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ALPINE AN ITW COMPANY

COA #0 278

Florida Certificate of Product Approval #FL 1999

07/07/2023

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 20-4958B
Job Description: Jackson Residence	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.01 through 22.02.00 *JRef #:* 1XR52150012
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 17 truss drawing(s) and 1 detail(s).

Item	Drawing Number	Truss
1	188.23.0800.45080	A01
3	188.23.0800.49597	A03
5	188.23.0801.46683	A05
7	188.23.0801.49753	A07
9	188.23.0801.56087	A09
11	188.23.0801.59723	A11
13	187.23.1201.02985	J01
15	187.23.1201.03095	J02
17	187.23.1201.03000	J04

Item	Drawing Number	Truss
2	188.23.0800.47390	A02
4	188.23.0800.51620	A04
6	188.23.0801.48230	A06
8	188.23.0801.54487	A08
10	188.23.0801.58170	A10
12	188.23.0802.02880	A12
14	187.23.1201.02986	J01HJ
16	187.23.1201.03094	J03
18	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

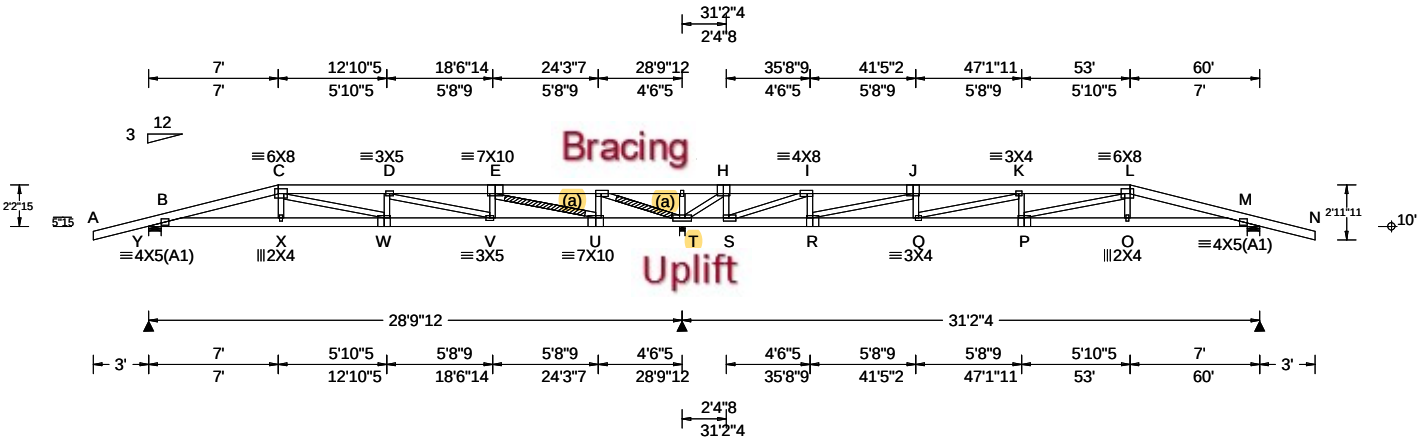
Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 464580 HIPS FROM:	Ply: 2 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A01	Cust: R 215 JRef:1XR52150012 T10 DrwNo: 188.23.0800.45080 SSB / DF 07/07/2023		

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 10.99 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.40 ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.174	K	999	240
VERT(CL): 0.360	K	999	180
HORZ(LL): 0.038	M	-	-
HORZ(TL): 0.078	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.381
Max BC CSI: 0.241
Max Web CSI: 0.993

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y	1745	/-	/-	/-	/331	/-
T	5762	/-	/-	/-	/904	/-
M	2006	/-	/-	/-	/381	/-

Wind reactions based on MWFRS
Y Brg Wid = 8.0 Min Req = 1.5 (Truss)
T Brg Wid = 3.5 Min Req = 2.0 (Truss)
M Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings Y, T, & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	369	- 2413	G - H	4085	- 642
C - D	391	- 2558	H - I	2390	- 376
D - E	206	- 1375	J - K	398	- 2550
E - F	1196	- 193	K - L	534	- 3392
F - G	4085	- 642	L - M	464	- 2919

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - X	2320	- 350	T - S	401	- 2577
X - W	2346	- 350	R - Q	2473	- 396
W - V	2536	- 398	Q - P	3387	- 543
V - U	1280	- 201	P - O	2838	- 442
U - T	208	- 1352	O - M	2811	- 442

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - V	202	- 1222	S - I	453	- 2822
V - E	591	- 33	I - R	893	- 86
E - U	415	- 2607	R - J	359	- 2229
U - F	964	- 95	J - Q	477	- 16
F - T	468	- 2951	Q - K	152	- 882
T - H	309	- 1932	P - L	581	- 96
H - S	1162	- 152			

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 7X8 except as noted.

Loading

#1 hip supports 7-0-0 jacks W/2 panel TC and no end vert.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 3-0-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 3-0-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

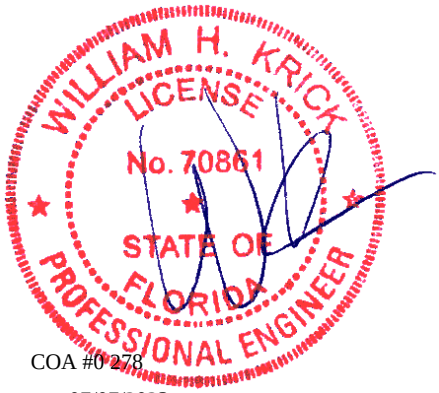
Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 2-2-15.



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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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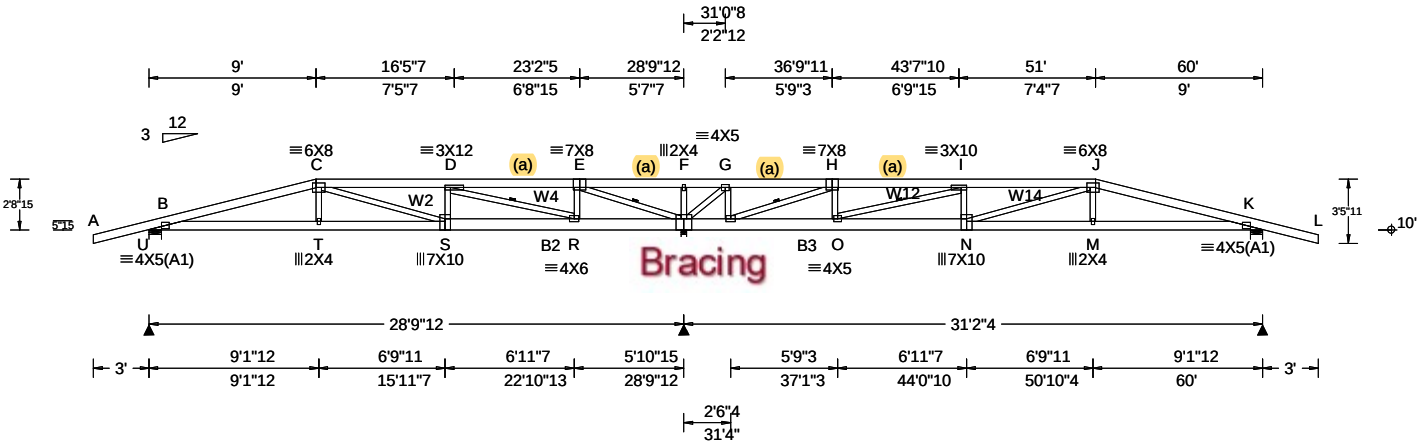
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464583 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A02	Cust: R 215 JRef:1XR52150012 T1 DrwNo: 188.23.0800.47390 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 8.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in		loc	L/defl	L/#	
VERT(LL):	0.132	N	999	240	
VERT(CL):	0.287	N	999	180	
HORZ(LL):	0.035	K	-	-	
HORZ(TL):	0.069	K	-	-	

Creep Factor: 2.0
Max TC CSI: 0.274
Max BC CSI: 0.199
Max Web CSI: 0.712

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
U	1188	/-	/-	/644	/229	/60
Q	2881	/-	/-	/1476	/546	/-
K	1280	/-	/-	/705	/250	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.4
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	429	- 2531	G - H	1604	- 262
C - D	524	- 2325	H - I	415	- 1491
D - E	830	- 538	I - J	602	- 2967
E - F	2944	- 515	J - K	514	- 2895
F - G	2944	- 515			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	2408	- 340	P - O	1414	- 331
T - S	2419	- 336	O - N	2959	- 533
S - R	2305	- 480	N - M	2772	- 418
R - Q	453	- 893	M - K	2761	- 423
Q - P	382	- 1737			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	110	- 427	G - P	1019	- 145
D - R	414	- 2095	P - H	518	- 2699
R - E	705	- 44	H - O	644	- 32
E - Q	546	- 2852	O - I	340	- 1704
Q - G	330	- 1774			

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E; B2,
B3 2x8 SP 2400f-2.0E;
Webs: 2x4 SP #3; W2,W14 2x4 SP M-31; W4,
W12 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 2-8-15.

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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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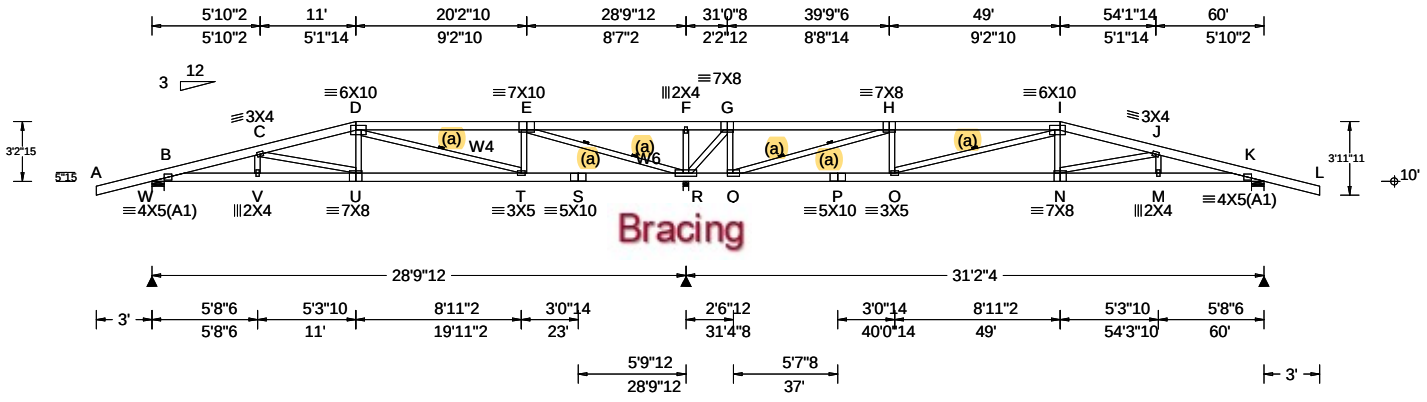
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464585 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A03		Cust: R 215 JRef:1XR52150012 T14 DrwNo: 188.23.0800.49597 SSB / DF 07/07/2023	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.127 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.263 N 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.042 K - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.083 K - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.298
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.211
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.997
	C&C Dist a: 6.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 8.50 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	1205	/-	/-	/662	/233	/66
R	2835	/-	/-	/1455	/536	/-
K	1297	/-	/-	/723	/254	/-

Wind reactions based on MWFRS
W Brg Wid = 8.0 Min Req = 1.5 (Truss)
R Brg Wid = 3.5 Min Req = 2.0 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings W, R, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	457	- 2699	G - H	1011	- 140
C - D	407	- 2303	H - I	449	- 2081
D - E	369	- 1424	I - J	493	- 2674
E - F	2132	- 350	J - K	535	- 3036
F - G	2132	- 350			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	2583	- 384	Q - P	2049	- 363
V - U	2582	- 387	P - O	2049	- 363
U - T	2214	- 298	O - N	2574	- 381
T - S	1383	- 310	N - M	2909	- 463
S - R	1383	- 310	M - K	2909	- 459
R - Q	271	- 1114			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	92	- 410	R - G	321	- 1737
D - U	424	0	G - Q	986	- 119
D - T	198	- 1046	Q - H	556	- 2882
T - E	584	0	H - O	507	0
E - R	612	- 3195	O - I	125	- 662
F - R	172	- 421	I - N	418	0

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W4,W6 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

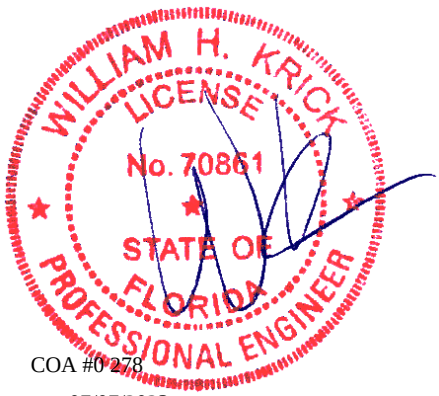
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 3-2-15.



COA #0278

07/07/2023

Florida Certificate of Product Approval #FL 1999

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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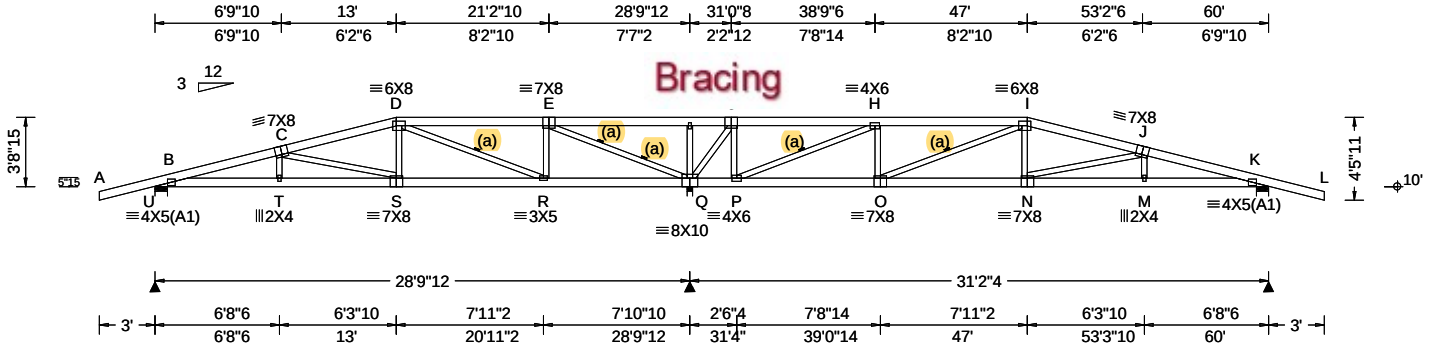
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464587 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A04		Cust: R 215 JRef:1XR52150012 T9 DrwNo: 188.23.0800.51620 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 8.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.106	N	999	240
VERT(CL): 0.224	N	999	180
HORZ(LL): 0.035	K	-	-
HORZ(TL): 0.069	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.259
Max BC CSI: 0.194
Max Web CSI: 0.929

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
U	1176	/-	/-	/649	/194	/73
Q	2908	/-	/-	/1494	/178	/-
K	1271	/-	/-	/713	/205	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.4
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	229	- 2595	G - H	1210	- 80
C - D	294	- 1869	H - I	345	- 1424
D - E	303	- 816	I - J	339	- 2251
E - F	2186	- 97	J - K	272	- 2950
F - G	2186	- 97			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	2480	- 185	P - O	1384	- 244
T - S	2476	- 189	O - N	2148	- 234
S - R	1778	- 219	N - M	2821	- 202
R - Q	769	- 343	M - K	2824	- 198
Q - P	155	- 1303			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	42	- 749	G - P	1053	- 82
D - S	470	0	P - H	317	- 2457
D - R	55	- 1236	H - O	587	0
R - E	665	0	O - I	46	- 921
E - Q	338	- 2693	I - N	465	0
Q - G	271	- 1624	N - J	42	- 716

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 3-8-15.

COA #0 278
07/07/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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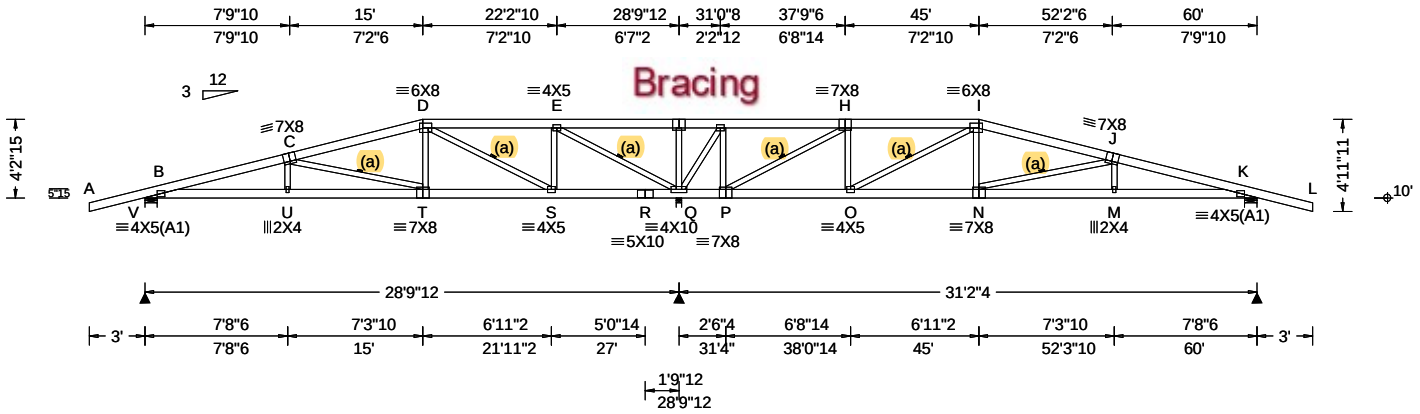
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464589 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A05	Cust: R 215 JRef:1XR52150012 T8 DrwNo: 188.23.0801.46683 SSB / DF 07/07/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 17.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.098	J	999	240
VERT(CL): 0.208	J	999	180
HORZ(LL): 0.029	K	-	-
HORZ(TL): 0.058	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.222
Max BC CSI: 0.182
Max Web CSI: 0.763

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
V	1139	/-	/-	/632	/174	/80
Q	2993	/-	/-	/1541	/255	/-
K	1238	/-	/-	/699	/188	/-

Wind reactions based on MWFRS
V Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.1 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings V, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	166	- 2421	G - H	1389	- 63
C - D	206	- 1416	H - I	238	- 869
D - E	591	- 295	I - J	263	- 1816
E - F	2264	- 103	J - K	221	- 2799
F - G	2264	- 103			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	2309	- 123	Q - P	153	- 1474
U - T	2303	- 126	P - O	824	- 159
T - S	1325	- 123	O - N	1715	- 148
S - R	243	- 632	N - M	2669	- 149
R - Q	243	- 632	M - K	2674	- 145

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	50	- 1036	G - P	1107	- 84
D - T	517	0	P - H	255	- 2126
D - S	65	- 1351	H - O	675	0
S - E	748	0	O - I	40	- 1095
E - Q	282	- 2316	I - N	510	0
Q - G	235	- 1562	N - J	49	- 1008

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 4-2-15.

COA #0278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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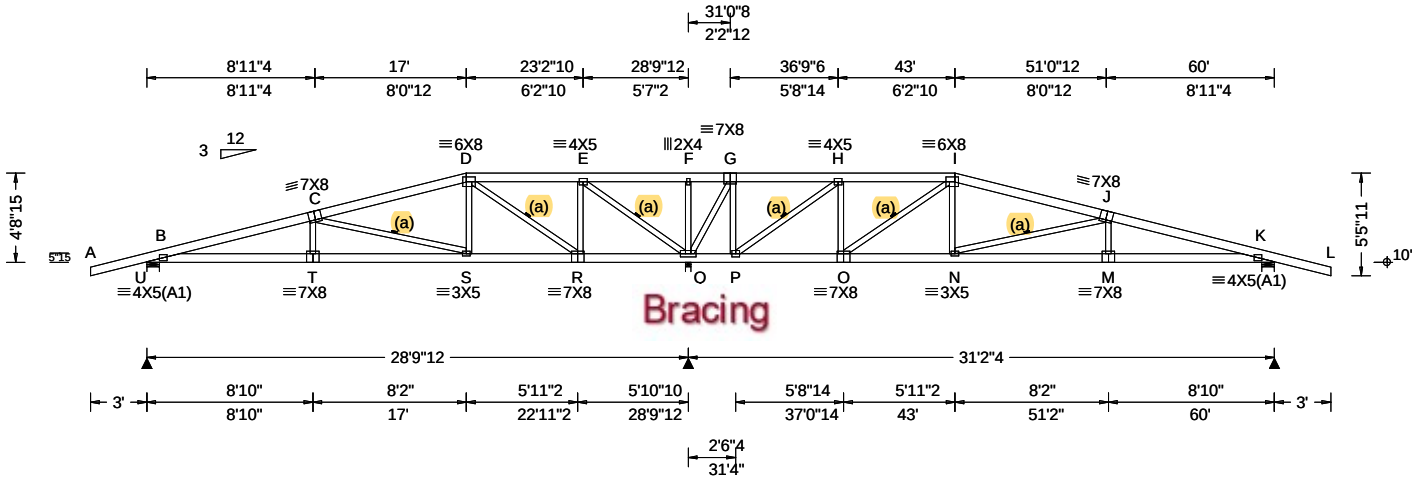
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464593 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A06		Cust: R 215 JRef:1XR52150012 T13 DrwNo: 188.23.0801.48230 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 17.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.096	J	999	240
VERT(CL): 0.209	J	999	180
HORZ(LL): 0.024	K	-	-
HORZ(TL): 0.048	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.209
Max BC CSI: 0.189
Max Web CSI: 0.667

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
U	1107	/-	/-	/615	/147	/87
Q	3073	/-	/-	/1584	/94	/-
K	1210	/-	/-	/686	/152	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.2 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	77	- 2225	G - H	1506	0
C - D	137	- 974	H - I	390	- 425
D - E	827	0	I - J	106	- 1392
E - F	2291	- 18	J - K	94	- 2624
F - G	2291	- 18			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	2114	- 89	P - O	375	- 427
T - S	2107	- 93	O - N	1293	0
S - R	887	- 114	N - M	2494	- 32
R - Q	123	- 867	M - K	2500	- 29
Q - P	102	- 1585			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	63	- 1287	G - P	1165	0
D - S	572	0	P - H	62	- 1875
D - R	42	- 1417	H - O	810	0
R - E	896	0	O - I	35	- 1196
E - Q	66	- 2006	I - N	564	0
Q - G	52	- 1527	N - J	62	- 1263

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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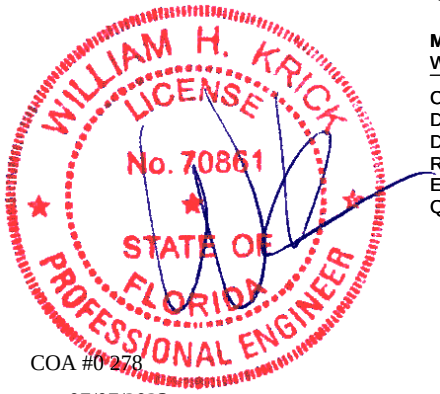
The overall height of this truss excluding overhang is 4-8-15.



COA #0 278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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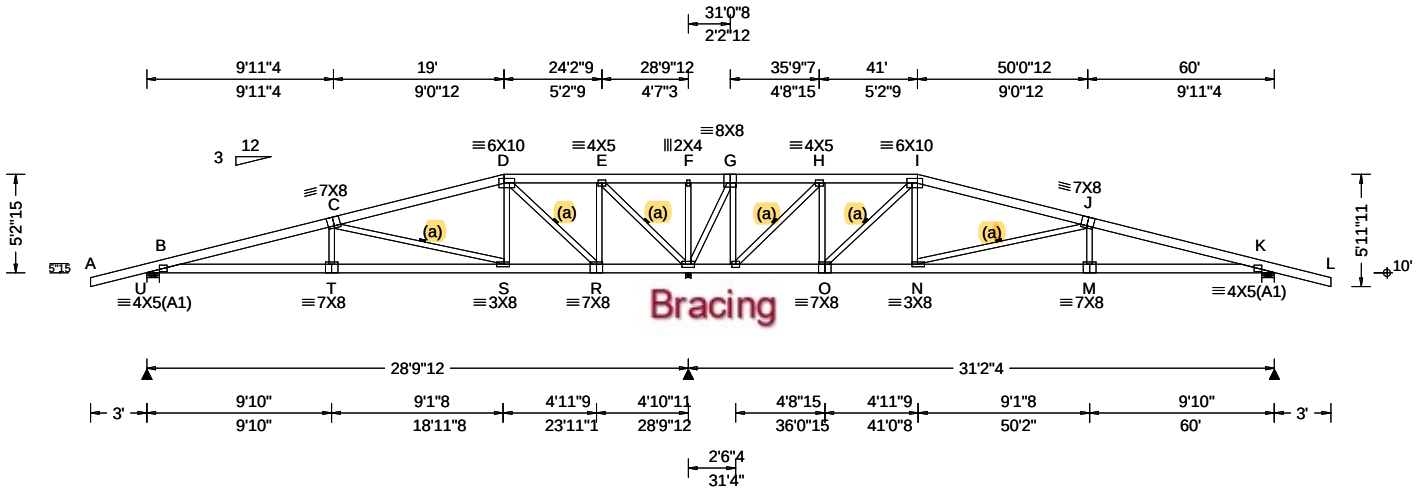
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464598 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A07		Cust: R 215 JRef:1XR52150012 T7 DrwNo: 188.23.0801.49753 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 17.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.097	M	999	240
VERT(CL): 0.214	M	999	180
HORZ(LL): 0.021	K	-	-
HORZ(TL): 0.042	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.194
Max BC CSI: 0.200
Max Web CSI: 0.778

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
U	1080	/-	/-	/602	/146	/94
Q	3138	/-	/-	/1620	/95	/-
K	1187	/-	/-	/676	/151	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.2 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	80	- 2047	G - H	1549	0
C - D	400	- 560	H - I	594	- 71
D - E	1004	0	I - J	99	- 995
E - F	2261	- 9	J - K	98	- 2464
F - G	2261	- 9			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	1937	- 121	P - O	109	- 632
T - S	1930	- 124	O - N	898	- 49
S - R	476	- 368	N - M	2334	- 38
R - Q	124	- 1044	M - K	2340	- 35
Q - P	107	- 1623			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
T - C	389	0	G - P	1205	- 2
C - S	74	- 1526	P - H	54	- 1676
D - S	630	0	H - O	969	0
D - R	43	- 1461	O - I	36	- 1271
R - E	1039	0	I - N	621	0
E - Q	57	- 1761	N - J	74	- 1505
Q - G	50	- 1510	J - M	386	0

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 5-2-15.

COA #0278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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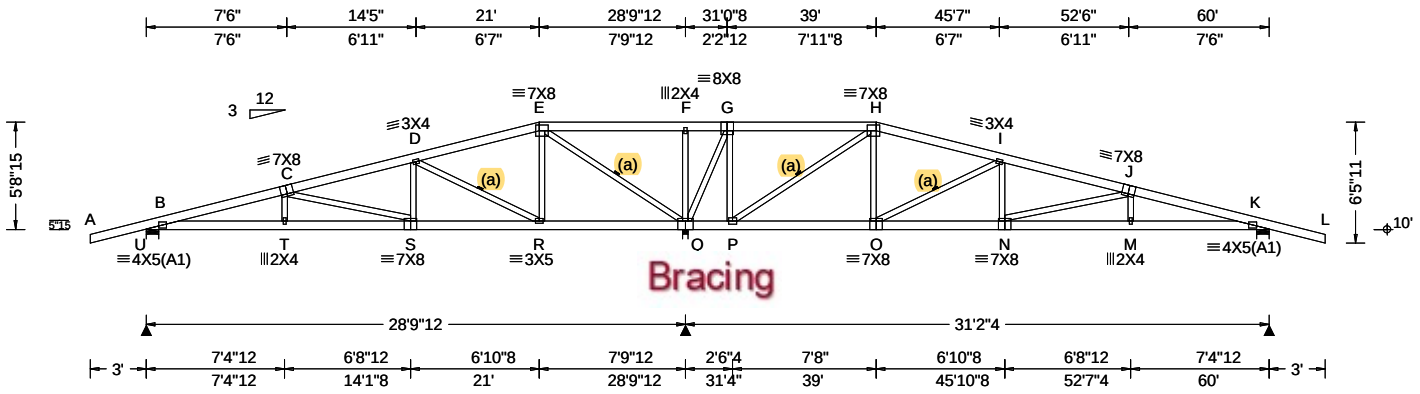
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464601 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A08		Cust: R 215 JRef:1XR52150012 T6 DrwNo: 188.23.0801.54487 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 17.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.086	J	999	240
VERT(CL): 0.191	J	999	180
HORZ(LL): 0.021	K	-	-
HORZ(TL): 0.042	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.281
Max BC CSI: 0.168
Max Web CSI: 0.930

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
U	1060	/-	/-	/592	/145	/100
Q	3187	/-	/-	/1649	/95	/-
K	1170	/-	/-	/669	/151	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.6
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	88	-2130	G - H	1488	0
C - D	107	-1223	H - I	319	-558
D - E	662	-69	I - J	120	-1655
E - F	2139	0	J - K	105	-2556
F - G	2139	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	2028	-144	P - O	495	-293
T - S	2023	-148	O - N	1534	-43
S - R	1117	-131	N - M	2435	-63
R - Q	129	-629	M - K	2440	-60
Q - P	111	-1552			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	43	-939	G - P	1150	0
S - D	490	0	P - H	58	-1970
D - R	85	-1268	H - O	728	0
E - R	727	0	O - I	68	-1188
E - Q	47	-2078	I - N	483	0
F - Q	90	-450	N - J	46	-931
Q - G	56	-1538			

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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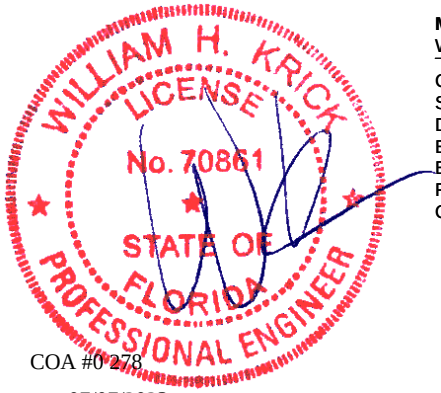
The overall height of this truss excluding overhang is 5-8-15.



COA #0278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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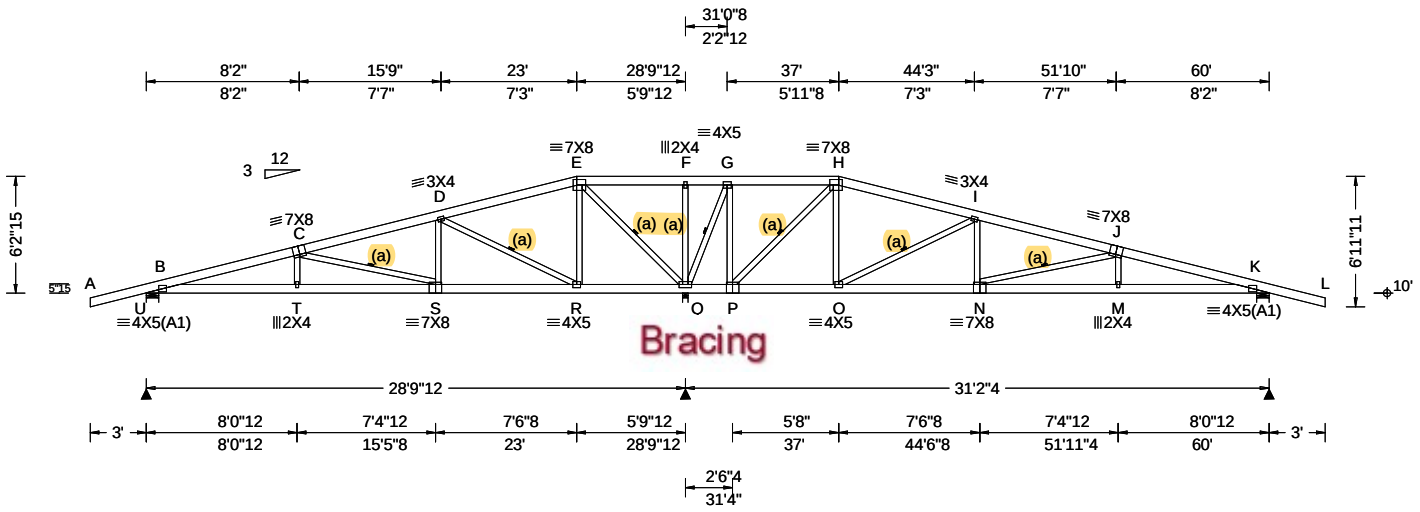
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464603 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A09		Cust: R 215 JRef:1XR52150012 T12 DrwNo: 188.23.0801.56087 SSB / DF 07/07/2023	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.084 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.190 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.017 K - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.034 K - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.218
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.165
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.640
	C&C Dist a: 6.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 17.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
U	1022	/-	/-	/569	/143	/107
Q	3282	/-	/-	/1700	/96	/-
K	1137	/-	/-	/650	/150	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.3 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	113	-1944	G - H	1616	0
C - D	323	-881	H - I	631	-96
D - E	997	0	I - J	153	-1334
E - F	2224	0	J - K	139	-2389
F - G	2224	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	1845	-172	P - O	106	-596
T - S	1839	-175	O - N	1217	-69
S - R	779	-313	N - M	2270	-94
R - Q	134	-935	M - K	2276	-90
Q - P	118	-1678			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	51	-1097	G - P	1243	-1
S - D	554	0	P - H	52	-1778
D - R	92	-1409	H - O	784	0
E - R	769	0	O - I	79	-1345
E - Q	46	-1830	I - N	545	0
Q - G	53	-1517	N - J	53	-1088

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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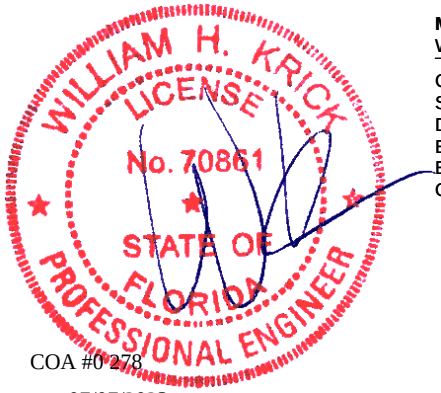
The overall height of this truss excluding overhang is 6-2-15.



COA #0278

07/07/2023

Florida Certificate of Product Approval #FL 1999

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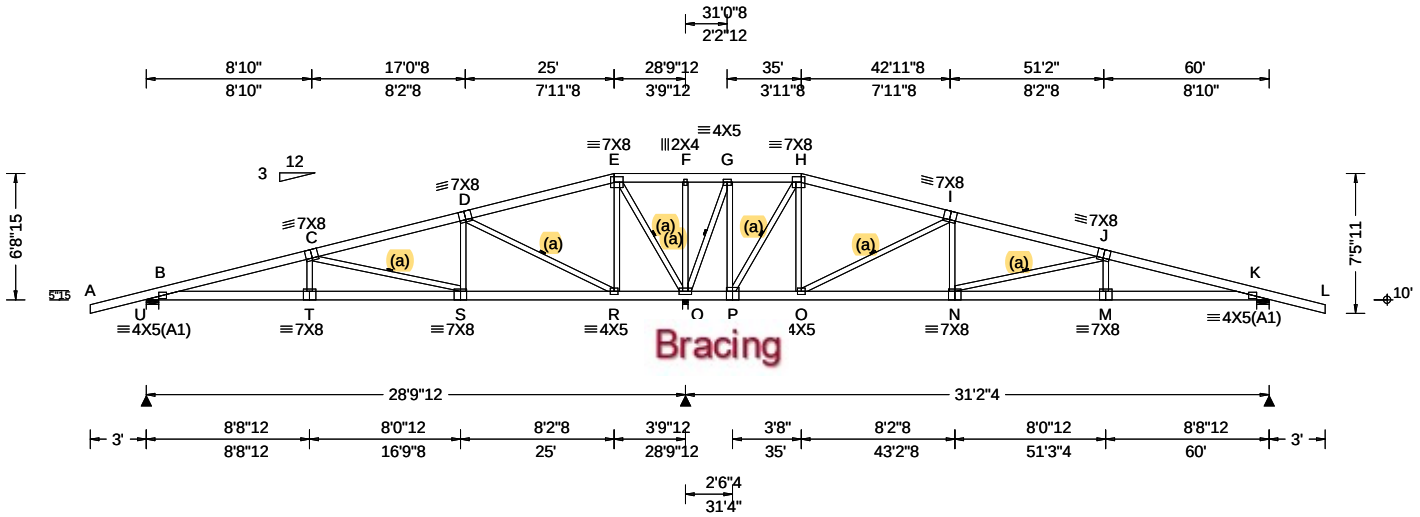
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464605 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A10		Cust: R 215 JRef:1XR52150012 T5 DrwNo: 188.23.0801.58170 SSB / DF 07/07/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 6.00 ft
Loc. from endwall: not in 17.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.085	J	999	240
VERT(CL): 0.195	J	999	180
HORZ(LL): 0.015	K	-	-
HORZ(TL): 0.030	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.205
Max BC CSI: 0.172
Max Web CSI: 0.682

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
U	999	/-	/-	/556	/142	/114
Q	3331	/-	/-	/1731	/96	/-
K	1117	/-	/-	/640	/149	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.4 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	128	- 1807	G - H	1619	0
C - D	510	- 594	H - I	881	- 20
D - E	1440	0	I - J	183	- 1060
E - F	2180	0	J - K	164	- 2266
F - G	2180	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	1709	- 191	P - O	111	- 837
T - S	1703	- 194	O - N	946	- 177
S - R	494	- 494	N - M	2148	- 115
R - Q	141	- 1352	M - K	2154	- 112
Q - P	124	- 1680			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	59	- 1249	G - P	1320	- 22
S - D	611	0	P - H	50	- 1638
D - R	100	- 1538	H - O	847	0
E - R	796	0	O - I	88	- 1483
E - Q	41	- 1596	I - N	604	0
Q - G	49	- 1493	N - J	62	- 1240

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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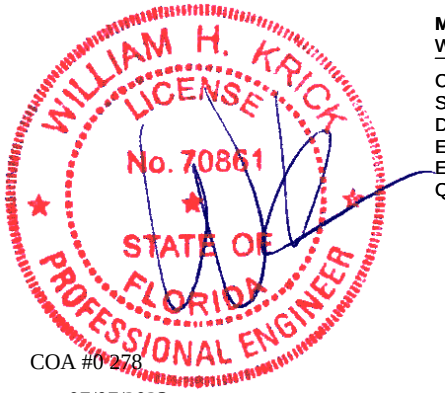
The overall height of this truss excluding overhang is 6-8-15.



COA #0278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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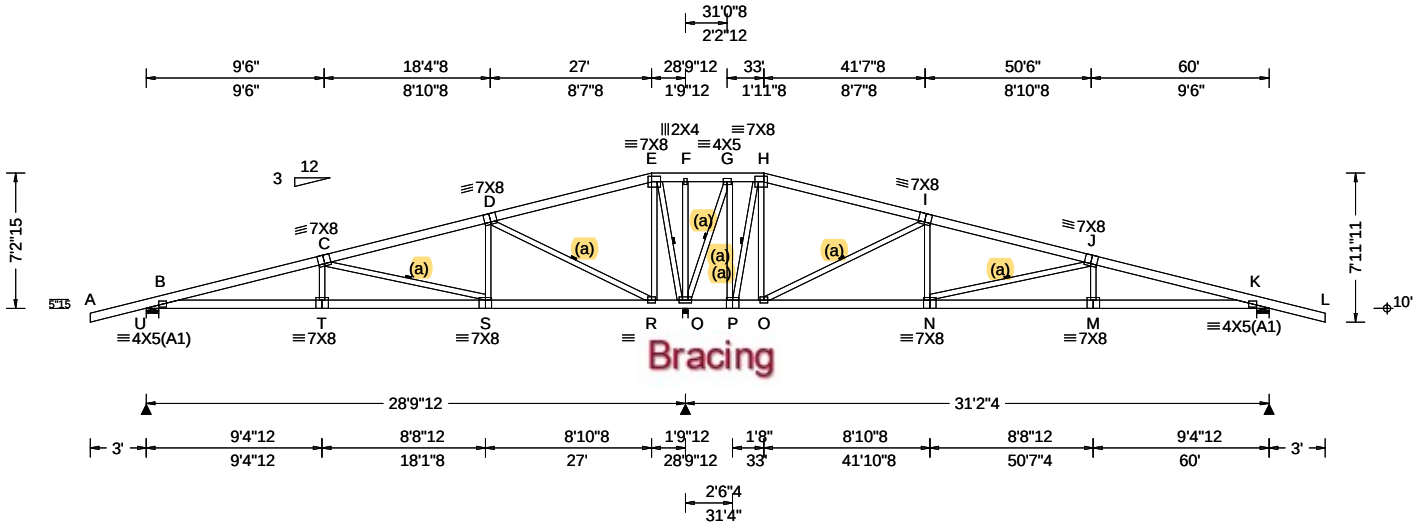
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464607 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A11		Cust: R 215 JRef:1XR52150012 T4 DrwNo: 188.23.0801.59723 SSB / DF 07/07/2023	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.089 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.204 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 K - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.030 C - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.250
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.181
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.852
	C&C Dist a: 6.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 17.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
U	988	/-	/-	/549	/141	/121
Q	3356	/-	/-	/1746	/72	/-
K	1109	/-	/-	/636	/149	/-

Wind reactions based on MWFRS
U Brg Wid = 8.0 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.4 (Truss)
K Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings U, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	134	-1711	G - H	1530	0
C - D	652	-355	H - I	1260	0
D - E	1822	0	I - J	326	-836
E - F	2054	0	J - K	179	-2182
F - G	2053	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	1612	-201	P - O	118	-1168
T - S	1605	-205	O - N	723	-314
S - R	257	-630	N - M	2062	-128
R - Q	151	-1718	M - K	2069	-124
Q - P	132	-1587			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - S	46	-1392	G - P	1343	-50
S - D	663	0	P - H	48	-1515
D - R	111	-1671	H - O	874	0
E - R	736	0	O - I	98	-1634
E - Q	25	-1296	I - N	662	0
Q - G	52	-1492	N - J	49	-1379

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 7-2-15.

COA #0278

07/07/2023

Florida Certificate of Product Approval #FL 1999

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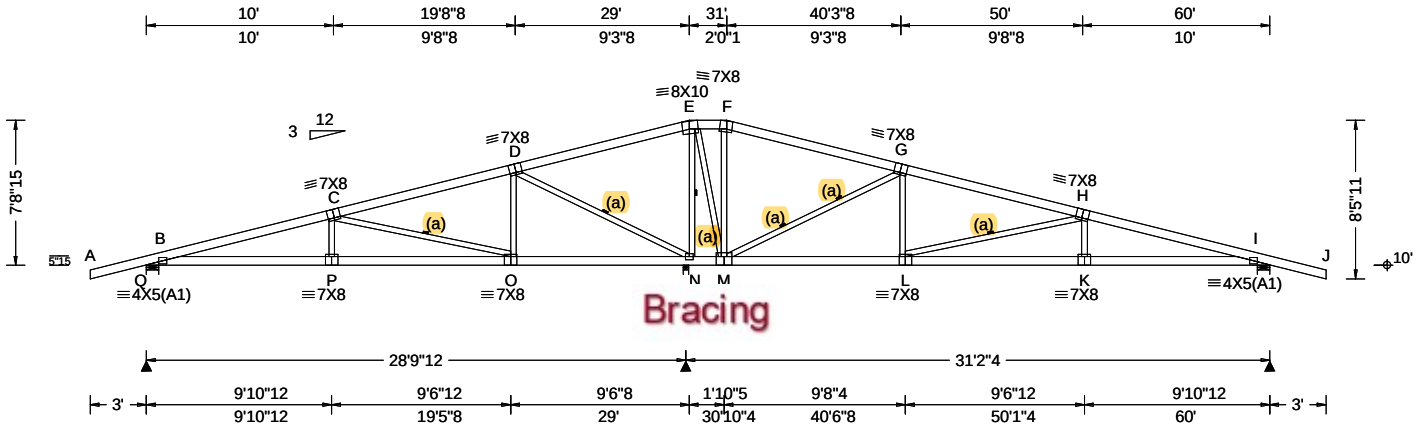
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 464609 HIPS FROM:	Ply: 1 Qty: 2	**Job Number:** 20-4958B Jackson Residence **Truss Label:** A12	Cust: R 215 JRef:1XR52150012 T11 DrwNo: 188.23.0802.02880 SSB / DF 07/07/2023	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.100 H 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.225 H 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.019 I - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.037 I - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.255
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.194
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: Yes	Max Web CSI: 0.970
	C&C Dist a: 6.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 17.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	1007	/-	/-	/562	/142	/128
N	3292	/-	/1	/1720	/79	/0
I	1135	/-	/-	/650	/151	/-

Wind reactions based on MWFRS
Q Brg Wid = 8.0 Min Req = 1.5 (Truss)
N Brg Wid = 3.5 Min Req = 2.4 (Truss)
I Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings Q, N, & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	127	- 1759	F - G	1485	0
C - D	664	- 209	G - H	357	- 753
D - E	1910	0	H - I	186	- 2248
E - F	1393	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	1658	- 200	M - L	636	- 342
P - O	1650	- 203	L - K	2124	- 132
O - N	341	- 1274	K - I	2131	- 129
N - M	142	- 1789			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
P - C	402	0	M - F	57	- 582
C - O	62	- 1595	M - G	108	- 1789
O - D	662	0	G - L	719	0
D - N	104	- 1667	L - H	61	- 1526
E - N	92	- 2406	H - K	381	0
E - M	1603	- 50			

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

The overall height of this truss excluding overhang is 7-8-15.

COA #0 278

07/07/2023

Florida Certificate of Product Approval #FL 1999

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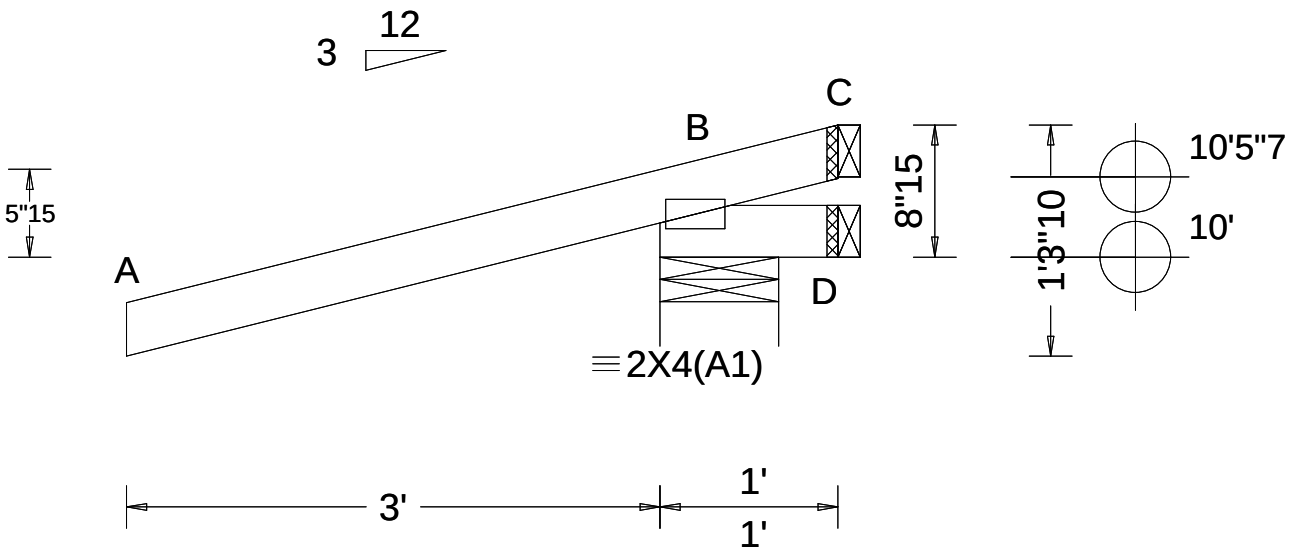
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 43712 / JACK FROM:	Ply: 1 Qty: 8	**Job Number:** 20-4958B Jackson Residence **Truss Label:** J01	Cust: R 215 JRef:1XR52150012 T53 / DrwNo: 187.23.1201.02985 AK / YK 07/06/2023



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.002	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.650
Max BC CSI: 0.091
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	616	/-	/-	/381	/347	/44
D	-	/-73	/-	/54	/44	/-
C	-	/-267	/-	/175	/174	/-

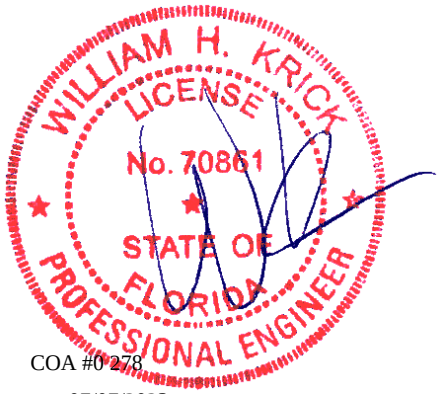
Wind reactions based on MWFRS
B Brg Wid = 8.0 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -267# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

It is the responsibility of the building designer and truss fabricator to review this dwg prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans, specifications and fabricator's truss layout.



COA #0 278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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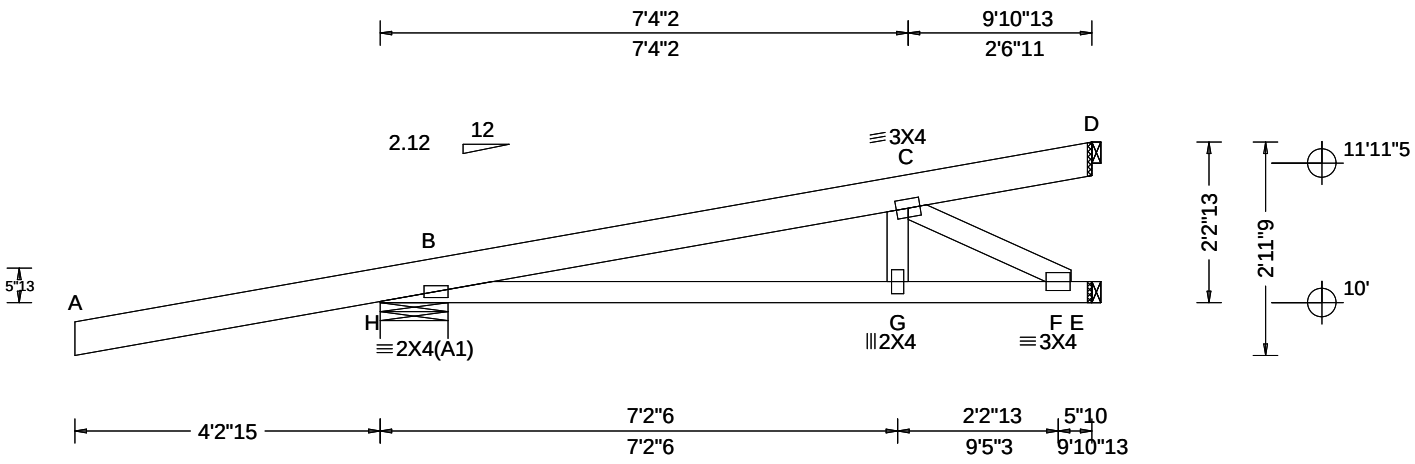
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 43725 / HIP_ FROM:	Ply: 1 Qty: 4	**Job Number:** 20-4958B Jackson Residence **Truss Label:** J01HJ	Cust: R 215 JRef:1XR52150012 T2 / DrwNo: 187.23.1201.02986 AK / YK 07/06/2023



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.017	G	999	240
VERT(CL): 0.033	G	999	180
HORZ(LL): 0.005	F	-	-
HORZ(TL): 0.009	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.366
Max BC CSI: 0.329
Max Web CSI: 0.058

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	738	/-	/-	/-	/195	/-
E	340	/-	/0	/-	/19	/-
D	200	/-	/-	/-	/70	/-

Wind reactions based on MWFRS
H Brg Wid = 11.3 Min Req = 1.5 (Truss)
E Brg Wid = 1.5 Min Req = -
D Brg Wid = 1.5 Min Req = -
Bearing H is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	71	-597

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	548	-55	G - F	537	-61

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	71	-626

Lumber
Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading
Hipjack supports 7-0-0 setback jacks with no webs.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

COA #0278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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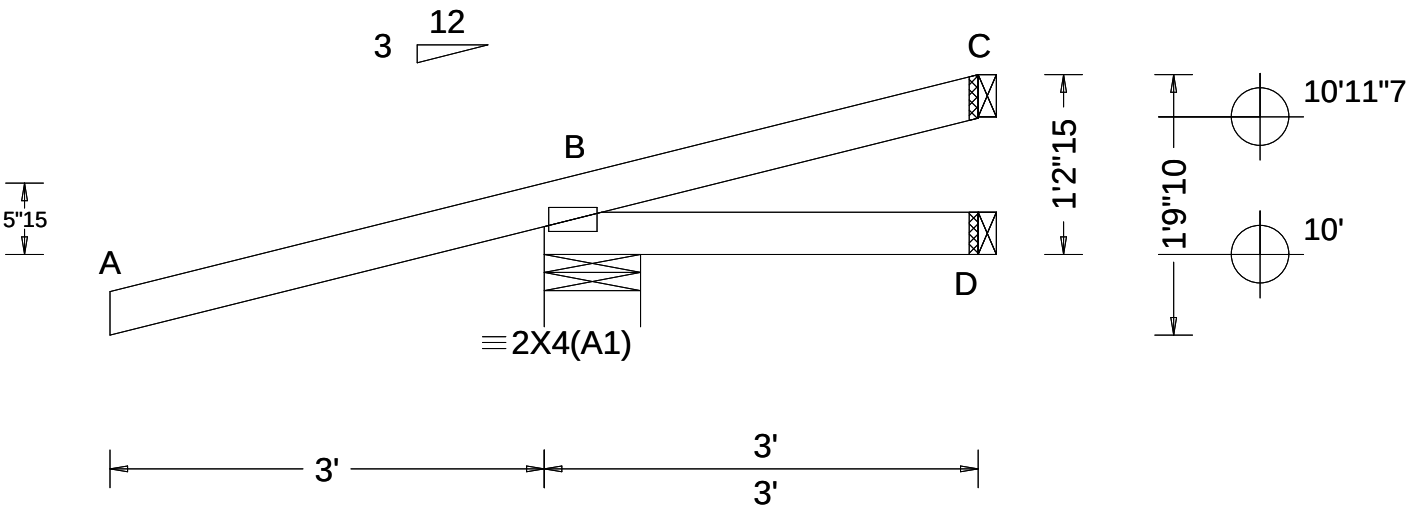
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 43714 / JACK FROM:	Ply: 1 Qty: 8	**Job Number:** 20-4958B Jackson Residence **Truss Label:** J02	Cust: R 215 JRef:1XR52150012 T45 / DrwNo: 187.23.1201.03095 AK / YK 07/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.003 B	-	-
HORZ(TL):	0.005 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.650
Max BC CSI: 0.154
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	433	/-	/-	/257	/195	/59
D	39	/-5	/-	/36	/6	/-
C	8	/-	/-	/36	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 8.0 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

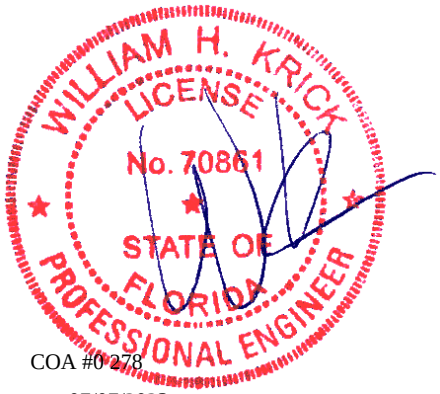
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

It is the responsibility of the building designer and truss fabricator to review this dwg prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans, specifications and fabricator's truss layout.



COA #0 278
07/07/2023
Florida Certificate of Product Approval #FL 1999

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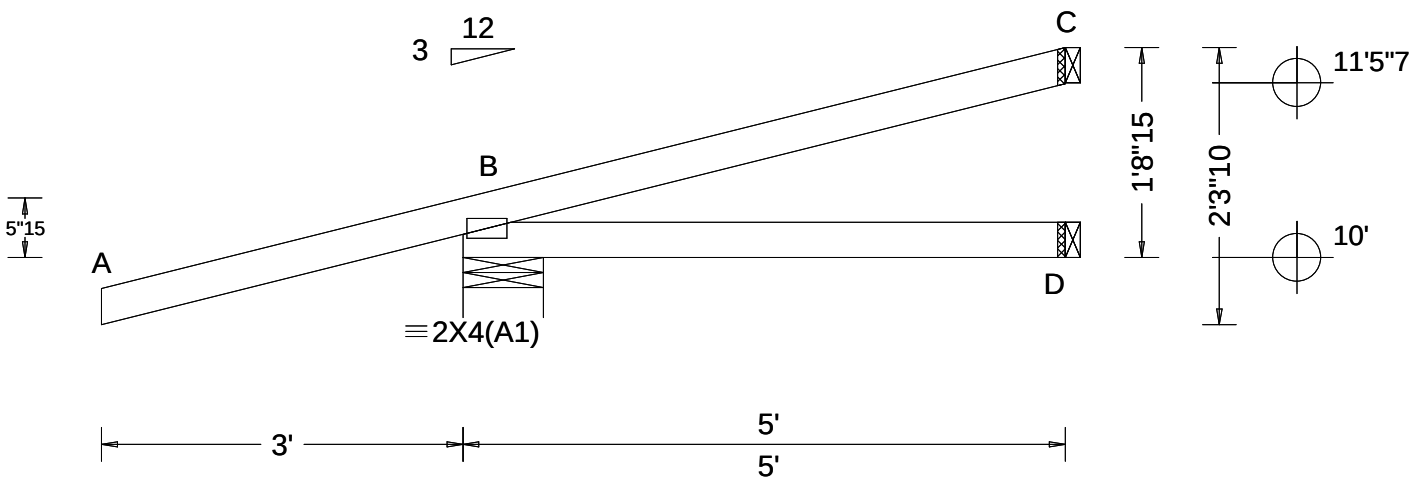
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 43716 / JACK FROM:	Ply: 1 Qty: 8	**Job Number:** 20-4958B Jackson Residence **Truss Label:** J03	Cust: R 215 JRef:1XR52150012 T44 DrwNo: 187.23.1201.03094 AK / YK 07/06/2023



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.006 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.650
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.198
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	468	/-	/-	/269	/175	/74
D	82	/-	/-	/43	/-	/-
C	97	/-	/-	/43	/46	/-

Wind reactions based on MWFRS
B Brg Wid = 8.0 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

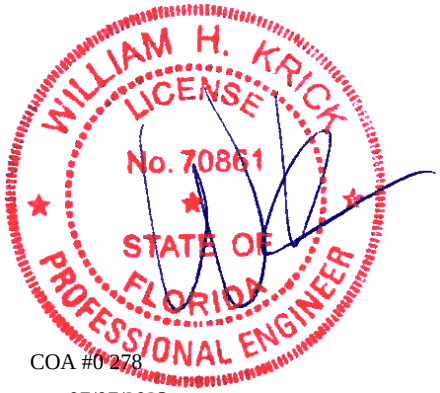
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

It is the responsibility of the building designer and truss fabricator to review this dwg prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans, specifications and fabricator's truss layout.



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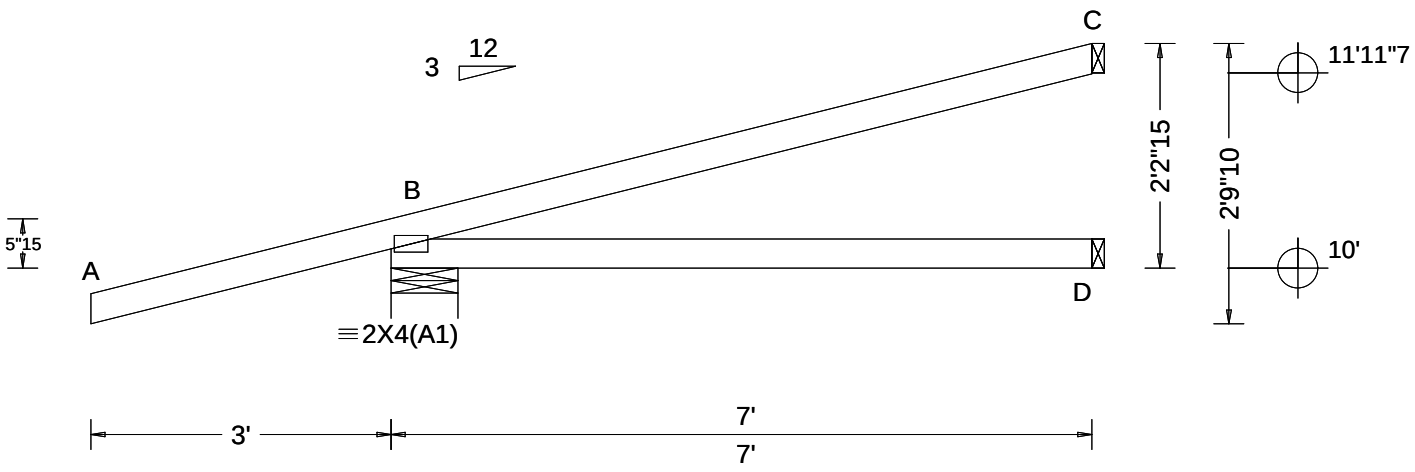
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 43718 / EJAC FROM:	Ply: 1 Qty: 48	**Job Number:** 20-4958B Jackson Residence **Truss Label:** J04	Cust: R 215 JRef:1XR52150012 T57 / DrwNo: 187.23.1201.03000 AK / YK 07/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.006 B	-	-
HORZ(TL):	0.012 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.650
Max BC CSI: 0.464
Max Web CSI: 0.000

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	529	/-	/-	/299	/171	/89
D	122	/-	/-	/65	/-	/-
C	165	/-	/-	/67	/72	/-

Wind reactions based on MWFRS
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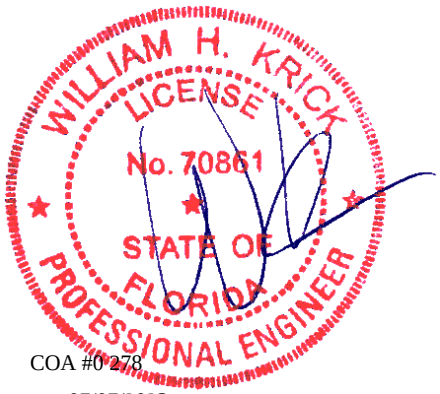
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

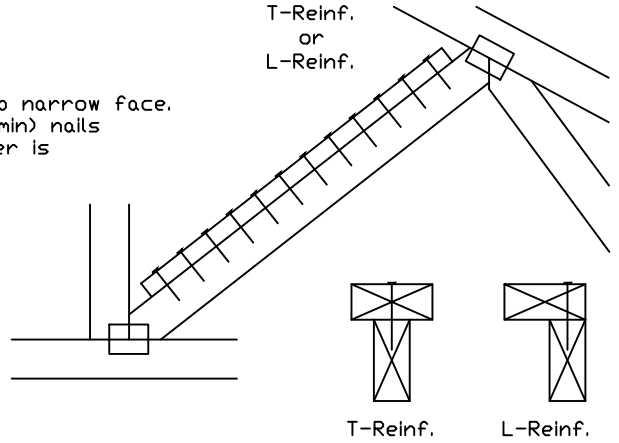
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

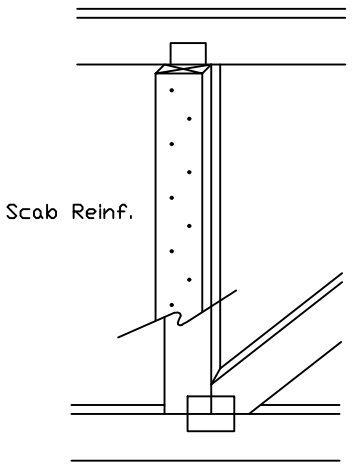
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC LL	PSF	
TC DL	PSF	
BC DL	PSF	
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119