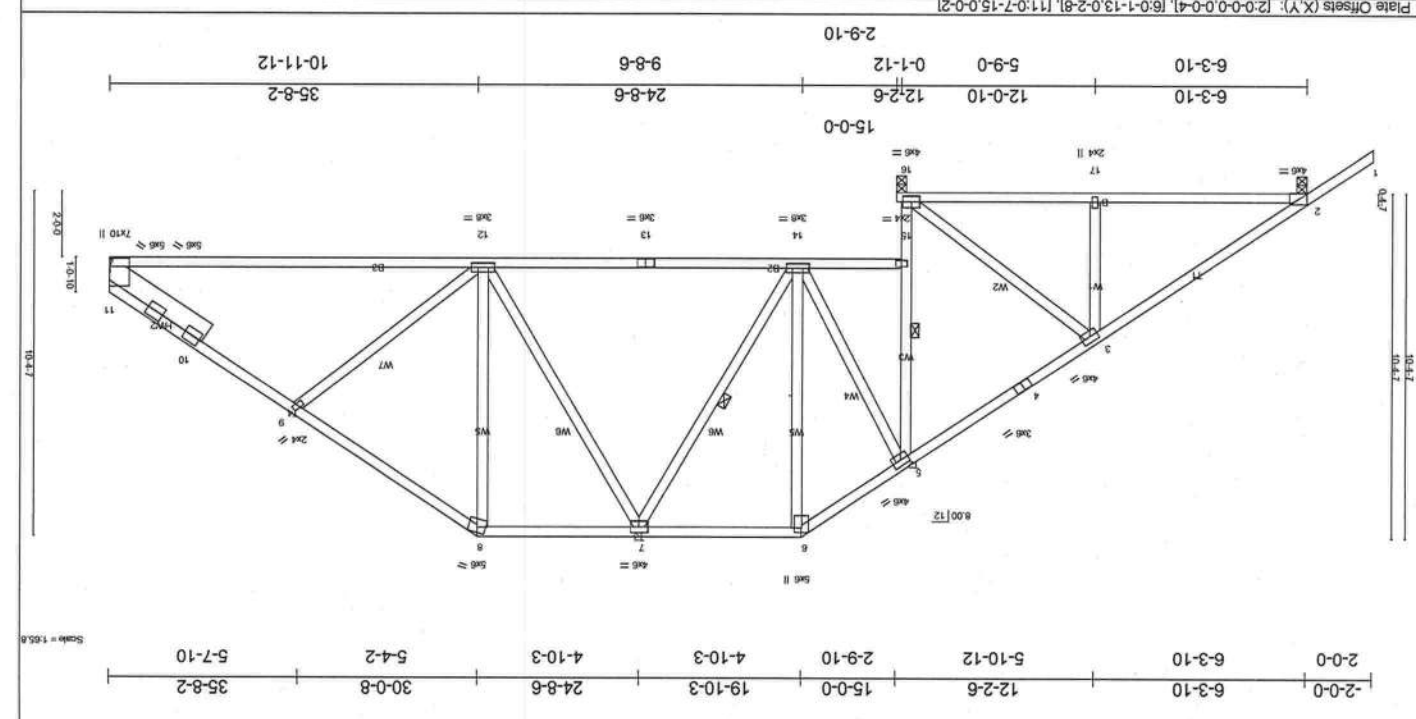
REACTIONS (lb/size) 15=1372/Mechanical, 7=1495/0-3-8
 Max Horiz 15=449(load case 6)
 Max Uplift 15=453(load case 4), 7=636(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-1087/570, 2-3=-1266/787, 3-4=-1266/787, 4-5=-1427/833, 5-6=-1597/806, 6-7=-2087/899, 7-8=0/0, 8-9=-1280/669
 BOT CHORD 1-4=184/430, 13-14=420/827, 11-12=-364/1257, 10-11=-364/1257, 9-10=-507/1641, 7-9=-507/1641
 WEBS 2-14=-371/302, 2-12=-481/758, 3-12=-386/422, 4-12=-284/154, 4-10=-180/471, 6-10=-481/381, 6-9=0/212, 1-14=-453/972

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); n=20ft; TCDL=4; 2psf; BCDL=3; 0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 15 and 636 lb uplift at joint 7.
 LOAD CASE(S) Standard

Job	L117440	Truss Type	T23	Qty	3	Job Reference (optional)	1
Dwg.#	062705853						

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LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2001/ANSI95	CSI	WB	BC	TC	DEFL	Vert(L)	Horz(TL)	BRACING
20.0	2-0-0	1.25	1.25	YES	2-0-0	0.29	0.42	0.53	0.29	in (loc)	0.02	0.02	TOP CHORD
TCLL										L/d	11	11	BOT CHORD
TCDL										>993	>993	>993	WEBS
BCLL										n/a	n/a	n/a	TOP CHORD
BCDL										240	240	240	BOT CHORD
										180	180	180	WEBS
										244/190	244/190	244/190	TOP CHORD
										MT20	MT20	MT20	BOT CHORD
										GRIP	GRIP	GRIP	WEBS
										Weight: 226 lb	Weight: 226 lb	Weight: 226 lb	SLIDER

LUMBER TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 4 SYP No.2D
 WEBS 2 X 4 SYP No.3
 SLIDER Right 2 X 8 SYP No.1D 3-6-7

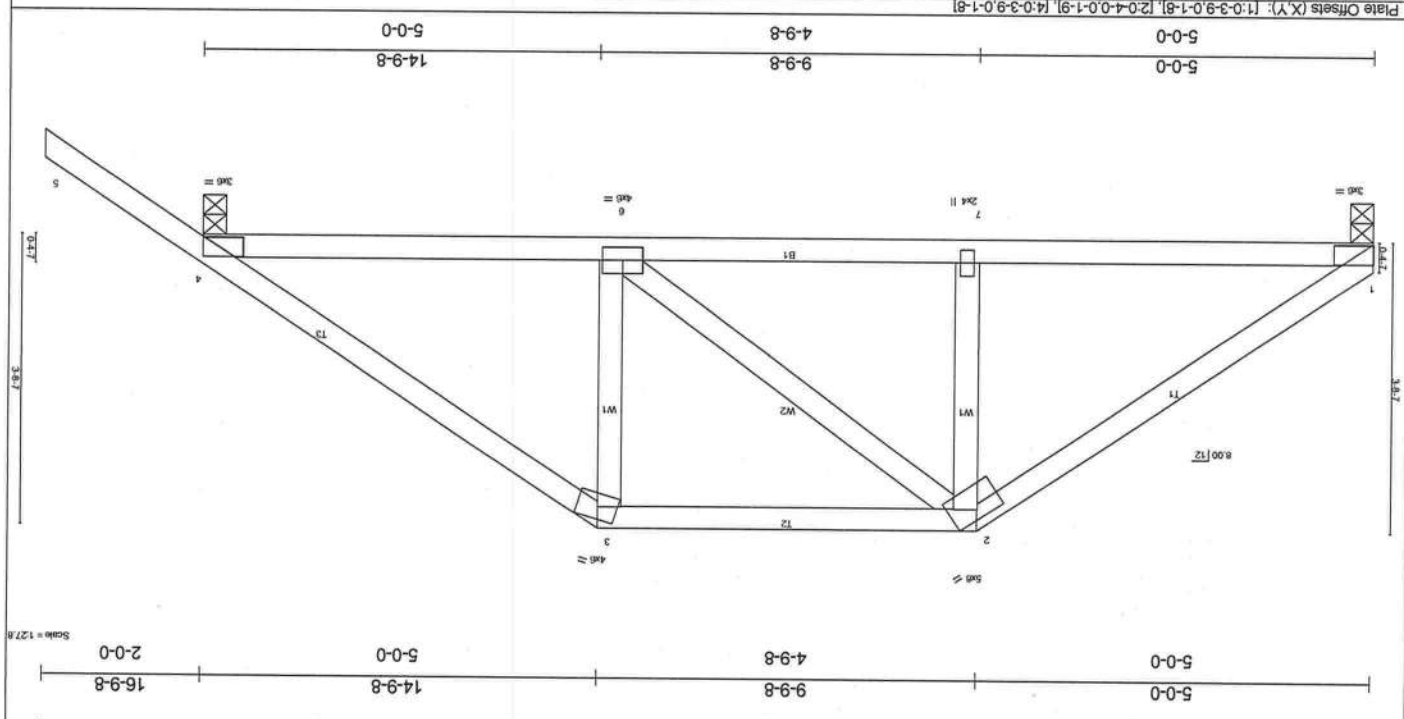
BRACING TOP CHORD Sheathed or 6-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing. Except:
 WEBS 6-0 oc bracing: 14-15,
 1 Row at midpt 7-14, 5-16

REACTIONS (lb/size) 11=993/Mechanical, 2=610/0-3-8, 16=1498/0-3-8
 Max Horiz 2=442(load case 4)
 Max Uplift 11=433(load case 6), 2=324(load case 5), 16=649(load case 4)
 Max Grav 11=993(load case 1), 2=610(load case 7), 16=1498(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/60, 2-3=553/399, 3-4=222/368, 4-5=207/391, 5-6=481/500, 6-7=358/457, 7-8=775/607, 8-9=1004/625, 9-10=1087/686,
 10-11=1245/665
 BOT CHORD 2-17=268/363, 16-17=268/363, 14-15=6/12, 13-14=242/641, 12-13=242/641, 11-12=407/936
 WEBS 5-15=-138/528
 3-16=485/341, 5-14=-285/761, 6-14=-158/116, 7-14=-571/302, 7-12=-68/269, 8-12=-89/232, 9-12=-211/321, 15-16=-142/528

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior (2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide adequate drainage to prevent water ponding.
 (4) Refer to girders(s) for truss to truss connections.
 (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 11, 334 lb uplift at joint 2 and 649 lb uplift at joint 16.
LOAD CASES Standard

Job	L117440	Truss	T126	HIP	1	City	1	Ply	1
Builders FirstSource, Lake City, FL 32055									
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LOADING (psf)	TCLL 20.0	SPACING 2-0-0	Code FBC2001/ANSI95	CSI (Matrix)	DEFL in (loc)	VERT (LL) 0.04	VERT (TL) -0.06	HORZ (TL) 0.02	Weight: 68 lb
REACTIONS (lb/size)	Max Horz 1=-179/0-3-8, 4=112/0-3-8	Max Uplift 1=-563/load case 5)	Rep Stress Incr NO	WB 0.16	in (loc) 4	VERT (LL) 6-7 > 999	VERT (TL) 6-7 > 999	Horz (TL) n/a	PLATES MT20 244/190
FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD 1-2=-1476/892, 2-3=-1170/790, 3-4=-1473/880, 4-5=0/59	BOT CHORD 1-7=-787/1173, 6-7=-796/1193, 4-6=673/1151	Plates Increase 1.25	BC 0.36	L/D n/a	VERT (LL) 6-7 > 999	VERT (TL) 6-7 > 999	L/D n/a	BRACING
	WEBS 2 X 4 SYP No.2D	WEBS 2 X 4 SYP No.3	Plates Increase 1.25	TC 0.39	Sheathed or 5-2-13 oc purlins	TOP CHORD	TOP CHORD	Rigid ceiling directly applied or 7-3-4 oc bracing.	

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 1 and 729 lb uplift at joint 4.
- 5) Girders carry hip end with 5-0-0 end setback.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 230 lb up at 9-9-8, and 245 lb down and 230 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (psf)

Vert: 1-2=-54, 2-3=-91(F=-37), 3-5=-54, 1-7=-30, 6-7=-50(F=-20), 4-6=-30

Concentrated Loads (lb)

Vert: 7=-245(F) 6=-245(F)

Job	L117440	Truss	127	HIP	1	Qty	1	Ply	1	Job Reference (optional)
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Builders FirstSource, Lake City, Fl 32055 6,200 S Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:37:17 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER	REACTIONS	FORCES	NOTES
20.0	2-0-0	TC 0.39	in (loc)	TOP CHORD Sheathed or 6-0-0 oc purlins.	TOP CHORD 2 X 4 SYP No.2D	Max Horiz 1=-231(load case 3) Max Uplift1=-224(load case 5), 4=-389(load case 6)	(lb) - Maximum Compression/Maximum Tension	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 389 lb uplift at joint 4.
TCLL 7.0	Lumber Increase 1.25	BC 0.42	Vert(TL) -0.12	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.	BOT CHORD 2 X 4 SYP No.3			
BCLL 10.0	Rep Stress Incr YES	WB 0.11	Horz(TL) 0.01					
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)						
Weight: 66 lb								

JUNE 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33649

DWG.#062705855

Job	L17440	Truss	T28	COMMON	1	Job Reference (optional)
Truss Type					2	
Qty					1	
Ply						

Builders FirstSource, Lake City, FL 32055
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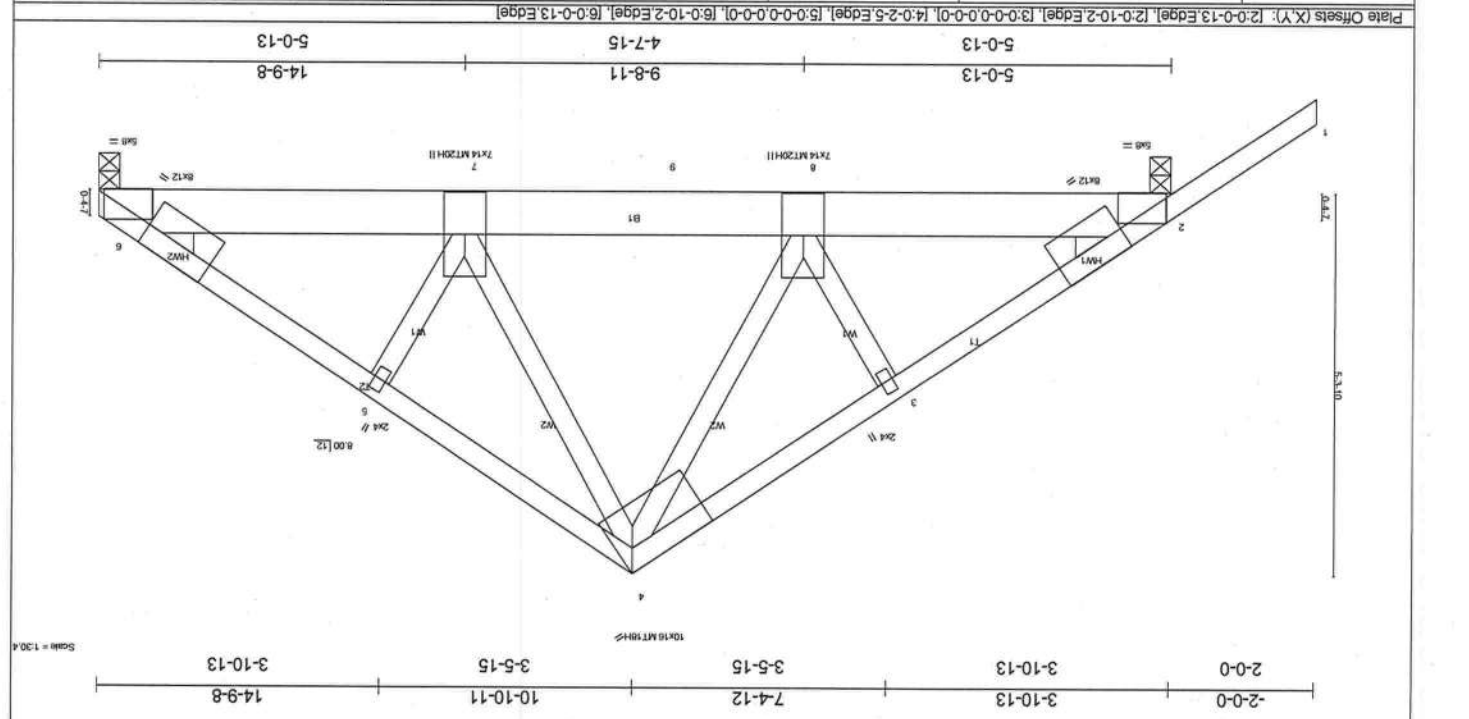
LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP STRESS INC	Code FBC2001/ANSI95	CSI	DEFL	VERT(L)	HORIZ(TL)	BRACING
20.0	2-0-0	1.25	1.25	1.25	TC 0.39	0.11	1-5	>999	240	TOP CHORD
7.0					BC 0.43	-0.15	1-5	>999	180	BOT CHORD
10.0					WB 0.09	0.01	3	n/a	n/a	Sheathed or 6-0-0 oc purlins
5.0										Rigid ceiling directly applied or 10-0-0 oc bracing.
BCLL										
BCLL										
TOLL										
PLATES										
GRIP										
MT20										
Weight: 60 lb										

REACTIONS (lb/size) 1=600/0-3-8, 3=733/0-3-8
 Max Horiz 1=241(load case 3)
 Max Uplift 1=227(load case 5), 3=-392(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-739/311, 2-3=-744/319, 3-4=0/59
 BOT CHORD 1-5=-129/533, 3-5=-129/533
 WEBS 2-5=0/284

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60 plate gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 1 and 392 lb uplift at joint 3.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP STRESS INC	NO	Code FBC2001/ANSI95	CSI	DEFL	in (loc)	Wdth	L/D	PLATES	GRIP
20.0	2-0-0	1.25	1.25	0.32	TC	0.32	0.11	7-8	>999	240	240	MT20	244/190
TCLL		7.0			BC	0.50	0.15	7-8	>999	160	160	MT20H	187/143
TCDL		10.0			WB	0.47	0.02	2	n/a	n/a	n/a	MT18H	244/190
BCDL	5.0												Weight: 198 lb

LUMBER
 TOP CHORD 2 X 4 SYP No. 2D
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No. 2D - Except
 W1 2 X 4 SYP No. 3, W1 2 X 4 SYP No. 3
WEDGE
 Left: 2 X 4 SYP No. 3, Right: 2 X 4 SYP No. 3

BRACING
 TOP CHORD Sheathed or 4-10-6 cc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 cc bracing.

REACTIONS (lb/size) 6=5670/0-3-8, 2=3635/0-3-8
 Max Horiz 6=239(load case 2)
 Max Uplift 6=2731(load case 4), 2=-1815(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 4-5=-7631/3735, 5-6=-7726/3694, 1-2=0/65, 2-3=-6380/2990, 3-4=-6269/3015
 BOT CHORD 2-8=-2399/5242, 8-9=-1817/3987, 7-9=-1817/3987, 6-7=-3083/6395
 WEBS 5-7=-134/203, 4-7=-2698/5407, 4-8=-1332/2810, 3-8=-88/174

NOTES
 (1) 2-ply truss to be connected together with 0.131"X3" Nails as follows:
 Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 cc.
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 cc.
 (2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 (3) Unbalanced roof live loads have been considered for this design.
 (4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gnp DOL=1.60.
 (5) All plates are MT20 plates unless otherwise indicated.
 (6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2731 lb uplift at joint 6 and 1815 lb uplift at joint 2.
 (7) Girder carries tie-in span(s): 33-0-8 from 7-0-0 to 14-9-8
 (8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3034 lb down and 1462 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
 (1) Regular; Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 4-6=-54, 1-4=-54, 2-9=-30, 6-9=-676(F)=646
 Concentrated Loads (lb)
 Vert: 9=-3034(F)

Job	L117440	Truss Type	HIP	Qty	1	Job Reference (optional)	1
Builders FirstSource, Lake City, FL 32055						6.200 s Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:37:32 2005 Page 1	



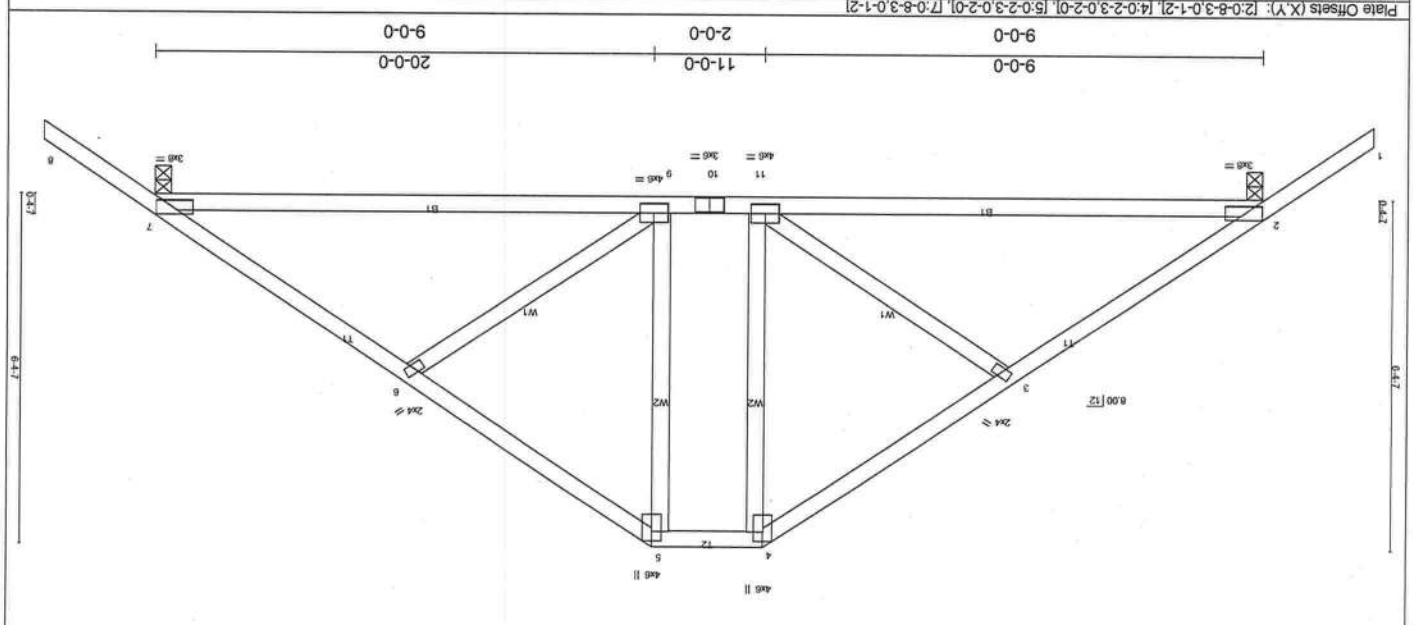
LOADING (psf)	SPACING	CS	TC	BC	WB	DEFL	VERT(L)	VERT(R)	HORIZ(TL)	TOP CHORD	BOT CHORD	BRACING	WEBS	REACTIONS	FORCES	TOP CHORD	WEBS	NOTES
2.00	2.0-0	2.00	1.25	1.25	NO	CS	0.55	0.58	0.25	2 X 4 SYP No. 2D	2 X 4 SYP No. 2D	2 X 4 SYP No. 3	2 X 4 SYP No. 3	2=1774/0-3-8, 6=1774/0-3-8	(b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/59, 2-3=2558/1691, 3-4=2147/1565, 4-5=2147/1564, 5-6=2558/1692, 6-7=0/59 WEBS 3-10=4677/94, 3-9=-421/298, 4-9=-261/490, 5-9=422/298, 5-8=-4677/94	2-10=-1462/2026, 9-10=-1476/2050, 8-9=-1332/2050, 6-8=-1319/2026	1) Unbraced roof live loads have been considered for this design. 2) Wind: ASCE 7-98, 120mph (3-second gust); h=20ft; T.CDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grp DOL=1.60. 3) Provide adequate drainage to prevent water ponding. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1227 lb uplift at joint 2 and 1227 lb uplift at joint 6. 5) Girders carries hip end with 7'-0" end setback. 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) is the responsibility of others. 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).	

LOAD CASE(S) Standard
 (1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-3=-54, 3-5=-118(F=64), 5-7=-54, 2-10=30, 8-10=-65(F=-35), 6-8=-30
 Concentrated Loads (lb)
 Vert: 10=-539(F) 8=-539(F)

Dwg.#062705859

Job	L117440
Truss	T32
Truss Type	HIP
Qty	1
Job Reference (optional)	1

Builders FirstSource, Lake City, FL 32055
 6:200 s Dec 15 2004 Milltek Industries, Inc. Fri Jun 24 12:37:19 2005 Page 1
 Scale = 1/32



LOADING (psf)	TCLD 20.0	SPACING	2'-0-0
TCCL 7.0	Lumber Increase 1.25	Plates Increase 1.25	CSI TC 0.33
BCLL 10.0	Rep Stress Incr YES	Code FBC2001/ANSI95	(Matrix)
BCDL 5.0			WB 0.14
LUMBER	TOP CHORD 2 X 4 SYP No.2D		BC 0.45
	BOT CHORD 2 X 4 SYP No.2D		TC 0.33
WEBS	2 X 4 SYP No.3		

REACTIONS (lb/size) 2=944/0-3-8, 7=944/0-3-8
 Max Horiz Z=259 (load case 4)
 Max Uplift Z=-461 (load case 5), 7=-461 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=-1096/494, 3-4=-888/429, 4-5=-683/426, 5-6=-888/429, 6-7=-1096/494, 7-8=0/59
 BOT CHORD 2-1=324/868, 10-11=-121/683, 9-10=-121/683, 7-9=-201/688
 WEBS 3-11=-254/271, 4-11=-90/293, 5-9=-90/293, 6-9=-254/272

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98; 120mph (3-second gust); n=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Extent(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 461 lb uplift at joint 7.
 (b) Standard

JUNE 27, 2005 TRUSS DESIGN ENGINEER:
 THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60987
 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	L117440	Truss	COMMON	5	1
Truss Type					
City					
Ply					
Job Reference (optional)	6:200 s Dec 15 2004 Miller Industries, Inc. Fri Jun 24 12:37:20 2005 Page 1				



LOADING (psf)	SPACING	PLATES Increase	Lumber Increase	Rep Stress Incr	NO	Code FBC2001/ANSI95	CSI	DEFL	in (loc)	l/def	L/d	PLATES GRIP	MT20	MT20H	Weight 107 lb
2.0	2'-0"	1.25	1.25	1.25	NO		TC 0.34	Vert(LL) -0.47	8'-10"	>501	240	244/190	244/190	187/143	
7.0	7'-0"	1.25	1.25	1.25	NO		BC 0.93	Vert(TL) -0.67	8'-10"	>351	180	MT20H	187/143		
10.0	10'-0"	1.25	1.25	1.25	NO		WB 0.59	Horz(TL) 0.03	6	n/a	n/a				
5.0	5'-0"	1.25	1.25	1.25	NO										

LUMBER	TOP CHORD 2 X 4 SYP No.1D	BOT CHORD 2 X 4 SYP No.1D	WEBS 2 X 4 SYP No.3
REACTIONS (lb/size)	2=1183/0-3-8, 6=1183/0-3-8	Max Horiz 2=-266(load case 3)	Max Uplift 2=-584(load case 5), 6=-584(load case 6)
FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD 1-2=0/59, 2-3=-1732/677, 3-4=-1678/857, 4-5=-1678/857, 5-6=-1732/677, 6-7=0/59	BOT CHORD 2-10=511/1345, 9-10=-221/810, 8-9=-221/810, 6-8=-397/1345	WEBS 4-10=-499/941, 4-8=-499/941, 3-10=-199/318, 5-8=-199/318

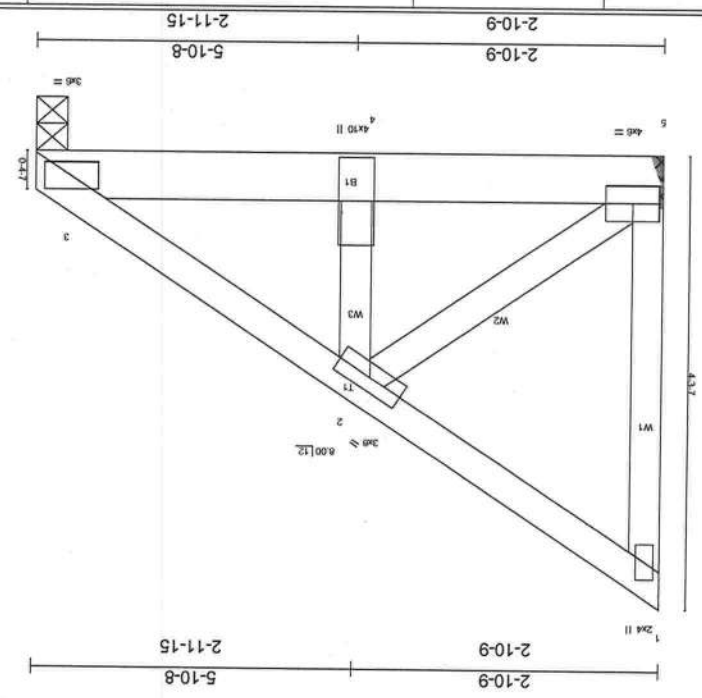
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TC.DL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
- All plates are MT20 plates unless otherwise indicated.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 584 lb uplift at joint 2 and 584 lb uplift at joint 6.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 (1) Regular; Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

Job	L171440	Builder/Source, Lake City, FL 32055	6/20/05 Dec 15 2004 Mittek Industries, Inc. Fri Jun 24 12:37:20 2005 Page 1
Truss Type	T34	SPECIAL	Job Reference (optional)
Qty	1	Ply	1

Scale = 1/20'



LOADING (psf)	20.0	PLATES INCREASE	1.25	CS1	0.17	DEFL	in (loc)	3.4	L/D	n/a
TCLL	7.0	PLATES INCREASE	1.25	TC	0.38	Vert(TL)	-0.02	3.4	>999	180
TCDL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	3	n/a	n/a
BCLL	5.0	Code FBC2001/ANSI95		(Matk)						

LUMBER
 TOP CHORD 2 X 4 SYP No.2D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 5-3-2 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=1483/Mechanical, 3=1483/0-3-8
 Max Horiz 5=208/load case 4)
 Max Uplift 5=-806/load case 4), 3=-665/load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-5=-64/75, 1-2=-55/27, 2-3=-150/1620
 BOT CHORD 4-5=-475/1212, 3-4=-475/1212
 WEBS 2-5=-1492/823, 2-4=-704/1535

NOTES
 1) Wind: ASCE 7-98: 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DCL=1.60 plate grp DCL=1.60.
 2) Refer to girder(s) for truss to truss connections.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 806 lb uplift at joint 5 and 665 lb uplift at joint 3.
 4) Girder carries tie-in span(s): 23-5-0 from 0-0-0 to 5-10-8
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pl)
 Vert: 1-3=-54, 3-5=-477(F)=447)

ULD-VL-MA002-02

DOUBLE DOOR

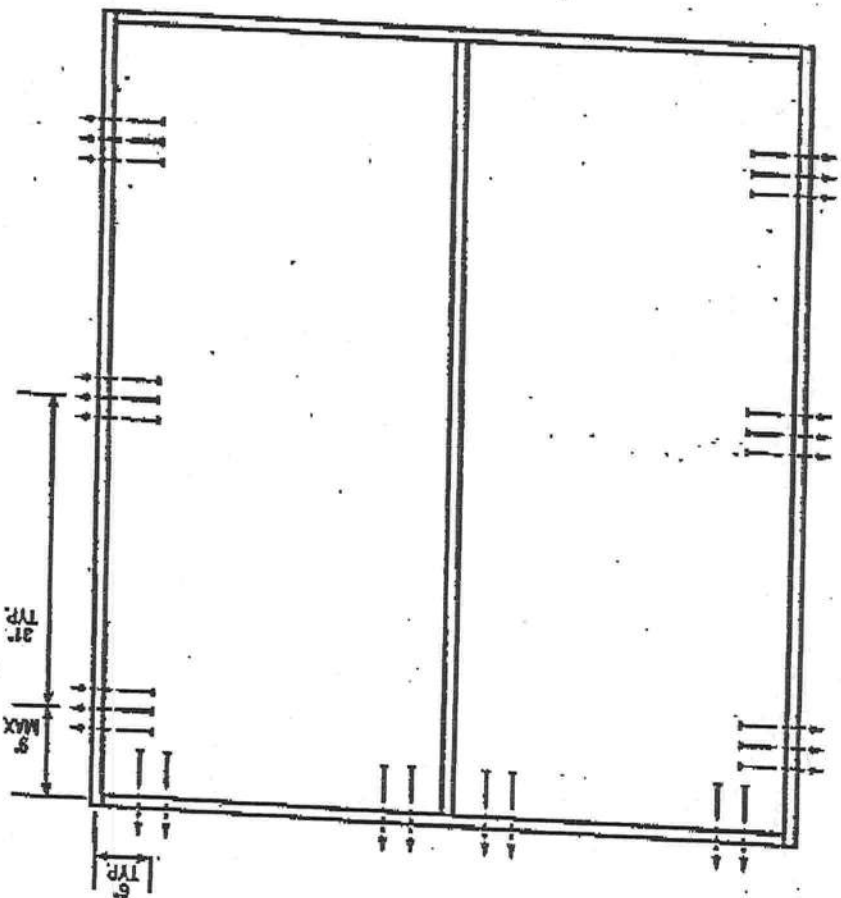
XX
Unit

Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 5 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"



Latching Hardware:

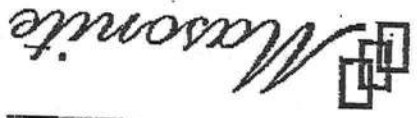
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0247, 0267, 0242, 0247, 0267, 0247, 0267, 0267 or 0267
- Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

Based on required Design Pressure -- see COP sheet for details.

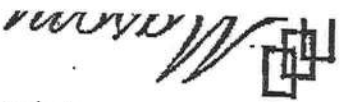
Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners being considered for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear values come from ANSIAF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003
Our continuing program of product improvement makes specifications design and product detail subject to change without notice.



March 19, 2003
 Our standard program of product improvement makes specifications design and product code subject to change without notice.



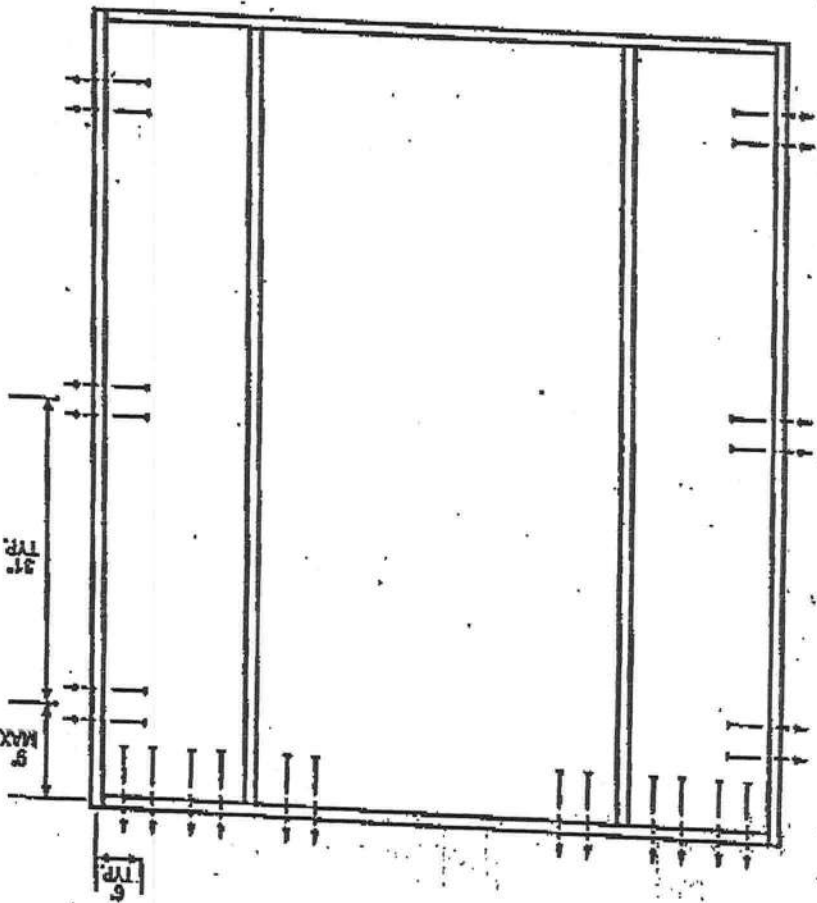
- Notes:**
1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fast analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equivalent adhesive).
 2. The common nail single shear design values come from ANSIRF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" achievement of minimum embedment of 1-1/4".
 3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Based on required Design Pressure - see COP sheet for details.

- Compliance requires that 2" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- UNITS COVERED BY COP DOCUMENT 8249; 8269; 8244; 8249; 8264 or 8269
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Latching Hardware:

Test Data: Product Certificates 83025447C, 83025447B, 83025447C and COP/Retest Report Validation Links 83025447A-001, 802, 803, 804; 83025447B-001, 802, 803, 804; 83025447C-001, 802, 803, 804 provide address information - available from the IFS/WHI website (www.ifswhi.com), the Resource website (www.resource.com) or the hardware technical center.



- Hinge and strike plates require two 2-1/2" long screws per location.
- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 12 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.
- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

SINGLE DOOR WITH 2 SIDELITE

MID-WL-7MA0004-D2

OXO
Unit

March 1, 2003
Our commitment to product improvement makes specifications
a continuous process and we reserve the right to make changes without notice.



2

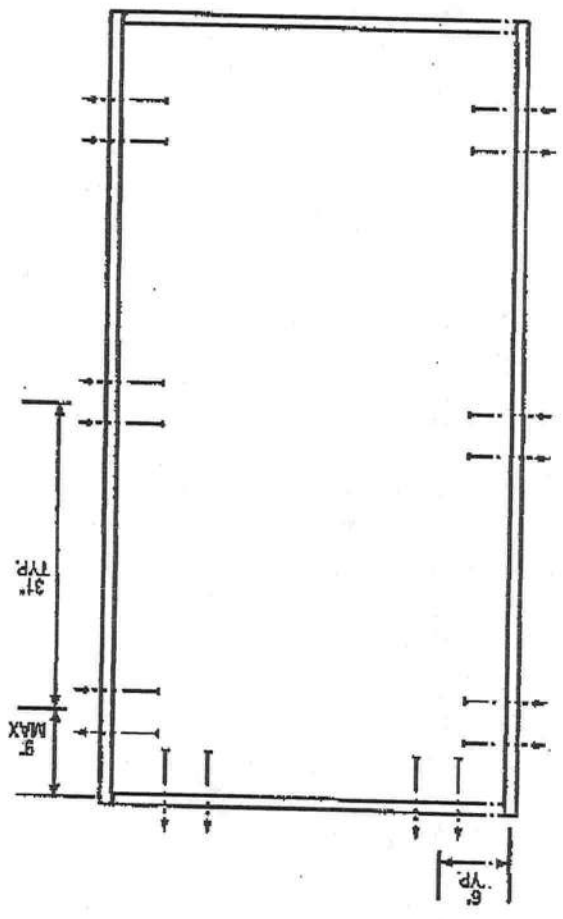
1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/APA & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wind bucks by others, must be anchored properly to transfer loads to the structure.

Notes:

- Based on required Design Pressure - see COP sheet for details.
- All (1) at bottom.
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Latching Hardware:

Test Data Review Certificate #302647A, #302647B, #302647C and DDP/Ret Report Vibration Mark
#302647A-001, 002, 003, 004; #302647B-001, 002, 003, 004; #302647C-001, 002, 003, 004 provides
additional information - available from the IFS/VH website (www.ifs-vh.com), the Masonite website
(www.masonite.com) or the responsible technical center.

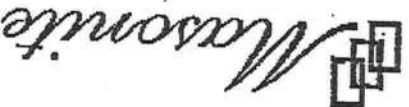


- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.
- **Minimum Fastener Count**
- **Rough Opening (RO)**
- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

SINGLE DOOR

MID-WL-MA001-02

Unit X



March 10, 2003
Our continuing program of product improvement makes specifications design and product detail subject to change without notice.

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AP & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Notes:

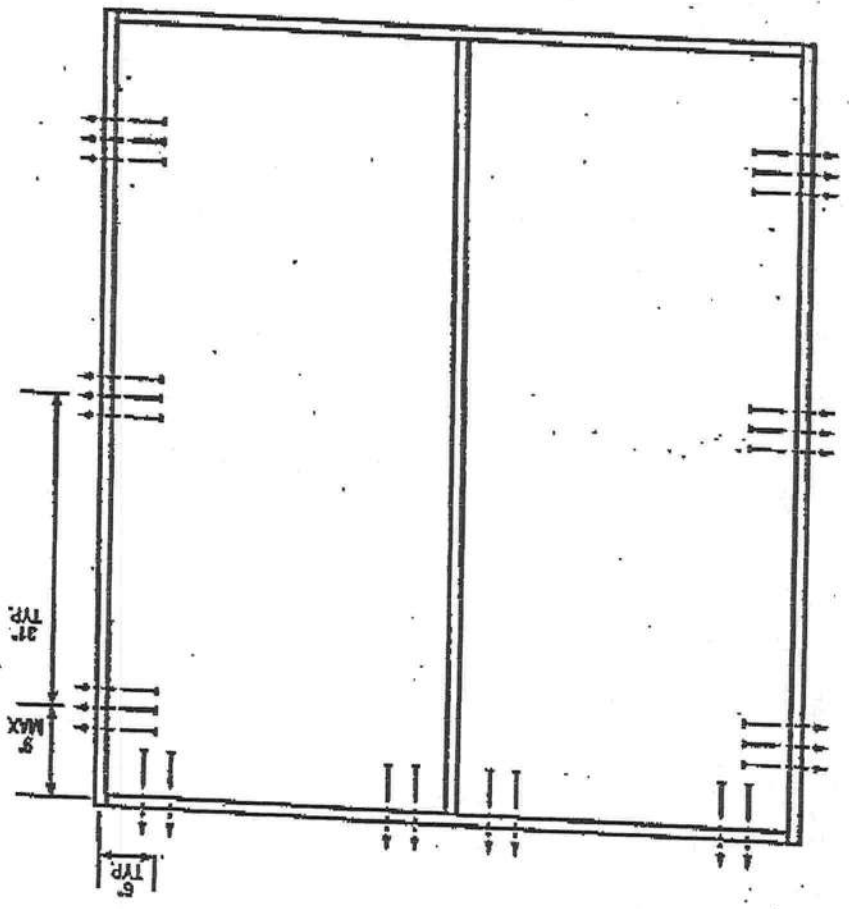
Based on required Design Pressure -- see COP sheet for details.

- UNITS COVERED BY COP DOCUMENT 0247, 0267, 0242, 0247, 0262 or 0267
- Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Latching Hardware:

Product, Material
 Test Data Review Database: 73024474, 73024475, 73024476 and COP/UL Report Location Mark
 Addressed Annotations - include from the ITSMH website (www.masonite.com), the Masonite website
 (www.masonite.com) or the Masonite technical center

- Minimum Fastener Count**
- 6 per vertical framing member for 7" heights and smaller
 - 8 per vertical framing member for heights greater than 7"
 - 8 per horizontal framing member
 - Hinge and strike plates require two 2-1/2" long screws per location.
- Rough Opening (RO)**
- Width of door unit plus 1/2"
 - Height of door unit plus 1/4"

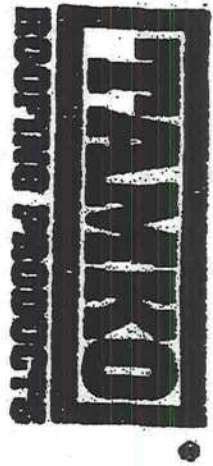


DOUBLE DOOR

MID-MT-MA0002-02

XX Unit

78-2160



January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

EMBER MASTER™

VENT-FREE GAS LOG HEATERS

The EMI Ember Master Gas Log Heaters offer you what ordinary gas logs and wood cannot...the warmth, cleanliness and economy of vent-free space heating combined with the beauty and convenience of gas logs. EMI's unique, clean burning design and precision positioned logs permit vent-free operation with the fireplace damper closed. With no venting required, there is no heat loss up the chimney. Plus they require no electricity so you always have heat - even in the event of power outage.

A Model To Fit Every Need
EMI's Ember Master Gas Log Heaters are available in two log sizes: 18" and 24", each model is available in propane and natural gas. Choose from two types of controls: *Variable Manually Controlled Models* - Allow you to choose any heat setting and flame height desired by simply turning the control knob. *Remote Control "Ready" Models* - Offer you heat at the push of a button or the flip of a switch. EMI's Remote Control Ready Heaters offer you the choice of four types of remote controls as well as the ability to operate the log heater with a variable manual control. Optional remote accessories include: the hand held thermostat remote control, the hand held on/off remote control, the wall mount thermostat control, and the wall mount on/off switch.

EMI Vent-Free Gas Logs install in any masonry, manufactured solid fuel burning fireplace, or AGA or UL certified Vent-Free Universal Firebox. All (manual variable) log heaters are also listed to the ANSI Z21.60 Vented Log Decorative Standard.

Safety Features Built Into Every Gas Log Heater
EMI Vent-Free Gas Log Heaters are designed certified by the American Gas Association (ANSI Z21.1.2) and meet or exceed all regulations and safety performance standards for vent-free gas heaters. Additionally, these log heaters perform well within nationally recognized guidelines for indoor air quality.

The dual-purpose safety pilot system protects against oxygen depletion and any interruption in the fuel supply. If either occurs, it shuts off the gas flow to the burner turning the heater off. An internal pressure regulator controls fluctuations in your gas pressure. These features ensure clean and reliable heat without the worry and inconvenience of vented gas logs or burning wood.

Log Size	No. of Logs	Part/Model Number		Shipping Wt.
		Natural	Propane	
18"	5	01984/VFN18MV	01985/VFP18MV	26 Lbs.
24"	6	01986/VFN24MV	01987/VFP24MV	28 Lbs.
Remote Control Ready/Manual Model				
18"	5	01988/VEN18R	01989/VFP18R	32 Lbs.
24"	6	01990/VEN24R	01991/VFP24R	34 Lbs.
Remote Control Accessories (Must be purchased separately)				
P/N Model Number		Description		
01994/FHRC	3 Lbs.	Receiver and Hand Held Thermostat Remote Control Kit		
01995/FHRC	3 Lbs.	Receiver and Hand Held On/Off Remote Control Kit		
01996/PWMT1	1 Lb.	Wall Mount Thermostat Control Kit		
01997/PWMS2	1 Lb.	Wall Mount On/Off Switch Kit		
Accessories				
01244/HDABK	5 Lbs.	Hood - Flat Black Enamel - Adjustable 28" to 49"		
01245/HDABR	5 Lbs.	Hood - Polished Solid Brass - Adjustable 28" to 49"		

Log Sizing Requirements

Log Size	Height	Depth	Front Width		Gas Connection
			at 1/2" Depth	at 1/2" NPT	
24"	17"	14"	26"	21"	1/2" NPT
18"	17"	14"	20"	20"	1/2" NPT

A fireplace hood accessory may be required to deflect heat away from the mantel shelf. Fire fireplace openings from 28" to 49" wide. Install easily with glass doors or to the hinged fire fireplace opening. Refer to the Ember Master Vent Free Gas Log Over Mantel Manual for more details.

IMPORTANT

Installation must be done by qualified service person.
Read Owner's Manual before using.
Check local codes and ordinances for permitted use.
Approved for masonry (brick, stone, tile, etc.) Not for use in recreational vehicles.

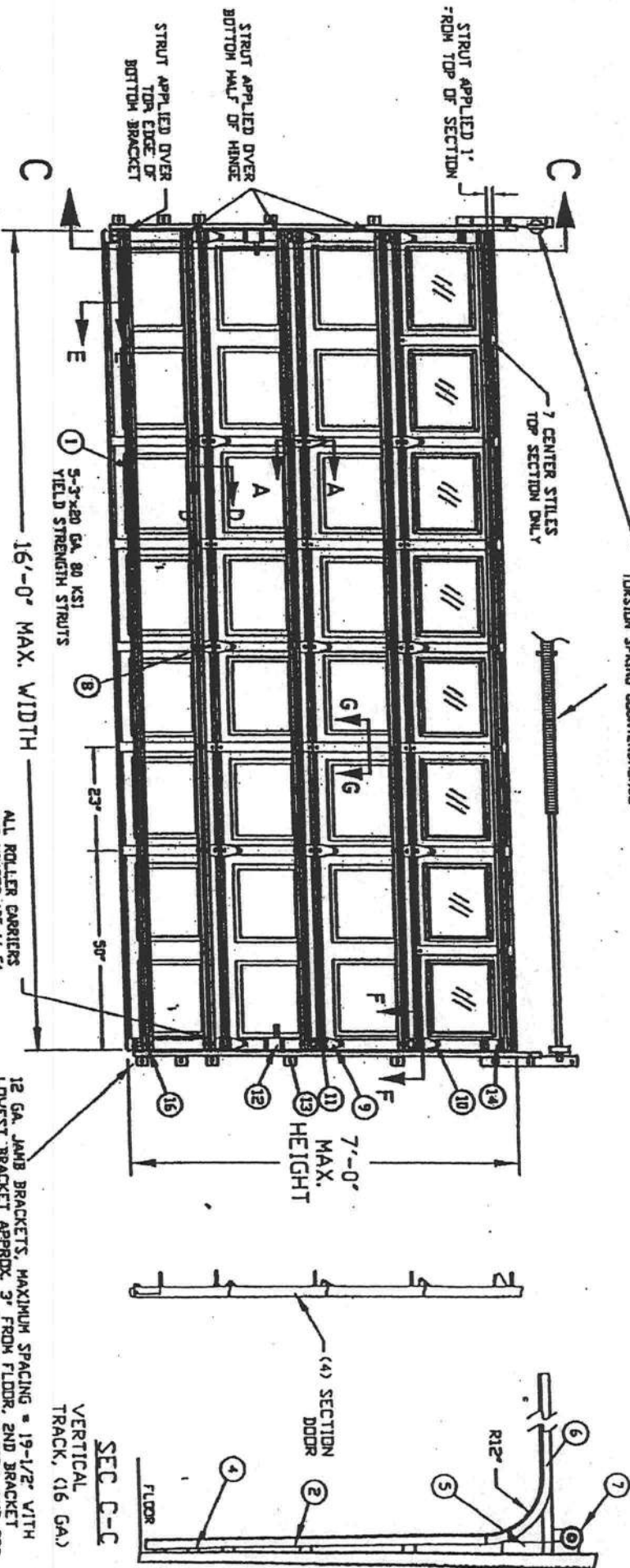
The cover the right to amend product specifications without notice. We reserve the right to amend product specifications without notice. The only warranty we offer is our standard warranty.
Our product cannot be combined between fuel types.
Operating heater at very high elevations could cause damage to the heater.
Provide water vapor in the area heated. Refer to Owner's Manual for specifics.

All products carry a one year warranty.
Please read the warranty for any limitations or restrictions.



For More Information Call 1-800-888-2050
Fireplace Manufacturers Incorporated
P.O. #55488 5/99 Printed in U.S.A.
http://www.emberlakes.com

- NOTES:**
1. TESTED TO POSITIVE AND NEGATIVE 20 PSF SECTION AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
 2. MAXIMUM SECTION HEIGHT = 21'
 3. SECTION HEIGHTS OF 21.0' AND 19.50' ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ADOPTIVE VARIOUS DOOR HEIGHTS.
 4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION, AS TESTED WITH 1/8" BS3 GLASS OR EQUIVALENT OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
 5. MINIMUM LENGTH OF ROLLER STOCK IS 5 1/2" (7" AS TESTED)
 6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOW.
 7. STRUTS SECURED AT ALL LOCATIONS WITH TEX SCREWS.
 8. QUANTITY OF SIDE LOCKS CAN BE DL OR (2 AS TESTED).
 9. DOOR IN TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR, 2ND BRACKET NEAR THE HORIZONTAL & OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION

SEC C-C
VERTICAL TRACK, (16 GA.)

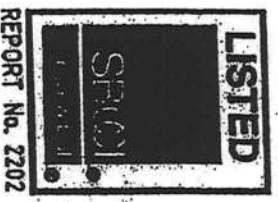
TEST REPORTS ON FILE VIDEO 10/19/00 (002933)

DESIGN LOAD +200 PSF & -200 PSF
TEST LOAD +300 PSF & -300 PSF

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

GAPCO DOORS
SERIES 7400, EXTERIOR STEEL = 0.17 MIN G&S TESTED
SERIES 7825, EXTERIOR STEEL = 0.19" MIN
SERIES 7824, EXTERIOR STEEL = 0.24" MIN
TESTED WITH WINDOWS

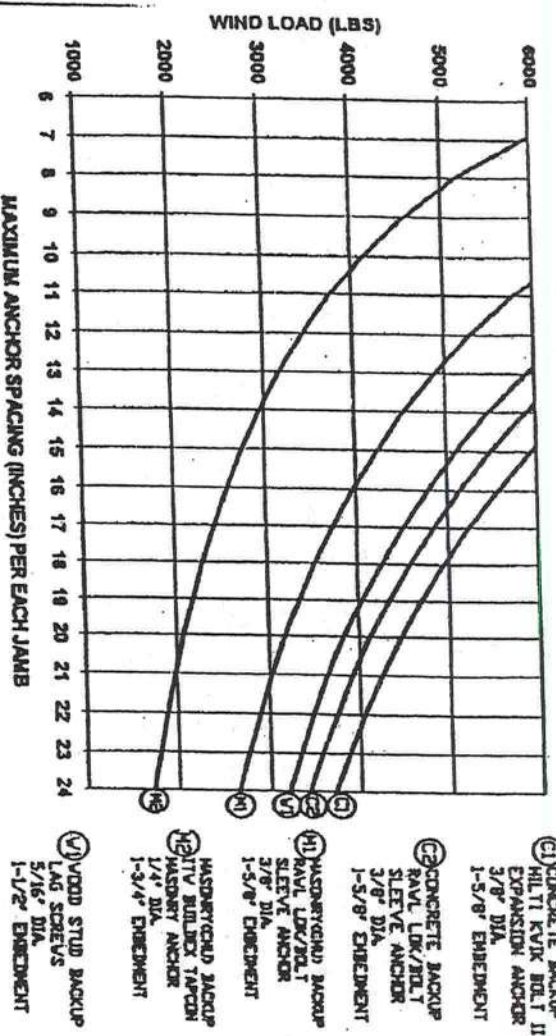
MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CRT. STILE SPACING	STRUTS 80 KSI	VERTICAL TRACK
16'	7'	23"	5	2 IN.



The seal on this drawing only certifies that the product(s) illustrated and described herein represent the dimensions and installation(s) of the door as tested.

SCALE 1/8" = 1'-0"	DATE 10-20-00	REVISIONS (A) 11-10-00
DESCRIPTION 16' X 7' MAX. RAISED PANEL STEEL DOOR - WINDOW LOAD ±20 PSF	APPROVED BY	DESIGNED BY
DRAWN BY S. VICKHAM	REVIEWED (A)	DATE 11-10-00
PAGE 1 OF 2	BRACING TABLE	V13220-1

WIND LOAD vs ANCHOR SPACING



DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)
 LOAD $\frac{FT^2}{FT^2}$

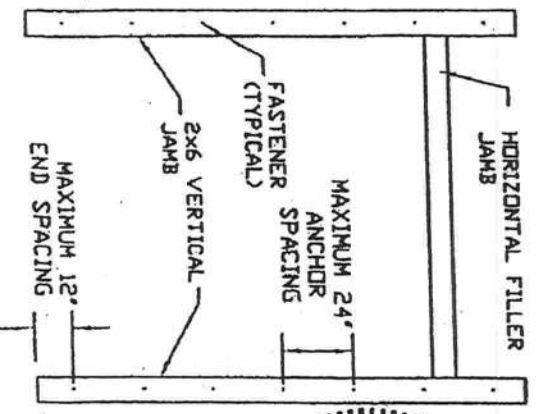
EXAMPLE

30 LBS X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(1) USE 22' SPACING
 (2) USE 21' SPACING
 (3) USE 19' SPACING

(4) USE 16' SPACING
 (5) USE 10' SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



SEAL
 PE No. 024280

PROFESSIONAL ENGINEER
 NAGER R. KEYVAN

STATE OF MISSISSIPPI
 REGISTERED PROFESSIONAL ENGINEER
 No. 53774

3/8/2002

2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

- 1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER 'HURRICANE' POSTS.
- 2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SECCI 'STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION' SST'D 10, 'CURRENT EDITION.
- 3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
- 4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.
- 5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTH C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- 6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- 7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"
- 8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.
- 9) WASHERS ARE REQUIRED ON ALL FASTENERS.
- 10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- 11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKET SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
 5050 BASELINE ROAD
 MONTGOMERY, IL 60538

GABCO

SALE NONE
 DATE 8-30-99
 APPROVED BY: [Signature]
 DIV: [Blank]

DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS

DATE: 8/27/02
 DRAWING NUMBER: A10560

The Texan

Manufacturing Facility
Jasper, Florida
Phone: 386-792-1121
Fax: 386-792-0712
Sales E-mail: sales@espl.com
Web Page: www.espl.com

“Our Name Says It All”

Series 130-131

Single Hung Window Integral Fin

FEATURES

CLEANING SIMPLICITY: Sash tilts inward to interior for easy cleaning of glass exterior.

SPECIFICATION CONFORMANCE: Meets and exceeds the A.N.S.I./A.A.M.A. Specifications and Quality Certified by laboratory testing and in plant inspection. Meets Specification DP35/50.

RIGID CONSTRUCTION: All members are mortise-coped to provide interfitting and joined with plated steel screw fasteners. Sash bottom latch rail is tubular for added structural strength against twisting and bowing. Meeting rails inter-lock to assure weatherstrip compression.

Integral extruded stiffener appendages have been added to sash horizontal rails for added resistance to high velocity winds.

FINGER TOUCH LOCKING: Cam latches engage an integral part of frame for security protection.

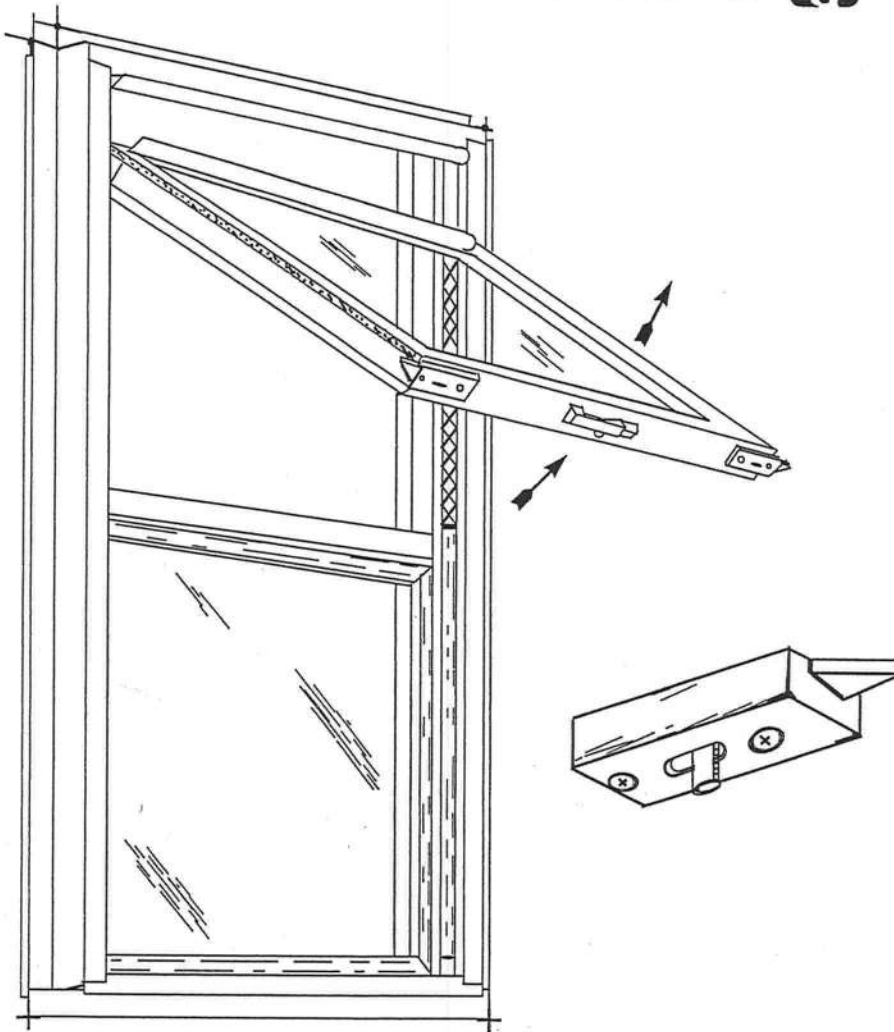
MULTIPLE GLAZING OPTIONS: May be glazed with single or double strength glass or with 1/2" Sealed Insulating Glass.

COLONIAL MUNTINS: Available in various grid patterns for aesthetic appeal. Sealed Insulating Glass has the muntin grid located between the glass to facilitate ease of cleaning.

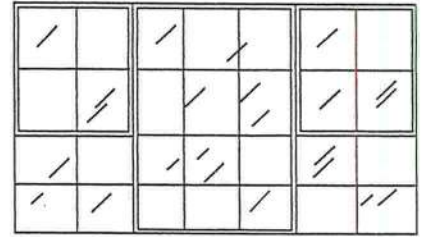
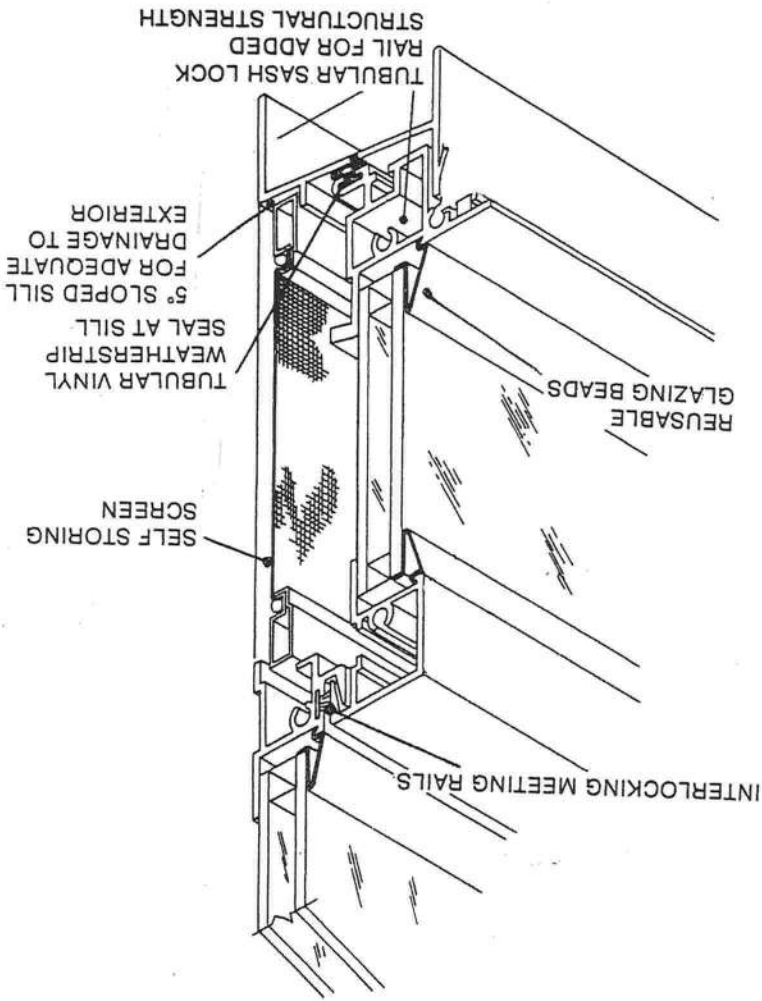
MATCHING PICTURE WINDOW: Choose from a variety of types and sizes to match vented windows.

BAKED ENAMEL FINISH: Available in White or Bronze Acrylic painted finish to match any decor and enhance the beauty of the architecture.

EASY INSTALLATION: One inch nailing fin simplifies installation. Fin also doubles as a flashing around the exterior wall opening.



PPG Glass Technology
Since 1883



MULLED UNIT

STANDARD MOUNTING GRIDS

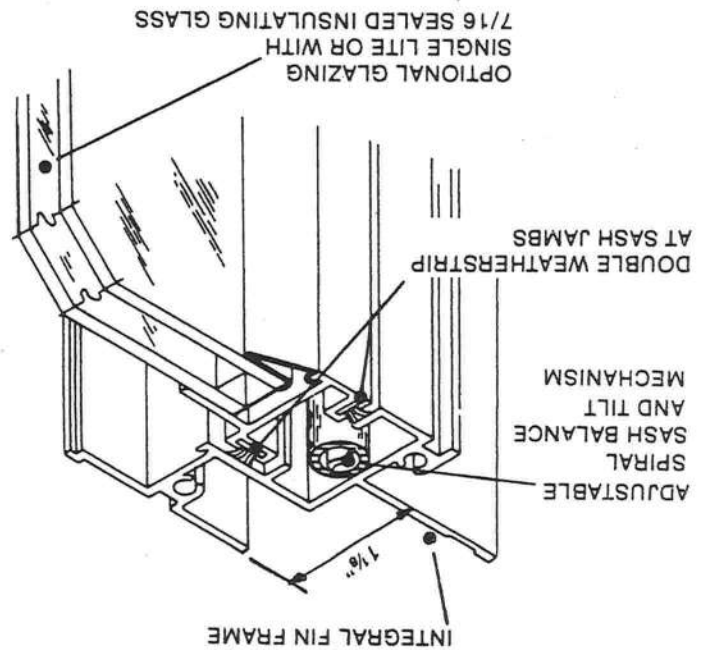
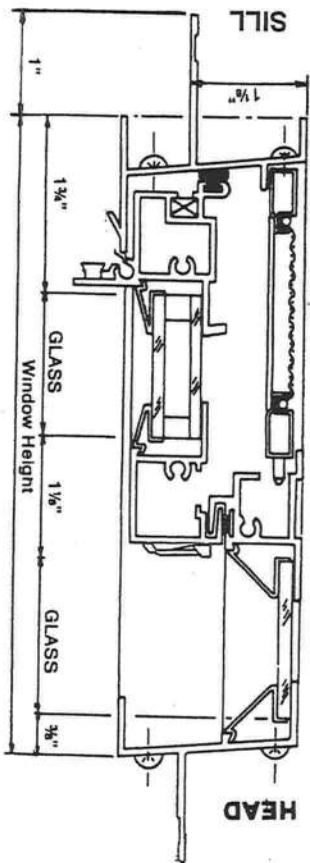
VENTED		PICTURE	
60	15	15	30
50	12	16	24
44	12	16	24
40	12	16	24
38	12	16	24
30	12	16	24

Number of Lites: 24, 30, 34, 38, 40, 50, 60

WINDOW WIDTH

Man'l Code: E-076-1
 Model: The Texan
 Designation: DH-R35/50
 4664/3254

FMA Keystone Certification Program
 Accredited By:
 American National Standards Institute
 Std: ANSI/AAMA/WDMA 101/IS2 - 97



March 11, 2003
Our current program of product improvement makes specifications design a 1/2" product detail subject to change without notice.

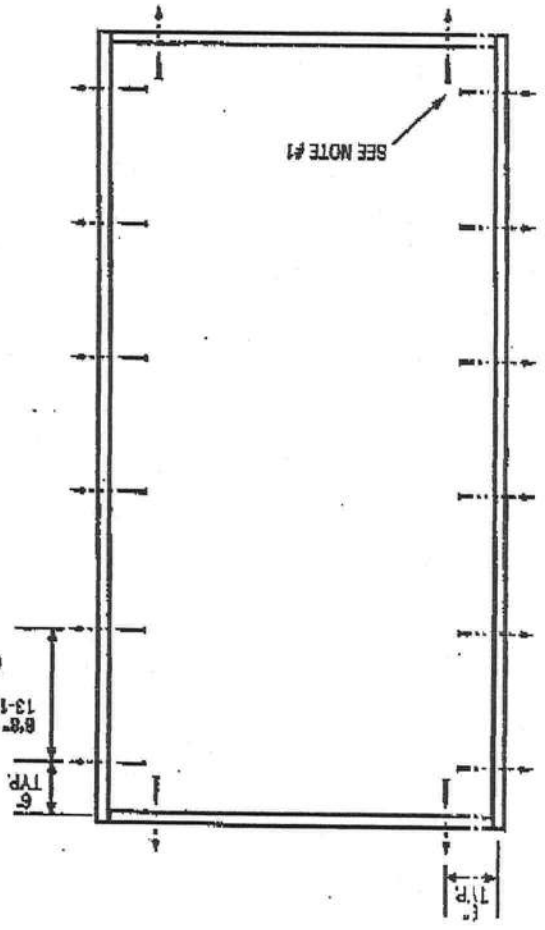


1

- Notes:**
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and mid fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
 2. The wood screw single shear design values come from Table 11.3A of ANSI/APA & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
 3. Wind buxks by others, must be anchored properly to transfer loads to the structure.

- Latching Hardware:**
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
 - COMPLIANCE COVERED BY CDP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266
 - Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- Based on required Design Pressure - see CDP sheet for details.

Test Data Review Certificate #3026447A, #3026447B, #3026447C and CDP Test Report Validation Matrix
 #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITW/WH website (www.iteamh.com), the Masonite website (www.masonite.com) or the Resonance technical center.



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

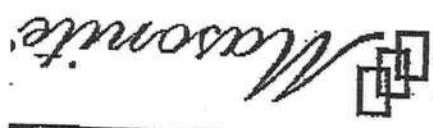
SINGLE DOOR

MID-VL-MA0001-02

X Unit

March 10, 2005

The underlying purpose of product improvement makes specifications for existing projects subject to change without notice.



- Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
- The wood screw single shear design values come from Table 11.3A of ANS/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and E.L.C.O Dade County approvals respectively, each with minimum 1-1/4" embedment.
- Wood bucks by others, must be anchored properly to transfer loads to the structure.

Notes:

Based on required Design Pressure - see COP sheet for details.

Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

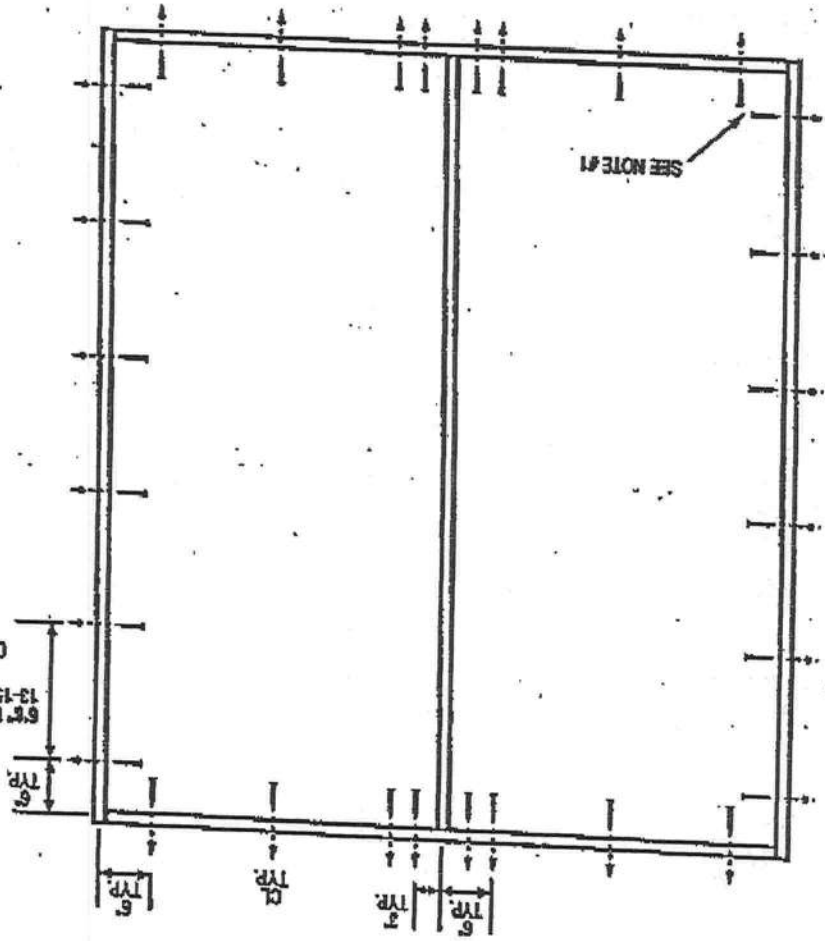
Compliance requires that GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

Latching Hardware:

Test Data Review Certificate #2025-4474, #2025-4475, #2025-4476, #2025-4477 and COP/Field Report Verification Marking #2025-4478-4481, #02, #03, #04, #2025-4479-4481, #02, #03, #04 provides additional information - available from the ITW website (www.itsw.com), the Masamite website (www.masamite.com) or the Masamite technical office.



- 6 per vertical framing member
- 8 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.
- **Rough Opening (RO)**
- Width of door unit plus 1/2"
- Height of door unit plus 1/4"



DOUBLE DOOR

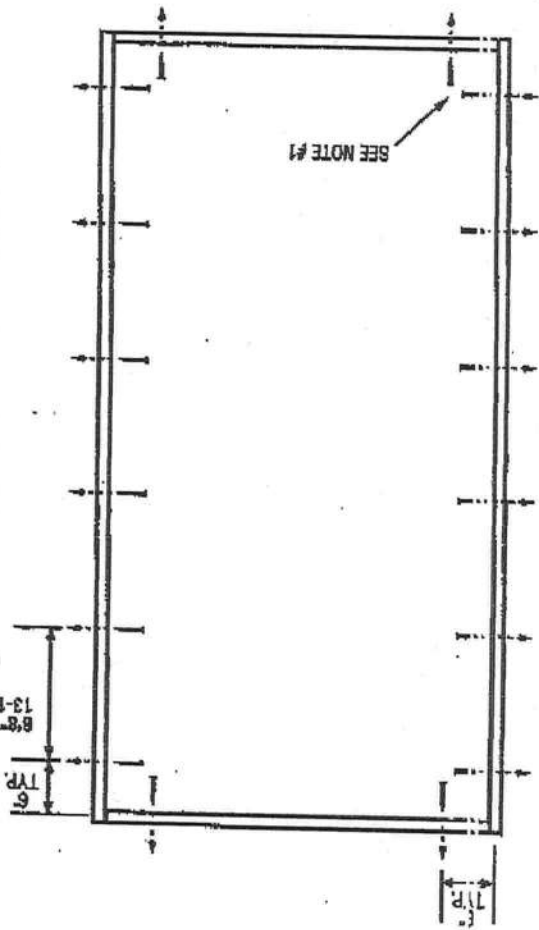
MID-VL-MAD02-02



SINGLE DOOR

MID-WL-MA001-02

X
Unit



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #302647A, #302647B, #302647C and COP/Ret Report Validation Matrix #302647A-001, 002, 003, 004; #302647B-001, 002, 003, 004; #302647C-001, 002, 003, 004 provides additional information - available from the TRS/VH website (www.dbsent.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- Based on required Design Pressure - see COP sheet for details.

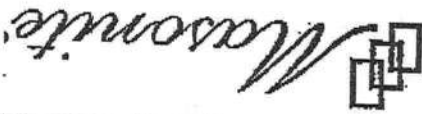
NOTES:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and mid fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the TW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wind bucks by others, must be anchored properly to transfer loads to the structure.



March 11, 2003
Our continuing program of product improvement makes specifications design a product detail subject to change without notice.

March 10, 2005
 The continuing program of product improvement makes specifications design and product detail subject to change without notice.



1

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw shear design values come from Table 11.2A of AMS/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and EICO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

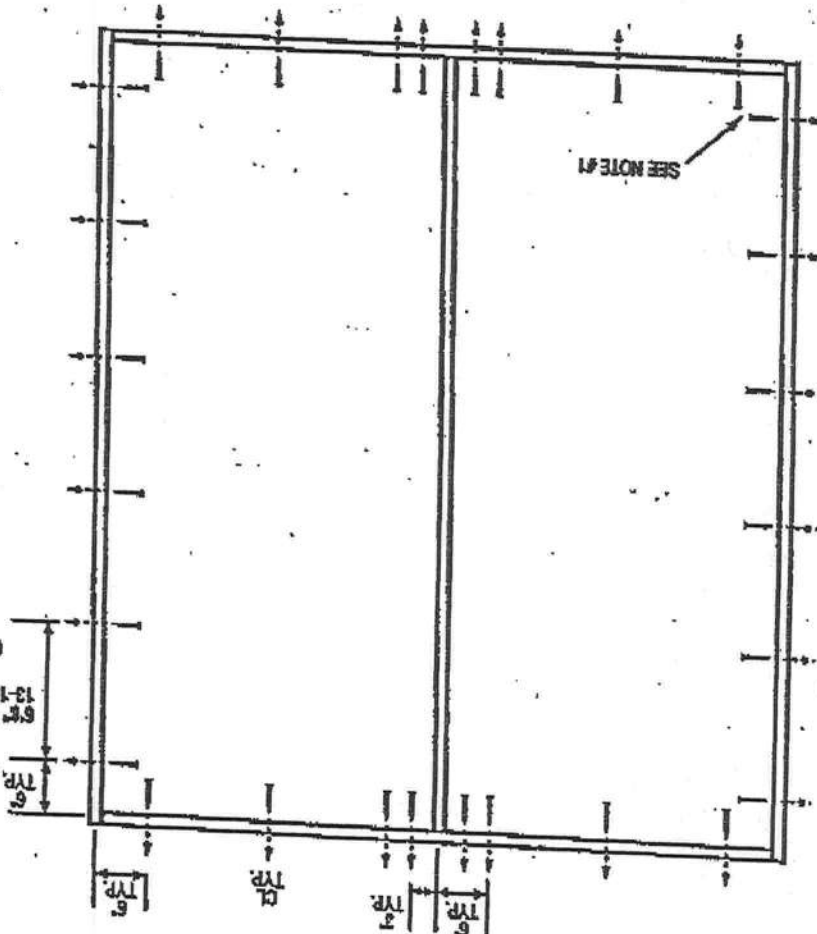
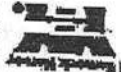
Notes:

Based on required Design Pressure - see COP sheet for details.
 and (1) at bottom.

Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
 UNITS COVERED BY COP DOCUMENT #247, #267, #242, #247, #267 or #267
 Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top

Latching Hardware:

For Data Review Certificate #3025479, #3025479, #3025479, #3025479 and COP-Test Report Validation Mark:
 #3025479-001, 002, 003, 004, #3025479-001, 002, 003, 004, #3025479-001, 002, 003, 004 provides
 additional information - available from the ITW website (www.itw.com), the hardware website
 (www.masonite.com) or the hardware technical center.

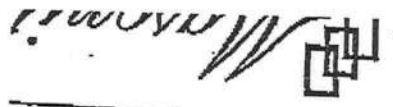


- Minimum Fastener Count**
- 5 per vertical framing member
- 8 per horizontal framing member
- Hinge and strike plates require two
- 2-1/2" long screws per location.
- Rough Opening (RO)**
- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

DOUBLE DOOR

MID-VL-MA002-02

XX
 Unit

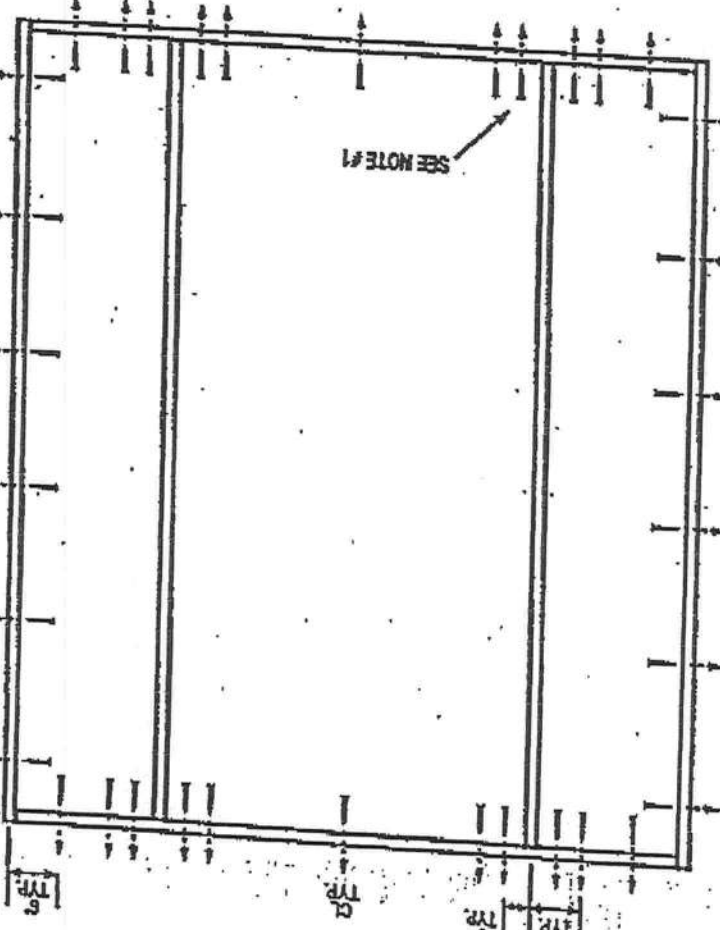


March 10, 2003
 Our continuing program of product improvement means specifications design and product detail subject to change without notice.

- Notes:**
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
 2. The wood screw single shear design values come from Table 11.3A of ANSI/APA & PA NDS for southern pine lumber with a side member thickness 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELOD Dade County approvals respectively, each with minimum 1-1/4" embedment.
 3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

- Latching Hardware:**
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
 - UNITS COVERED BY COP DOCUMENT 0249, 0268, 3244, 3249, 3264, or 3269
 - Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
 - Based on required Design Pressure - see COP sheet for details.

Threshold Hardware
 Post Data Network Corporation 73025477A, 73025477B, 73025477C and COP Detail Report Threshold Hardware
 73025477A-001, 002, 003, 004; 73025477B-001, 002, 003, 004; 73025477C-001, 002, 003, 004 provides
 additional information - available from the (13/9/01) website (www.threshold.com), the hardware website
 (www.threshold.com) or the hardware technical center.



- Minimum Fastener Count**
- 6 per vertical framing member
 - 11 per horizontal framing member
 - Hinge and strike plates require two 2-1/2" long screws per location.
- Fough Opening (FO)**
- Width of door unit plus 1/2"
 - Height of door unit plus 1/4"

6" UNIT
 12-15/16"
 17-1/8"
 MAXIMUM
 ON CENTER TYP.

SINGLE DOOR WITH 2 SIDELITE

MID-WEST-77A0004-02

AUTH. # 9461
 386-362-6133 (FAX)
 386-362-3678
 LIVE OAK, FL 32064
 P.O. BOX 187
 GTC DESIGN GROUP, LLC
 GARY GIRL, PE

		GENERAL INFORMATION
PAGE 2	DESIGN BASIS	
PAGE 3	CALCULATION / DESIGN SUMMARY	
PAGE 4	DESIGN LOADS	
		WALL COMPONENTS
PAGE 5 - 7	SHEARWALL DESIGN - N/S	
PAGE 8 - 10	SHEARWALL DESIGN - E/W	STRUCTURAL ITEMS
PAGE 11 - 16	WIND LOADS - ASCE 7-98	

INDEX

WIND LOAD AND STRUCTURAL CALCULATIONS
FOR
GATEWAY DEVELOPMENT, LLC
"Mackenzie" MODEL HOME
LOT 14
ARBOR GREENE @ EMERALD LAKES

PROJECT NAME: GATEWAY DEVELOPMENT
 PROJECT NUMBER: PF05-033

GTC DESIGN GROUP



Project name: Mackenzie Windload Analysis
Project: PF05-034
Client: Gateway Dev
Calculations: Gary Gill, PE
Date: 4/7/2005

Design Basis

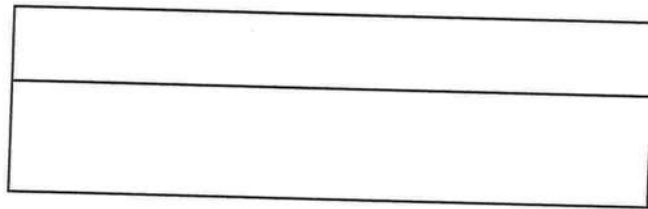
Design Loads

Wind Load 110
 Floor Live Load
 Sleep Areas =
 All Others =
 Floor Dead Load
 Wall Dead Load
 Roof Live Load
 Roof Dead Load
 30 psf
 40 psf
 10 psf
 10 psf
 20 psf
 10 psf

Load Combinations

DL + LL(floor) + LL (roof)
 DL + LL(floor) + WL
 DL + WL
 Wind load

Exposure B



Building Information

Shape Rectangle
 Length 46.17 ft
 Width 56.4 ft
 Type 1 storey sog

References

2001 Florida Building Code
 ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
 AITC Timber Construction Manual

Calculation / Design Summary

Truss / Rafter Assembly	
Truss1	Uplift max =
	1305.00 lb
	* per truss manufacturer
	Stud uplift
	Uplift =
	870.00
	Shearwalls
	Max. shearwall load =
	262 plf
	Roof Diaphragm
	Perpendicular to roof
	Shear =
	48.23 lb/in
	Parallel to roof
	Shear =
	106.21 lb/in
Hardware / Nailing requirement	
Simpson H10	Capacity= 1490 lb
Simpson SP4 @ 32" O.C.	Capacity= 442.5
8ft wall -8d nails @ 4" O.C.	Capacity = 525
	* increase 1.4 for wind
8d @ 6 in. edges, 8d @ 12 in. field	capacity = 252 lb/in (unblocked)
8d @ 6 in. edges, 8d @ 12 in. field	capacity = 252 lb/in (unblocked)

LOAD SHEET DATA

Project Name Mackenzie Windload Analysis
 Project Number PF05-034
 Client Gateway Development
 Date 4/7/2005

Windloading

WALL LOADS

Dead 8 psf
 Wall height 8 psf

DATA SHEET

Wind 110 mph
 Building Length 63 ft
 Building Width 46 ft
 Type 1 storey sog

PERFORATED SHEARWALLS (WOOD FRAMED WALLS)

Surface1	12.49 psf
Surface2	6.58 psf
Surface3	4.22 psf
Surface4	-3.21 psf
Surface5	9.79 psf
Surface6	-1.86 psf

1st Floor Height
 2nd Floor height

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	8	Roof Dia	14.42
Vertical Roof height	14.00	Length of Building	63
2nd Floor height	0	Width of Building	31.6
1st Floor height	8		

Wind Pressure per ASCE 7 - Normal to surface Case A

Windward Roof - Surface 2	psf	6.58	Wall - Leeward	12.49 psf
Leeward Roof - Surface 3	psf	-4.22	Wall - Leeward Surface 4	-3.21 psf
Horizontal loads from wind perpendicular to ridge (N / S)				Total Wall
				15.7 psf

Roof Pressure (interior)

Windward Roof Horiz. (psf)	3.65
Leeward Roof Horiz. (psf)	-2.34
Total	5.99
Tributary area (roof)	661.50
Roof shear values	3962.89

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	15.7
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	15.7
Tributary area to each Shearwall (sf)	317.50
Wall shear values to each shearwall	4984.75
Total shear to top of 2nd floor (lb) per wall (actual)	0.00
Total shear to top of 1st floor (lb) per wall (actual)	8947.64

1st Floor Shearwall (ft)

Number of shearwall segments in each column	1	1
Full wall length	32.625	46.17
Shearwall #1 length	26.33	37.2
Shearwall #2 length	0	0
Wall height ratio (h/b)	0.30	0.22
Rigidities of shearwalls	10.64	15.26
Lateral load on shearwall column (lbs) based on rigidity	3675.80	5271.84
Percent Full-Height Sheathing	80.70%	80.57%
Shearwall #1	0.9	0.9
Shear capacity adjustment	0.9	0.9
Shearwall rating (pif) w/ 1.4 increase for wind	483	483
Design Shear Capacity	11445.65	16170.84
Stress Ratio	0.32	0.33
uplift at shear ends	1861.40	1889.55
Shear and uplift between holdown, v and u	155.12	157.46

Anchor Bolt Shear Capacity pif	24"	36"	48"
Bolt size / spacing	1/2" dia	1/2" dia	1/2" dia
	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data	
Root Pitch (x:12)	8 Roof Dia
Vertical Roof height	14.00 Building Length of
2nd Floor height	0 Width of Building
1st Floor height	8
	31.6
	63
	14.42

Wind Pressure per ASCE 7 - Normal to surface Case B

Windward Wall - Surface 5	9.79	psf
Leeward Wall - Surface 6	-1.86	psf
Total Wall	11.65	

Horizontal loads from parallel to ridge (EM) =

2

Root Pressure (Interior)	
Windward Roof Horiz. (psf)	9.79
Leeward Roof Horiz. (psf)	-1.86
Total	11.65
Tributary area (roof) to each shearwall (sf)	322.00
Root shear values to each shearwall	3751.30

Wall Pressure - 1st Floor

11.65	Sum. of wind. & lee. (psf)
294.00	Tributary area to each Shearwall (sf)
3425.10	Wall shear values to each shearwall

0.00	Total shear to top of 2nd floor (lb) per wall (actual)
7176.40	Total shear to top of 1st floor (lb) per wall (actual)

1st Floor Shearwall (ft)

Number of shearwall segments in each column	1	1
Shearwall #1 length	17.5	14
Shearwall #2 length	11.75	0
Wall height ratio (h/b)	0.46	0.57
Rigidities of shearwalls	6.82	5.26
Lateral load on shearwall column (lbs) based on rigidity	4050.50	3125.90
Percent Full-Height Sheathing	0.58	0.74
Shearwall #1	0.58	0.74
Shear capacity adjustment	0.75	0.85
Shearwall rating (pif) w/ 1.4 increase for wind	483	483
Design Shear Capacity	6339.38	5747.70
Stress Ratio	0.64	0.54
uplift at shear ends	3703.32	3152.16
shear and uplift between holdown, v and u	184.64	262.68

Anchor Bolt Shear Capacity pif			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

WIND98 v3-02
Wind Load Design per ASCE 7-98

Description: Mackinsey - Arbor Green
Analysis by: Gary Gill

User Input Data	
Structure Type	Building
Basic Wind Speed (V)	110 mph
Structural Category	II
Exposure	B
Struc Nat Frequency (n1)	1 Hz
Slope of Roof (Theta)	33.69 Deg
Type of Roof	Gabled
Kd (Directionality Factor)	0.85
Eave Height (Eht)	11.00 ft
Ridge Height (Rht)	26.25 ft
Mean Roof Height (Ht)	22.00 ft
Width Perp. To Wind Dir (B)	63.46 ft
Width Paral. To Wind Dir (L)	42.40 ft
Damping Ratio (beta)	0.02

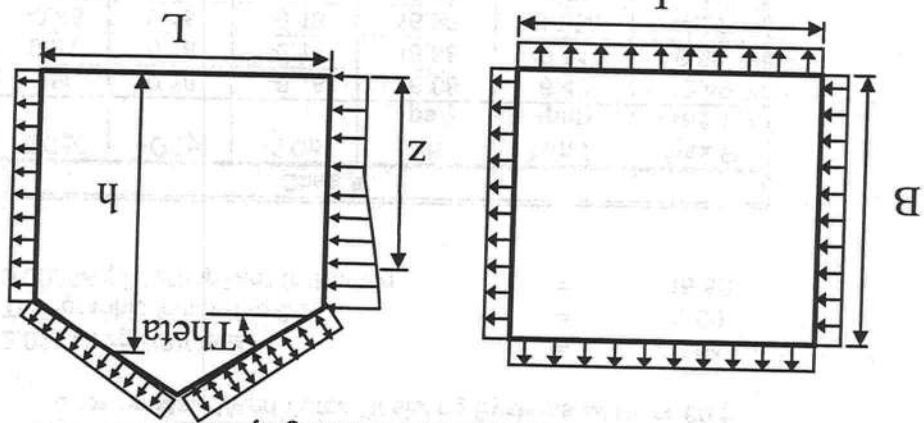
Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.52
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
Hurricane Prone Region (>100 mph)	
Alpha =	7.000
Zg =	1200.000
Table C6-4 Values	
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
I =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method	
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85
Gust Factor Category II: Rigid Structures - Complete Analysis	
Zm	30.00 ft
Zmin	30.00 ft
Lzm	$Cc * (33/z)^{0.167}$
Lzm	$I * (z_m/33)^{Epsilon}$
Q	$(1/(1+0.63 * ((Min(B,L)+Ht)/Lzm)^{0.63}))^{0.5}$
Gust2	$0.925 * ((1+1.7 * Izm^{3.4 * Q}) / (1+1.7 * 3.4 * Izm))$
Gust Factor Summary	
Gust2	0.8661
Q	0.9002
Lzm	309.99 ft
Lzm	0.3048
Zm	30.00 ft
Since this is not a flexible structure the lesser of Gust1 or Gust2 are used	
G	0.85

Figure 6-3 - External Pressure Coefficients, Cp
 Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (H/z_g)^{2/\alpha}$	0.64	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot Kh \cdot Kht \cdot Kd$	16.88	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	$.00256 \cdot V^2 \cdot 1 \cdot Khcc \cdot Kht \cdot Kd$	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	Cp 0.8

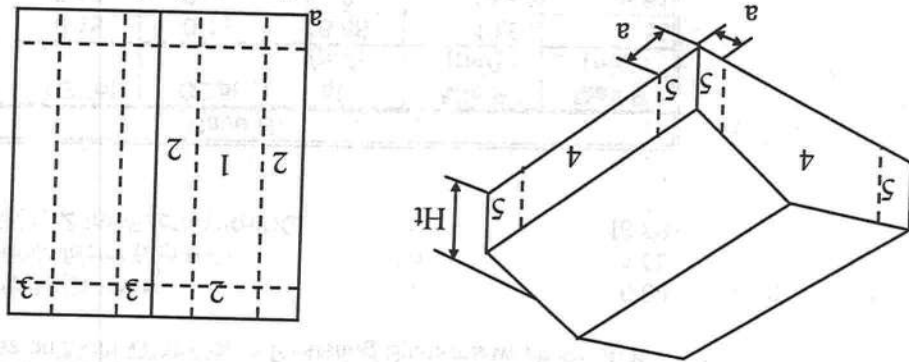
Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 63.46 ft Face			
Additional Runs may be req'd for other wind directions			
Cp	Pressure (psf)	+Gcpi	-Gcpi
Leeward Walls (Wind Dir Normal to 63.46 ft wall)	-0.50	-10.21	-4.14
Side Walls	-0.70	-13.08	-7.01
Roof - Wind Normal to Ridge ($\theta \geq 10^\circ$) - for Wind Normal to 63.46 ft face	-0.20	-5.92	0.15
Windward - Max Negative	-0.20	-5.92	0.15
Windward - Max Positive	0.27	0.85	6.93
Leeward Normal to Ridge	-0.60	-11.65	-5.57
Overhang Top (Windward)	-0.20	-2.88	-2.88
Overhang Top (Leeward)	-0.60	-8.61	-8.61
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 63.46 ft face	-0.92	-16.17	-10.09
Dist from Windward Edge: 0 ft to 11 ft	-0.92	-16.17	-10.09
Dist from Windward Edge: 11 ft to 22 ft	-0.89	-15.85	-9.77

* Horizontal distance from windward edge

WIND98 v3-02
 Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCP
 Loads on Components and Cladding for Buildings w/ $Ht \leq 60$ ft



Gabled Roof
 $10 < \theta \leq 45$

$a = 4.24$ ==> 4.24 ft

Component	Width (ft)	Span (ft)	Area (ft ²)	Zone	GCP		Wind Press (lb/ft ²)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

23992

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
 Company Address: 301 NW Cole Terrace
 Company Business License No. 3118075
 FHA/VA Case No. (if any) _____
 State FL City Lake City Zip 32055
 Company Phone No. 386-755-3611

Section 2: Builder Information

Company Name: 2-persons 2 Family
 Company Phone No. _____

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip): 219 NW Hilltop Dr. Lake City, FL 32055
 Type of Construction (More than one box may be checked) Slab Basement Crawlspace Other _____
 Approximate Depth of Footing: Outside 12 Inside 24
 Type of Fill R.F.T.

Section 4: Treatment Information

Date(s) of Treatment(s): 5-2-06
 Brand Name of Product(s) Used: Term-X 2
 EPA Registration No.: 53443-92
 Approximate Final Mix Solution %: 0.25
 Approximate Size of Treatment Area: Sq. ft. 2962 Linear ft. 234
 Approximate Total Gallons of Solution Applied: 674
 Was treatment completed on exterior? Yes No
 Service Agreement Available? Yes No
 Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____
 Comments _____
 Name of Applicator(s): Steve Brennan
 Certification No. (if required by State law): JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature: _____ Date: 5-2-06

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

Order Product #2581 • from CROWMAX • 1-800-252-4011

5/17/06
 [Handwritten signature]

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 60" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

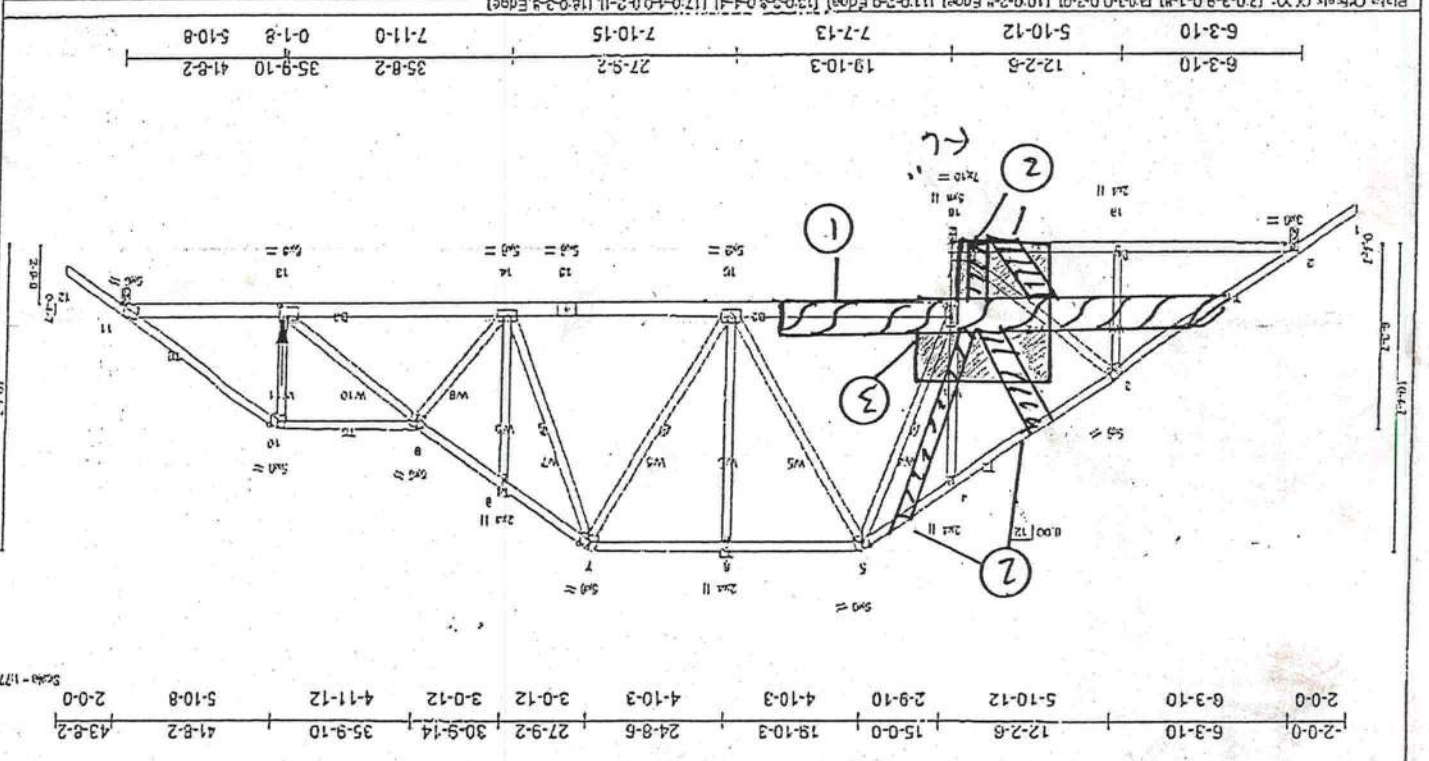
REACTIONS (lb/ft) 2=391/0-3-8, 10=206/0-0-3-8, 11=254/0-0-2
 Max Horiz=112(road case 3), 15=1102(road case 5), 11=1200(road case 5)
 Max Uplift=52(road case 5), 15=1102(road case 5), 11=1200(road case 5)
 Max Grav=2595(road case 5), 12=2069(road case 1), 11=2511(road case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 1-2=407/1007, 11-12=0/82
 10-11=307/1007, 11-12=0/82
 2-19=408/351, 18-19=117/351, 15-16=222/494, 15-16=57/1263, 6-16=273/296, 7-16=72/244, 7-14=107/2059, 8-14=128/212, 9-14=197/2449, 10-14=108/1824, 17-18=170/775, 4-17=214/229

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.10 except
 B1 X 4 SYP No.2
 2 X 4 SYP No.3

BRACING
 TOP CHORD
 SHEATHED OR 3-0-0 cc purlins.
 BOT CHORD
 Rigid ceiling directly applied or 5-2-14 cc bracing.
 WEBS
 1 Row at midpt
 5-17, 7-16, 7-14, 4-18

LOADING (psf)	SPACING	PLATE INCREASE	LUMBER INCREASE	Code FBC2001/ANSI95	CSI	DEFL	VER(L)	VER(TL)	HOR(TL)	PLATES	CRIP	Weight: 234 lb
20.0	2-0-0	1.25	1.25	NO	0.51	0.03	0.27	13-14	11	M120	2x4/180	
TCL							>989	>989	n/a			
TCDL							150	150	n/a			
BCLL												
BCLD												



Job	Truss Type	Truss	Truss Type	Truss	Truss Type	Truss	Truss Type	Truss	Truss Type	Truss	Truss Type	Truss	Truss Type	Truss	Truss Type
L117440	T18														

Handwritten notes: 5/17/06, [Signature]

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

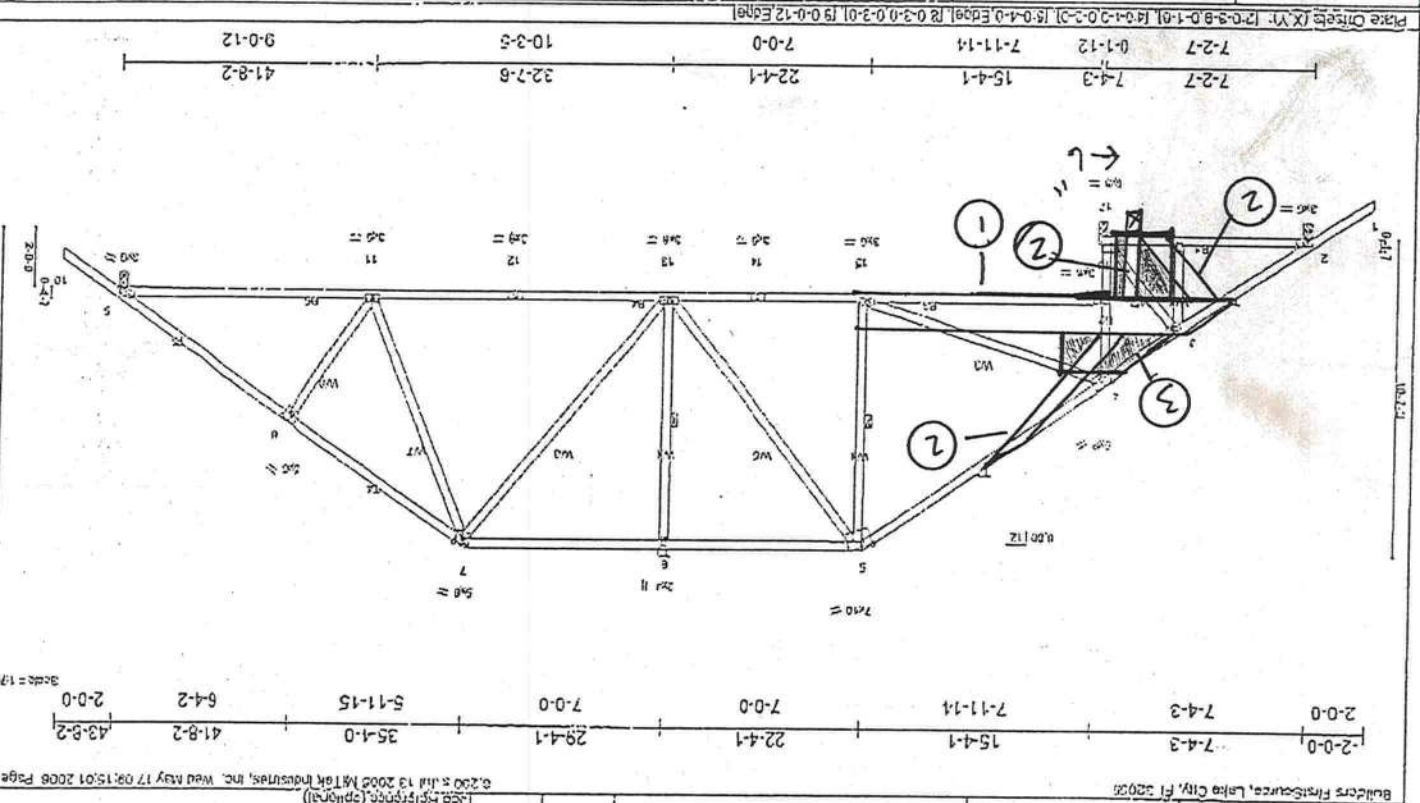
1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 60" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

REACTIONS (lbs/ft) 2-269/0-3-0, 9-165/0-3-0, 17-170/0-3-0
 Max Horiz 2-42 (load case 4)
 Max Up/12-590 (load case 6), 17-251 (load case 4)
 Max Grv 2-372 (load case 1), 9-155 (load case 1), 17-179 (load case 1)

LUMBER
 TOP CHORD 2 X 4 SYP No. 2
 BOT CHORD 2 X 4 SYP No. 2 - EXCEPT
 B2 2 X 4 SYP No. 3
 WEBS 2 X 4 SYP No. 3

ERACING
 TOP CHORD Shathing or 4-1-2 cc purline.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 cc bracing.
 WEBS 1 Row at midpt 6-15, 6-15

LOADING (psf)	SPACING	PLATE INCREASE	LUMBER INCREASE	REP STEPS IN	Code FECD000/ANSP195	CS1	TC	BC	WB	(Matk)	DEFL	In (occ)	vdell	L/D	PLATES	GRP	Weight 244 lb
20.0	2-0	1.25	1.25	YES		0.58	0.67	0.65			-0.27	11-13	> 959	180	MT20	24/180	
7.0											0.05	9	NA	NA			
10.0																	
5.0																	



Job	L117410	Truss	T10	Truss Type	SPECIAL	Qty	1
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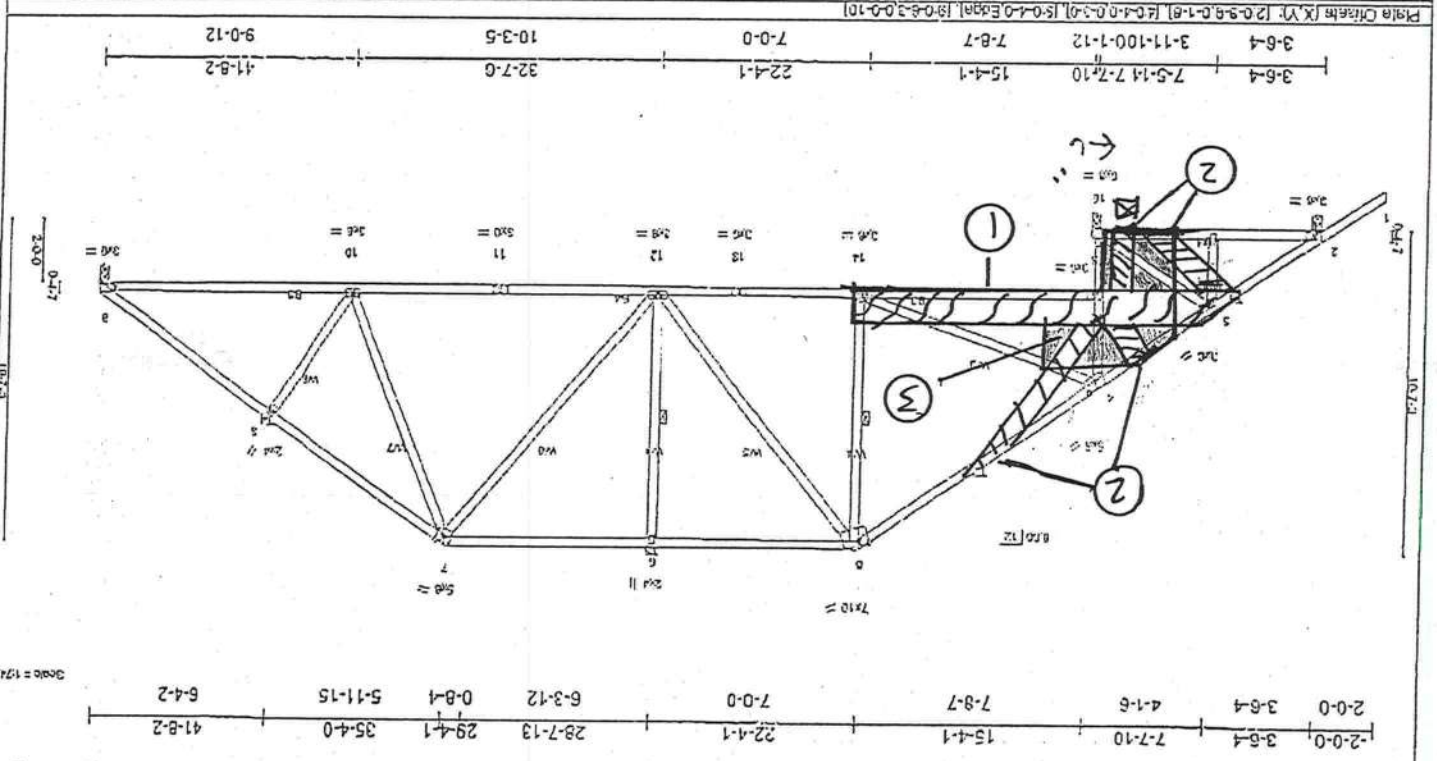
Handwritten notes: *1/2" x 48" x 60" Plywood or OSB W / (20) 10D'S AT EACH MEMBER.*

ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FLORIDA 33559
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA-7882
1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 60" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

REACTIONS (lbs/ft) 8=1425/0-2-8, 2=204/0-2-8, 18=1783/0-3-8
 Max Horiz=2521(lead case 4)
 Max Uplift=416(lead case 6), 2=520(non case 8), 16=932(lead case 4)
 Max Grav=1425(lead case 1), 2=387(lead case 7), 16=1783(lead case 1)

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B2 2 X 4 SYP No.3
 W2 X 4 SYP No.3
BRACING
 TOP CHORD Strained or 5-10-14 oc purlins
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
 WEBS 1 Row at midpt 5-14, 6-12

LOADING (psf)	SPACING	CSI	DEFL	in (occ)	WOOD L/D	PLATES	GRIP
20.0	2'-0"	TC 0.55	Ven(L)	-0.20	10-12	MT20	2W/190
7.0	Lumber Increase	EC 0.88	Ven(R)	-0.37	10-12		
10.0	Red Stress Incr	WB 0.87	Horz(L)	0.05	9		
5.0	Code FB2007/NMSIS	(Altha)			N/A		



Job	L117440	Truss Type	T11	City	Flv	1	Job Reference (optional)
Builder	Builders FirstSource, Lake City, FL 32025	Truss Type	SPECIAL	City	Flv	1	Job Reference (optional)

5/17/06
Lutz

ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FLORIDA 33559
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

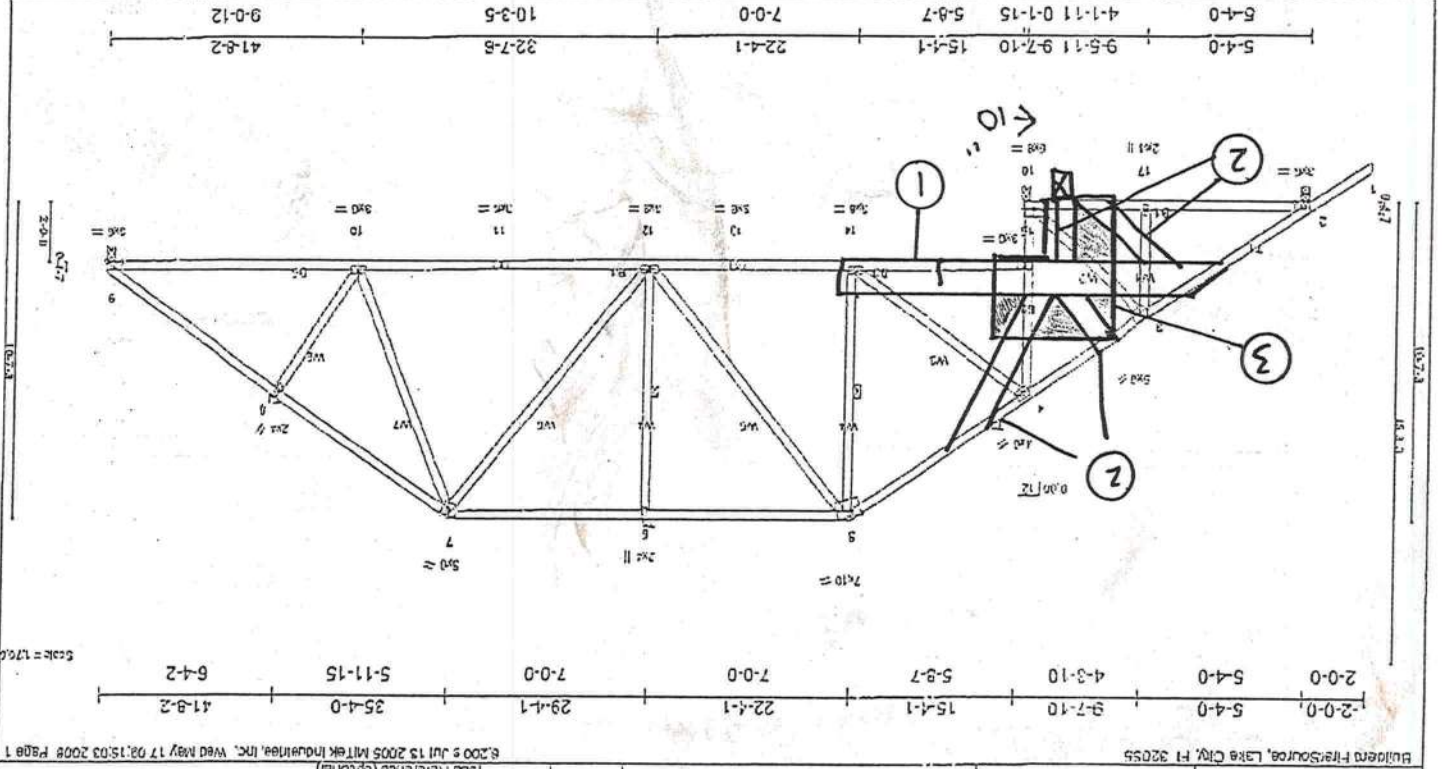
REACTIONS (lbs/ft) 6=1340/0-3-8, 2=1750/0-3-8, 16=1777/0-3-8
 Max Horiz=451(load case 4)
 Max Lift=550(load case 5), 16=515(load case 6), 2=295(load case 7), 10=1777(load case 1)
 Max Gmv=9=1540(load case 1), 2=1784(load case 7), 10=1777(load case 1)

BRACING
 TOP CHORD Sheathed or 4-0-10 cc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 cc bracing.
 WEBS 1 Row at Midpt 5-14, 6-12

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 Except
 WEBS 2 X 4 SYP No.3

LOADING (psf)	SPACING	Code F2003 (ANSI)	DEFL	PLATES
TCLL 20.0	2'-0"	Plates Increase 1.25	in (loc)	CRIP
TCDL 7.0		Lumber Increase 1.25	Vert(TL) 0.23 10-12 > 998 240	MT20 2x4/190
BCLL 10.0		Rep Size Incr YES	Horz(TL) 0.04 9 N/A N/A	Weight 2x4 D
BCDL 5.0		Code F2003 (ANSI)		

Plym Options (X,Y): [2'-0-0-0-1-8], [3'-0-0-0-2-0], [5'-0-0-0-3-0], [6'-0-0-0-0-0]



Job Reference (optional)	1	1	1	1
Truss Type	SPECIAL			
Truss	T12			
Job	L117440			

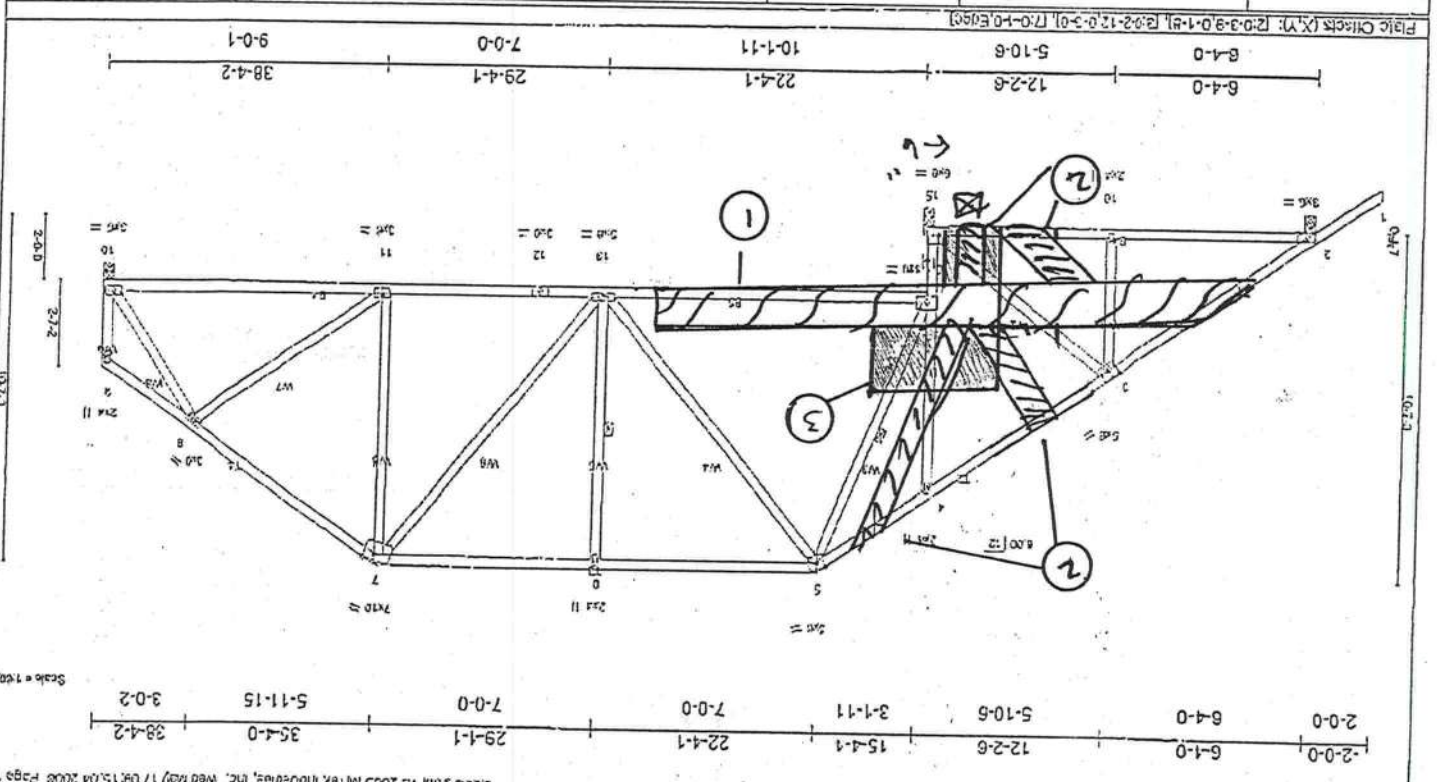
MAY. 17. 2006 9:25AM BUILDERS FIRST SOURCE NO. 0297 P. 2

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ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

LOADING (psf)	SPACING	PLATE INCREASE	LUMBER INCREASE	REP STRESS INC	CODE	CS1	DEFL	VERT(L)	HORIZ(TL)	WEBS	TOP CHORD	BOT CHORD	WEBS	REACTIONS
20.0	2'-0"	1.25	1.25	YES	FBC200 (ANSI)	TC 0.01	in (ft)	0.02	0.02	10	Sheathed or 6-0-0 oc purlins, except end verticals.	1 Row at midpt.	2 X 4 SYP No.3	Max Horiz=2522(load case 5), 15+92(load case 4), 10+11(load case 3) Max Uplift=150(load case 5), 15+92(load case 4), 10+11(load case 3) Max Grv=2617(load case 7), 15+162(load case 1), 10+107(load case 1)
TCLL														
TCDL														
BCLL														
BCLD														



Job	L177440	Truss	T13	SPECIAL	Qty	1	1
Job	BUILDER'S FIRST SOURCE						
Job	NO. 0297 P. 5						

Handwritten notes:
 5/17/02
 M. J. ...

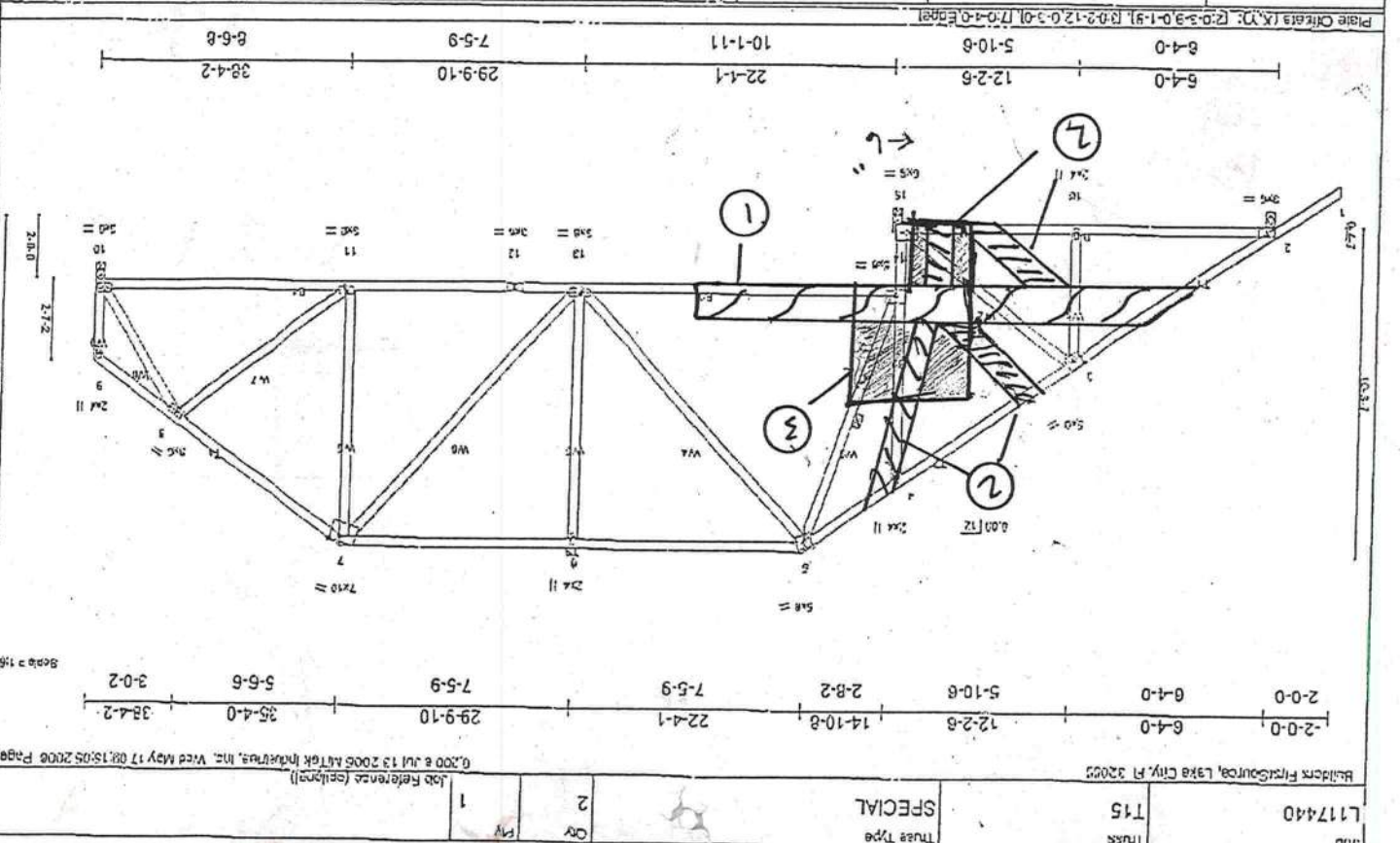
ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

LOADING (psf)	SPACING	PLATE INCREASE	LUMBER INCREASE	REP STRESS INCR	YES	Code FECD001/ANSIBS	CS1	TC	BC	VB	Horz(L)	Vert(L)	DEFL	in (oc)	in (oc)	L/D	MTZO	GRUP	PLATES	Weight 239 lb
20.0	2'-0"	1.25	1.25	YES			0.59	0.71	0.28	0.02	10	15-14	0.31	15-14	0.99	180	24-1190			
7.0																				
10.0																				
5.0																				

BRACING: TOP CHORD, BOT CHORD, WEBS
 Sheathed or 5-11-12 oc purlins, except end verticals.
 Fixed ceiling directly applied or 8-0-2 oc bracing.
 1 Row at midpt
 S-14

REACTIONS (lb/ft): 2=518/0-3-8, 15=1895/0-3-8, 10=1071/0-3-8
 Max Horiz 2=631 (load case 5), 15=938 (load case 5), 10=157 (load case 3)
 Max Uplift 2=151 (load case 5), 15=938 (load case 5), 10=157 (load case 3)



5/17/08
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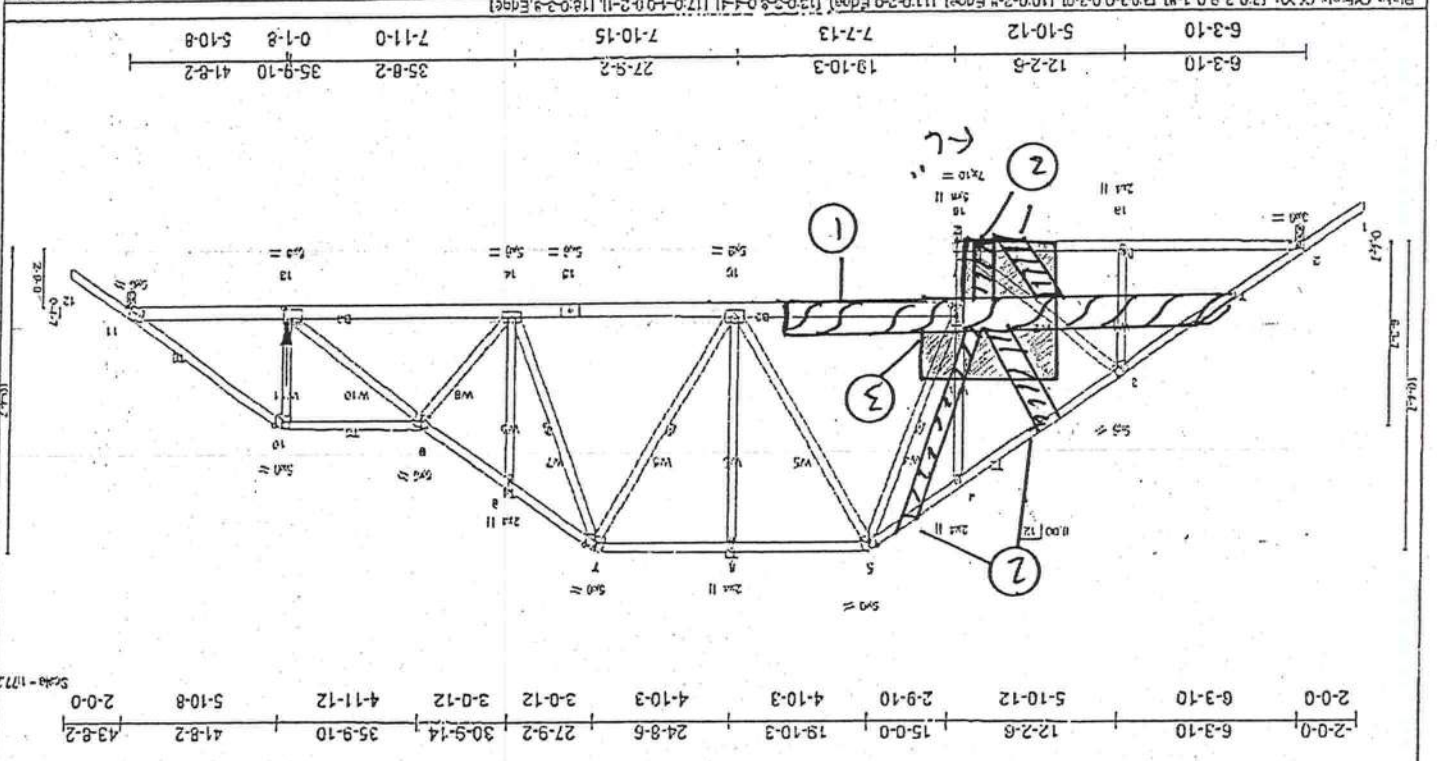
ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 60" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

REACTIONS (k/ft): 2=391/0.3-8, 10=206/0.3-8, 11=2541/0.2-2
 Max Horiz 2=112(load case 2), 10=1102(load case 3), 11=1204(load case 5)
 Max Uplift 2=52(load case 3), 10=1102(load case 3), 11=2511(load case 1)
 Max Grav 2=595(load case 4), 10=2069(load case 4), 11=2511(load case 1)
 FORCES (lb) - Maximum Compression Tension
 TOP CHORD 1-2=408/261, 10-11=3076/1097, 11-12=0/82
 BOT CHORD 2-19=408/261, 18-19=117/65, 10-11=222/494, 15-16=576/1263, 6-16=273/295, 7-16=728/244, 7-14=1970/2059, 8-14c=128/212, 8-14e=1977/1010, 8-14f=131/65, 10-11=108/1824, 17-18=1769/775, 4-17=241/229

LUMBER 2 X 4 SYP No.2
 TOP CHORD 2 X 6 SYP No.1D Except
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 BRACING
 TOP CHORD Sheathed or 3-0-0 or pultr.
 BOT CHORD Right ceiling directly applied or 6-8-14 or bracing.
 WEBS 1 Row at midpt 5-17, 7-16, 7-14, 4-18

LOADING (psf)	SPACING	CSI	DEFL	in (in)	WEBS	L/D	PLATES	CRIP	Weight: 234 lb
TCLL 20.0	2-0-0	0.51	Vert(L)	0.18	11	13-14	M120	2x4/180	
TCDL 7.0	Lumber Increase 1.25	BC 0.19	Horz(TL)	0.03	11	13-14			
BCLL 10.0	Red Stress Incr NO	WB 0.53							
BCDL 5.0	Code FBC2001/ANSI/S15	(Metric)							



Job	117440	Truss Type	SPECIAL	Qty	1	Job Reference (optional)	
Builder's First/Source, Lake City, FL 32055						01.2005 Jul 13 2005 Match Industries, Inc. Wed May 17 09:15:07 2008 Page 1	

WALL
5/17/04

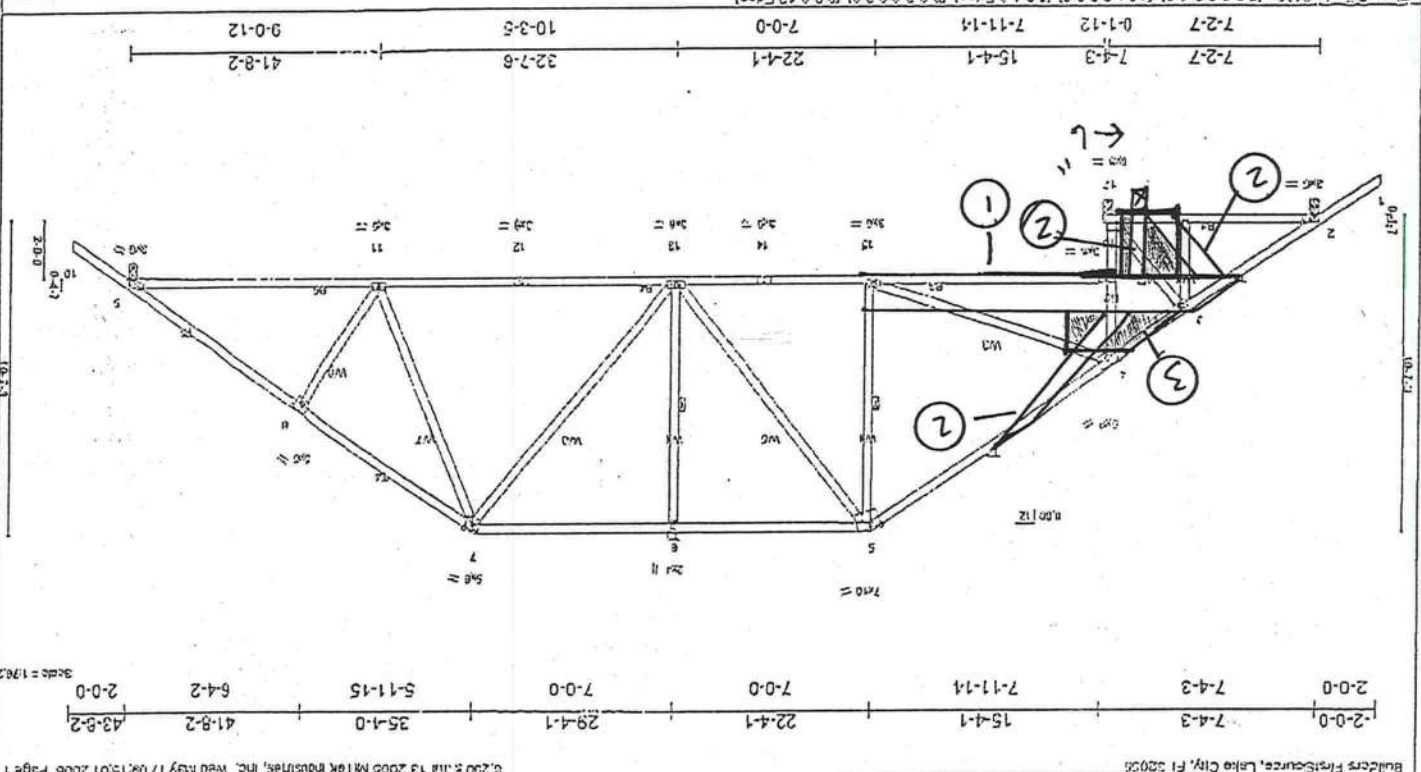
ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 6" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

REACTIONS (lb/in): 2-3630-3-0, 9-16500-3-0, 17-17030-0-0
 Max Horiz 2-42 (load case 4)
 Max Uplift 2-392 (load case 6), 9-450 (load case 5), 17-450 (load case 4)
 Max Gravity 2-372 (load case 1), 9-1550 (load case 1), 17-1703 (load case 1)

LUMBER
 TOP CHORD 2 X 4 SYP No. 2
 BOT CHORD 2 X 4 SYP No. 3
 WEBS 2 X 4 SYP No. 3
 SHEATHING OR 4-1-2 OC PURLINE
 RIGID CEILING DIRECTLY APPLIED OR 0-0 OC BRACING
 1 Row at midpt
 G-13, G-15

LOADING (psf)	2.0	SPACING	2-0	CSI	TC	0.58	DEFL	In (loc)	1.13	WELL	L/D	240	PLATES	GRP	24/180
TCDL	7.0	Lumber Increase	1.25	BC	BC	0.67	Vent(L)	-0.33	1-13	>959	180	MT20			
BCLL	10.0	Rep Stays Incr	YES	WB	WB	0.65	Horz(L)	0.05	9	N/A	N/A	PLATES	GRP	24/180	
BCLD	5.0	Code FBC2001/ANSI B5										PLATES	GRP	24/180	



Job	L117410	Truss	T10	SPECIAL	2	1
Builder's FirstSource, Lake City, FL 32056						

Handwritten notes: *W. J. ...*

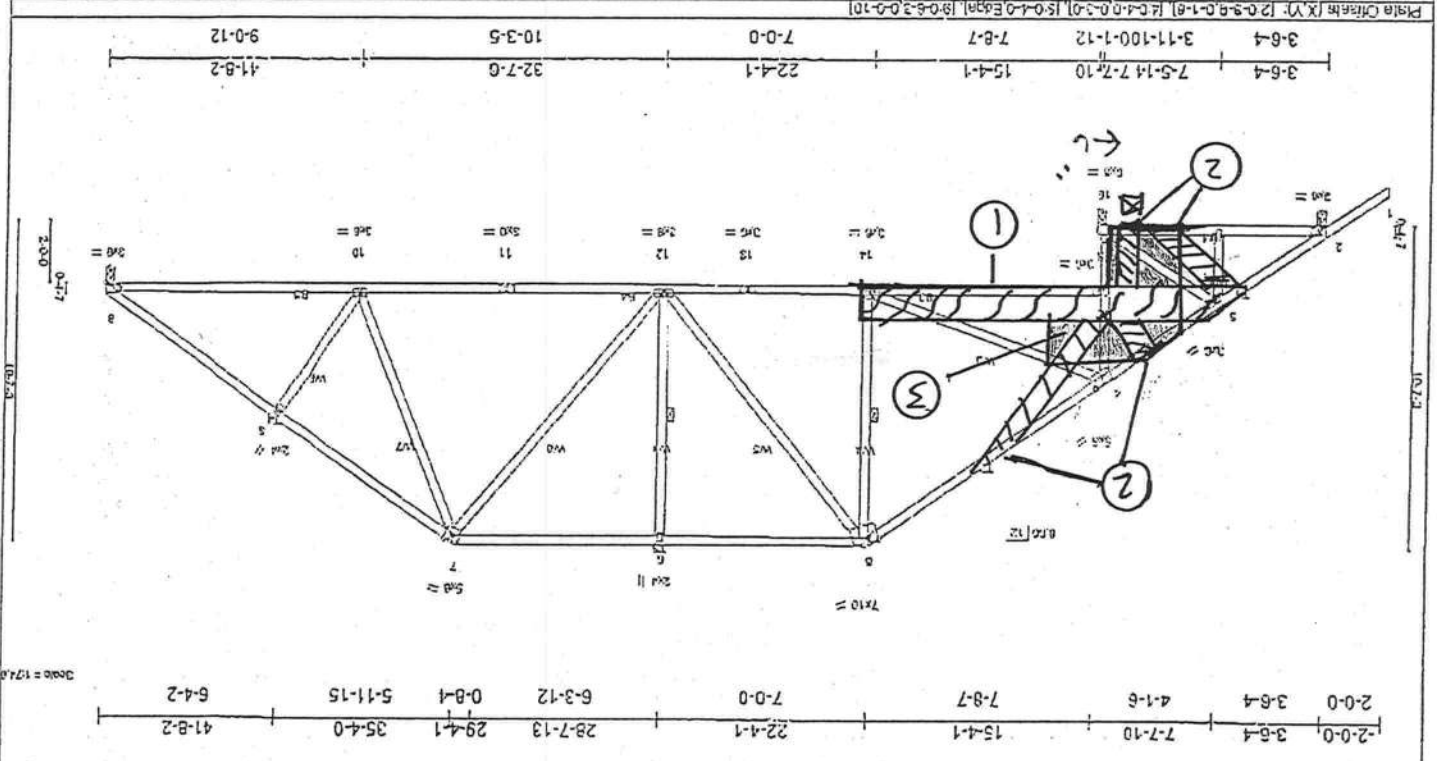
ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FLORIDA 33559
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA-7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 60" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

REACTIONS (k/ft): 8=14250-2.8, 2=2040-2.8, 18=1780-3.8
 Max Horiz=2521(load case 4)
 Max Horiz=516(load case 6), 2=220(load case 8), 16=952(load case 4)
 Max Grav=1425(load case 1), 2=267(load case 7), 18=1783(load case 1)

LUMBER
 TOP CHORD 2 X 4 SYP No. 2
 BOT CHORD 2 X 4 SYP No. 2 "Except"
 WEBS 2 X 4 SYP No. 3
BRACING
 TGP CHORD
 BCT CHORD
 WEBS
 Snailed or 3-10-14 cc putins.
 Right ceiling directly applied or 6-0-0 cc bracing.
 1 Row at midpt 5-14, 6-12

LOADING (psi)	SPACING	CSI	DEFL	Weight: 210 lb
TCLL 20.0	2-0-0	0.55	in (1cc)	PLATES GRIP
TCDL 7.0	Plates Increase 1.25	TC 0.55	in (1cc)	MTZO 244/150
BCLL 10.0	Lumber Increase 1.25	EC 0.68	Vert(LL) -0.26 10-12 > 969	
BCDL -5.0	Req Stress Incr YES	WB 0.67	Vert(UL) -0.37 10-12 > 569	
	Code FBC2001/NMSIS	(k/ft)	Horz(UL) 0.05	
			9 n/a	
			180 n/a	



Job	L117440	Trust Type	SPECIAL	Job Reference (optional)	1
TRUST	T11	City	FLY		

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ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

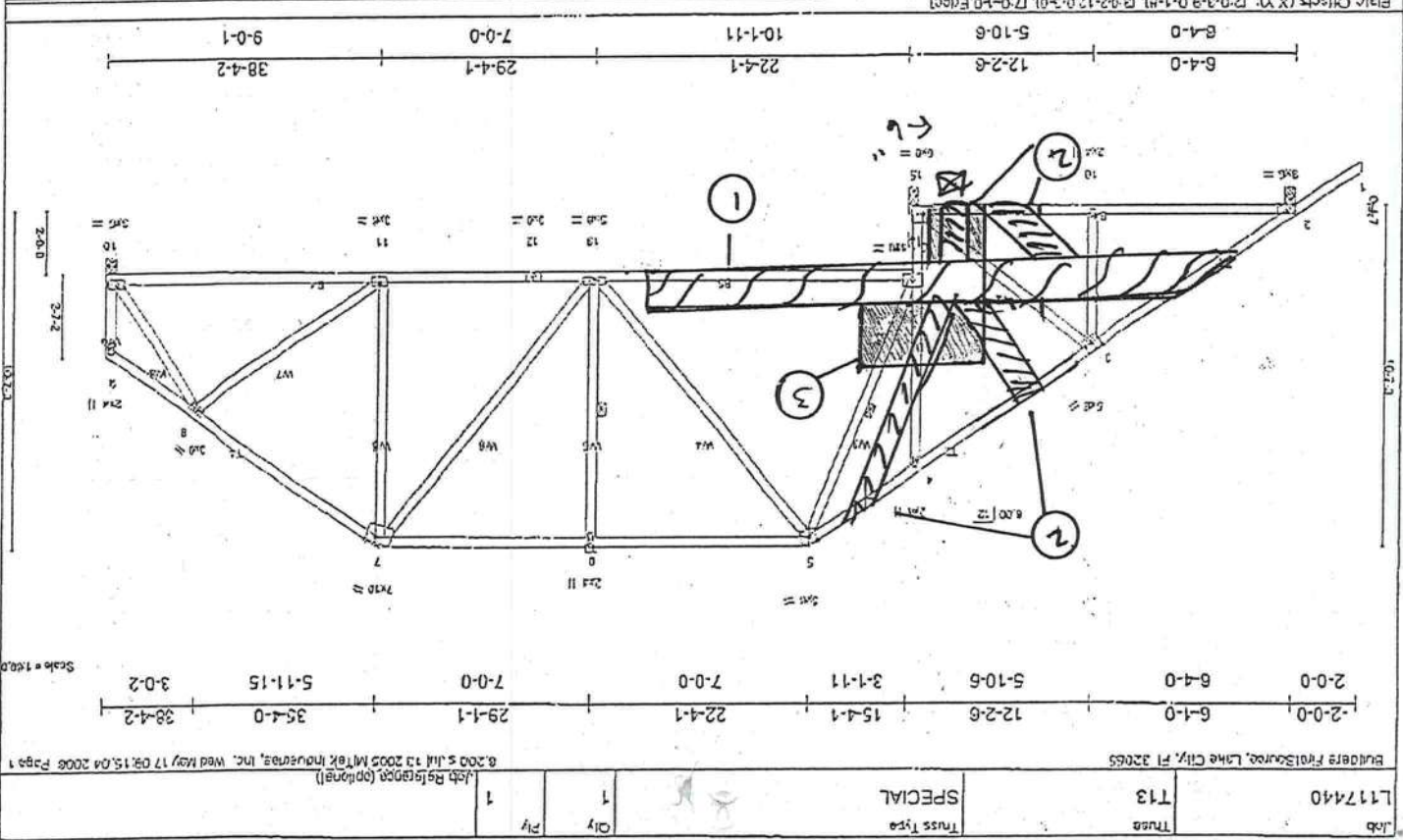
1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

REACTIONS (kips): 2x54/7-3-8, 15=1684/11-8, 10=1071/10-8
 Max Horiz=528(loaded case 5)
 Max Uplift=150(loaded case 5), 15=990(loaded case 4), 10=660(loaded case 3)
 Max Crw=2-617(loaded case 7), 15=1684(loaded case 1), 10=1071(loaded case 1)

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 WEBS 2 X 4 SYP No.3
 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 6-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-0-2 oc bracing.
 WEBS 1 Row at midspan
 6-14, 6-15

LOADING (psf)	SPACING	CS1	DEFL	in (ft)	Wdth	Ld	PLATES	CRIP
20.0	2'-0"	0.81	-0.21	13'-14"	>#59	2#0	1/4" x 3"	2x4/190
7.0	2'-0"	0.70	-0.32	13'-14"	>#55	1#0	1/4" x 3"	
10.0	2'-0"	0.72	0.02	10	N/A	N/A		
5.0	2'-0"							Weight 2x2 lb



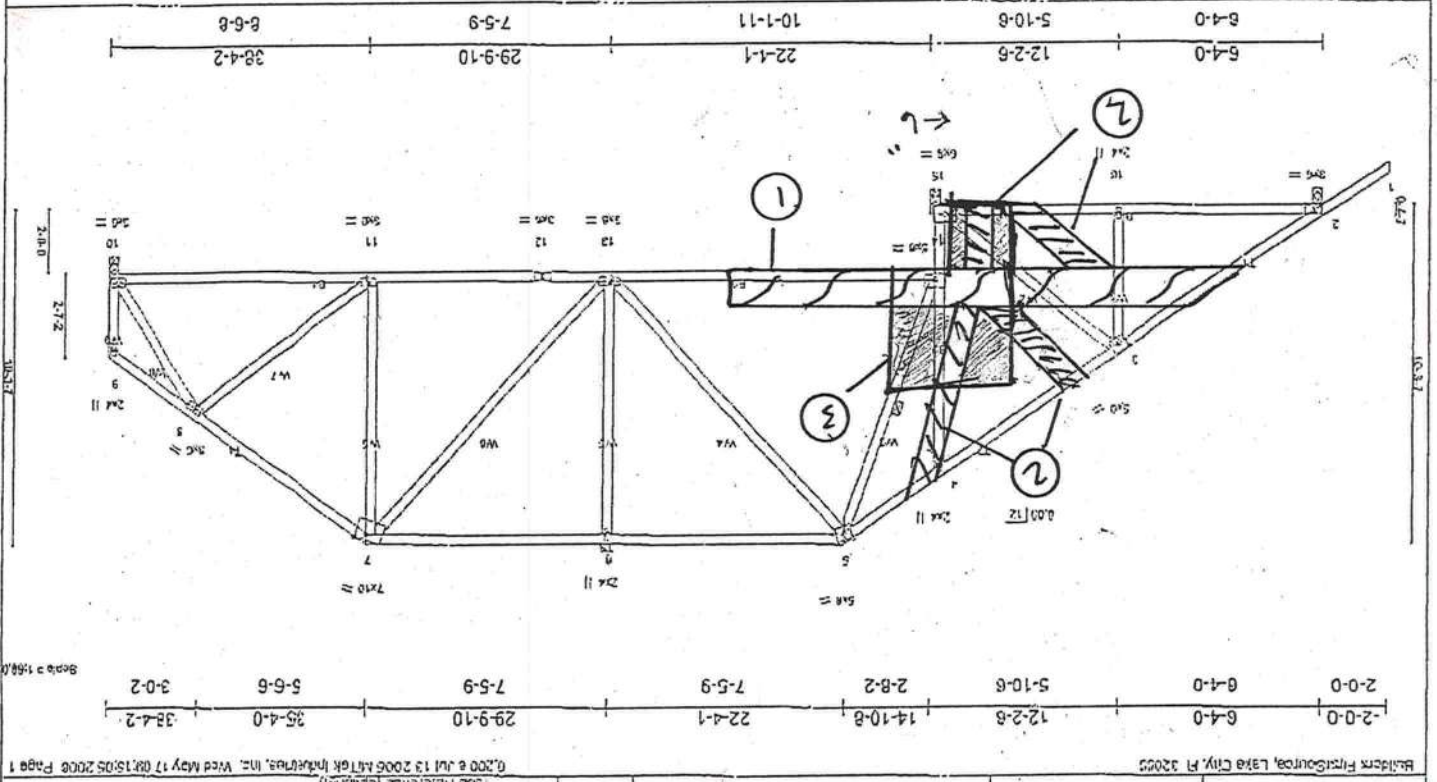
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ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

LOADING (psf)	SPACING	CS1	DEFL	VERT(L)	Horz(TL)	WTS	BRACING
TCLL 20.0	2'-0"	TC 0.59	in (100)	-0.21 13-14 >e99 240	0.02 10 N/A	1 Row at midlx	Specified or 5-11-12 cc purlins, except end verticals.
TCDL 7.0	Lumber increase 1.25	BC 0.71	-0.31 13-14 >e99 180	0.02 10 N/A	1 Row at midlx	1 Row at midlx	Fixed ceiling directly applied or 5-0-2 cc bracing.
BCLL 10.0	Rap Stress Incr YES	WB 0.88	0.02 10 N/A	0.02 10 N/A	1 Row at midlx	1 Row at midlx	5-14
BCDL 5.0	Code FECC2001/ANSI85						

REACTIONS (lb/ft): 2=516-0-2-8, 15=1595-0-3-9, 10=1071-0-3-8
 Max Horiz=45 (load case 5), 15=393 (load case 4), 10=-157 (load case 3)



Job Reference (optional)	1
Truss Type	SPECIAL
Truss	T15
Job	L177440

5/17/04
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ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FLORIDA 33559
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA 7882

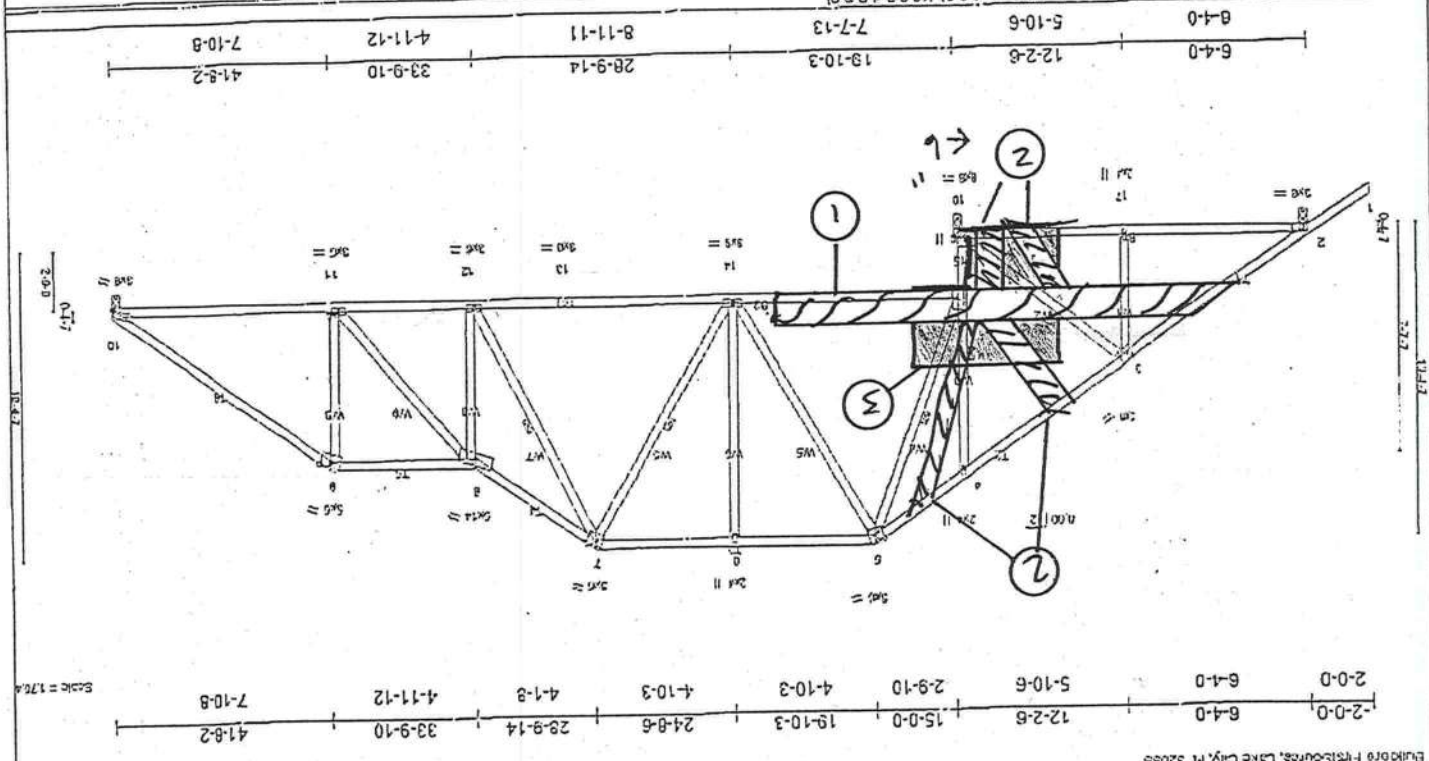
1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

JOINT STRESS INDEX
 2 = 0.61, 3 = 0.58, 4 = 0.27, 5 = 0.24, 6 = 0.27, 7 = 0.59, 8 = 0.59, 9 = 0.65, 10 = 0.62, 11 = 0.56, 12 = 0.73, 13 = 0.75, 14 = 0.58, 15 = 0.28, 16 = 0.14 and 17 = 0.27

REACTIONS (lb/ft²): 10=125(100-2.8), 2=597(0-2.8), 16=176(0-2.8)
 Max Horiz Z=442(load case 4)
 Max Uplift=556(load case 6), 2=505(load case 5), 16=571(load case 4)
 Max Grav=10=125(load case 1), 2=501(load case 7), 16=176(load case 1)
 FORCES (lb) - Maximum Compressive/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=59/59, 3-4=101/478, 4-5=101/478, 5-6=91/478, 6-7=91/478, 7-8=211/11297, 8-9=1420/70, 9-10=1808/59
 BOT CHORD 2-17=172/371, 16-17=172/371, 14-15=177/358, 13-14=386/1100, 12-13=329/1100, 11-12=716/1700, 10-11=590/1408
 WEBS 4-15=227/327
 2-17=292/214, 3-16=180/821, 5-16=1160/508, 5-14=368/1024, 6-14=276/312, 7-14=585/225, 7-12=742/1242, 8-12=1005/716, 8-11=118/227, 8-11=147/530, 15-16=1409/43

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 BRACING
 TOP CHORD Sheathed or 4-2-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-5-3 oc bracing.
 WEBS 1 Row at midpt.
 5-15, 7-14, 7-12, 4-16

LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP SEAS INT	Code F302011/ANSI/S15	CS1	TC	BC	WB	(Mark)	DEFL	In (oc)	L/D	PLATES GRIP	PLATES GRIP	Weight: 253 lb
20.0	2-0-0	1.25	1.25	YES		0.57	0.82	0.83	0.83		-0.21	12-14	210	M120	24/150	
7.0	7-0	1.25	1.25	YES		0.57	0.82	0.83	0.83		-0.21	12-14	210	M120	24/150	
10.0	10-0	1.25	1.25	YES		0.57	0.82	0.83	0.83		0.02	10	180	M120	24/150	
5.0	5-0	1.25	1.25	YES		0.57	0.82	0.83	0.83		0.02	10	180	M120	24/150	



Job	L117440	Truss Type	SPECIAL	Job Reference (optional)	1
Job	T17	Truss Type	SPECIAL	Job Reference (optional)	1

5/17/04
 [Signature]

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

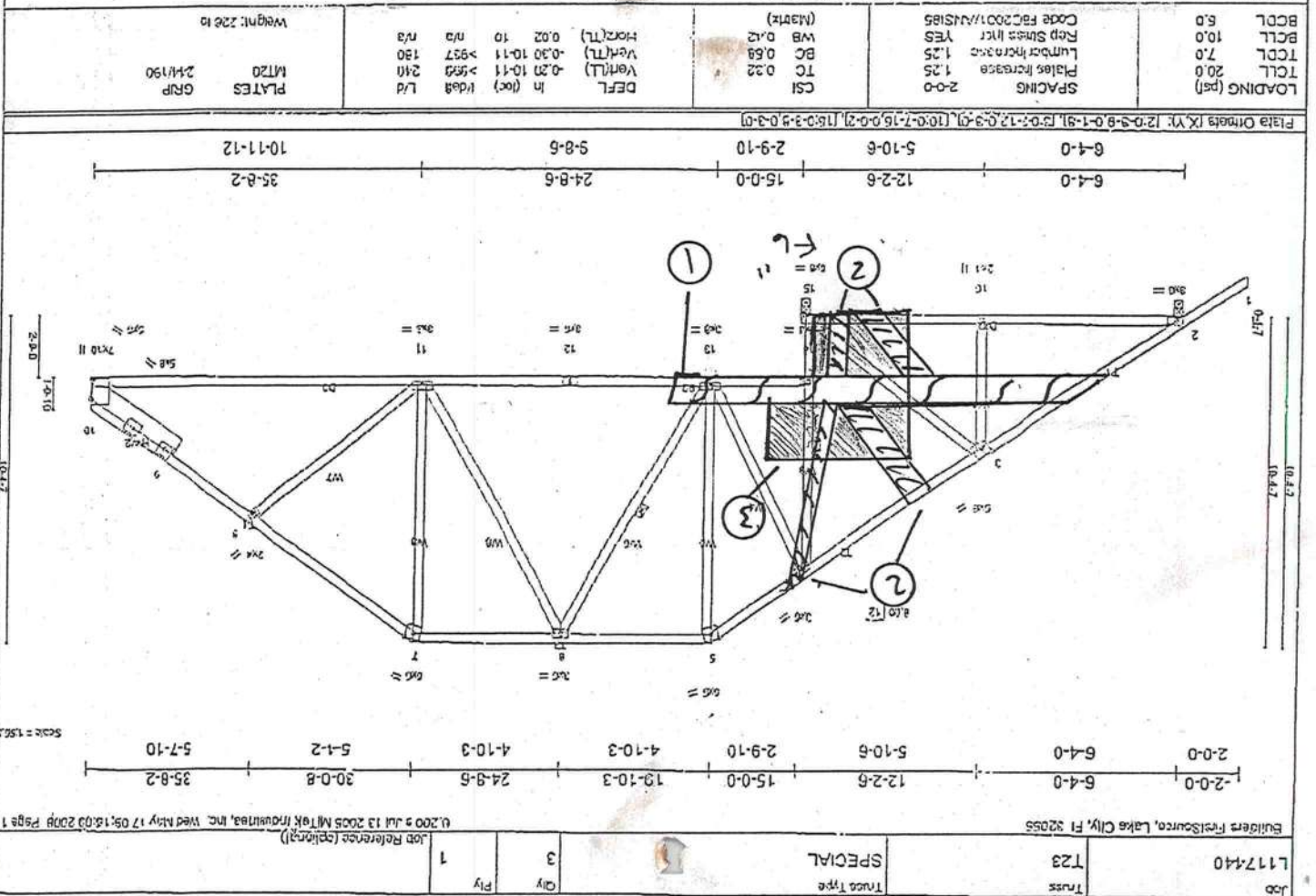
REACTIONS (k/ft/2c) 10=993/Maximum, 2=807/0-3-8, 15=1498/0-3-8
 Max Horiz 2=412(load case 4)
 Max Uplift 10=-132(load case 5), 2=509(load case 5), 15=-853(load case 4)
 Max Grav 10=993(load case 1), 2=809(load case 7), 15=1498(load case 1)

FORCES (k) - Maximum Compression/Maximum Tension
 TOP CHORD 2-16=-652/614, 2-5=-652/614, 4-4=-235/589, 4-4=-180/499, 5-0=-356/453, 6-7=-771/605, 7-8=-1004/623, 8-8=-1087/654, 9-10=-1245/683
 BOT CHORD 2-16=492/264, 15-16=-469/249, 13-14=71/5, 12-13=240/810, 11-12=240/840, 10-11=-403/325
 WEBS 2-16=309/214, 2-15=-189/836, 4-13=-287/764, 5-15=-157/117, 6-13=-574/503, 8-11=-89/269, 7-11=-89/269, 8-11=-211/221, 14-15=-1145/530, 4-14=-1142/530

LOADING (psi)	SPACING	CSI	DEFL	IN (loc)	IDEAL	L/D	PLATES	GRP
20.0	2'-0"	0.22	0.20	10-11	> 957	240	MT20	241/190
7.0	10.0	0.68	0.30	10-11	> 957	130		
5.0	6.0	0.12	0.02	10	n/a	n/a		

BRACING
 TOP CHORD Sheathed or 5-11-15 cc puling.
 BOT CHORD Rigid ceiling directly applied or B-0-0 cc bracing.
 WEBS 1 Row at midpa B-13, 4-15

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 SLIDER Right 2 X 8 SYP No.1D 3-4-7



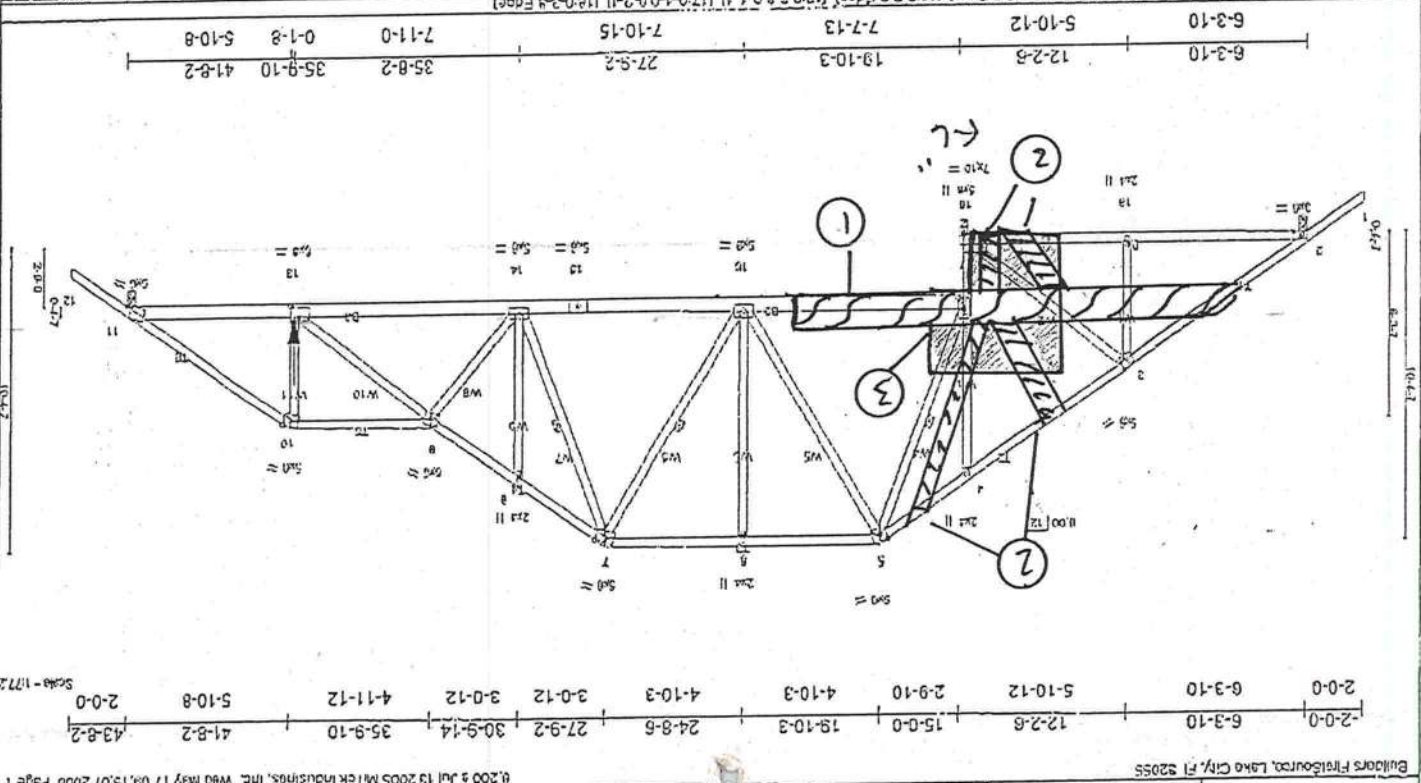
5/17/06
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ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 60" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

RECTIONS (lb/ft): 2=391/0-3-8, 10=2008/0-2-8, 11=2541/0-2-3
 Max Uplift=52c(load case 5), 18=1102(load case 5), 11=1204(load case 5)
 Max Gravity=59c(load case 5), 11=2541(load case 5)
 FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/59, 2-3=528/673, 3-4=321/192, 4-5=75/588, 5-6=1200/765, 6-7=1200/765, 7-8=278/1526, 8-9=2828/1294, 9-10=3288/1890,
 10-11=3878/1007, 11-12=0/62
 BOT CHORD 2-19=408/361, 18-19=117/305, 16-17=222/494, 15-16=560/1575, 14-15=690/1575, 13-14=1554/3692, 11-13=1449/3218
 3-19=213/215, 3-18=483/531, 5-17=1479/654, 5-16=578/1363, 6-16=273/285, 7-16=728/344, 7-14=1070/2059, 8-14c=128/212,
 9-14c=1927/1000, 9-13c=174/165, 10-13=1038/834, 17-13=1709/775, 4-17=241/229

LOADING (psf)	SPACING	PLATE INCREASE	LUMBER INCREASE	REP STRESS (lb)	NO	Code FBC2007/ANSI95	CSL	TC	BC	WB	(Max)	DEFL	h (in)	h (in)	L/D	W/dth	W/dth	W/dth	W/dth
20.0	2-0-0	1.25	1.25	1.25	NO		0.51	0.49	0.98			0.27	13-14	>989	240	160	160	160	160
TCL												Horz(TL)	0.03	11	N/A	N/A	N/A	N/A	N/A
TCOL												Vert(TL)	-0.18	13-14	>989	240	160	160	160
BCDL	5.0																		
BCL	10.0																		
PLATES	M120																		
CRIP	2x4/180																		
Weight:	284 lb																		



Job Reference (optional)	1	Job Reference (optional)	1
Trace Type	SPECIAL	Trace Type	SPECIAL
True	T18	True	T18
Job	L17440	Job	L17440

WALSH
5/17/06

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

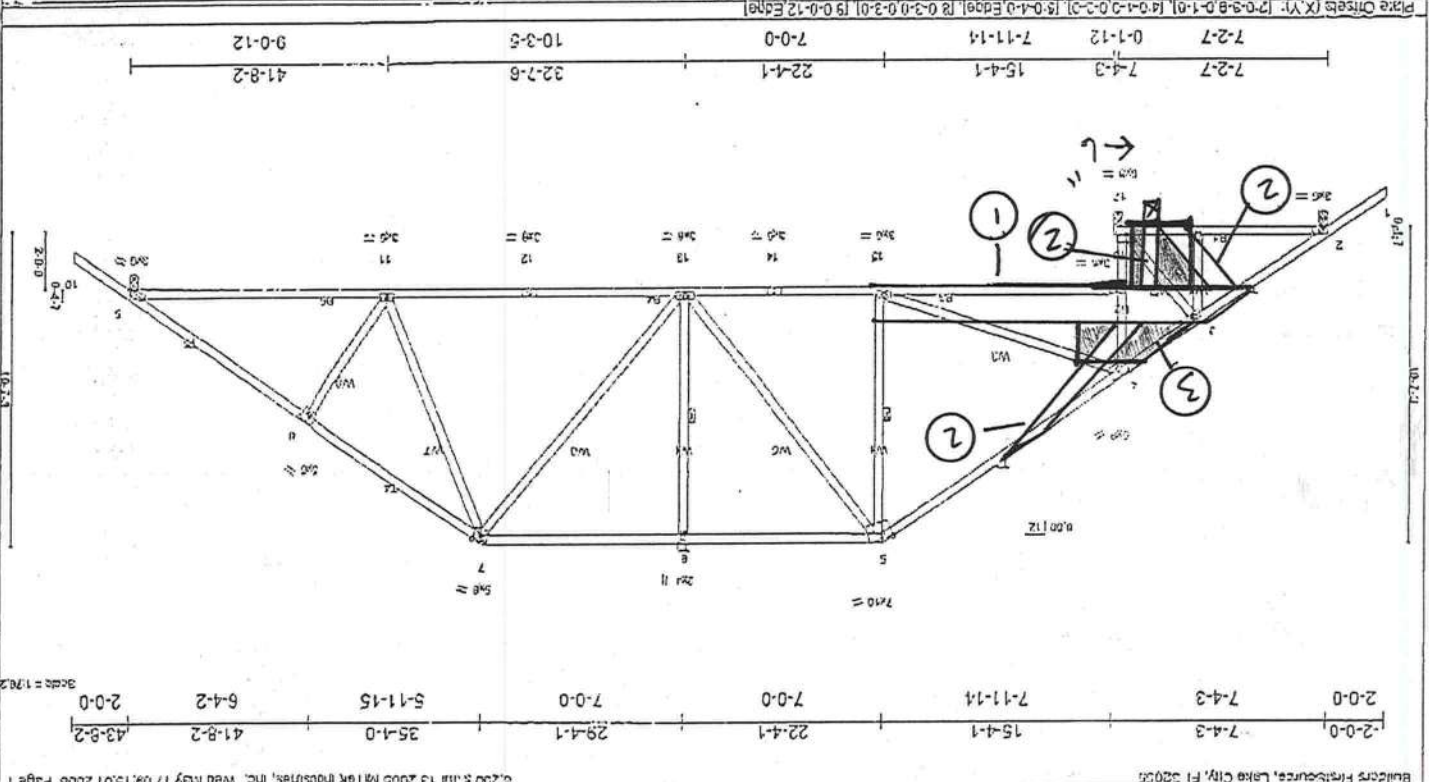
1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 6" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

REACTIONS (k/ft) 2=388/0.3-8.8, 9=1050/0.3-8, 17=1783/0.3-8
 Max Horiz 2=424(load case 4), 9=450(load case 5), 17=351(load case 6)
 Max Uplift=-499(load case 4), 9=-450(load case 5), 17=-1783(load case 6)
 Max GY/2=372(load case 7), 9=1558(load case 1), 17=1783(load case 1)

LUMBER TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "EXCEPT"
 B2 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING TOP CHORD
 Shation or 4-1-2 cc purline.
 Right ceiling directly applied or 6-0-0 cc bracing.
 1 Row at midpt
 6-13, 6-15

LOADING (psf)	20.0	TCL	7.0	TCDL	10.0	BCLL	5.0	BCDL
SPACING	2-0-0	Plates Increase	1.25	Lumber Increase	1.25	Rep Bracs Incr	YES	Code FEC2003(NAS195)
CSI	0.58	TC	0.58	BC	0.67	WB	0.66	(Mabx)
DEFL	In (loc)	Udel	Ld	Ver(L)	-0.27	11-13	>959	100
				Horz(TL)	9.05	9	N/A	N/A
PLATES	GRP	MT20	2.4/190	WT	2.4/190	WT	2.4/190	WT
WEBS	2.4/190	WT	2.4/190	WT	2.4/190	WT	2.4/190	WT



Job	L117440	Truss Type	T10	SPECIAL	2	1
Job Reference (optional)	3200 S. Rd 13 2005 Mitek Industries, Inc. Wed May 17 09:15:01 2006 Page 1					

Handwritten notes: *W. 5/17/00*

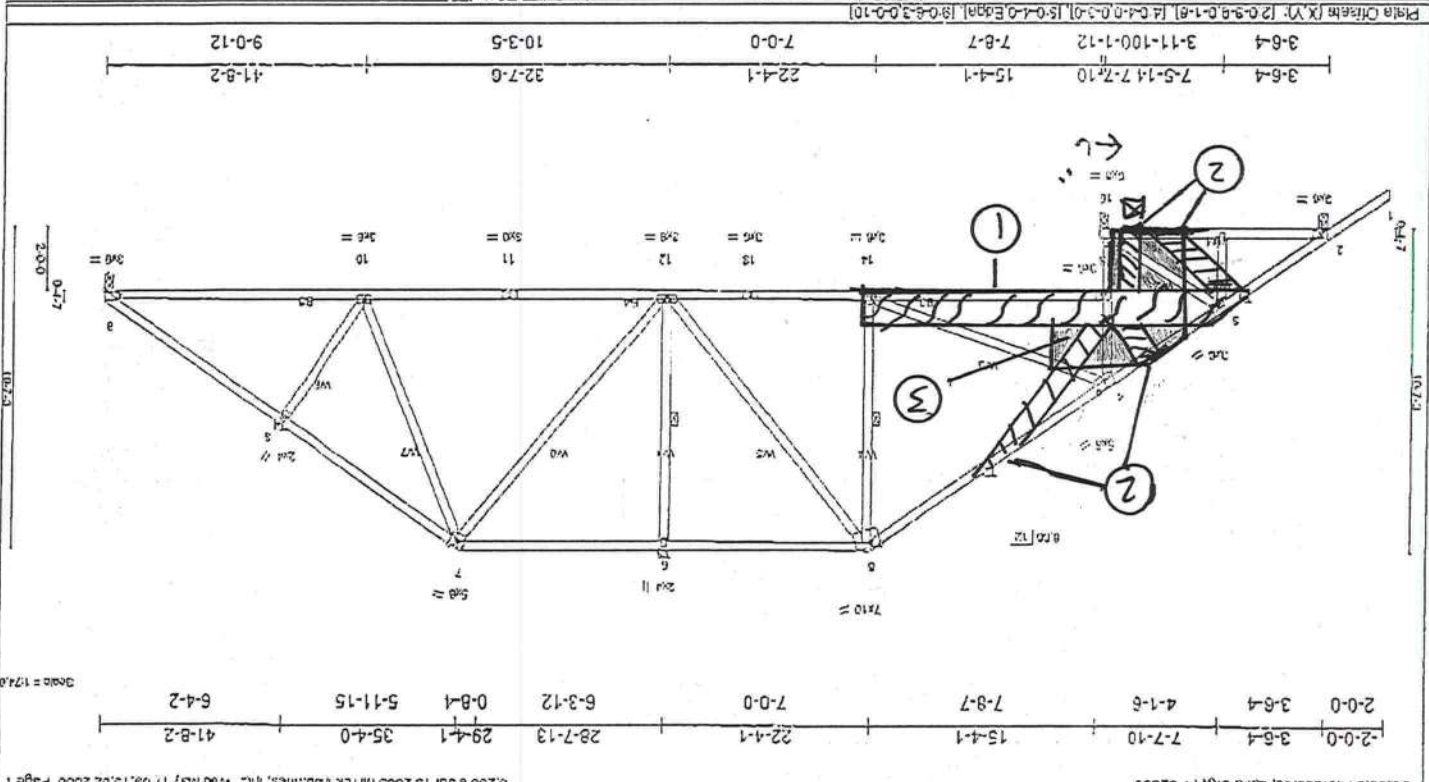
ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA-7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 60" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

REACTIONS (lb/lin) 9=1425/0-2-8, 2=204/0-2-8, 18=1783/0-2-8
 Max Horiz=521 (load case 4)
 Max Uplift=516 (load case 6), 2=320 (load case 8), 16=932 (load case 4)
 Max Grav=1425 (load case 1), 2=387 (load case 7), 16=1793 (load case 1)

LUMBER 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 Except
 E2 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3
 BRACING TOP CHORD
 Snaginned or 3-10-14 cc pulins.
 Right ceiling directly applied or 6-0 cc bracing.
 1 Row at midsp 5-14, 6-12

LOADING (psf)	SPACING	PLATES	PLATES INCREASE	LUMBER INCREASE	REP STRESS INCR	YES	Code FECC2001/ANSI/S5	CS1	TC	EC	WB	Horz(TL)	Vert(L)	DEFL	in (loc)	Wght	L/D	PLATES	GRIP	Weight 240 lb
20.0	2.0-0	2.00	1.25	1.25	1.25	YES		0.55	0.55	0.68	0.87	0.05	-0.27	10-12	9	N/A	N/A	MT20	Z44V190	



W. J. W.
5/17/02

ARCHITECTURAL SERVICES AND ENGINEERING
24710 STATE ROAD 54
LUTZ, FLORIDA 33559
ROBERT WALL, PE 46021
FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

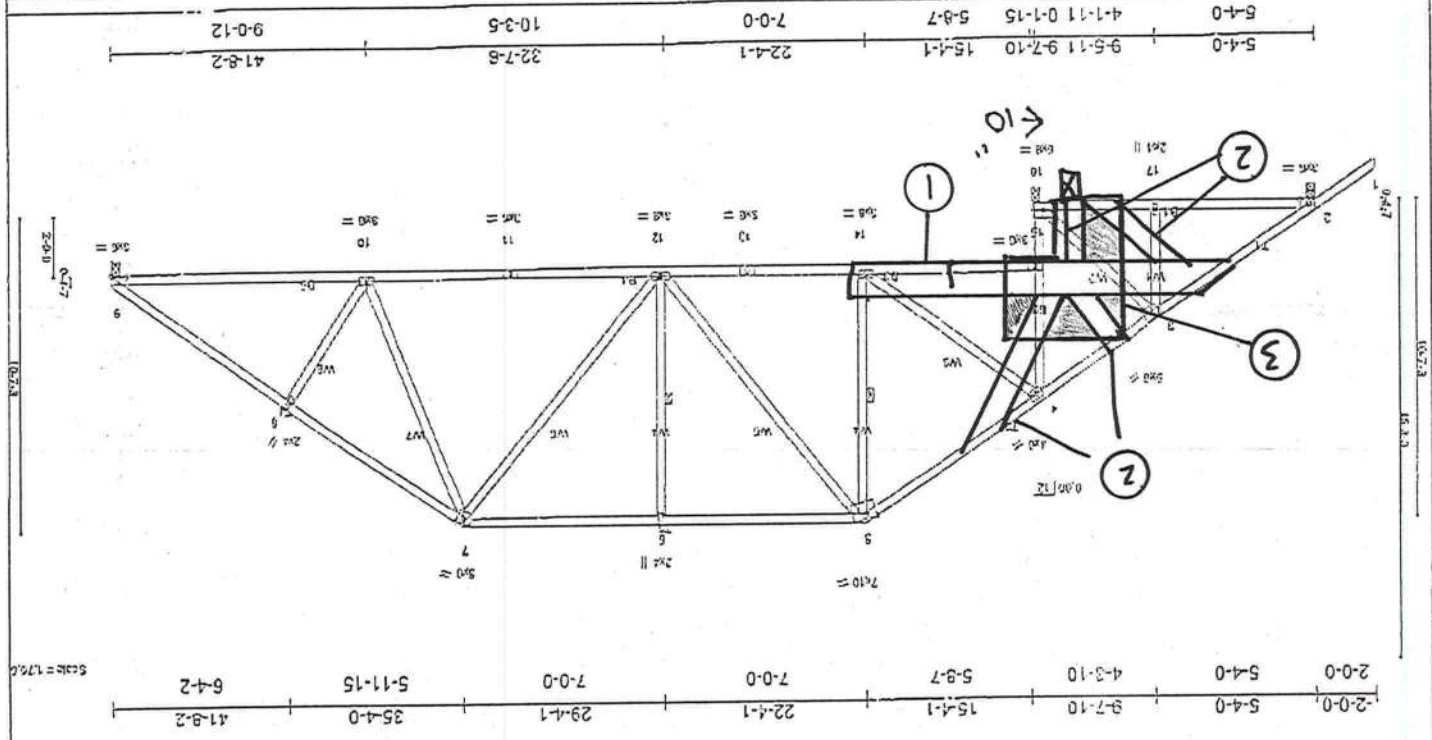
REACTIONS (lbs/ft): e=1310/0-3-8, 2=1750/0-3-8, 1a=1777/0-3-8
 Max Horiz=45 (load case 4)
 Max Util=509 (load case 6), 2=950 (load case 7), 1b=913 (load case 8)
 Max GMV=1340 (load case 1), 2=479 (load case 7), 1c=1777 (load case 8)

WEBS: 2 X 4 SYP No.3
 2 X 4 SYP No.3
TOP CHORD: 2 X 4 SYP No.2
BOT CHORD: 2 X 4 SYP No.2 - Except

BRACING: Sheathed or 4-0-10 cc putins.
TOP CHORD: Right ceiling directly applied or 6-0-0 cc bracing.
BOT CHORD: 1 Row at midspan
 WEBS: 5-14, 6-12

LOADING (psi)	SPACING	CSF	DEFL	DI (occ)	Local L/d	Local L/d
20.0	2'-0"	TC 0.39	VERTLL -0.23	10-12	> 998	240
1.0		BC 0.78				180
10.0		WB 0.88	Horz(TL) 0.04	9	N/A	N/A
5.0						

Code FEC2001/MNS195
 Rep Size Inct YES
 Lumber Increase 1.25
 Plates Increase 1.25
 SYP No. 2
 SYP No. 3
 SYP No. 2 - Except



Truss Type	Job Reference (Optional)
Truss	1
T12	1
SPECIAL	1

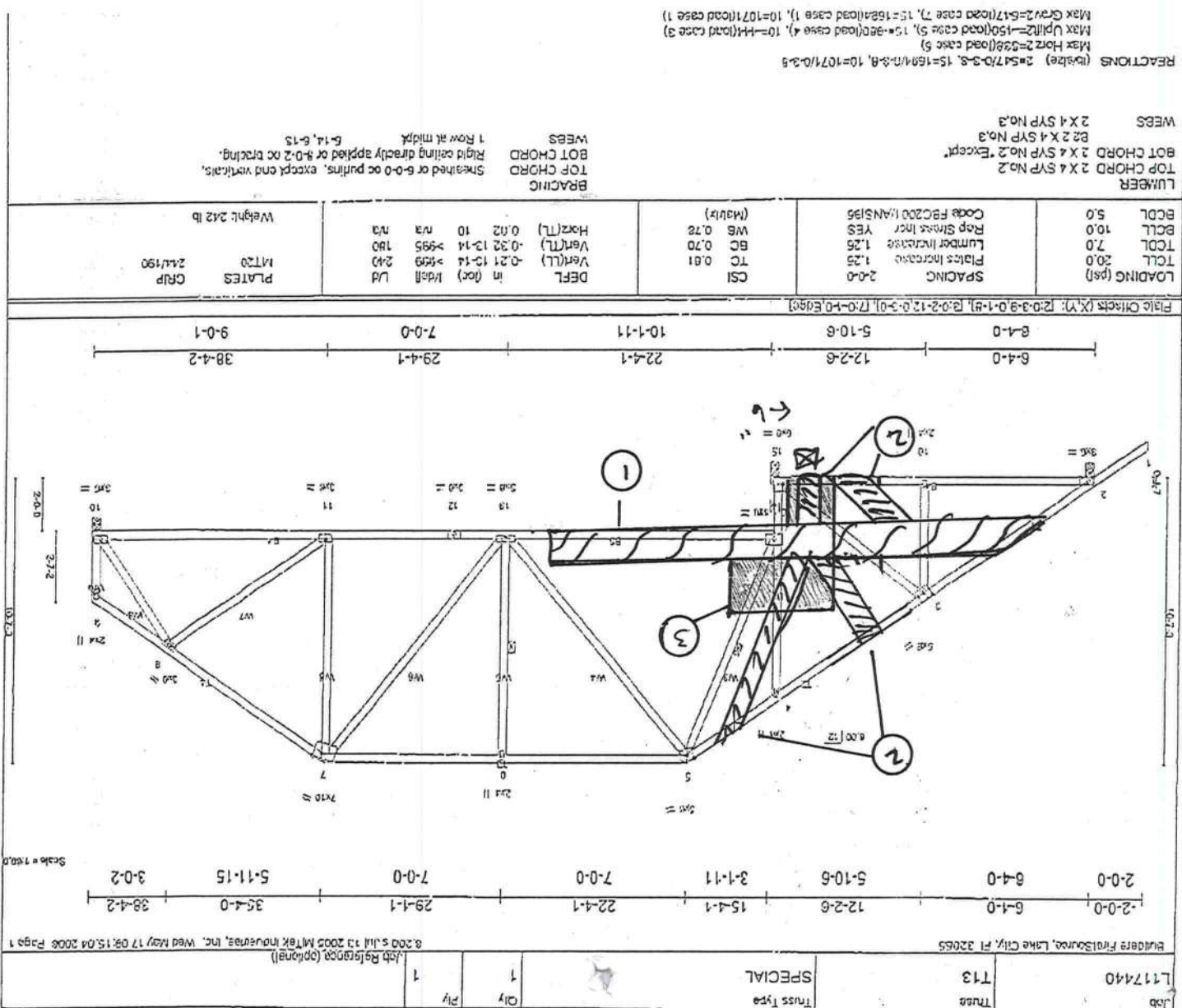
Buildings First Source, Lake City, FL 32055
 Scale = 1/8" = 1'-0"
 Job Reference: 2005 JUL 15 2005 MITTEK INDUSTRIAL, INC. WED MAY 17 08:15:03 2006 Page 1

MAY 17, 2006 9:25AM BUILDERS FIRST SOURCE

NO. 0297 P. 2

Handwritten notes: 5/17/06

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882
 1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.



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 5/17/06
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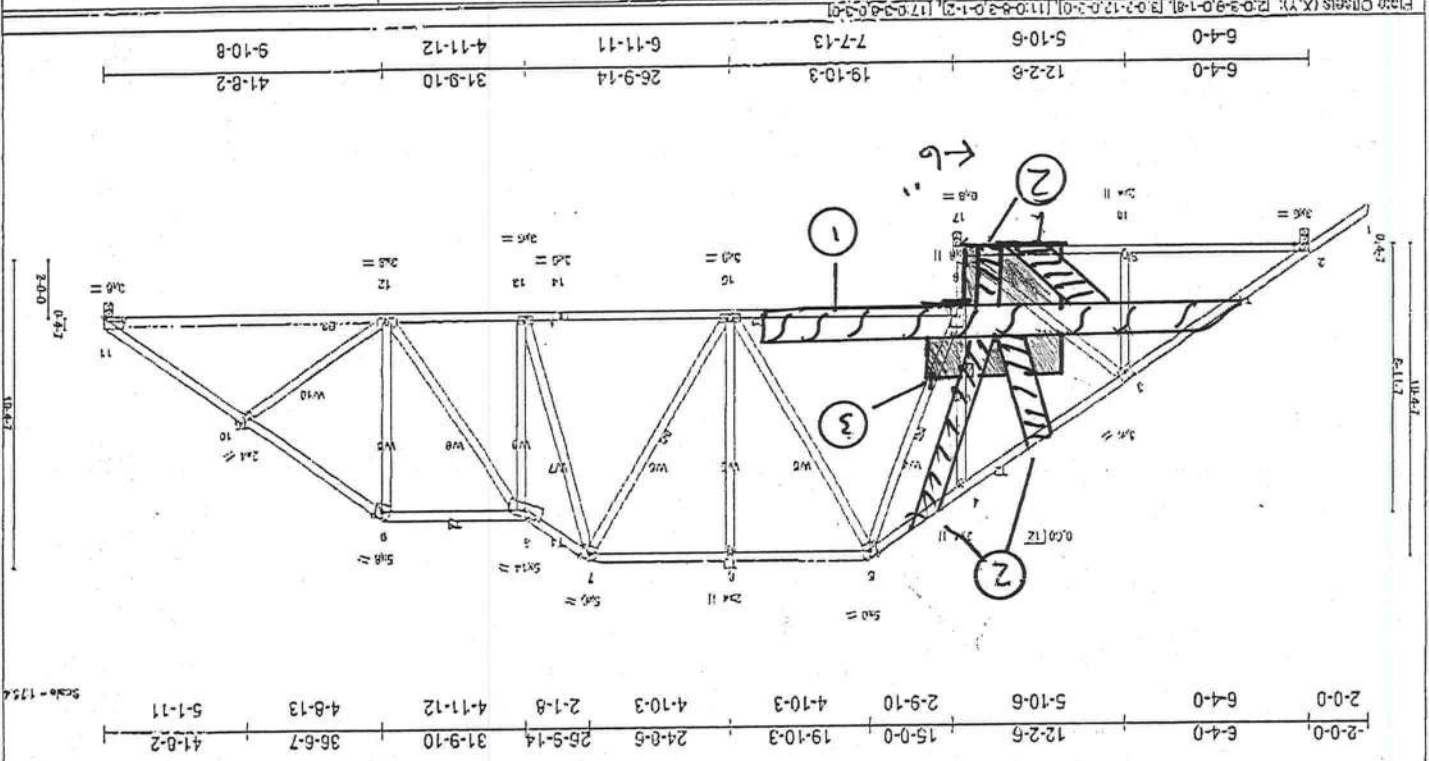
ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 72" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

REACTIONS (k/ft²) 11=123W0-3-0, 2=60C10-3-0, 17=175010-3-0
 Max Horiz 2=412(load case 4)
 Max Uplift 1=550(load case 6), 2=505(load case 5), 17=583(load case 4)
 Max Grav 11=230(load case 1), 2=603(load case 7), 17=1750(load case 1)

BRACING
 TOP CHORD Sheathed or 4-2-1 cc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-5-15 cc bracing.
 WEBS 1 Row at midpt
 5-16, 7-15, 4-17

LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP SPACES INCR	YES	Code FBD2000 (ANSI) 135	CSI	DEFL	In (Loc)	VERT (LL)	Horz (TL)	WEIGHT 200 LB
20.0	2-0-0	1.25	1.25				TC	0.36		-0.27	11-12	2x10
7.0							BC	0.65		-0.39	11-12	2x10
10.0							WB	0.75		0.02	11	n/a
5.0												n/a



Job Reference (Optional)	1	Job Reference (Optional)	1
Truss Type	SPECIAL	Truss Type	Py
Truss	T16	Truss	
Job	L117440	Job	

Handwritten signature and date:
 5/17/04

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

1. 2X8 #2 SP SCAB W / (13) 10D'S AT EACH MEMBER.
 2. 2X6 #2 SP SCABS W / (9) 10D'S AT EACH MEMBER.
 3. 1/2" X 48" X 7/2" PLYWOOD OR OSB W / (20) 10D'S AT EACH MEMBER.

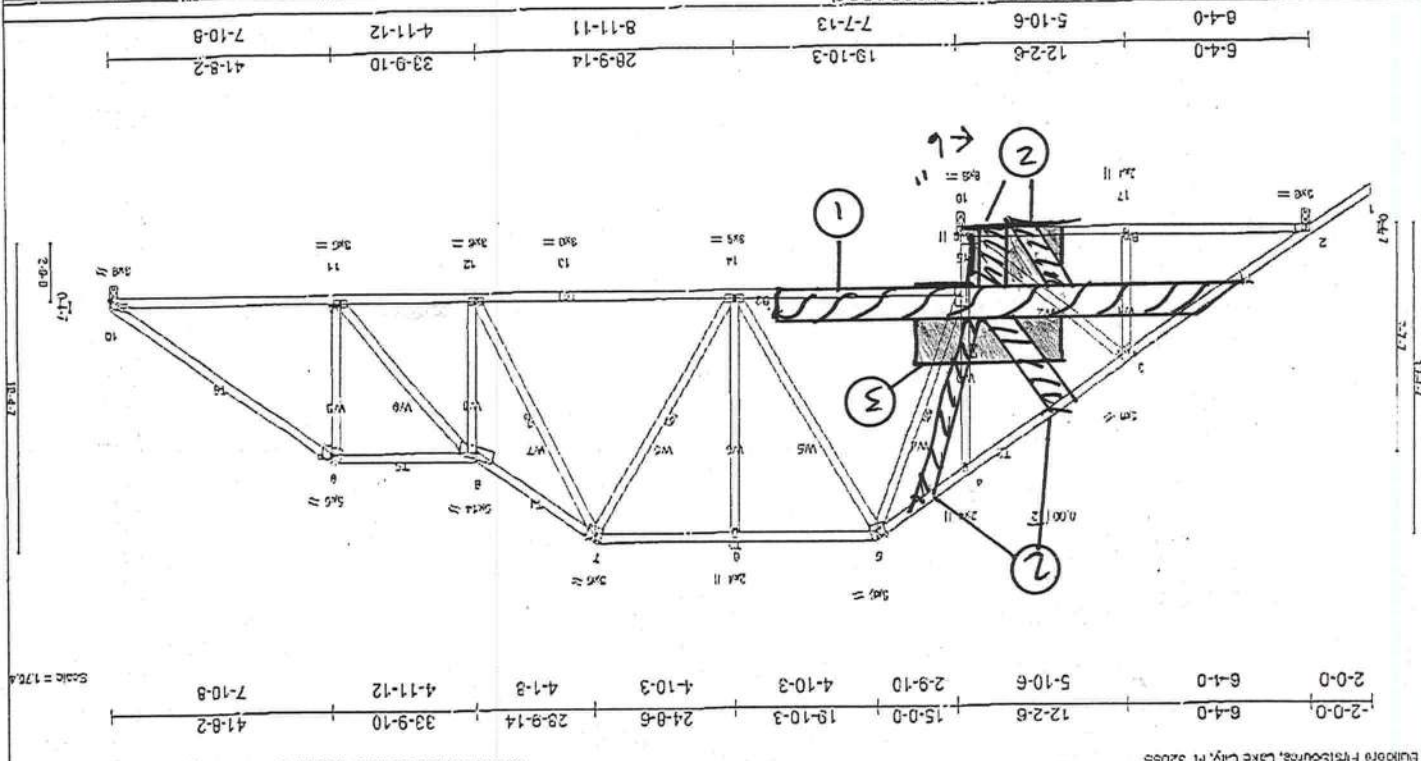
JOINT STRESSES INDEX
 2 = 0.51, 3 = 0.59, 4 = 0.27, 5 = 0.24, 6 = 0.27, 7 = 0.59, 8 = 0.55, 9 = 0.05, 10 = 0.62, 11 = 0.26, 12 = 0.73, 13 = 0.75, 14 = 0.59, 15 = 0.28, 16 = 0.14 and 17 = 0.27

REACTIONS (lb/ft²): 10=1295M-3-4, 2=5970-3-9, 10=1760/0-3-9
 Max Horiz 2=442 (load case 4)
 Max Uplift 10=556 (load case 6), 2=505 (load case 5), 16=97 (load case 4)
 Max Grav 10=1259 (load case 1), 2=601 (load case 7), 16=1760 (load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-17=1723/71, 16-17=1723/71, 16-17=1723/71, 14-15=1723/71, 13-14=368/1100, 12-13=309/1100, 11-12=716/1700, 10-11=590/1408
 BOT CHORD 2-17=2922/14, 3-16=4818/21, 5-16=1160/508, 5-14=368/1024, 6-14=276/312, 7-14=266/225, 7-12=743/1242, 8-12=1005/716, 8-11=418/227, 8-11=147/596, 15-16=1409/643, 4-15=227/227

WEBS
 2-17=2922/14, 3-16=4818/21, 5-16=1160/508, 5-14=368/1024, 6-14=276/312, 7-14=266/225, 7-12=743/1242, 8-12=1005/716, 8-11=418/227, 8-11=147/596, 15-16=1409/643, 4-15=227/227

LOADING (psf)	SPACING	PLATES INCREASE	LUMBER INCREASE	REP STRESS INCR	Code FAC2011/ANSI55	CSI	DEFL	IN (OCC)	LD	VERT (LL)	VERT (TL)	HORIZ (TL)	WEBS	TOP CHORD	BRACING	PLATES	GRIP	Weight 259 lb	
20.0	2-0-0	1.25	1.25	YES		0.57	-0.21	12-14	240	>988	12-14	0.02	10	N/A	N/A	N/A	N/A	N/A	244/130



Job: L117440
 Truss: T17
 Truss Type: SPECIAL
 Qty: 1
 Piv: 1
 Lab Reference (optional):
 2005 JUL 15 2005 METEK INDUSTRIES, INC. WVD MAY 17 08:15:07 2008 Page 1
 Builders FirstSource, Lake City, FL 32055

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5/17/04

ARCHITECTURAL SERVICES AND ENGINEERING
 24710 STATE ROAD 54
 LUTZ, FLORIDA 33559
 ROBERT WALL, PE 46021
 FLORIDA LICENSE NUMBER CA 7882

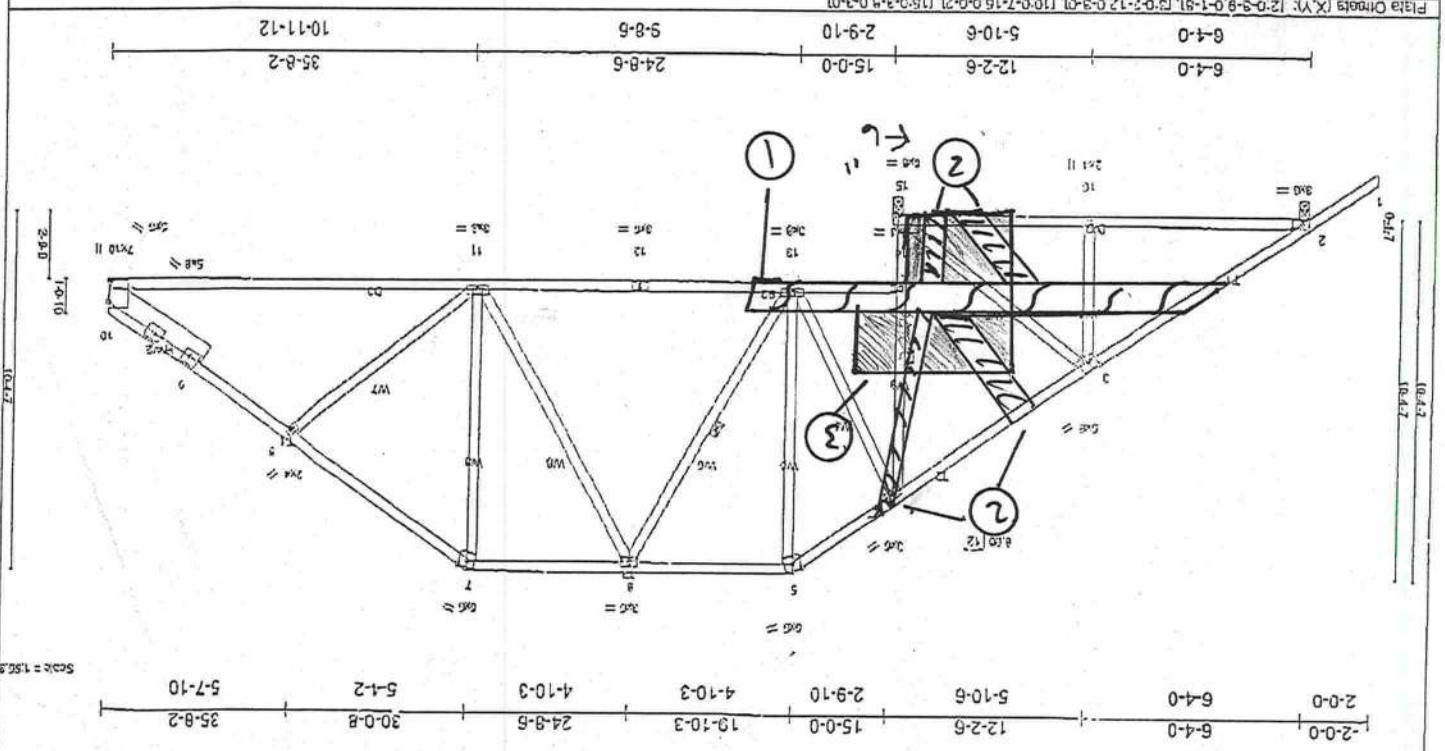
1. 2X8 #2 SP SCAB W/ (13) 10D'S AT EACH MEMBER.
2. 2X6 #2 SP SCABS W/ (9) 10D'S AT EACH MEMBER.
3. 1/2" X 48" X 72" PLYWOOD OR OSB W/ (20) 10D'S AT EACH MEMBER.

REACTIONS (k/ftic) 10=993/Mechanical, 2=009/0-3-8, 15=1498/0-3-8
 Max Uplift=110=-122(load case 6), 2=-502(load case 5), 15=-853(load case 4)
 Max Grav=10=993(load case 1), 2=309(load case 7), 15=1498(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/60, 2-6=552/614, 3-4=225/329, 4-8=180/499, 5-0=366/453, 6-7=774/805, 7-8=1004/823, 8-9=1087/854, 9-10=1215/863
 BOT CHORD 2-16=492/264, 15-16=189/289, 12-14=715, 12-13=240/810, 11-12=240/640, 10-11=403/925
 WEBS 3-16=303/214, 3-15=189/289, 4-13=287/781, 5-13=157/117, 6-13=571/203, 8-11=89/289, 7-11=89/289, 8-11=211/221, 14-15=1145/530, 4-14=1142/530

BRACING
 TOP CHORD Shearred or S-11-15 cc purlins.
 BOT CHORD Rigid ceiling directly applied or B-0-0 cc bracing.
 WEBS 1 Row at Midpk E-13, 4-15

LOADING (psf)	SPACING	CSI	DEFL	Vert(L)	Horz(RL)	CS	BC	TC	Plates Increase	Lumber Increase	Rep Stress Incr	Code FAC2001/MS1B5
20.0	2'-0"	2-0-0	in (loc)	10-11 > 553	0.02	10	0.68	1.25	1.25	1.25	YES	
7.0	2'-0"	2-0-0	in (loc)	10-11 > 937	0.02	10	0.68	1.25	1.25	1.25	YES	
10.0	10'-0"	10-0-0	n/a	n/a	n/a	n/a	0.42					
5.0	5'-0"	5-0-0	n/a	n/a	n/a	n/a	0.42					



Job Reference (optional)	1
Qty	3
Truss Type	SPECIAL
Truss	T23
Job	L117440
Builder's First Source, Lake City, FL 32055	

GTC Design Group, LLC
P.O. Box 187
Live Oak, FL 32064
(Phone) 386.362.3678
(Fax) 386.362.6133
ggill@gctcdesigngroup.com

GTC DESIGN GROUP



August 2, 2006

Joe Haltiwanger
Columbia County Building and Zoning Department
135 NE Hernando Ave, Suite B-21
Lake City, FL

SUBJECT: Tom Eagle

Emerald Lake - Arbor Green Lot 14
Permit # 000023992

Joe,

Per Tom Eagle's request, I inspected the truss system for the abovementioned project. The trusses were installed according to the manufacturer's truss layout. The only concern I have is that one of the short trusses bears on an interior wall. Mr. Eagle informed me that the concrete slab was not thickened under the wall. The truss bears on the double top plate between the 2x studs. I recommended adding 3 or 4 additional studs directly under the truss. This will help in two ways. First, the top plate is not longer in bending. The vertical load from the truss will transfer directly to the studs. Second, the point load of the truss will be distributed to a greater area over the slab and the bottom plate compression loading will decrease. The additional studs will eliminate the need to modify the slab.

If you have any questions or require additional information, please contact me at your convenience.



NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property:
Lot 14 Arbor Greene @ Emerald Lakes Subdivision

2. General description of improvement: Construction of Dwelling

3. Owner information:

a. Name and address: Thomas H. Eagle
116 NW Egret Lane
Lake City, FL 32055

b. Interest in property: Fee Simple

c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: David Mangrum

5. Surety n/a

a. Name and address:

Inst: 2006000117 Date: 01/04/2006 Time: 13:43
DC, P. Dewitt Cason, Columbia County B: 1070 P: 413

b. Amount of bond:

6. Lender: N/A

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Signature of Owner

The foregoing instrument was acknowledged before me this _____ day of _____

_____ 2006
by Thomas H. Eagle,
who are personally

take an oath.

Susan H. Holt

Notary Public
My commission

expires: _____
Susan L. Holt
Commission # DD431203
Expires: MAY 19, 2009
www.AARONNOTARY.com



CERTIFICATES OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-3S-16-02372-514

Building permit No. 000023992

Use Classification SFD, UTILITY

Fire: 16.74

Permit Holder DAVID MANGRUM

Waste: 50.25

Owner of Building THOMAS EAGLE

Total: 66.99

Location: 219 NW HERITAGE DR, LAKE CITY, FL

Date: 07/02/2007

John Kerce

Building Inspector



**POST IN A CONSPICUOUS PLACE
(Business Places Only)**

Project Manager
Gary Gill, P.E. #51942

8/2/06
Thank you

If you have any questions or require additional information, please contact me at your convenience.

The truss bears on the double top plate between the 2x studs. I recommended adding 3 or 4 additional studs directly under the truss. This will help in two ways. First, the top plate is not longer in bending. The vertical load from the truss will transfer directly to the studs. Second, the point load of the truss will be distributed to a greater area over the slab and the bottom plate compression loading will decrease. The additional studs will eliminate the need to modify the slab.

Per Tom Eagle's request, I inspected the truss system for the abovementioned project. The trusses were installed according to the manufacturer's truss layout. The only concern I have is that one of the short trusses bears on an interior wall. Mr. Eagle informed me that the concrete slab was not thickened under the wall.

Joe,

SUBJECT: Tom Eagle
Emerald Lake - Arbor Green Lot 14
Permit # 000023992

Joe Haltwanger
Columbia County Building and Zoning Department
135 NE Hernando Ave, Suite B-21
Lake City, FL

August 2, 2006

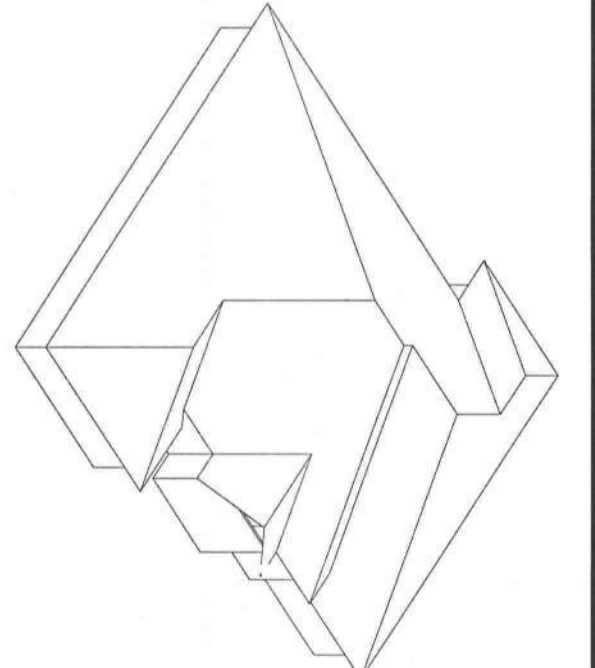
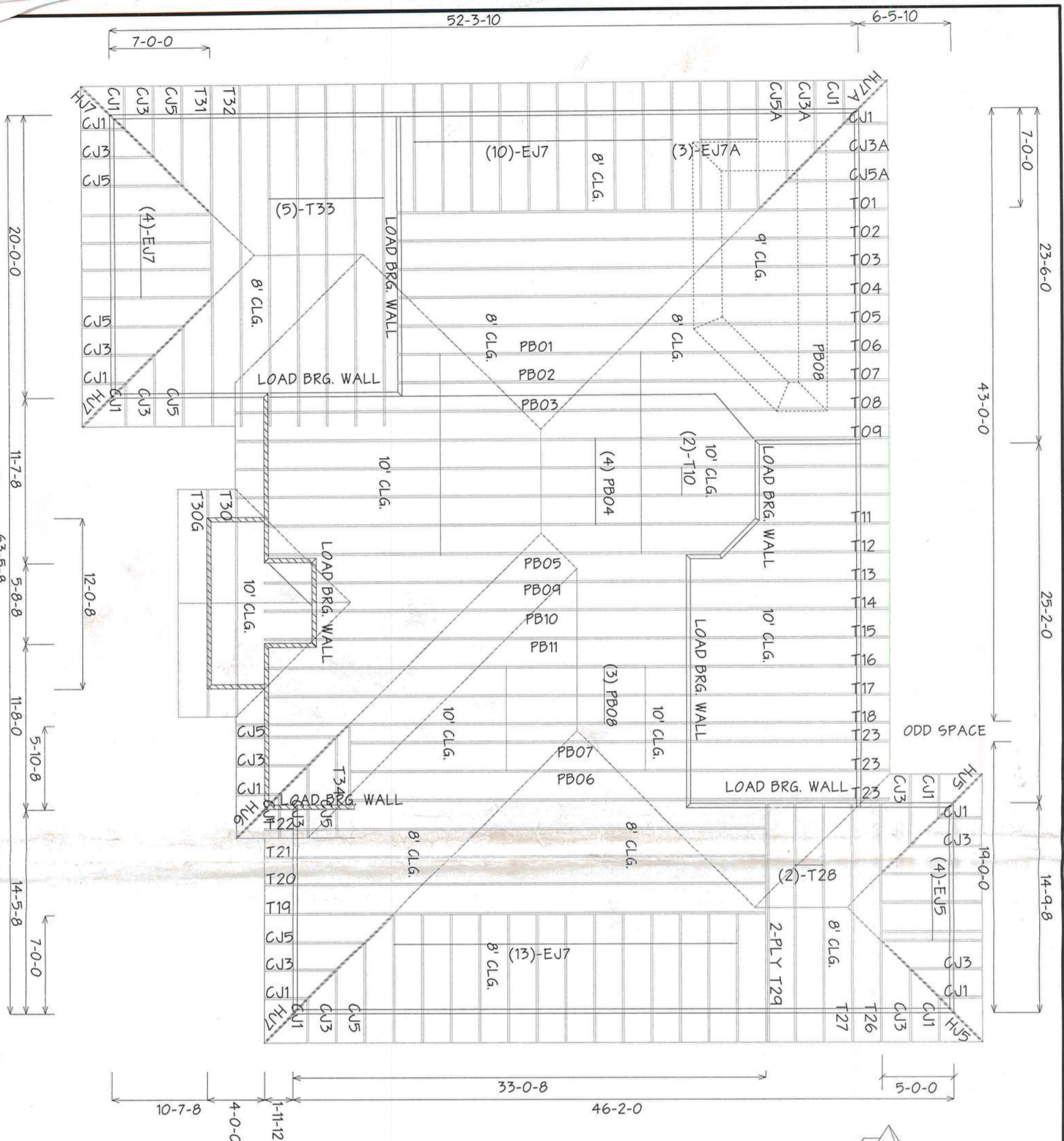
GTC DESIGN GROUP



Dave Mangrum

GTC Design Group, LLC
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BEARING HEIGHT SCHEDULE

	10'-0"
	8'-0"

ROOF PITCH 8/12
 OVERHANG EAVE 1'-6"
 GABLE 1'-0"
 HANGER SCHEDULE (8) HUS26
 (148) P.B. PLATES

NOTES:

- REFER TO MB 91 (RECOMMENDATIONS FOR TRUSS INSTALLATION AND TEMPORARY BRACING REQUIRED).
- ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- ALL TRUSSES ARE DESIGNED FOR 2 O.C. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 5/4x2 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
- DEWATER/DRY-INTEL (DR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION TRUSSES AND VOBS. ALL REVISIONS ARCHITECTURAL OR TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFICATION TO INSURE AGAINST CHANGES THAT WILL BE IN EXTRA CHARGES TO YOU.

Requested Return Date: _____

Approved By: _____ Date: _____



Builder FirstSource
 Bunnell
 Jacksonville
 PHONE: 904-437-5349 FAX: 904-437-7722
 PHONE: 904-772-6100 FAX: 904-772-6100
 Lake City
 PHONE: 904-725-6894 FAX: 904-725-6894
 Sanford
 PHONE: 407-322-0094 FAX: 407-322-0094

BUILDER: LIPSCOMB EAGLE
13500 COLUMBIA CO.
ARBOR GREEN
 SCALE: N
 DATE: 6/20/05
 DRAWN BY: ROBERT L. 1174
 CHECKED BY: DAN OTT