

DATE 11/17/2005

# Columbia County Building Permit

PERMIT 000023867

This Permit Expires One Year From the Date of Issue

APPLICANT MELANIE RODER PHONE 752.2281  
 ADDRESS 387 SW KEMP CT LAKE CITY FL 32025  
 OWNER ABRAM & ANGELA HUBER PHONE 755.1102  
 ADDRESS 225 SW FINLEY LITTLE LN LAKE CITY FL 32024  
 CONTRACTOR ISAAC BRATKOVICH PHONE 719.7143

LOCATION OF PROPERTY 47-S TO LITTLE RD, TL (SOUTHWOOD ACRES ENTRANCE), TL ON FINLEY LITTLE LN AND ITS THE 3RD LOT DOWN ON THE L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 240000.00  
 HEATED FLOOR AREA 4800.00 TOTAL AREA 6825.00 HEIGHT 21.00 STORIES 2  
 FOUNDATION CONC WALLS FRAMED ROOF PITCH 3.12 FLOOR CONC  
 LAND USE & ZONING A-3 MAX. HEIGHT 35  
 Minimum Set Back Requirements: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00  
 NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 01-55-16-03387-012 SUBDIVISION  
 LOT BLOCK PHASE UNIT TOTAL ACRES 2.05

000000890  
 Culvert Permit No. Culvert Waiver Contractor's License Number  
 18"X32"MITERED 05-1036-N BLK JTH Applicant/Owner/Contractor  
 Driveaway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident  
 COMMENTS: NOC ON FILE, SECTION 14.9 SPECIAL FAMILY LOT PERMIT.

1 FOOT ABOVE ROAD.  
 Check # or Cash 1629

## FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power Foundation Monolithic Sheathing/Nailing  
 Under slab rough-in plumbing Slab  
 Framing Rough-in plumbing above slab and below wood floor  
 Electrical rough-in Heat & Air Duct Pert. beam (Lintel)  
 Permanent power C.O. Final Culvert  
 M/H tie downs, blocking, electricity and plumbing date/app. by  
 Pump pole Utility Pole Pool  
 Reconnection date/app. by  
 M/H Pole Travel Trailer Re-roof  
 date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 1200.00 CERTIFICATION FEE \$ 34.13 SURCHARGE FEE \$ 34.13  
 MISC. FEES \$ .00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ .00 WASTE FEE \$  
 FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$  
 CULVERT FEE \$ 25.00 TOTAL FEE 1343.26  
 INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

## This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION. IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.



**COLUMBIA COUNTY INSPECTION SHEET**

*GF*

*6/16/06 Fri*

DATE 06/15/2006 TAKEN BY \_\_\_\_\_ INSPECTION DATE: \_\_\_\_\_

BUILDING PERMIT # 000023867 CULVERT / WAIVER PERMIT # 000000890 WAIVER

PARCEL ID # 01-5S-16-03387-012 ZONING A-3

TYPE OF DEVELOPMENT SFD/UTILITY

SETBACKS: FRONT 30.00 REAR 25.00 SIDE 25.00 HEIGHT 21.00

FLOOD ZONE X SEPTIC 05-1036-N NO. EXISTING D.U. 0

SUBDIVISION Lot Block Unit Phase

OWNER ABRAM & ANGELA HUBER PHONE 755.1102

ADDRESS 225 SW FINLEY LITTLE LN LAKE CITY FL 32024

CONTRACTOR ISAAC BRATKOVICH PHONE 719.7143

LOCATION 47-S TO LITTLE RD, TL (SOUTHWOOD ACRES ENTRANCE), TL ON FINLEY LITTLE LN AND IT'S THE 3RD LOT DOWN ON THE L.

COMMENTS: NOC ON FILE. SECTION 14.9 SPECIAL FAMILY LOT PERMIT.

1 FOOT ABOVE ROAD.

INSPECTION(S) REQUESTED:

Temp Power 11/23/2005 RTJ Foundation 11/23/2005 RJ Set backs 11/23/2005 RJ

Mono Slab Under Slab Rough-in 12/13/2005 RJ Slab 12/15/2005 HD

Sheathing/Nailing Framing Other

Above slab Rough-in Electrical Rough-in

Heat & A/C Beam (Lintel) Perm Power

CO Final Culvert Reconnection

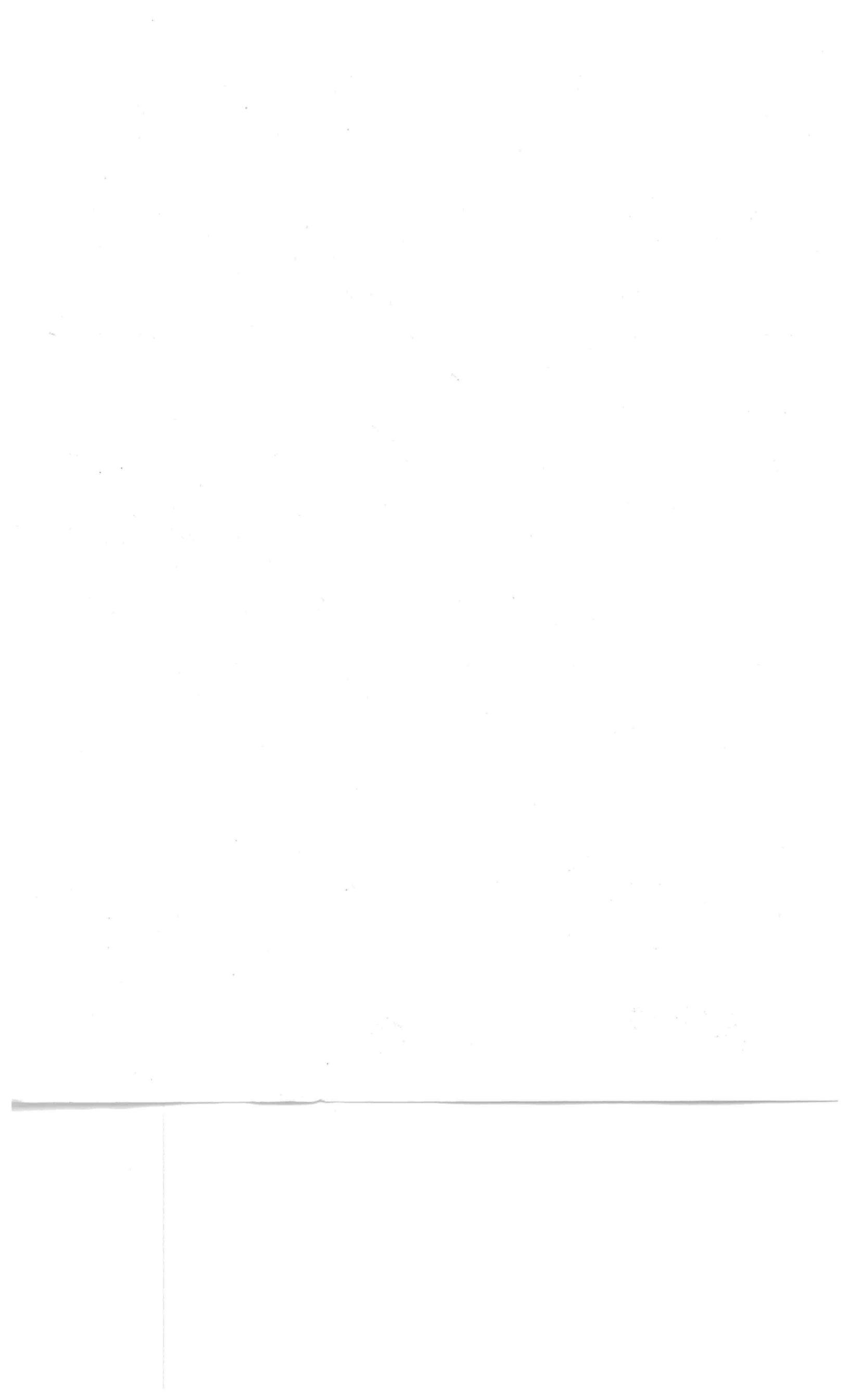
Pool MH Perm Power Utility Pole

RV Power Re-Roof MH Pole

INSPECTORS:

APPROVED NOT APPROVED BY \_\_\_\_\_ POWER CO. CLAY

INSPECTORS COMMENTS:



**HAND DELIVERED**

Mr. John Kerce  
Planning and Zoning Department  
County Administrative Offices  
135 NE Hernando Avenue  
Lake City, Florida 32055

Re: Abram B. Huber

Dear John:

This will confirm our telephone conference of June 14, 2006 regarding the custom built window issue for Mr. Huber's residence here in Columbia County.

As stated in previous correspondence to Mr. Huber, Florida Statute 553.842(10) provides products which are custom fabricated or assembled shall not require separate approval under the section provided the component parts have been approved for the fabricated or assembled products used and the components meet the standards and requirements of the Florida Building Code which applies to the product's intended use. Section R613.3.1 provides that exterior windows shall be tested by an approved independent testing laboratory, etc. The determination of load resistance of glass for specified loads of products tested and certified in accordance with this section shall be designed to comply with ASTM E 1300. Section R613.4.2 provides for and allows exterior door assemblies (presumably including those containing glass) may be engineered in accordance with accepted engineering practices in lieu of testing by an approved testing laboratory.

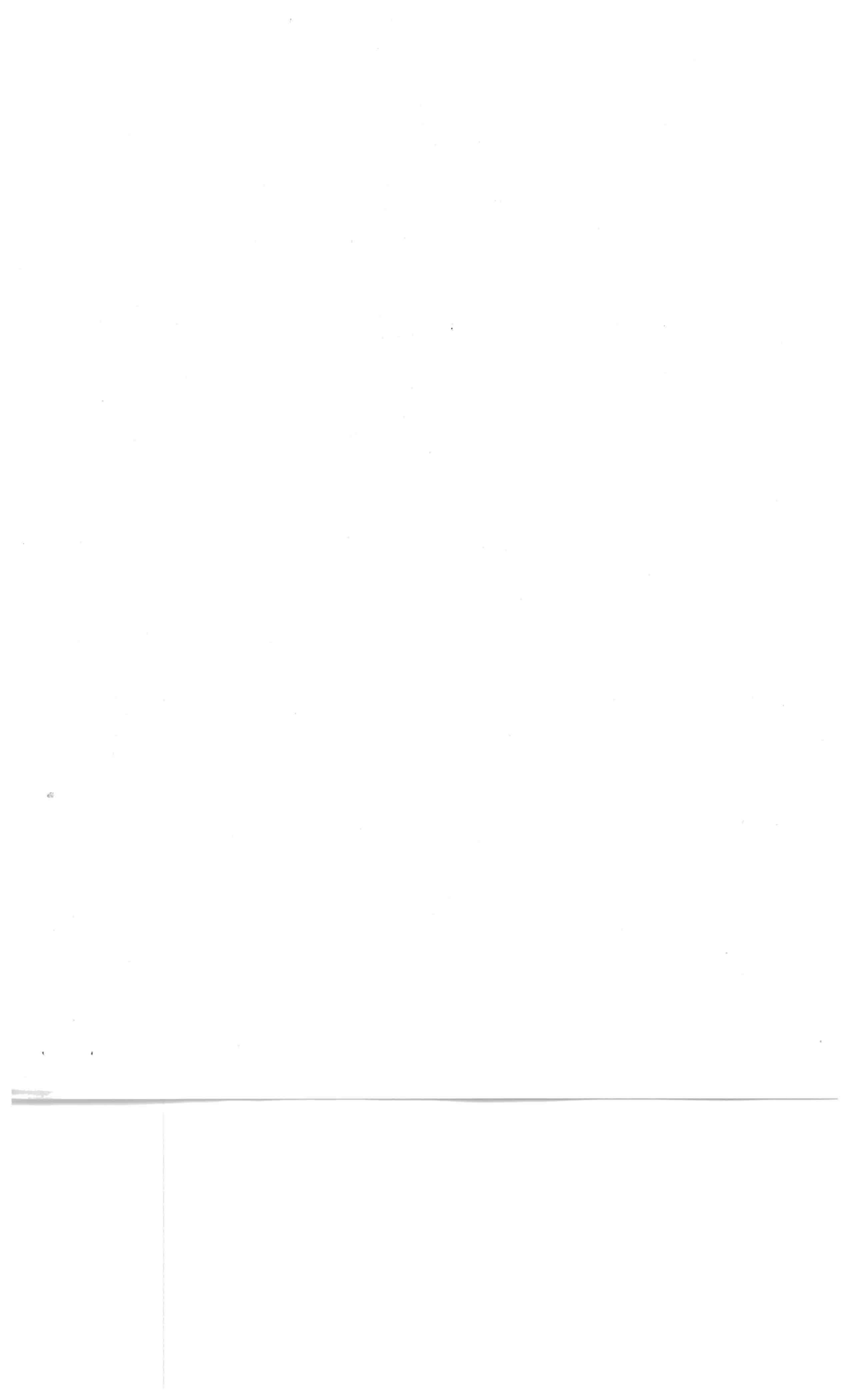
Mr. Huber has obtained an engineering analysis and certification from Mark Disosway, P.E., as evidenced in his letter to Columbia County dated May 30, 2006. Engineer Disosway has given the opinion that in accordance with accepted engineering practices both the windows and the doors used in Mr. Huber's house meet the requirements of the 2004 Florida Building Code. I further discussed this with Mr. Disosway on June 14, 2006 at which time Mr. Disosway explained his letter and report. Attached to the Disosway report of May 30, 2006 is the ASTM E 1300 calculation for the largest window pane. This calculation shows the 110 MPH wind load pressure requirement as 30.0 per square feet (PSF). The calculated load resistance for the Huber

**FEAGLE & FEAGLE, ATTORNEYS, P.A.**  
ATTORNEYS AT LAW  
153 NE MADISON STREET  
POST OFFICE BOX 1653  
LAKE CITY, FLORIDA 32056-1653  
(386) 752-7191  
Fax: (386) 758-0950

June 14, 2006

Mark E. Feagle  
e-mail: leagle@bellsouth.net

Mark E. Feagle  
e-mail: mfeagle@bellsouth.net



Mr. John Kerce  
Page 2  
June 14, 1006

window panes is 43.8 PSF. Thus, the Huber window panes are 13.8 PSF greater than that required by the Florida Building Code according to Mr. Disosway's calculations. Mr. Disosway's letter of opinion further states the window glass meets the 2004 Florida Building Code structural requirements. He did caution that our building inspector should make sure the installation is in compliance with Section 3CV of his letter. This section requires that the builder attach window jambs to wall studs with #8 by 3-inch wood screws at 12-inch OC maximum.

It appears the engineer's analysis letter and opinion meets the intent of Section 613 of the Florida Building Code regarding exterior windows. However, as an additional precaution of the county, I would suggest that the owner and contractor sign the enclosed indemnity and hold harmless agreement for the county. If you have any additional questions, please do not hesitate to give me a call.

Very truly yours,

*Martin M. Feagle*  
Martin M. Feagle

MMF:dse

Enclosure



“OWNER”

“CONTRACTOR”

Print Name: \_\_\_\_\_

ABRAM HUBER

ABRAM B. HUBER



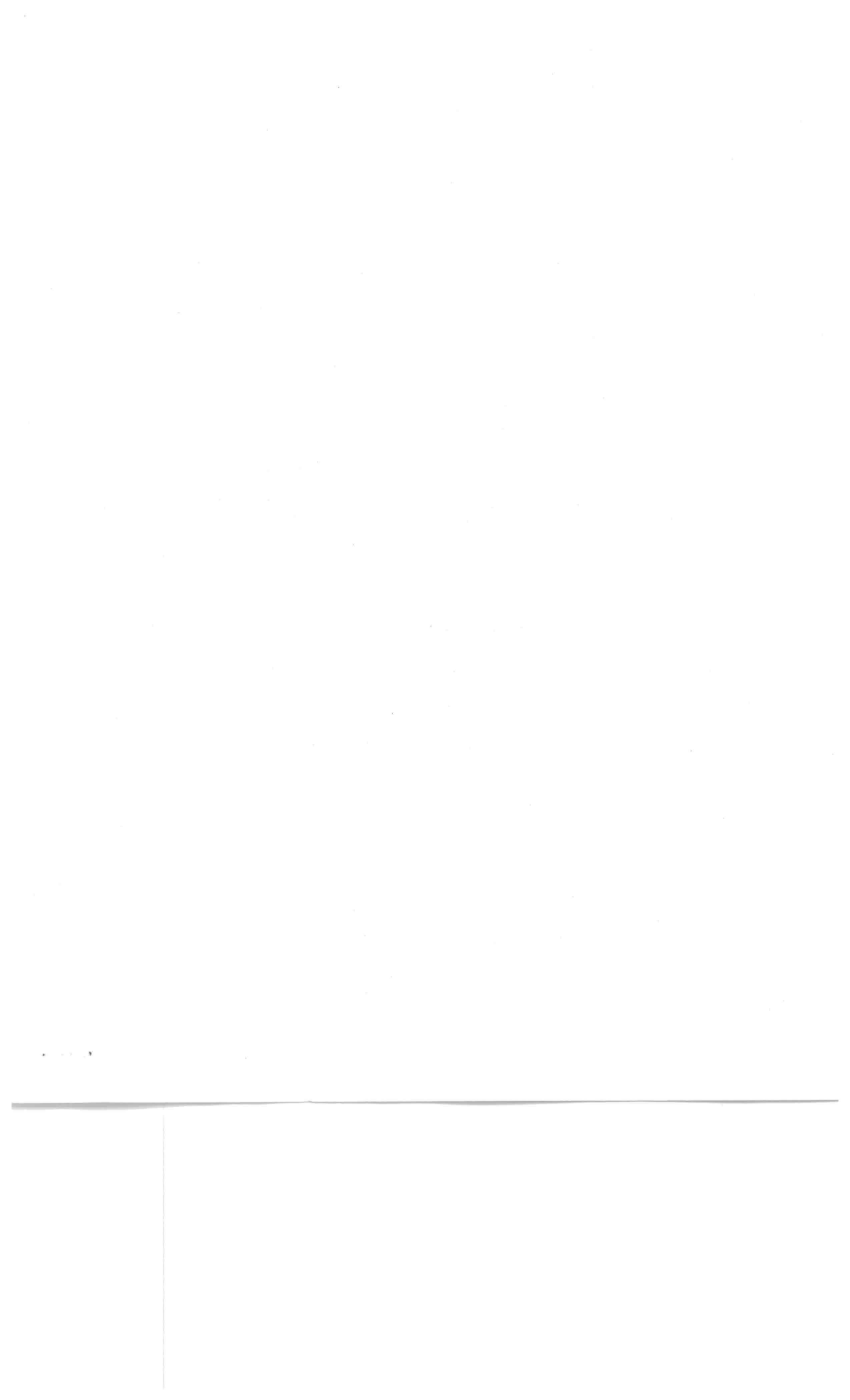
Print Name: ISAAC BARTOVICH



DATED this 15<sup>th</sup> day of June, 2006.

The undersigned, **ABRAM B. HUBER**, (“Owner”), and **ISAAC BARTOVICH**, (“Contractor”), each as to themselves, their heirs, successors and assigns, having been advised of the requirements of Section R613 of the Florida Building Code regarding exterior windows and door assemblies, and in consideration of the Columbia County Building Department approving the manufactured or custom-built doors and windows for the Owner’s residence located in Columbia County, Florida, do hereby release and agree to defend, reimburse, and hold harmless Columbia County, Florida, (“County”), regarding any claims, losses or damages against County as a result of the Owner’s residence not being constructed in full compliance with the provisions of the Florida Building Code; specifically as to Section R613 thereof. The undersigned represents and warrants that the residence has been and will be constructed in accordance with the Florida Building Code and other applicable rules and regulations.

**RELEASE AND HOLD HARMLESS AGREEMENT**



Mark Disosway

Wall Zone	Effective Wind Area (F12)	110 mph	110 mph
4	10	21.8	-23.6
4	20	20.8	-22.6
4	50	19.5	-21.3
4	100	18.5	-20.4
5	10	21.8	-29.1
5	20	20.8	-27.2
5	50	19.5	-24.6
5	100	18.5	-22.6

a) Design loads from Table R301.2(2) adjusted for height and exposure per Table R301.2(3) (Design wind speed = 100 mph, Exposure = B, Mean Roof Height = 30 ft, Coefficient = 1.00)

- 3) Engineering analysis
  - c) Doors are attached using #8X3" wood screws
  - b) Glass panels are double insulated low-e glass panels, each 1/8" thickness, with a 1/4" spacer gasket, for a total glass panel thickness of 1/2" all glass annealed except door glass panels which are tempered in accordance with building codes. Glass panels are attached using 1/16" polyethylene glazing tape and held in place at specified 25% compression with mechanically fastened muntins
  - a) Continuous flashing / bracket / flange 0.032 Aluminum attached to teak frame using 5/8" length 1/4" crown galv. staples fastened through polyurethane flashing glue into teak wood. This bracket extends 4" onto wall, and is fastened to wall every 12" using 1.75" #8 pan head wood screws.
- 2) Window assembly description
  - a) Florida Statute 553.842(10) states: "Products, other than manufactured buildings, which are custom fabricated or assembled shall not require separate approval under this section provided the component parts have been approved for the fabricated or assembled products use and the components meet the standards and requirements of the Florida Building Code which applies to the products intended use." The windows and doors are custom fabricated of two basic materials, glass and teak wood. This letter will demonstrate that the glass and teak for the windows and doors used in this house meet the requirements of FBC2004.
  - b) FBC2004, R613.4.2 Custom doors states: "Custom (one of a kind) exterior door assemblies shall be tested by an approved testing laboratory or be engineered in accordance with accepted engineering practices." This letter will demonstrate with accepted engineering practices that the windows and doors used in this house meet the requirements of FBC2004. We are asking the building department to accept approval of the custom windows also even though the code language is for doors.
- 1) Code basis for one time product approval of custom windows and doors.
 

Abram and Angela Huber are requesting one time local product approval for use of custom teak windows and doors for their new residence here in Columbia County. The owner's objective of using these custom products is to provide a more stable, rot and insect proof window and door to withstand the rigorous weather and tropical conditions here in Florida, and the desired aesthetic of which windows and doors are largely a part of in the house design.

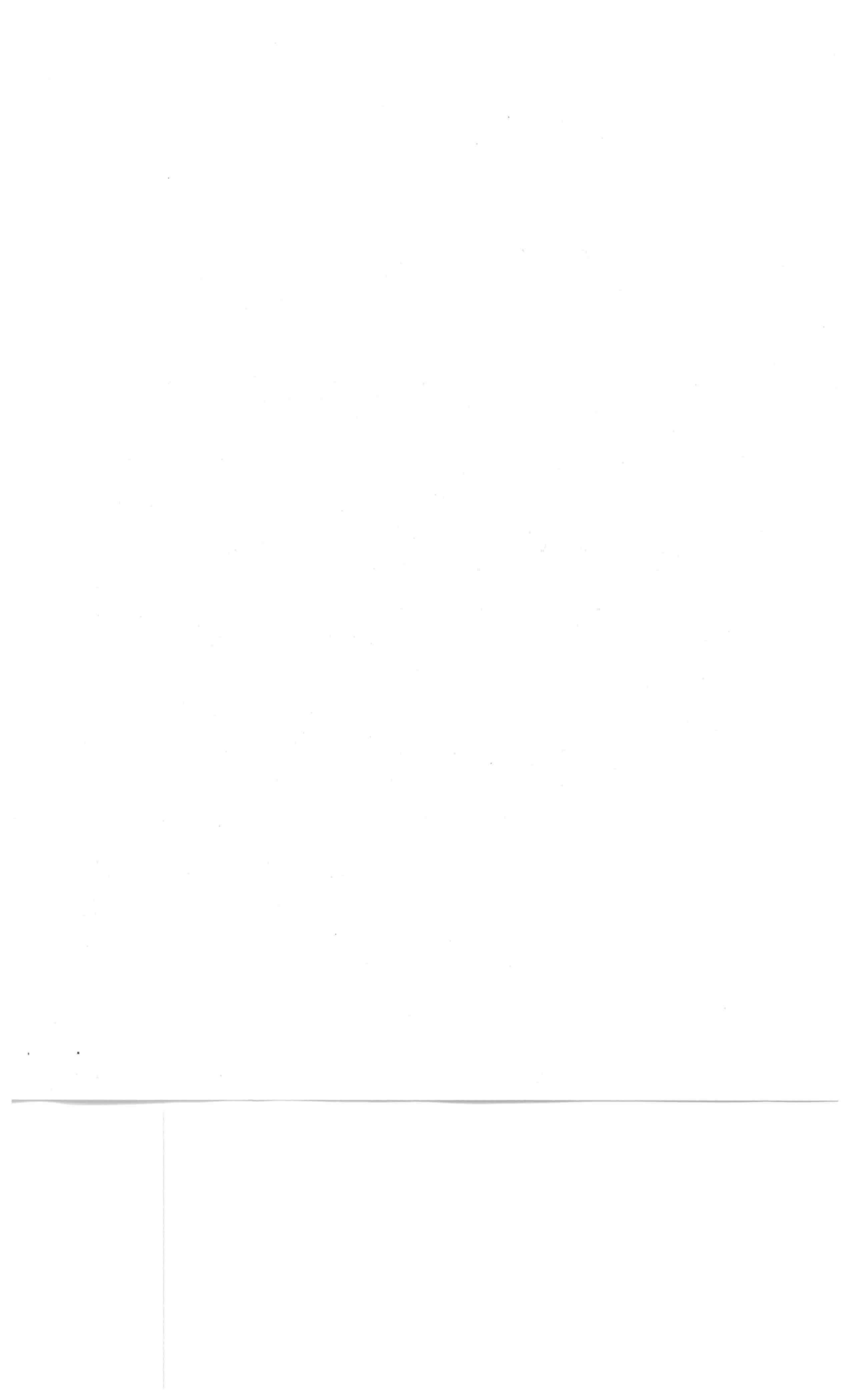
Dear Building Inspector:

Re: Columbia County Building Permit # 23867  
 One Time Product Approval for Custom Teak Windows and Doors  
 Abram and Angela Huber Residence, 01-5S-16-03387-012 Columbia County, Florida

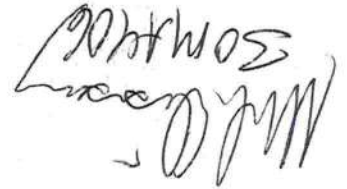
Building and Zoning, Columbia County, Florida

May 30, 2006

**Mark Disosway, P.E.**  
 POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4871



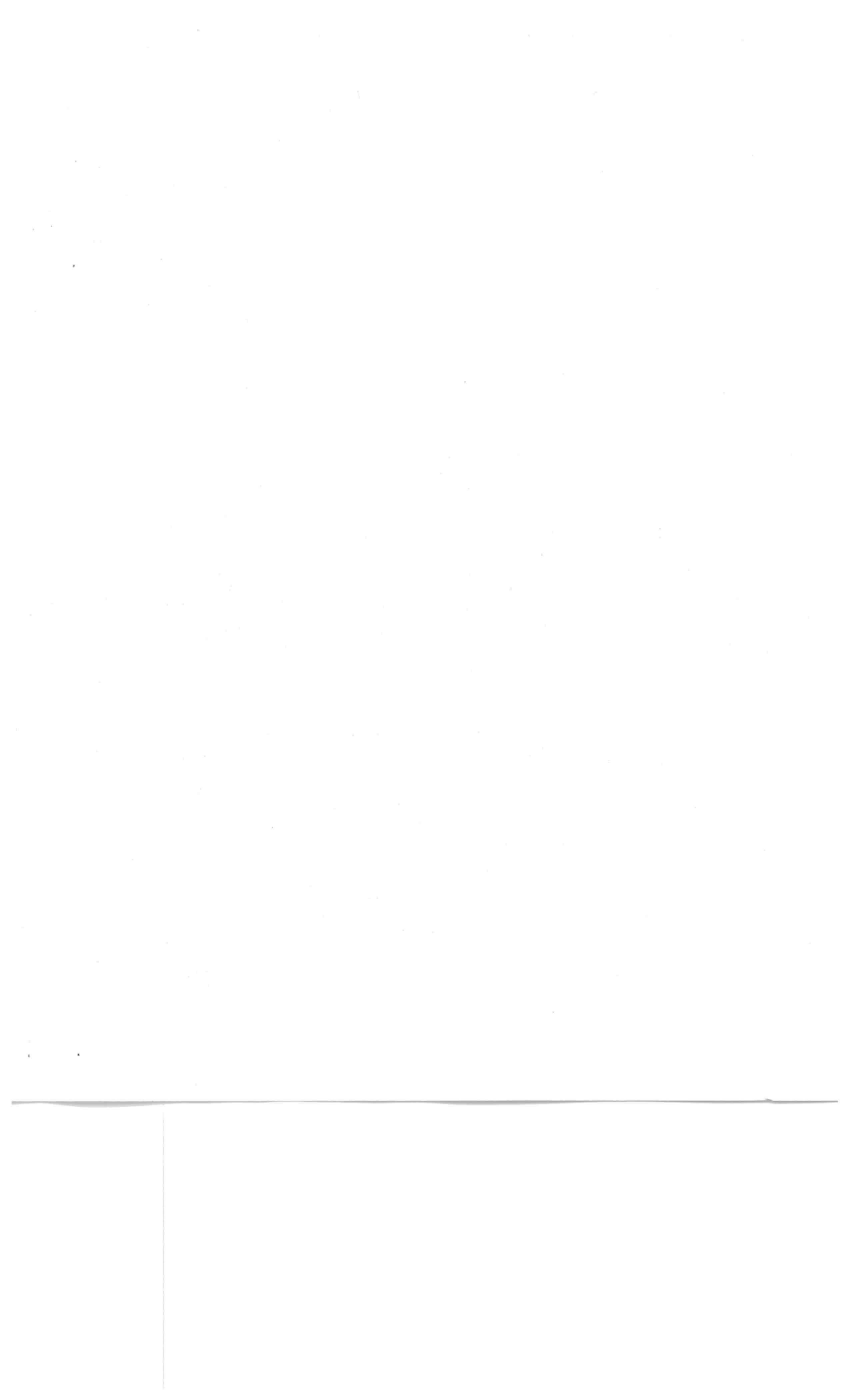
Mark Disosway, PE



Sincerely,

Please accept this letter as documentation that the custom one of a kind windows and doors used in this house meet the structural requirements of FBC2004 based on accepted engineering practices.

- b) Doors**
- i) Worst case wind pressure = -27.3 psf for 20 ft2 door in Zone 5.
  - ii) Worst case door structural element is the latch side stile which is simply supported from a latch mechanism to be installed by owner which latches at top and bottom of the stile. Due to the thin section of the stile deflection controls. Assume: 8" door height, latch at top and bottom of door, 2.0" x 5.0" stile, teak wood.
  - iii) Max deflection =  $WL^3 / 76.8EI = 27.3 \text{ psf} \cdot 8'' \cdot 1.5'' \cdot 96'' \cdot 3 / 76.8 \cdot 1700000 \text{ psi} \cdot 2'' \cdot 3 / 12 = 0.666'' = L/144$  is greater than code limits of L/180 for a window but the deflection specified for a door is that after 10 seconds of testing at 1.5 times design wind pressure there will be "no permanent deformation of any main frame or panel member in excess of 0.4 percent of its span after the load is removed. After each specified loading, there shall be no glass breakage, permanent damage to fasteners, hardware parts, or any other damage which causes the door to be inoperable." Based on experience I would expect a wood door to have no permanent deflection after 10 seconds at deflection of  $1.5 \times L/144 = L/96$ .
  - iv) Attach door jambs to wall studs with #8 x 3" wood screws at 12"OC max. (123 lb per screw / (1.5 x 27.3 psf x 3') = 1.0')
- c) Windows**
- i) Worst case wind pressure = -28 psf for 15 ft2 window in Zone 5.
  - ii) Window glass as described above meets FBC2004 structural requirements for 30 psf design pressure as shown in the ASTM E1300 calculation for the largest pane, attached.
  - iii) Worst case door structural element is the latch side or hinge side stile which is simply supported from a latch or hinge mechanism to be installed by owner with a maximum spacing between latches of 42". Due to the thin section of the stile deflection controls. Assume: 8" door height, latch at top and bottom of door, 2.0" x 2.5" stile, teak wood.
  - iv) Max deflection =  $WL^3 / 76.8EI = 28 \text{ psf} \cdot 3.5'' \cdot 1.5'' \cdot 42'' \cdot 3 / 76.8 \cdot 1700000 \text{ psi} \cdot 2'' \cdot 3 / 12 = 0.05'' = L/840$  is less deflection than code limits of L/180
  - v) Attach window jambs to wall studs with #8 x 3" wood screws at 12"OC max. (123 lb per screw / (1.5 x 27.3 psf x 3') = 1.0')



**Results**

### Double Glazed Insulating Unit

**Short Duration Load, Resistance and Deflection Data:**

Load (<=60 sec.): 30.0 psf  
 Load Resistance: 43.8 psf  
 Approximate Center of Glass Deflection under the Applied Load: 0.4 in.

Based on your design information, this glass configuration will resist the specified loading.

**OK**

**ASTM E 1300 Design**

**Class Construction**

- Double Glazed Insulating Unit
- Monolithic Single Glazing
- Laminated Single Glazing

**System of Units**

- US Standard
- SI

**Glazing Position**

- Vertical
- Sloped

**Rectangular Dimensions**

Width: 30.25 in.  
 Height: 60.5 in.

**Loads**

Short Duration (<=60 sec): 30 psf  
 Long Duration (app 30 days): 0 psf

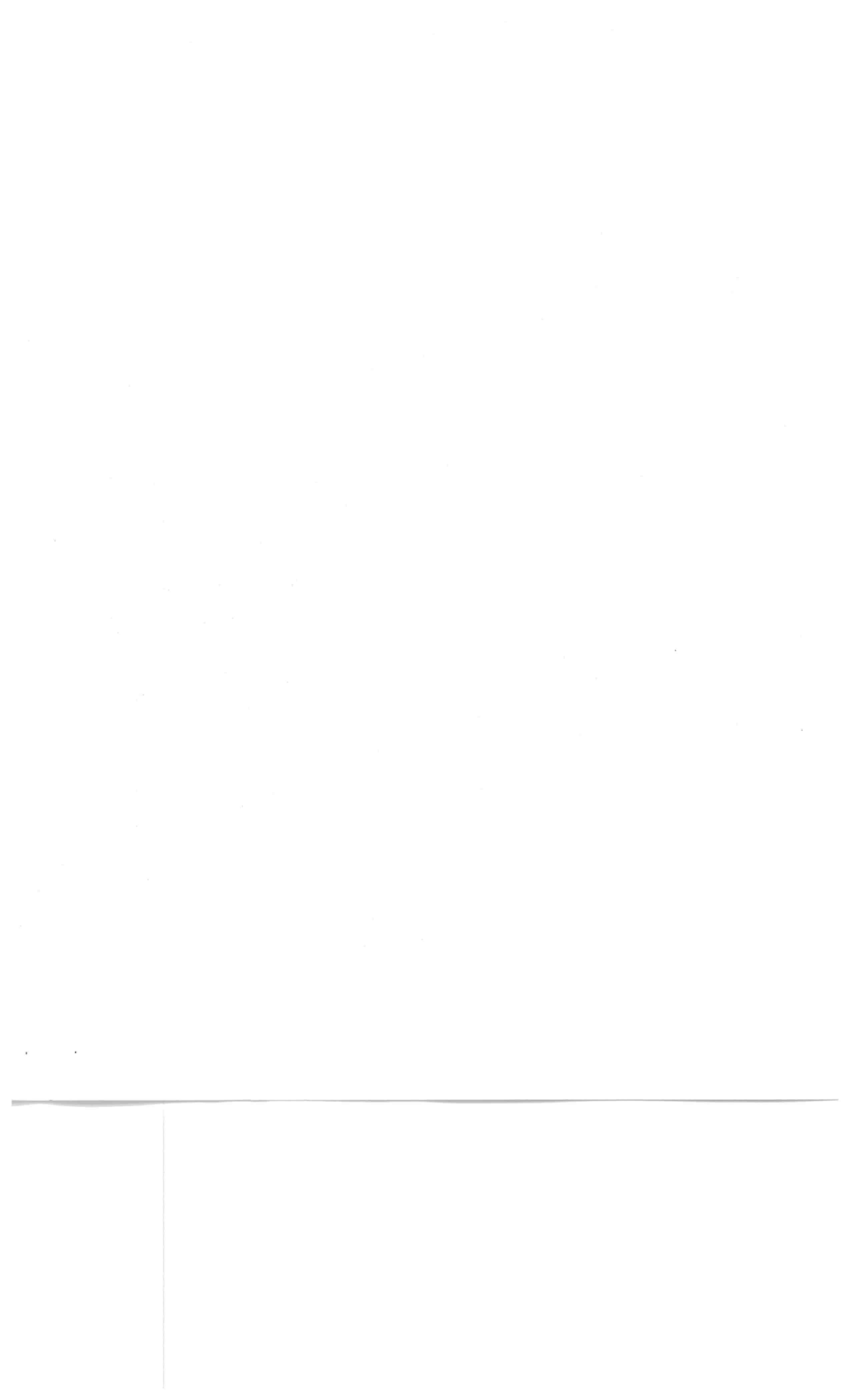
**Outboard Lite**

Check for Laminated  
 Glass Type: Annealed  
 Thickness Designation: 1/8 in.

**Inboard Lite**

Check for Laminated  
 Glass Type: Annealed  
 Thickness Designation: 1/8 in.

**Calculate**



SECTION R613

EXTERIOR WINDOWS AND DOOR ASSEMBLIES

R613.1 General.

This section prescribes performance and construction requirements for exterior window systems installed in wall systems. Waterproofing, sealing and flashing systems are not included in the scope of this section.

R613.2 Performance.

Exterior windows and doors shall be designed to resist the design wind loads specified in Table R301.2(2) adjusted for height and exposure per Table R301.2(3).

R613.3 Exterior windows, sliding and patio glass doors.

R613.3.1 Testing and Labeling.

Exterior windows and glass doors shall be tested by an approved independent testing laboratory, and shall be labeled with an approved label identifying the manufacturer, performance characteristics and approved product certification agency, testing laboratory, evaluation entity or Miami-Dade notice of acceptance to indicate compliance with the requirements of one of the following specifications:

ANSI/AAMA/NWDA 101/I.S. 2-97 or 101/I.S. 2/NAFS or TAS 202 (HVHZ shall comply with TAS 202 utilizing ASTM E 1300-98 or ASTM E 1300-02).

Glass Strength: Determination of load resistance of glass for specified loads of products tested and certified in accordance with s. R613.3.1 shall be designed to comply with ASTM E 1300.

R613.4 Exterior door assemblies.

Exterior door assemblies not covered by R613.3 or R613.4.1 shall be tested for structural integrity in accordance with ASTM E 330 Procedure A at a load of 1.5 times the required design pressure load. The load shall be sustained for 10 seconds with no permanent deformation of any main frame or panel member in excess of 0.4 percent of its span after the load is removed. HVHZ shall comply with TAS 202. After each specified loading, there shall be no glass breakage, permanent damage to fasteners, hardware parts, or any other damage which causes the door to be inoperable.

The minimum test sizes and minimum design pressures shall be as indicated in Table R613.4

The unit size tested shall qualify all units smaller in width and/or height of the same operation type and be limited to cases where frame, panels and structural members maintain the same profile as tested.

R308.5 Site built windows.

Site built windows shall comply with Section 2404 of the Florida Building Code, Building.

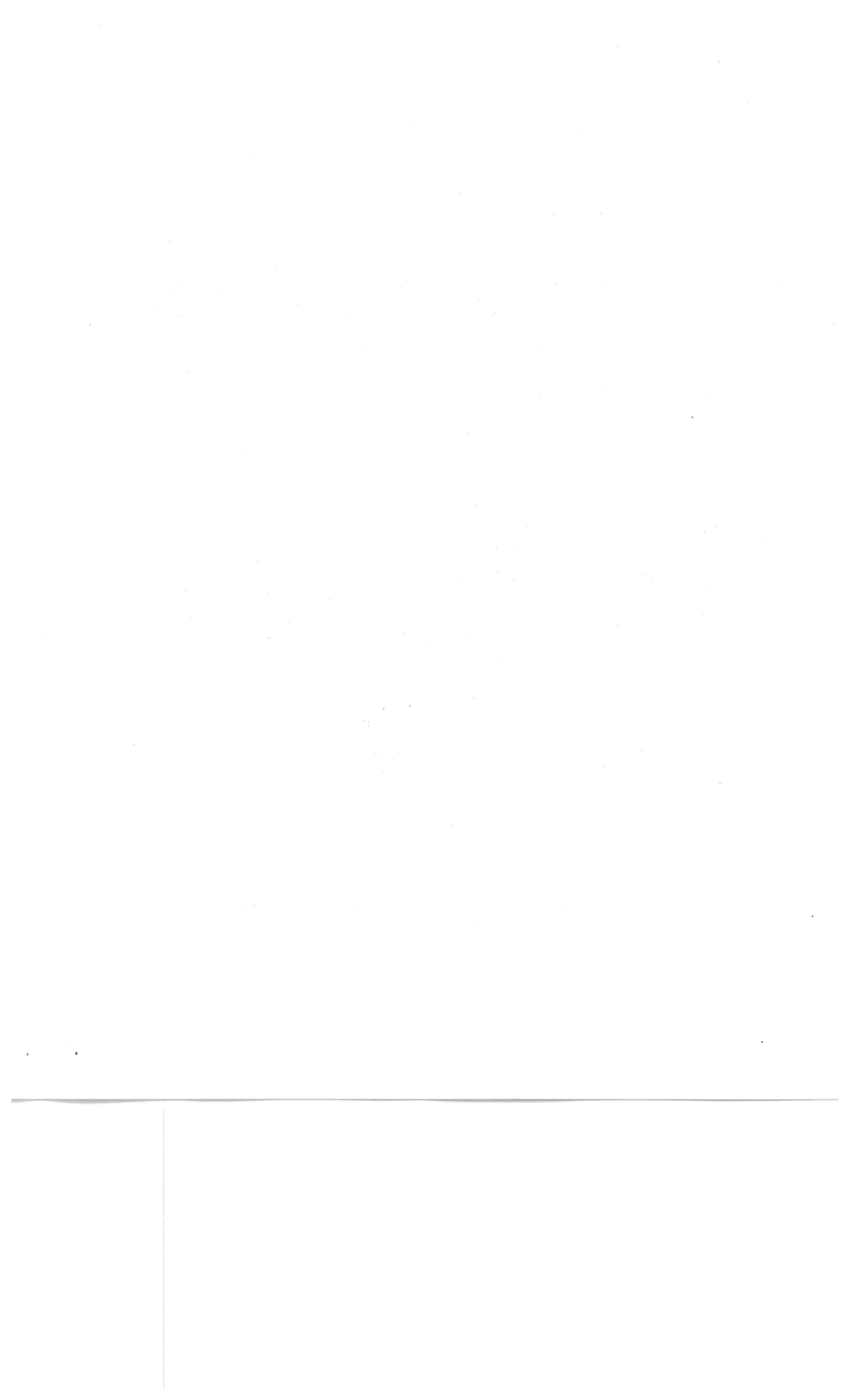
2404.1 Vertical glass.

Glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads for components and cladding. The load resistance of glass under uniform load shall be determined in accordance with ASTM E 1300. Design of exterior windows and glass doors in accordance with Section 2404.1 shall utilize the same edition of ASTM E 1300 used for testing in accordance with Section 1714.5. The design of vertical glazing shall be based on the following equation:

Fgw ≤ Fga (Equation 24-1)

where:

Fgw is the wind load on the glass computed in accordance with Section 1609 and Fga is the short duration load resistance of the glass as determined in accordance with ASTM E 1300.



(1) The commission shall adopt rules under ss. 120.36(1) and 120.54 to develop and implement a product evaluation and approval system that applies statewide to operate in coordination with the Florida Building Code. The commission may enter into contracts to provide for administration of the product evaluation and approval system. The product evaluation and approval system shall provide:

- (a) Appropriate promotion of innovation and new technologies.
- (b) Processing submittals of products from manufacturers in a timely manner.
- (c) Independent, third-party qualified and accredited testing and laboratory facilities, product evaluation entities, quality assurance agencies, certification agencies, and validation entities.
- (d) An easily accessible product acceptance list to entities subject to the Florida Building Code.
- (e) Development of stringent but reasonable testing criteria based upon existing consensus standards, when available, for products.
- (f) Long-term approvals, where feasible. State and local approvals will be valid until the requirements of the code on which the approval is based change, the product changes in a manner affecting its performance as required by the code, or the approval is revoked.
- (g) Criteria for revocation of a product approval.
- (h) Cost-effectiveness.
- (2) The product evaluation and approval system shall rely on national and international consensus standards, whenever adopted by the Florida Building Code, for demonstrating compliance with code standards. Other standards which meet or exceed established state requirements shall also be considered.

(3) Products or methods or systems of construction that require approval under s. 553.77, that have standardized testing or comparative or rational analysis methods established by the code, and that are certified by an approved product evaluation entity, testing laboratory, or certification agency as complying with the standards specified by the code shall be approved for statewide use. Products required to be approved for statewide use shall be approved by one of the methods established in subsection (6) without further evaluation.

(4) Products or methods or systems of construction requiring approval under s. 553.77 must be approved by one of the methods established in subsection (5) or subsection (6) before their use in construction in this state. Products may be approved by the commission for statewide use. Notwithstanding a local government's authority to amend the Florida Building Code as provided in this act, statewide approval shall preclude local jurisdictions from requiring further testing, evaluation, or submission of other evidence as a condition of using the product so long as the product is being used consistent with the conditions of its approval.

(5) Statewide approval of products, methods, or systems of construction may be achieved by one of the following methods. One of these methods must be used by the commission to approve the following categories of products: panel walls, exterior doors, roofing, skylights, windows, shutters, and structural components as established by the commission by rule.

(a) Products for which the code establishes standardized testing or comparative or rational analysis methods shall be approved by submittal and validation of one of the following reports or listings indicating that the product or system of construction was evaluated to be in compliance with the Florida Building Code and that the product or method or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code:

1. A certification mark or listing of an approved certification agency.
2. A test report from an approved testing laboratory.
3. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, from an approved product evaluation entity, or
4. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, developed and signed and sealed by a professional engineer or architect, licensed in this state.

A product evaluation report or a certification mark or listing of an approved certification agency which demonstrates that the product or system of construction complies with the Florida Building Code for the purpose intended shall be equivalent to a test report and test procedure as referenced in the Florida Building Code.

(b) Products, methods, or systems of construction for which there are no specific standardized testing or comparative or rational analysis methods established in the code may be approved by submittal and validation of one of the following:

1. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, from an approved product evaluation entity indicating that the product or method or system of construction was evaluated to be in compliance with the intent of the Florida Building Code and that the product or method or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code; or
2. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, developed and signed and sealed by a professional engineer or architect, licensed in this state, who certifies that the product or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code.

(6) The commission shall ensure that product manufacturers that obtain statewide product approval operate quality assurance programs for all approved products. The commission shall adopt by rule criteria for operation of the quality assurance programs.

(7) For state approvals, validation shall be performed by validation entities approved by the commission. The commission shall adopt by rule criteria for approval of validation entities, which shall be third-party entities independent of the product's manufacturer and which shall certify to the commission the product's compliance with the code.

(8) The commission may adopt rules to approve the following types of entities that produce information on which product approvals are based. All of the following entities, including engineers and architects, must comply with a nationally recognized standard demonstrating independence or no conflict of interest:

(a) Evaluation entities that meet the criteria for approval adopted by the commission by rule. The commission shall specifically approve the National Evaluation Service, the International Conference of Building Officials Evaluation Services, the International Code Council Evaluation Services, the Building Officials and Code Administrators International Evaluation Services, the Southern Building Code Congress International Evaluation Services, and the Miami-Dade County Building Code Compliance Office Product Control. Architects and engineers licensed in this state are also approved to conduct product evaluations as provided in subsection (5).

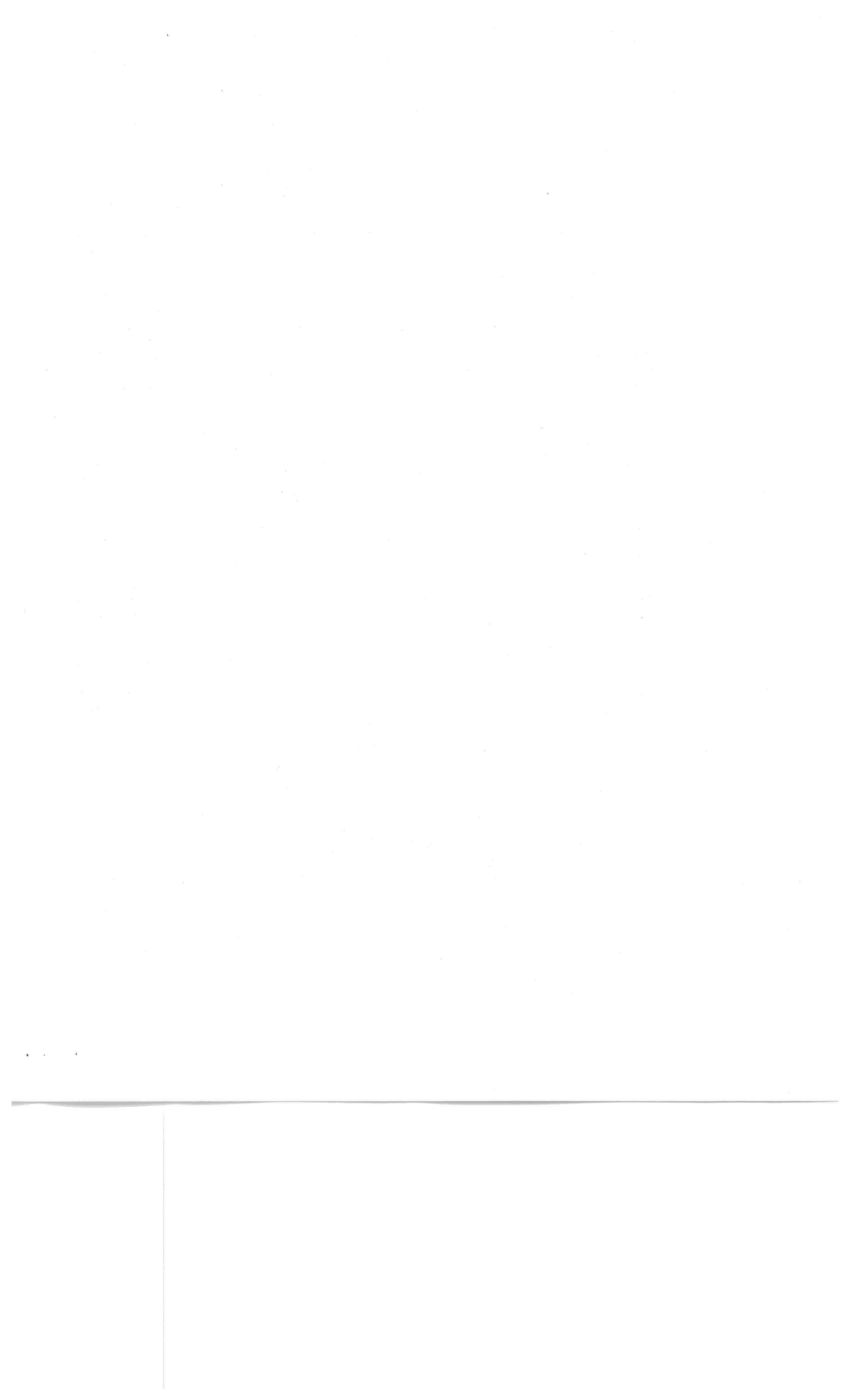
(b) Testing laboratories accredited by national organizations, such as A2LA and the National Voluntary Laboratory Accreditation Program, laboratories accredited by evaluation entities approved under paragraph (a), and laboratories that comply with other guidelines selected by the commission and adopted by rule.

(c) Quality assurance entities approved by evaluation entities approved under paragraph (a) and by certification agencies approved under paragraph (d) and other quality assurance entities that comply with guidelines selected by the commission and adopted by rule.

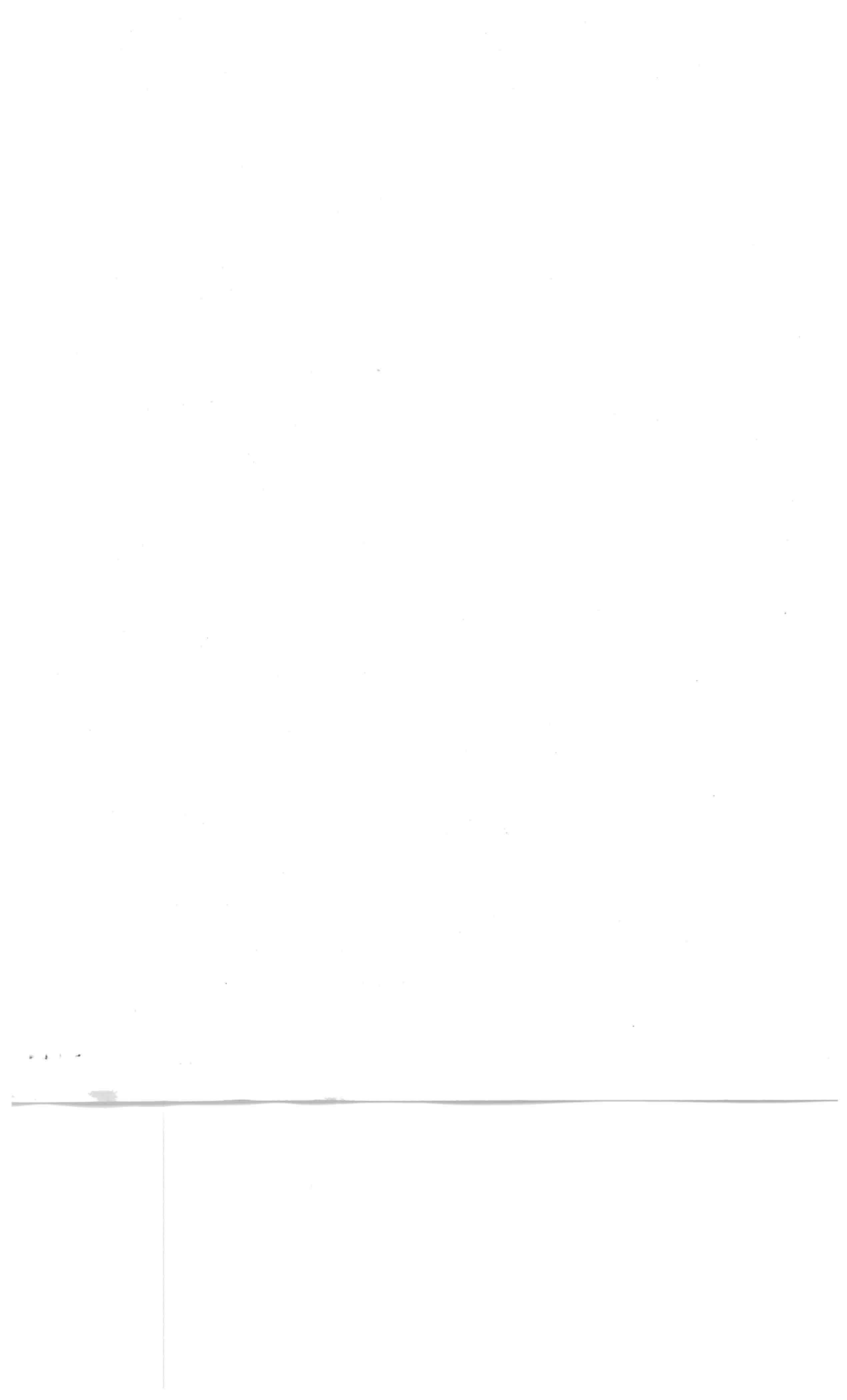
(d) Certification agencies accredited by nationally recognized accreditors and other certification agencies that comply with guidelines selected by the commission and adopted by rule.

(e) Validation entities that comply with accreditation standards established by the commission by rule.

(9) A building official may deny the local application of a product or method or system of construction which has received statewide approval, based upon a written report signed by the official that concludes the product application is inconsistent with the statewide approval and that states the reasons the application is inconsistent. Such denial is subject to the provisions of s. 553.77 governing appeal of the building official's interpretation of the code.



(10) Products, other than manufactured buildings, which are custom fabricated or assembled shall not require separate approval under this section provided the component parts have been approved for the fabricated or assembled products use and the components meet the standards and requirements of the Florida Building Code which applies to the products intended use.



**COLUMBIA COUNTY BUILDING DEPARTMENT**

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR**

**ONE (1) AND TWO (2) FAMILY DWELLINGS**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL**

**GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:**

- Applicant
- Plans Examiner
- All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
- Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.

**Site Plan including:**

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

- a) Plans or specifications must state compliance with FBC Section 1606
- b) Wind-load Engineering Summary, calculations and any details required

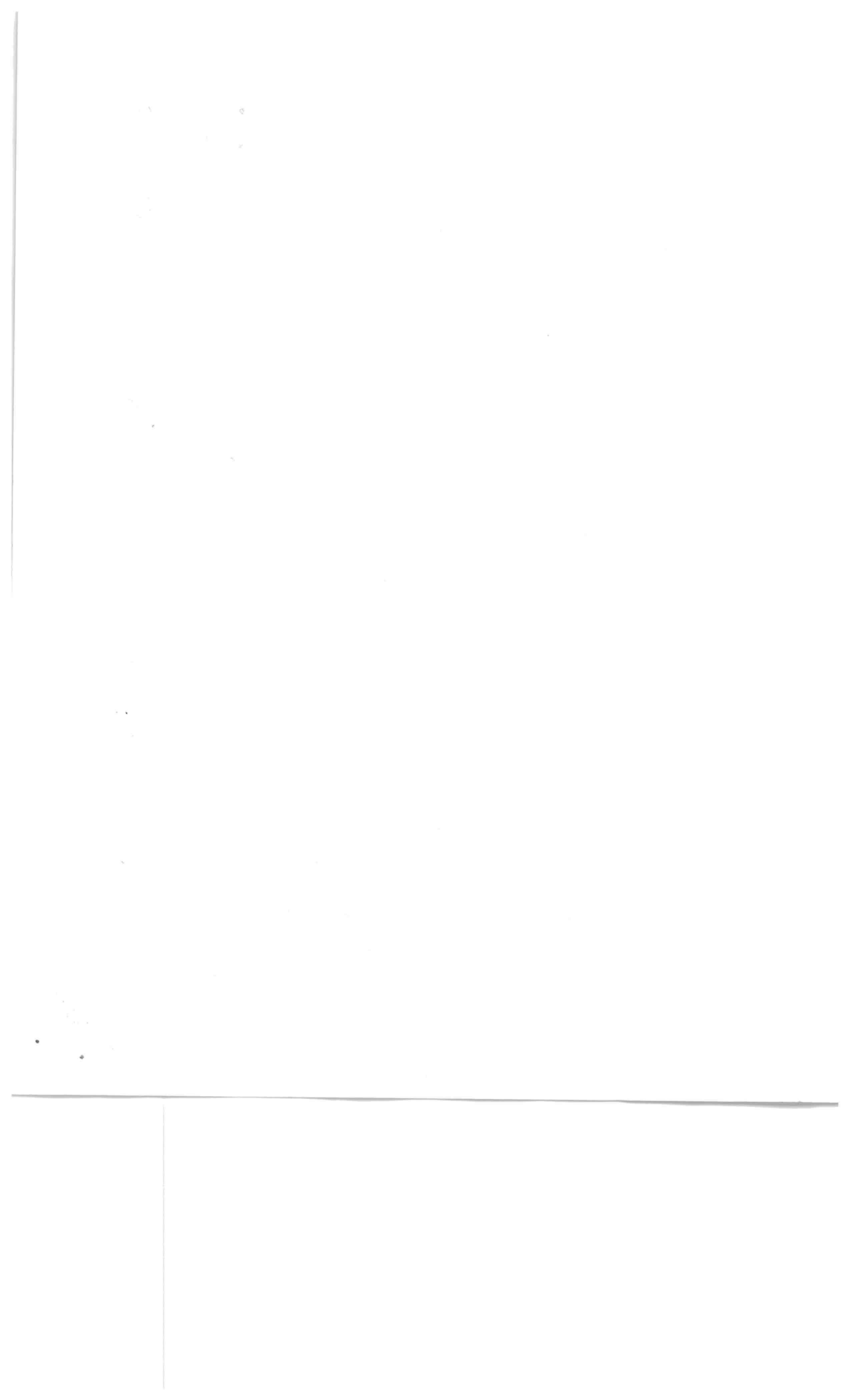
- a) Basic wind speed (MPH)
- b) Wind importance factor (I) and building category
- c) Wind exposure - If more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- d) The applicable internal pressure coefficient
- e) Components and Cladding. The design wind pressure in terms of psf (kN/m<sup>2</sup>), to be used for the design of exterior component and cladding materials not specially designed by the registered design professional

**Elevations including:**

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

See Note 2

*Handwritten signature*



1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termicide or alternative method)
10. Slab on grade
  - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
  - b. Must show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
  - a. Attic space
  - b. Exterior wall cavity
  - c. Crawl space (if applicable)

**Wall Sections including:**

- a) Masonry wall
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- a) Truss package including:

**Roof Systems:**

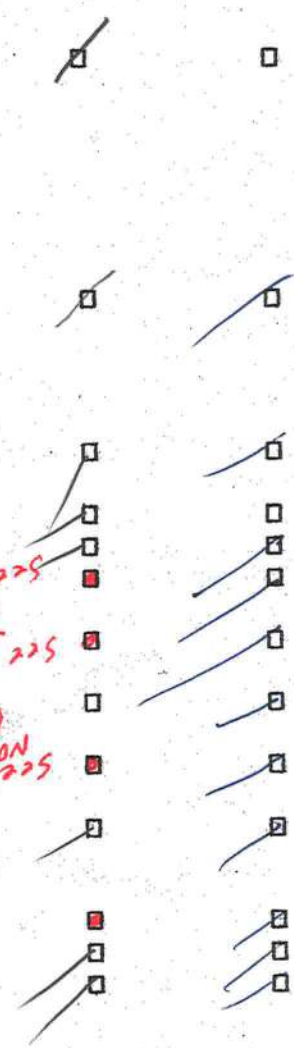
- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

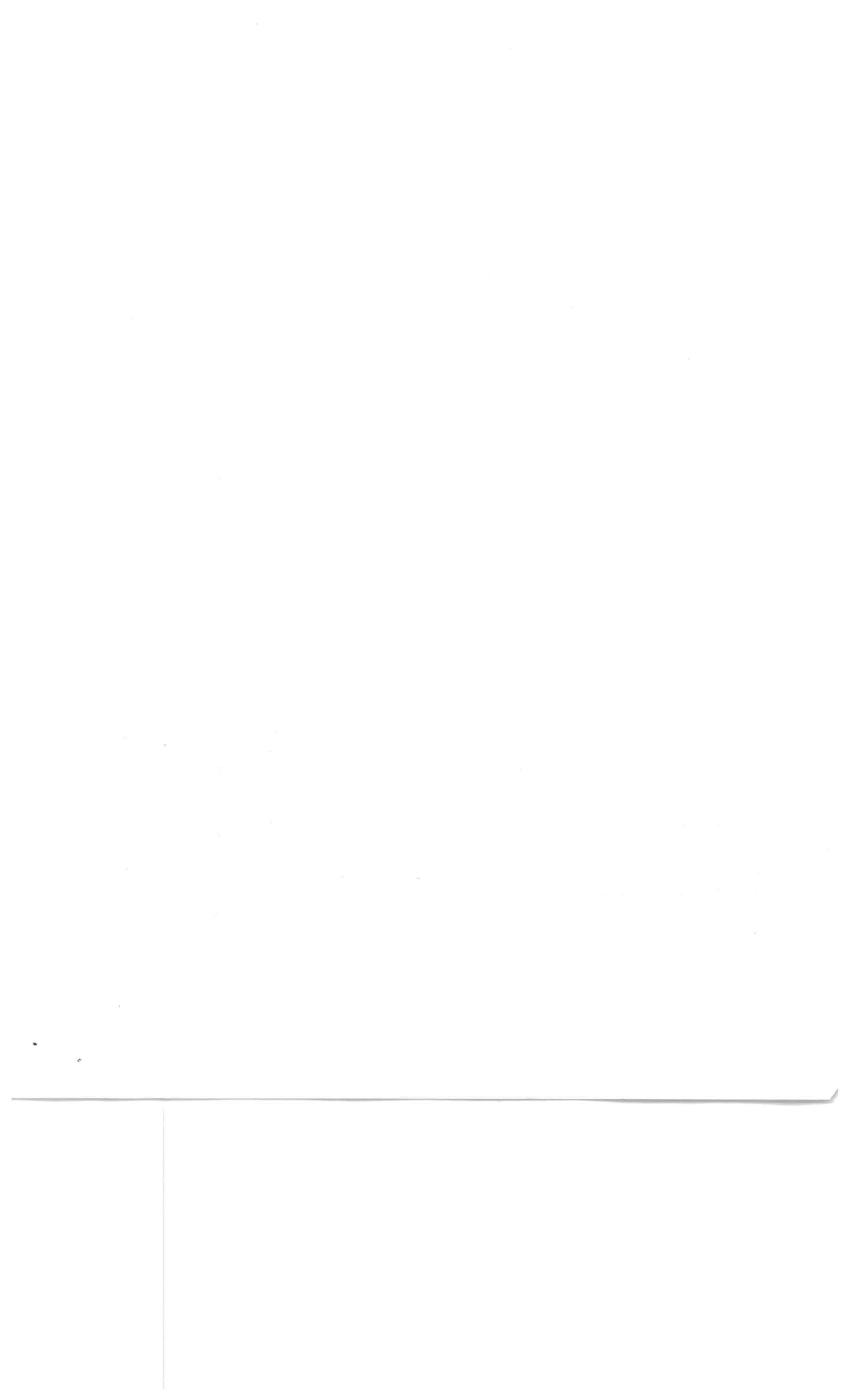
**Foundation Plan including:**

- f) Must show and identify accessibility requirements (accessible bathroom)
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown) **NOTE 3**
- b) Shear walls
- a) Rooms labeled and dimensioned

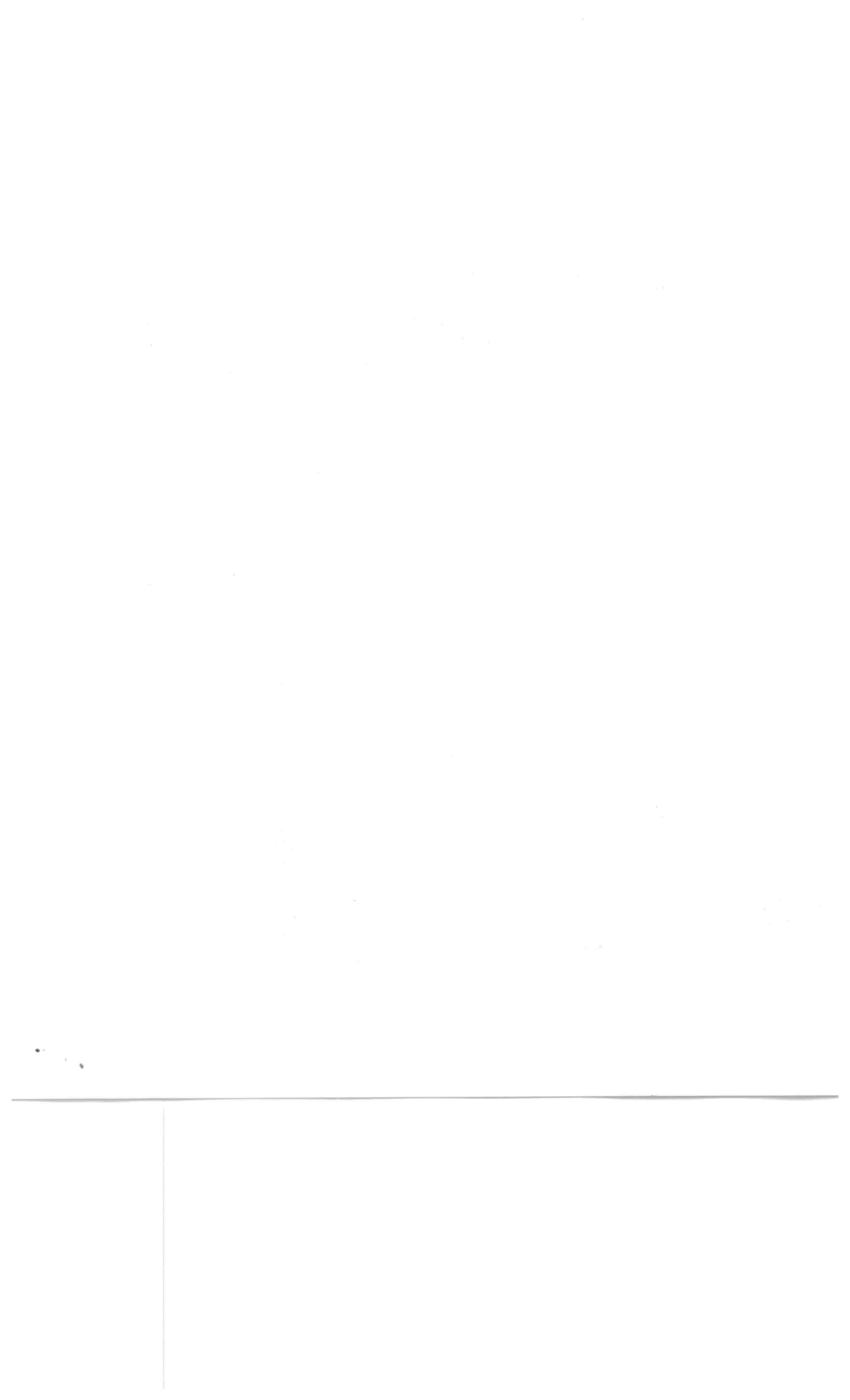
**Floor Plan including:**

*NOTE 4*  
*NOTE 7*  
*NOTE 6*  
*NOTE 7*  
*NOTE 8*  
 MR STR  
 13/17/14











*1 APR 2002*

MAH:nb

Mark A. Hess, Technician

*Mark A. Hess*

For ARCHITECTURAL TESTING, INC.

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

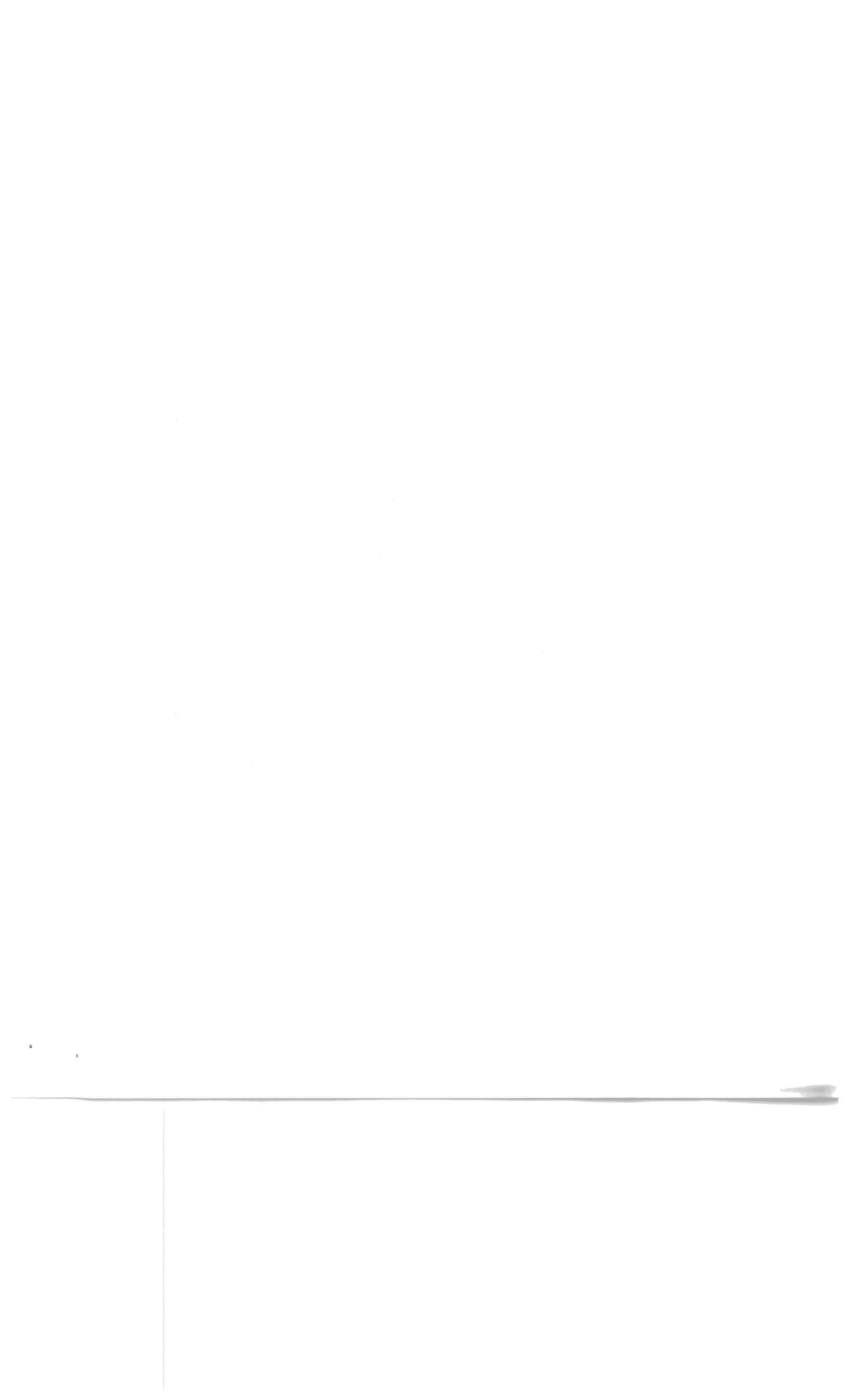
Title of Test	Results
Rating	H-R40 52 X 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cm <sup>3</sup> /ft <sup>2</sup>
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
De-glazing	Passed
Forced Entry Resistance	Grade 10

TYPE: Aluminum Single Hung Window  
 SERIES/MODEL: 650 FM

MI HOME PRODUCTS, INC.

Rendered to:

TEST REPORT SUMMARY  
 AAMA/NWDA 101/S.2-97





Alan M. R...  
1 APRIL 2002

130 Derry Court  
York, PA 17402-9405  
Phone: 717.764.7700  
Fax: 717.764.4125  
www.archtest.com

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced gully spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Finish: All aluminum was white.

Screen Size: 4'-0-1/4" wide by 2'-11-1/8" high

Daylight Opening Size: 3'-11-3/8" wide by 2'-9-1/2" high

Active Sash Size: 4'-1-3/4" wide by 3'-0-5/8" high

Overall Size: 4'-4-1/4" wide by 6'-0-3/8" high

Type: Aluminum Single Hung Window

Series/Model: 650 Fin

Test Specimen Description:

Doors

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/LS-2-97, Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethtown, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Report No: 01-41134-01  
Test Date: 03/07/02  
Report Date: 03/26/02  
Expiration Date: 03/07/06

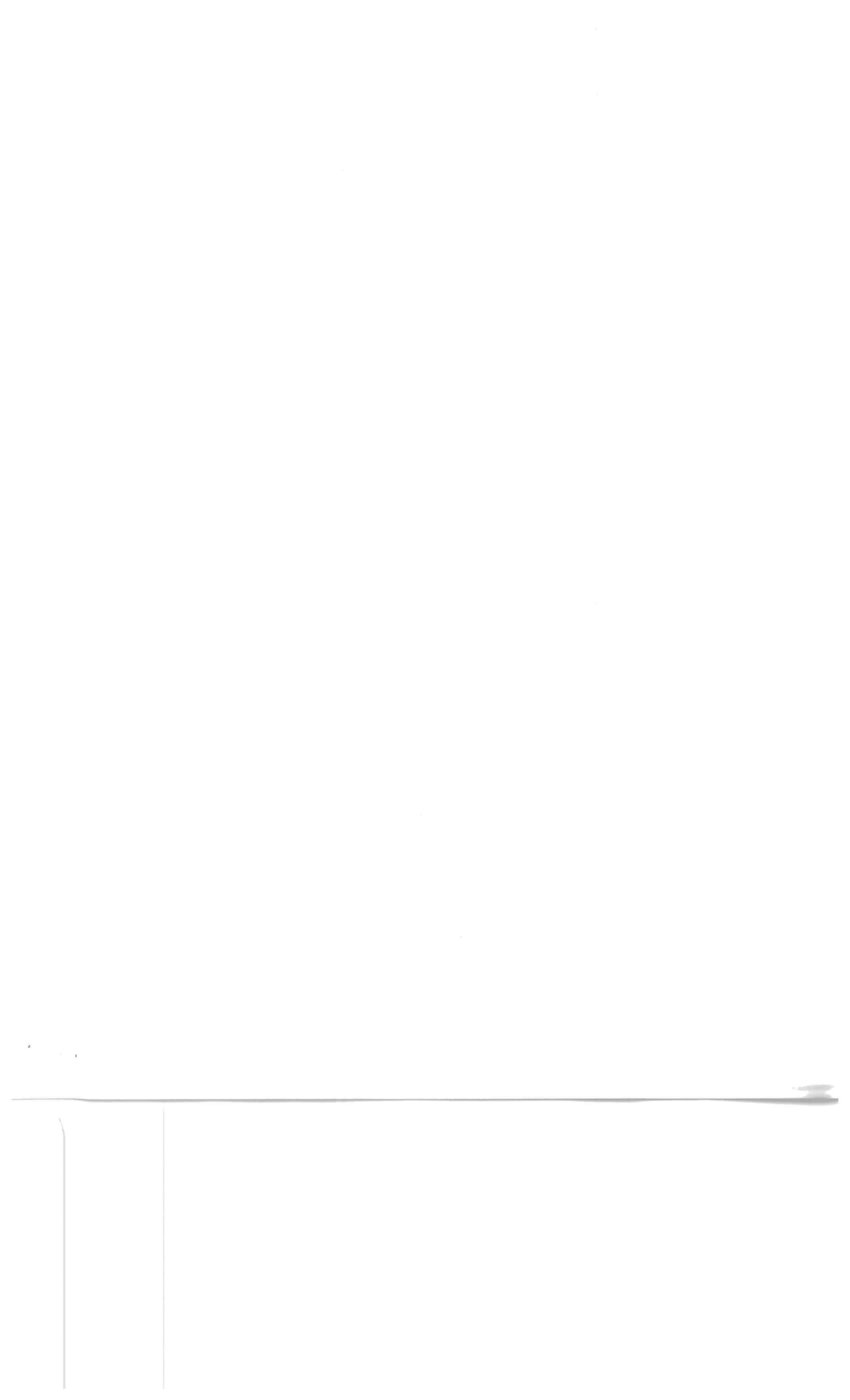
MI HOME PRODUCTS, INC.  
650 West Market Street  
P.O. Box 370  
Gatz, Pennsylvania 17030-0370

Rendered to

AAMA/NWDA 101/LS-2-97 TEST REPORT

Architectural Testing







Allen H. Reeves  
1 APRIL 2002

Description	Quantity	Location
Screen plunger	2	4" from rail ends on top rail
Balance assembly	2	One in each jamb
Metal tilt pin	2	Active sash, bottom rail ends
Plastic tilt latch	2	Active sash, meeting rail ends
Metal cam lock with keeper		Midspan, sash meeting rail with keeper adjacent on fixed meeting rail

**Hardware:**

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, beveled and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

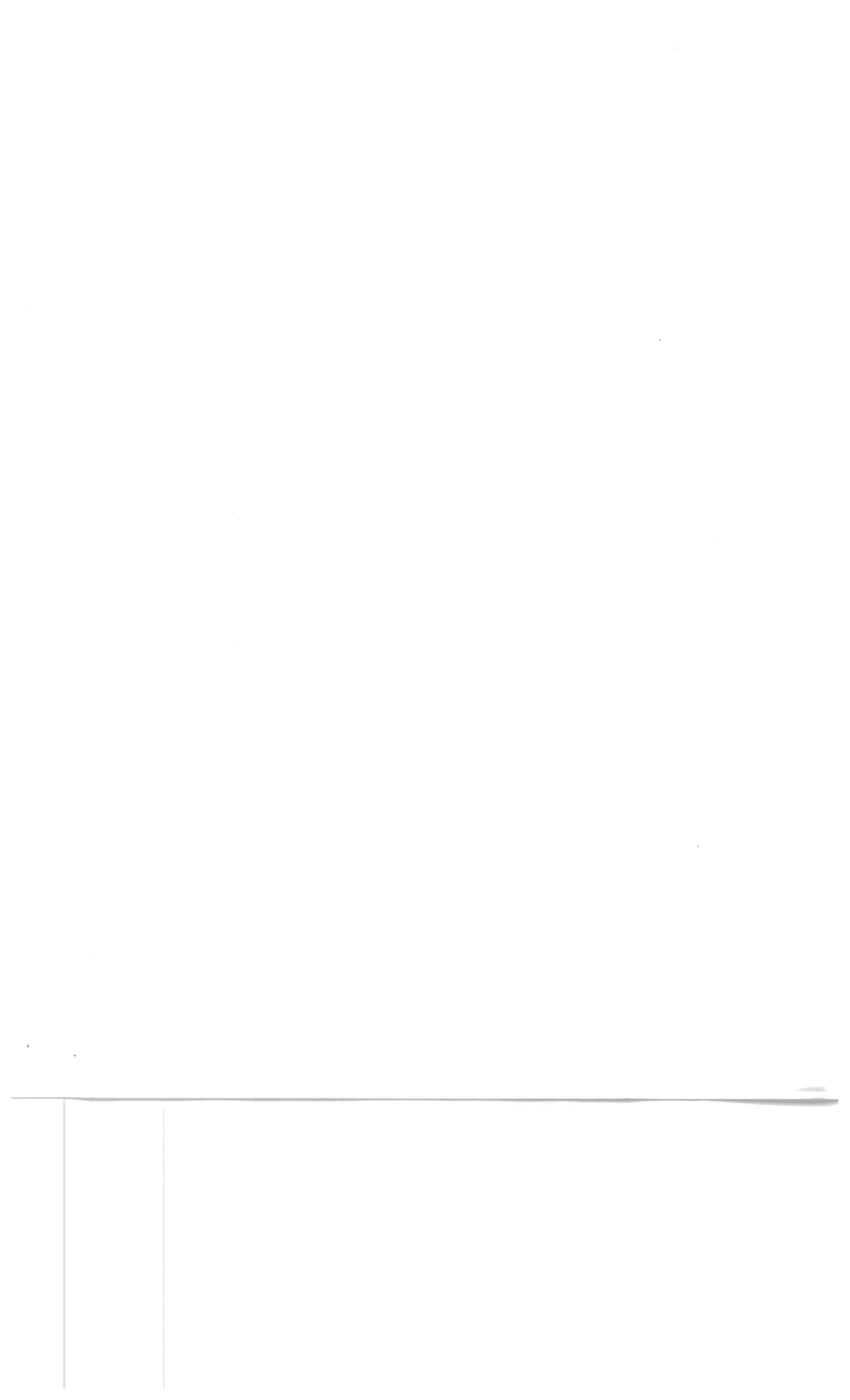
**Frame Construction:** The frame was constructed of extruded aluminum with coped, beveled, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Description	Quantity	Location
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
0.250" high by 0.187" backed polyfyle with center fin	2 Rows	Active sash stiles
0.230" high by 0.270" backed polyfyle with center fin	1 Row	Fixed meeting rail

**Weatherstripping:**

Test Specimen Description: (Continued)

III





*Allen H. Reeves*  
1 APR 12 2002

2.14.2	Uniform Load Structural (ASTM E 330-97)	(Measurements reported were taken on the meeting rail)	(Loads were held for 10 seconds)	@ 38.9 psf (positive)	@ 52.1 psf (negative)	0.02"	0.02"	0.18" max	0.18" max
*Exceeds L/175 for deflection, but passes all other test requirements.									
2.14.1	Uniform Load Deflection (ASTM E 330-97)	(Measurements reported were taken on the meeting rail)	(Loads were held for 33 seconds)	@ 25.9 psf (positive)	@ 34.7 psf (negative)	0.42"	0.43"	0.26" max	0.26" max
	Water Resistance (ASTM E 547-00)	(with and without screen)	WTP = 2.86 psf	No leakage					

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/S-2-97 for air infiltration.

2.2.16.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91)	@ 1.57 psf (25 mph)	0.13 cfm/ft <sup>2</sup>
			0.3 cfm/ft <sup>2</sup> max

The results are tabulated as follows:

**Test Results:**

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

**Reinforcement:** No reinforcement was utilized.

**Drainage:** Sloped sill

**Test Specimen Description: (Continued)**

*IV*





ALL N. REEKS  
APRIL 2002

Uniform Load Structural (ASTM E 330-97)  
 (Measurements reported were taken on the meeting rail)  
 Loads were held for 10 seconds)  
 @ 67.5 psf (positive)  
 @ 70.8 psf (negative)  
 0.05"  
 0.05"

\*Exceeds L/175 for deflection, but passes all other test requirements.  
 Uniform Load Deflection (ASTM E 330-97)  
 (Measurements reported were taken on the meeting rail)  
 Loads were held for 33 seconds)  
 @ 45.0 psf (positive)  
 @ 47.2 psf (negative)  
 0.47"  
 0.46"  
 0.26" max.  
 0.26" max.

Water Resistance (ASTM E 547-00)  
 (with and without screen)  
 WTP = 6.00 psf  
 No leakage  
 No leakage

43

Optional Performance

Lock Manipulation Test  
 Tests A1 through A5  
 Test A7  
 Lock Manipulation Test  
 No entry  
 No entry  
 No entry  
 No entry  
 No entry  
 No entry

Type: A  
 Grade: 10  
 Forced Entry Resistance (ASTM F 588-97)

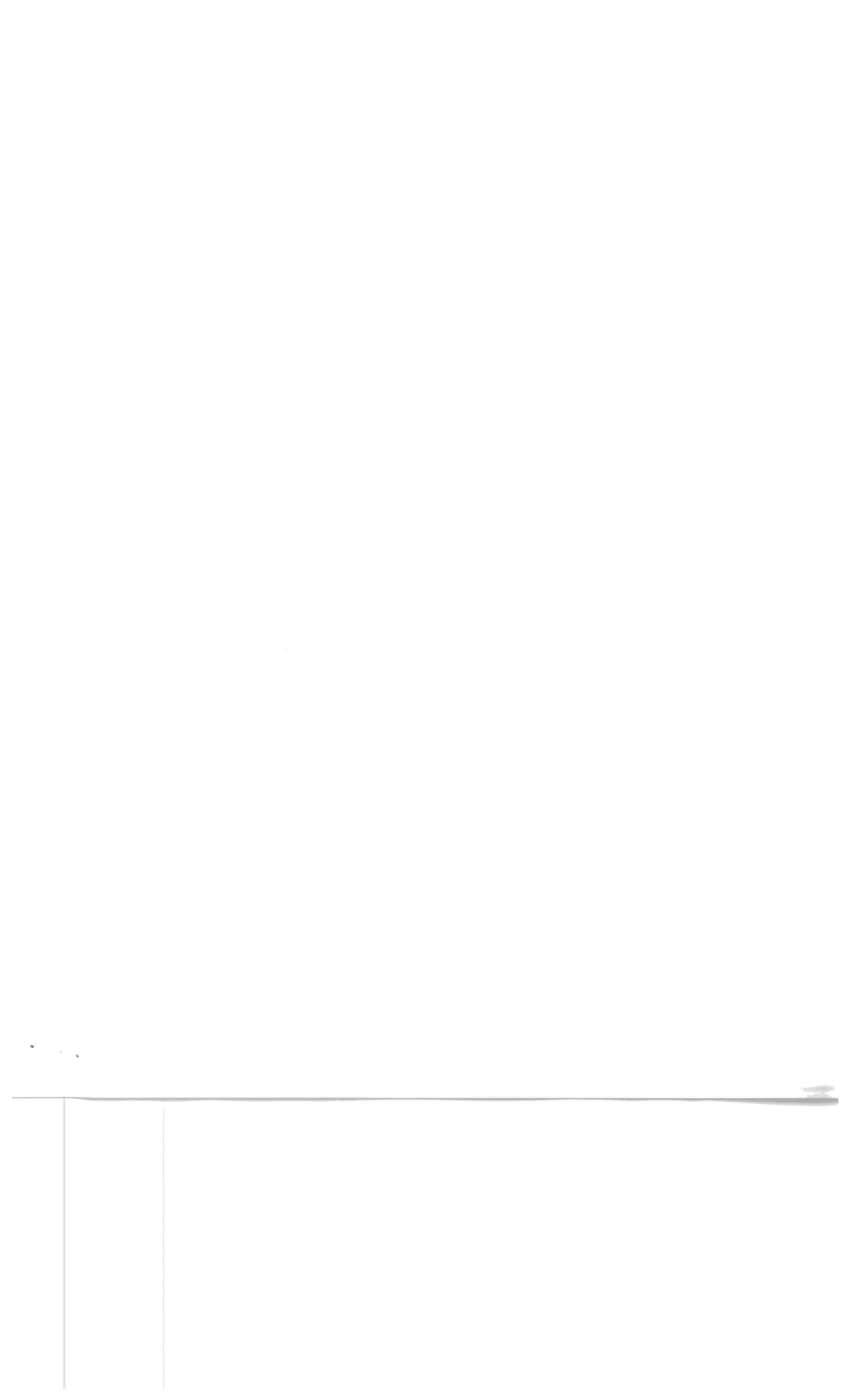
Meeting rail  
 Bottom rail  
 Left stile  
 Right stile  
 In remaining direction at 50 lbs  
 In operating direction at 70 lbs  
 0.12"/25%  
 0.12"/25%  
 0.06"/12%  
 0.06"/12%  
 0.50"/100%  
 0.50"/100%  
 0.50"/100%  
 0.50"/100%

2.2.1.6.2

Parameter Title of Test - Test Method Results Allowed

Test Specimen Description: (Continued)

IV





01-41134.01  
MAR:mb

Technician

Mark A. Hess

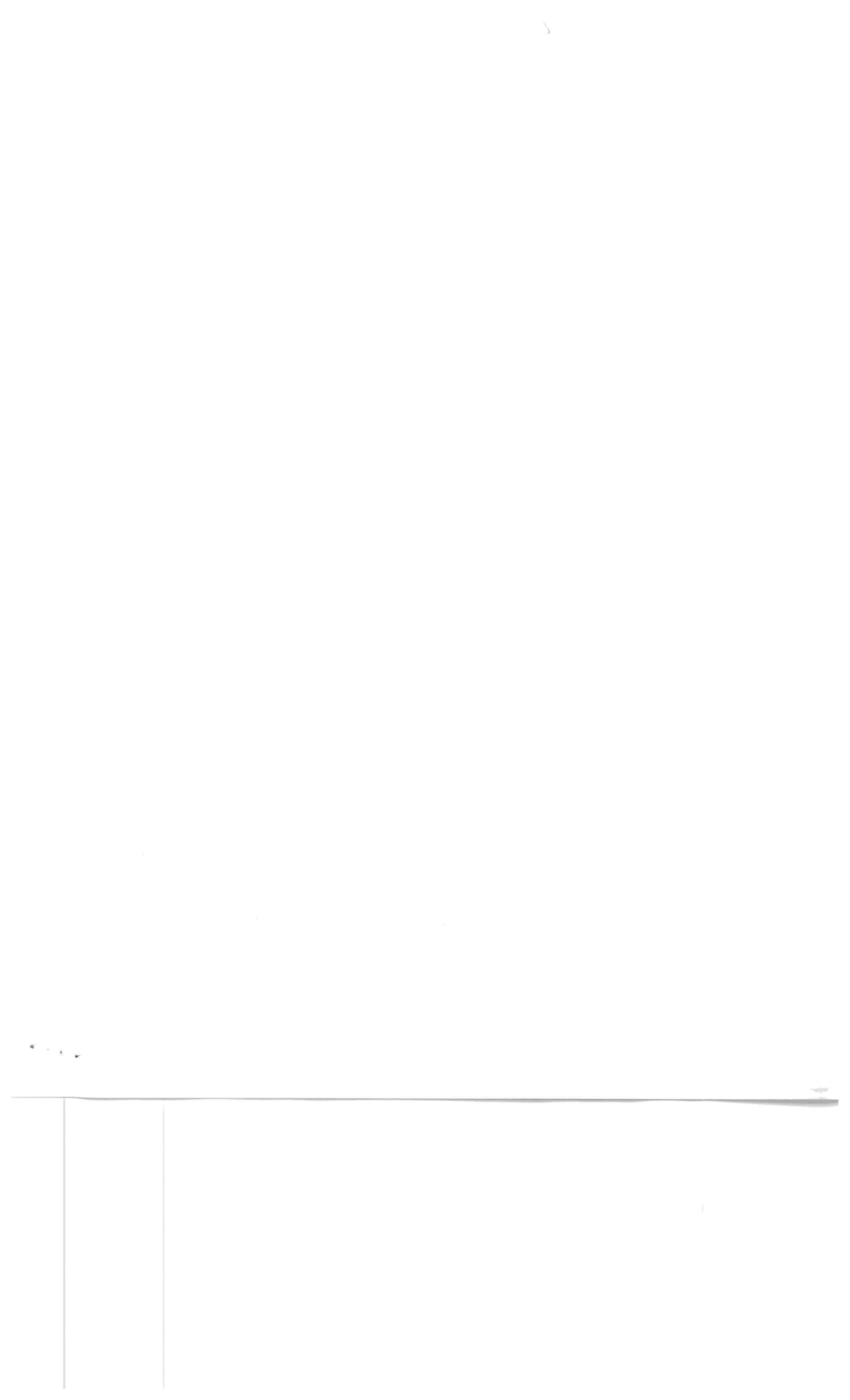
Allen N. Reeves, P.E.  
Director - Engineering Services  
1 APRIL 2002

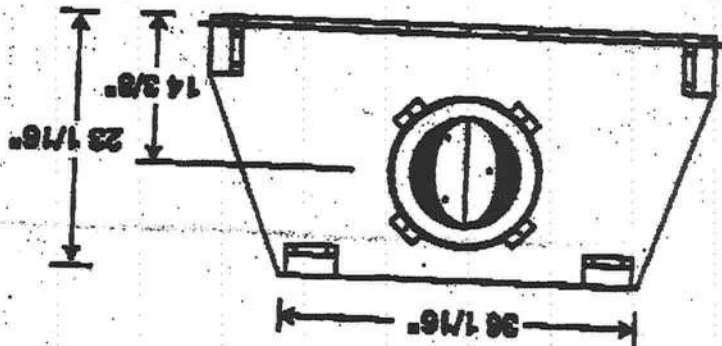
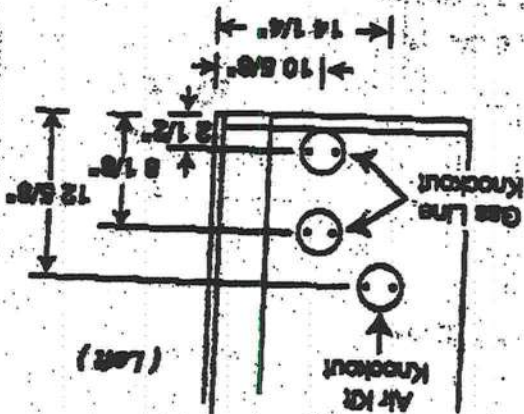
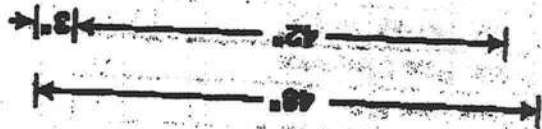
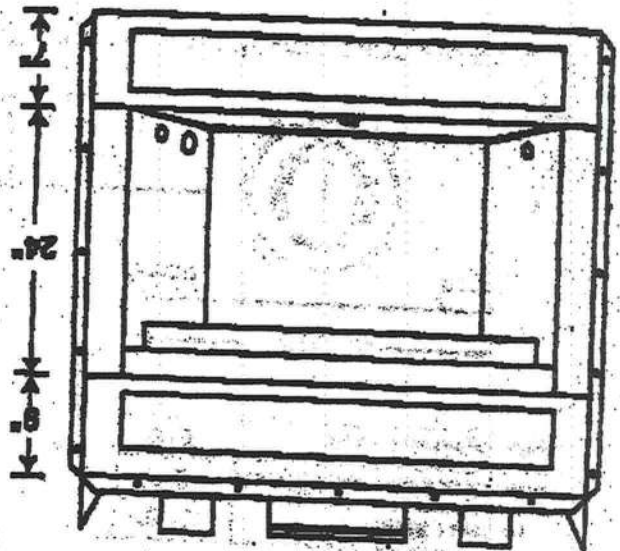
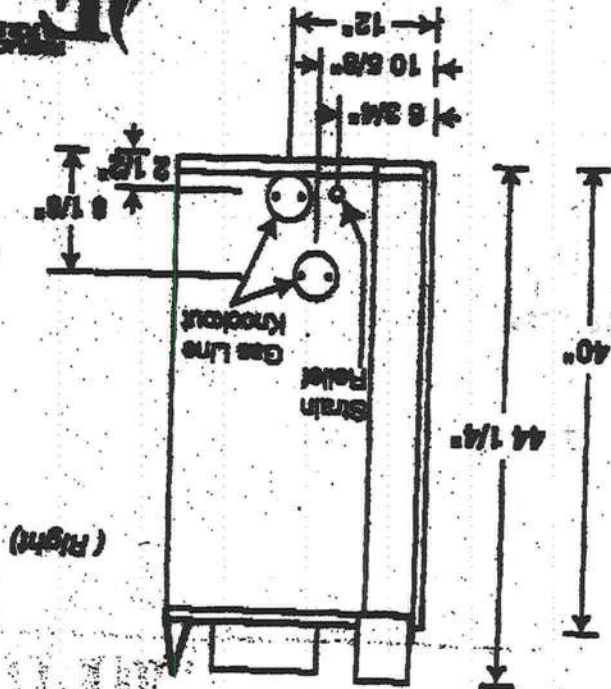
For ARCHITECTURAL TESTING, INC.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.



VI

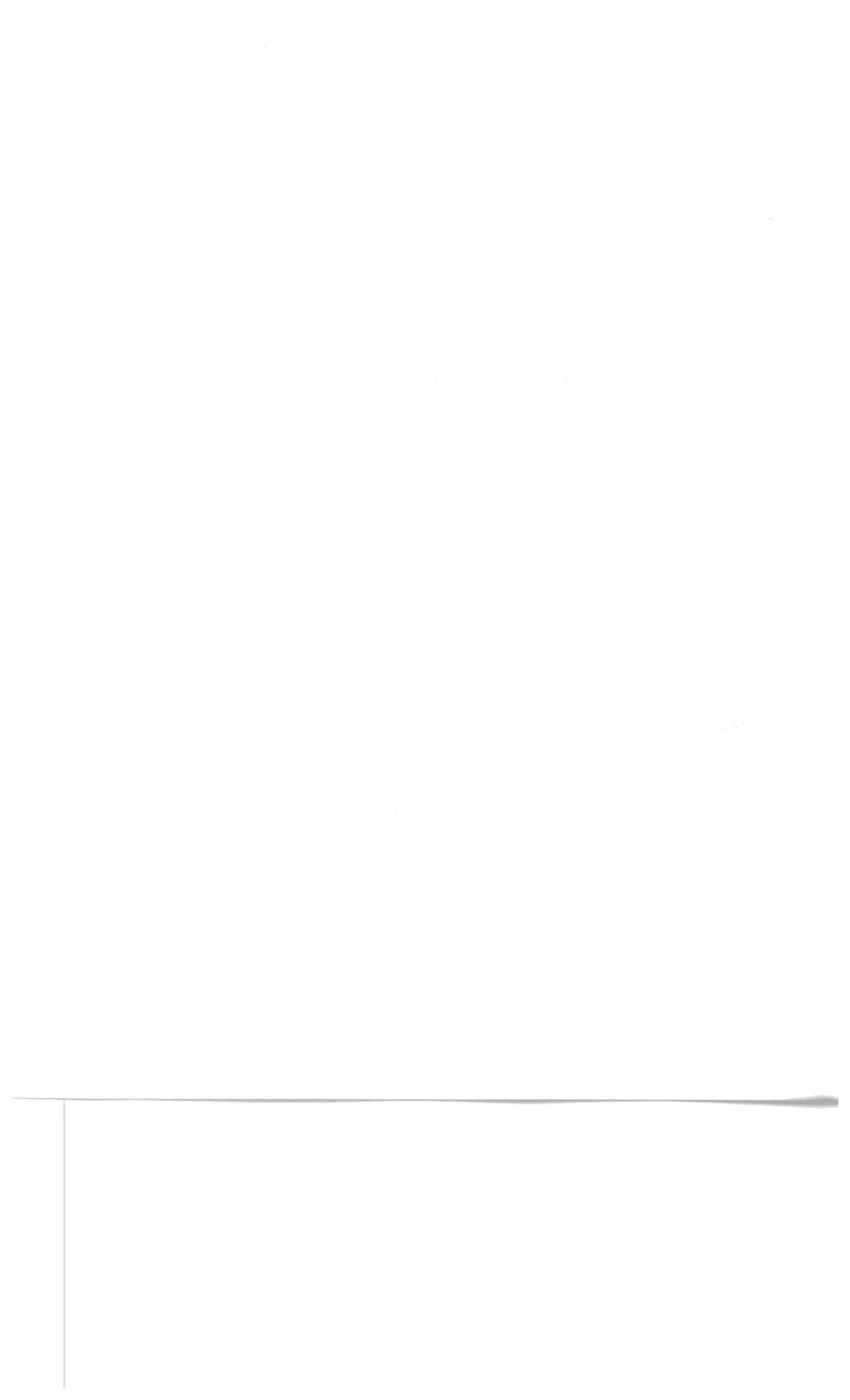


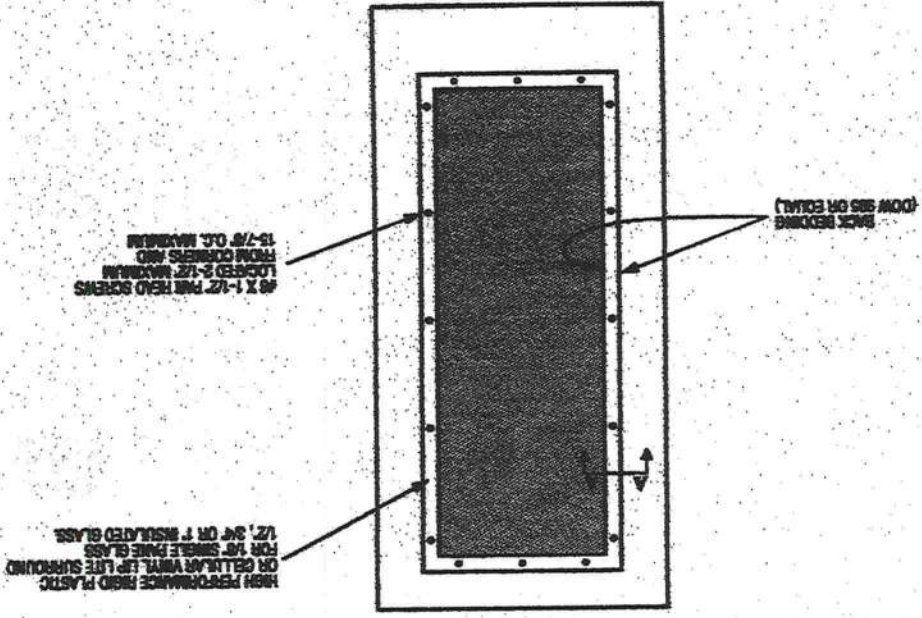
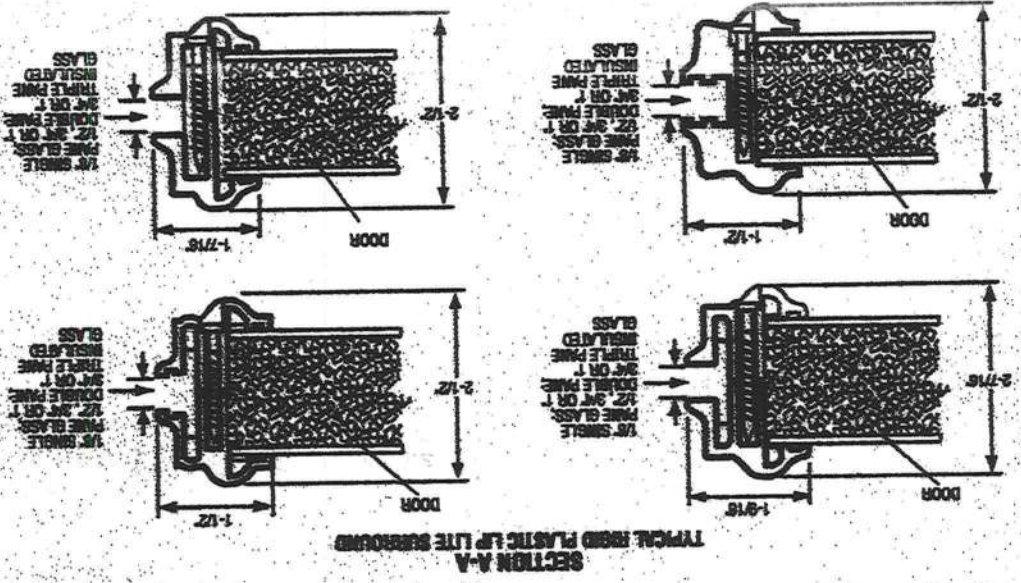


10"	Vent Pipe Size
1"	Min. Pipe Clearance
14' 6"	Min. System Height
14' 6"	- w/ Single Offset
22' 0"	- w/ Two Offsets
6' 0"	Max. Dist. Between Elbows
60' 0"	Max. System Height

42" Woodburning Fireplace

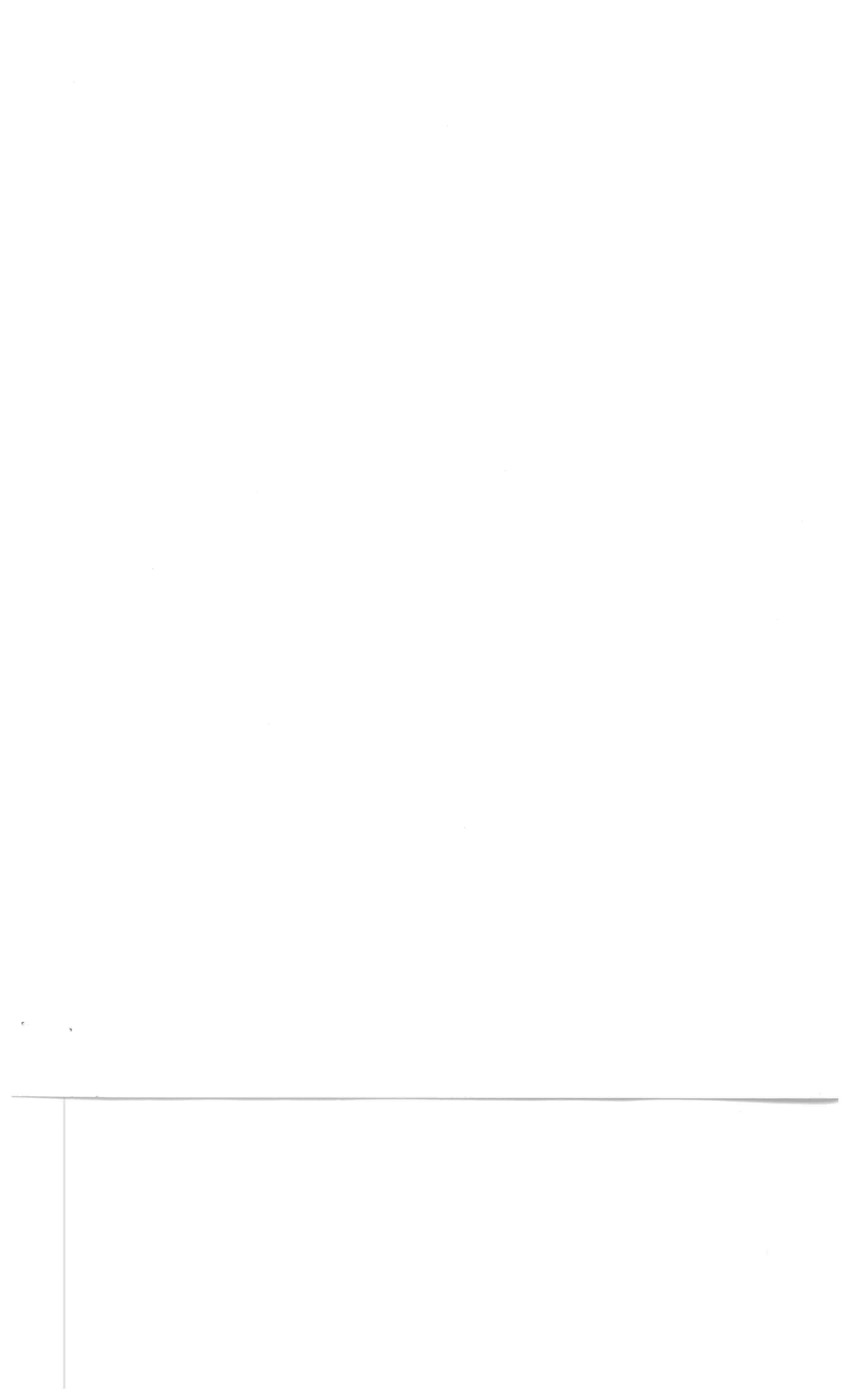
Craftsman

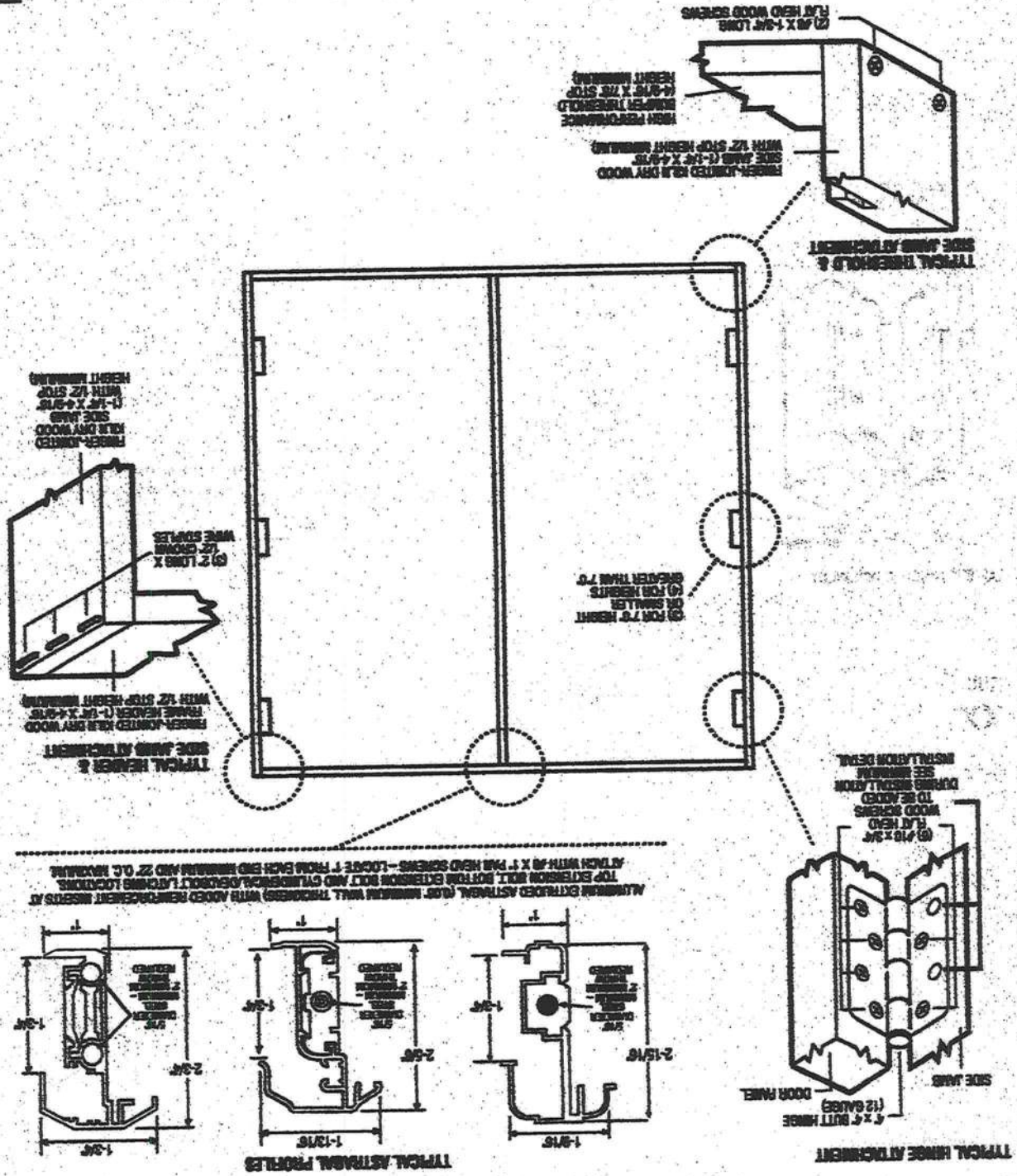




**GLASS INSERT IN DOOR OR SIDELITE PANEL**

MAD-WF-MA0041-02





**OUTSWING UNITS WITH DOUBLE DOOR**

CAD FILE: MADO12 02

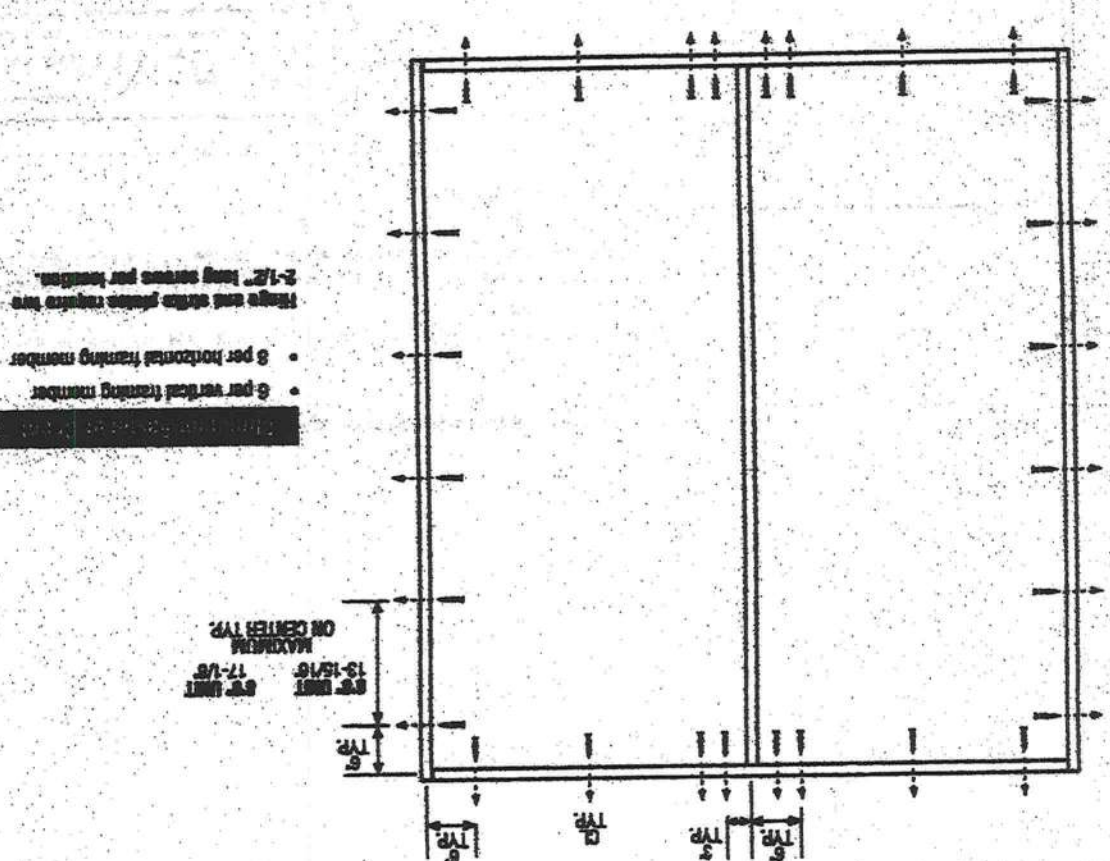
Unit  
**XX**



- Notes:**
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
  2. The wood screw single shear design values come from Table 11.3A of ANS/A-3 & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the RW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
  3. Wood beds by others, must be anchored properly to transfer loads to the structure.

**Latching Hardware:**

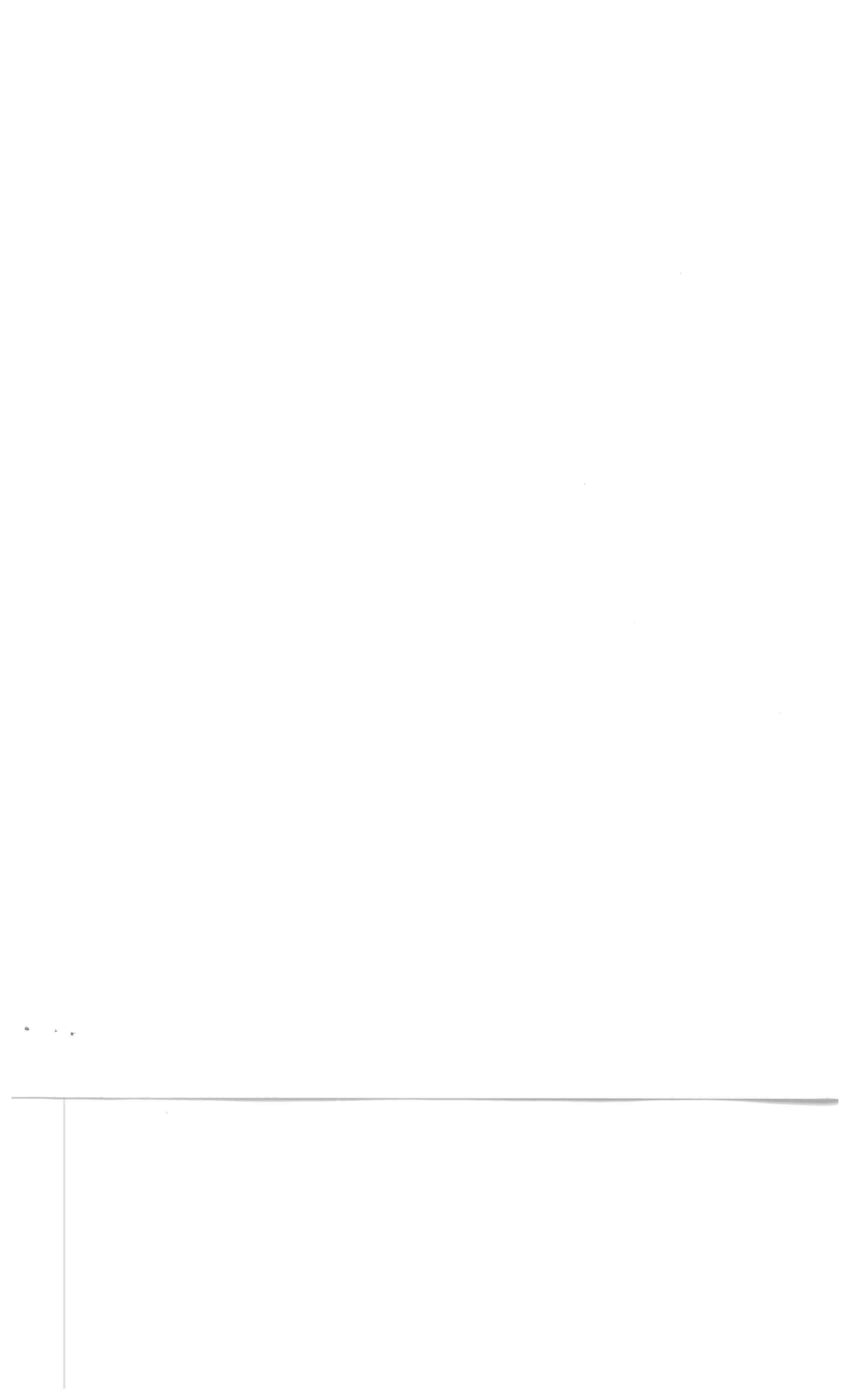
- Compliance requires that GRADE 2 or better (A307/B7M/A156.2) cylindrical and deadlock hardware be installed.



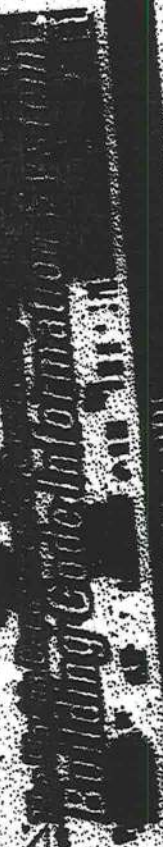
**DOUBLE DOOR**

IBD-VII-MA002-02

**XX**  
 Unit



Florida Building Code Online



Organization Registration  
User Registration  
Organization Publication  
User Organization Search  
Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (All)

Organizations: General American Door - Product Manufacturer

Search

Cancel

Result List for Organizations

Name	City	Contact	Phone	Type	Expires	Status
General American Door	Montgomery	James Campbell	630839000	Product Manufacturer	01/01/2009	Approved

Displaying 1-1 of 1

[http://www.floridabuilding.org/Connective\\_org\\_regi\\_search.asp](http://www.floridabuilding.org/Connective_org_regi_search.asp)

07/17/2004

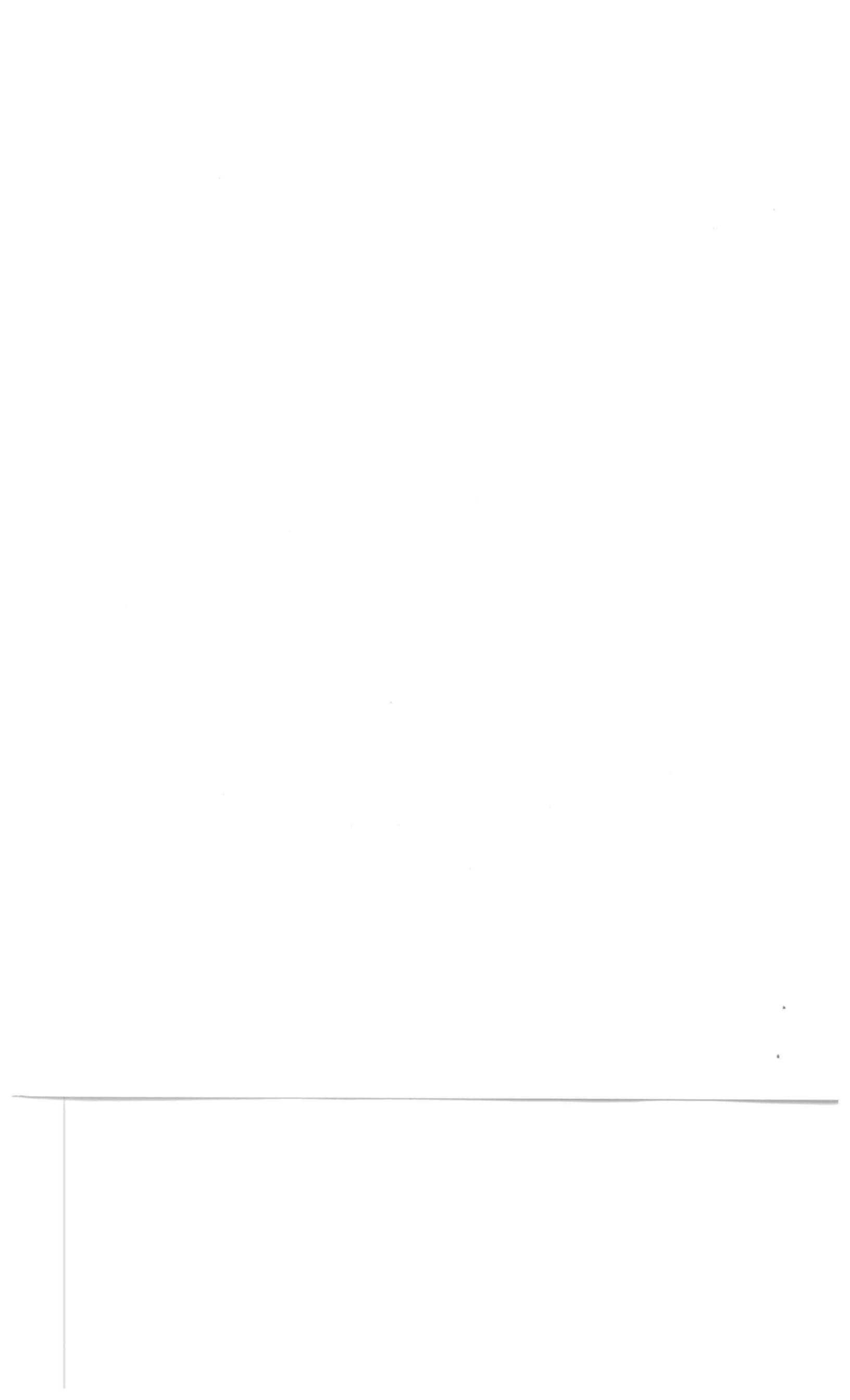
Jun. 28 2004 07:37AM P1

FRX NO. : 386-754-9993

FROM: Columbia Door Company

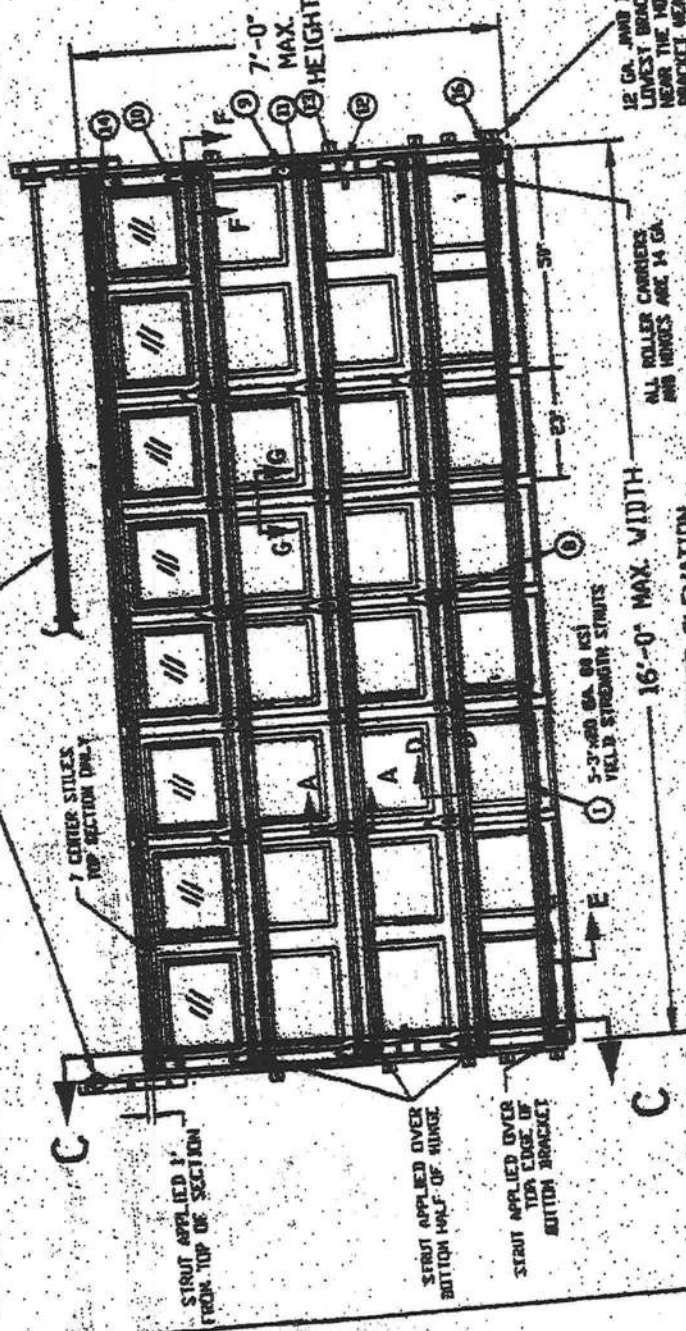
0000 000 000 000 0000

WADCO FLORIDA



- NOTES:**
- TESTED TO POSITIVE AND NEGATIVE 20 PSF RESIDUAL AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
  - MAXIMUM SECTION HEIGHT = 21'
  - SECTION HEIGHTS OF 21'0" AND FEWER ARE ALLOWABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS
  - WEDGES MAY BE INSTALLED IN THE TOP SECTION, AS TESTED WITH 1/8" WEDGES OR CONVENIENT ON BY THE SECTION IMMEDIATELY BELOW THE TOP SECTION
  - MAXIMUM LENGTH OF ROLLER TRACK IS 47' AS TESTED
  - THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE WEDGE PROBE
  - STRUTS SECURED AT ALL APPLICABLE WITH TIE SCREWS
  - QUANTITY OF TIE LINES FOR M. O. L. OR O. AS VERTICAL
  - DOOR IN TYPE OF INSULATION IS OPTIONAL

**A** NOT PART OF WIND LOAD SYSTEM  
EXTENSION SPRING COUNTERBALANCE  
TORSION SPRING COUNTERBALANCE



STRUT APPLIED FROM TOP OF SECTION

STRUT APPLIED OVER BOTTOM HALF OF HINGE

STRUT APPLIED OVER TOP EDGE OF BOTTOM BRACKET

7 CENTER STILES TOP SECTION ONLY

5-3/8\"/>

16'-0" MAX. WIDTH

INSIDE ELEVATION

ALL ROLLER CARRIERS AND HINGES ARE 14 GA.

SEC. C-C  
VERTICAL TRACK, (16 GA.)

12 GA. AND BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR AND BRACKET NEAR THE HORIZONTAL S OF THE BOTTOM SECTION AND 300 BRACKET NEAR THE TOP OF THE BOTTOM SECTION

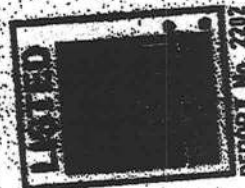
DESIGN LOAD +20.0 PSF & -20.0 PSF  
TEST LOAD +30.0 PSF & -30.0 PSF



GENERAL AMERICAN DOOR COMPANY  
5820 BASELINE ROAD  
MONTGOMERY, IL 60038

TEST REPORTS ON FILE [VIDEO 10/10/00 002320]

TESTED WITH WEDGES	MAXIMUM DOOR HEIGHT	VERTICAL CENTER STILE SPACING	STRUTS PER SET	VERTICAL TRACK INCH
GAIBED DOORS SERIES 7400, EXTERIOR STEEL = .017 MIN O.S. SERIES 7625, EXTERIOR STEEL = .017 MIN O.S. SERIES 7524, EXTERIOR STEEL = .024 MIN O.S.	16'	7"	23"	3"
(TESTED WITH WEDGES)	MAXIMUM DOOR HEIGHT	VERTICAL CENTER STILE SPACING	STRUTS PER SET	VERTICAL TRACK INCH
	16'	7"	23"	3"

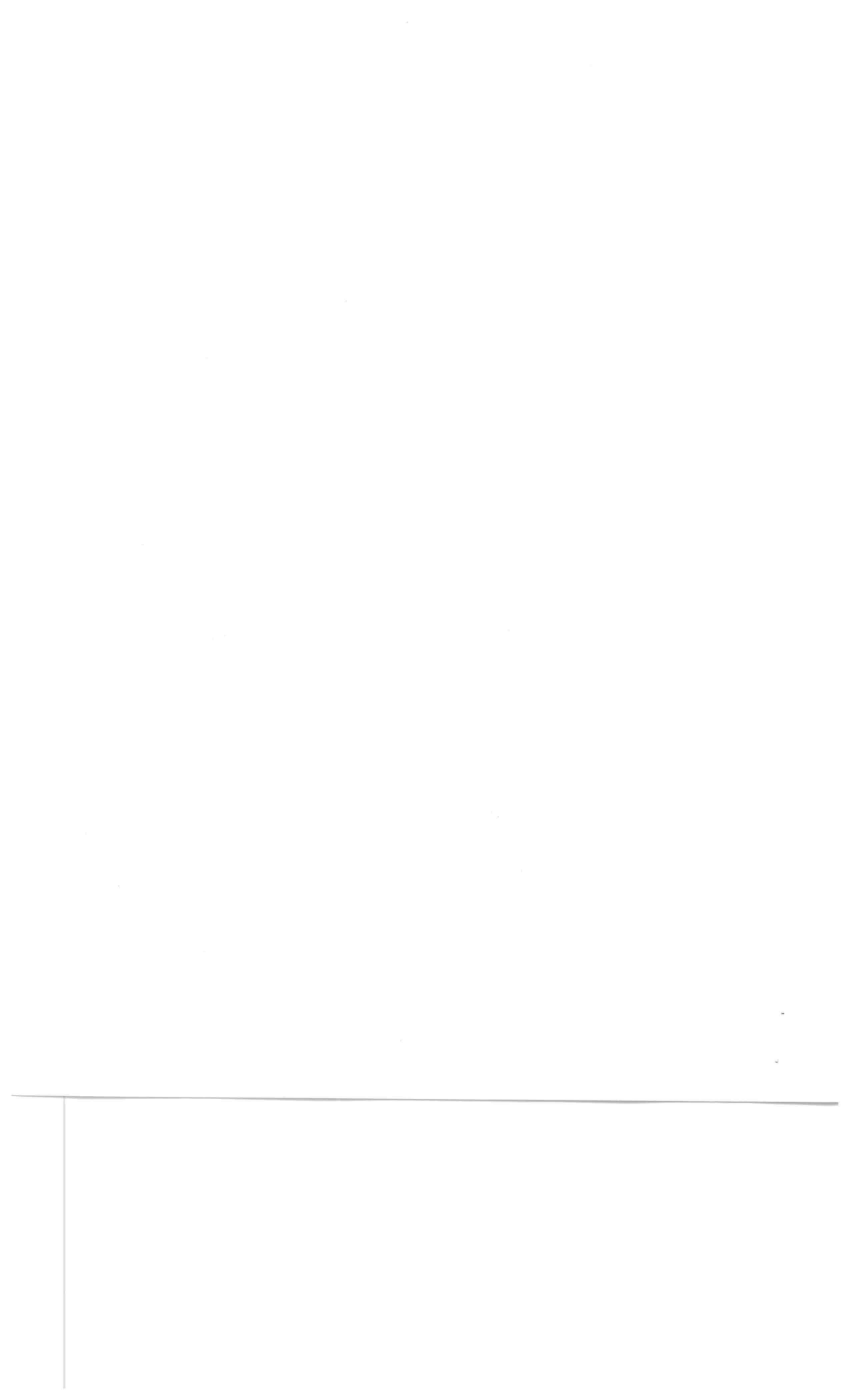


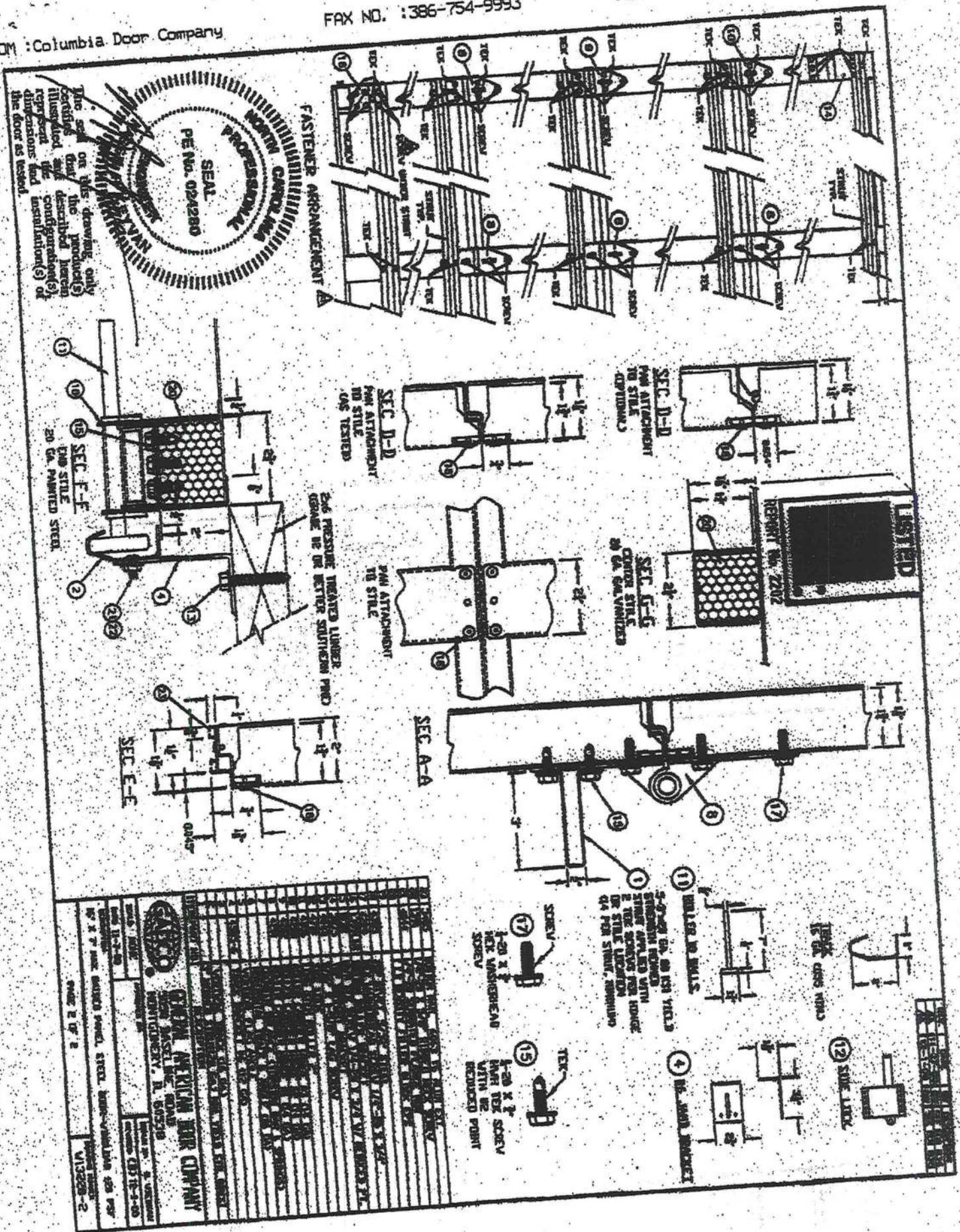
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

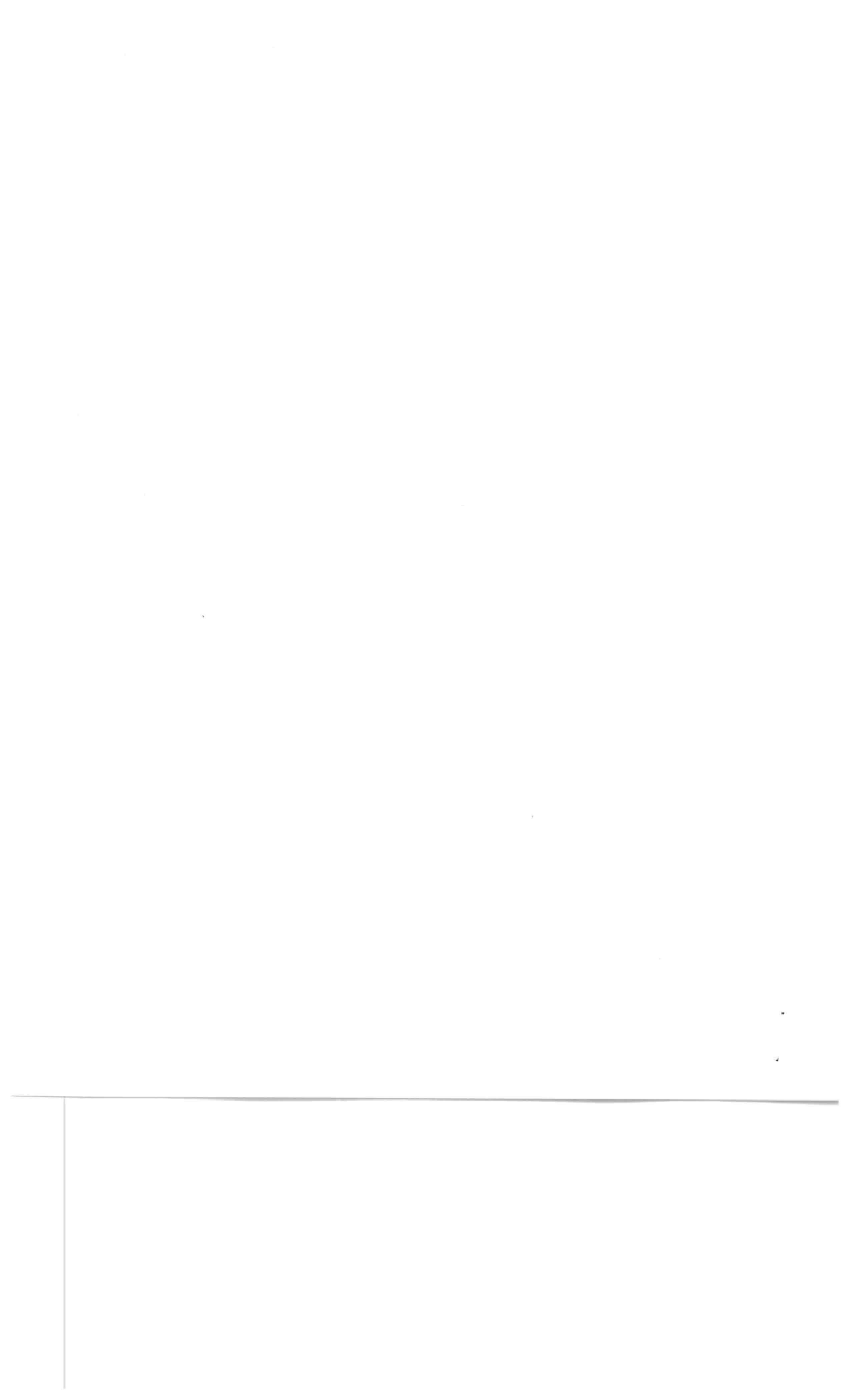
Jun. 28 2004 07:38PM P2

FRX NO. : 386-754-9993

FROM: Columbia Door Company



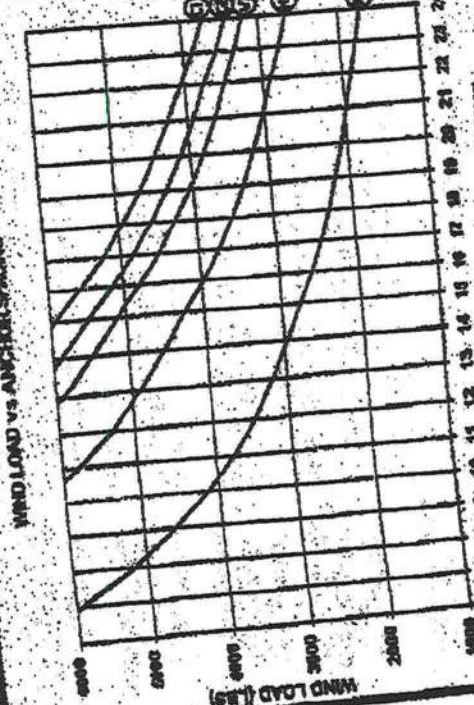




**2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT**

2x6 PRESSURE TREATED GRADE 12 OR BETTER SOUTHERN PINE WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (MU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

- ① CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ② CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ③ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ④ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑤ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑥ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑦ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑧ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑨ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT
- ⑩ CONCRETE MASONRY UNIT (MU) SHALL BE 12" MIN. THICK AND 8" MIN. HIGH. 1-1/2" EMBEDMENT



DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-TT X HEIGHT-TT) = WIND LOAD (LBS)

**EXAMPLE**

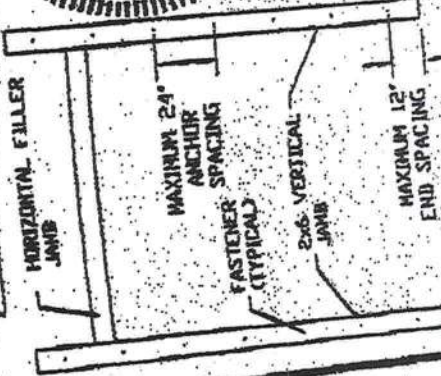
30 LBS X (6 FT WIDE X 8 FT HIGH) = 2400 LBS

① USE 22" SPACING    ② USE 15" SPACING

③ USE 21" SPACING    ④ USE 17" SPACING

⑤ USE 19" SPACING

SEE NOTE B FOR ANCHORING REQUIREMENTS



- NOTES**
- 1) ALL JOIST OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" FASTENERS.
  - 2) ALL JOIST OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SPECIALLY STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTS 10, CURRENT EDITION.
  - 3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
  - 4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE MINIMUM 2x6 PRESSURE TREATED GRADE 12 OR BETTER SOUTHERN PINE (OR PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAYERS) OF 2x6 PRESSURE TREATED SOUTHERN PINE (OR GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.
  - 5) REINFORCED CONCRETE OR CONCRETE MASONRY UNIT (MU) WALLS OR FULLY GROUTED AND REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT SHALL BE AS SHOWN IN THIS CHART WITH A MINIMUM OF 2x6 PRESSURE TREATED SOUTHERN PINE (OR GRADE OR BETTER) WALL STUDS AT EACH SIDE OF DOOR OPENING. THIS IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C50 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2000 PSI GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
  - 6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
  - 7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (MU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3'-3/4"
  - 8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2x6 WALL STUDS.
  - 9) WASHERS ARE REQUIRED ON ALL FASTENERS.
  - 10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 10' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
  - 11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2x6 WOOD JAMB ARCHERS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2x6 WOOD JAMB ARCHERS, AN ADDITIONAL 2x6 WOOD JAMB ARCHER NEAR THAT STEEL BRACKET TO BRIDGE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ARCHERS.

**GENERAL AMERICAN DOOR COMPANY**

2509 W. WASHINGTON ST. CHICAGO, ILL. 60638

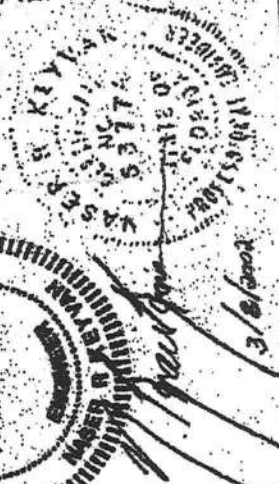
TELEPHONE: (773) 487-1100

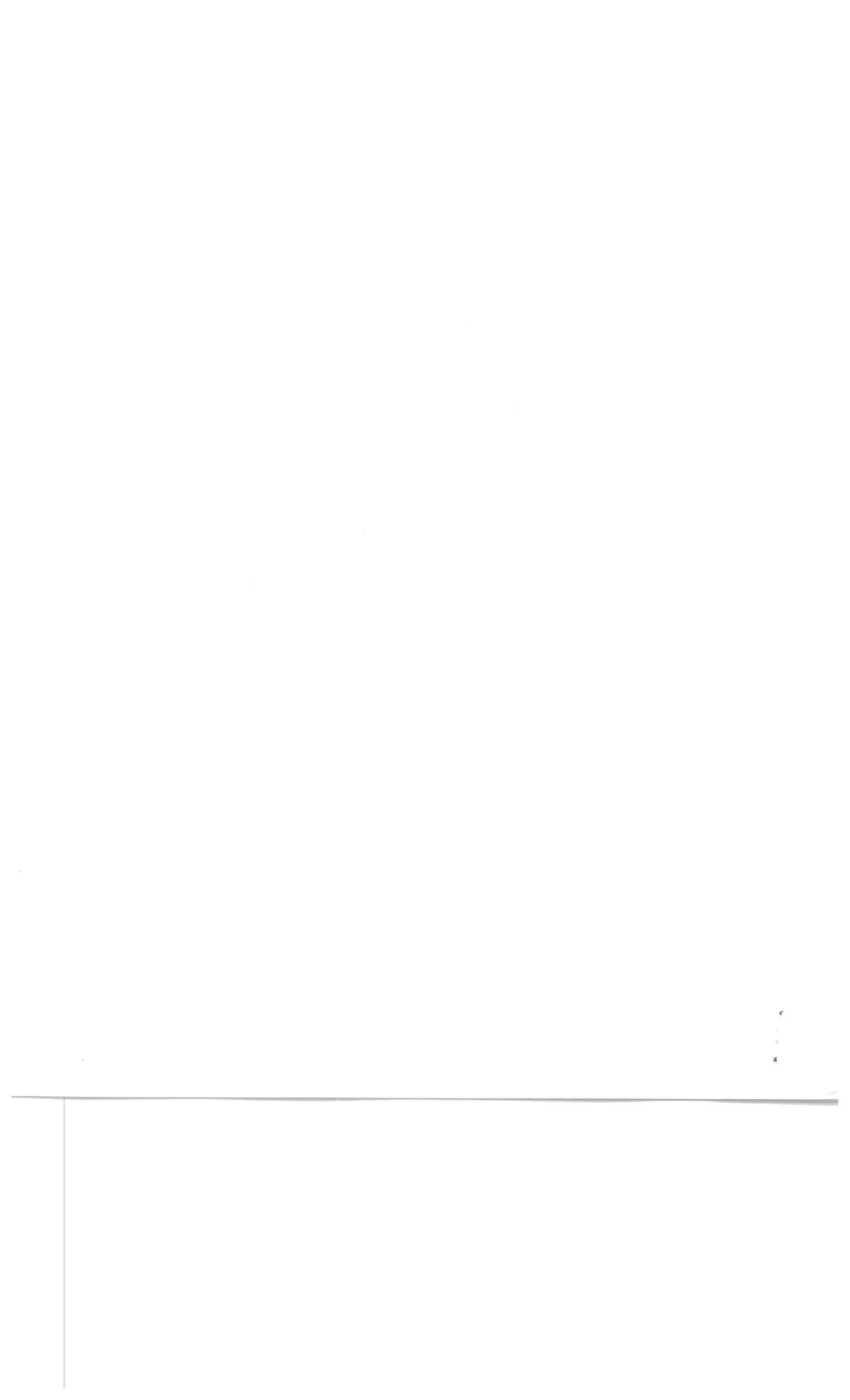
FAX: (773) 487-1101

DATE: 6/28/04

PROJECT: GARAGE DOOR ATTACHMENT

FOR WIND LOADED GARAGE DOORS





\*\* LAMAR BOOZER \*\* PROJECT: HUBER RES UP STAIRES  
 900 EAST PUTNAM STREET CLIENT: HUBER RES UP STAIRES  
 LAKE CITY, FL 32055 DATE: 10 22 05  
 RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

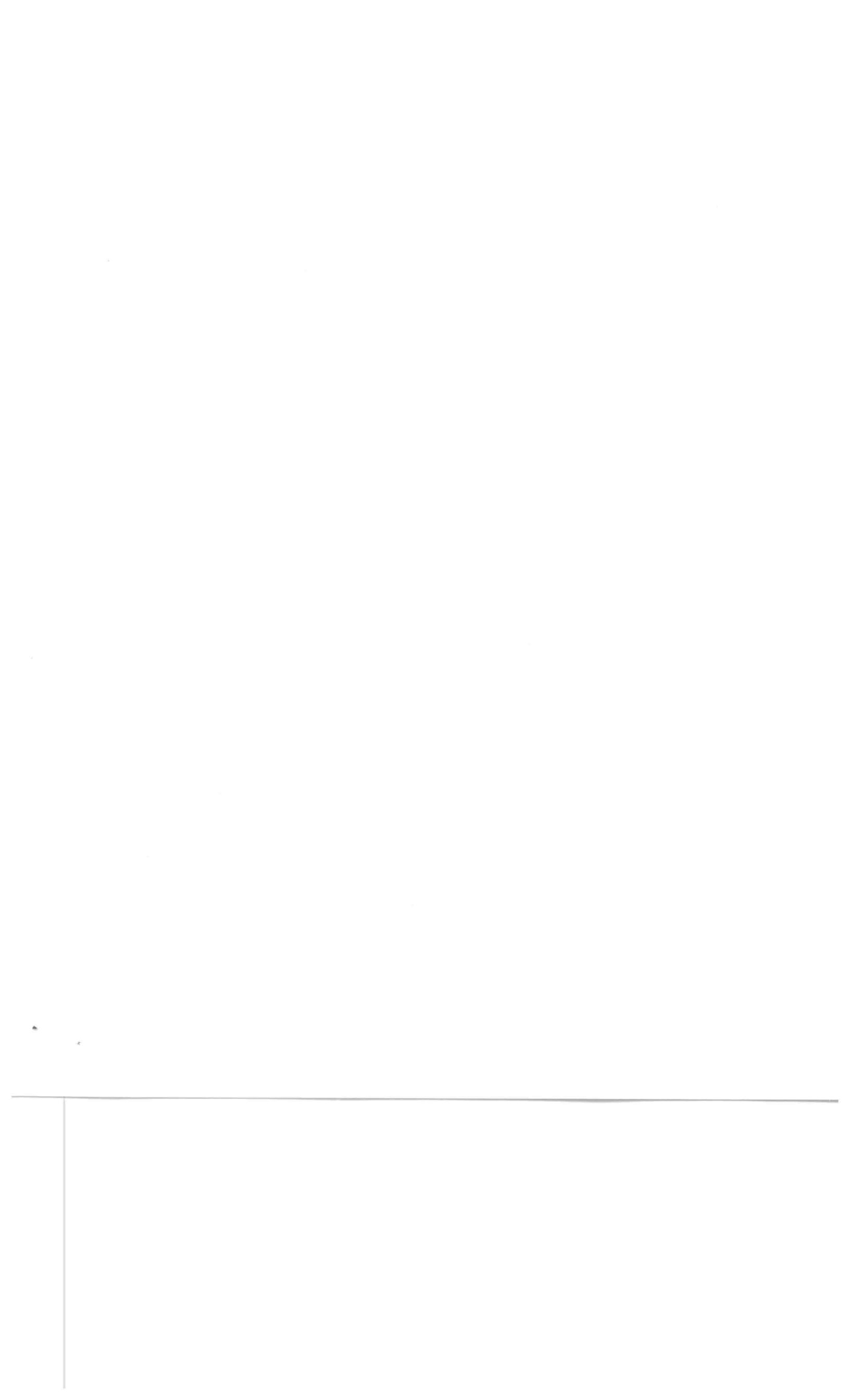
NAME: HUBER RES UP STAIRES  
 ADDRESS: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD	AREA	SEN. LOSS	LAT. +	SEN. = TOTAL
3-C WINDOW DBL PANE CLR GLS METL FR	126	4,111	0	8,387
12-D WALL R-11 + 1/2" ASPHLT BRD(R-1.3)	1,186	4,268	0	2,334
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462
16-G CEILING R-30 INSULATION	1,246	1,850	0	1,850
22-A SLAB ON GRADE NO EDGE INSUL	155	5,649	0	0
SUBTOTALS FOR STRUCTURE:				
	2,753	16,724	0	13,033
PEOPLE	19	0	0	5,700
APPLIANCES	0	0	1,500	3,300
DUCTWORK	0	837	0	2,054
INFILTRATION W.CFM:	0.0	0	0	0
VENTILATION W.CFM:	0.0	0	0	0
SENSIBLE GAIN TOTAL				
TEMP. SWING MULTIPLIER			22,587	
BUILDING LOAD TOTALS				
		17,561	1,500	24,087

SUPPLY CFM AT 20 DEG DT: 1,027  
 SQUARE FT. OF ROOM AREA: 1,246  
 TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 17,561 MBH  
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2,007 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.  
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.



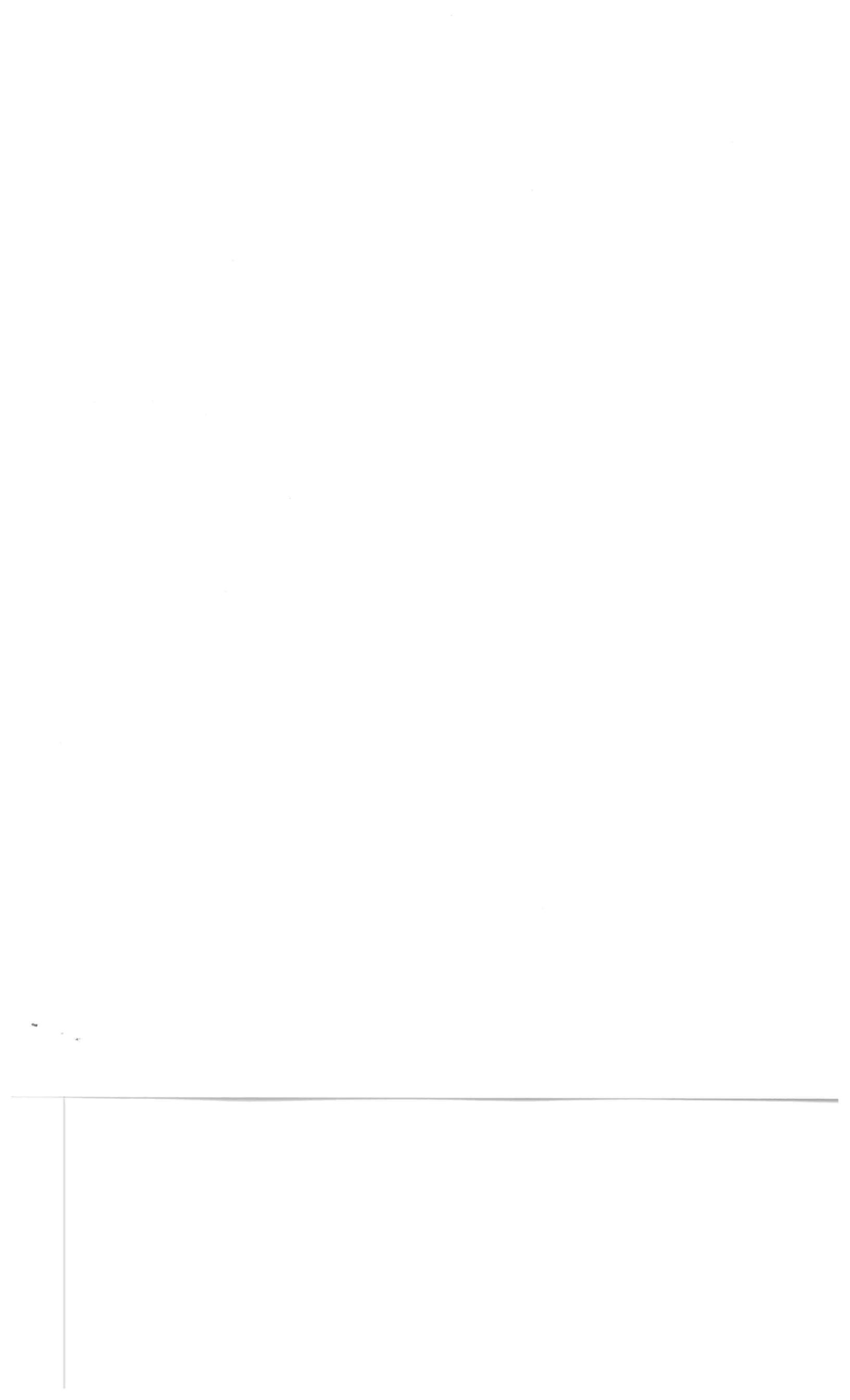
\*\* LAMAR BOOZER \*\*  
 900 EAST PUTNAM STREET  
 LAKE CITY, FL 32055  
 RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS  
 PROJECT: HUBER RES  
 CLIENT: HUBER RES  
 DATE: 10 22 05  
 DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:  
 NAME: HUBER RES  
 ADDRESS: LAKE CITY, STATE: FLORIDA 32055  
 TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA	SEN. LOSS	LAT. + GAIN	SEN. GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	294	9,589	0	18,808	18,808
9-I FRENCH DOOR DBL CLR GLS METL FR	84	2,851	0	6,148	6,148
12-E WALL R-11 + 1/2" EXT POLY BD (R-2.5)	2,409	8,131	0	4,808	4,808
11-C DOOR METAL POLYSTYRENE CORE	57	1,206	0	713	713
16-G CEILING R-30 INSULATION	3,591	4,662	0	4,868	4,868
22-A SLAB ON GRADE NO EDGE INSUL	289	10,535	0	0	0
SUBTOTALS FOR STRUCTURE: 6,724 36,974 0 35,345 35,345					
PEOPLE	28	0	0	8,400	8,400
APPLIANCES	0	0	0	8,400	8,400
DUCTWORK	0	0	1,800	1,500	3,300
INFILTRATION	0.0	1,849	0	4,525	4,525
VENTILATION	0.0	0	0	0	0
W.C.F.M:	0.0	0	0	0	0
S.C.F.M:	0.0	0	0	0	0
SENSIBLE GAIN TOTAL 49,770					
TEMP. SWING MULTIPLIER X 1.00					
BUILDING LOAD TOTALS 38,823 1,800 49,770 51,570					

SUPPLY CFM AT 20 DEG DT: 2,262  
 SQUARE FT. OF ROOM AREA: 3,591  
 CFM PER SQUARE FOOT: 0.721  
 TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 38,823 MBH  
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 6,298 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.  
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.



# Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844  
 Florida Engineering Certificate of Authorization Number: 567  
 Florida Certificate of Product Approval # FL1999  
 Page 1 of 2 Document ID:ISS0487-Z0109095751

Truss Fabricator: Anderson Truss Company  
 Job Identification: 5-344-ISAAC CONST/HUBER - ROOF  
 Truss Count: 135  
 Model Code: Florida Building Code  
 Truss Criteria: ANSI/TPI-2002(STD)/FBC  
 Engineering Software: Alpine Software, Version 7.04.  
 Structural Engineer of Record:  
 Address:  
 Minimum Design Loads: Roof - 45.0 PSF @ 1.25 Duration  
 Floor - N/A  
 Wind - 110 MPH ASCE 7-98 - Closed

**Notes:**

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11030EC-GBLLETIN-A11015EC

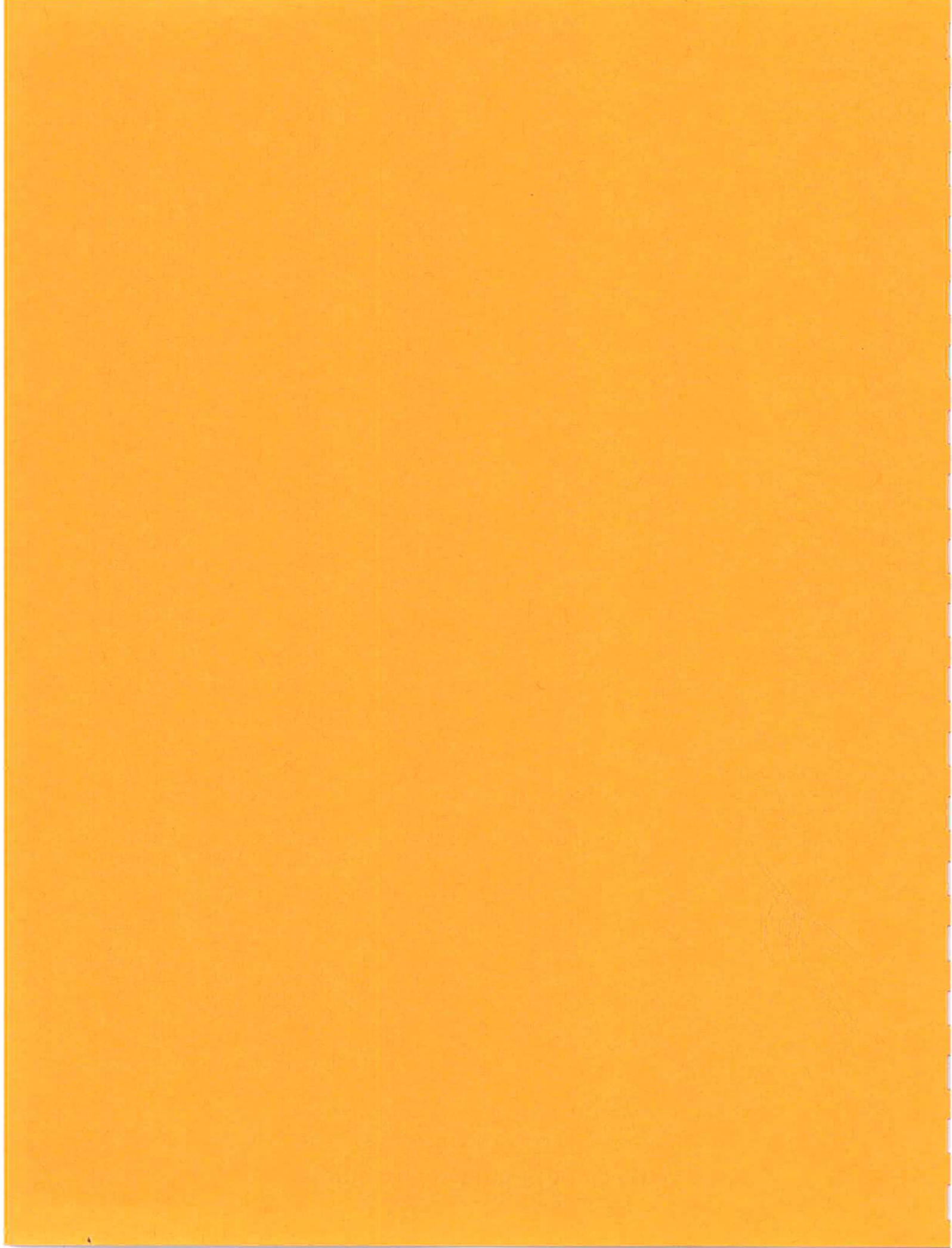
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3	35425	--H188A	05313062	11/09/05
4	35426	--H20A	05313063	11/09/05
5	35427	--H214A	05313064	11/09/05
6	35428	--A	05313065	11/09/05
7	35429	--HS148A	05313066	11/09/05
8	35430	--H16A	05313067	11/09/05
9	35431	--HS8AT	05313068	11/09/05
10	35432	--HS94AT	05313069	11/09/05
11	35433	--HS108AT	05313070	11/09/05
12	35434	--HS12AT	05313071	11/09/05
13	35435	--HS134AT	05313072	11/09/05
14	35436	--H8B	05313073	11/09/05
15	35437	--H904B	05313159	11/09/05
16	35438	--H1008B	05313001	11/09/05
17	35439	--H12B	05313002	11/09/05
18	35440	--H1304B	05313074	11/09/05
19	35441	--H1408B	05313075	11/09/05
20	35442	--H16B	05313076	11/09/05
21	35443	--B	05313077	11/09/05
22	35444	--H76C	05313078	11/09/05
23	35445	--H810C	05313079	11/09/05
24	35446	--H102C	05313080	11/09/05
25	35447	--H116C	05313081	11/09/05
26	35448	--H1210C	05313082	11/09/05
27	35449	--H142C	05313008	11/09/05
28	35450	--H156C	05313012	11/09/05
29	35451	--H1610C	05313018	11/09/05
30	35452	--H182C	05313083	11/09/05
31	35453	--C	05313019	11/09/05
32	35454	--CG	05313084	11/09/05
33	35455	--H8D	05313085	11/09/05
34	35456	--H94D	05313086	11/09/05
35	35457	--D	05313087	11/09/05
36	35458	--H748E	05313088	11/09/05



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38	35460	--ES1	05313090	11/09/05
39	35461	--ES2	05313091	11/09/05
40	35462	--ES3	05313092	11/09/05
41	35463	--ES	05313093	11/09/05
42	35464	--FG	05313094	11/09/05
43	35465	--F	05313095	11/09/05
44	35466	--GGE	05313096	11/09/05
45	35467	--G	05313020	11/09/05
46	35468	--GG	05313097	11/09/05
47	35469	--EJ8	05313098	11/09/05
48	35470	--CJ14	05313021	11/09/05
49	35471	--HJ8	05313099	11/09/05
50	35472	--CJ28	05313022	11/09/05
51	35473	--CJ4	05313023	11/09/05
52	35474	--CJ54	05313100	11/09/05
53	35475	--CJ68	05313101	11/09/05
54	35476	--HJ28	05313102	11/09/05
55	35477	--HJ68	05313103	11/09/05
56	35478	--EJ8T	05313024	11/09/05
57	35479	--EJ8T1	05313104	11/09/05
58	35480	--EJ8T2	05313025	11/09/05
59	35481	--CJ14P	05313026	11/09/05
60	35482	--HJ8P	05313105	11/09/05
61	35483	--CJ28P	05313027	11/09/05
62	35484	--CJ4P	05313041	11/09/05
63	35485	--CJ54P	05313042	11/09/05
64	35486	--CJ68P	05313043	11/09/05
65	35487	--EJ8P	05313044	11/09/05
66	35488	--CJ14T	05313045	11/09/05
67	35489	--HJ8T	05313106	11/09/05
68	35490	--HJ8T1	05313107	11/09/05
69	35491	--CJ28T	05313046	11/09/05
70	35492	--CJ4T	05313047	11/09/05
71	35493	--CJ54T	05313048	11/09/05
72	35494	--CJ68T	05313049	11/09/05

Seal Date: 11/09/2005

-Truss Design Engineer-  
 Arthur R. Fisher  
 Florida License Number: 59687  
 1950 Marley Drive  
 Haines City, FL 33844



# Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844  
 Florida Engineering Certificate of Authorization Number: 567  
 Florida Certificate of Product Approval # FL1999  
 Page 2 of 2 Document ID:ISS0487-Z0109095751

Truss Fabricator: Anderson Truss Company  
 Job Identification: 5-344-ISAAC CONST/HUBER - ROOF  
 Truss Count: 135  
 Model Code: Florida Building Code  
 Criteria: ANSI/TPI-2002(STD)/FBC  
 Engineering Software: Alpine Software, Version 7.04.  
 Structural Engineer of Record:  
 Address:  
 Minimum Design Loads:  
 Roof - 45.0 PSF @ 1.25 Duration  
 Floor - N/A  
 Wind - 110 MPH ASCE 7-98 - Closed

**Notes:**

- Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
- The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
- As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11030EC-GBLETTIN-A11015EC

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73	35495	--H68TG	05313108	11/09/05
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75	35497	--H8TG	05313109	11/09/05
76	35498	--EJ4T	05313051	11/09/05
77	35499	--CJ68TG	05313110	11/09/05
78	35500	--HJ4T	05313111	11/09/05
79	35501	--HJ54T	05313112	11/09/05
80	35502	--H14M	05313113	11/09/05
81	35503	--H14T	05313114	11/09/05
82	35504	--H2M	05313115	11/09/05
83	35505	--HJ14T	05313116	11/09/05
84	35506	--EJ54G	05313117	11/09/05
85	35507	--HTG	05313118	11/09/05
86	35508	--H4K	05313119	11/09/05
87	35509	--KG	05313120	11/09/05
88	35510	--MB	05313121	11/09/05
89	35511	--MB	05313122	11/09/05
90	35512	--MBH	05313123	11/09/05
91	35513	--MBH1	05313124	11/09/05
92	35514	--MBH2	05313125	11/09/05
93	35515	--MBH3	05313126	11/09/05
94	35516	--MBGE	05313127	11/09/05
95	35517	--MFGG	05313128	11/09/05
96	35518	--MGB	05313129	11/09/05
97	35519	--MPGE	05313130	11/09/05
98	35520	--MDR	05313161	11/09/05
99	35521	--MDL	05313131	11/09/05
100	35522	--H28M	05313132	11/09/05
101	35523	--MG	05313133	11/09/05
102	35524	--MD	05313052	11/09/05
103	35525	--MF	05313053	11/09/05
104	35526	--MP	05313054	11/09/05
105	35527	--ML	05313055	11/09/05
106	35528	--MLG	05313134	11/09/05
107	35529	--N	05313135	11/09/05
108	35530	--NGE	05313136	11/09/05

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111	35533	--H680	05313138	11/09/05
112	35534	--H8P	05313139	11/09/05
113	35535	--P	05313140	11/09/05
114	35536	--H94P	05313141	11/09/05
115	35537	--H108P	05313142	11/09/05
116	35538	--H12P	05313143	11/09/05
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118	35540	--H1408P	05313145	11/09/05
119	35541	--H68R	05313146	11/09/05
120	35542	--R	05313057	11/09/05
121	35543	--RS	05313058	11/09/05
122	35544	--RG	05313147	11/09/05
123	35545	--H510S	05313148	11/09/05
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125	35547	--S	05313059	11/09/05
126	35548	--SG	05313150	11/09/05
127	35549	--H54T	05313151	11/09/05
128	35550	--H54MT	05313152	11/09/05
129	35551	--T	05313060	11/09/05
130	35552	--T	05313153	11/09/05
131	35553	--T	05313154	11/09/05
132	35554	--H68MT	05313155	11/09/05
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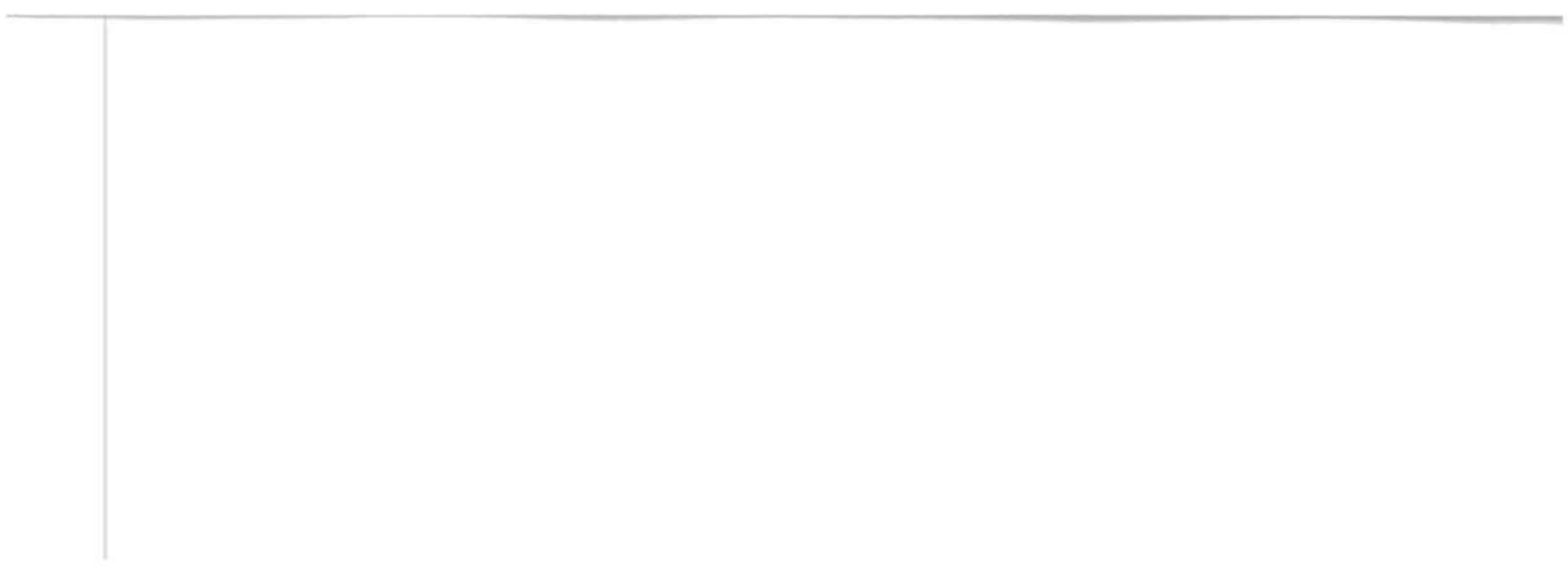


Seal Date: 11/09/2005

-Truss Design Engineer-  
 Arthur R. Fisher  
 Florida License Number: 59687  
 1950 Marley Drive  
 Haines City, FL 33844



17



719-7143

JOB LOCATION:

JOB DESCRIPTION:  
ISAAC CONST/HUBER - ROOF

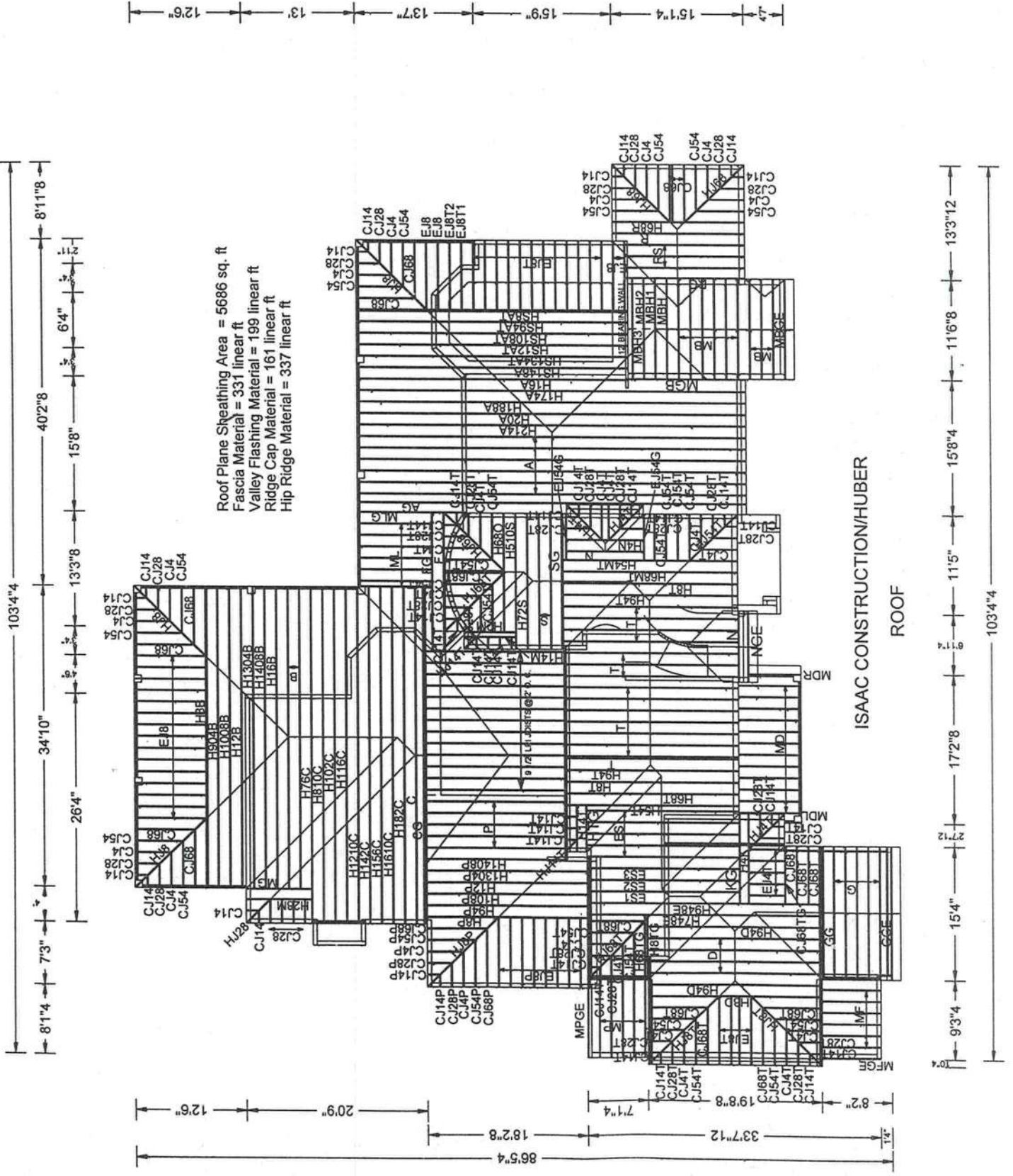
DESIGNED BY:

JOB NO:

5-344

PAGE NO:

1 OF 1

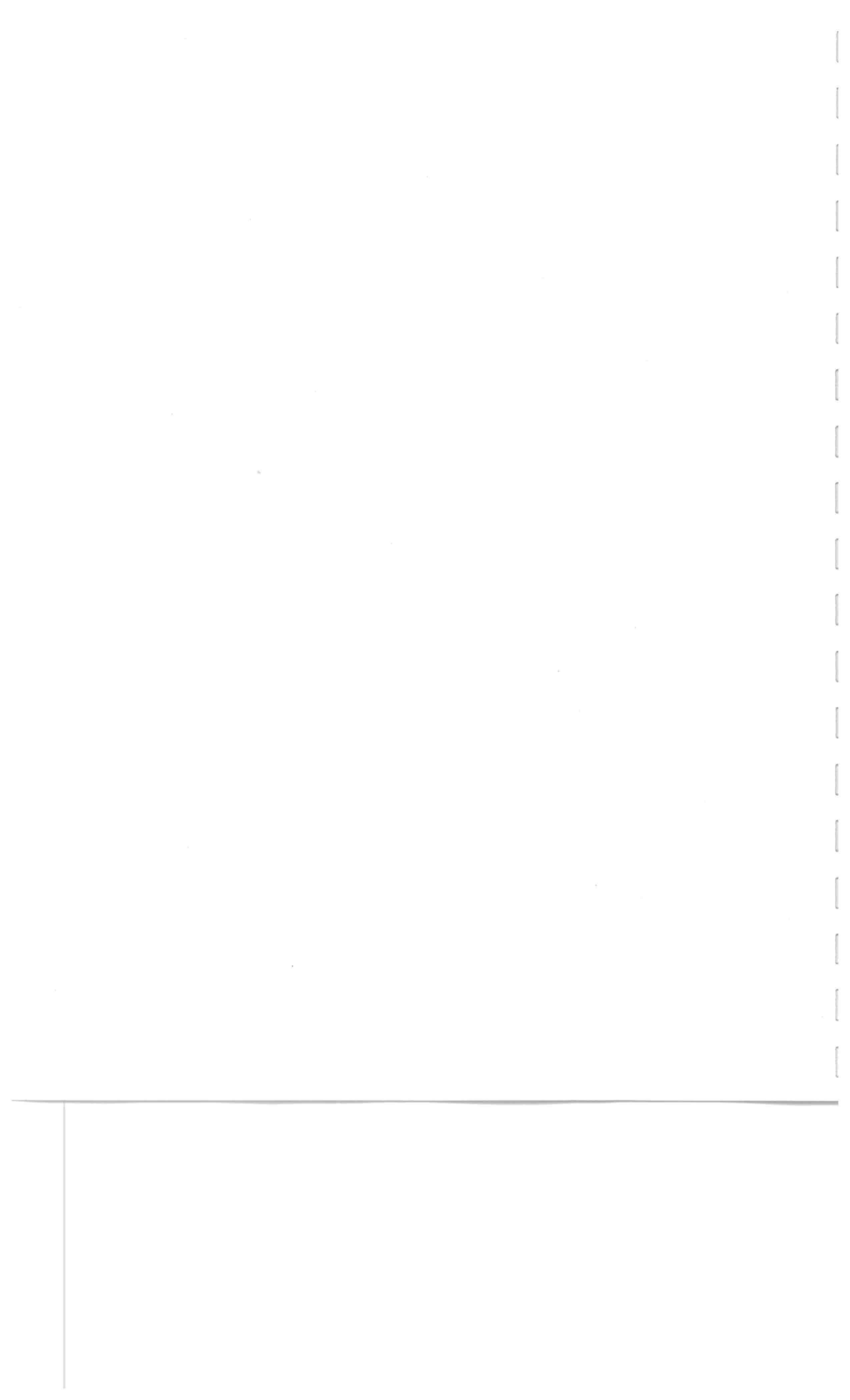




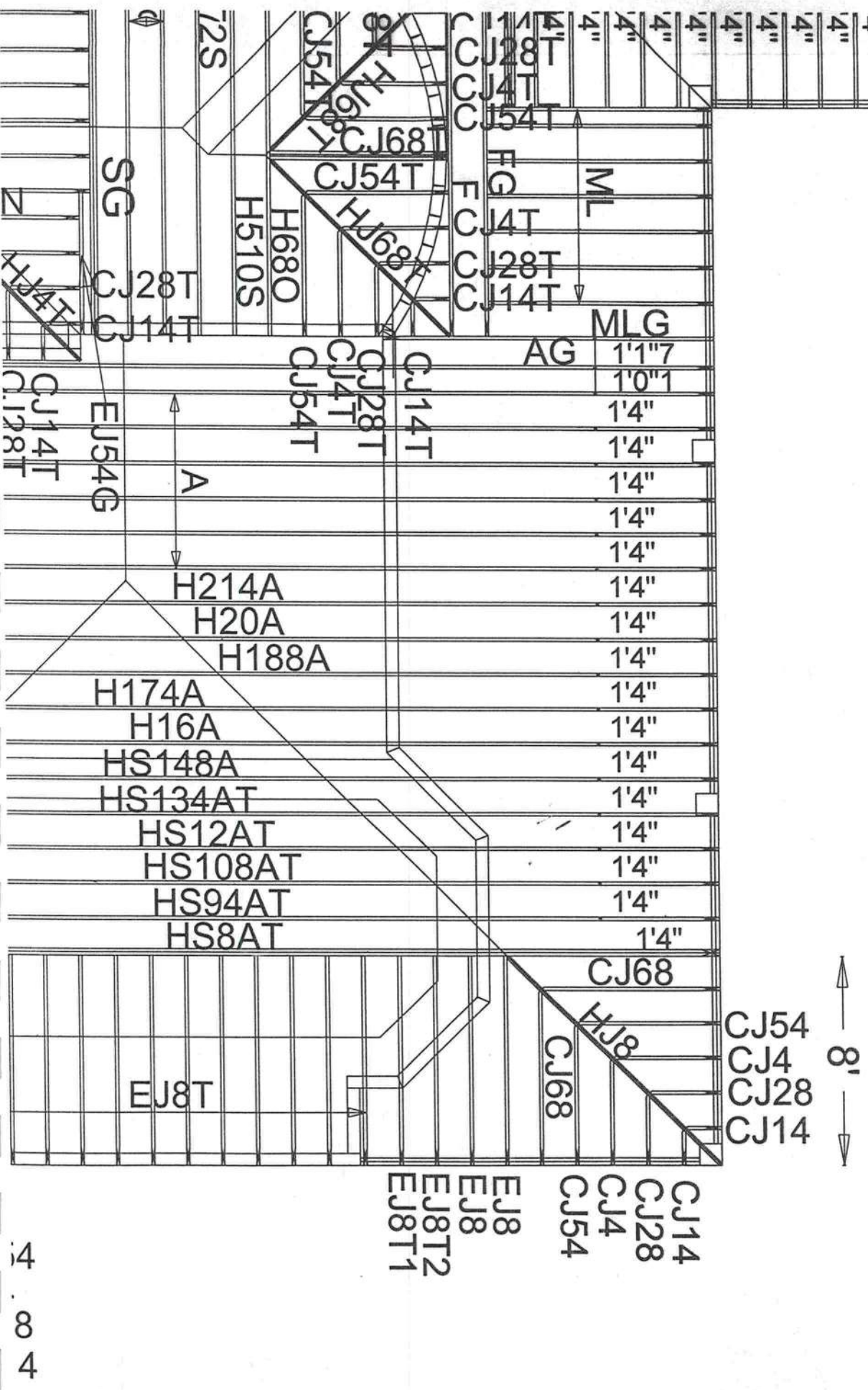
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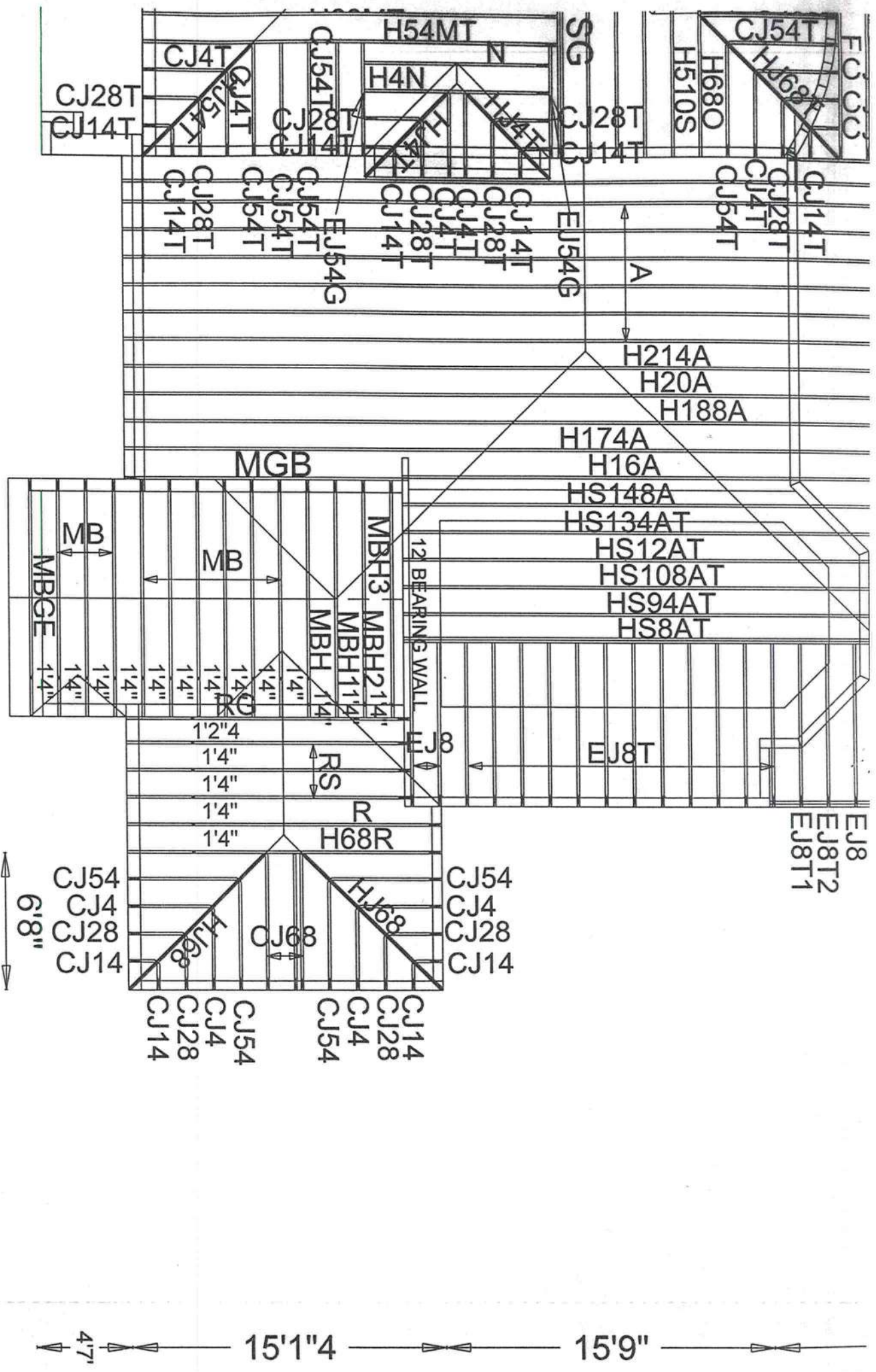
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 Fascia Material = 331 linear ft  
 Valley Flashing Material = 199 linear ft  
 Ridge Cap Material = 161 linear ft  
 Hip Ridge Material = 337 linear ft

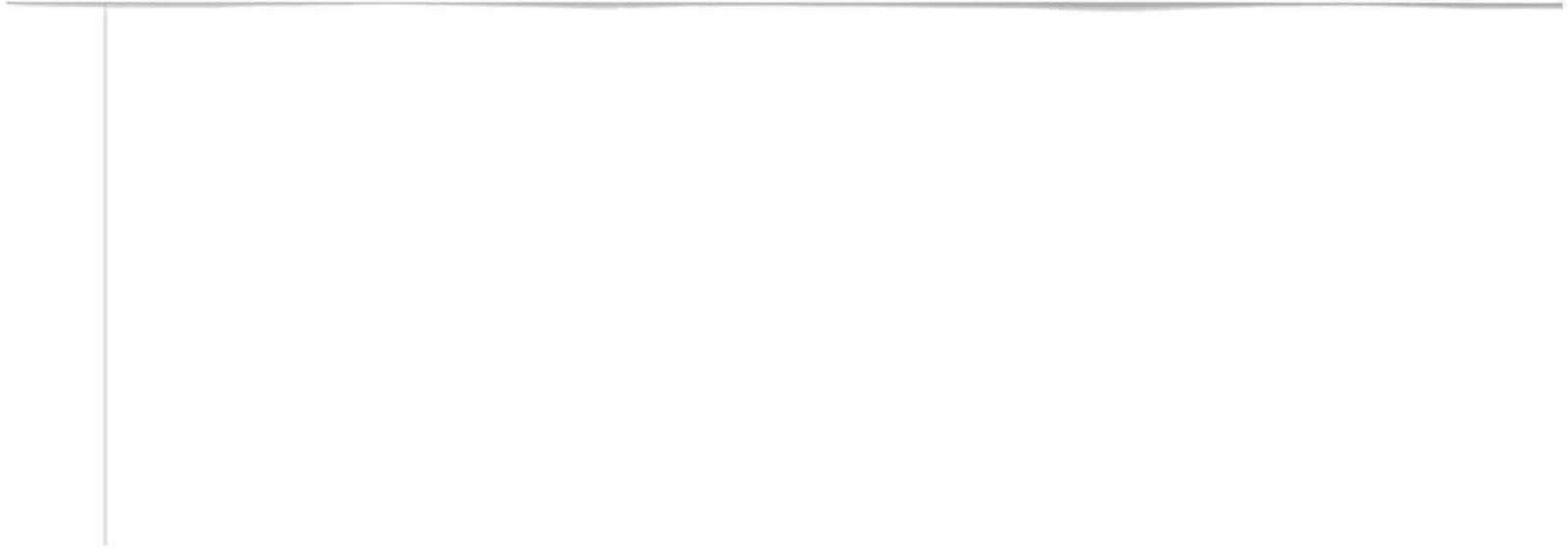


1/4  
 .  
 8  
 4



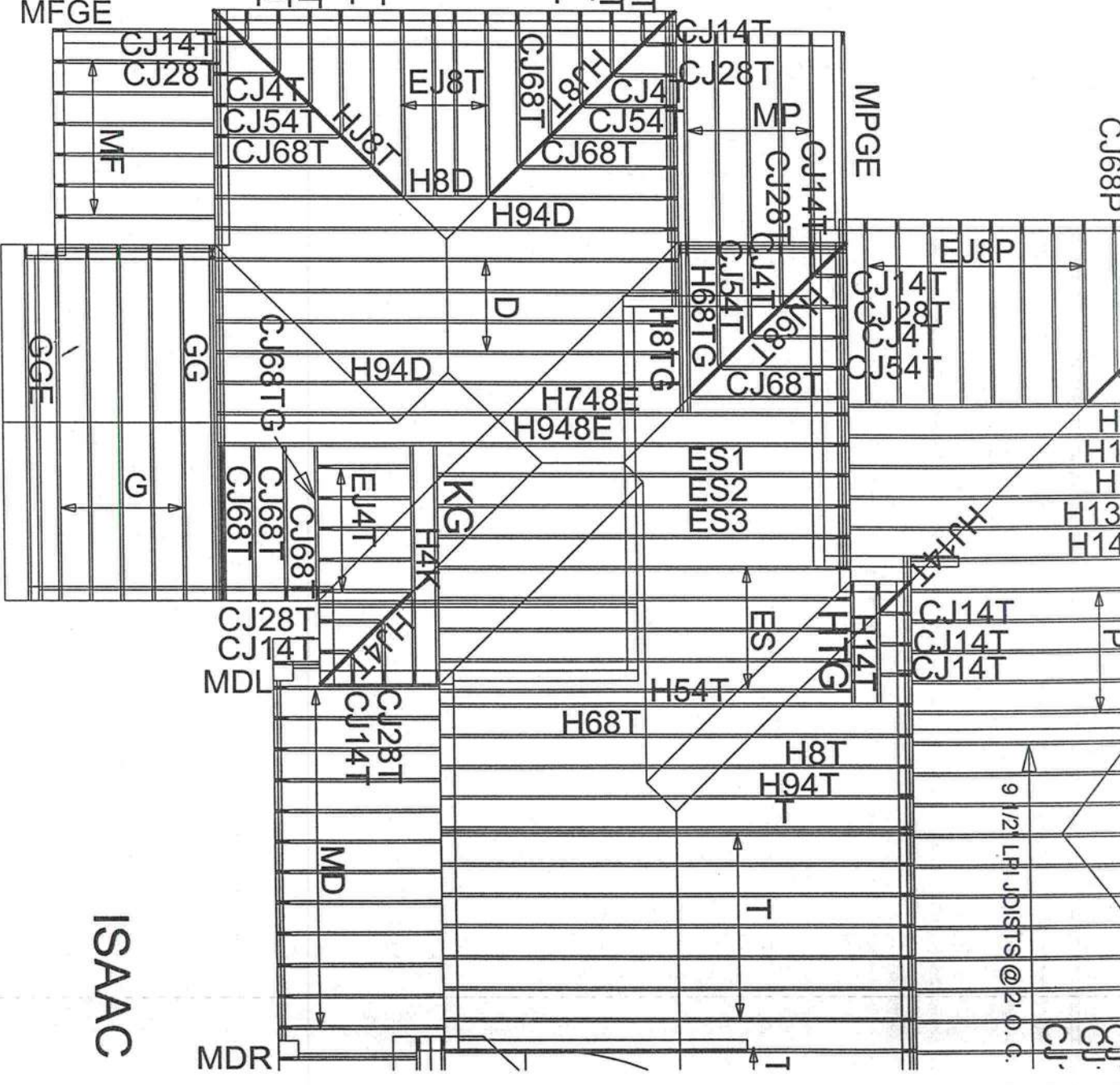
# SECTION/HUBER





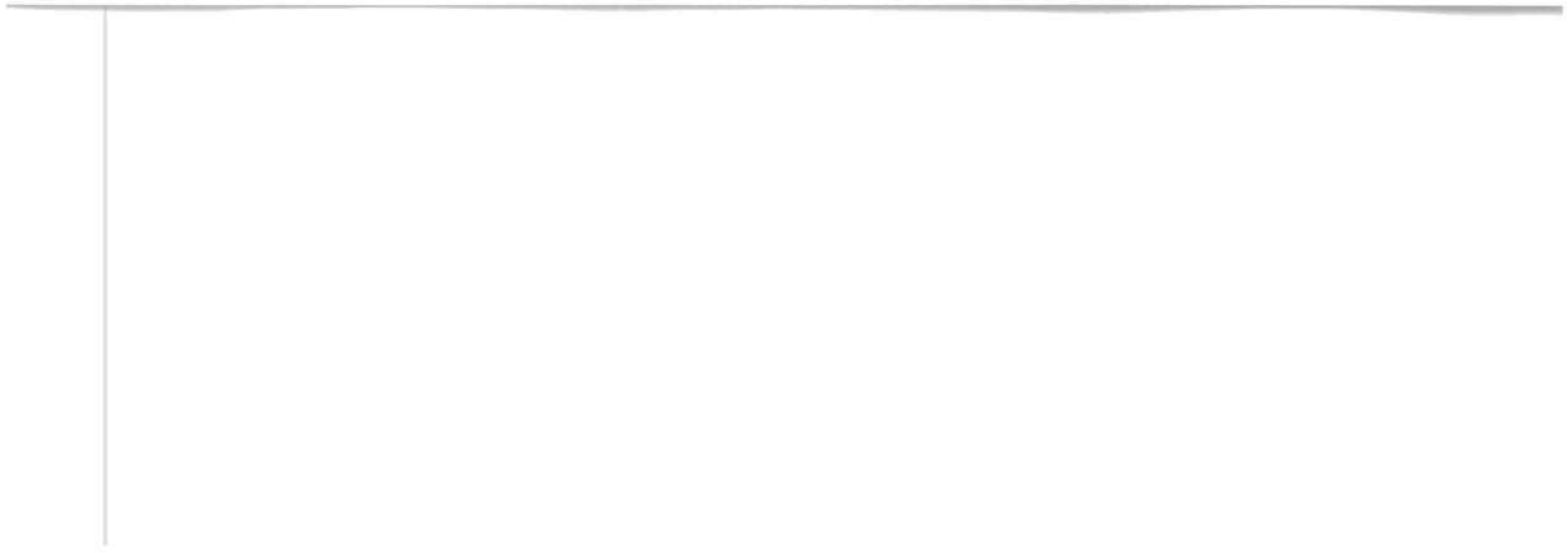
86'5"4  
33'7"12  
18'2"8  
1'4"

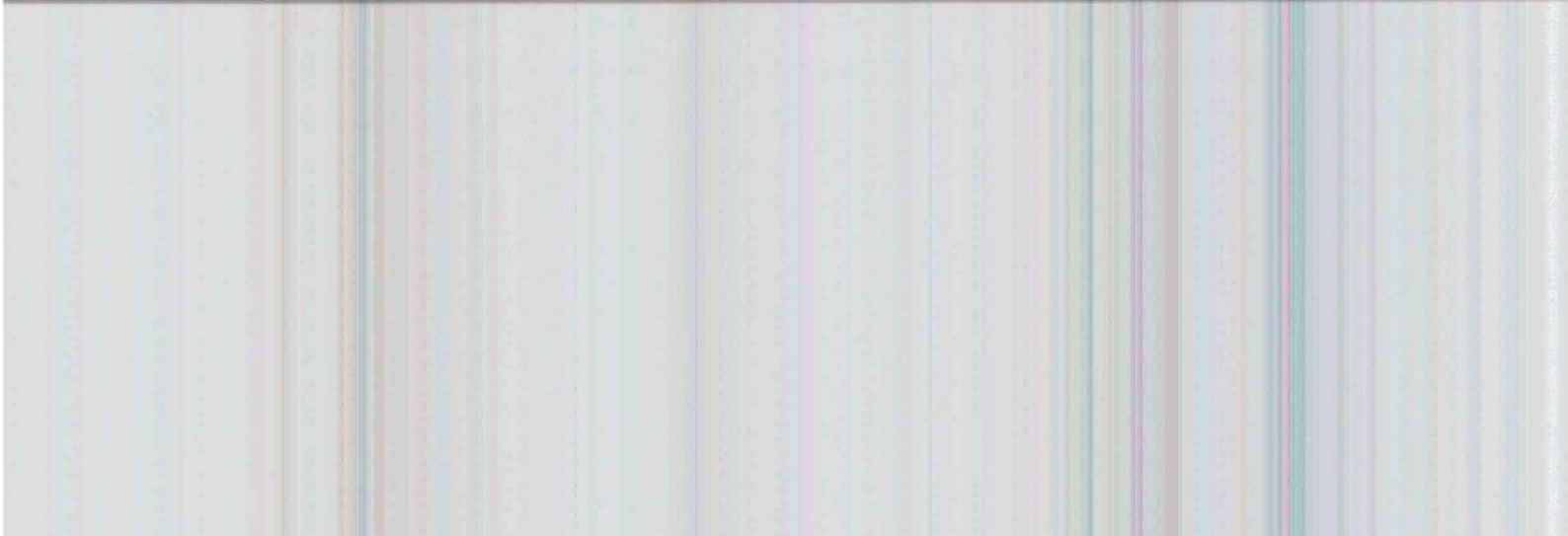
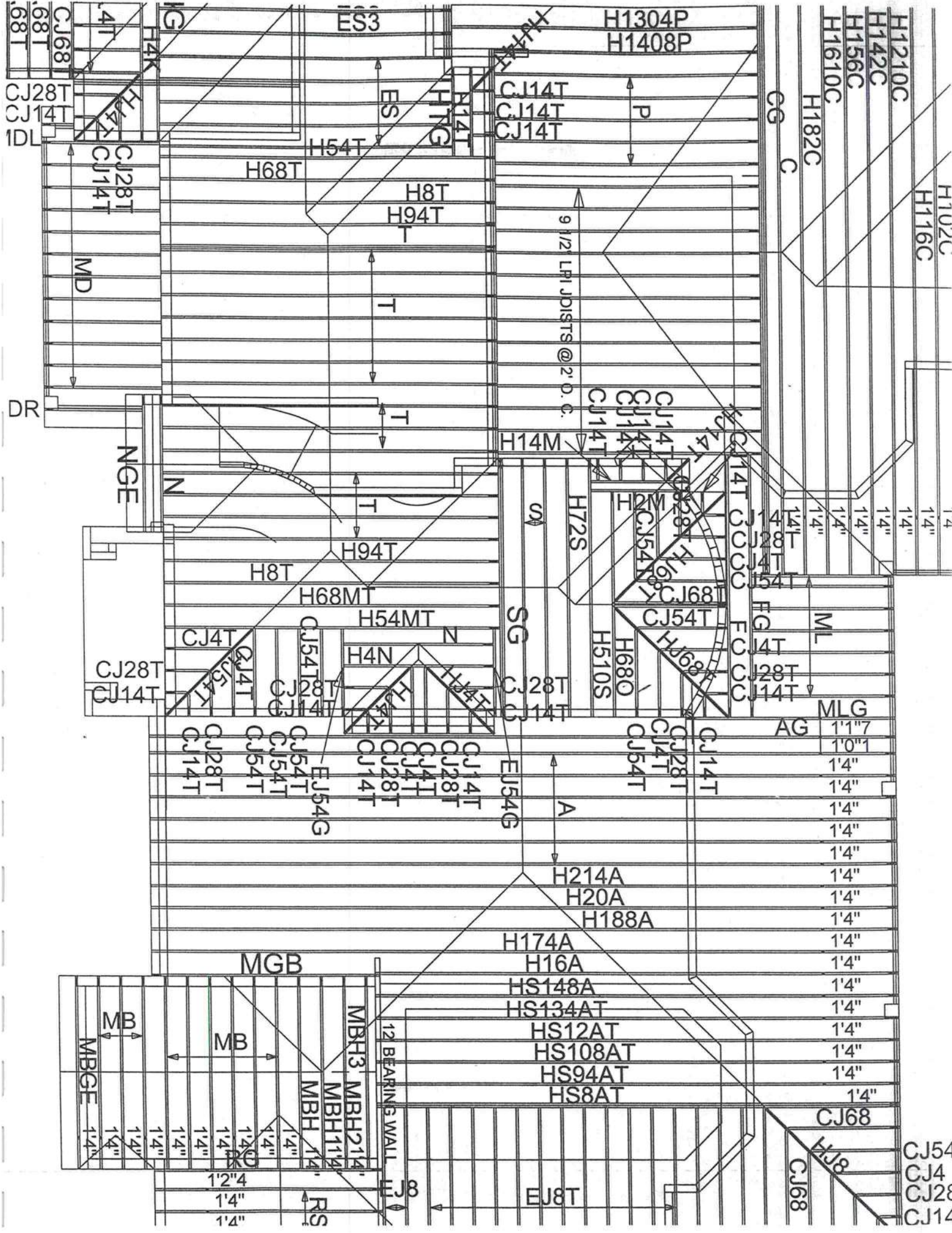
8'2" 19'8"8 7'1"4  
CJ14T  
CJ28T  
CJ4T  
CJ54T  
CJ68T  
CJ14T  
CJ28T  
CJ4T  
CJ54T  
CJ68T

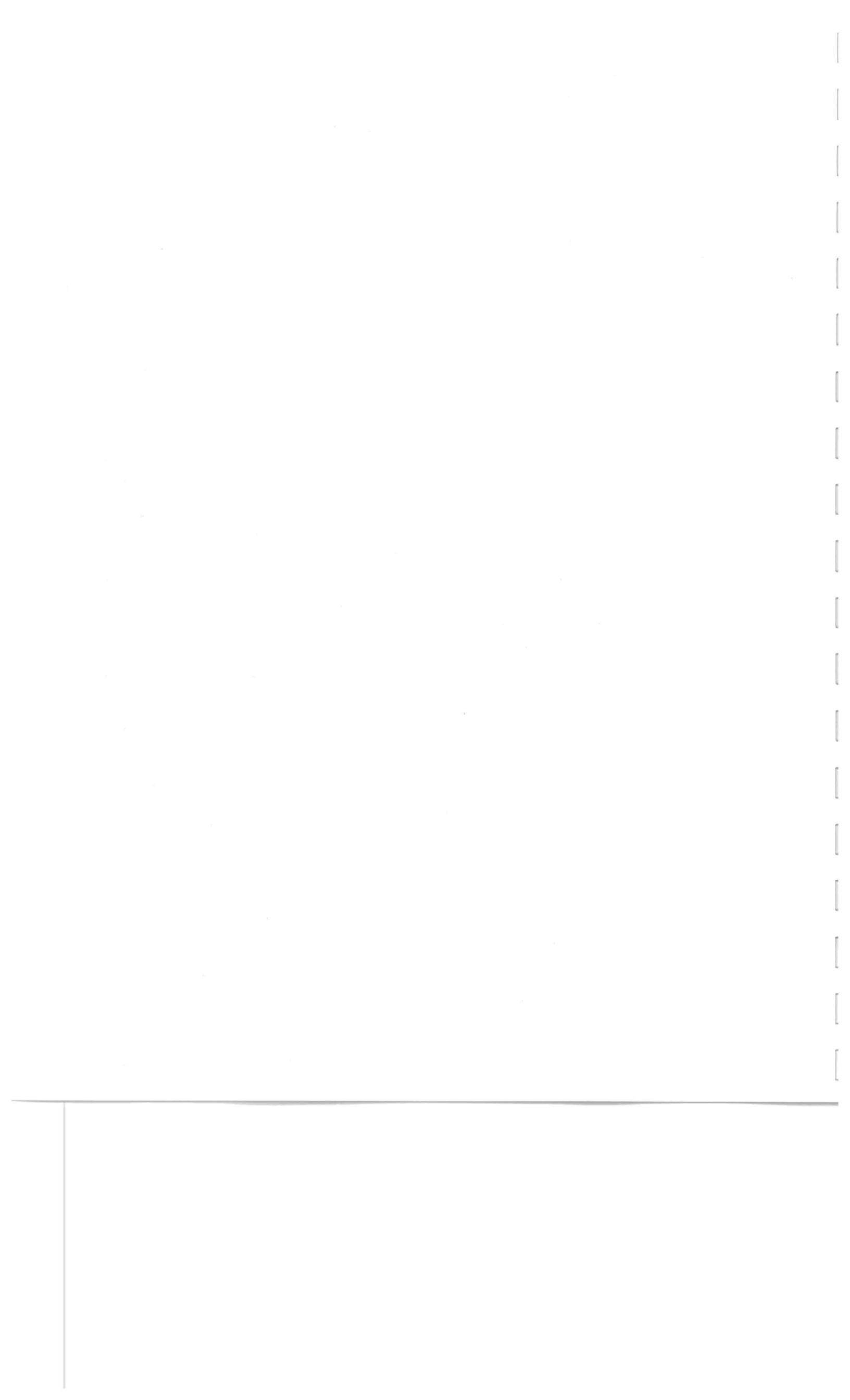


ISAAC

9 1/2' LRI JDISTS @ 2' O.C.





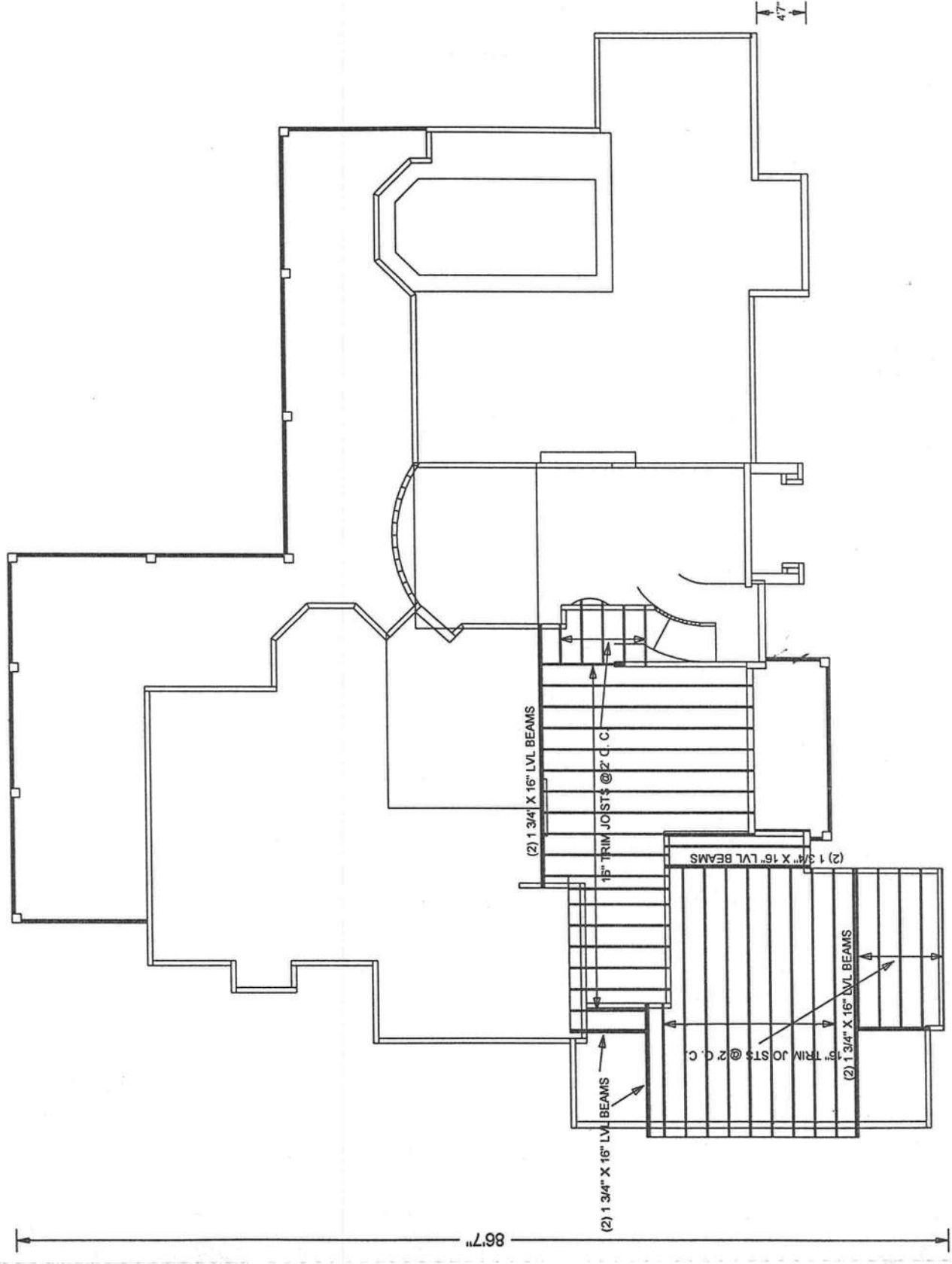


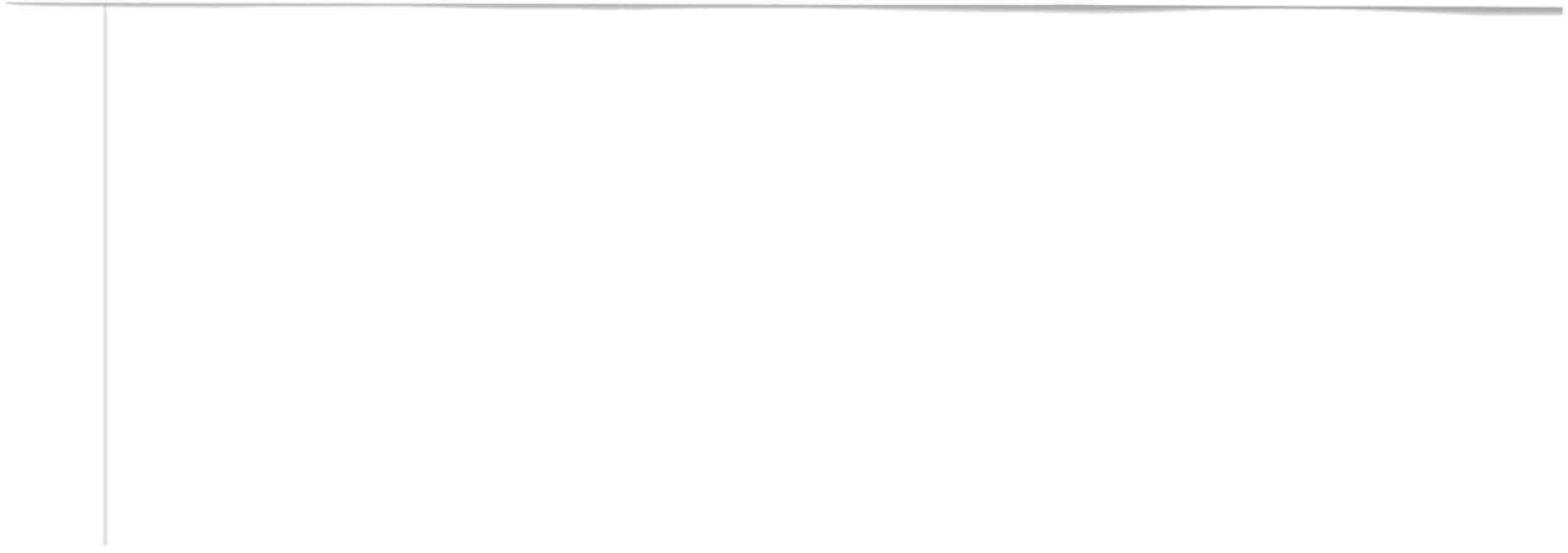
DESIGNED BY:

JOB DESCRIPTION:  
ISAAC CONST/HUBER - FLOOR

JOB LOCATION:

ISAAC CONSTRUCTION/HUBER  
FLOOR





Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

Dead loads are stated on projected horizontal area basis.

(A) Continuous lateral bracing equally spaced on member.

\*\* Wall girder loading on this truss. \*\*

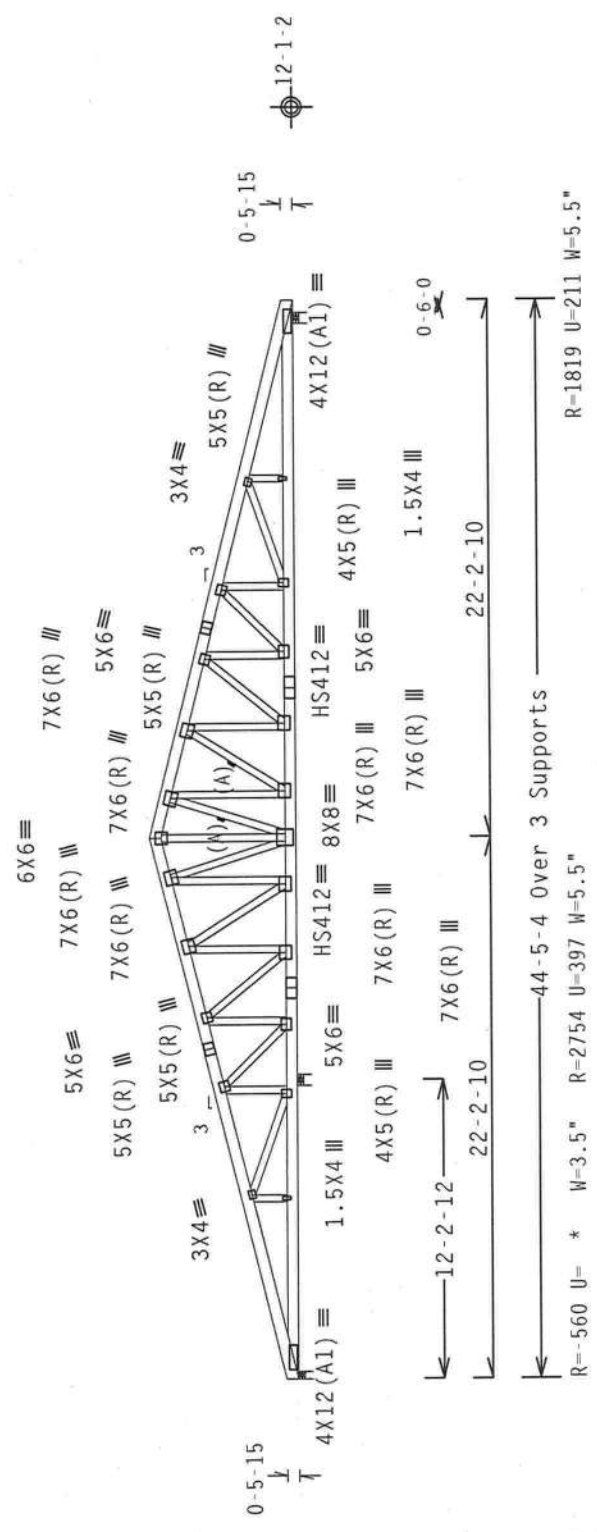
SPECIAL LOADS

---(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 68 PLF at 23.83 to 68 PLF at 32.75  
 BC - From 383 PLF at 23.83 to 383 PLF at 32.75

110 mph wind, 15.36 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

\* PROVIDE CONNECTION FOR 560# UP OR DOWN AT A 1.25 DURATION FACTOR.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS, Wave

Scale = .125" / Ft.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CERUSS PLATE INSTITUTE, 983 D'ONDRED DR., SUITE 200, MADISON, MI 48061 AND NCEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 48061) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI SPEC. ALPINE CONNECTOR PLATES ARE MADE OF 2018/176GA (M-J/S/FX) ASTH A553 GRADE 40/60 OR, POSITION PER DRAWINGS 160A-2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, SHALL BE PER ANEY A3 OF TPI-2002-SEC.3. A SEAL ON THIS AND INSPECTION PLATE SHALL BE PROVIDED. THE SEAL SHALL BE APPLIED TO THE TRUSS COMPONENT AS SHOWN. THE DESIGNER'S ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 1950 Manley Drive  
 Haines City, FL 33844  
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FL / - / 3 / - / - / R / -	FL / - / 3 / - / - / R / -
TC LL 20.0 PSF	REF R487 -- 35423
TC DL 15.0 PSF	DATE 11/09/05
BC DL 10.0 PSF	DRW HCUR487 05313061
BC LL 0.0 PSF	HC-ENG JB/AF
TOT.LD. 45.0 PSF	SEQN- 126836
DUR.FAC. 1.25	JREF- 1SS0487_Z01
SPACING 24.0"	



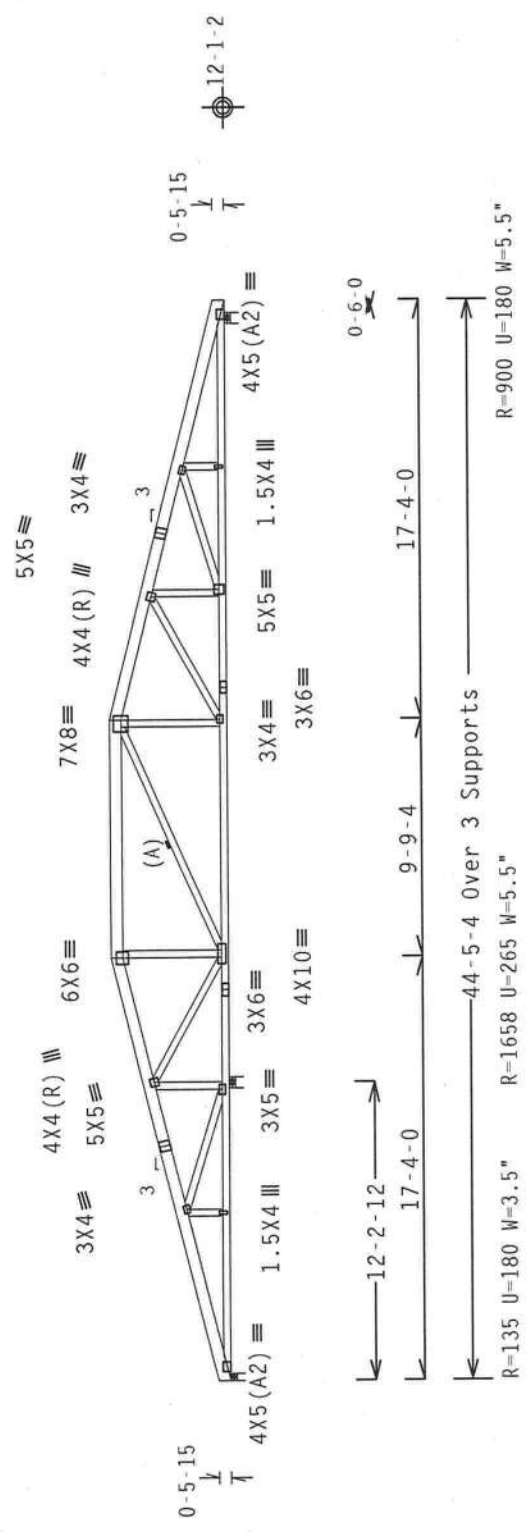
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.75 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

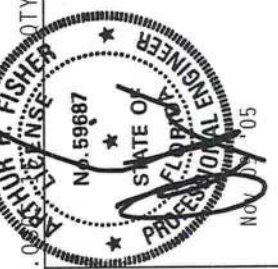
Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .125"/Ft.
TC DL	15.0 PSF	REF R487-- 35424	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313160	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 160781	
SPACING	16.0"	JREF- ISS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, MI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

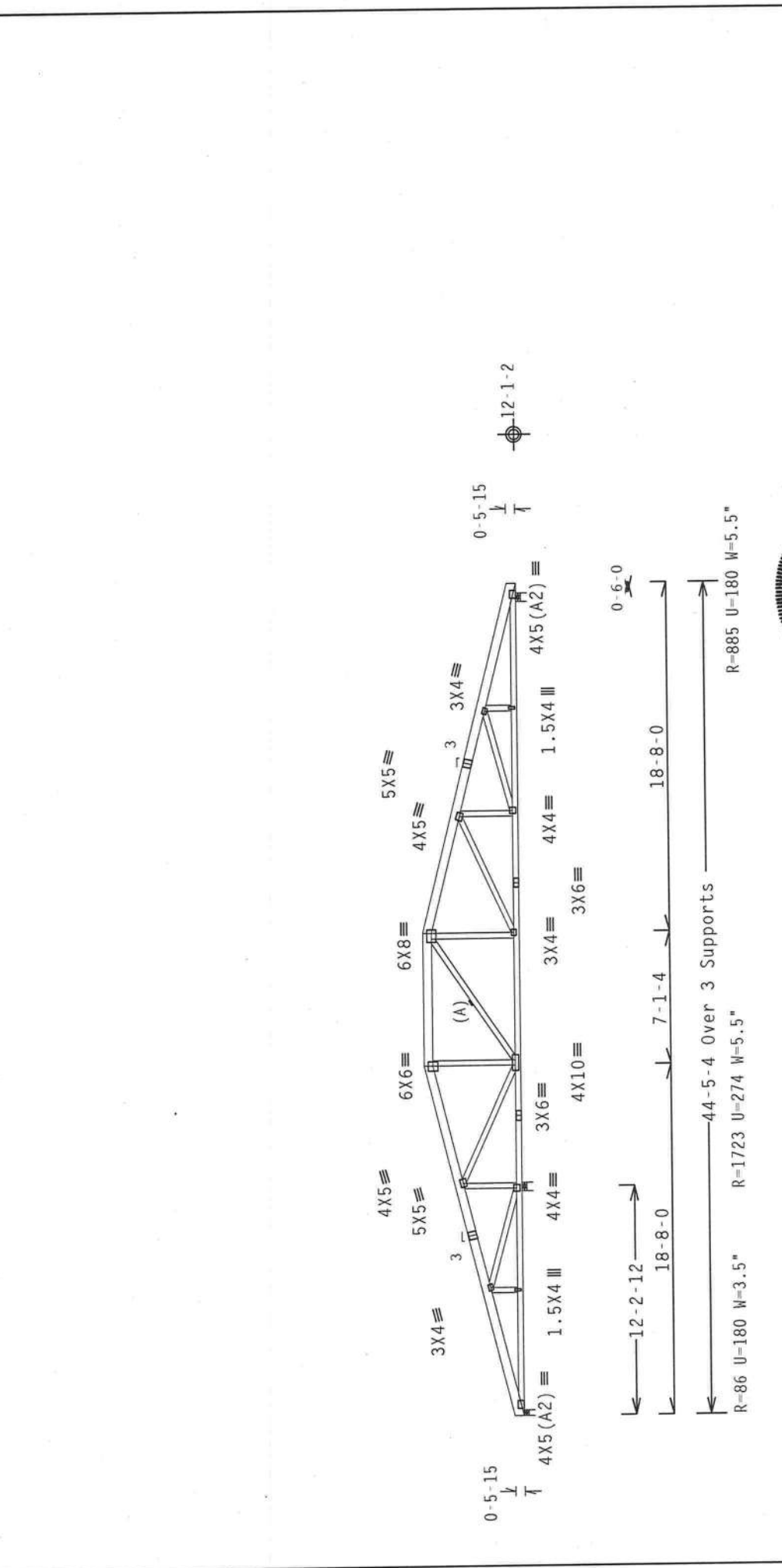
**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES OR TRUSS CHORDS IN CONFORMANCE WITH TPI OR FABRICATING, INSTALLING, HANDLING, SHIPPING AND BRACING OF TRUSSES DESIGN CONFORMS TO THE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AIA/AIA AND TPI. ALPINE TRUSSES SHALL BE MADE OF 20/10/16GA. (H, W/S/3) ASTM A653 GRADE 40/60 (H, W/S/3) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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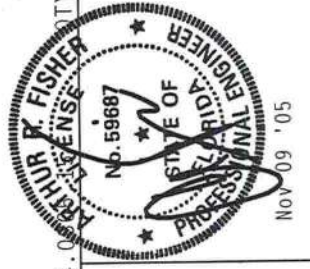
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



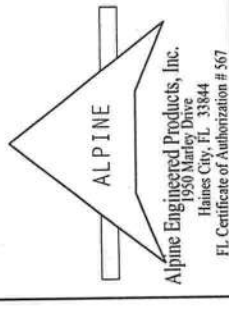
PLT TYP. Wave  
 Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00  
 Scale = .125\"/>

TC LL	20.0 PSF	FL/-/3/-/ - /R/-	REF	R487 -- 35425
TC DL	15.0 PSF		DATE	11/09/05
BC DL	10.0 PSF		DRW	HCUSR487 05313062
BC LL	0.0 PSF		HC-ENG	JB/AF
TOT.LD.	45.0 PSF		SEQN-	160775
DUR.FAC.	1.25			
SPACING	16.0"		JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (PCI DESIGN CONSTRUCTION SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 MADISON, MI 48320) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 48320) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (NATIONAL DESIGN SPEC. BY ARPA) AND TPI SPEC. APPLICABLE CONNECTOR PLATES ARE MADE OF 2019/16GA (4-H/5K) ASH ADDS GRADE 40760 (40/45) GRADE APPLICABLE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED OR IDENTIFIED, PERFORM PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE ACCEPTABLE TO THE DESIGNER. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE DESIGN. THE DESIGNER'S RESPONSIBILITY IS SOLELY FOR THE TRUSS COMPONENT AND NOT FOR THE INSTALLATION AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AHSI/TPI 1 SEC. 2.





12-344-ISAAC CONST/RODR - K001 - H20A7

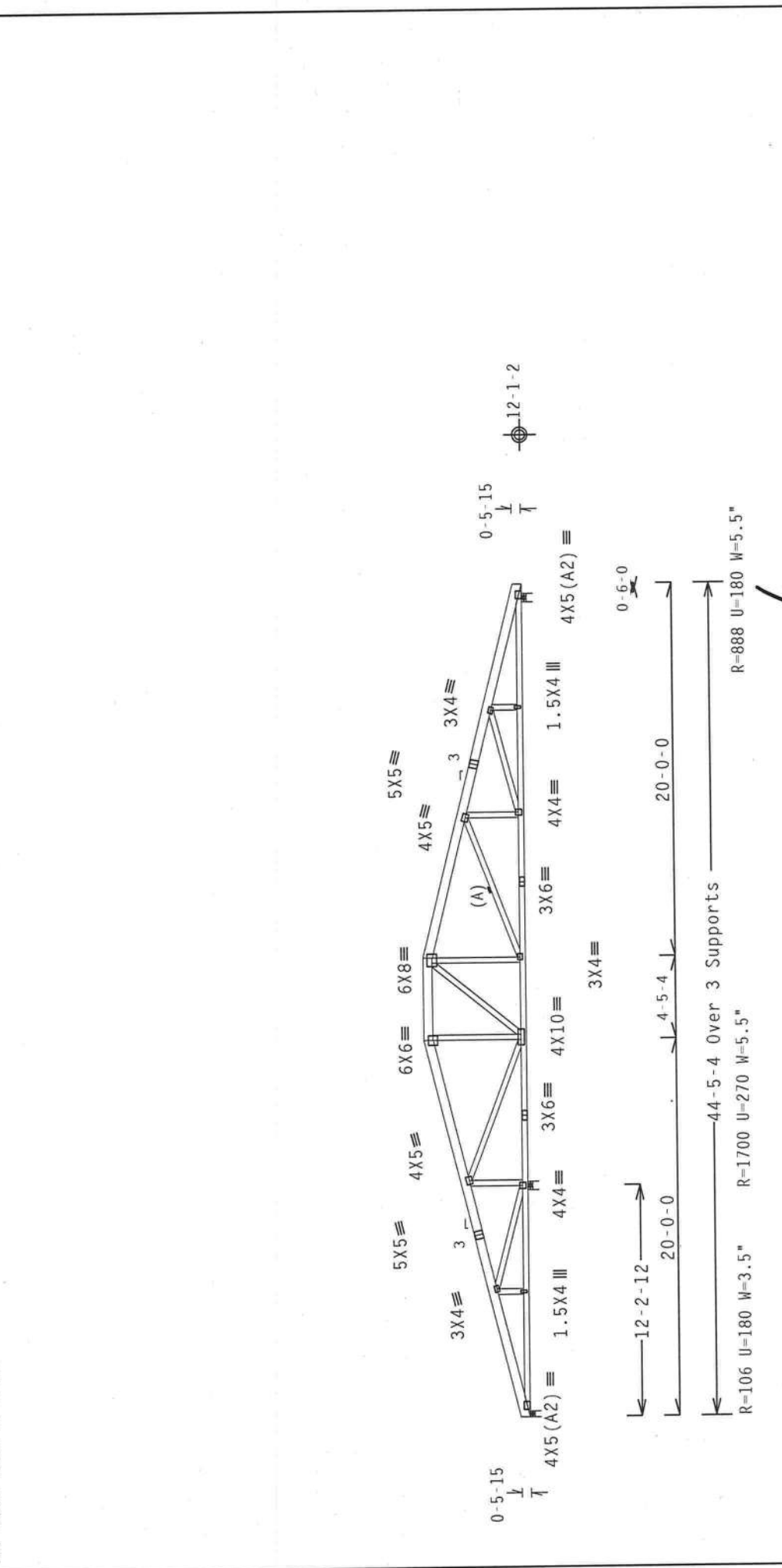
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.09 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind IC DL-5.0 psf, wind BC DL-5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

Scale = .125"/Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	REF	R487 -- 35426
TC DL	15.0 PSF		DATE	11/09/05
BC DL	10.0 PSF		DRW	HCUSR487 05313063
BC LL	0.0 PSF		HC-ENG	JB/AF
TOT.LD.	45.0 PSF		SEQN-	160770
DUR.FAC.	1.25			
SPACING	16.0"		JREF-	1SS0487_Z01

ALPINE ENGINEERED PRODUCTS, INC.  
 1950 Manley Drive  
 Gaines City, FL 33844  
 FL Certificate of Authorization # 567

ALPINE ENGINEERED PRODUCTS, INC. PROFESSIONAL ENGINEER  
 No. 59887  
 STATE OF FLORIDA  
 NOV 09 '05

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CORRECTIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND AISC STEEL CONNECTOR PLATES ARE MADE OF 2018/166A (4-HY/5X) A572 GRADE 40/50 (40-50) STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, PER AISC PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTABLE PROFESSIONAL ENGINEERING RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CORRECTIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND AISC STEEL CONNECTOR PLATES ARE MADE OF 2018/166A (4-HY/5X) A572 GRADE 40/50 (40-50) STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, PER AISC PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTABLE PROFESSIONAL ENGINEERING RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CORRECTIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND AISC STEEL CONNECTOR PLATES ARE MADE OF 2018/166A (4-HY/5X) A572 GRADE 40/50 (40-50) STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, PER AISC PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTABLE PROFESSIONAL ENGINEERING RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



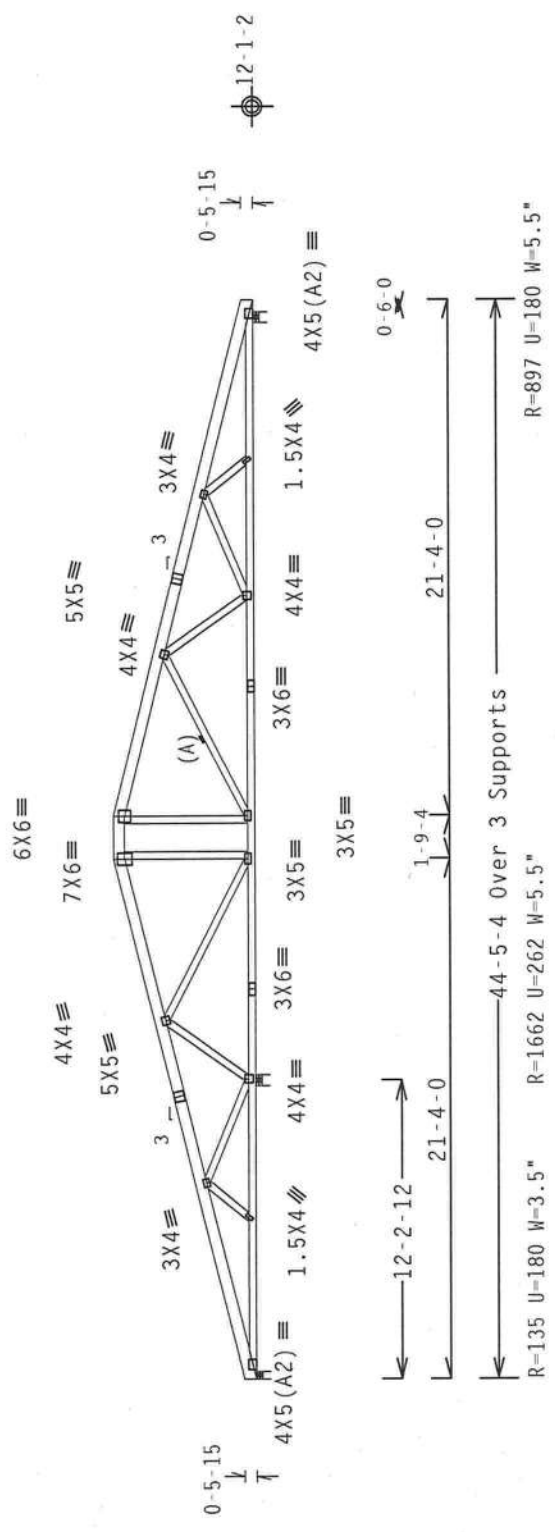
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.25 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

FL/-/3/-/R/-

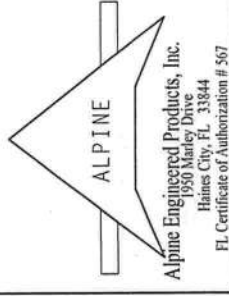
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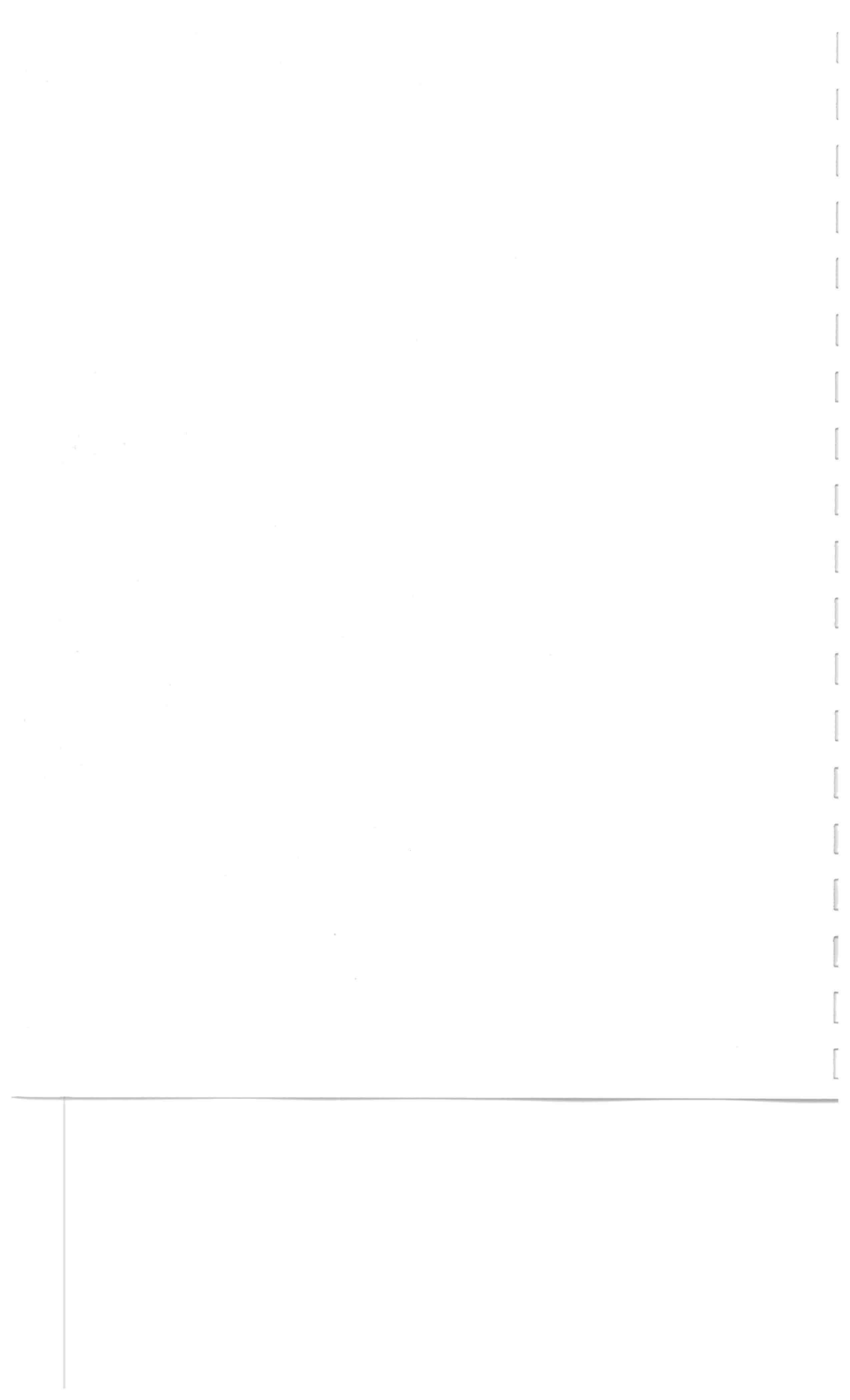
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TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313064
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	45.0 PSF	SEQN	160763
DUR.FAC.	1.25		
SPACING	16.0"	JREF	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE AND INTERLOCKING JOINT MANUFACTURERS ASSOCIATION (TPA) FOR THE LATEST REVISIONS AND PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BULLDO THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H, H/S/S) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16DA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





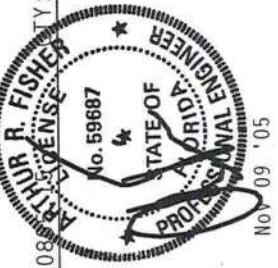
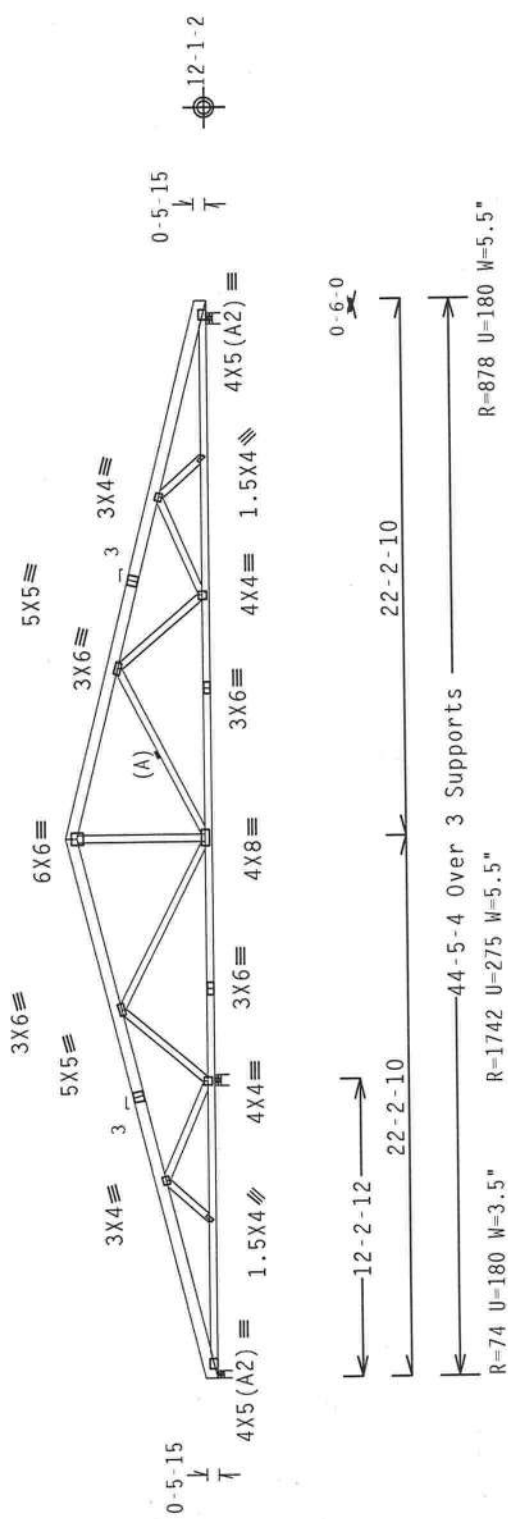
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.36 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 5.83 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

Scale = .125" / Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - 35428
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313065
TOT. LD.	45.0 PSF	HC-ENG JB/AF
DUR. FAC.	1.25	SEQN- 160791
SPACING	16.0"	JREF- 1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC65 1-03 (BUILDING COMPONENT SAFETY INFORMATION), THE LEISURE RITE TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53703 AND THE TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES TO BE FOLLOWED IN PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PAJ AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/H/S/P) ASTM A653 GRADE 40/60 (M. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. NO Z-ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002(STD) AND (2) SHALL BE PER ANNEX A3 OF TPI-2002(STD). DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 1950 Nalley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



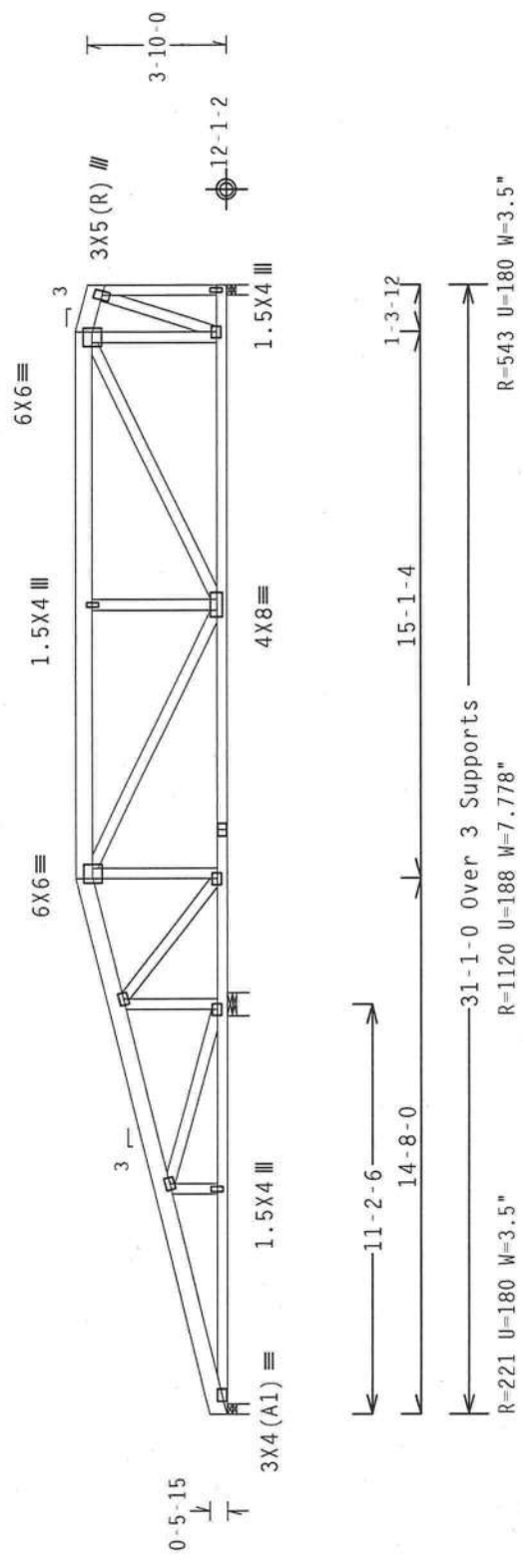
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.42 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor  
 for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00

Scale = .1875" / Ft.



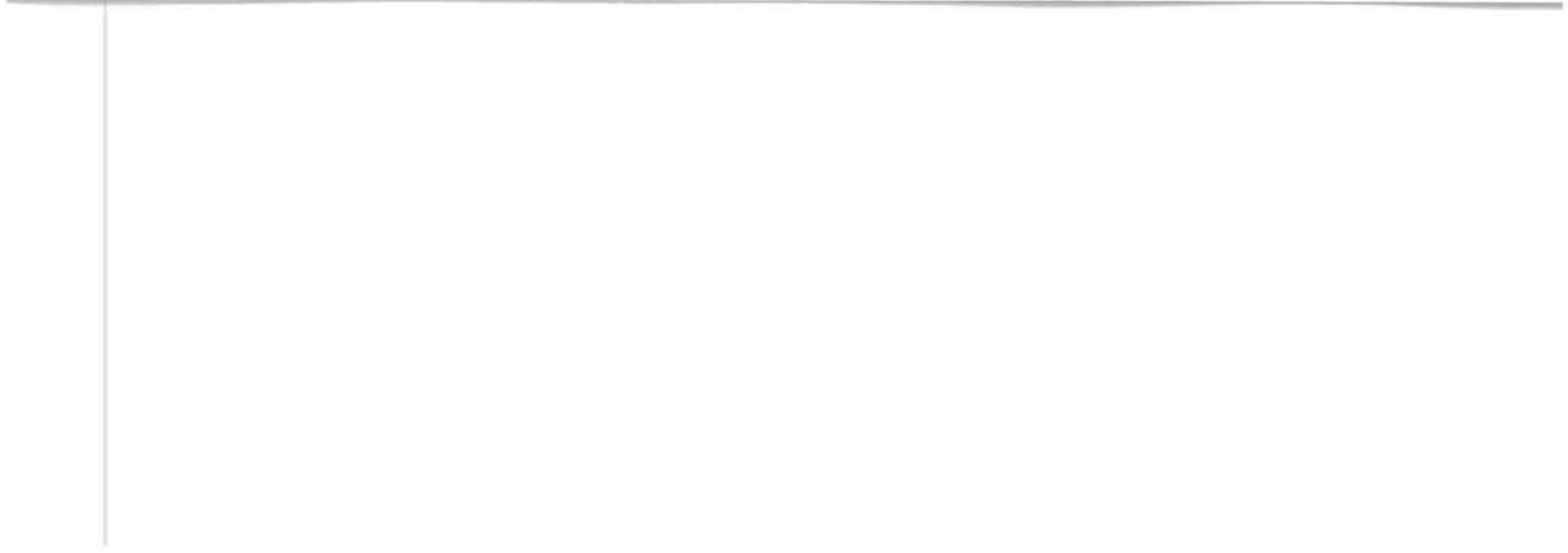
TC LL	20.0 PSF	FL/-/3/-/R/-
TC DL	15.0 PSF	REF R487-- 35429
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313066
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN 126704
SPACING	16.0"	JREF- ISS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISI (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018T/16GA (8-M/5/3) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISI 1002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUEPLY FOR THE AISC COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISI (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018T/16GA (8-M/5/3) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISI 1002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUEPLY FOR THE AISC COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.

ALPINE

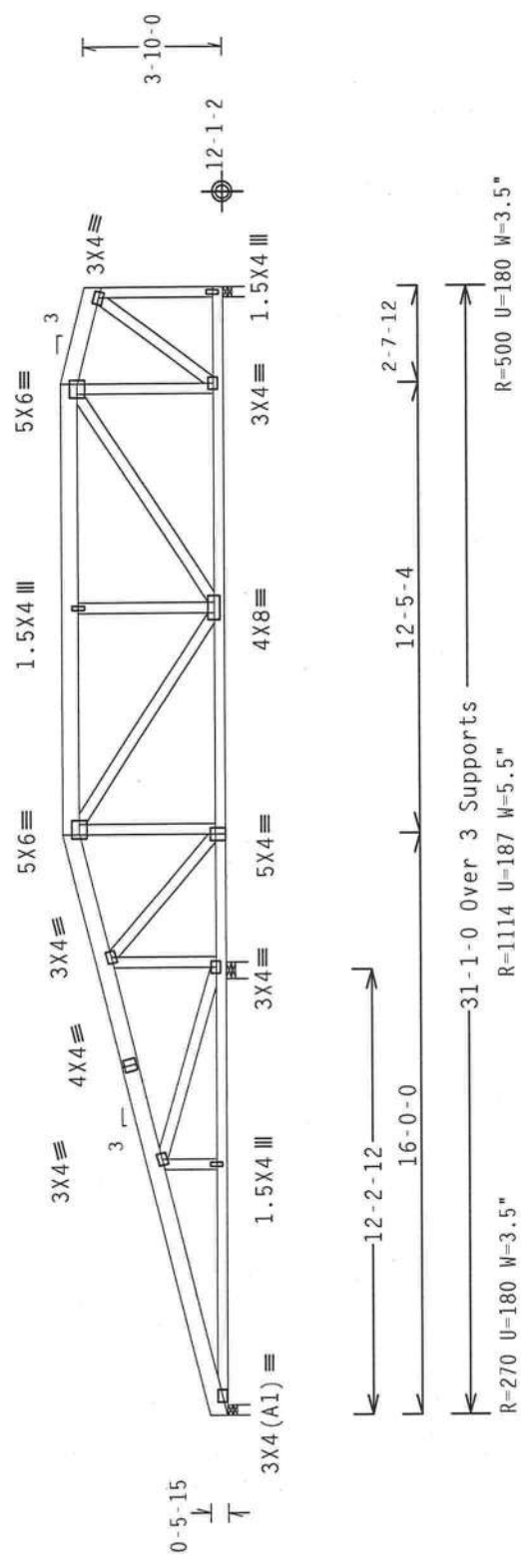
Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



110 mph wind, 14.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



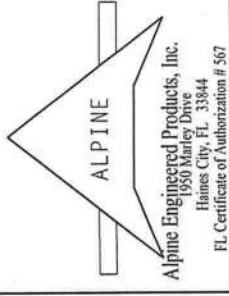
PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00  
 Scale = .1875" / Ft.

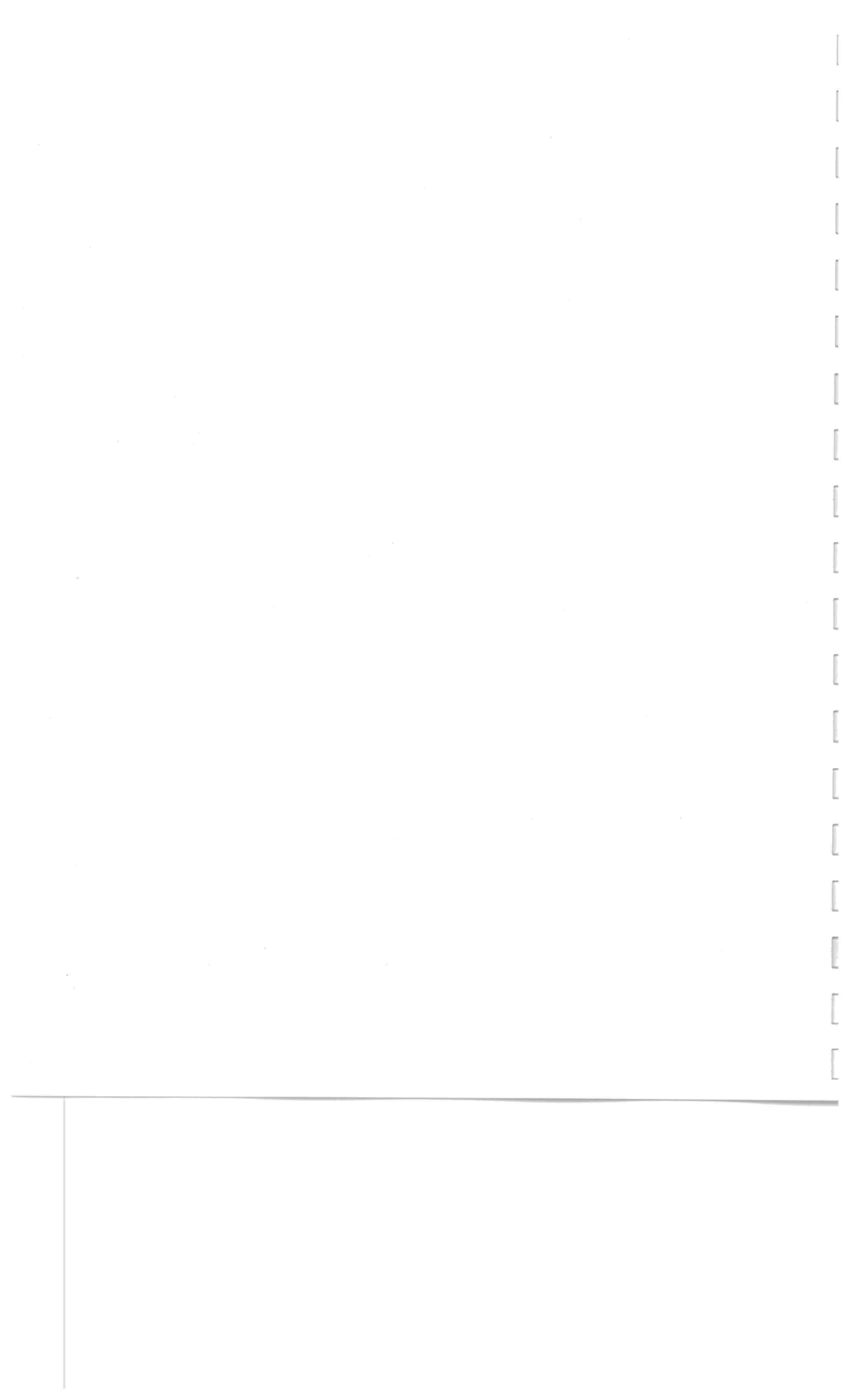


TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - - 35430
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313067
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN- 160727
SPACING	16.0"	JREF- 1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 B'ONOFILO DR., SUITE 200, MADISON, MI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN CONNECTION WITH THE PROVISIONS OF THIS NATIONAL DESIGN SPEC. BY ACPA) AND TPI. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





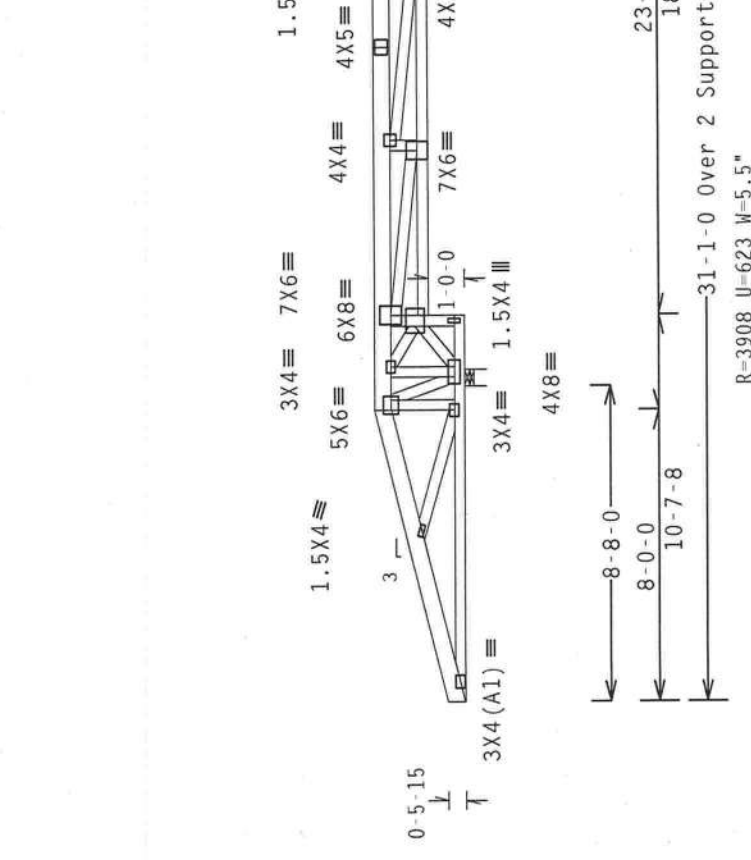
### 2 COMPLETE TRUSSES REQUIRED

Mailing Schedule: (10d Common\_(0.148"x3",\_min.)\_nails)  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

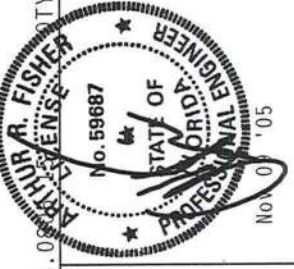
110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 #1 hip supports 8-0-0 jacks with no webs.

Right end vertical not exposed to wind pressure.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=2135 U=341 W=3.5"

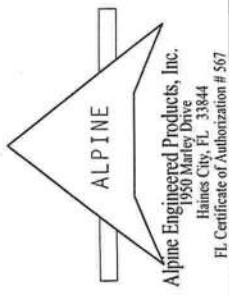


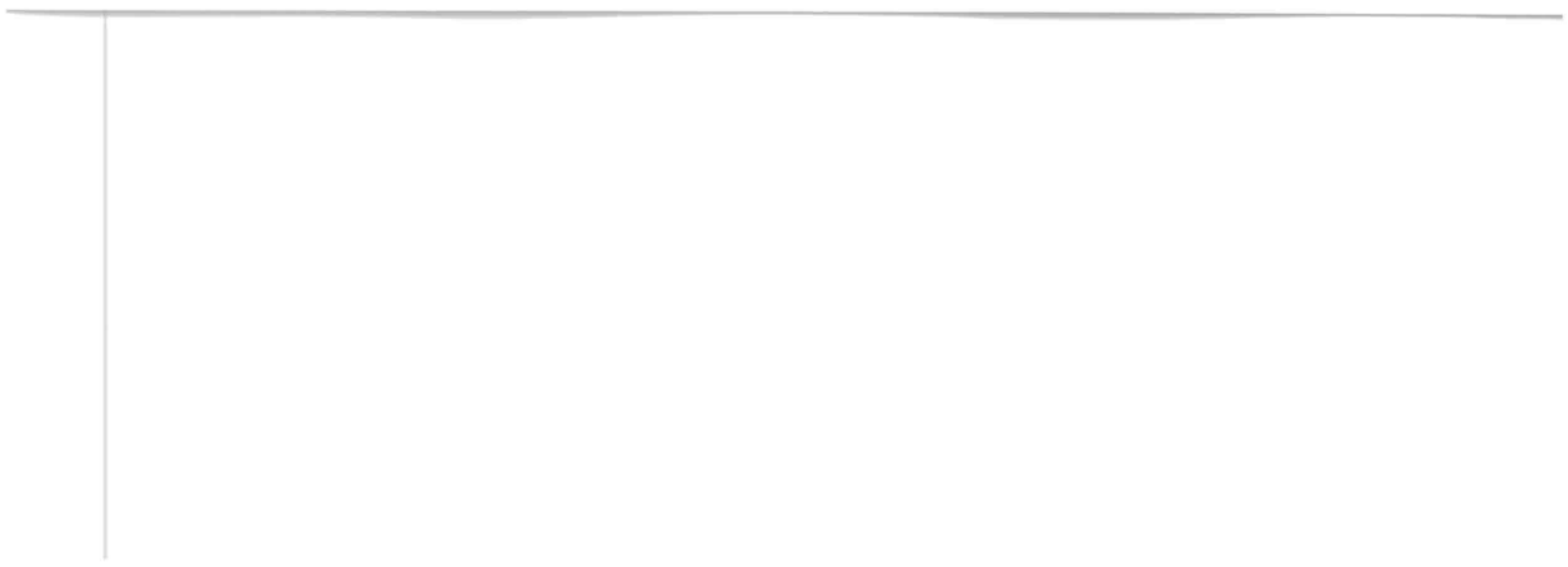
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04,08

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BECS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONDRILO DR., WHITE ZOO, MADISON, WI 53719) AND NITCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FABRICATOR OF TRUSSES IN CONFORMANCE WITH THESE PROVISIONS OF THIS NATIONAL DESIGN SPEC. BY ACP/A AND TPI SHALL BE RESPONSIBLE FOR THE PROVISIONS OF THIS NATIONAL DESIGN SPEC. BY ACP/A AND TPI. CONNECTOR PLATES ARE MADE OF 2010/16GA (4-H/5/3) ASTM A653 GRADE 40/60 (K/HS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave	FL/-/3/-/R/-	Scale = .1875" / Ft.
	TC LL	20.0 PSF
	TC DL	15.0 PSF
	BC DL	10.0 PSF
	BC LL	0.0 PSF
	TOT.LD.	45.0 PSF
	DUR.FAC.	1.25
	SPACING	16.0"
	REF	R487-- 35431
	DATE	11/09/05
	DRW	HCUR487 05313068
	HC-ENG	JB/AF
	SEQN-	126636
	JREF-	ISS0487_Z01





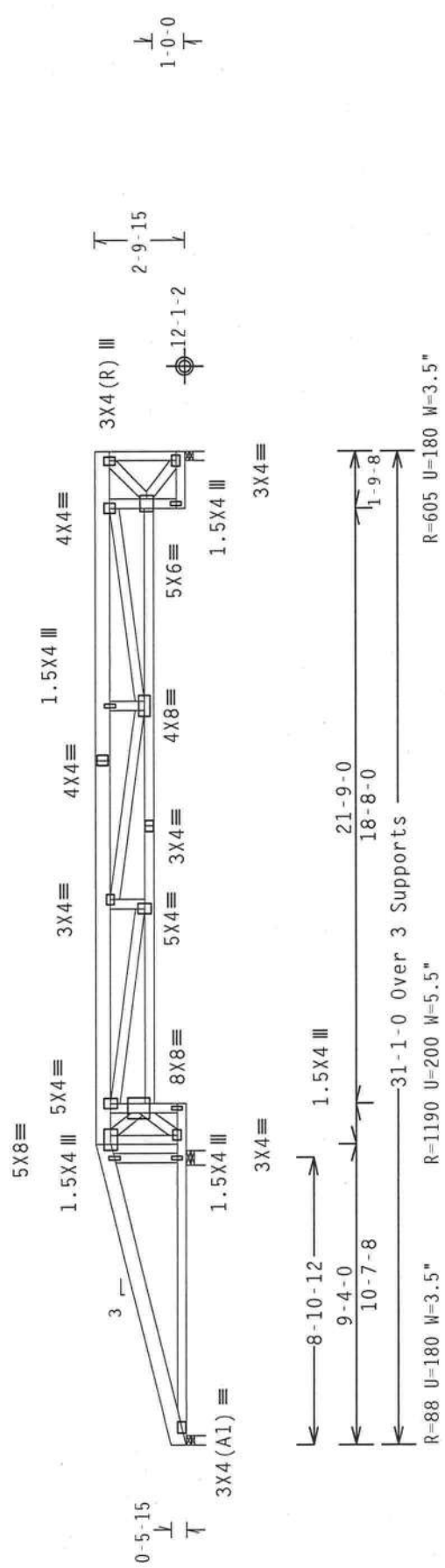
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 13.75 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

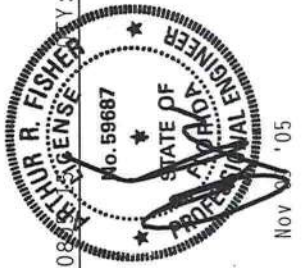
Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

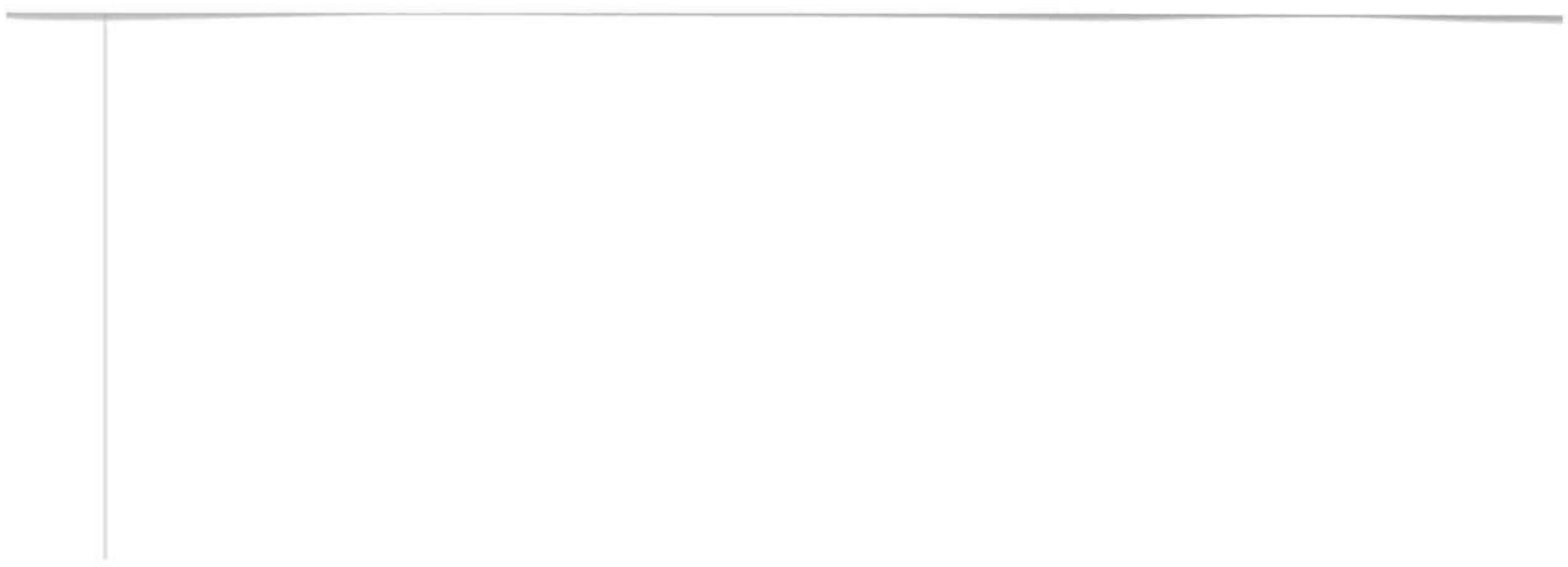
REF	R487--	35432
DATE	11/09/05	
DRW	HCUSR487	05313069
HC-ENG	JB/AF	
SEQN-	126667	
DUR.FAC.	1.25	
SPACING	16.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION) FOR THE PROPER PLACEMENT OF THE TRUSSES. CHECK THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR THE CORRECT CONNECTIONS AND BRACING. THE TRUSSES SHOULD BE STORED AND HANDLED IN A MANNER THAT WILL PREVENT DAMAGE TO THE TRUSSES. THE TRUSSES SHOULD BE STORED UPRIGHT AND PROTECTED FROM WEATHER. THE TRUSSES SHOULD BE STORED ON A LEVEL SURFACE AND SHOULD BE PROTECTED FROM DAMAGE TO THE TOP CHORDS. THE TRUSSES SHOULD BE STORED IN A MANNER THAT WILL PREVENT DAMAGE TO THE TOP CHORDS. THE TRUSSES SHOULD BE STORED IN A MANNER THAT WILL PREVENT DAMAGE TO THE TOP CHORDS.

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ALPINE  
 Alpine Engineered Products, Inc.  
 Heines City, FL 33844  
 FL Certificate of Authorization # 567

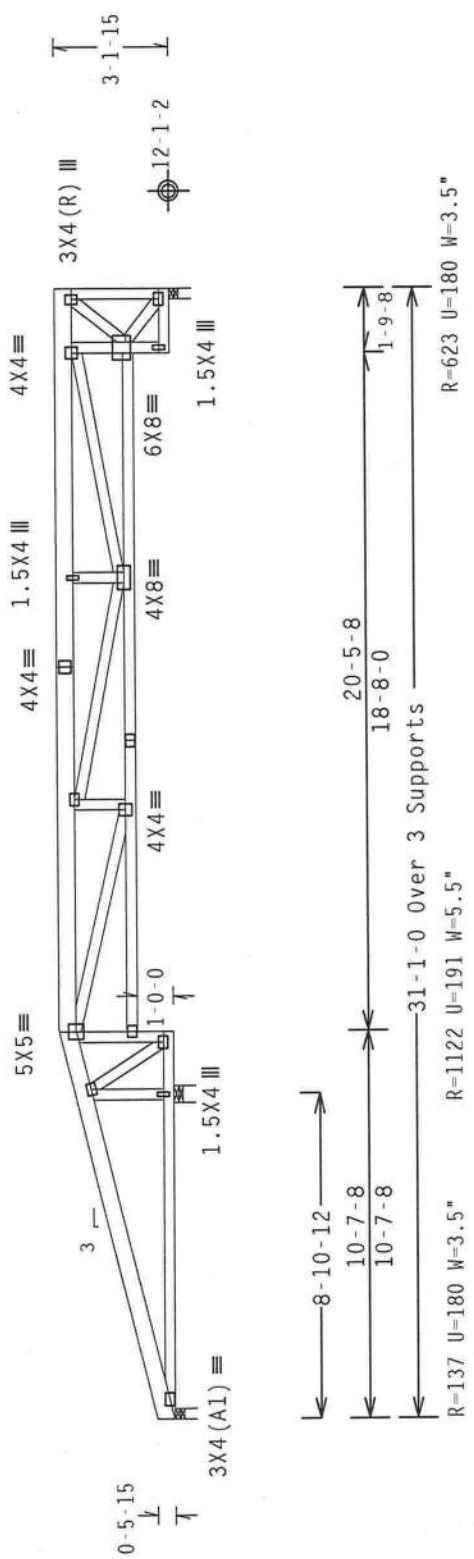


110 mph wind, 13.92 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

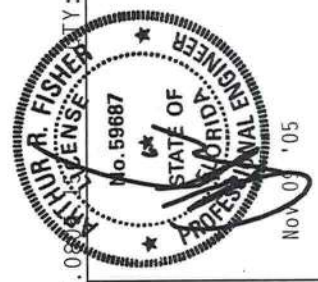
Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

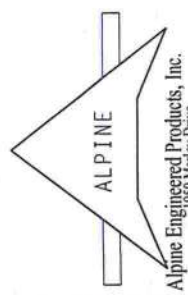


Note: All Plates Are 3X4 Except As Shown.  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08  
 PLT TYP. Wave

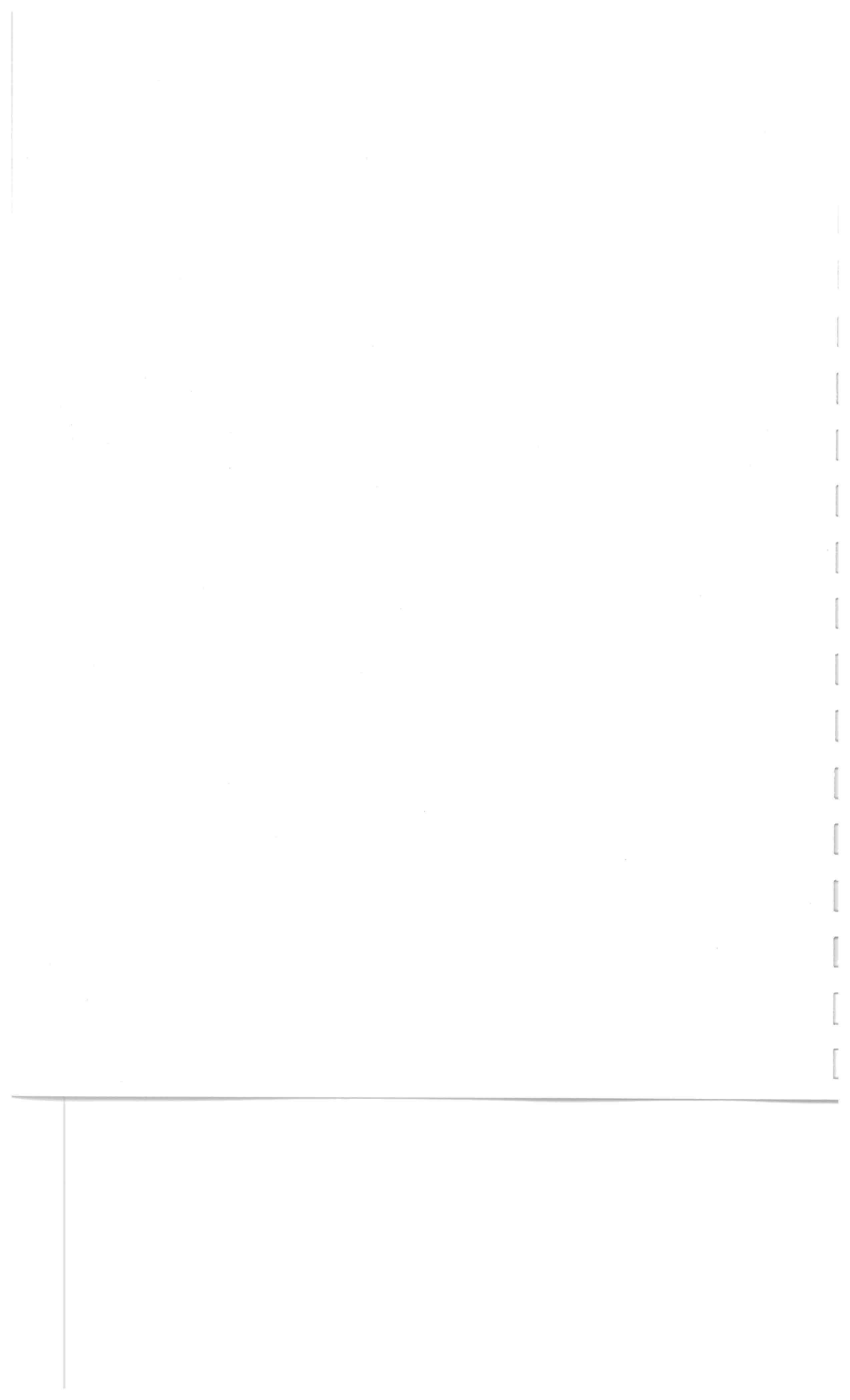


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FL / - / 3 / - / - / R / -	Scale = .1875" / Ft.
TC LL 20.0 PSF	REF R487 -- 35433
TC DL 15.0 PSF	DATE 11/09/05
BC DL 10.0 PSF	DRW HCUSR487 05313070
BC LL 0.0 PSF	HC-ENG JB/AF
TOT.LD. 45.0 PSF	SEQN- 126675
DUR.FAC. 1.25	
SPACING 16.0"	JREF - 1SS0487_Z01



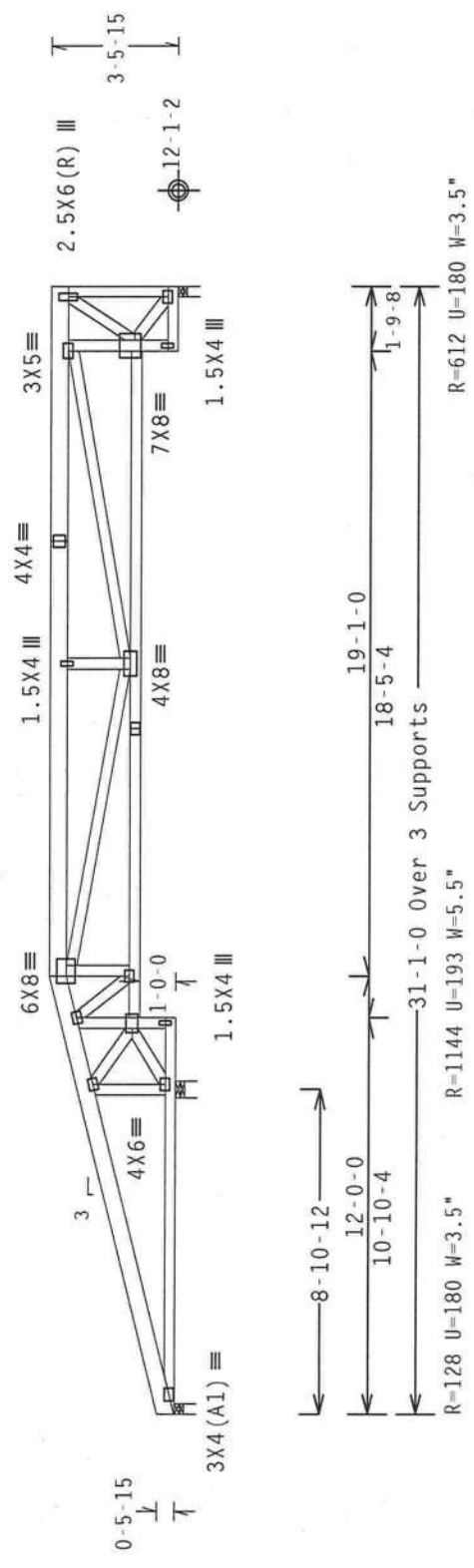
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.09 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor  
 for dead load is 1.50.



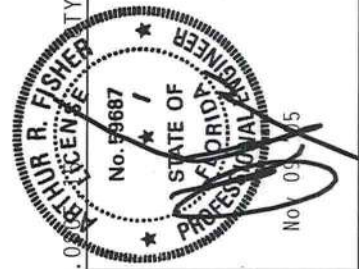
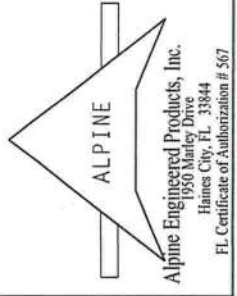
Note: All Plates Are 3X4 Except As Shown.  
 Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

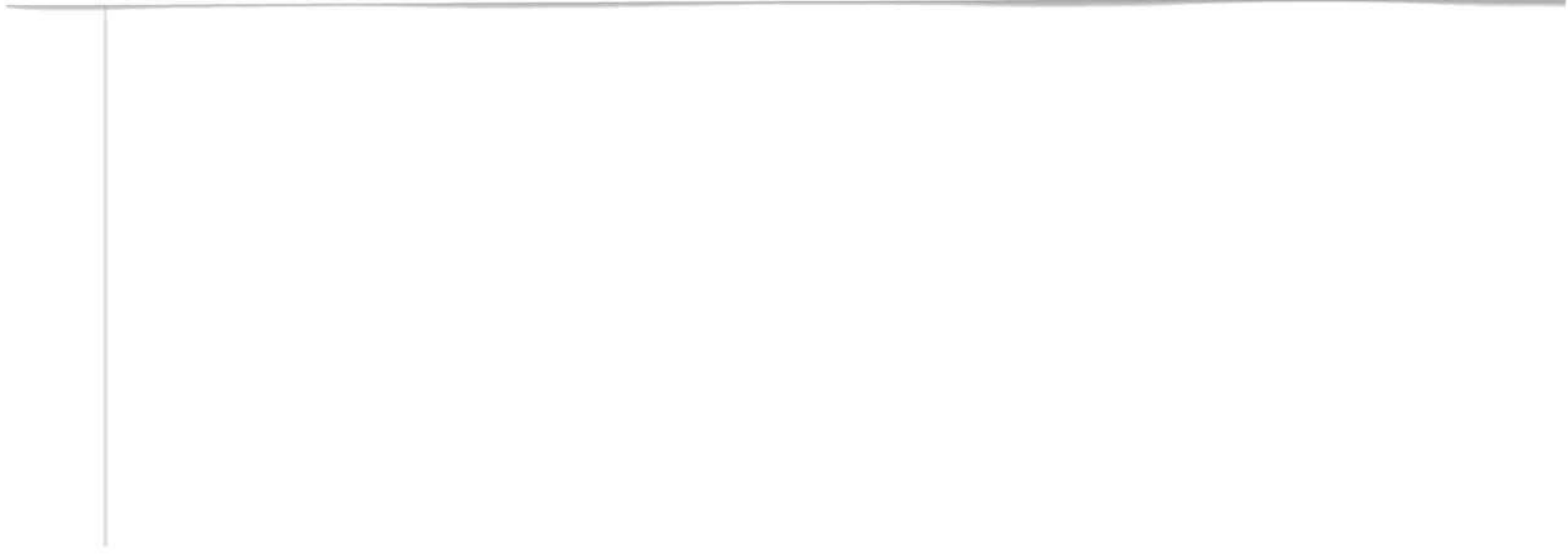
Scale = .1875" / Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - - 35434
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313071
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN- 126686
SPACING	16.0"	JREF- 1SS0487_Z01

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Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

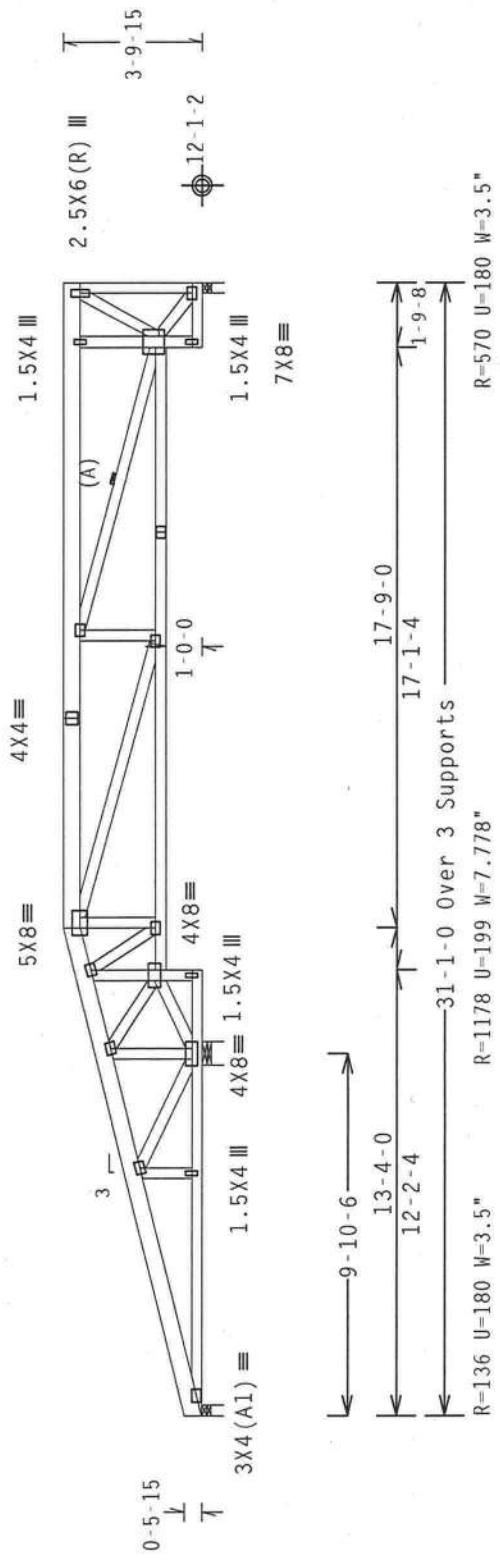
110 mph wind, 14.25 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002 (STD) / FBC

Cq/RT=1.00(1.25)/10(0)

7.04.08

FL/-/3/-/-/R/-

Scale = .1875" / Ft.

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

REF	R487 --	35435
DATE	11/09/05	
DRW	HCUSR487	05313072
HC-ENG	JB/AF	
SEQN-	126695	
JREF-	1SS0487_Z01	



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 Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 403 O'CONNOR DR., SUITE 200, MADISON, WI 53719, AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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(5) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.5. (6) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.6. (7) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.7. (8) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.8. (9) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.9. (10) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.10. (11) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.11. (12) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.12. (13) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.13. (14) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.14. (15) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.15. (16) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.16. (17) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.17. (18) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.18. (19) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.19. (20) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.20. (21) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.21. (22) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.22. (23) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.23. (24) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.24. (25) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.25. (26) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.26. (27) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.27. (28) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.28. (29) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.29. (30) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.30. (31) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.31. (32) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.32. (33) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.33. (34) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.34. (35) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.35. (36) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.36. (37) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.37. (38) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.38. (39) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.39. (40) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.40. (41) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.41. (42) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.42. (43) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.43. (44) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.44. (45) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.45. (46) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.46. (47) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.47. (48) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.48. (49) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.49. (50) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.50. (51) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.51. (52) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.52. (53) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.53. (54) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.54. (55) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.55. (56) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.56. (57) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.57. (58) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.58. (59) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.59. (60) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.60. 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(94) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.94. (95) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.95. (96) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.96. (97) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.97. (98) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.98. (99) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.99. (100) SHALL BE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 308, SECTION 10.1.100.



## 2 COMPLETE TRUSSES REQUIRED

Mailing Schedule: (10d Common\_(0.148"x3"\_.min.)\_nails)  
 Top Chord: 1 Row @ 8.00" o.c.  
 Bot Chord: 1 Row @ 12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

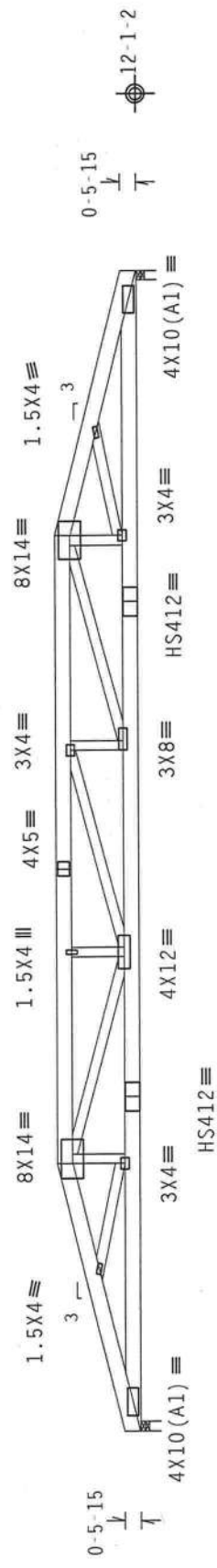
#1 hip supports 8-0-0 jacks with no webs.

Calculated vertical deflection is 0.45" due to live load and 0.85" due to dead load at X = 14-3-15.

Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3  
 110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



8-0-0      18-10-0      8-0-0  
 34-10-0 Over 2 Supports  
 R=3234 U=516 W=3.5"      R=3234 U=516 W=3.5"

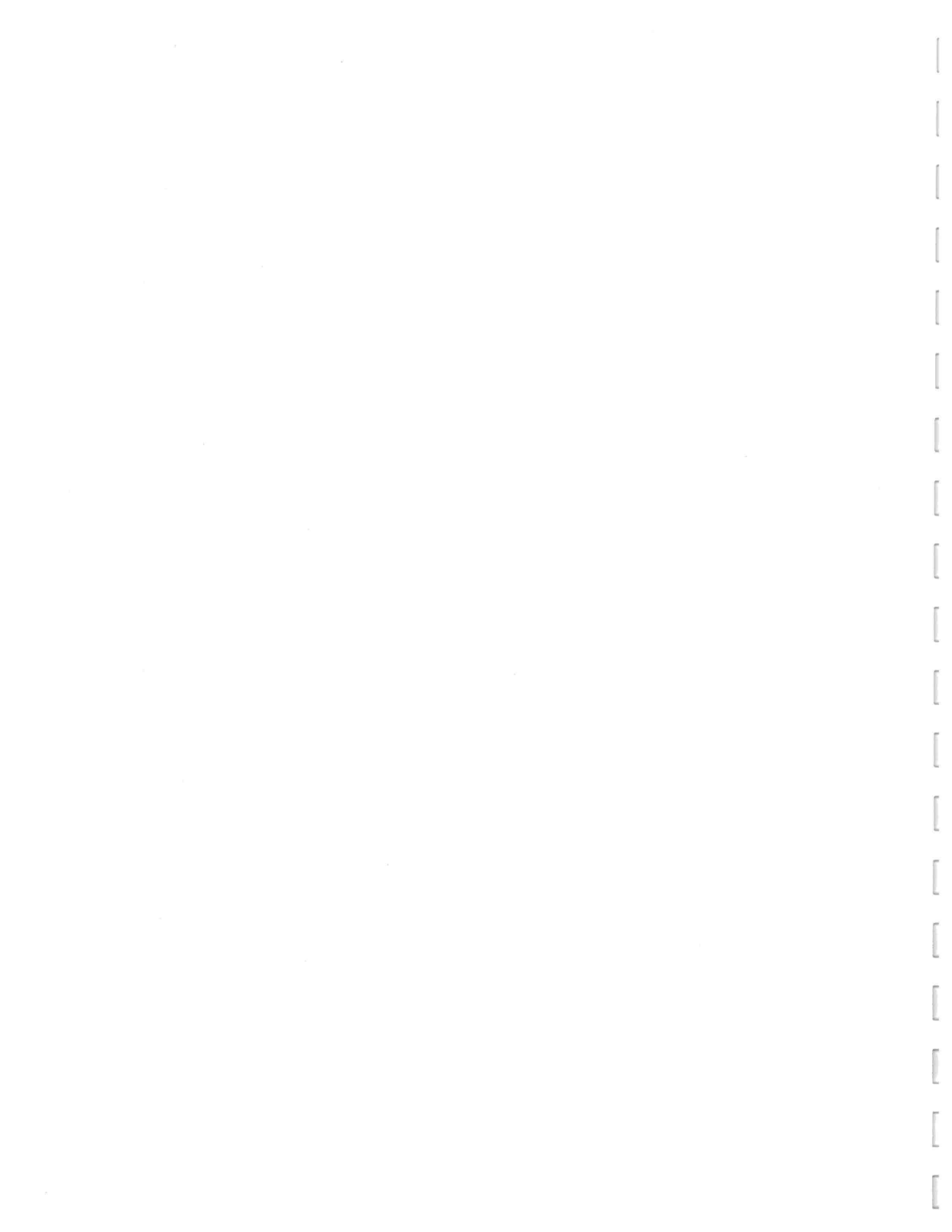
PLT TYP. 20 Gauge HS, Wave	Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)	7.04.0	FL/-/3/-/-/R/-	Scale = .1875" / Ft.
		TC LL	20.0 PSF	REF R487 - 35436
		TC DL	15.0 PSF	DATE 11/09/05
		BC DL	10.0 PSF	DRW HCUSR487 05313073
		BC LL	0.0 PSF	HC-ENG JB/AF
		TOT.LD.	45.0 PSF	SEQN- 126148
		DUR.FAC.	1.25	JREF- 1SS0487_Z01
		SPACING	16.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 483 D'ONOFRELO DR., SUITE 200, MADISON, WI 53719) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL ACCEPT RESPONSIBILITY FOR THE DESIGN AND FABRICATION OF THE TRUSSES. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE TRUSSES. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE TRUSSES. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE TRUSSES. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE TRUSSES.

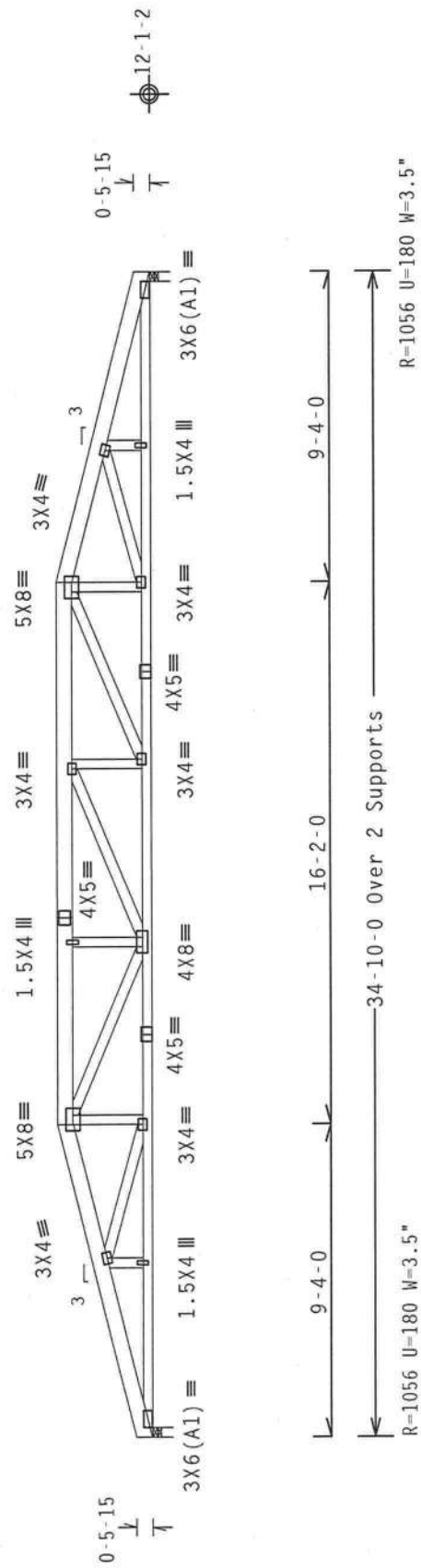
**ALPINE**  
 Alpine Engineered Products, Inc.  
 1950 Malaria Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 13.75 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase  
 factor for dead load is 1.50.



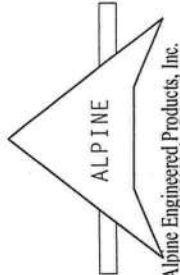
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

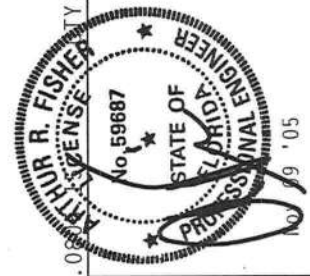
Scale = .1875" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE) 4000 W. GARDNER DR., SUITE 2000, MIDLAND, TX 79701-4000 FOR THE LATEST EDITIONS OF THE TPI TRUSS MANUFACTURING PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

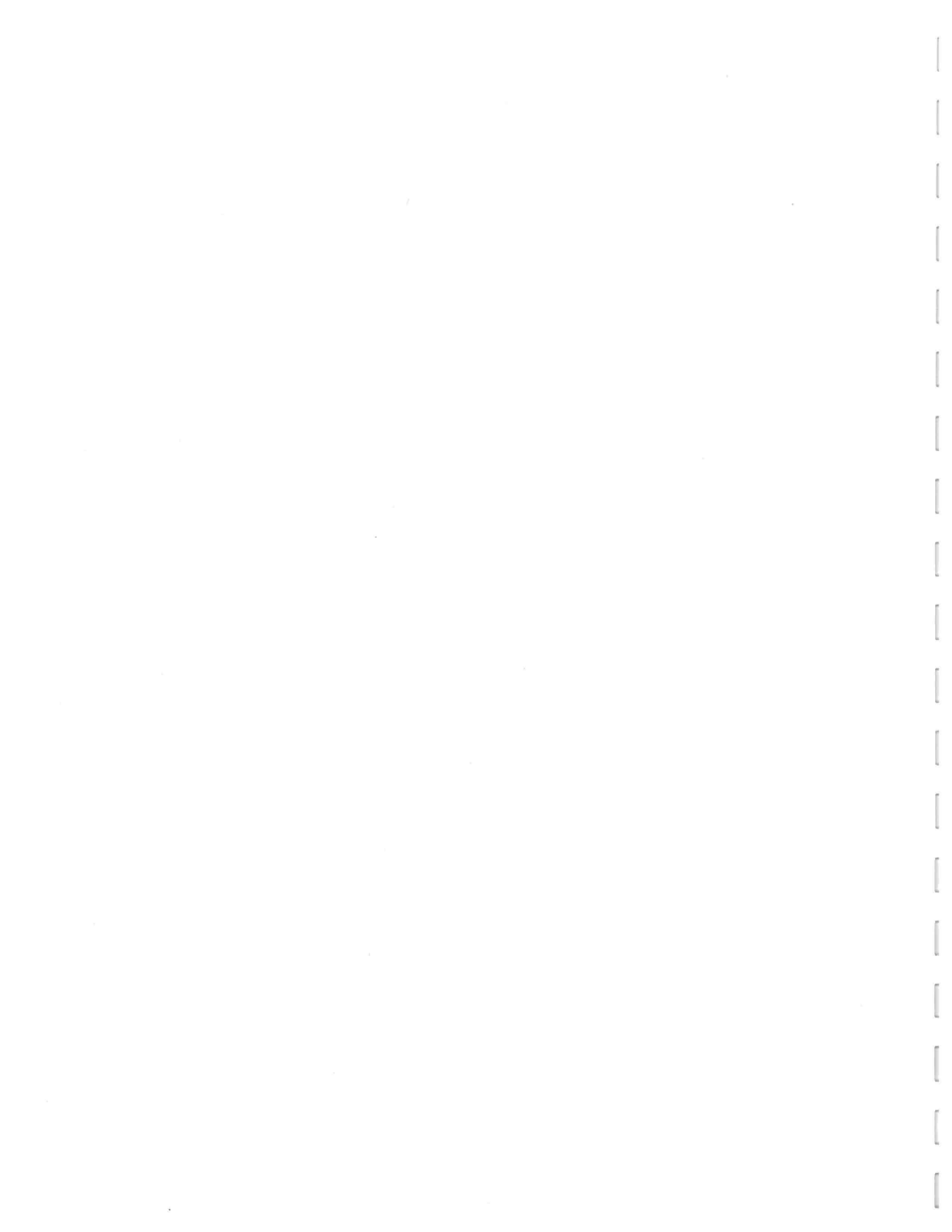
**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H-/H/S/K) ASTM A653 GRADE 40/60 (H, K/HI-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANSI A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - 35437
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUR487 05313159
TOT.LD.	45.0 PSF	HC-ENG JB/AF *
DUR.FAC.	1.25	SEQN- 126153
SPACING	16.0"	JREF- ISS0487_Z01

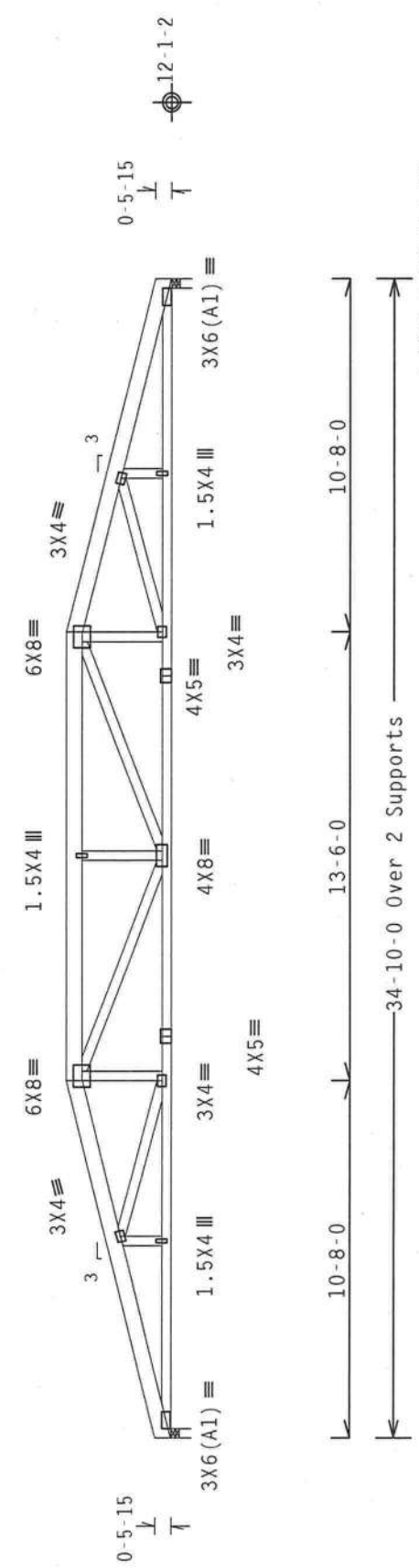


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 13.92 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487--	35438
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313001
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	126158	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS AS SHOWN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH WTC AND WTC PROVIDE THE FOLLOWING INFORMATION: 1. ALL TRUSS MEMBER CONNECTIONS SHALL BE MADE OF 20/18/16GA OR 16/5/13 ASTM A653 GRADE 40/40 (60 K/H.S) GALV. STEEL. 2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. 3. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. 4. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. 5. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.

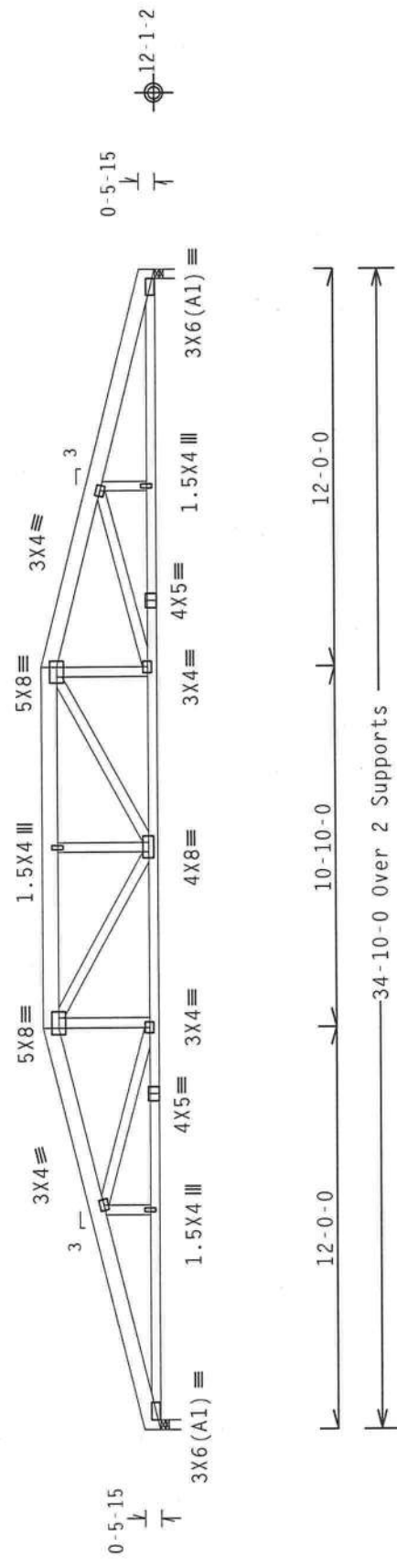


110 mph wind, 14.09 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

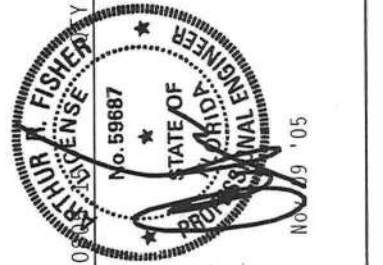


PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .1875"/Ft.

TC LL	20.0 PSF	FL/-/3/-/-/R/-
TC DL	15.0 PSF	REF R487-- 35439
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313002
TOT.LD.	45.0 PSF	HC-ENG JB/AF *
DUR.FAC.	1.25	SEQN- 126163
SPACING	16.0"	JREF- ISS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PRODUCTS, INC.), 1000 PINE BLVD., SUITE 200, MADISON, WI 53710, AND WFLA (WISCONSIN FABRICATING LOGS ENTERPRISE, INC.), 1000 PINE BLVD., SUITE 200, MADISON, WI 53710 FOR SAFETY PRACTICES REGARDING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI'S OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/M/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ARIEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1. SEC. 2.



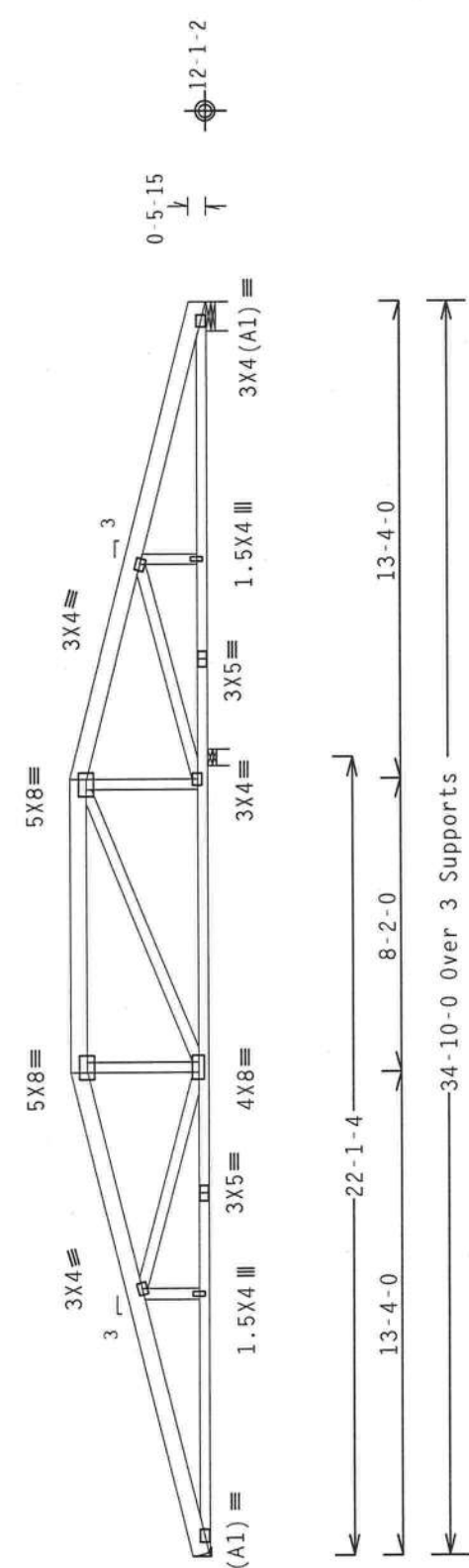
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 14.25 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



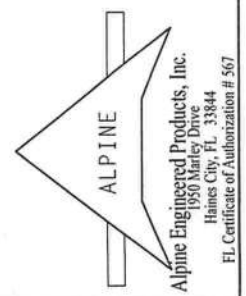
R=1078 U=180 W=5.5" R=371 U=180 W=10"

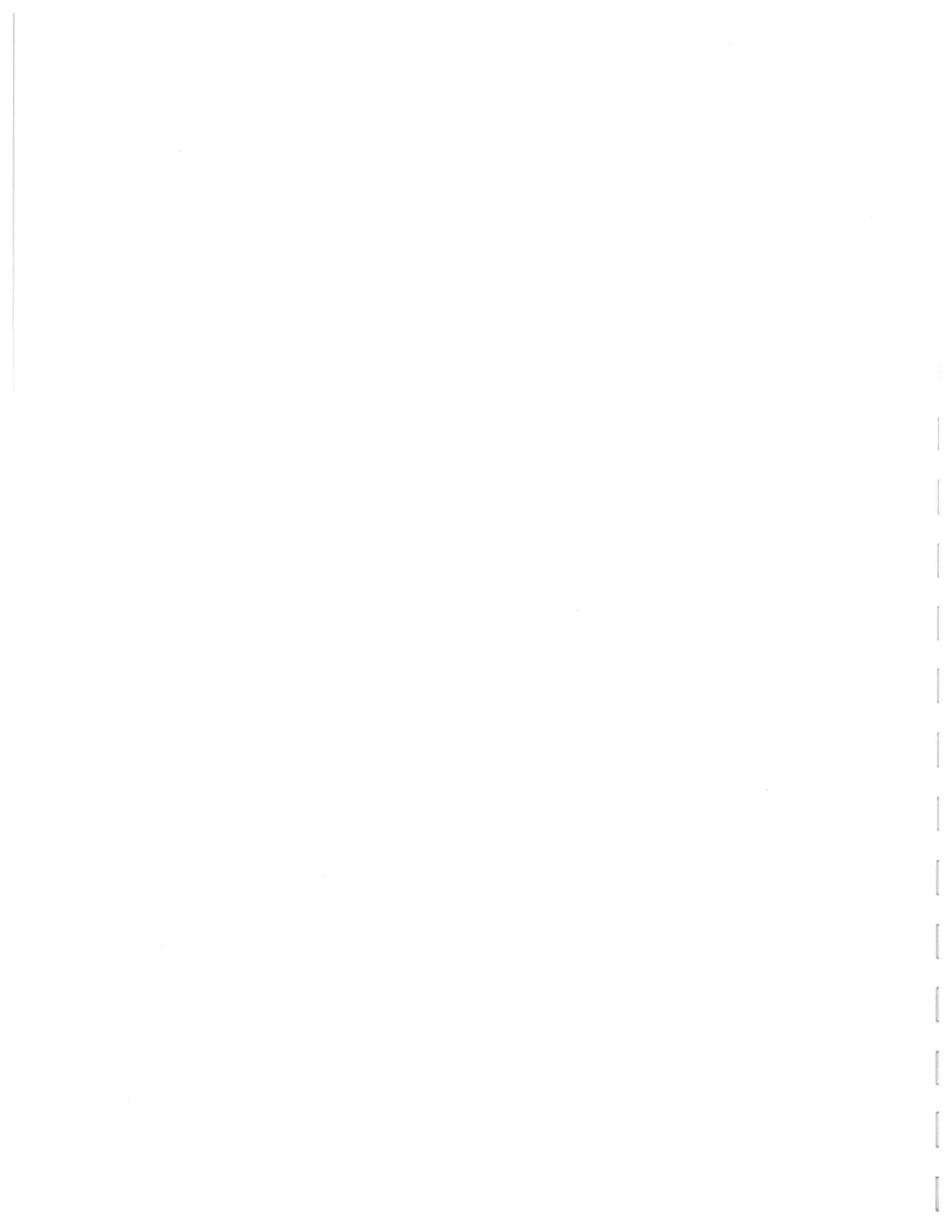
PLT TYP. Wave	FL/-/3/-/-/R/-	Scale = .1875"/Ft.
TC LL	20.0 PSF	REF R487 -- 35440
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUR487 05313074
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 126191
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THIS TRUSS IS DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE SPECIFICATIONS PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 5835 CONCORD DRIVE, SHILLINGTON, MARYLAND, MD 20854. THE USER SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE USER SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES. THE USER SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL AUTHORITIES.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018T/16GA (N-H/S/K) ASTH A653 GRADE 40/60 (N, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. EGD-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS PER THE TPI SPECIFICATIONS FOR THE TRUSS COMPONENTS DRAWING INDICATES. ACCEPTANCE OF THIS DESIGN BY THE USER SHALL BE THE USER'S RESPONSIBILITY FOR THE TRUSS COMPONENTS. THE USER SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS AND APPROVALS FROM THE LOCAL, STATE AND FEDERAL BUILDING DESIGNER PER ABSI/TPI 1 SEC. 4.





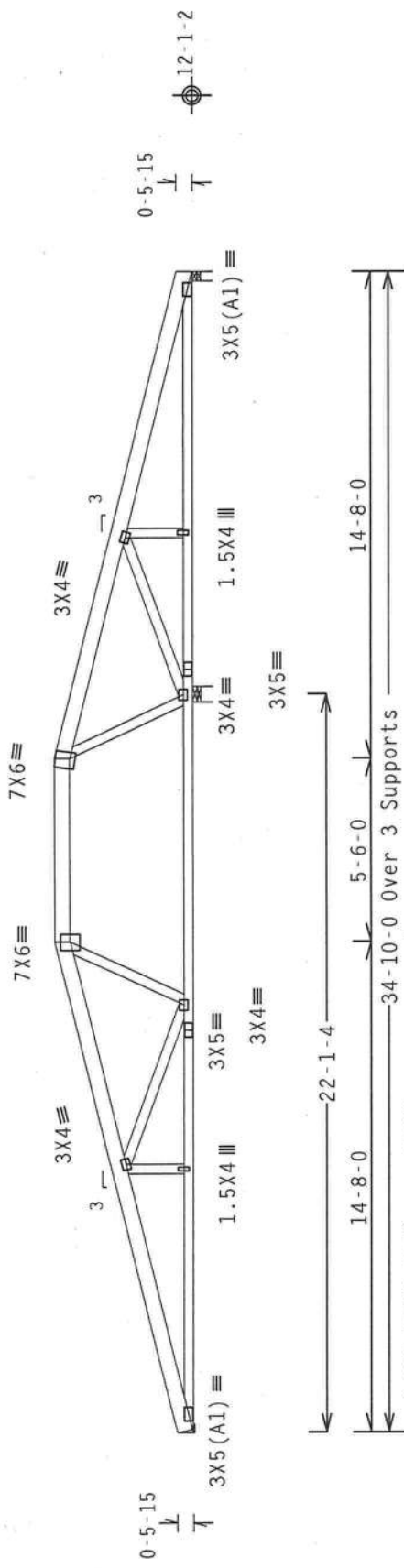
Top chord 2x6 SP #1 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 14.42 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

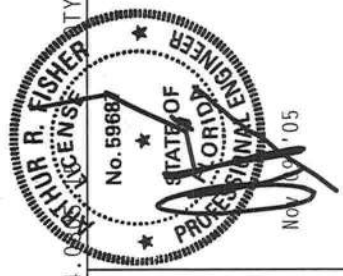


R-821 U=180 H=Simpson LUS26  
 w/ (4) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R-642 U=180 W=5.5"      R-648 U=180 W=3.5"

PLT TYP. Wave      Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)      7.04.0

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .1875" / Ft.
TC DL	15.0 PSF	REF R487 - -	35441
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313075
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126222
SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRELO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS SYSTEM AS SHOWN WILL BE THE RESPONSIBILITY OF THE INSTALLER. THE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W 8/S/S) ASTM A653 GRADE 40/60 (W, E/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**ALPINE**  
 Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Ft. Lauderdale, FL 33344  
 FL Certificate of Authorization # 567

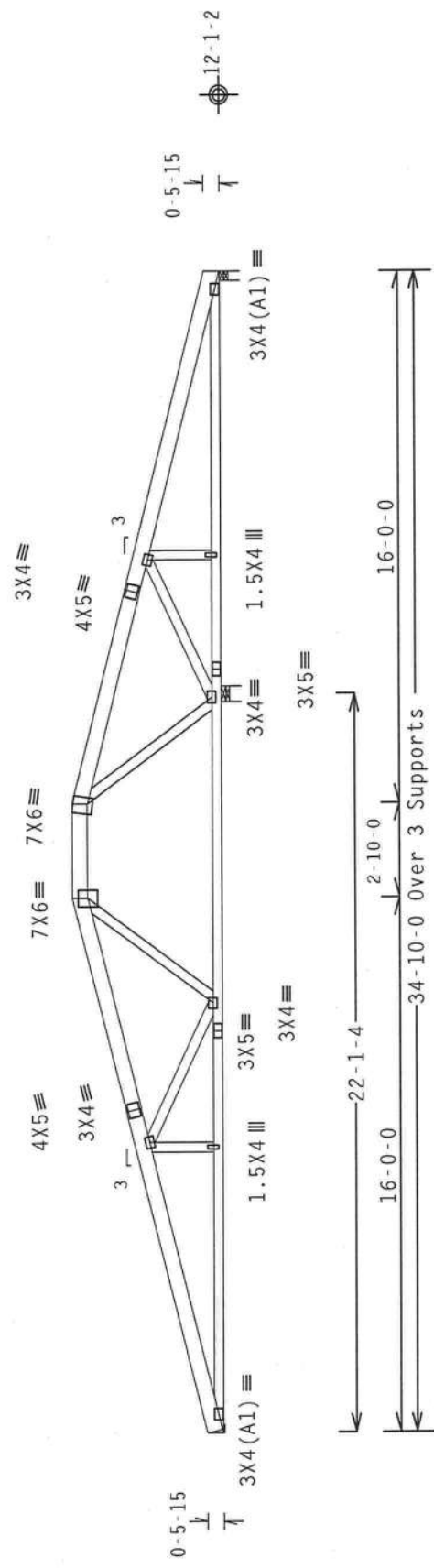


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

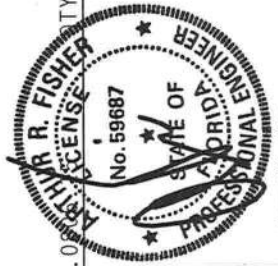
Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=685 U=180 H=Simpson LUS26  
 w/ (4) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00

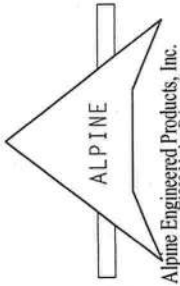
TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .1875" / Ft.
TC DL	15.0 PSF	REF R487 --	35442
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313076
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126237
SPACING	16.0"	JREF-	1SS0487_Z01



Nov 09 '05

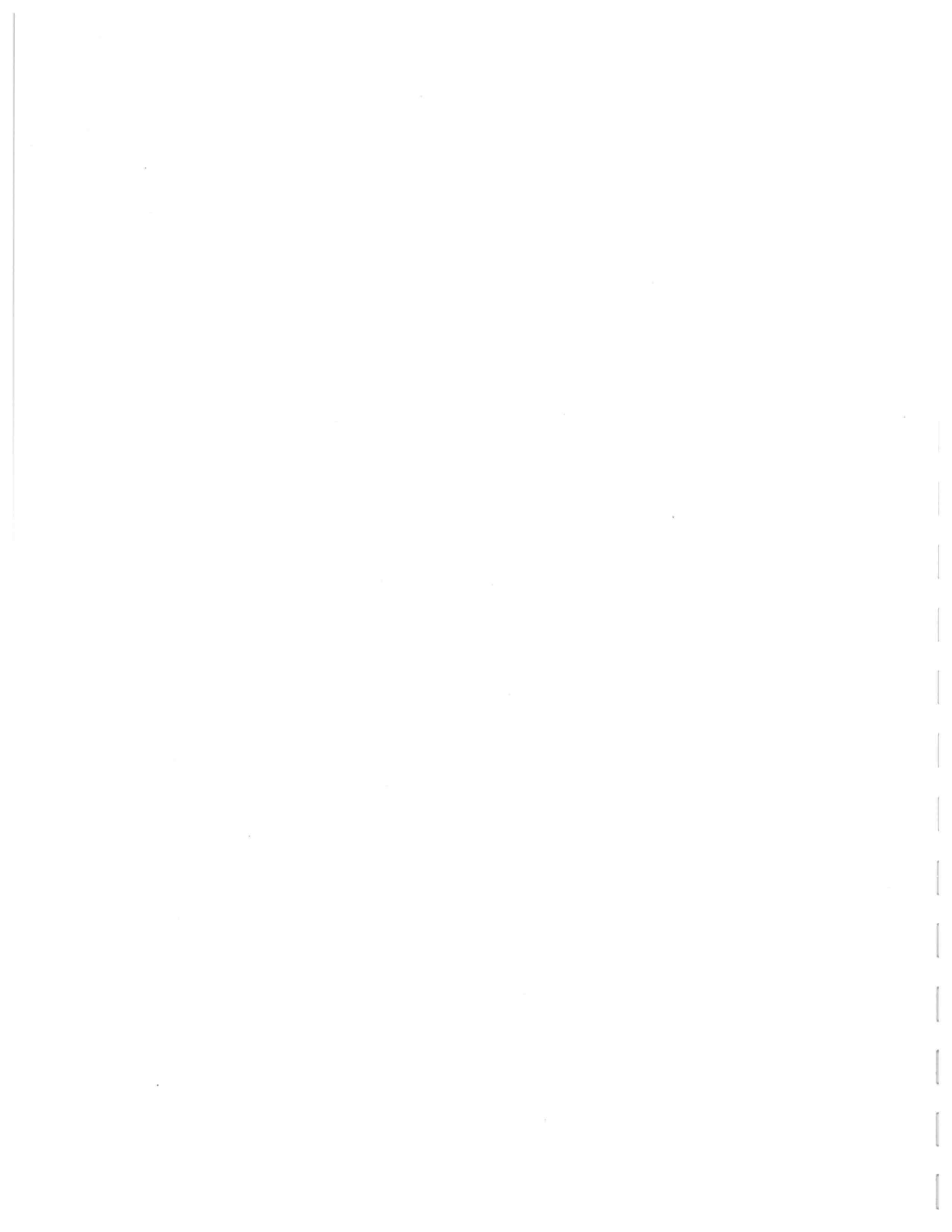
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/18GA (0.0156) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. DRAWINGS INDICATES ACCEPTABLE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/18GA (0.0156) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. DRAWINGS INDICATES ACCEPTABLE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
 1950 Mailey Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave



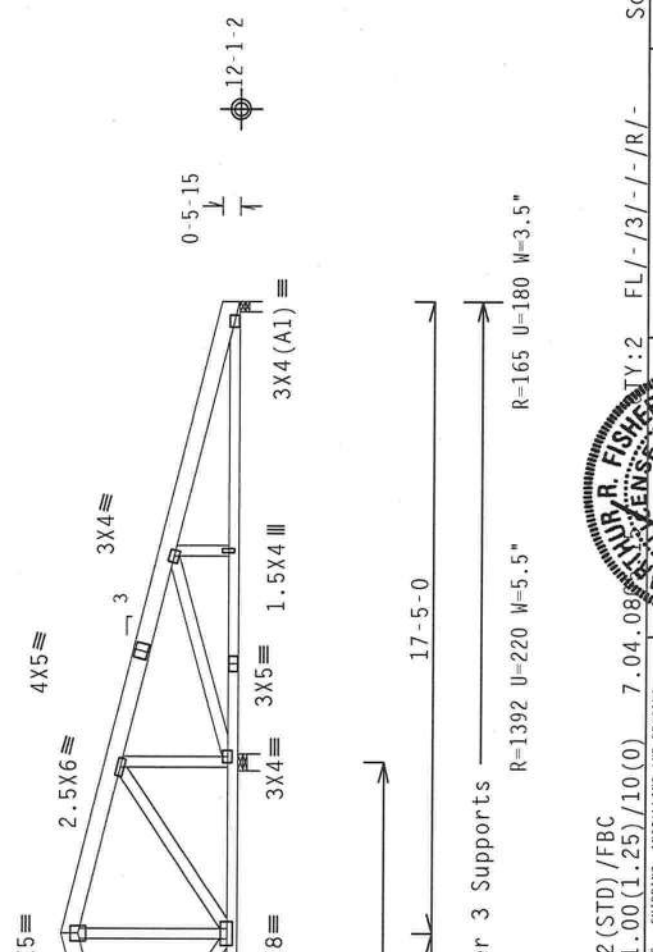
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



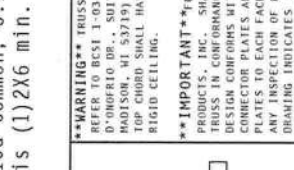
R=555 U=180 H=Simpson LUT6  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pi Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE INSTALLER SHALL VERIFY THAT THE DESIGN CONFORMS WITH APPLICABLE PREVIOUS EDITIONS OF THE TPI TRUSS MANUFACTURING AND INSTALLATION GUIDE. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H / W / S) ASTM A653. TOP CHORDS SHALL BE BUILT UP WITH TWO PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**ALPINE**

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TYP.2	FL / - / 3 / - / - / R / -	Scale = .1875" / Ft.
TC LL	20.0 PSF	REF R487 -- 35443
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUR487 05313077
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 126250
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01

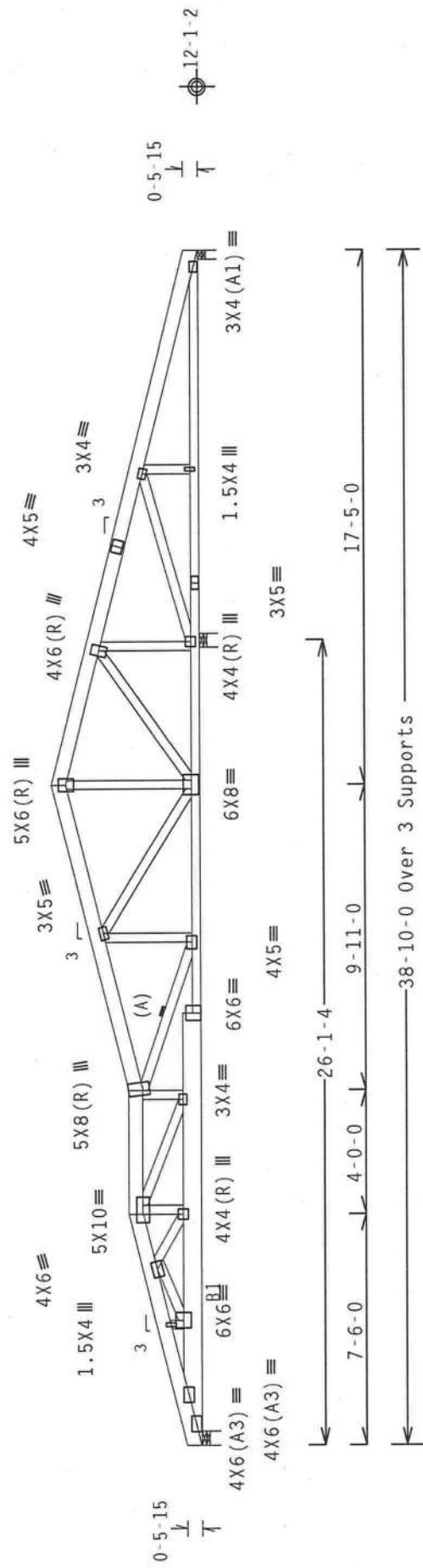


**SPECIAL LOADS**  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 11.50  
 TC - From 47 PLF at 11.50 to 47 PLF at 38.83  
 BC - From 13 PLF at 0.00 to 13 PLF at 38.83  
 BC - 322 LB Conc. Load at 2.73  
 BC - 1783 LB Conc. Load at 3.94

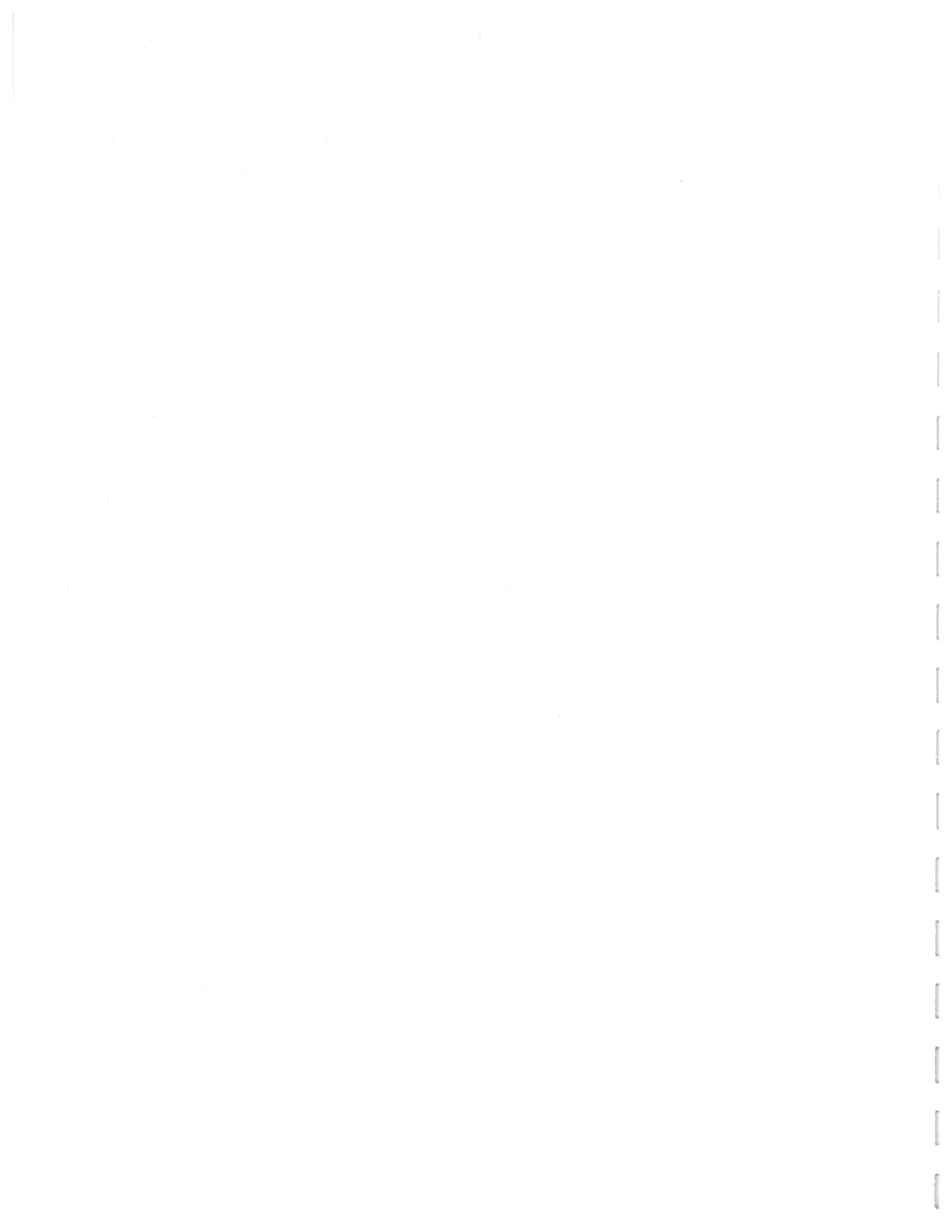
Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense :B1 2x8 SP SS:  
 Webs 2x4 SP #3  
 110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



PLT TYP. Wave ALPINE Alpine Engineered Products, Inc. 1950 Marley Drive Gainesville, FL 32644 FL Certificate of Authorization # 567	Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.04.0 **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-03 (BUILDING COMPONENT SAFETY INFORMATION), FORCE MAINS, AND THE ALPINE DESIGN MANUAL FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. **IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 70/18/16GA (4-H/5)K ASH A653 GRADE 40/60 (IN. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002, SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.	R=2333 U=448 W=5.5"	R=2296 U=397 W=5.5"	R=-169 U=180 W=3.5"	Scale = .1875"/Ft.
		TY: 1 FL / - / 3 / - / - / R / - /	TC LL 20.0 PSF TC DL 15.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 45.0 PSF DUR.FAC. 1.25 SPACING 16.0"	REF R487 - 35444 DATE 11/09/05 DRW HCUSR487 05313078 HC-ENG JB/AF SEQN- 126335 JREF- 1SS0487_Z01	



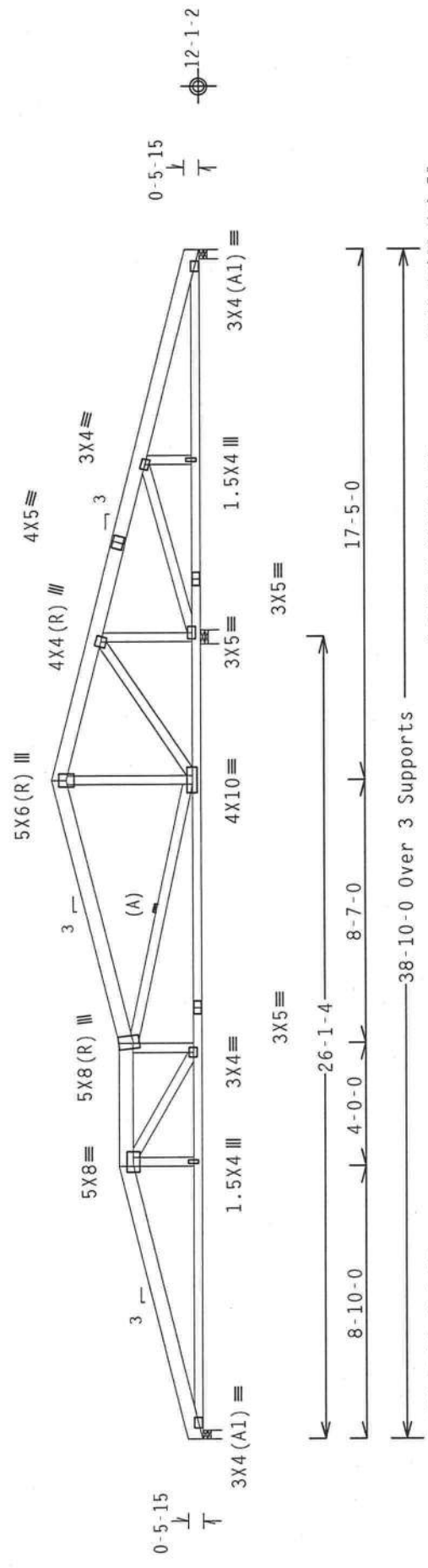
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

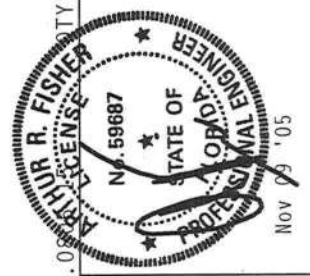
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

Scale = .1875" / Ft.



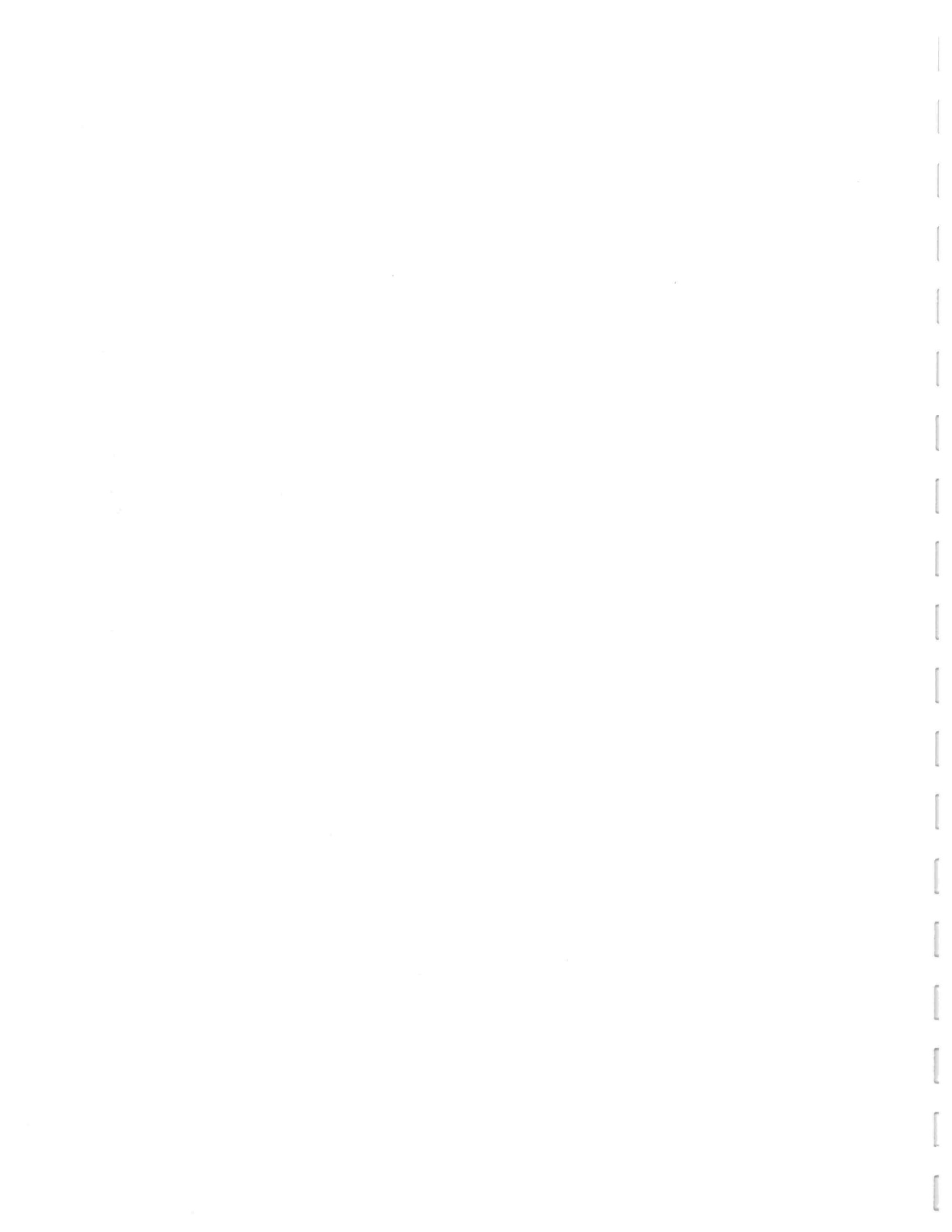
TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO UBCS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTION), 383 D'ONDRETO DR., SUITE 200, FRODOSSA, WY 82401, FOR MORE INFORMATION REGARDING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (4-N/5/K) ASTM A653 GRADE 40/60 (N, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave

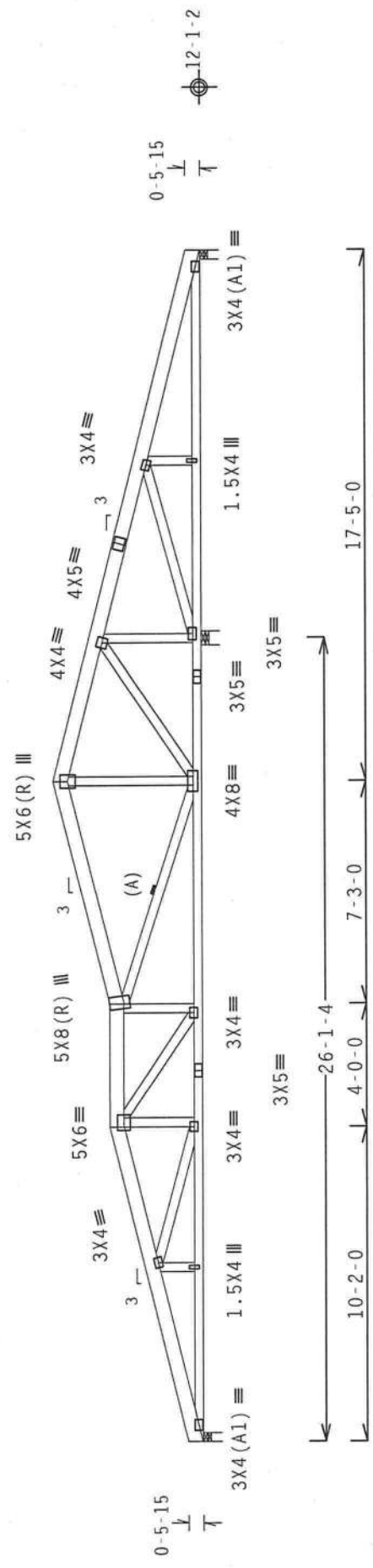


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

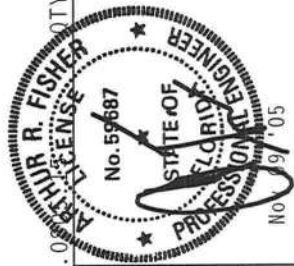
Trusses to be spaced at 16.0" OC maximum.

(A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor  
 for dead load is 1.50.



R=658 U=180 W=3.5"      R=1609 U=255 W=5.5"      R=87 U=180 W=3.5"

PLT TYP. Wave  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

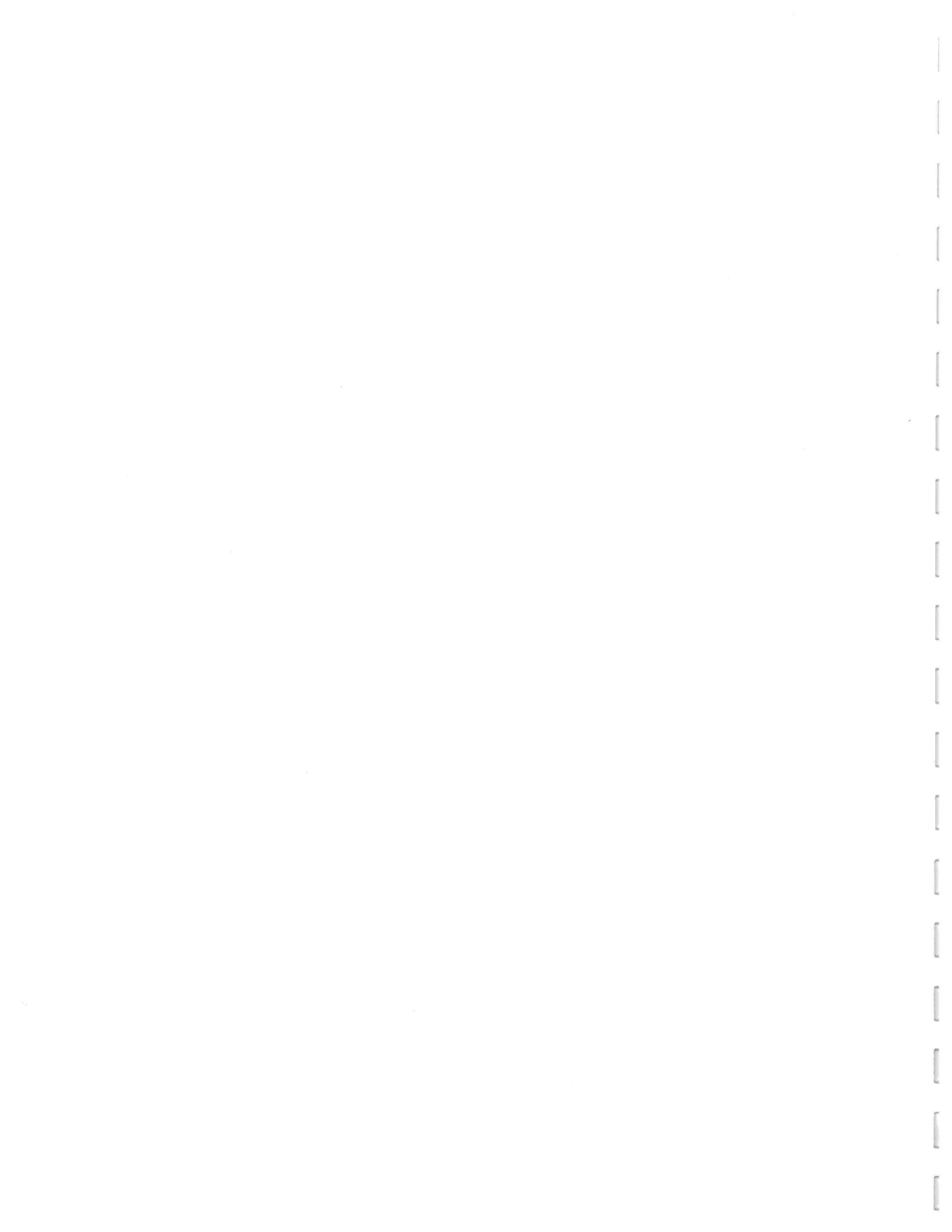


TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .1875" / Ft.
TC DL	15.0 PSF	REF R487 - - 35446	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUR487 05313080	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 126361	
SPACING	16.0"	JREF- 1SS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D-ONOFRO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE PROCEDURES. ALL WORKERS SHOULD HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/M/S/A) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

Alpine Engineered Products, Inc.  
 1950 Maple Drive  
 Gaines City, FL 32644  
 FL Certificate of Authorization # 567

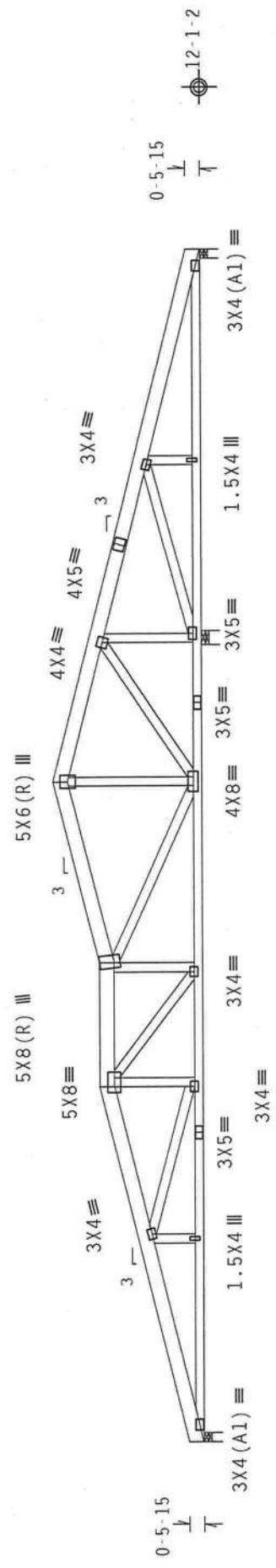


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

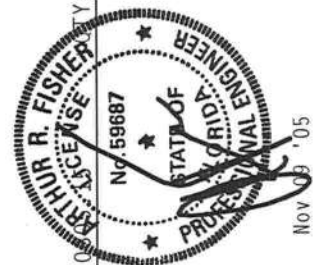
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



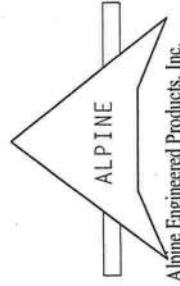
11-6-0  
 4-0-0  
 5-11-0  
 26-1-4  
 17-5-0  
 R=665 U=180 W=3.5"  
 R=1587 U=251 W=5.5"  
 R=102 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .1875" / Ft.



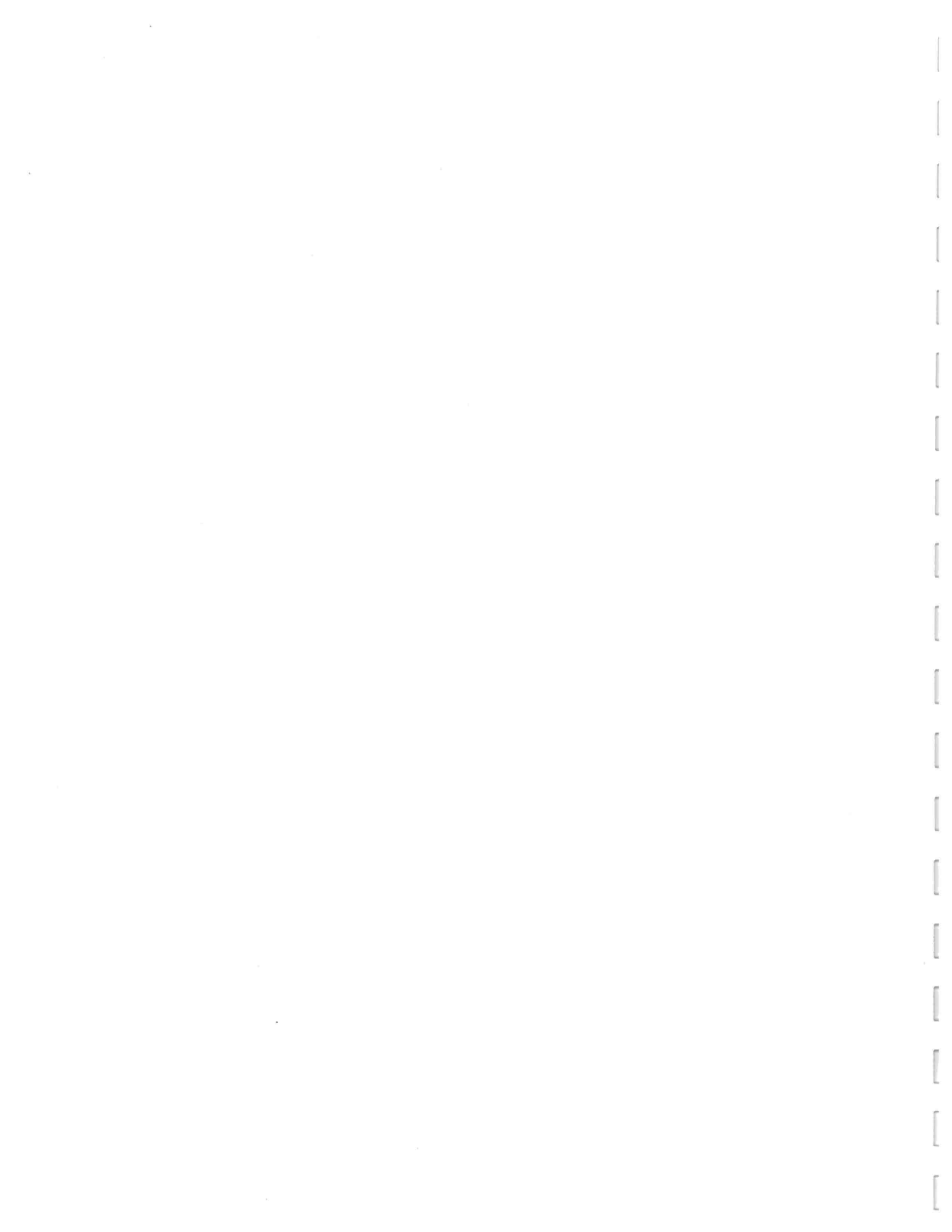
TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"



Alpine Engineered Products, Inc.  
 1900 Alameda Dr.  
 Gaines City, FL 32644  
 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 883 D-OMERIO DR., SUITE 200, MADISON, WI 53719) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE BL, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. ALL TRUSS CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (H/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

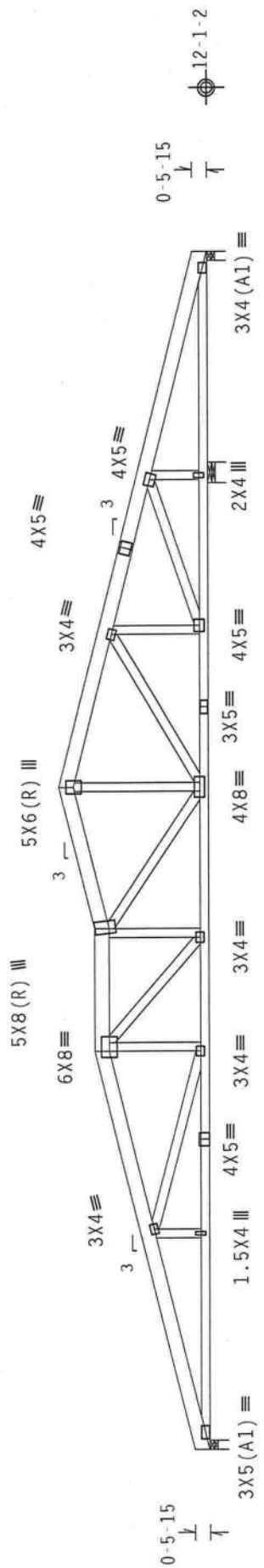


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



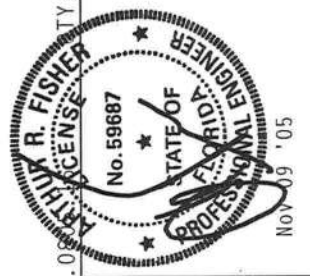
12-10-0  
 31-7-10  
 4-0-0  
 4-7-0  
 17-5-0  
 12-10-0  
 38-10-0 Over 3 Supports  
 17-5-0  
 R=897 U=180 W=3.5"  
 R=1510 U=239 W=7.778"  
 R=-53 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

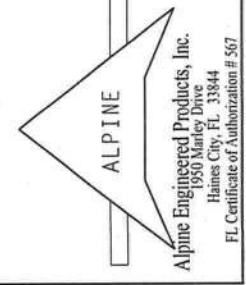
Scale = .1875" / Ft.

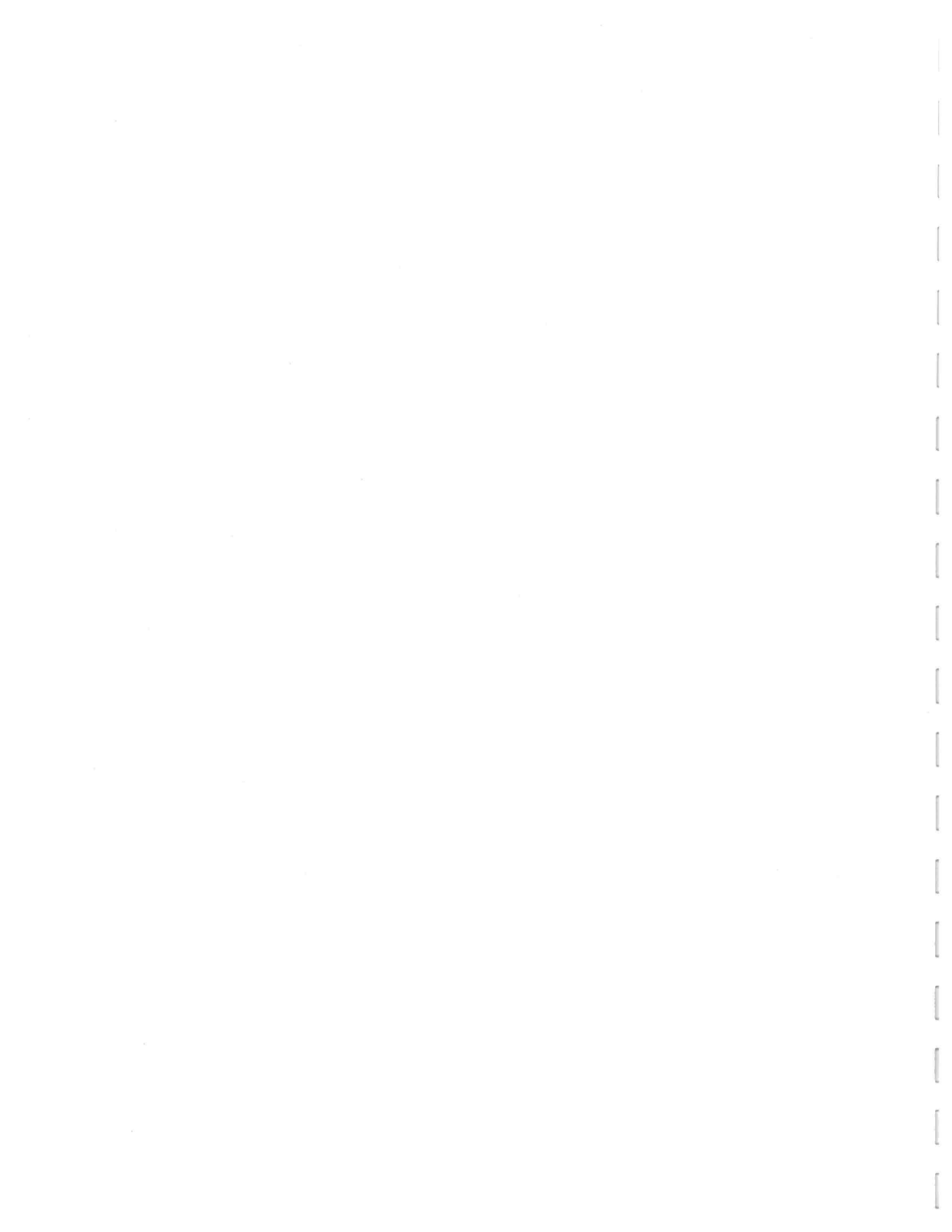
REF	R487 -	35448
DATE	11/09/05	
DRW	HCUSR487	05313082
HC-ENG	JB/AF	
SEQN-	126375	
DUR.FAC.	1.25	
SPACING	16.0"	
TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	
BC DL	10.0 PSF	
BC LL	0.0 PSF	
TOT.LD.	45.0 PSF	
JREF	ISS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFREO DR., SUITE 200, MADISON, WI 53719) AND NICKA (WOOD TRUSS COUNCIL OF AMERICA, 608 W. WASHINGTON ST., MADISON, WI 53719) FOR SAFETY PRACTICES TO FOLLOW. ALL TRUSS CHORDS AND BRACING MEMBERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/S) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AHEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



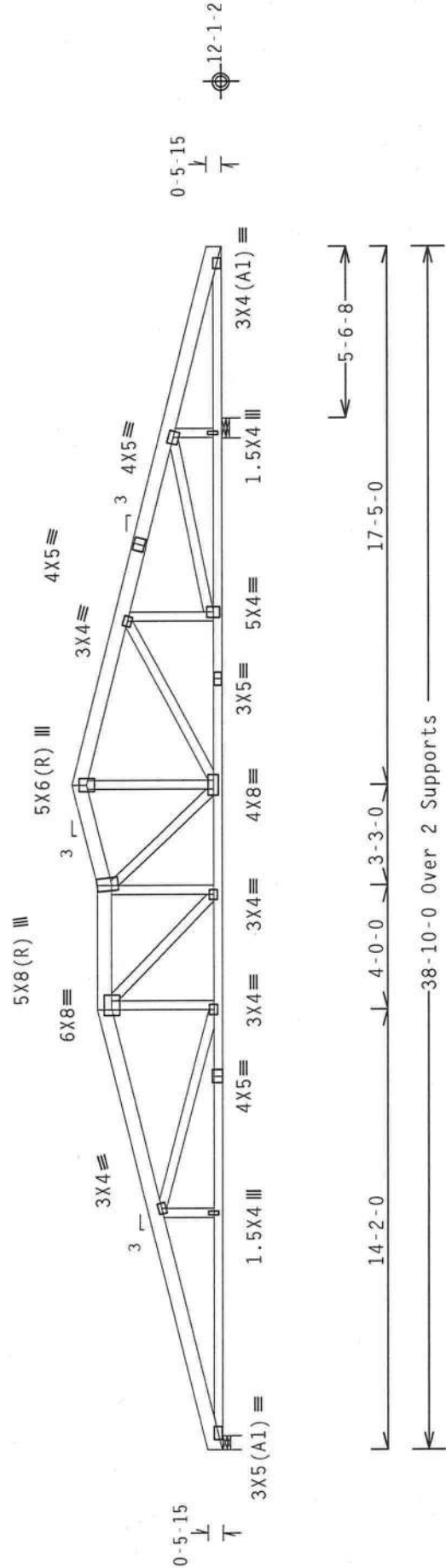


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

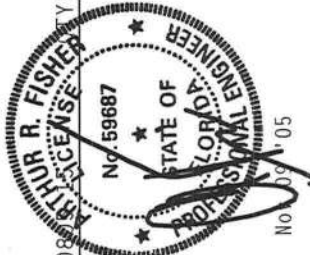
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



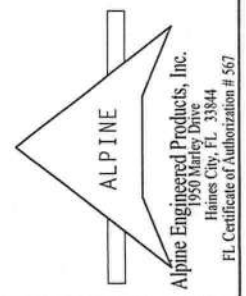
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

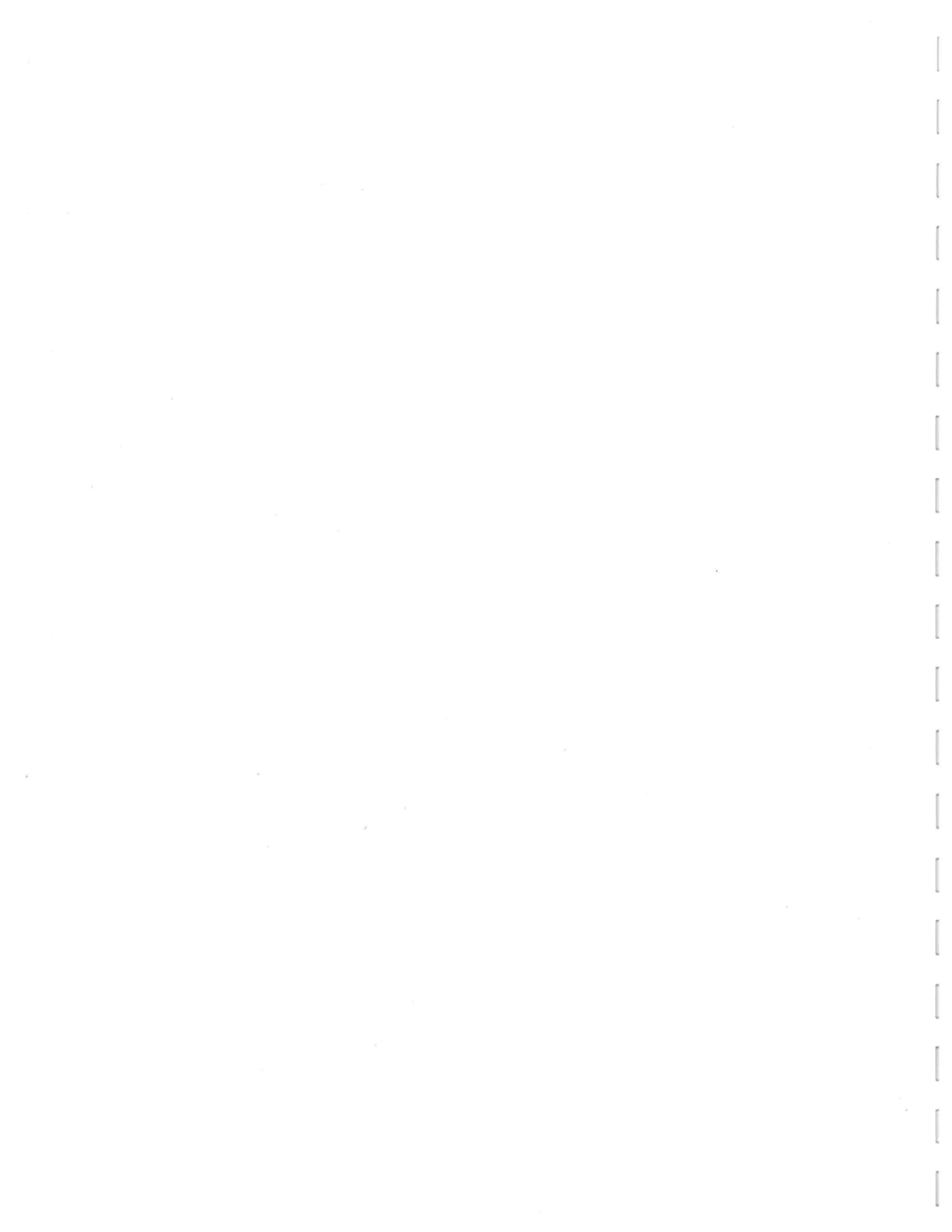
PLT TYP. Wave	Scale = .1875" / Ft.
REF R487 - 35449	TC LL 20.0 PSF
DATE 11/09/05	TC DL 15.0 PSF
DRW HCUSR487 05313008	BC DL 10.0 PSF
HC-ENG JB/AF *	BC LL 0.0 PSF
SEON- 126382	TOT.LD. 45.0 PSF
JREF- 1SS0487_Z01	DUR.FAC. 1.25
	SPACING 16.0"



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY OPERATIONS), PRODUCT LITERATURE, TPI-2002(STD) AND TPI-2002(STD) FOR ADDITIONAL SAFETY PRACTICES AND PRECAUTIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS, EXCEPT WHERE SHOWN OTHERWISE IN THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS, EXCEPT WHERE SHOWN OTHERWISE IN THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS, EXCEPT WHERE SHOWN OTHERWISE IN THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE NECESSARY PERMITS, EXCEPT WHERE SHOWN OTHERWISE IN THE DRAWINGS.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-11/5/8) ASTM A653 GRADE 40/60 (R. K/M-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABEX A3 OF TPI-2002, SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN.



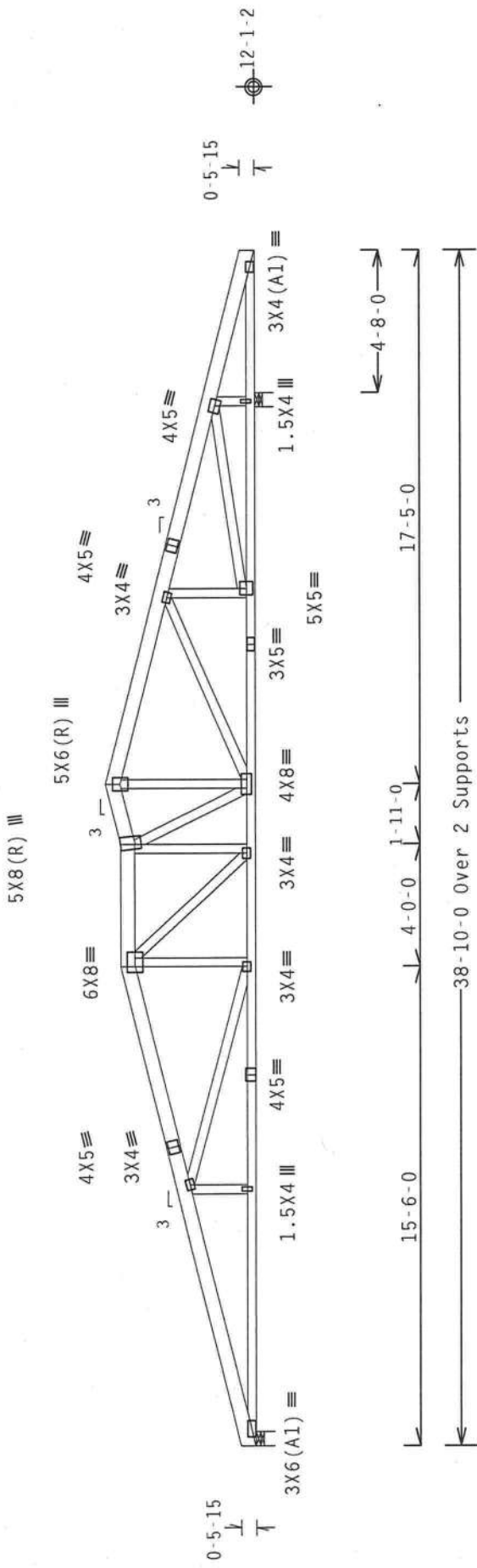


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.76 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

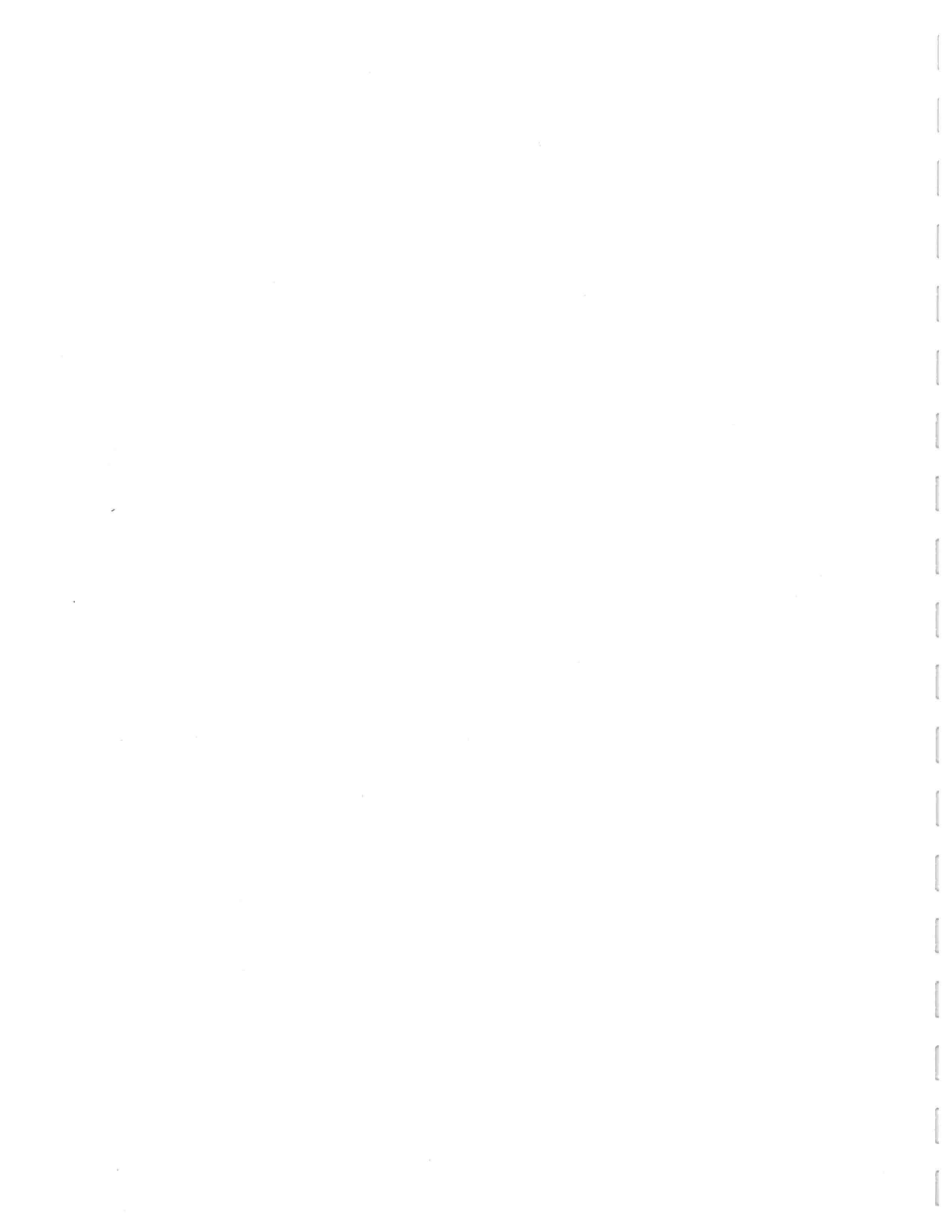
Scale = .1875" / Ft.

TC LL	20.0 PSF	REF	R487 - 35450
TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313012
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	45.0 PSF	SEQN-	126391
DUR.FAC.	1.25	JREF-	1SS0487_Z01
SPACING	16.0"		

ALPINE ENGINEERING  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 No. 59687  
 Nov 09 '05

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY AFPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 70/18/16GA (H-M/S/R) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUEPLY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

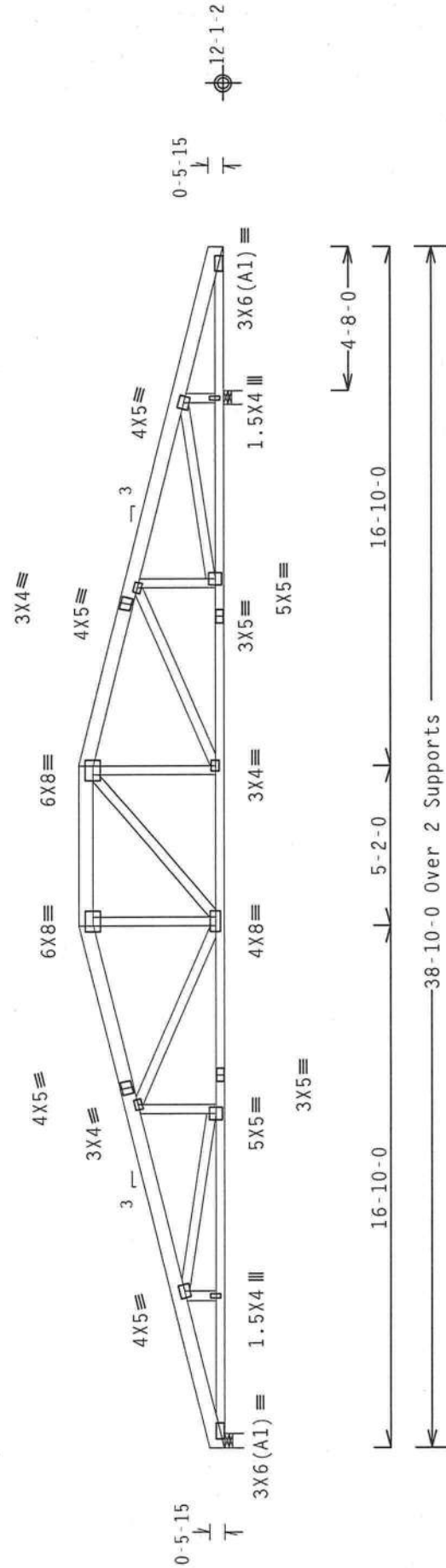


Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

110 mph wind, 14.69 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase  
factor for dead load is 1.50.



R=1011 U=180 W=5.5"

R=1343 U=192 W=5.5"

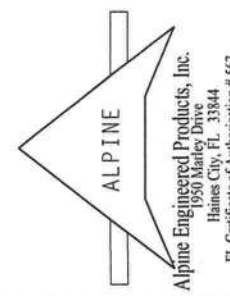
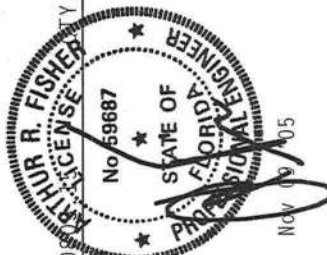
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

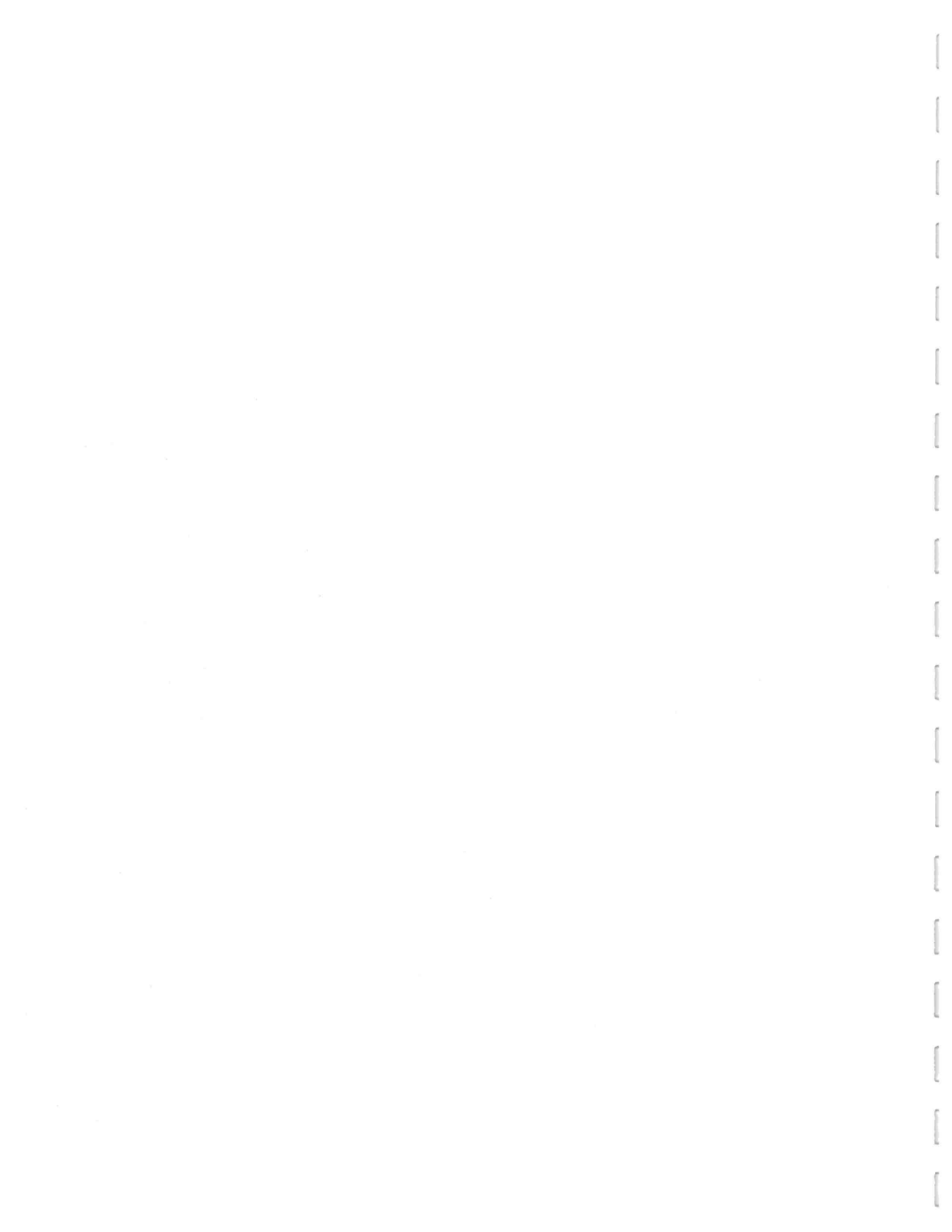
Scale = .1875" / Ft.

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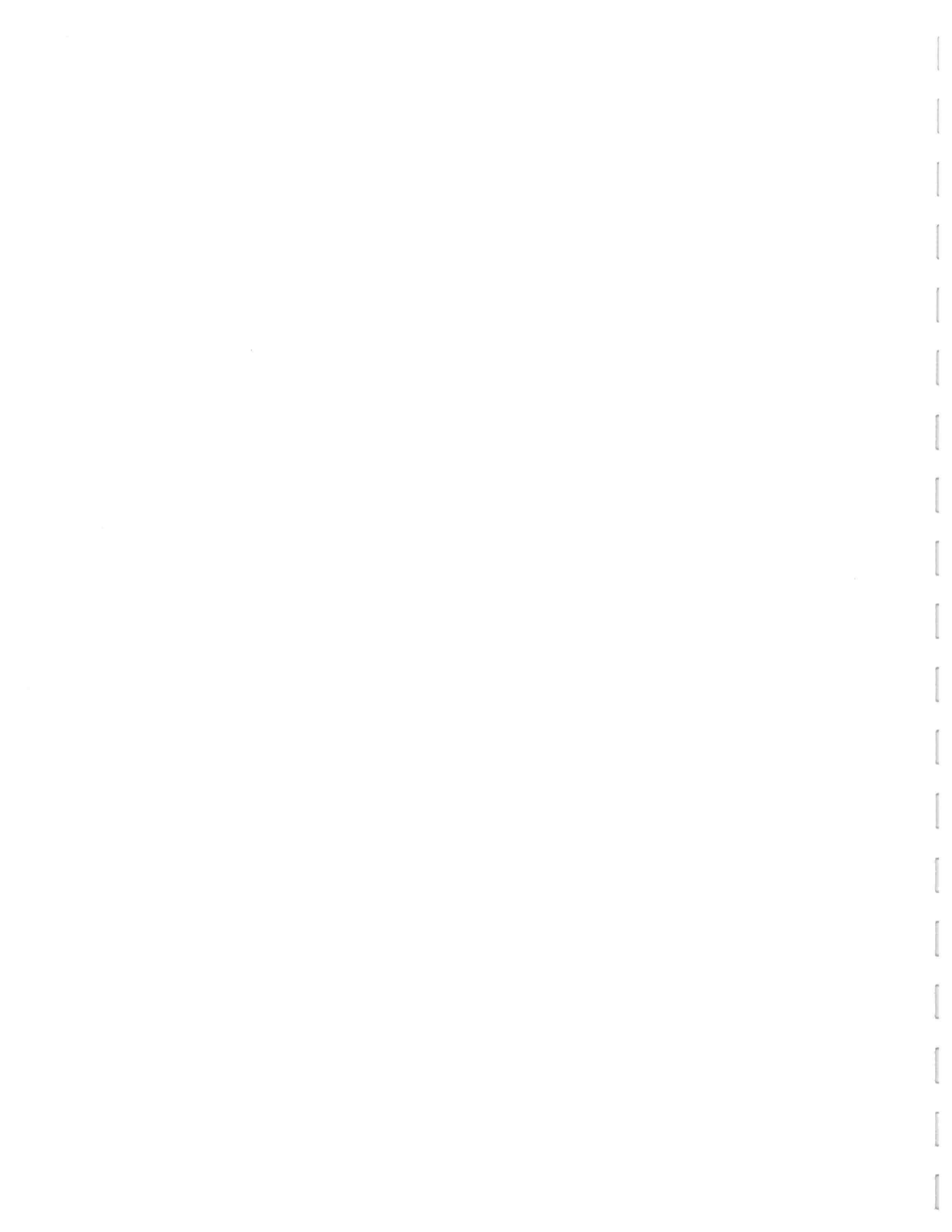
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TC LL	20.0 PSF	REF	R487--	35451
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313018
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEON-	126431	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	





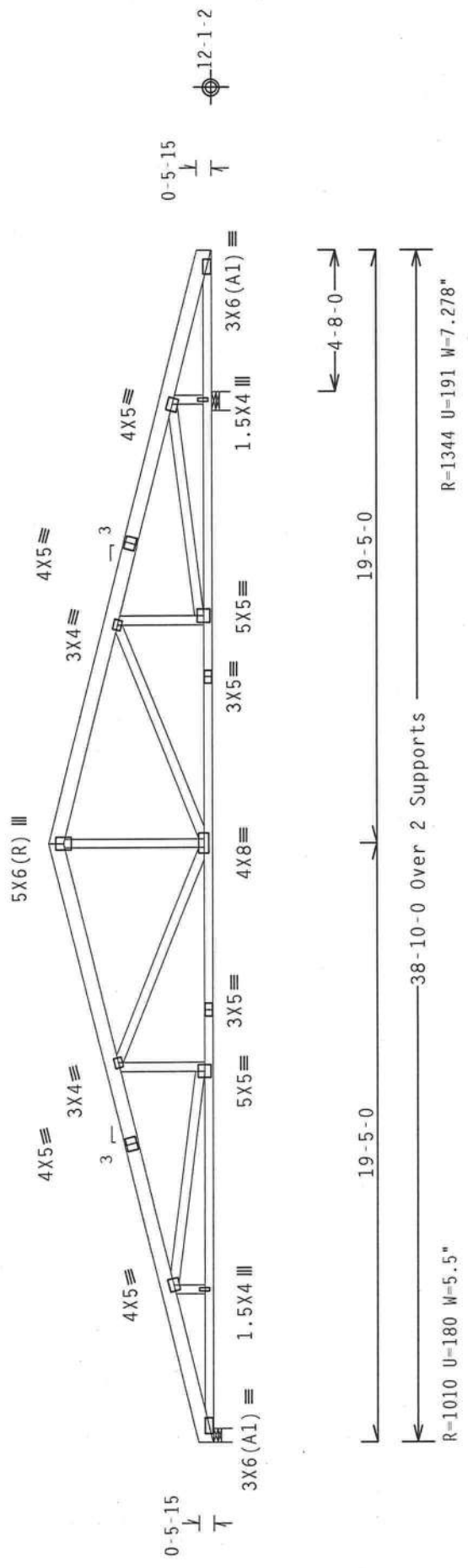


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.01 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

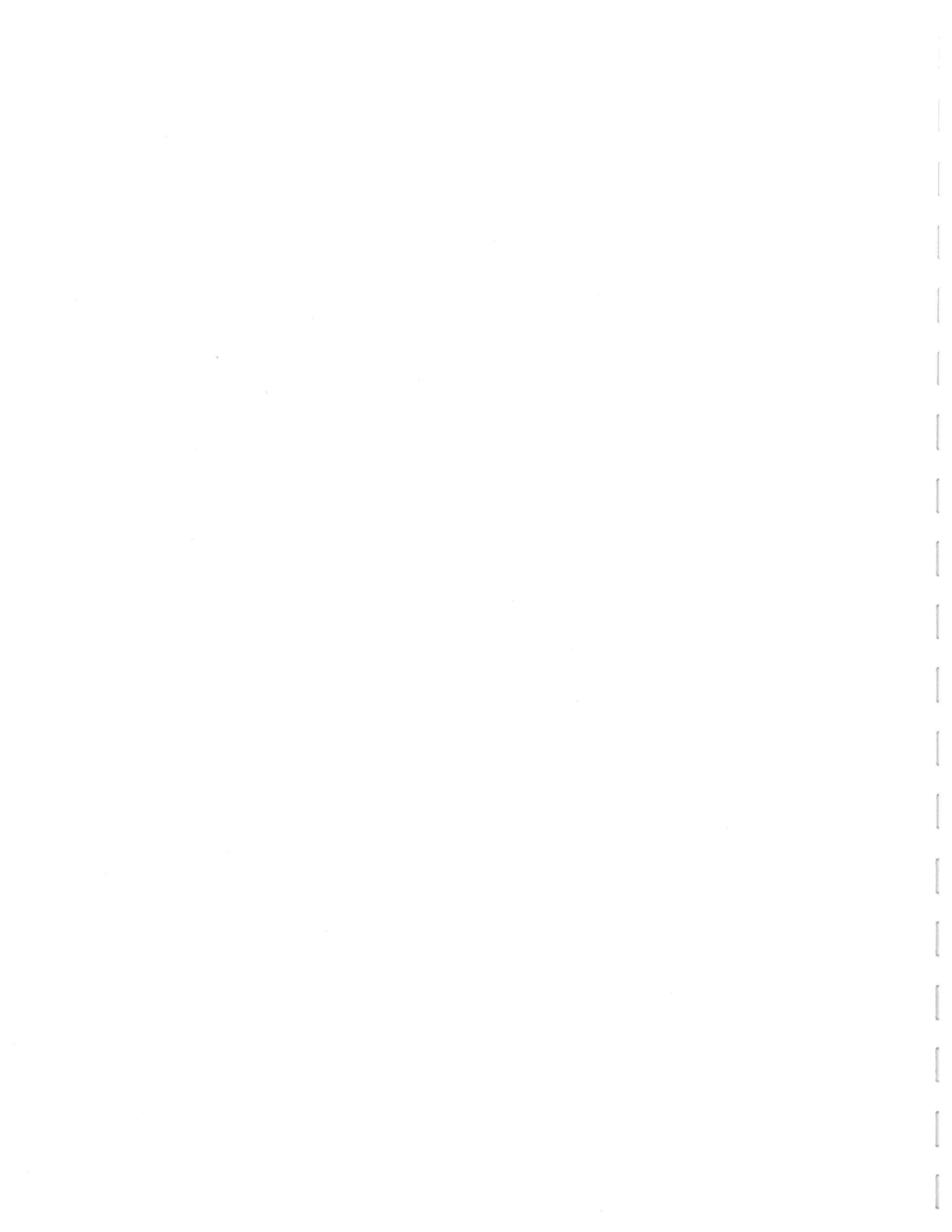
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase  
 factor for dead load is 1.50.



PLT TYP. Wave  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0  
 R=1010 U=180 W=5.5" 38-10-0 Over 2 Supports  
 R=1344 U=191 W=7.278\*  
 Scale = .1875" / Ft.

 Alpine Engineering Products, Inc. Haines City, FL 33844 FL Certificate of Authorization # 567	**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION AND BRACING. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (W-11/5/5) ASTM A653 GRADE 40/60 (H. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTER AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANEX/TPI 1 SEC. 2.	
	**IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (W-11/5/5) ASTM A653 GRADE 40/60 (H. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTER AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANEX/TPI 1 SEC. 2.	No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER No. 09 '05
	TC LL 20.0 PSF TC DL 15.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 45.0 PSF	FL / - / 3 / - / - / R / - Scale = .1875" / Ft.
	DUR.FAC. 1.25 SPACING 16.0"	REF R487 -- 35453 DATE 11/09/05 DRW HCUSR487 05313019 HC-ENG JB/AF SEQN- 126441
	JREF- ISS0487_Z01	



### 3 COMPLETE TRUSSES REQUIRED

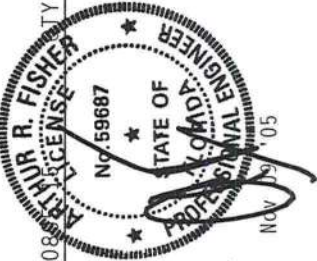
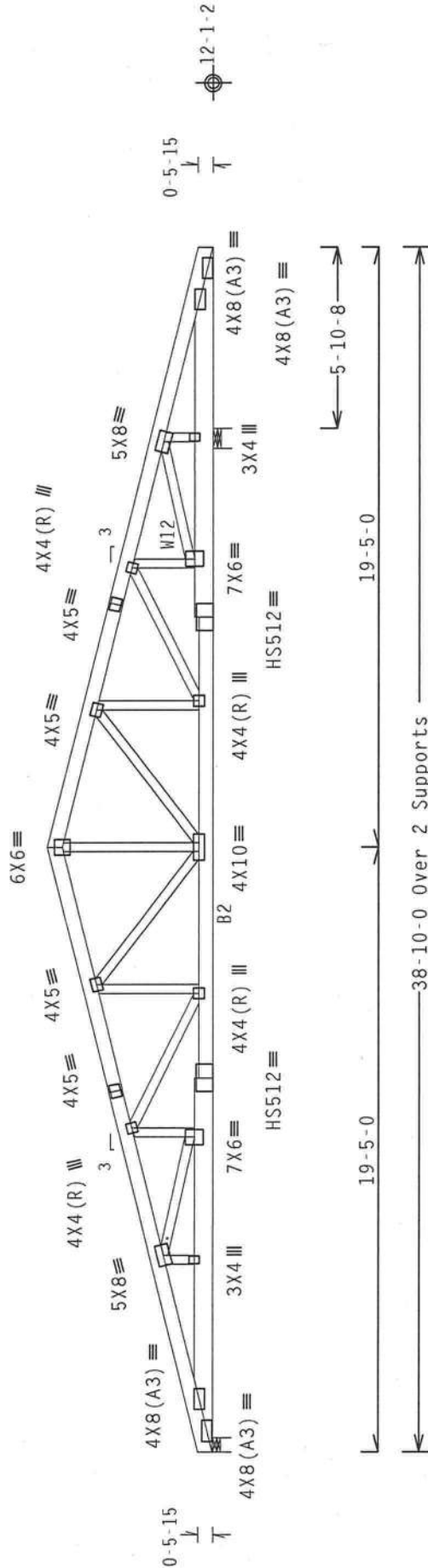
Nailing Schedule: (10d Common (0.148"x3", min.)\_nails)  
 Top Chord: 1 Row @ 8.00" o.c.  
 Bot Chord: 1 Row @ 5.50" o.c.  
 Webs: 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.01 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord	2x6 SP #1 Dense	DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25
Bot chord	2x8 SP SS :B2 2x6 SP #1 Dense:	
Web	2x4 SP #3 :W12 2x4 SP #2 Dense:	
<b>SPECIAL LOADS</b>		
TC	From 71 PLF at 0.00 to 20 PLF at 38.83	
BC	From 1436 LB Conc. Load at 0.81	
BC	From 561 LB Conc. Load at 2.15	
BC	From 558 LB Conc. Load at 3.48	
BC	From 482 LB Conc. Load at 7.48	
BC	From 481, 6.15	
BC	From 8.81, 10.15, 11.48, 12.81	
BC	From 492 LB Conc. Load at 15.48, 16.81, 18.15, 19.48, 20.81	
BC	From 22.15, 23.48, 24.81, 26.15, 27.48, 28.81, 30.15, 31.48	



TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .1875" / Ft.
TC DL	15.0 PSF <td>REF R487--</td> <td>35454</td>	REF R487--	35454
BC DL	10.0 PSF <td>DATE</td> <td>11/09/05</td>	DATE	11/09/05
BC LL	0.0 PSF <td>DRW</td> <td>HCUSR487 05313084</td>	DRW	HCUSR487 05313084
TOT.LD.	45.0 PSF <td>HC-ENG</td> <td>JB/AF</td>	HC-ENG	JB/AF
DUR.FAC.	1.25 <td>SEQN-</td> <td>161018</td>	SEQN-	161018
SPACING	24.0"	JREF-	1SS0487_Z01

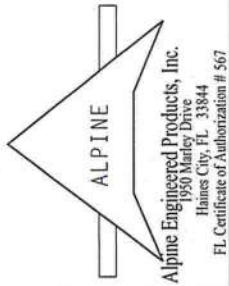
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

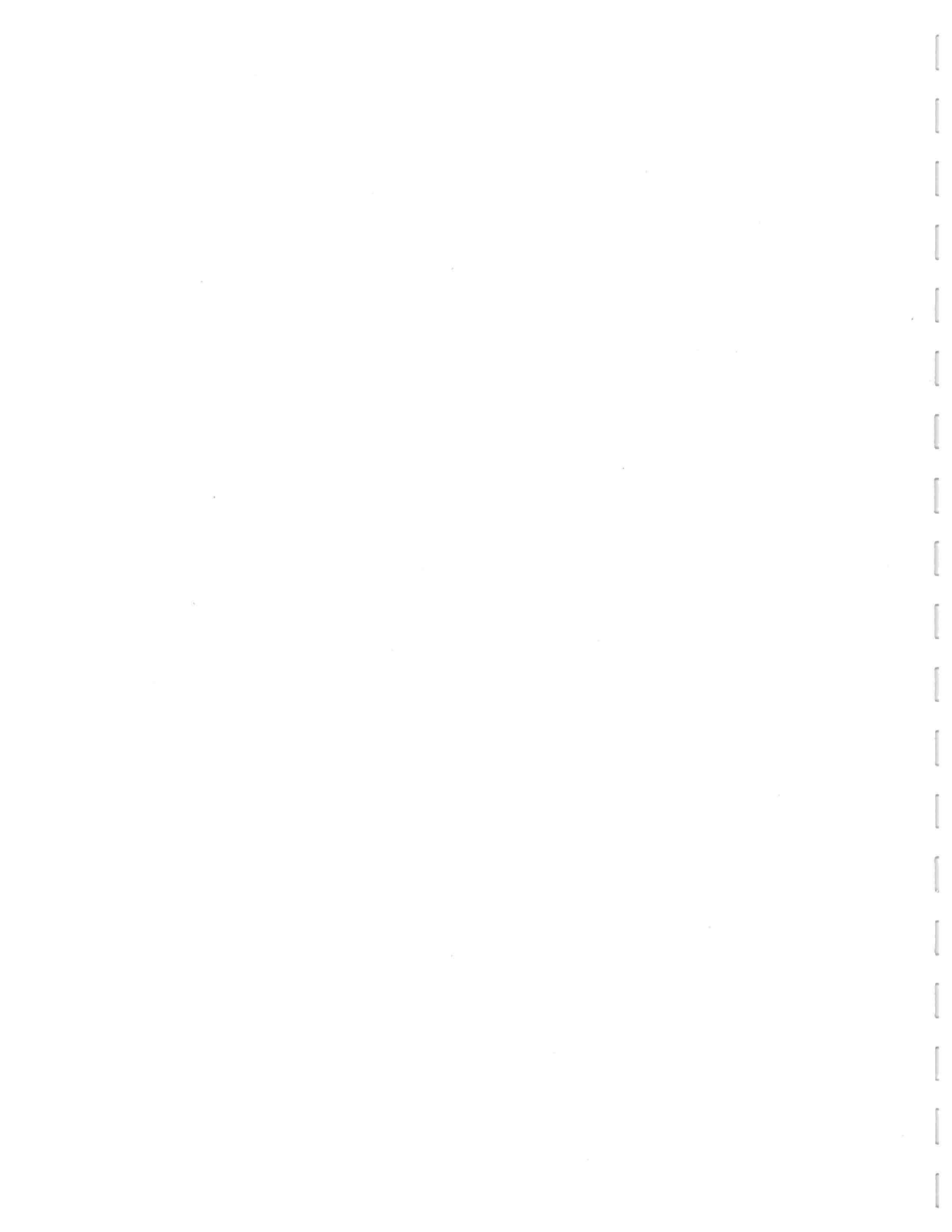
R-8556 U=1828 W=5.5"      38-10-0 Over 2 Supports      R=7936 U=1609 W=7.778"

PLT TYP. 20 Gauge HS, Wave

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 DORCHESTER DR., SUITE 200, HANSON, MI 48739) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, WILSONVILLE, OR 97148) FOR ADDITIONAL INFORMATION. ALL TRUSSING SHALL BE PROPERLY ATTACHED TO THE TOP CHORD AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/5/5) ASTM A653 GRADE 40/60 (K, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





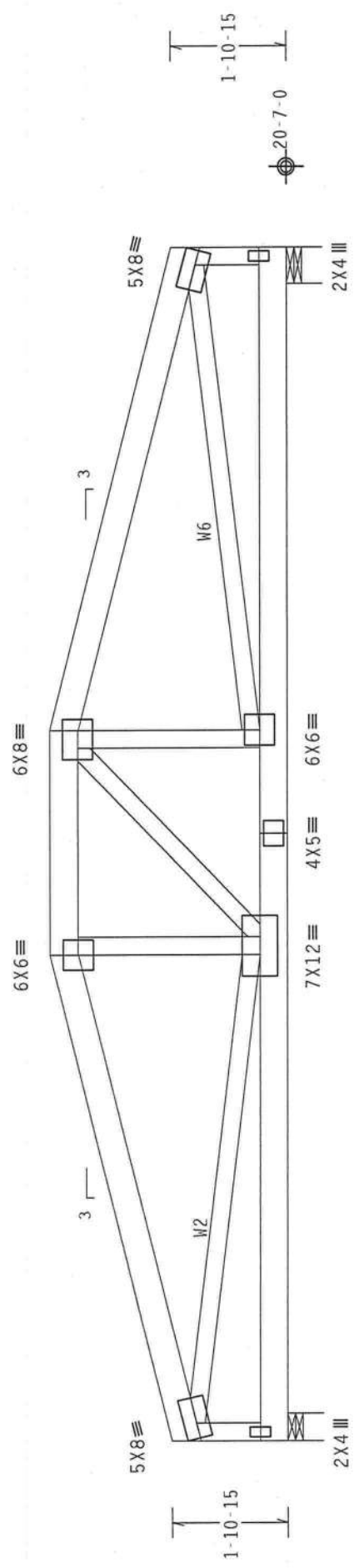
110 mph wind, 23.49 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 8-0-0 jacks with no webs.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3 :W2, W6 2x4 SP #2 Dense:

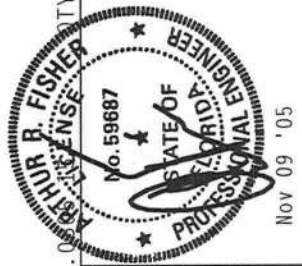
Trusses to be spaced at 16.0" OC maximum.



R=1680 U=499 W=5.375" R=1680 U=499 W=7.25"

Design Crit: TPI-2002(STD)/FBC

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0 PSF		REF R487 - 35455
BC DL	10.0 PSF		DATE 11/09/05
BC LL	0.0 PSF		DRW HCUSR487 05313085
TOT.LD.	45.0 PSF		HC-ENG JB/AF
DUR.FAC.	1.25		SEQN- 126767
SPACING	16.0"		JREF- 1SS0487_Z01

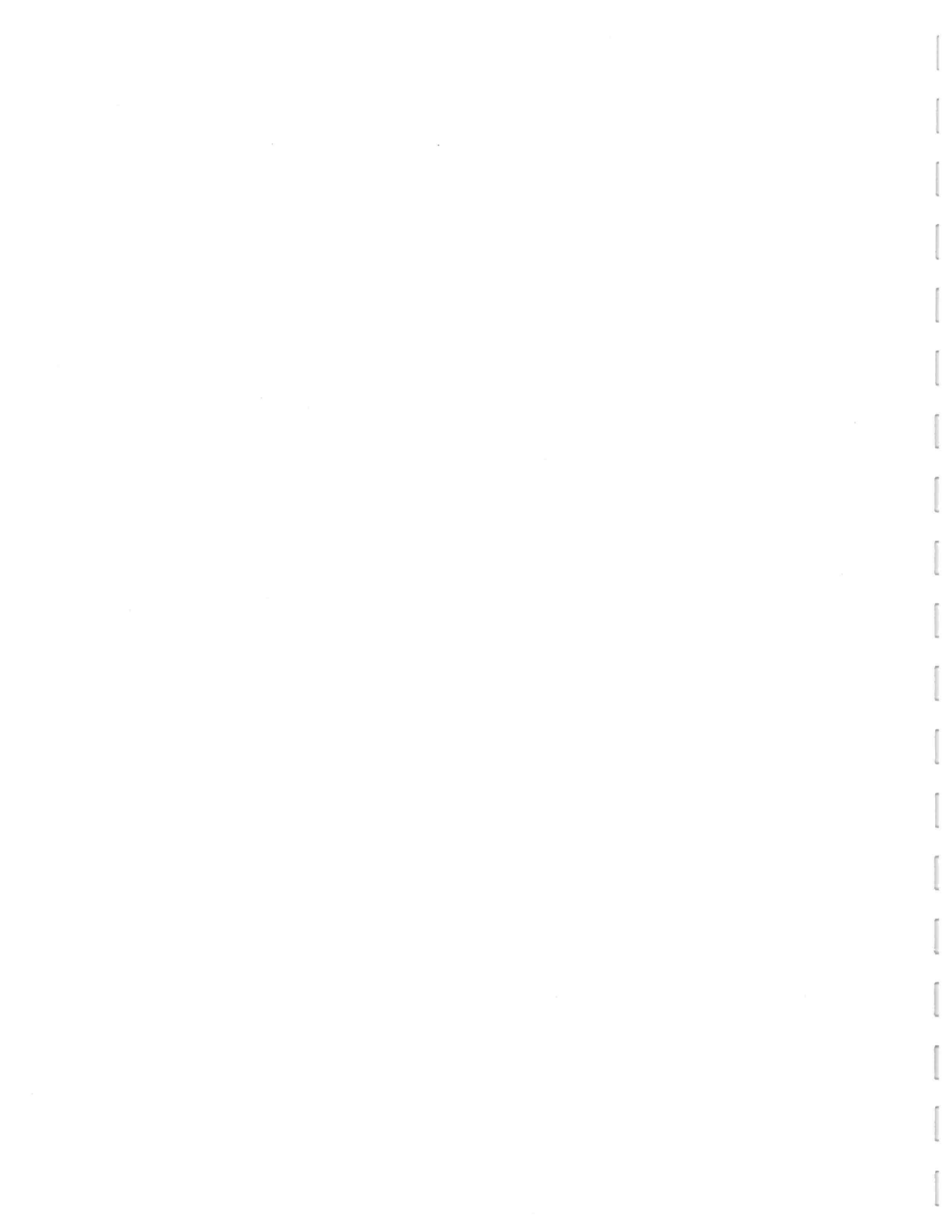


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 583 MADISON AVE #1719, FORT WORTH, TEXAS 76102, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N-H/S/K) ASTM A553 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SOLE RESPONSIBILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave

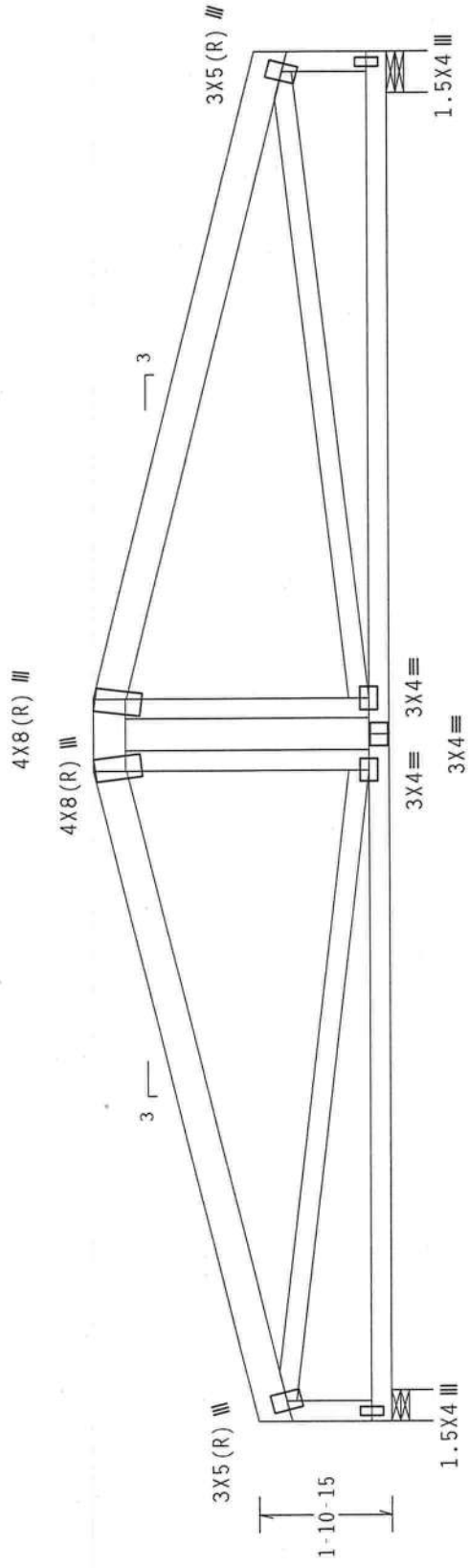


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

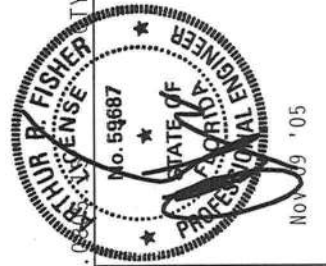
110 mph wind, 23.66 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



9-4-0  
 1-0-8  
 19-8-8 Over 2 Supports  
 R-597 U=180 W=7.25"



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

Scale = .375"/Ft.

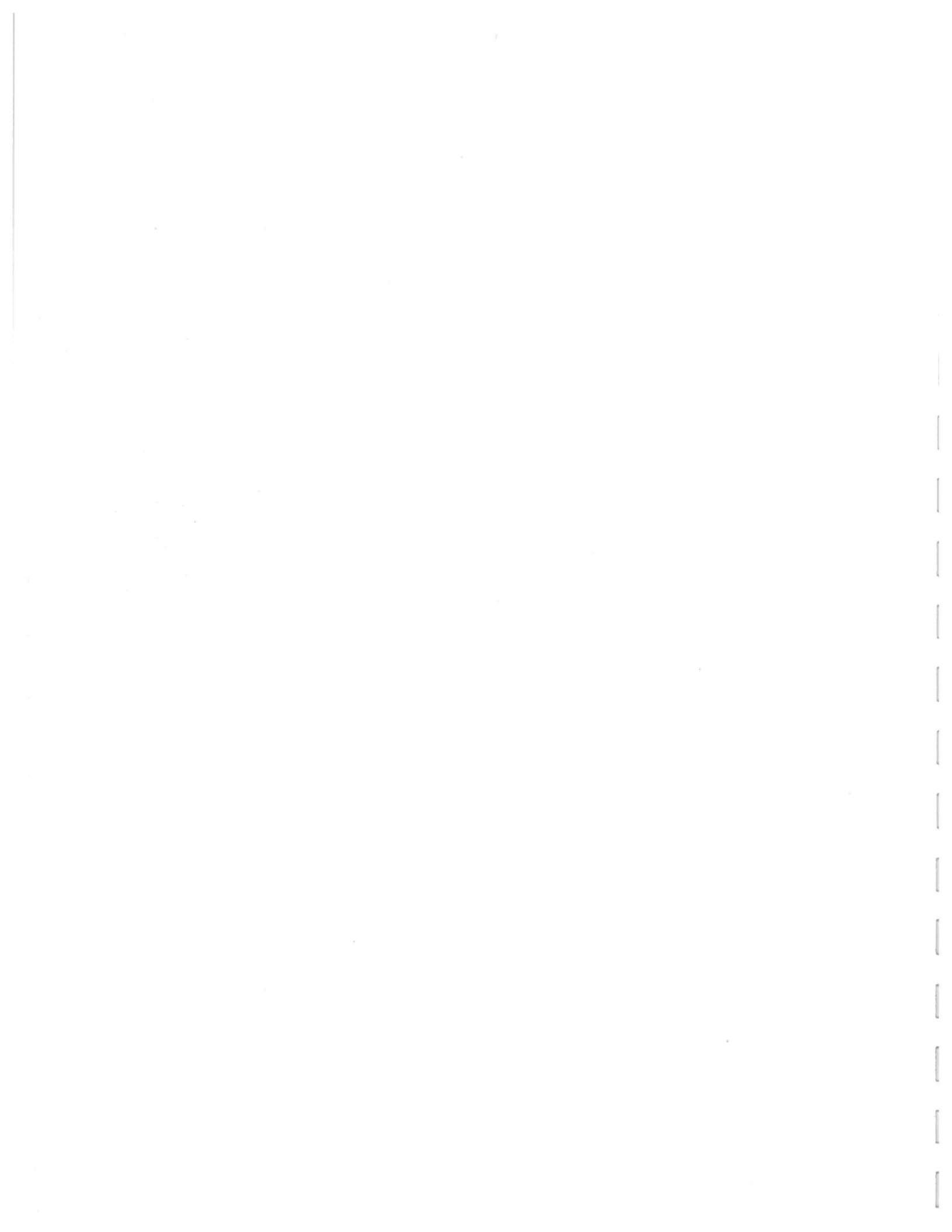
REF	R487 - 35456
DATE	11/09/05
DRW	HCUSR487 05313086
HC-ENG	JB/AF
SEQN-	126804
JREF-	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC3 1-03 (BUILDING COMPONENT SAFETY INFORMATION), THE 2001 INTERNATIONAL BUILDING CONGRESS OF AMERICA 6300 ENTERPRISE BL, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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PLT TYP. Wave

Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 1950 Manley Drive  
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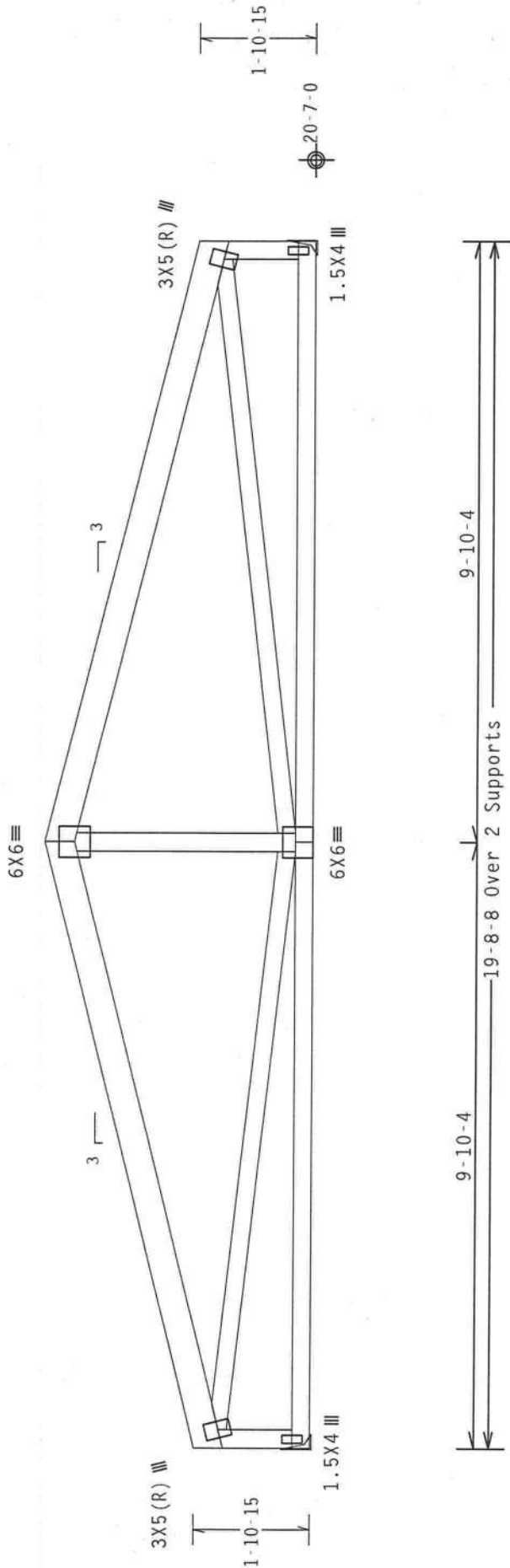
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 23.73 ft mean hgt, ASCE 7-98, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=597 U=180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=597 U=180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.

PLT TYP. Wave

Scale = .375" /Ft.

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

REF	R487 --	35457
DATE	11/09/05	
DRW	HCUSR487	05313087
HC-ENG	JB/AF	
SEQN-	126782	
JREF-	1SS0487_Z01	

**ALPINE**

Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Gaines City, FL 33844  
 FL Certificate of Authorization # 567

**ARTHUR R. FISHER**  
 LICENSE  
 No. 99687  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 Nov 09 '05

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 500 D'ONOFRELO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense :B2 2x4 SP #2 Dense:  
 Webs 2x4 SP #3 :W2 2x4 Sp #2 Dense:

110 mph wind, 23.49 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

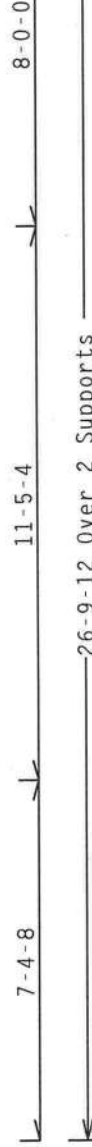
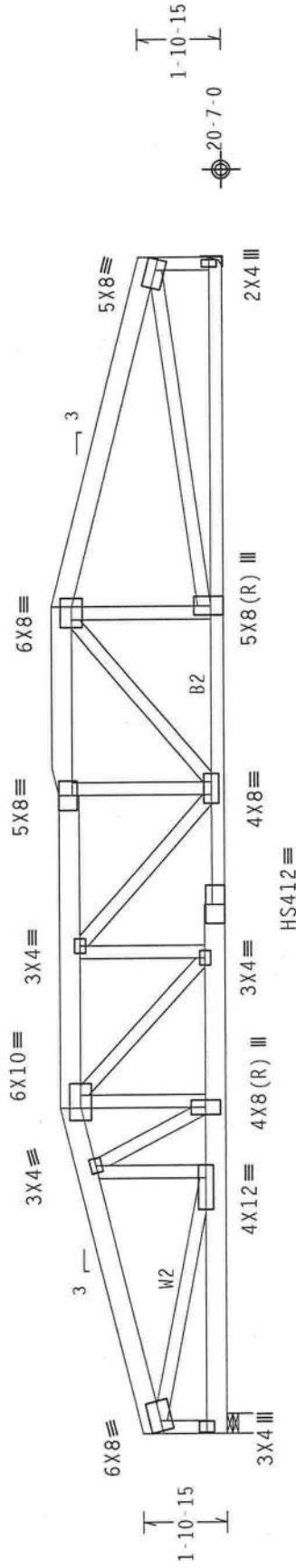
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 7.37  
 TC - From 47 PLF at 7.37 to 47 PLF at 26.81  
 BC - From 13 PLF at 0.00 to 13 PLF at 26.81  
 BC - 811 LB Conc. Load at 6.73  
 BC - 1635 LB Conc. Load at 7.04

Trusses to be spaced at 16.0" OC maximum.

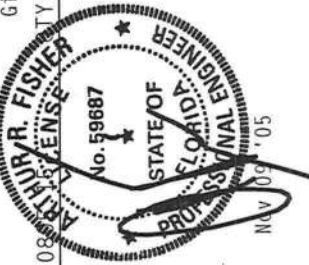
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=2626 U=719 W=5.375"

26-9-12 Over 2 Supports

R-1445 U-392 H-Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine



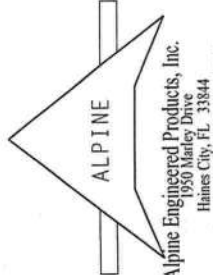
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

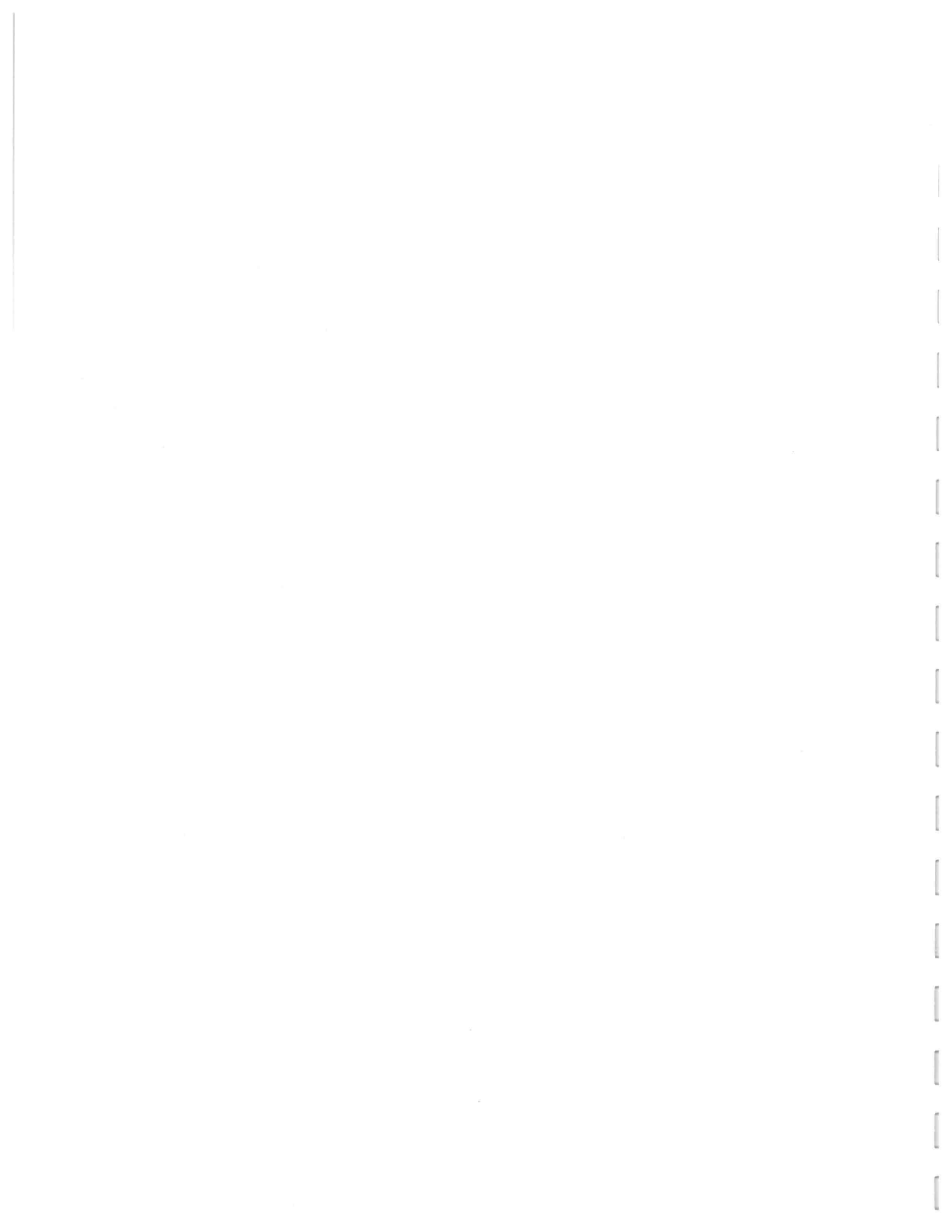
PLT TYP. 20 Gauge HS, Wave

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 (BUILDING COMPONENT SAFETY INFORMATION) FOR FABRICATION, HANDLING, SHIPPING AND BRACING REQUIREMENTS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND CONNECTIONS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND CONNECTIONS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND CONNECTIONS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL DIMENSIONS AND CONNECTIONS.

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TC LL	20.0 PSF	REF	R487--	35458
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313088
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126828	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	



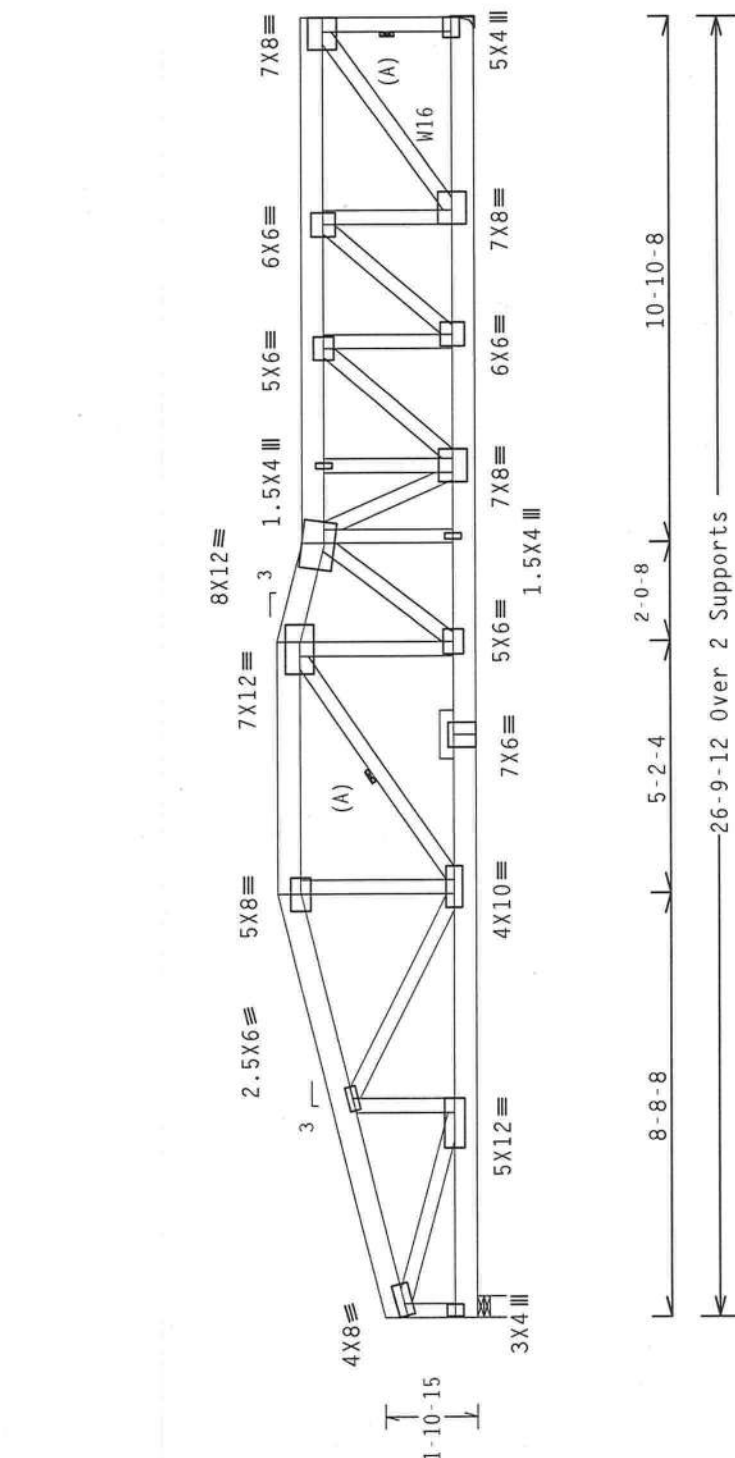


**SPECIAL LOADS**  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 26.81  
 BC - From 13 PLF at 0.00 to 13 PLF at 26.81  
 TC - From 158 LB Conc. Load at 24.00, 25.33, 26.67  
 BC - 2273 LB Conc. Load at 17.54  
 BC - 620 LB Conc. Load at 18.54  
 BC - 483 LB Conc. Load at 22.67  
 BC - 44 LB Conc. Load at 24.00, 25.33, 26.67

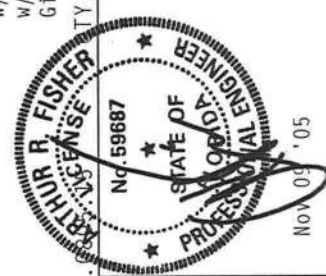
Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3 :W16 2x4 SP #2 Dense:  
 :Lt Splice Block 2x4 SP #3:  
 110 mph wind, 23.58 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.  
 Trusses to be spaced at 16.0" OC maximum.

Right end vertical not exposed to wind pressure.  
 (A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1896 U=544 W=5.375"  
 R=3711 U=1098 H=Simpson HGUS26  
 w/ (8) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (20) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (2)2X6 min. So.Pine  
 TY:1 FL/-/3/-/R/- Scale = .25" / Ft.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

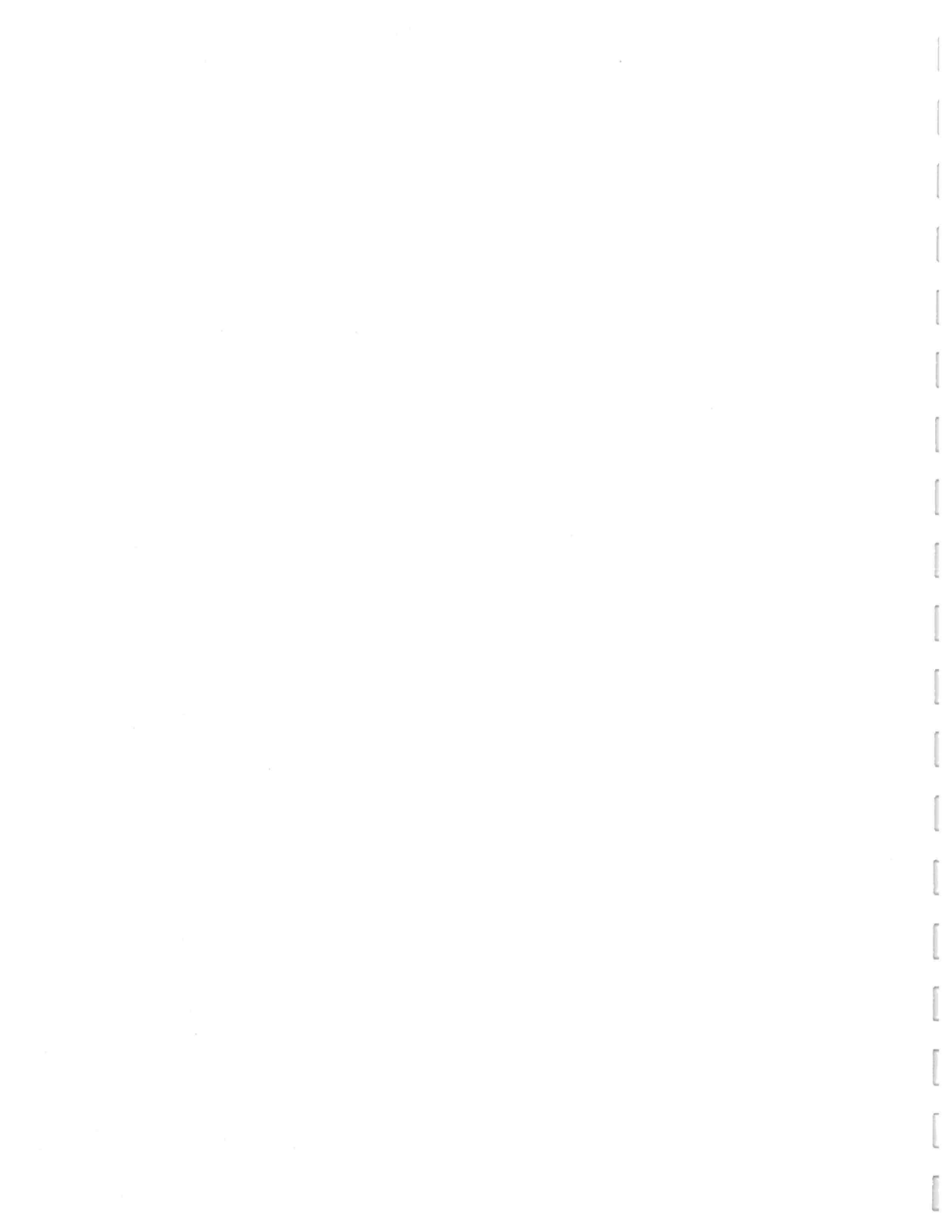
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 893 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED BRONCHES, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (N/A/5/6) ASTM A653 GRADE 40/60 (H, K/M/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002-SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave

Alpine Engineered Products, Inc.  
 1930 Mabrey Way  
 Gaines City, FL 33844  
 FL Certificate of Authorization # 567

TC LL	20.0 PSF	REF	R487 -	35459
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313089
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	161293	
DUR.FAC.	1.25	JREF-	ISS0487_Z01	
SPACING	16.0"			



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

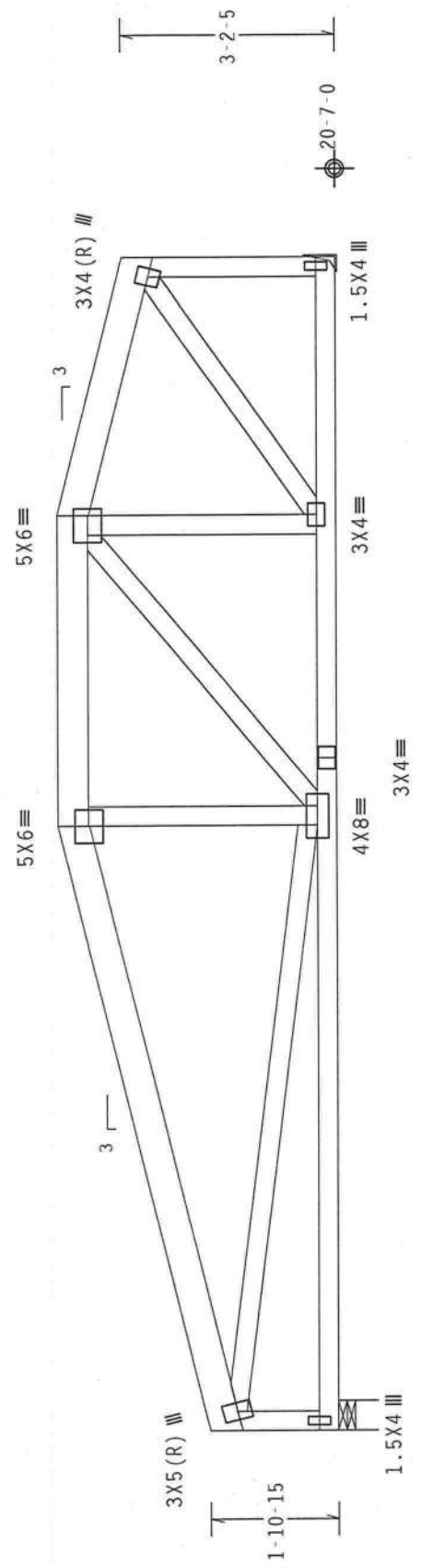
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 23.62 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.



R-534 U=180 W=5.375" 17-5-12 Over 2 Supports  
 R=526 U=180 H=Simpson LU24 w/ (2) 10d, 0.148"x1.5" nails in Truss w/ (4) 16d, 0.162"x2.5" nails in Girder. Girder is (1)2X6 min. So.Pine.

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

Scale = .375"/Ft.

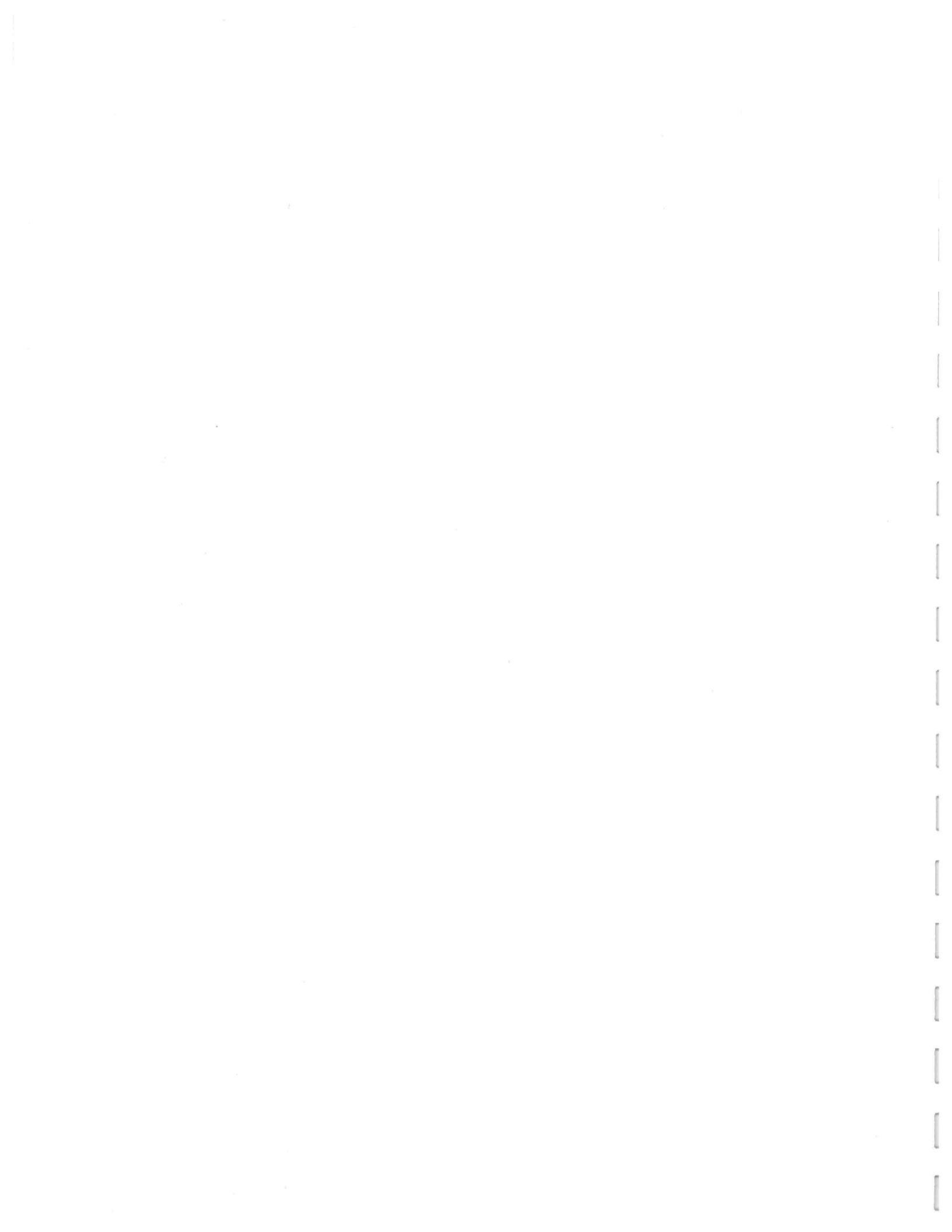


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING DEPARTMENT) FOR SAFETY. THIS TRUSS PLATE INSTITUTE (B3) HAS BEEN APPROVED BY THE NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI- OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI- ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/X) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELT FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487 - -	35460
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313090
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	161226	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	ISS0487_Z01	

**Alpine Engineered Products, Inc.**  
 1950 Matey Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



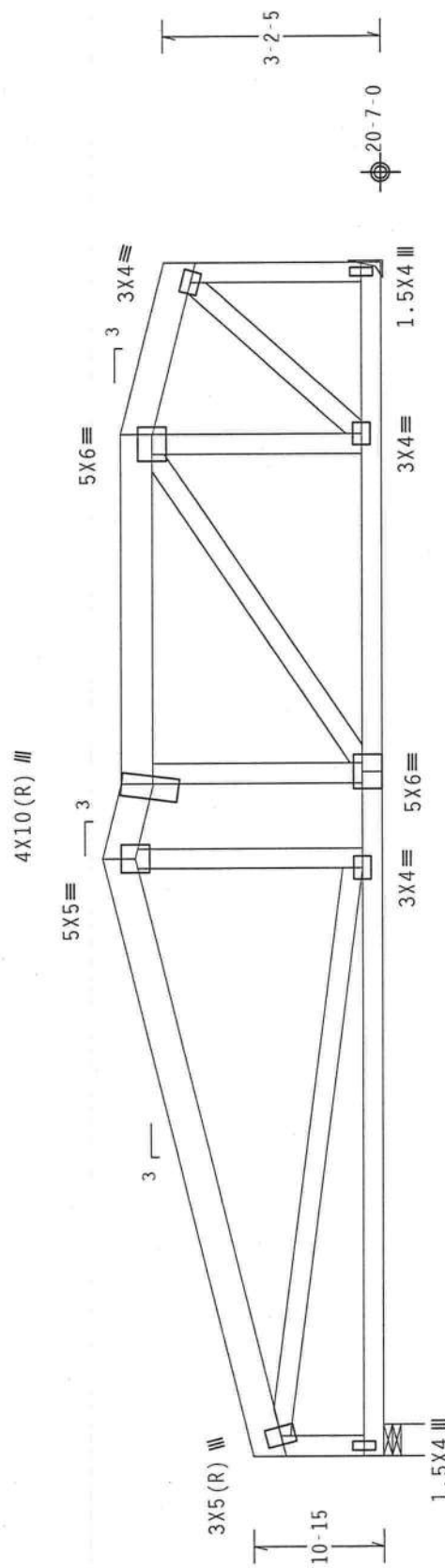
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 23.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.



R=534 U=180 W=5.375"

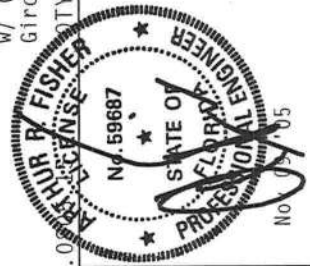
17-5-12 Over 2 Supports

R=526 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 16d, 0.162"x2.5" nails in Girder  
 Girder is (1)2X4 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00

PLT TYP. Wave

FL/-/3/-/-/R/- Scale = .375"/Ft.

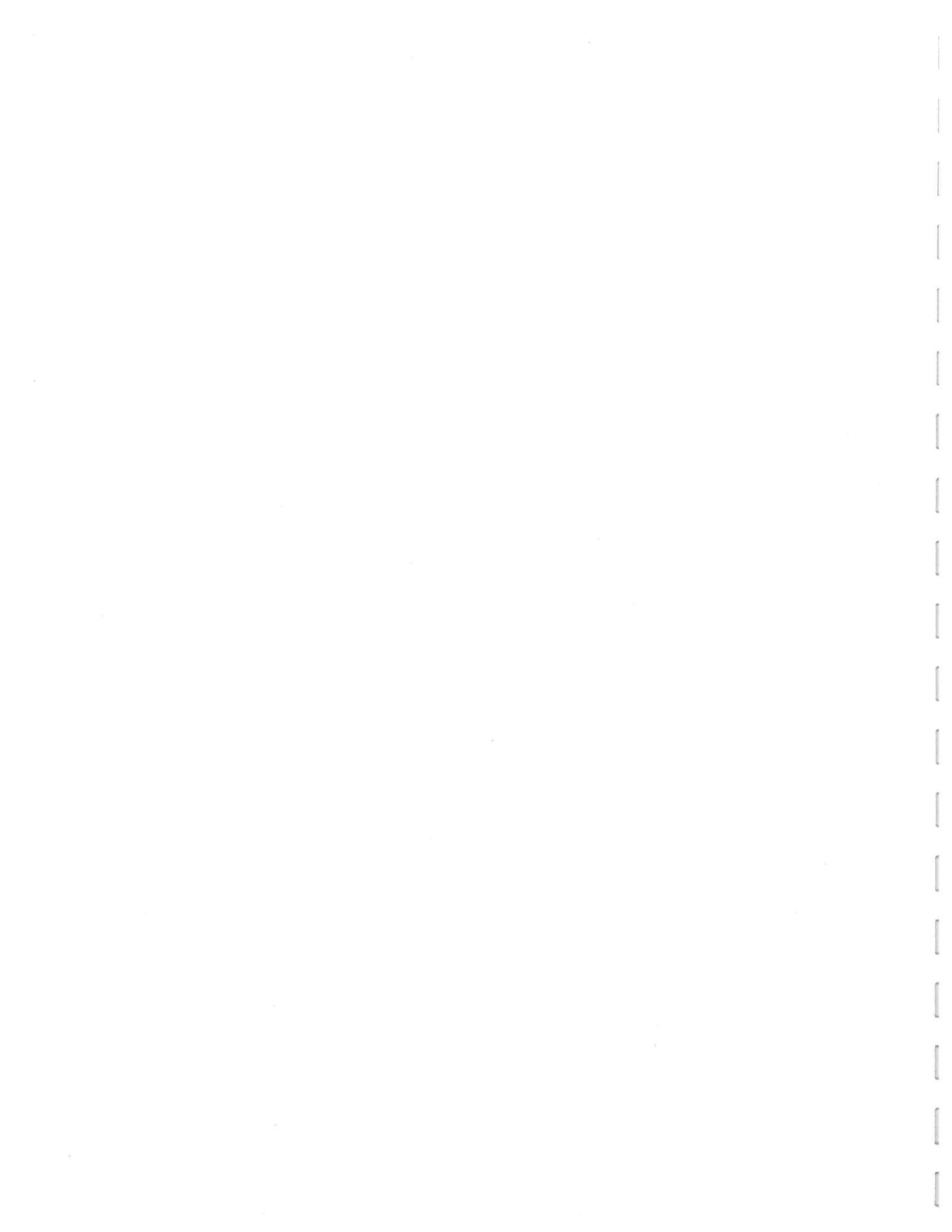


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), TRUSS COUNCIL OF AMERICA, ENDO ENTERPRISE INC, HAINES CITY, FL 33844 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (H-H/5/3) ASTM A653 GRADE 40/60 (H. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ADEK AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**Alpine Engineered Products, Inc.**  
 Haines City, FL 33844  
 1950 Matley Drive  
 FL Certificate of Authorization # 567

TC LL	20.0 PSF	REF	R487--	35461
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313091
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	161235	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

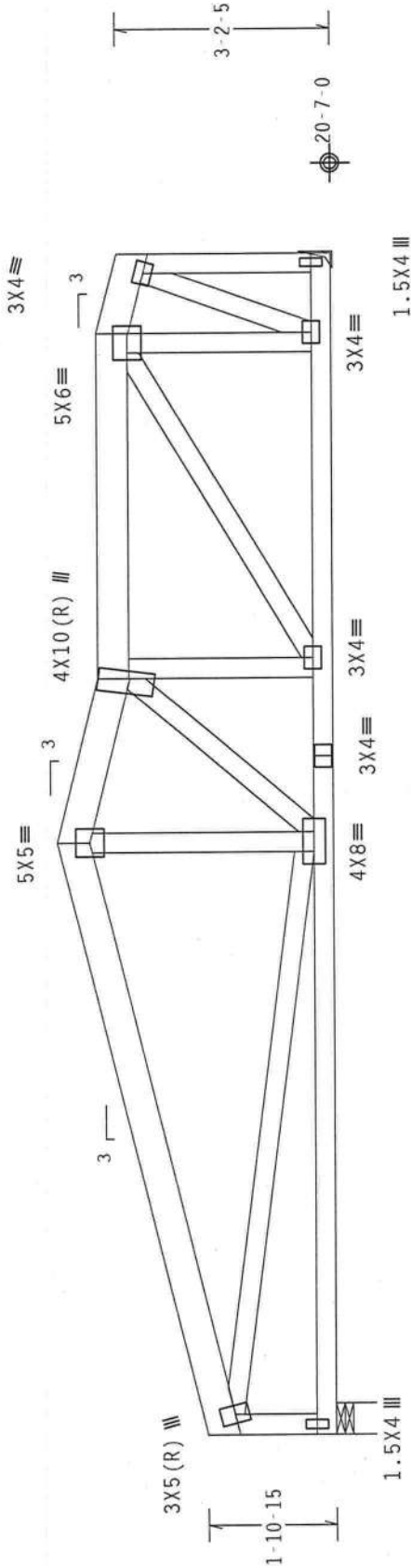
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 23.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.



17-5-12 Over 2 Supports

R=534 U-180 W=5.375"

R=526 U-180 H-Simpson LUZ4  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 16d, 0.162"x2.5" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TY:1 FL/-3/-/-R/- Scale = .375"/Ft.



PLT TYP. Wave

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST L-03 (BUILDING CODES) FOR SAFETY CONSIDERATIONS. TRUSSES MANUFACTURED BY TPI (TRUSS PLATE INSTITUTE, 803 MADISON, WI 53719) AND UFGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H-J/S/F) ASTM A653 GRADE 40/60 (H, K/J-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX AS OF 10/11/2002, SEC.3. UNLESS OTHERWISE INDICATED, DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

TC LL	20.0 PSF	REF	R487--	35462
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313092
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	161241	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	

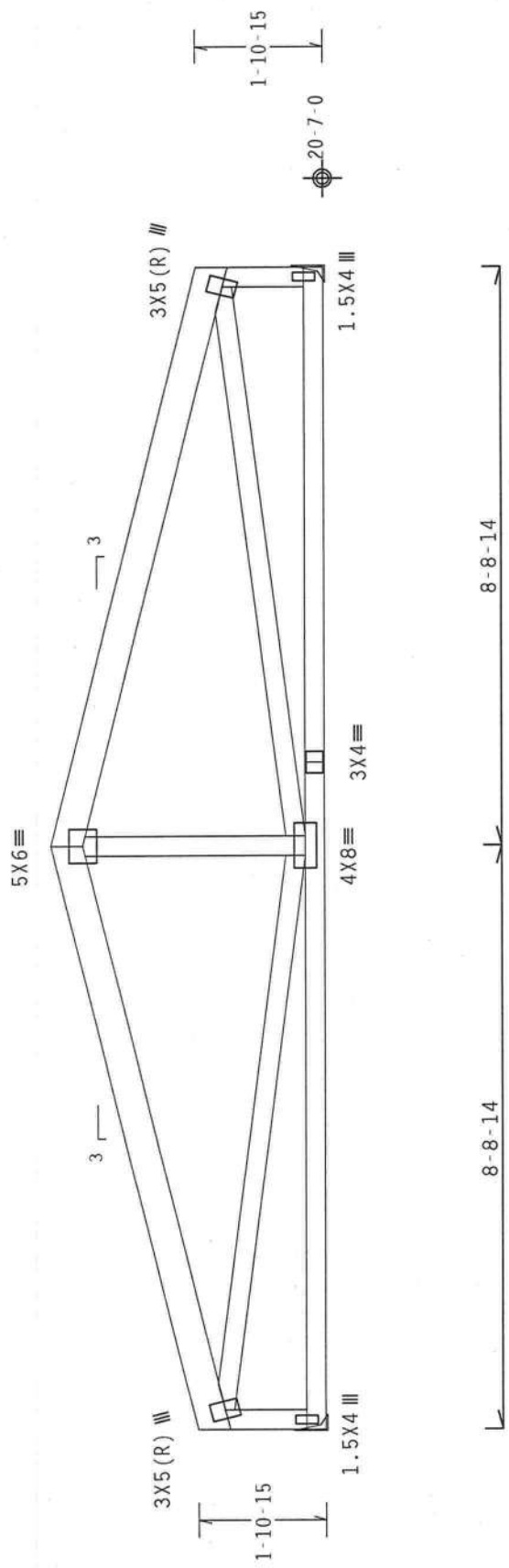


110 mph wind, 23.59 ft mean hgt. ASCE 7-98. CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3



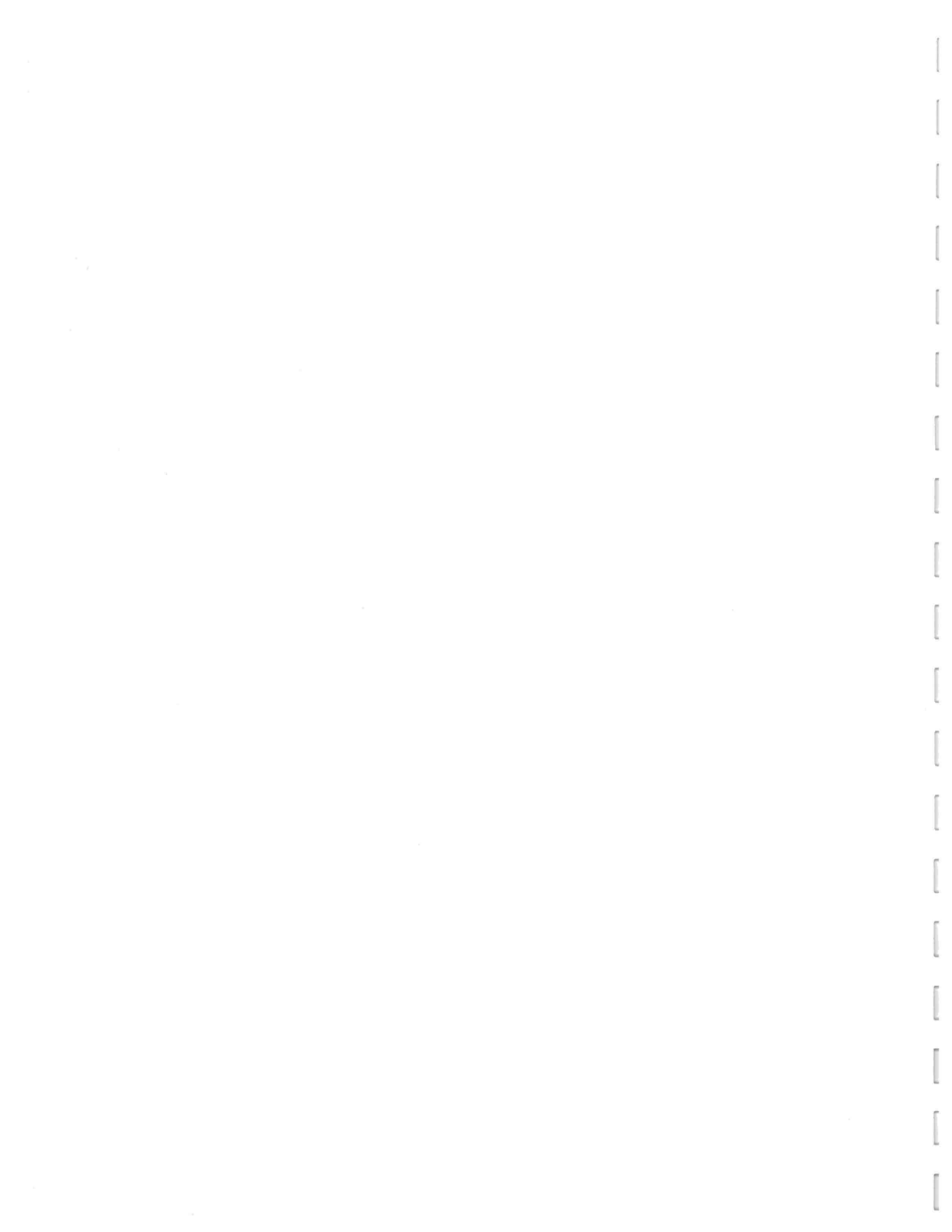
PLT TYP. Wave  
 R-530 U-180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 16d, 0.162"x2.5" nails in Girder  
 Girder is (1)2X6 min. So.Pi Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00  
 Scale = .375" / Ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 993 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND HCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE RD, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. ALL TRUSSES AND CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID SKELETON.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/S) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

<p>ALPINE                  Engineered Products, Inc.                  1930 W. Highway 10                  Gaines City, FL 32644                  FL Certificate of Authorization # 567</p>	<table border="1"> <tr> <td>TC LL</td> <td>20.0 PSF</td> <td>REF</td> <td>R487--</td> <td>35463</td> </tr> <tr> <td>TC DL</td> <td>15.0 PSF</td> <td>DATE</td> <td>11/09/05</td> <td></td> </tr> <tr> <td>BC DL</td> <td>10.0 PSF</td> <td>DRW</td> <td>HCUSR487</td> <td>05313093</td> </tr> <tr> <td>BC LL</td> <td>0.0 PSF</td> <td>HC-ENG</td> <td>JB/AF</td> <td></td> </tr> <tr> <td>TOT.LD.</td> <td>45.0 PSF</td> <td>SEON-</td> <td>161248</td> <td></td> </tr> <tr> <td>DUR.FAC.</td> <td>1.25</td> <td></td> <td></td> <td></td> </tr> <tr> <td>SPACING</td> <td>16.0"</td> <td>JREF-</td> <td>ISS0487_Z01</td> <td></td> </tr> </table>	TC LL	20.0 PSF	REF	R487--	35463	TC DL	15.0 PSF	DATE	11/09/05		BC DL	10.0 PSF	DRW	HCUSR487	05313093	BC LL	0.0 PSF	HC-ENG	JB/AF		TOT.LD.	45.0 PSF	SEON-	161248		DUR.FAC.	1.25				SPACING	16.0"	JREF-	ISS0487_Z01	
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DUR.FAC.	1.25																																			
SPACING	16.0"	JREF-	ISS0487_Z01																																	



Top chord 2x6 SP #2  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3 :W2, W4 2x4 SP #2 Dense:  
 110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Max JT VERT DEFL: LL: 0.11" DL: 0.20" recommended camber 3/8"  
 Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor  
 for dead load is 1.50.  
 The TC of this truss shall be braced with attached spans at 24" OC in  
 lieu of structural sheathing.

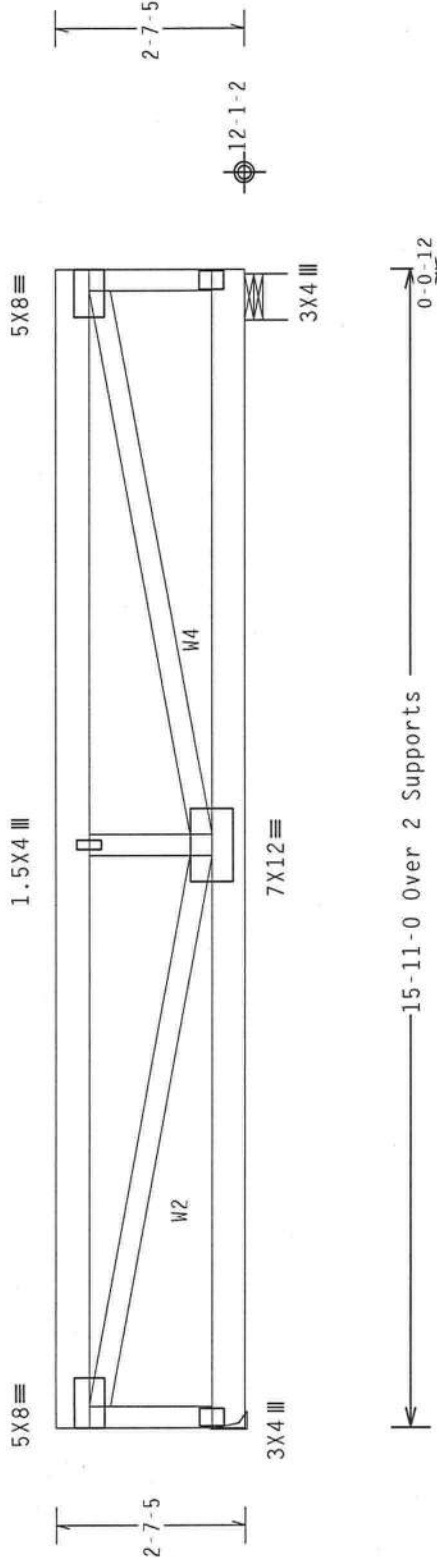
**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 15.92  
 BC - From 13 PLF at 0.00 to 13 PLF at 15.92  
 BC - 264 LB Conc. Load at 1.27, 2.60, 3.94, 5.27, 6.60  
 7.94, 8.56

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and  
 calculations. Conditions may exist that require different connections  
 than indicated. Refer to manufacturer publication for additional  
 information.

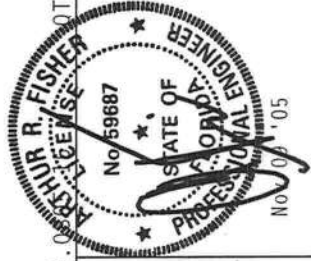
Truss must be installed as shown with top chord up.



R=1733 U=546 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

R=1067 U=329 W=7.778"



PLT TYP. Wave



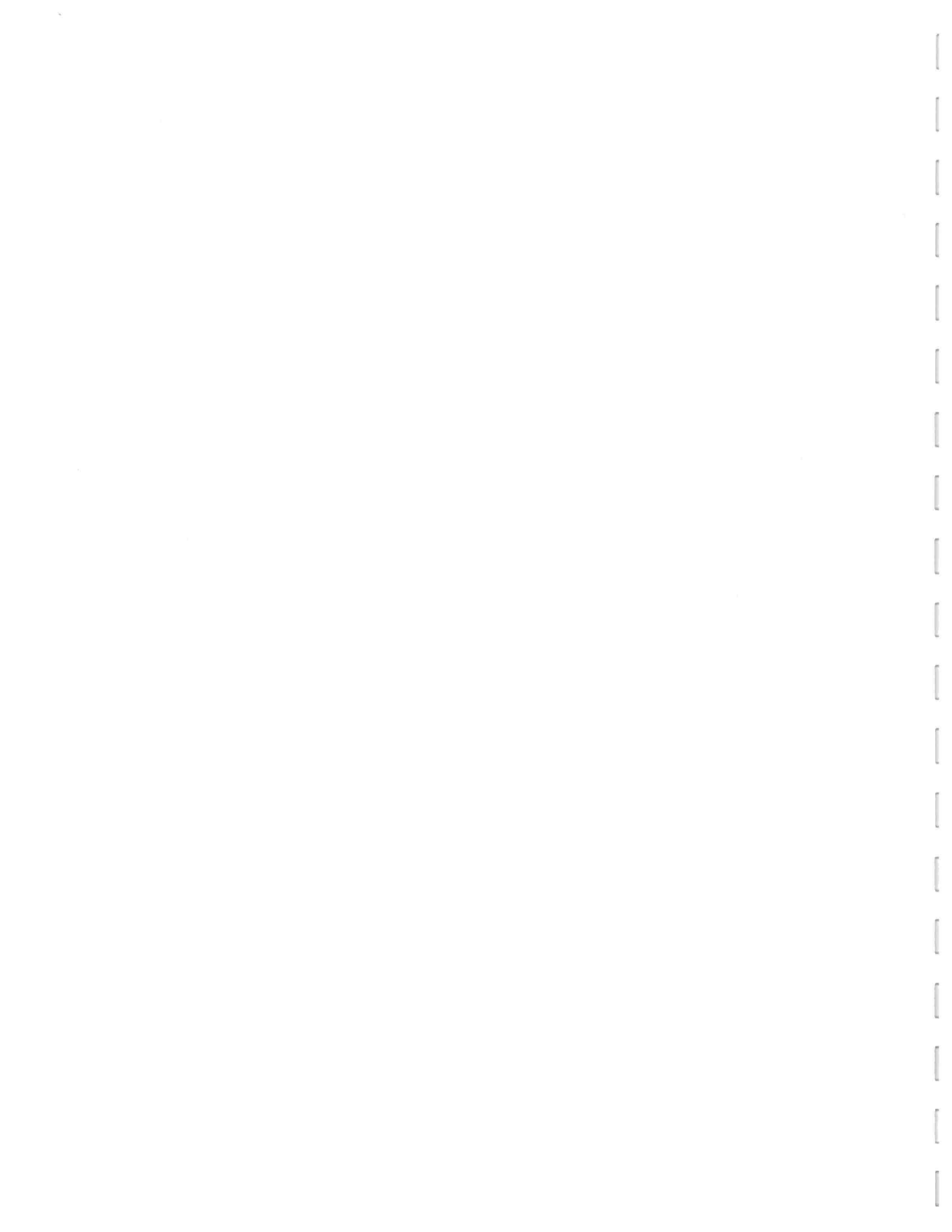
Alpine Engineering Products, Inc.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 393 D. HONORE DR., ZIP 32706, MADISON, WI 53719, AND NECA (WOOD TRUSS COUNCIL OF AMERICA), 608 WEST 10TH ST., TOP GIRDERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (4.4/5/6) ASTM A653 GRADE 40/60 (4. K/JH-5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BEING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487--	35464
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313094
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126923	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



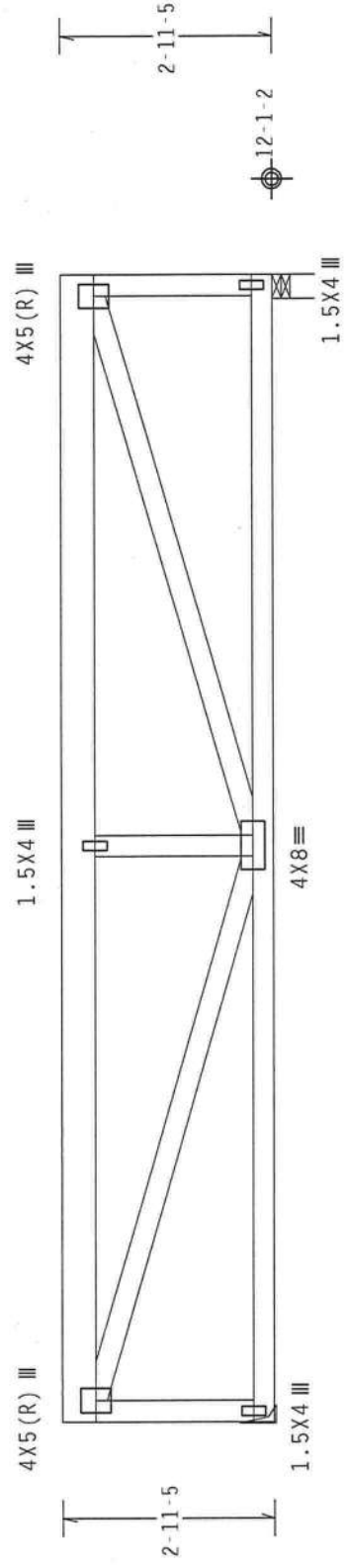
110 mph wind, 15.03 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Provide for complete drainage of roof.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

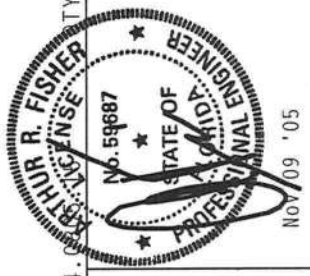
End verticals not exposed to wind pressure.  
Trusses to be spaced at 16.0" OC maximum.  
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.  
Truss must be installed as shown with top chord up.



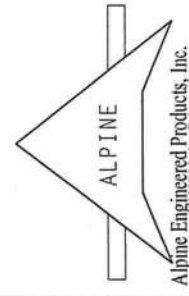
15-11-0 Over 2 Supports

R-478 U=180 H=Simpson LU24  
w/ (2) 10d, 0.148"x1.5" nails in Truss  
w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD)/FBC  
Cq/RT=1.00(1.25)/10(0)



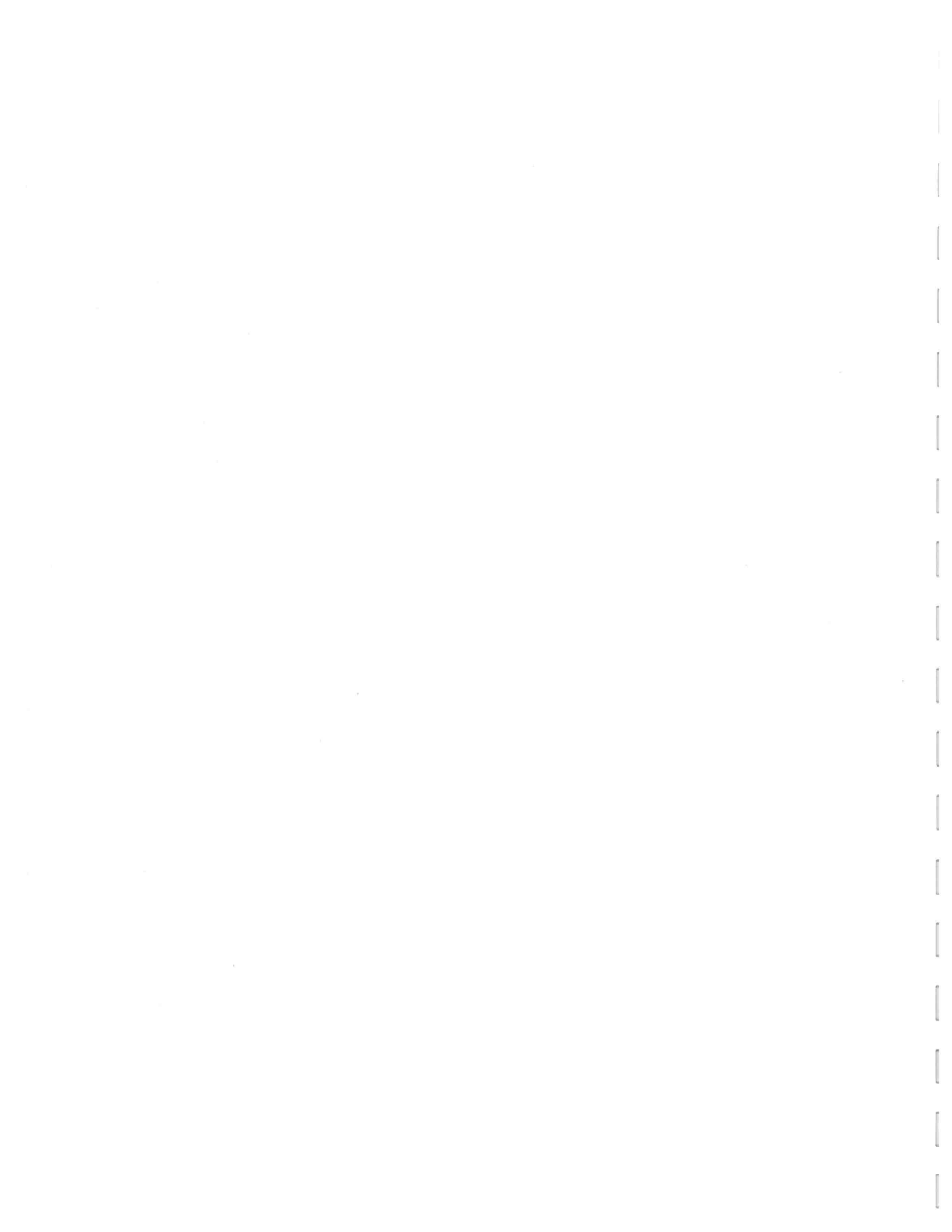
PLT TYP. Wave



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR BUILDING TRUSSES AND BRACING. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (0.0175/0.013) ASTM A653 GRADE 40/60 (4. K/II-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEC AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (0.0175/0.013) ASTM A653 GRADE 40/60 (4. K/II-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEC AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375"/Ft.
TC DL	15.0 PSF	REF R487 --	35465
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313095
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126929
SPACING	16.0"	JREF-	1SS0487_Z01



Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

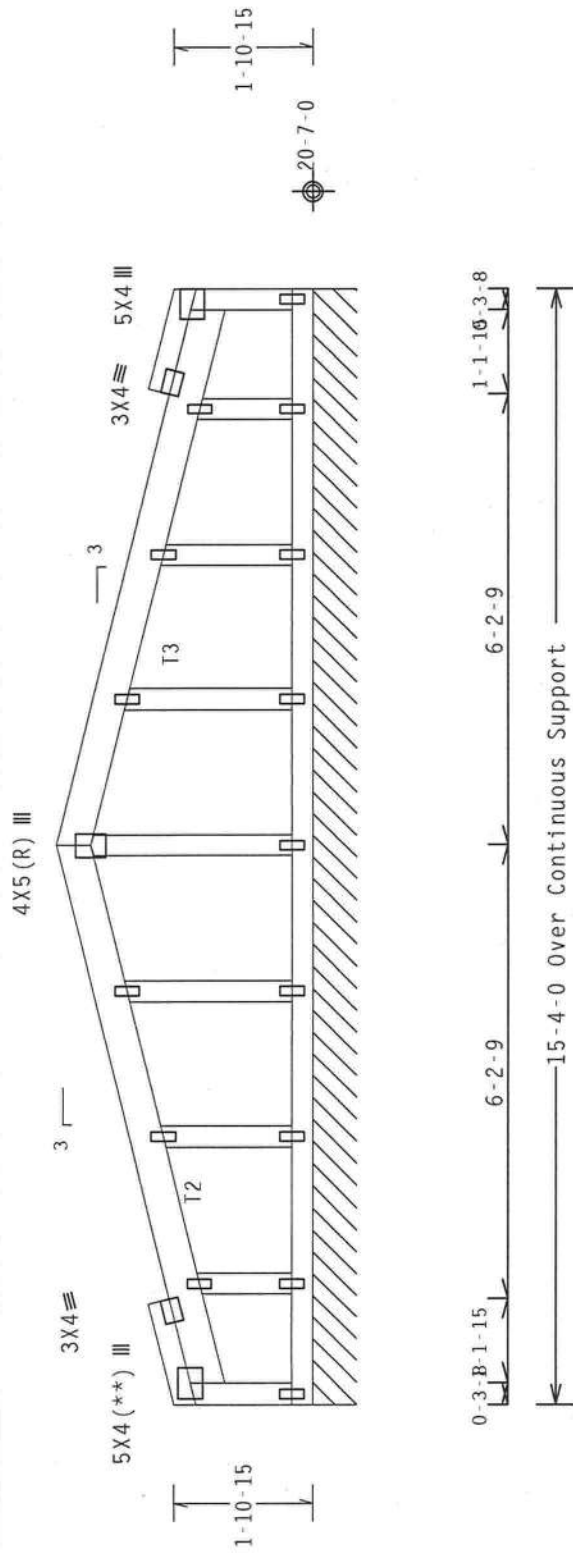
**SPECIAL LOADS**  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 70 PLF at 0.00 to 70 PLF at 0.29  
 TC - From 91 PLF at 0.29 to 91 PLF at 15.33  
 BC - From 20 PLF at 0.00 to 20 PLF at 15.33

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

(\*\*) Plate relocated as shown.

110 mph wind, 23.19 ft mean hgt. ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Dead loads are stated on projected horizontal area basis.  
 See DWGS A11030EC1103 & GBLLETIN0405 for more requirements.  
 Fasten rated sheathing to one face of this frame.



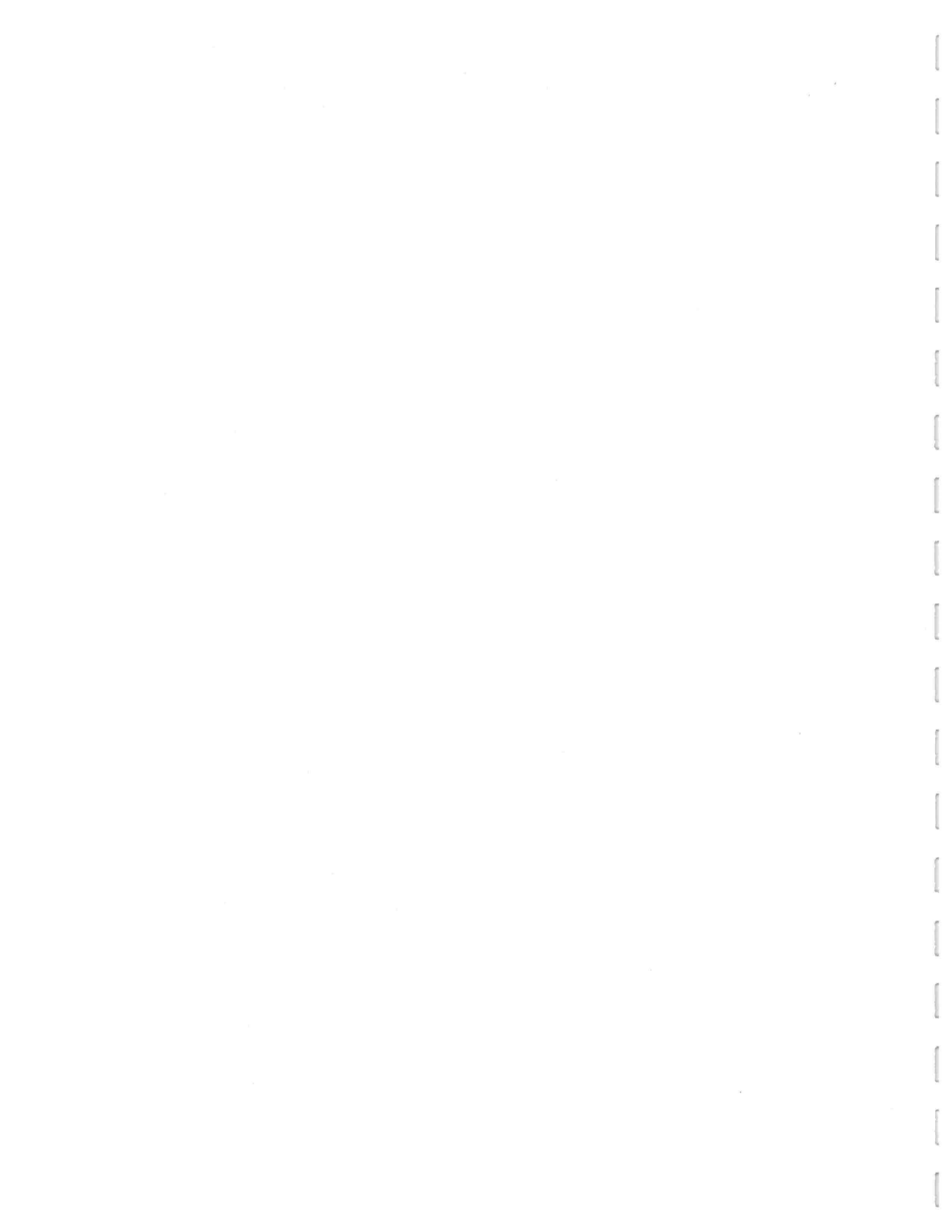
Note: All Plates Are 1.5X4 Except As Shown.  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



**ALPINE**  
 Alpine Engineered Products, Inc.  
 James City, FL 33844  
 1950 Manley Drive

FL Certificate of Authorization # 567

PLT TYP. Wave	FL/-/3/-/ -/R/-	Scale = .375" /Ft.
TC LL	20.0 PSF	REF R487-- 35466
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313096
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 126841
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SS0487_Z01

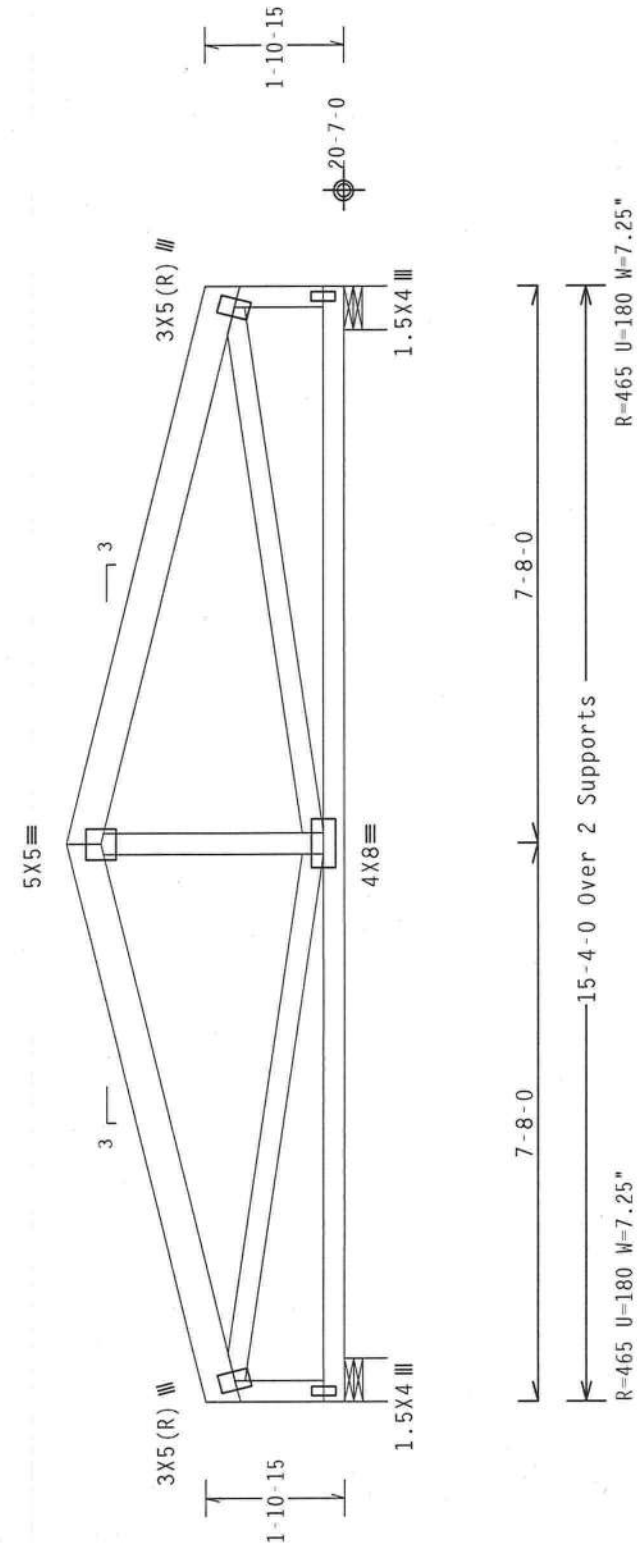


110 mph wind, 23.45 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

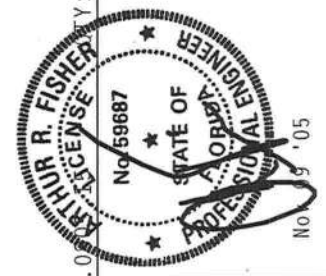
Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.



Design Crit: TPI-2002 (STD) / FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.0

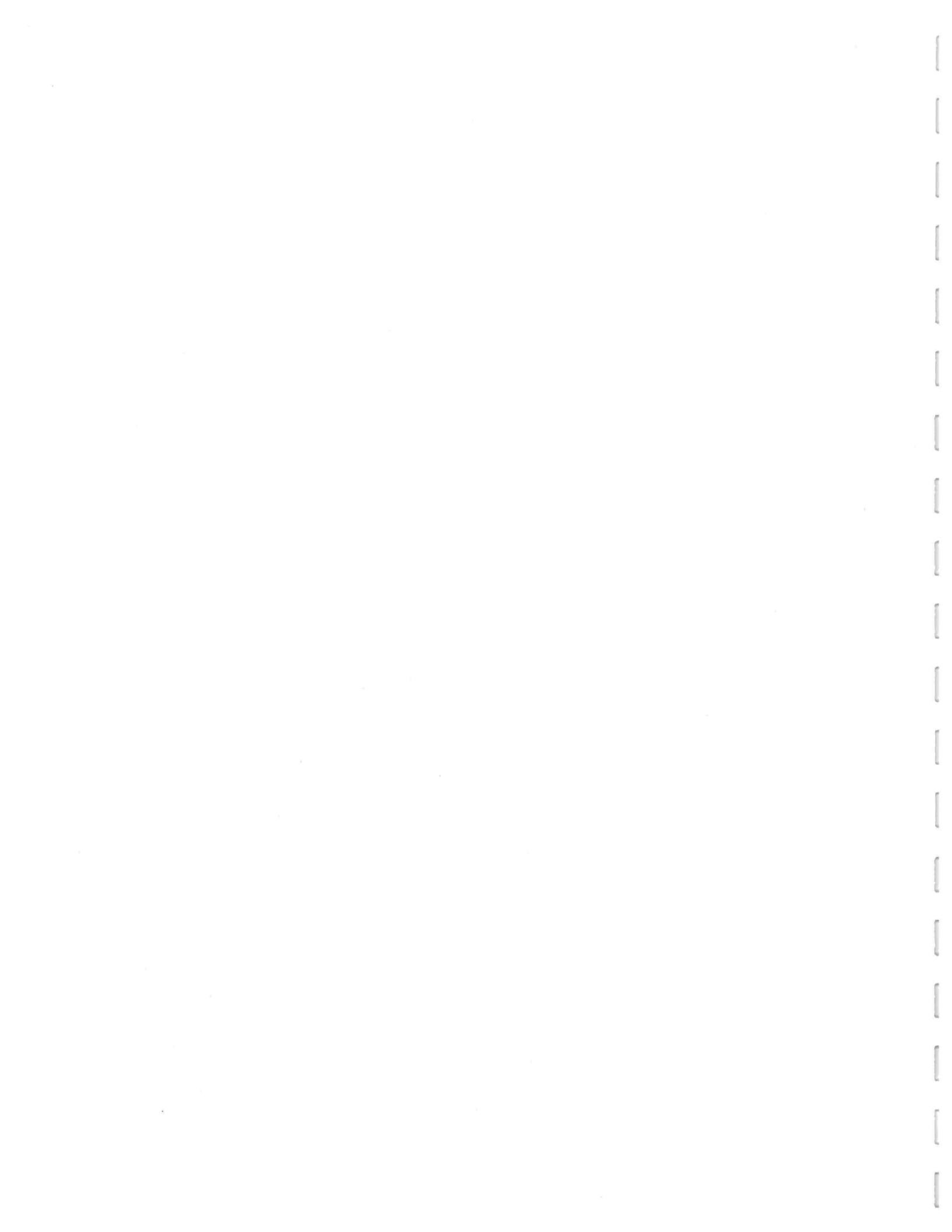
TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - - 35467	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313020	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	*
DUR.FAC.	1.25	SEQN- 160804	
SPACING	16.0"	JREF- ISS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 HADFIELD BLVD., SOUTH BEND, IN 46708) AND TO THE TPI HANDBOOK FOR TRUSS CONNECTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSS CONNECTIONS SHALL BE MADE IN ACCORDANCE WITH THE TPI HANDBOOK. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (MATRIBAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/5/5) ASTM A653 GRADE 40/60 (H, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**Alpine Engineered Products, Inc.**  
Haines City, FL 33844  
1950 Marley Drive  
FL Certificate of Authorization # 567



Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3 :W2 2x4 SP #2 Dense:

**SPECIAL LOADS**

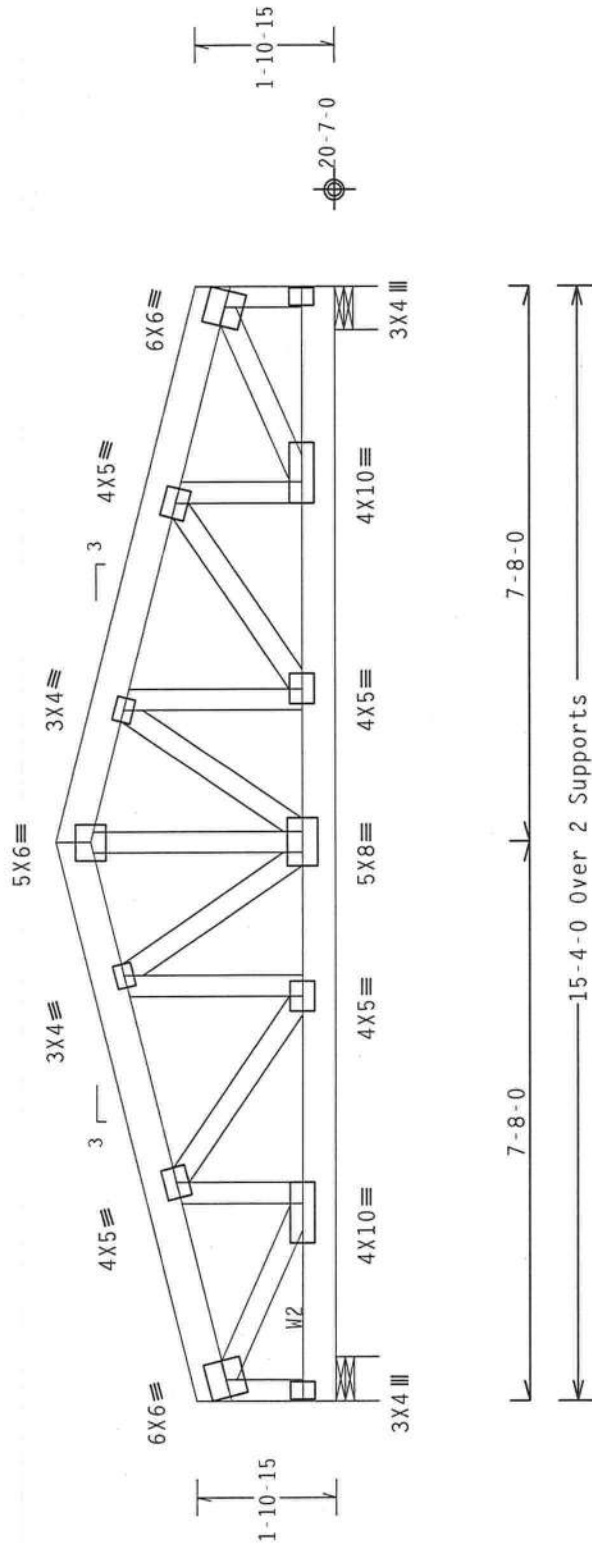
----- (LUMBER DUR\_FAC = 1.25 / PLATE DUR\_FAC = 1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 15.33  
 BC - From 20 PLF at 0.00 to 20 PLF at 15.33  
 BC - 597 LB Conc. Load at 0.60, 1.94, 3.27, 4.60, 5.94  
 BC - 1445 LB Conc. Load at 7.27  
 BC - 3711 LB Conc. Load at 8.60

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

**2 COMPLETE TRUSSES REQUIRED**

Nailing Schedule: (10d Common (0.148"x3" min.) nails)  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 1 Row @ 3.50" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails  
 in each row to avoid splitting.

110 mph wind, 23.45 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



R=5437 U=1590 W=7.25"

R=4100 U=1198 W=7.25"

PLT TYP. Wave

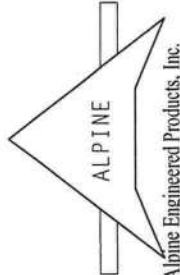
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08



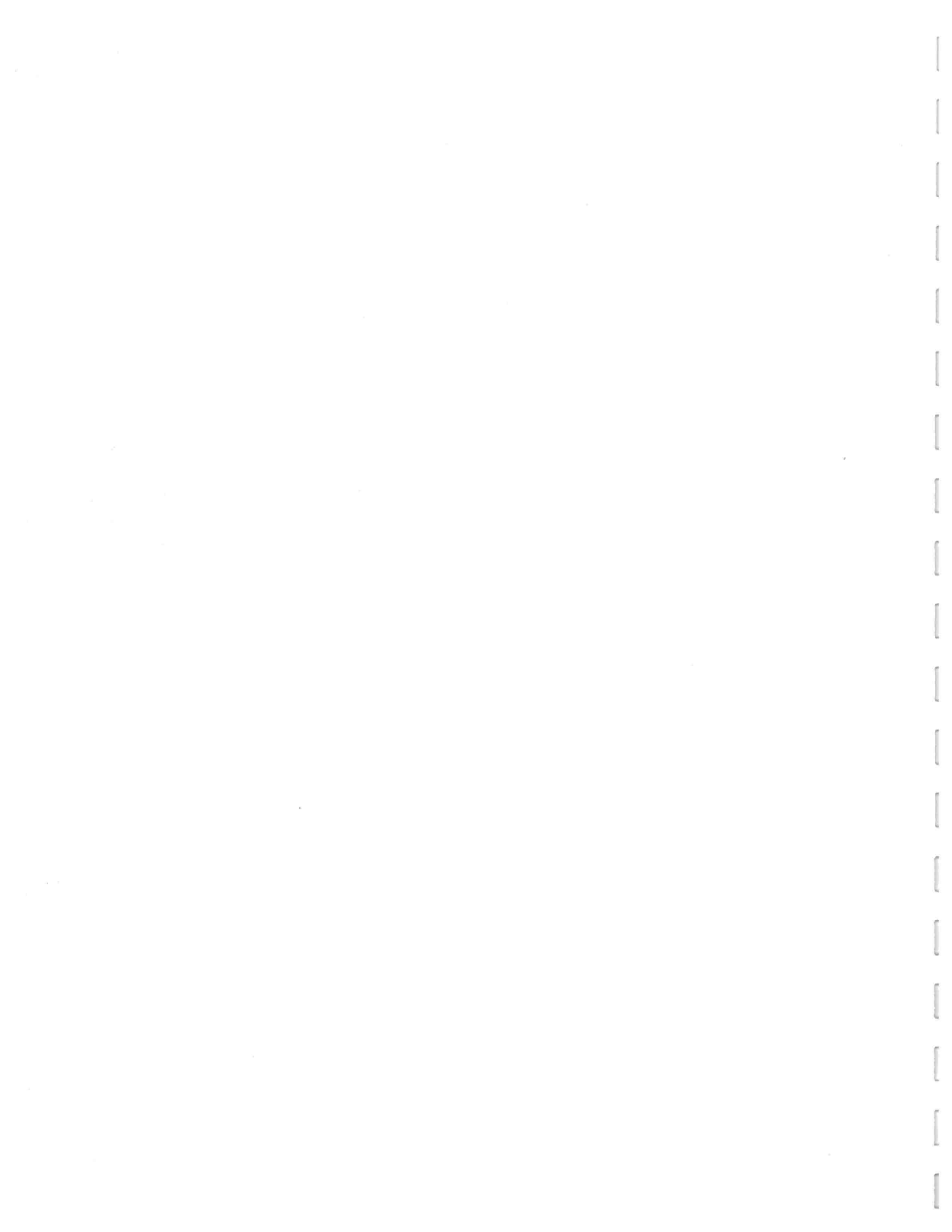
TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - - 35468	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313097	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 161320	
SPACING	24.0"	JREF- 1SS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CROSS PLATE INSTITUTE, 503 D'ONOFRI DR., SUITE 200, MADISON, WI 53719, AND NFCA (WOOD TRUSS COMPANY) UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.



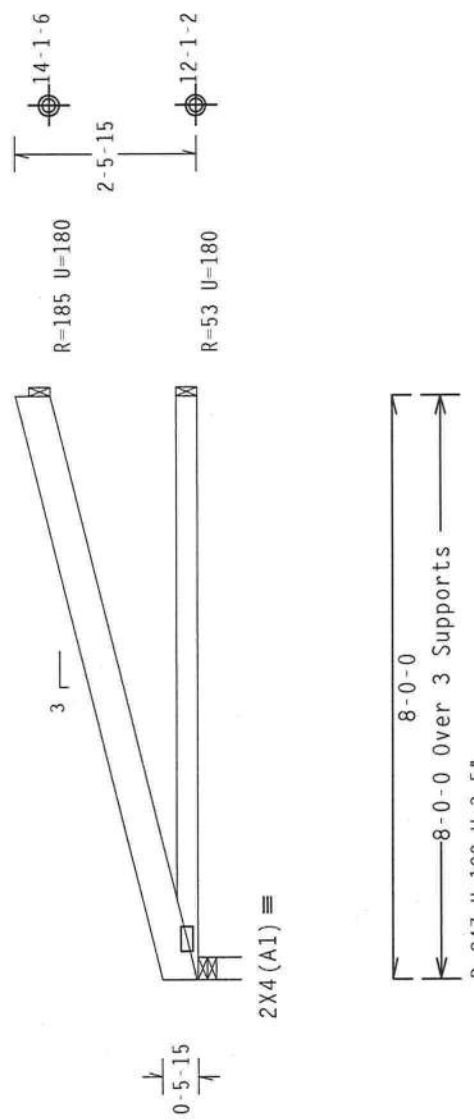
Alpine Engineered Products, Inc.  
 1950 Maple Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



110 mph wind, 13.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Trusses to be spaced at 16.0" OC maximum.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" /Ft.
TC DL	15.0 PSF		REF R487 -- 35469
BC DL	10.0 PSF		DATE 11/09/05
BC LL	0.0 PSF		DRW HCUSR487 05313098
TOT.LD.	45.0 PSF		HC-ENG JB/AF
DUR.FAC.	1.25		SEQN- 126582
SPACING	16.0"		JREF- 1SS0487_Z01

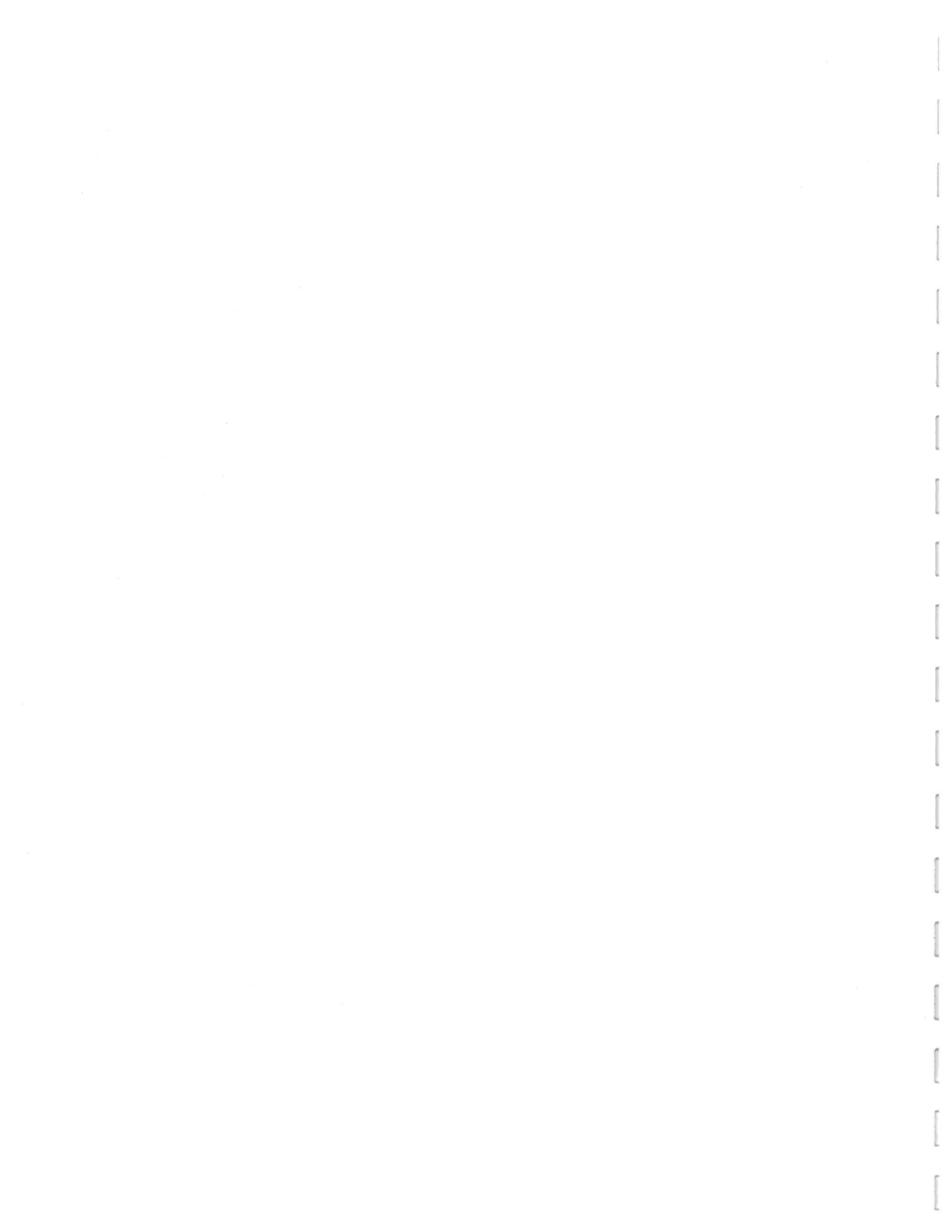


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CROSS PLATE INSTITUTE, 983 D'ONOFREO DR., SUITE 200, MADISON, WI 53719, AND WPCA (WOOD TRUSS CONNECTIONS), UNLESS OTHERWISE INDICATED, AND CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, ETC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H, H/5/16) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS/PTI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 1950 Males Drive  
 FL Certificate of Authorization # 567

PLT TYP. Wave



110 mph wind, 12.75 ft mean hgt, ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.

Provide { 2 } 16d common nails (0.162"x3.5"), toe nailed at Top chord.  
 Provide { 2 } 16d common nails (0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.

TY:12		FL/-	/-3/-	/-R/-	Scale = .375" / Ft.
TC LL	20.0	PSF			REF R487 -- 35470
TC DL	15.0	PSF			DATE 11/09/05
BC DL	10.0	PSF			DRW HCUSR487 05313021
BC LL	0.0	PSF			HC-ENG JB/AF *
TOT.LD.	45.0	PSF			SEQN- 126557
DUR.FAC.	1.25				JREF- 1SS0487_Z01
SPACING	16.0"				

**ALPINE**

Alpine Engineered Products, Inc.  
 Haines City, FL 33844

FL Certificate of Authorization # 567

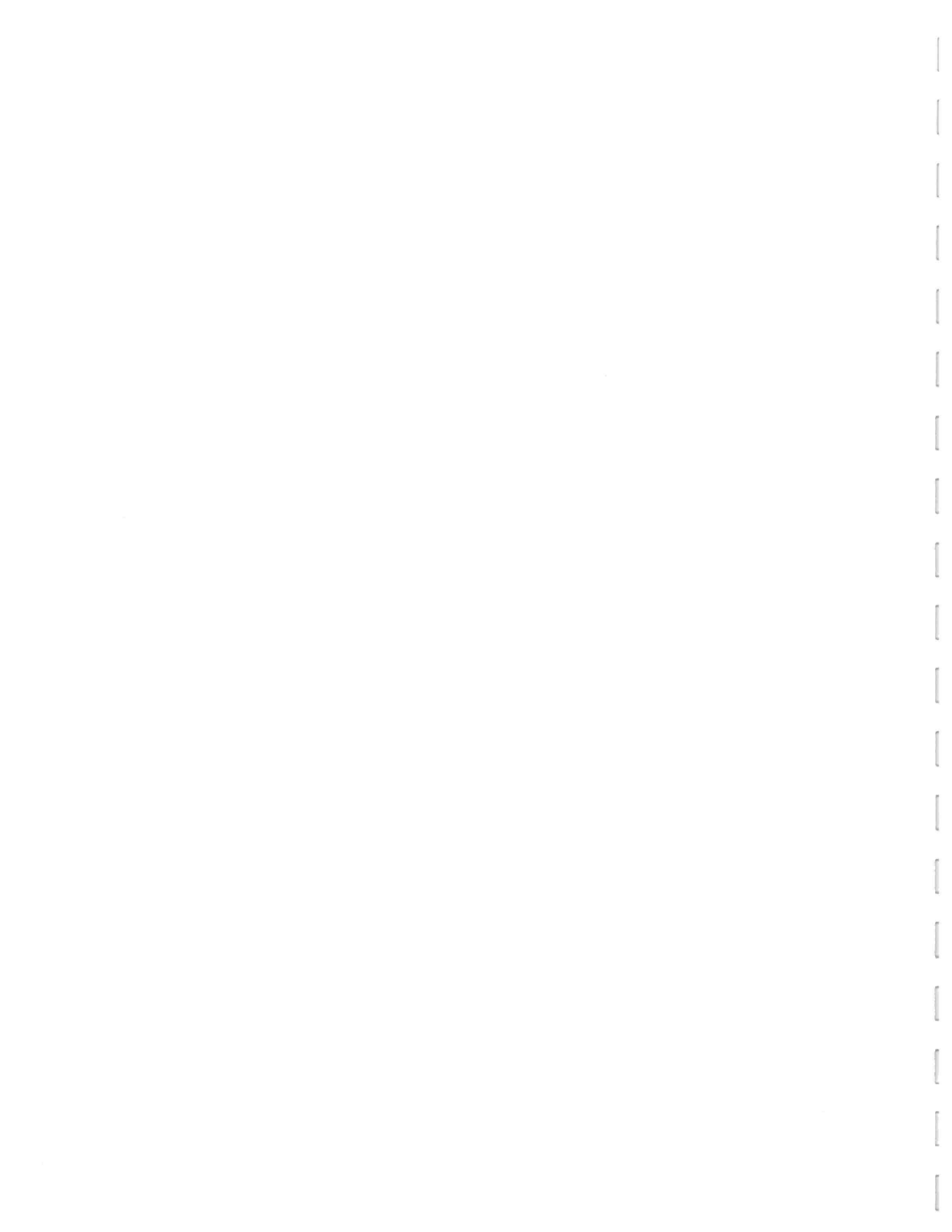
**PROFESSIONAL ENGINEER**

R. FISHER  
 No. 59687  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

NOV 09 2005

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), TPI TRUSS PLATE CONNECTIONS IN CONNECTION WITH THIS DESIGN. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS CHORDS AND BRACING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS CHORDS AND BRACING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS CHORDS AND BRACING. THE DESIGNER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS CHORDS AND BRACING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-11/5/3) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (\*) SHALL BE PER AMBEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

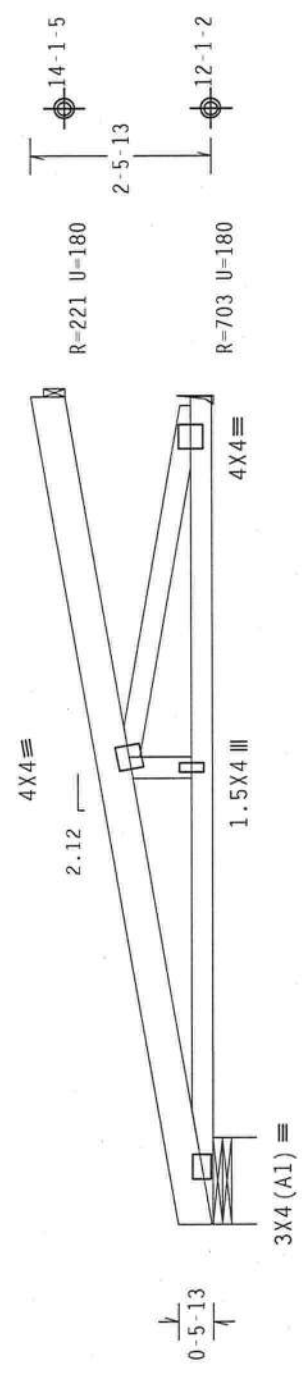
Hipjack supports 8-0-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



11-3-12 Over 3 Supports  
R=471 U=180 W=14.142"

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

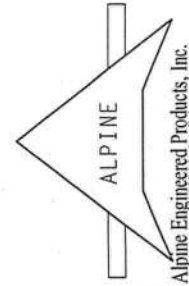
FL/-/3/-/-R/- Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487--	35471
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313099
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEON-	126599	
DUR.FAC.	1.25	JREF-	ISS0487_Z01	
SPACING	16.0"			



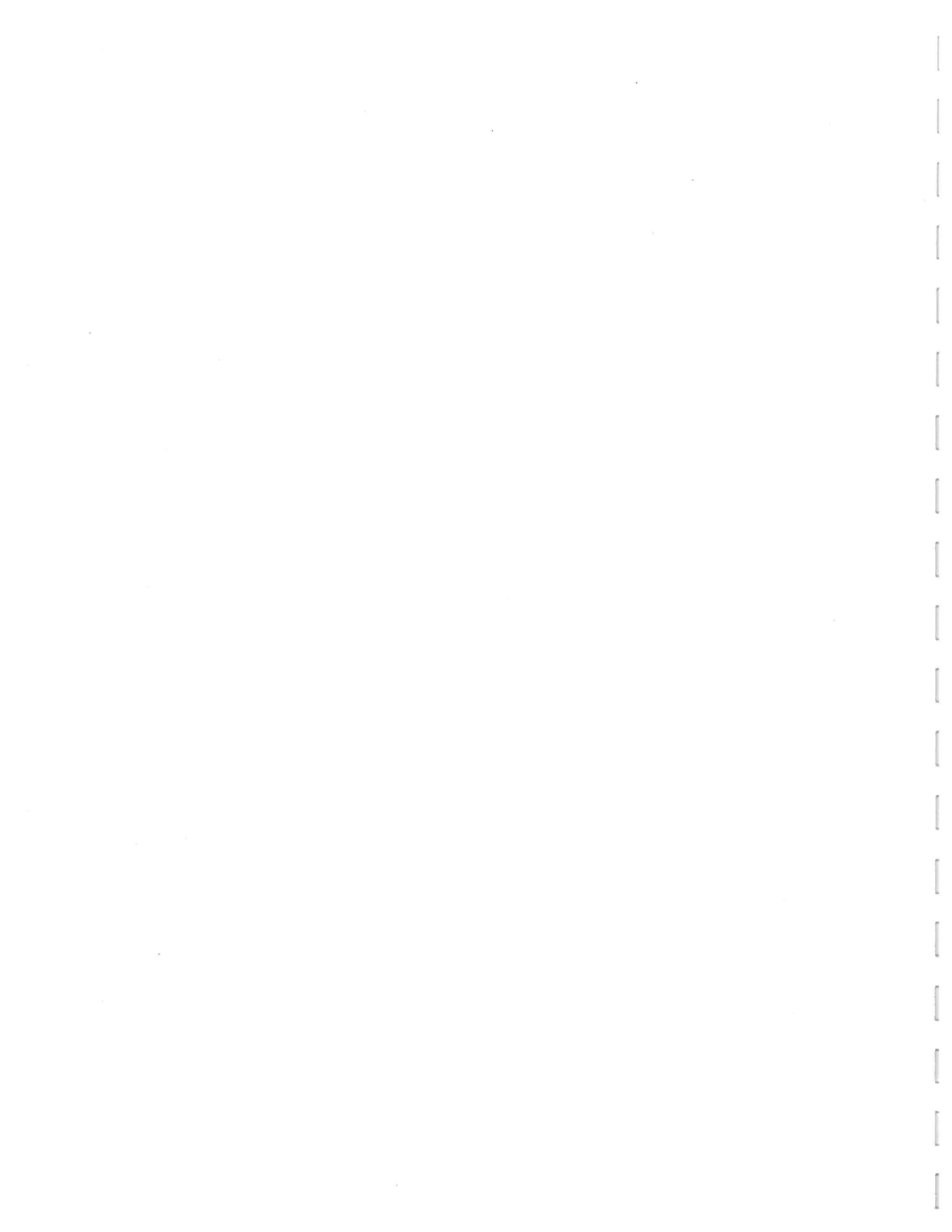
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE FLORIDA DEPARTMENT OF CONSTRUCTION, 6300 ENTERPRISE BL, MADISON FL 32719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/A) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.  
Haines City, FL 33844  
1950 Mailey Drive  
FL Certificate of Authorization # 567

PLT TYP. Wave



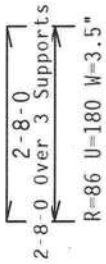
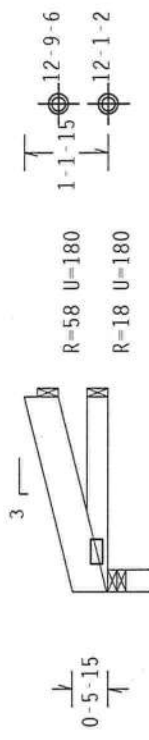
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 12.92 ft mean hgt, ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

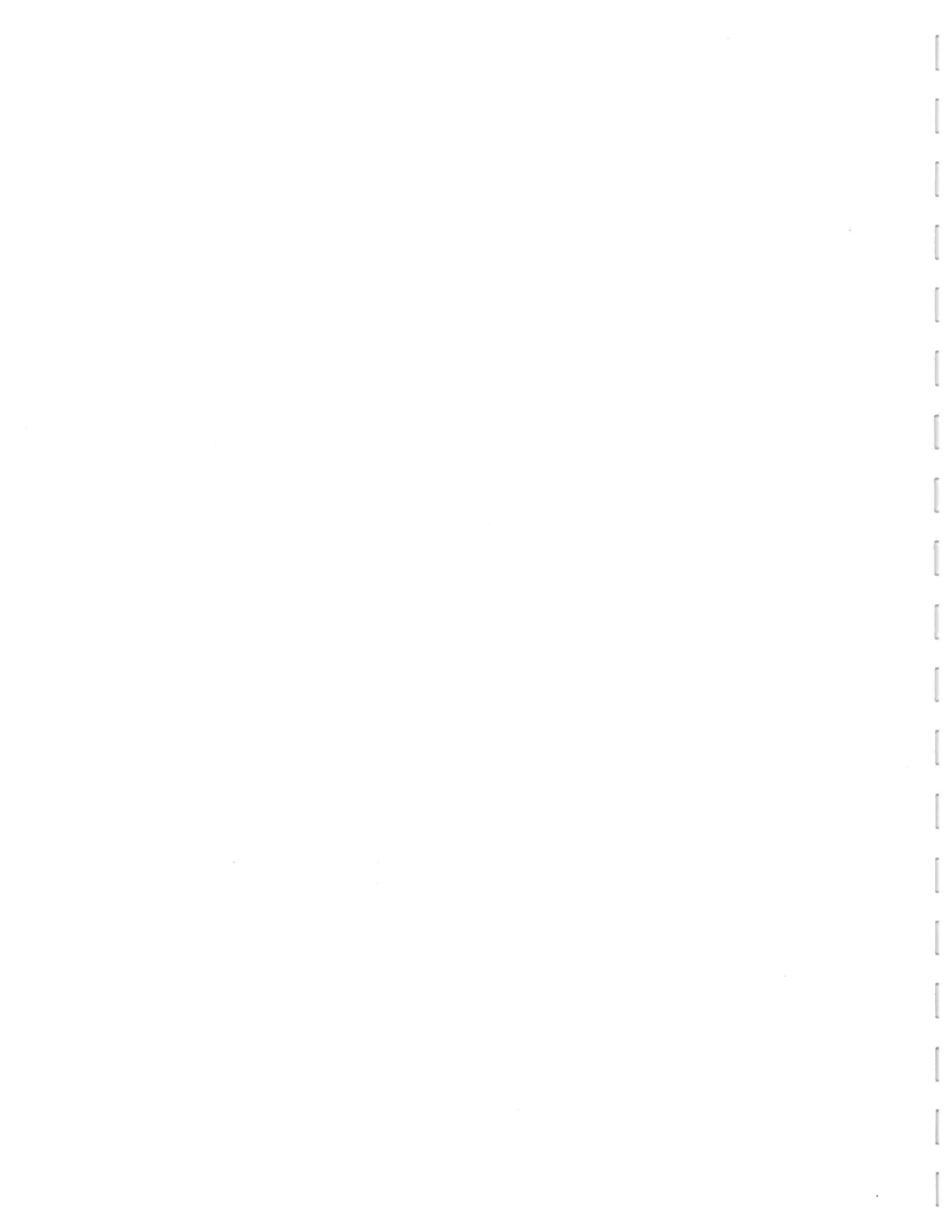
TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 -- 35472	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313022	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN - 126561	
SPACING	16.0"	JREF - 1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 HAZEN RD, WILSON, NJ 07097) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6000 ENTERPRISE LN, WASHINGTON, DC 20004) FOR ADDITIONAL INFORMATION. ALL TRUSS PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 70/30/16GA (#4 H/S/P) ASTM A563 GRADE 40/60 (#4 H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER A051/TPI 1 SEC. 2.

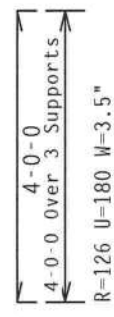
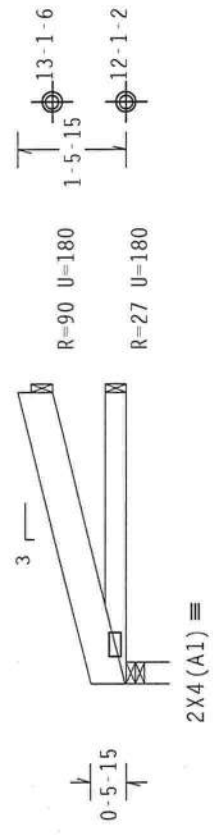
**ALPINE**  
 Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



110 mph wind, 13.09 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Trusses to be spaced at 16.0" OC maximum.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"); toe nailed at Bot chord.

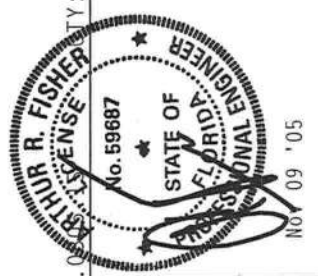


Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave

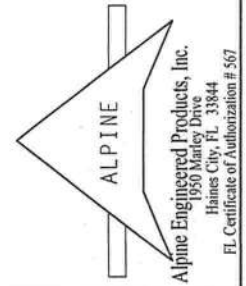
Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487--	35473
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313023
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	126565	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 983 DORRFIELD DR., SUITE 200, HOUSTON, TX 77057, AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6800 ENTERPRISE LN., WYOMING, WY 83094) FOR MORE INFORMATION. ALL TRUSS CHORDS AND BRACING MEMBERS SHALL BE PROPERLY ATTACHED TO THE TRUSS CHORDS AND BOTTOM CHORDS. TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W, H/5/16) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.





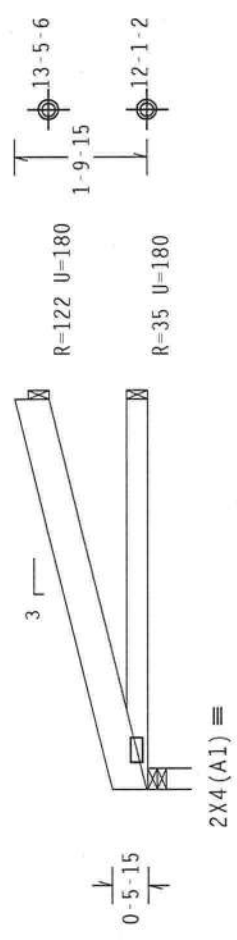
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense

Trusses to be spaced at 16.0" OC maximum.

Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 13.25 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

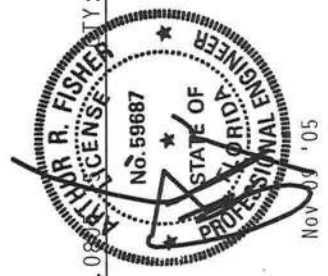


←-5-4-0 Over 3 Supports →  
 R=166 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

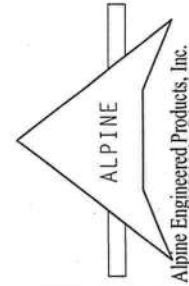
Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 - -	35474
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313100
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126569	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			

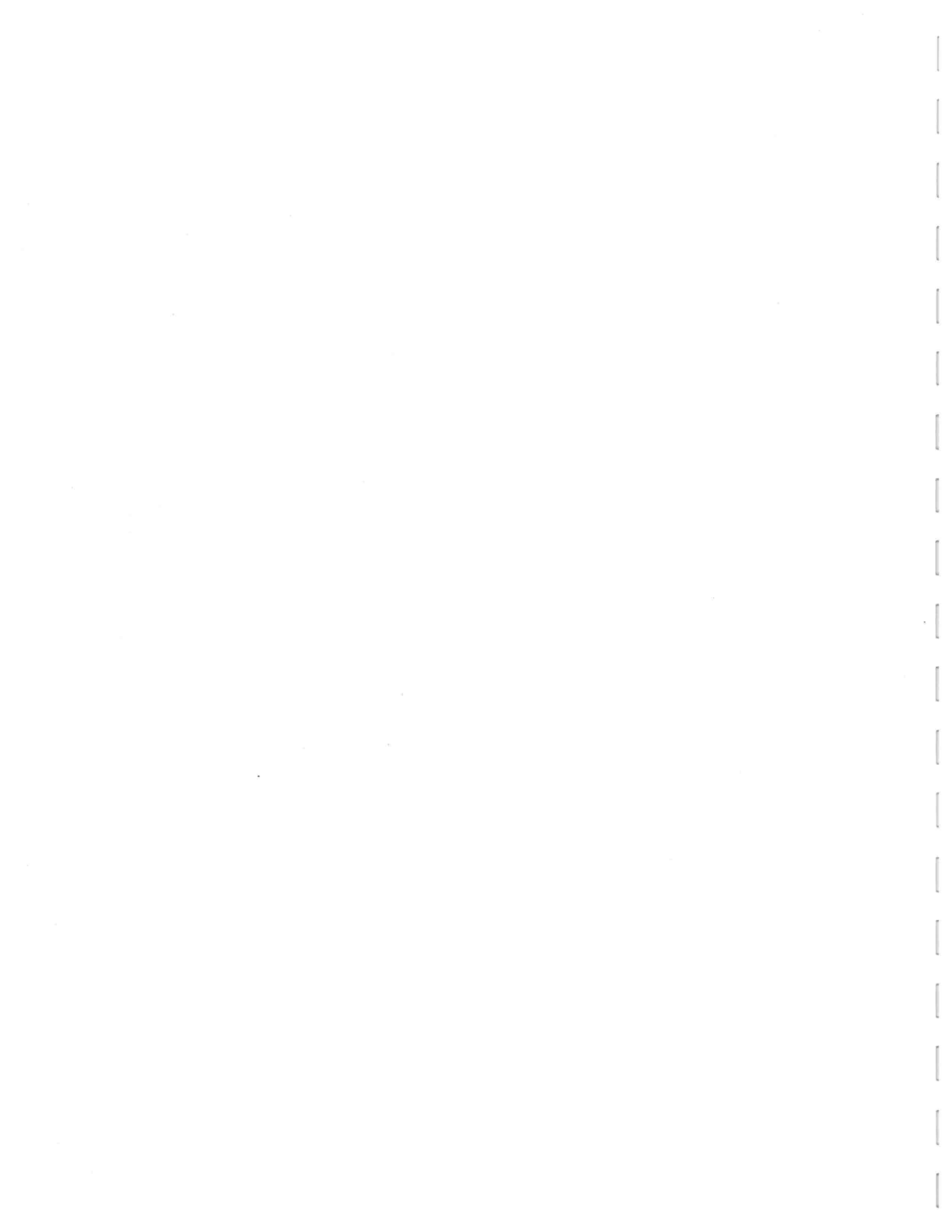


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 1000 W. 11TH AVE., SUITE 100, DENVER, CO 80202, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/19/16GA (W-11/5/3) ASTM A653 GRADE 40/60 (H, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



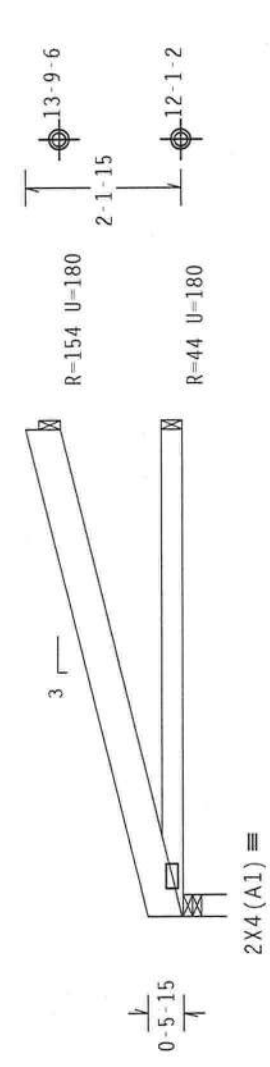
Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 1950 Marley Drive  
 FL Certificate of Authorization # 567



110 mph wind, 13.42 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

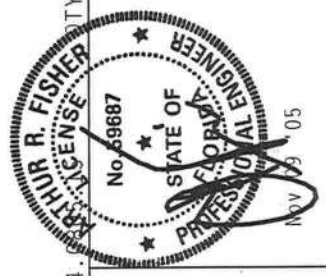
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Trusses to be spaced at 16.0" OC maximum.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



6-8-0 Over 3 Supports  
 R=207 U=180 W=3.5"

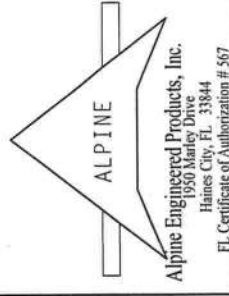
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

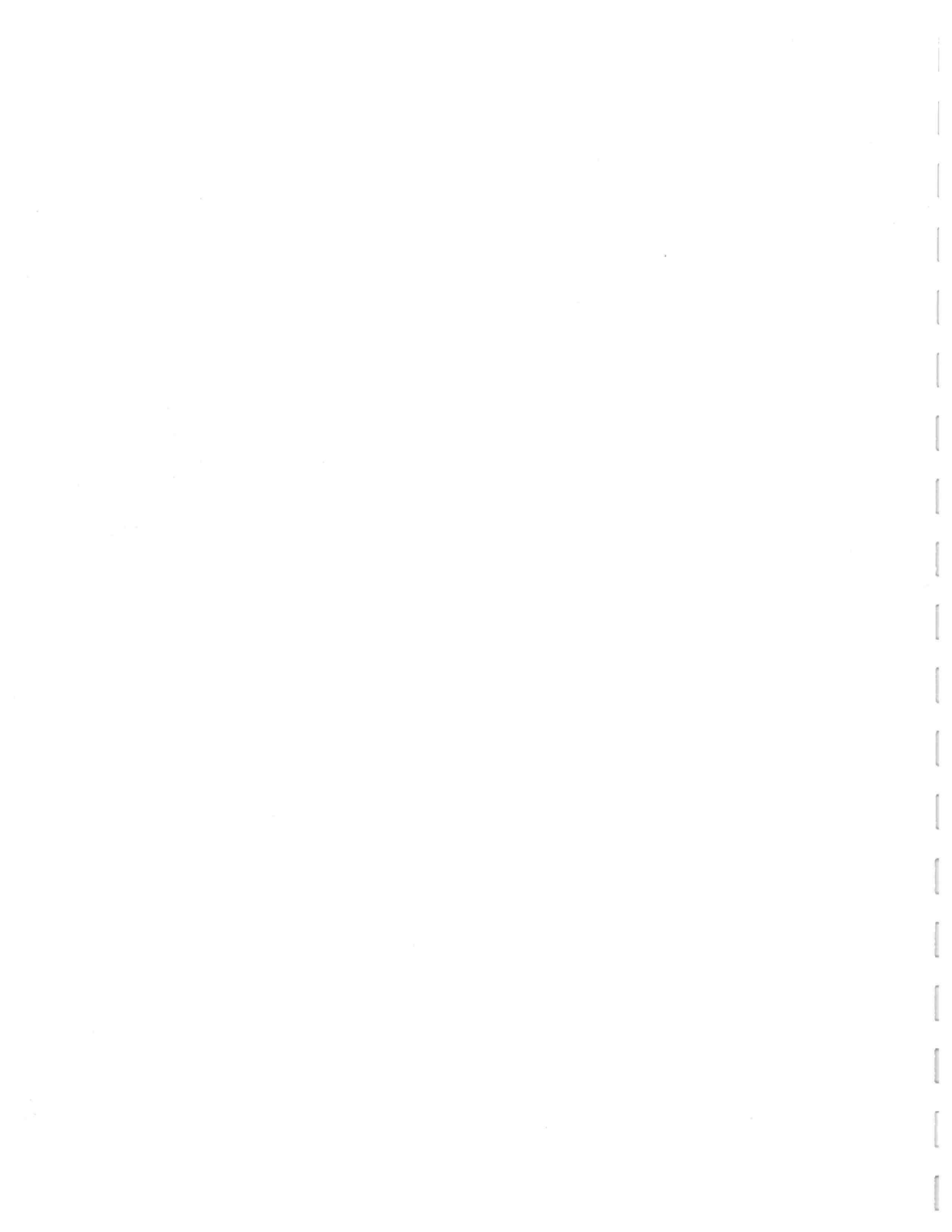
TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0 PSF		REF R487 - - 35475
BC DL	10.0 PSF		DATE 11/09/05
BC LL	0.0 PSF		DRW HCUSR487 05313101
TOT.LD.	45.0 PSF		HC-ENG JB/AF
DUR.FAC.	1.25		SEON- 126573
SPACING	16.0"		JREF- ISS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 903 D-ONOFREO DR., SUITE 200, MADISON, WI 53719) AND NICK (WOOD TRUSS COUNCIL OF AMERICA, 608 E. WISCONSIN, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INSTALLATION. ALL TRUSSES MUST BE PROPERLY BRACED, RIGID CEILING. ALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLECTION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/H/S/K) ASTM A653 GRADE 40/60 (K, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16DA-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.





110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 2-8-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Trusses to be spaced at 16.0" OC maximum.  
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 --	35476
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313102
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126986
SPACING	16.0"	JREF-	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR THE DESIGN AND MANUFACTURING OF THE TRUSS. THE USER SHALL BE RESPONSIBLE FOR THE TRUSS IN CONFORMANCE WITH TPI DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (H/H/S/S) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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1950 Kingsley Drive  
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FL Certificate of Authorization # 567



110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

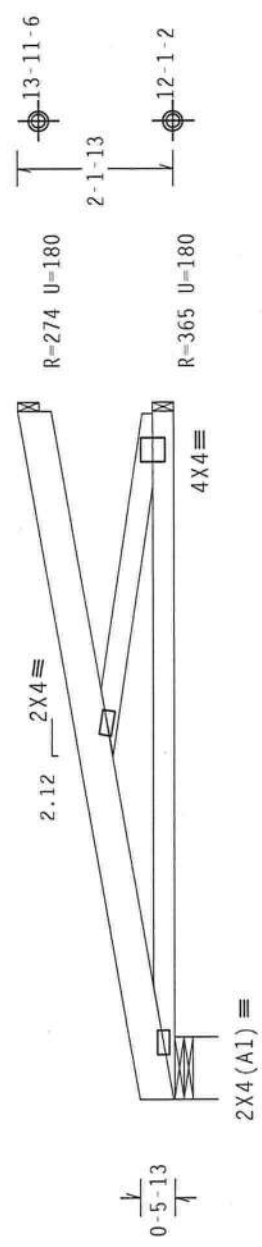
Hipjack supports 6-8-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=327 U=180 W=10.253"  $\longleftrightarrow$  9-5-2 Over 3 Supports  $\longleftrightarrow$

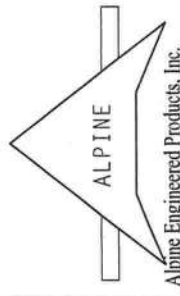
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.00

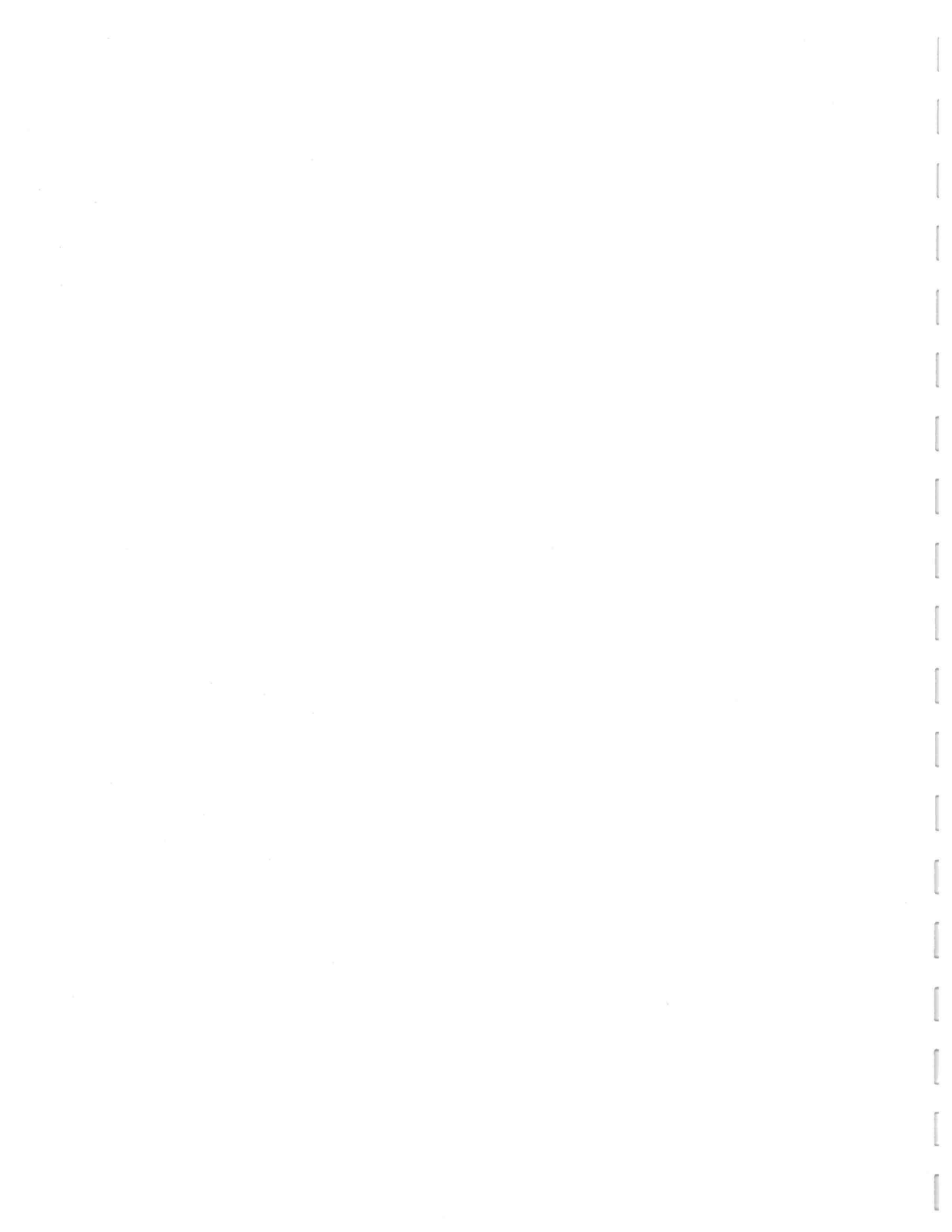


TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - - 35477	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313103	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEON - 126470	
SPACING	16.0"	JREF - 1SS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO FABRICATING AND BRACING. BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/S/P) ASTM A653 GRADE 40/50 (H, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTOR OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





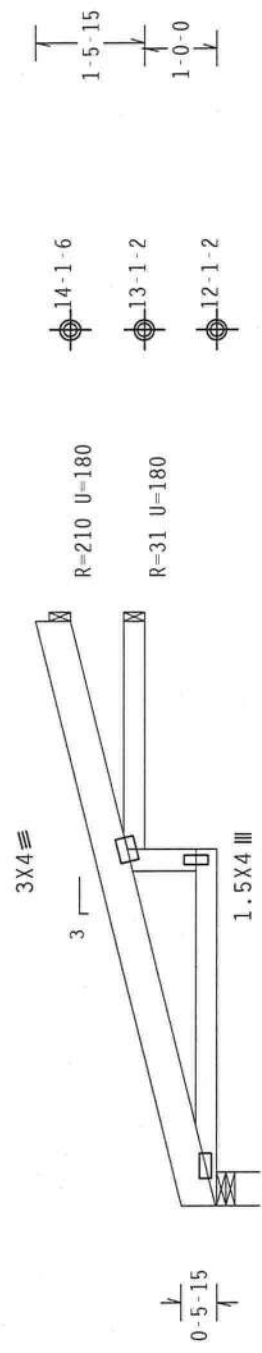
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"); toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"); toe nailed at Bot chord.

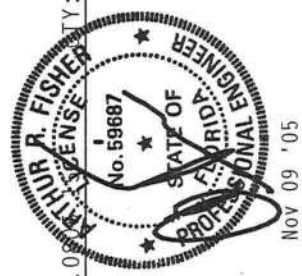
110 mph wind, 13.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase  
 factor for dead load is 1.50.



4-10-8  
 3-1-8  
 8-0-0 Over 3 Supports  
 R=244 U=180 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TC LL	20.0 PSF	REF	R487 --	35478
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313024
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	126586	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE DESIGN PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES WITH APPLICABLE PROVISIONS OF AISC 360 (4.8/5.3) GRY. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

**Alpine Engineered Products, Inc.**  
 1950 Manley Drive  
 Gaines City, FL 32644  
 FL Certificate of Authorization # 567



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

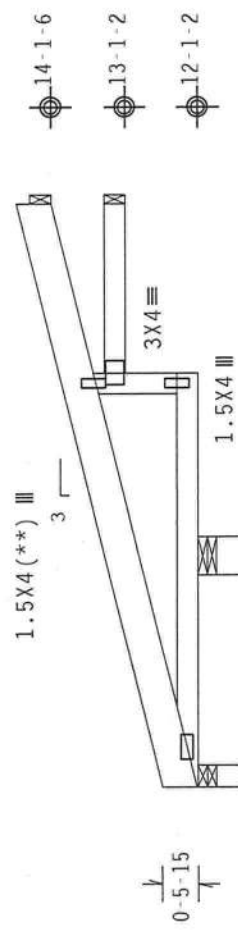
110 mph wind, 13.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

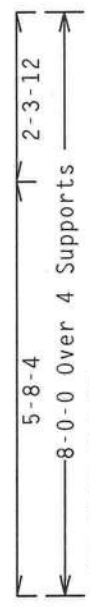
(\*\*) Plate relocated as shown.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

R=133 U=180



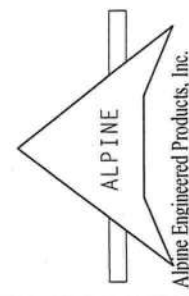
R=19 U=180



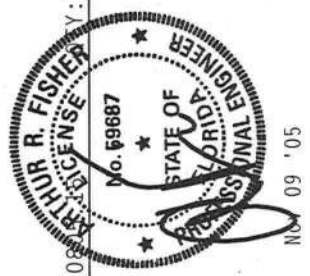
R=215 U=180 W=6.278"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

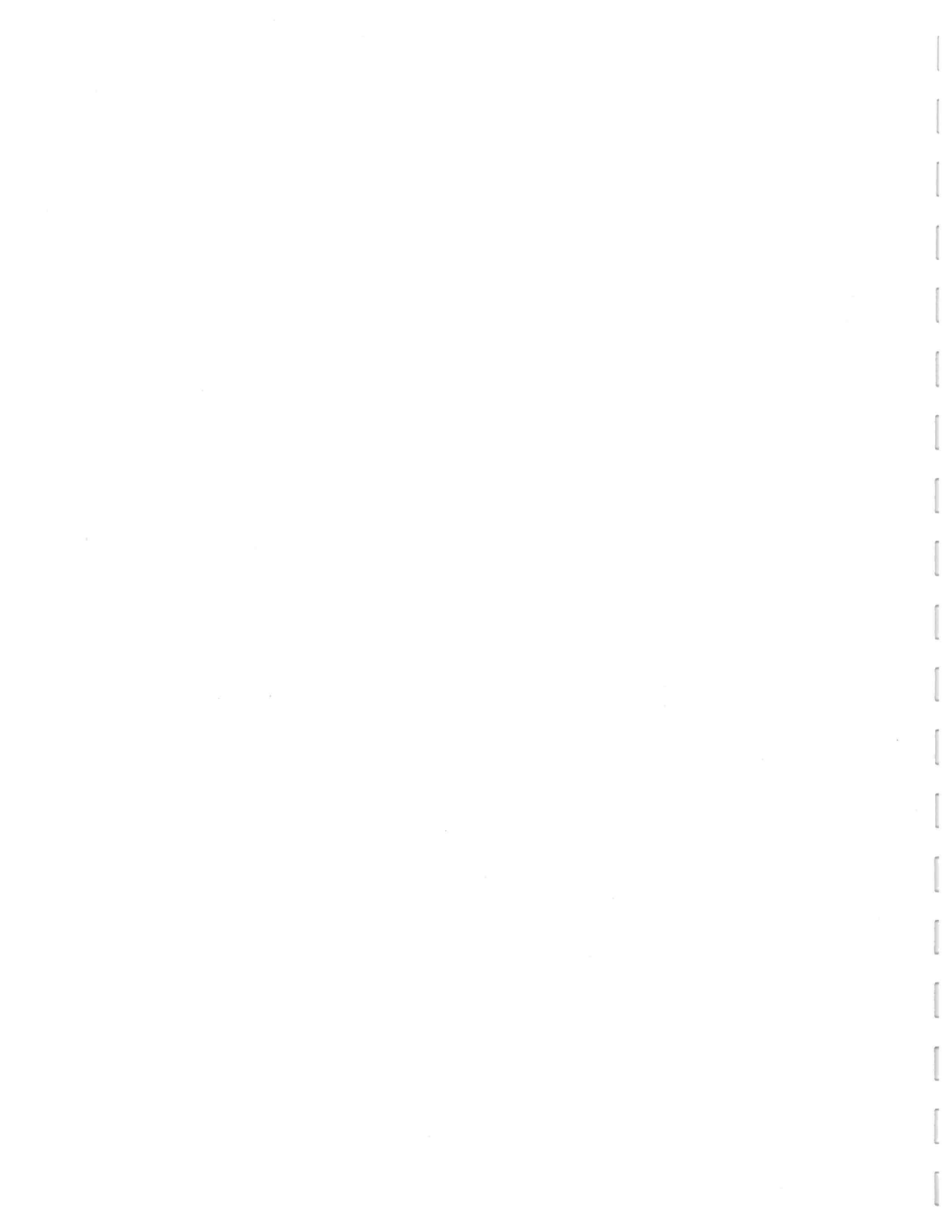


Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 1950 Mealey Drive  
 FL Certificate of Authorization # 567



Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487-- 35479
TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313104
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	45.0 PSF	SEQN-	126591
DUR.FAC.	1.25		
SPACING	16.0"	JREF-	1SS0487_Z01

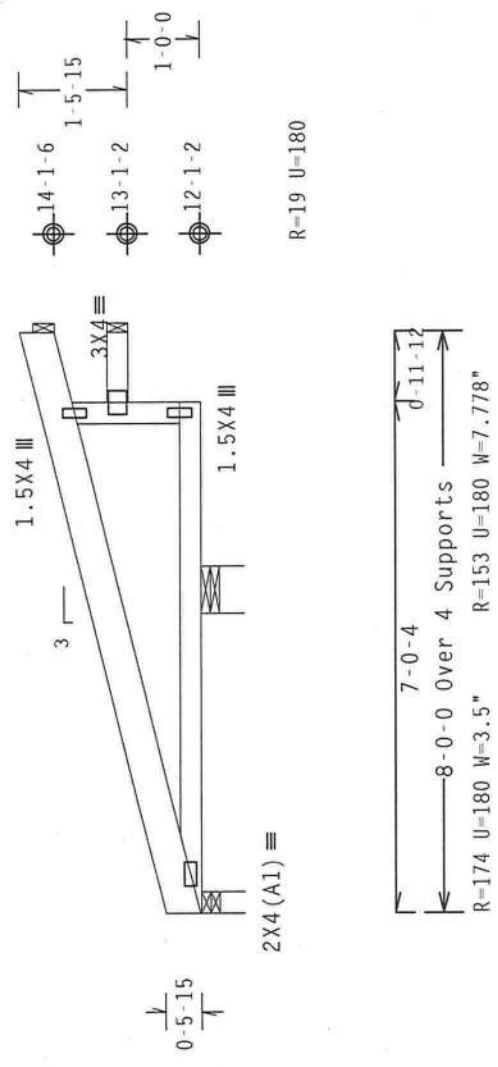


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

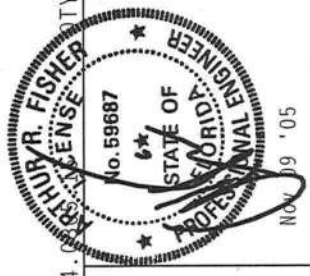
110 mph wind, 13.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind IC DL-5.0 psf, wind BC DL-5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

Scale = .375" / Ft.

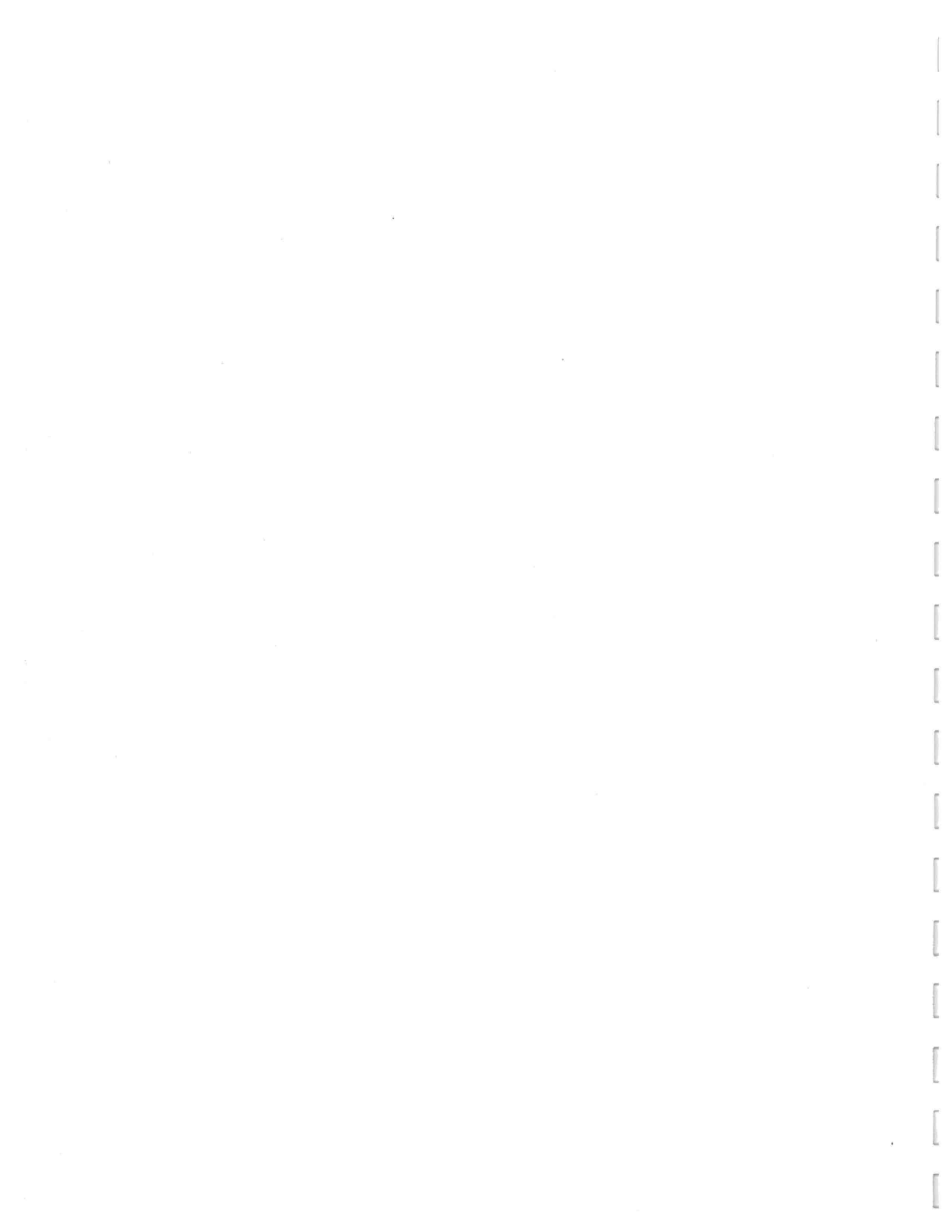


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH THE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/3/5/3) ASTM A653 GRADE 40/60 (N, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ARNE A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
REF R487 - -	20.0 PSF	35480
DATE 11/09/05	15.0 PSF	
DRW HCUSR487 05313025	10.0 PSF	
HC-ENG JB/AF	0.0 PSF	*
SEQN- 126595	TOT.LD. 45.0 PSF	
	DUR.FAC. 1.25	
	SPACING 16.0"	JREF- 1SS0487_Z01



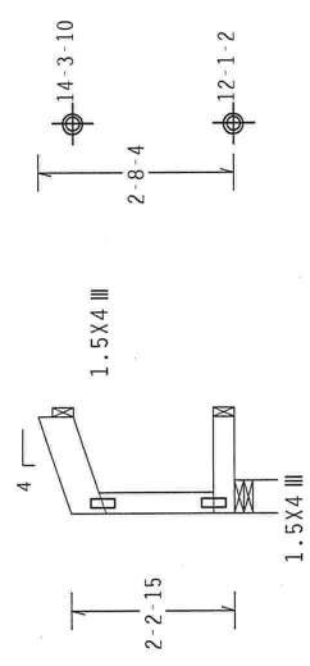
110 mph wind, 14.56 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

R=32 U=180

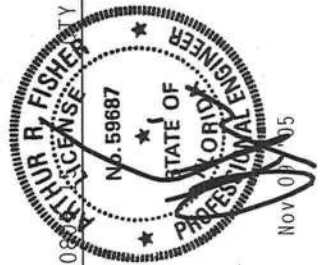


R=9 U=180

1-4-0 Over 3 Supports  
 R=41 U=180 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

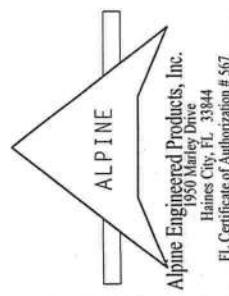
Scale = .375" / Ft.

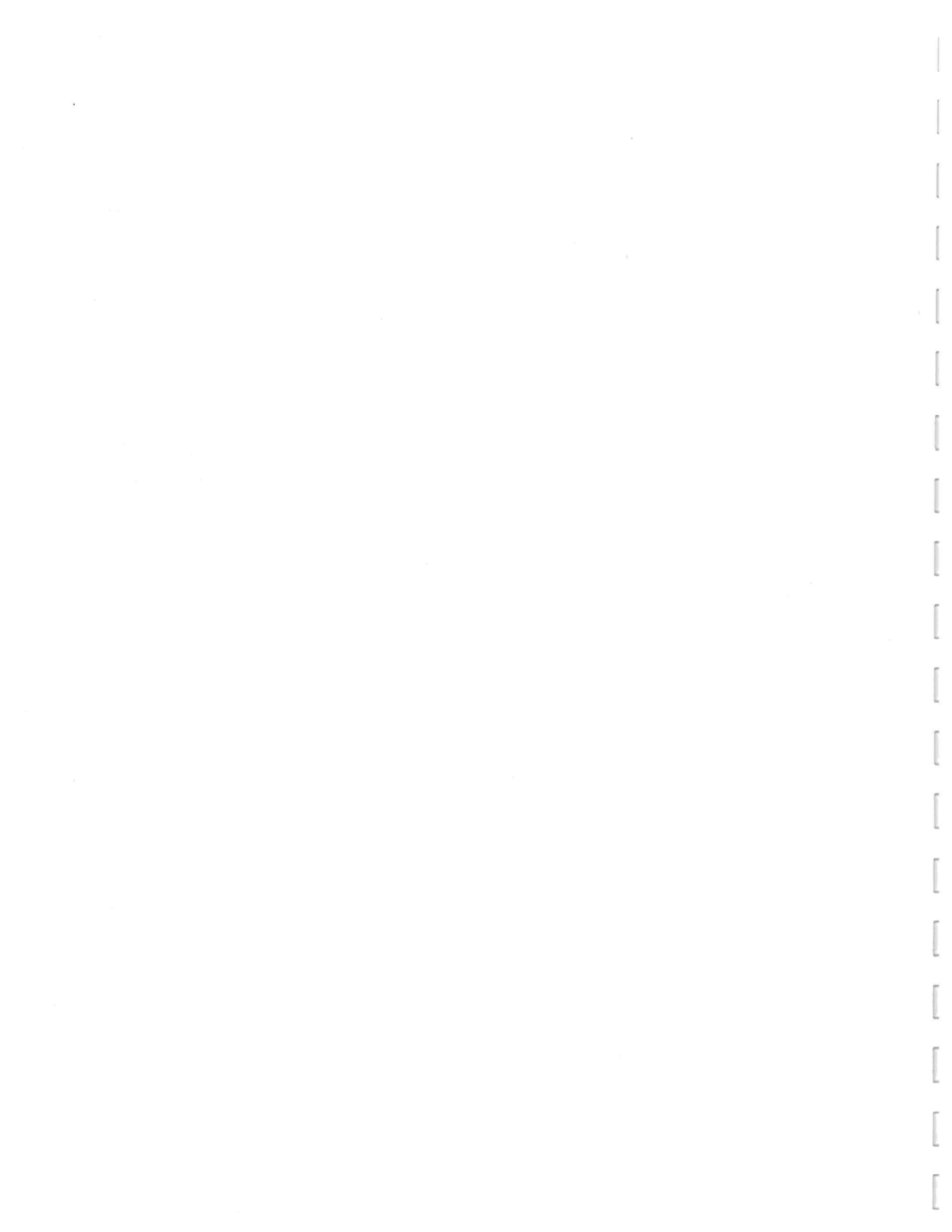


TC LL	20.0 PSF	REF	R487-- 35481
TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313026
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	45.0 PSF	SEQN-	160820
DUR.FAC.	1.25	SPACING	16.0"
JREF-	1SS0487_Z01		

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ARX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1. SEC. 2.



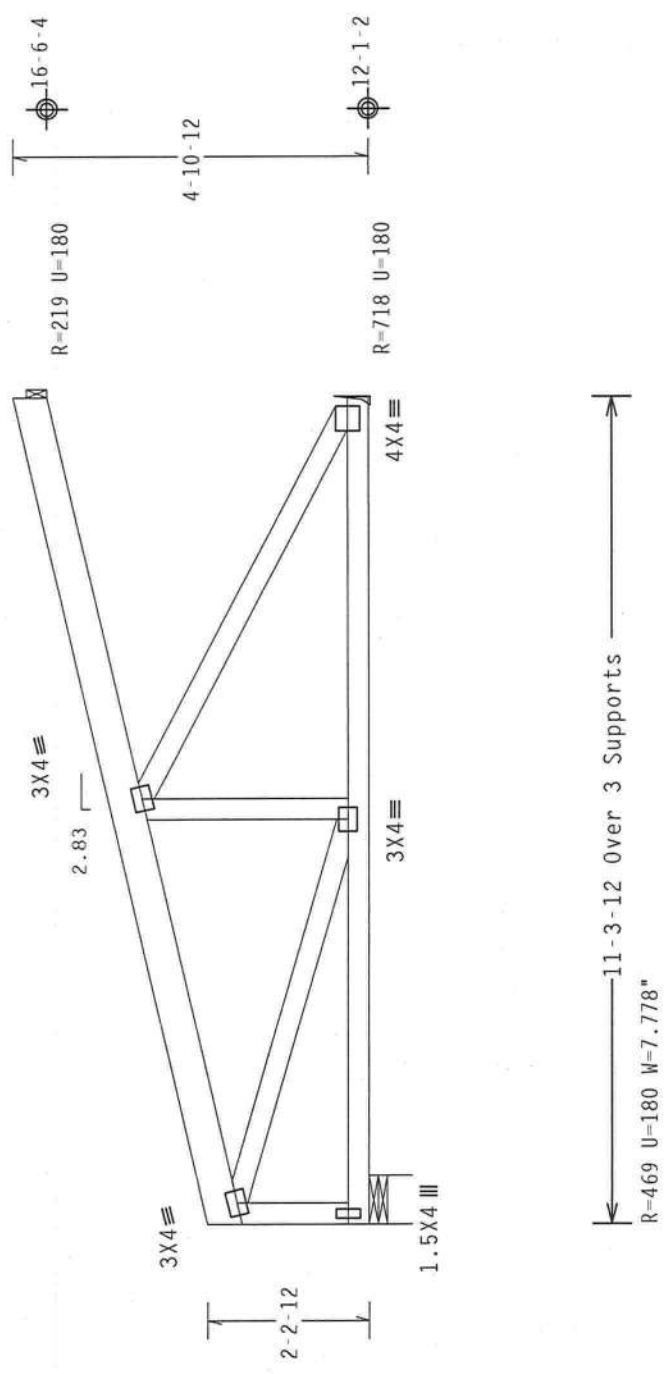


110 mph wind, 15.66 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

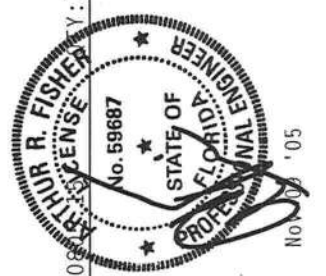
Left end vertical not exposed to wind pressure.  
 Hipjack supports 8-0-0 setback jacks with no webs.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - - 35482	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313105	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 160858	
SPACING	16.0"	JREF- 1SS0487_Z01	

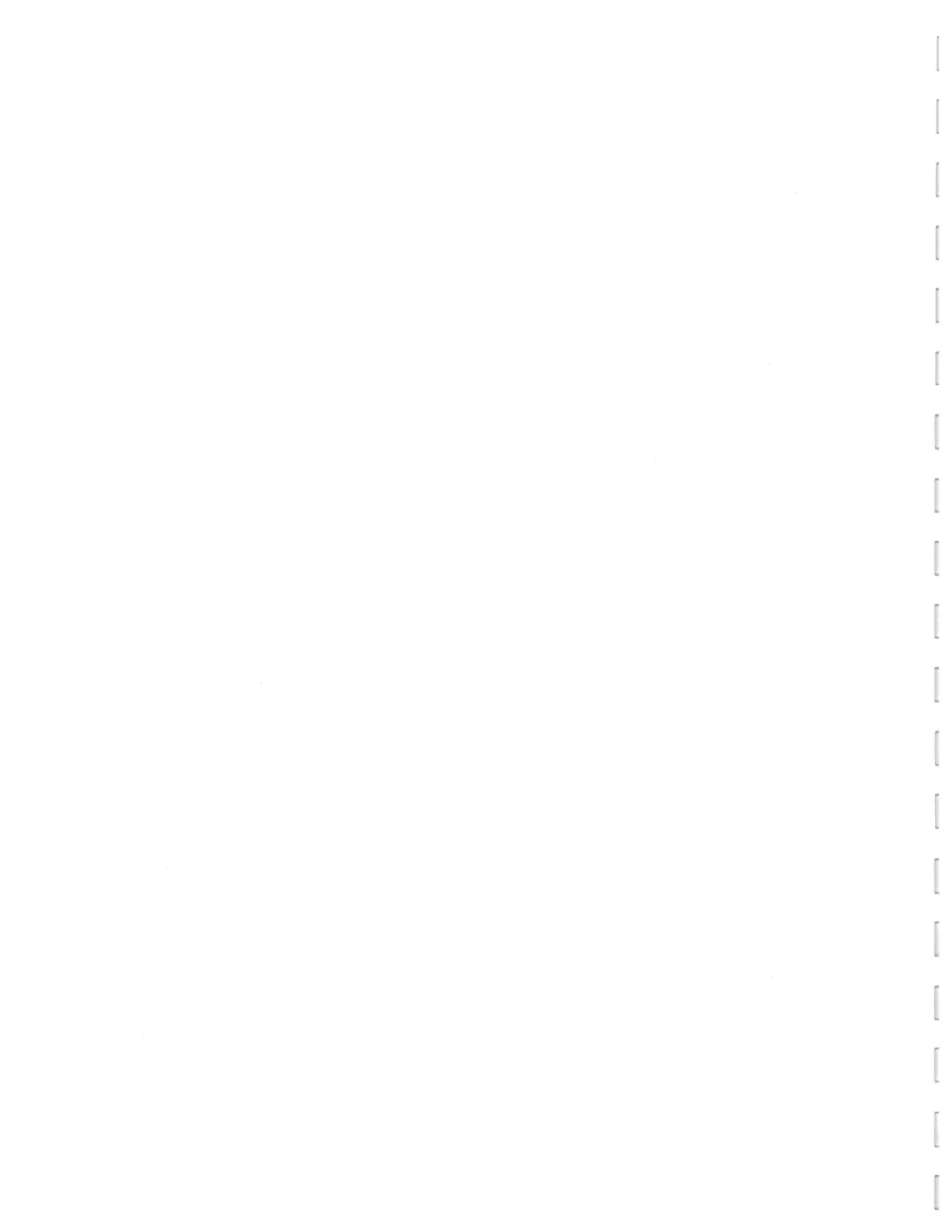


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 983 DUNFORD DR., SUITE 200, HOUSTON, TX 77059) AND WPCA (WOOD TRUSS CONSTRUCTION, 10000 WOODBRIDGE BLVD., SUITE 100, WOODBRIDGE, VA 22191) FOR THE PROPER CONSTRUCTION OF TRUSSES. ALL TRUSSES SHOWN ON THIS DRAWING SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFICIENCY FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N-11/5/3) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**Alpine Engineered Products, Inc.**  
 Haines City, FL 33844  
 1950 Moley Drive  
 FL Certificate of Authorization # 567

PLT TYP. Wave



110 mph wind, 14.78 ft mean hgt. ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.21" due to live load and 0.04" due to dead load.

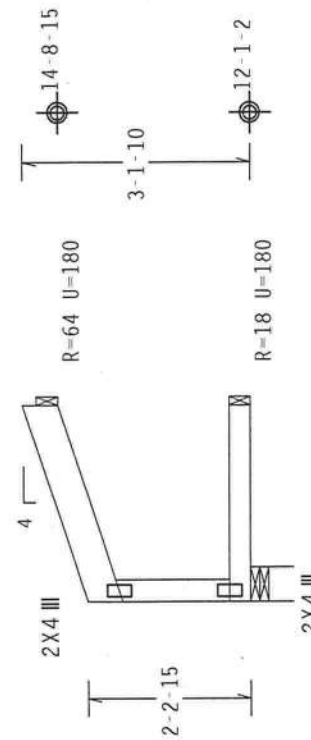
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

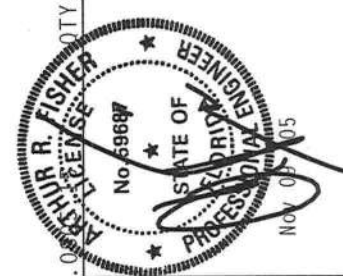
Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



2-8-0 Over 3 Supports  
R-81 U=180 W=5.5"

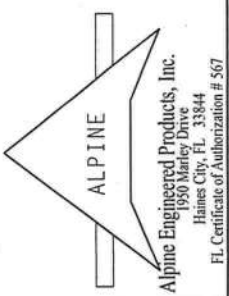
Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.0

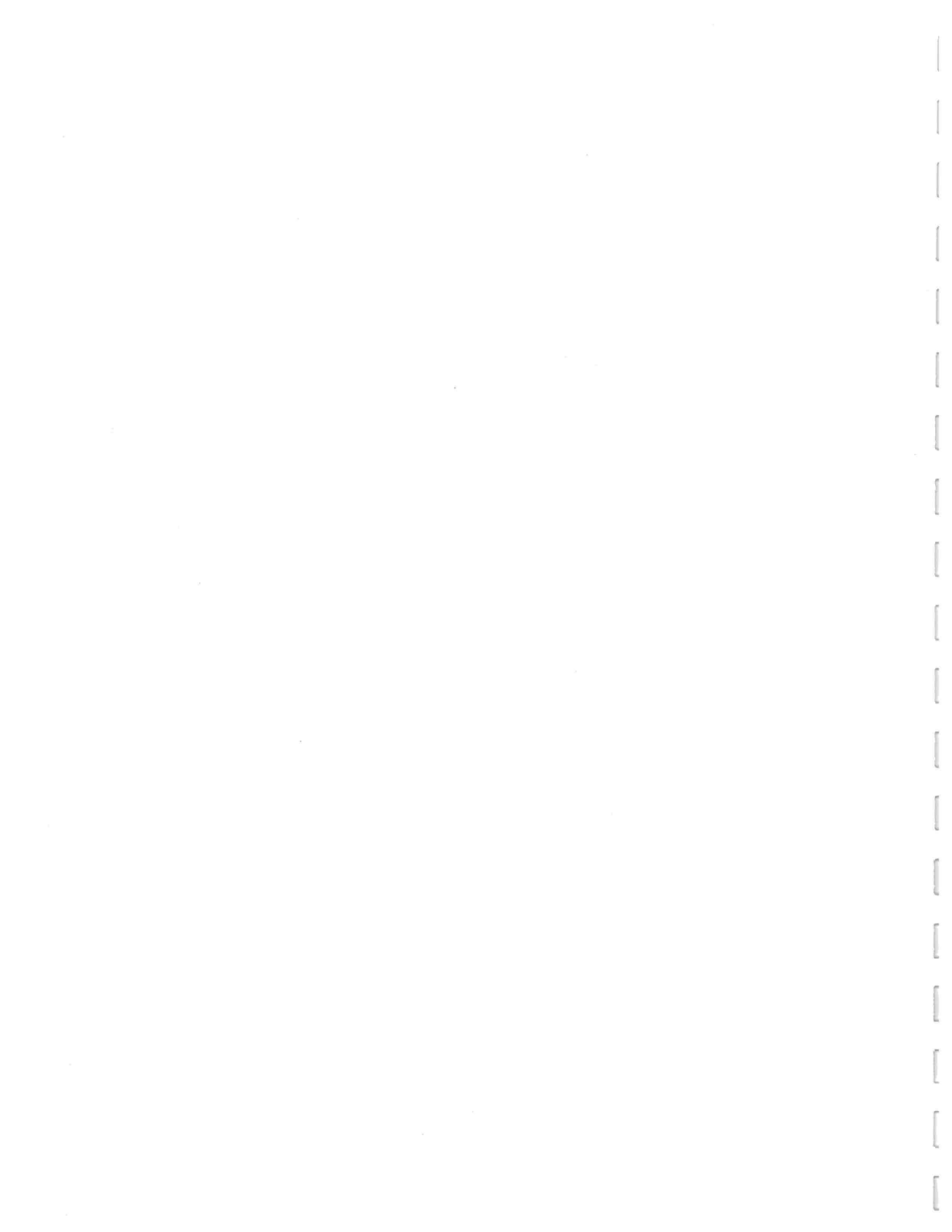


PLT TYP. Wave	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
REF	R487 - -	35483
DATE	11/09/05	
DRW	HCUSR487	05313027
HC-ENG	JB/AF	*
SEQN-	160825	
DUR.FAC.	1.25	
SPACING	16.0"	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (H. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16DA-2. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ABX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





110 mph wind, 15.01 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.29" due to live load and 0.06" due to dead load.

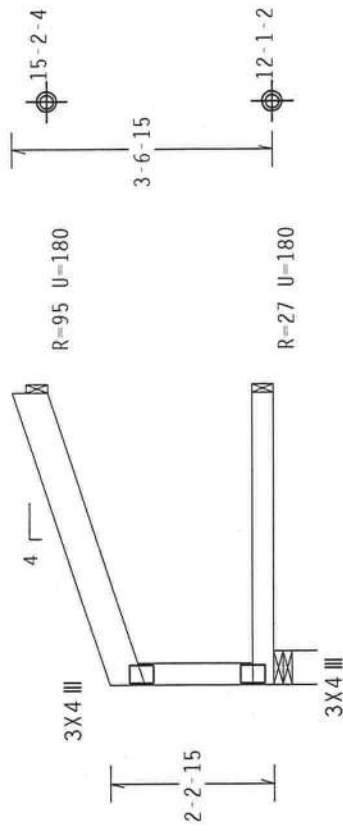
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



14-0-0 Over 3 Supports  
R=122 U=180 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

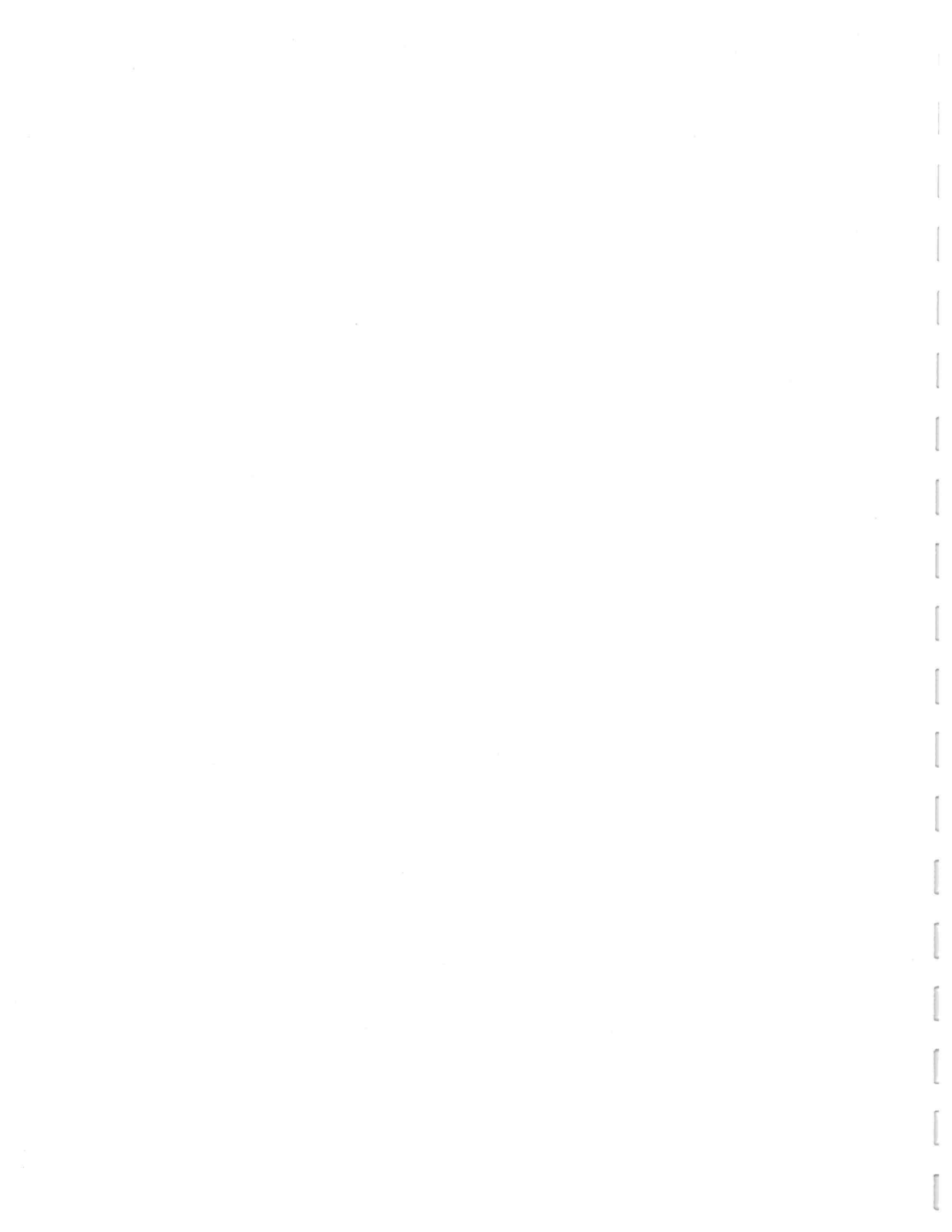


TY:2	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 - - 35484
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313041
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEQN- 160829
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 993 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 8300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. THE CONTRACTOR SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/S) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**ALPINE**  
 Alpine Engineered Products, Inc.  
 1930 Highway 14  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

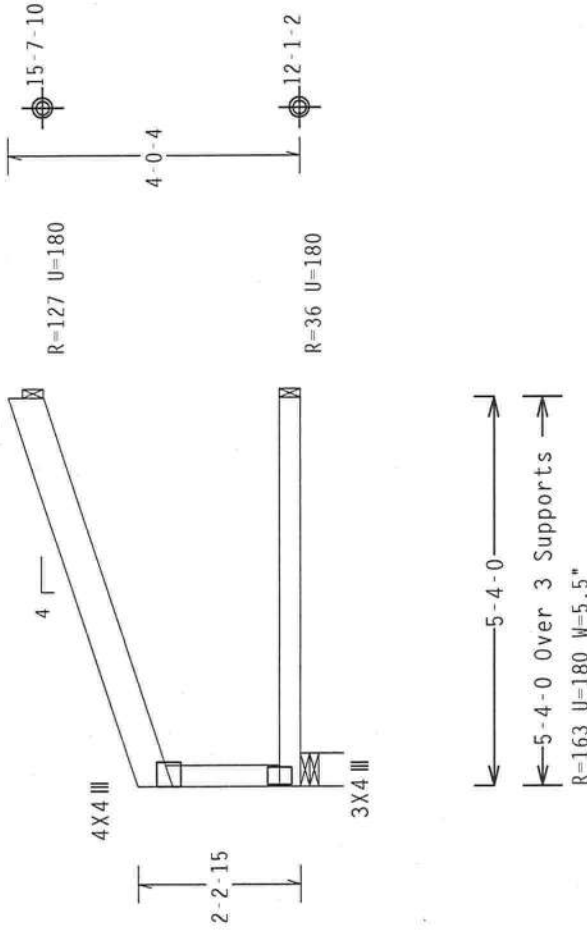
Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

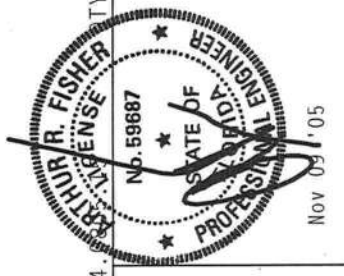
Calculated horizontal deflection is 0.36" due to live load and 0.07" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



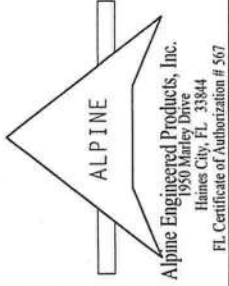
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

TY: 2	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 - 35485
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313042
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEQN- 160845
DUR.FAC.	1.25	JREF- 1SS0487_Z01
SPACING	16.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 983 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE DESIGNING OF THIS TRUSS DESIGN PROGRAMS WITH APPLICABLE PROVISIONS OF AIAA, NATIONAL DESIGN SPEC. BY AIAA) AND TPI (ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (W-3/5/8) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #2 Dense

110 mph wind, 15.45 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

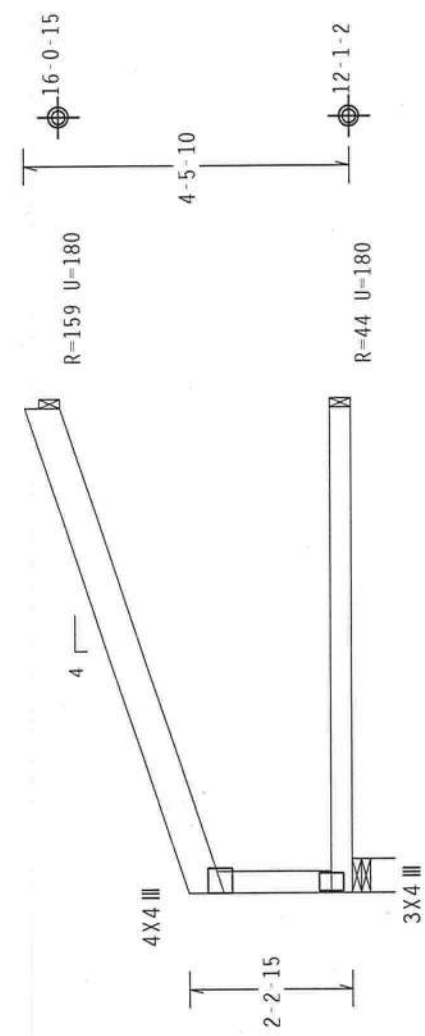
Calculated horizontal deflection is 0.35" due to live load and 0.08" due to dead load.

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Trusses to be spaced at 16.0" OC maximum.

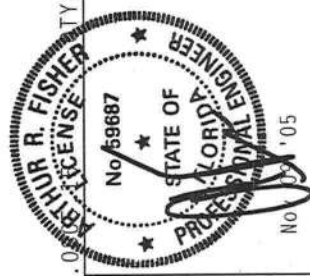
Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



6-8-0 Over 3 Supports  
 R=204 U=180 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TY: 2	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 - 35486
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313043
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEON- 160840
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01

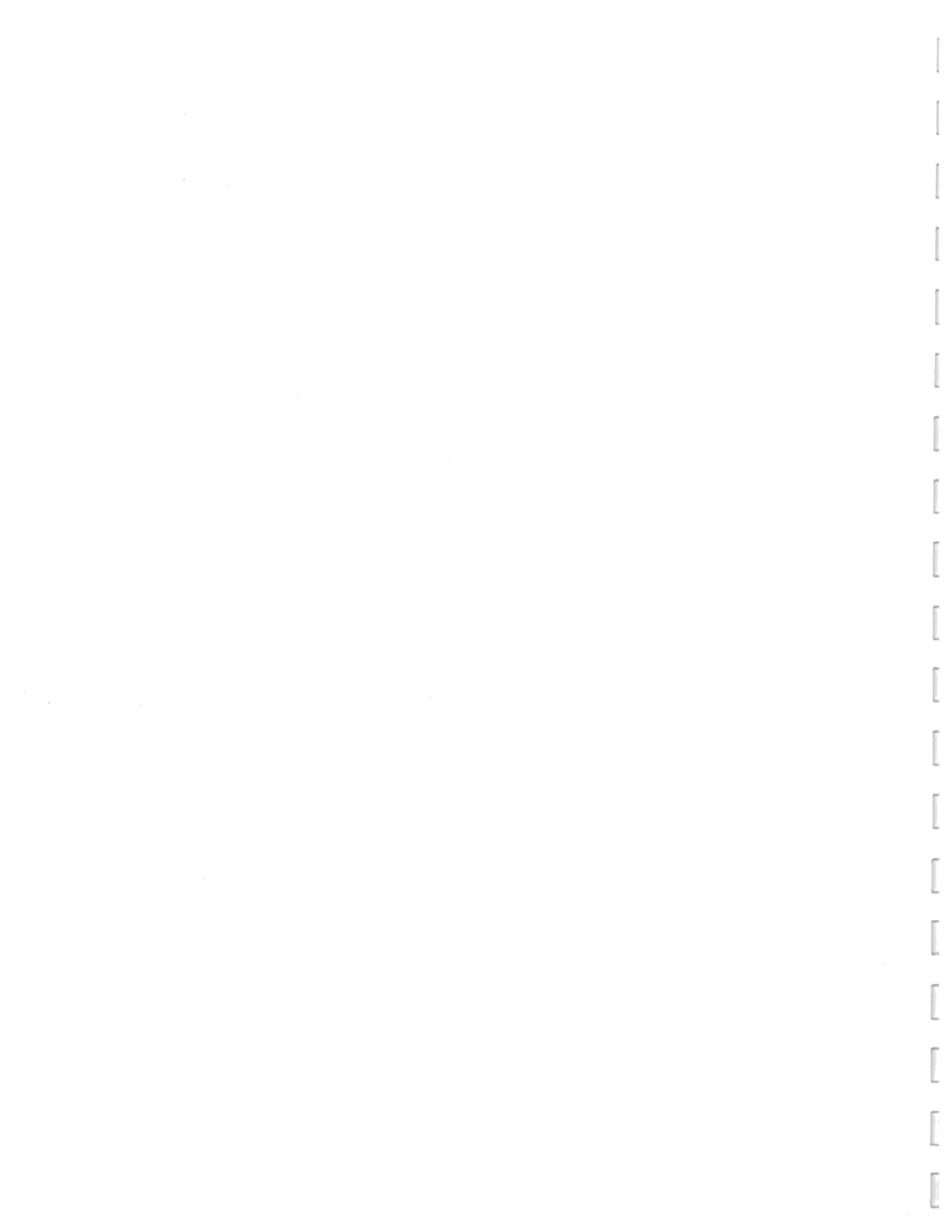
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NTECA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FABRICATION, BRACING, OR TRUSS IN CONFORMANCE WITH THE PROVISIONS OF THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE TRUSS IN CONFORMANCE WITH THE PROVISIONS OF THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (A/B/S/E) ASTM A653 GRADE 40/60 (K, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #2 Dense

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

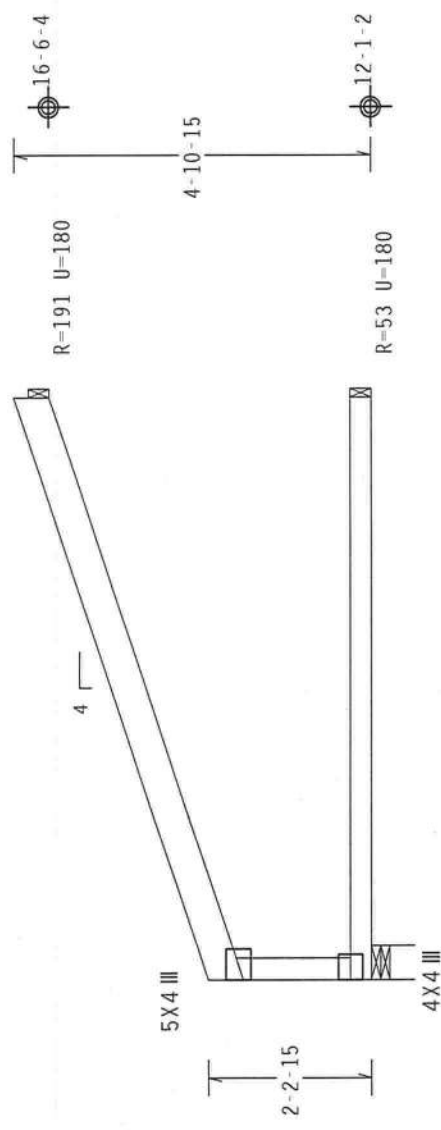
Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails (0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.67 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.39" due to live load and 0.09" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



8'-0-0 Over 3 Supports  
 R=244 U=180 W=5.5"

Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL 20.0 PSF	REF R487 - - 35487
TC DL 15.0 PSF	DATE 11/09/05
BC DL 10.0 PSF	DRW HCUSR487 05313044
BC LL 0.0 PSF	HC-ENG JB/AF
TOT.LD. 45.0 PSF	SEQN- 160851
DUR.FAC. 1.25	
SPACING 16.0"	JREF- 1SS0487_201

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AS THE ENGINEERED TRUSS IS THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR, ANY FAILURE TO FOLLOW THE TRUSS IN CONFORMANCE WITH TPI FOR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/160A (W/H/S/E) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 1950 N.W. 10th St.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 507

PLT TYP. Wave



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

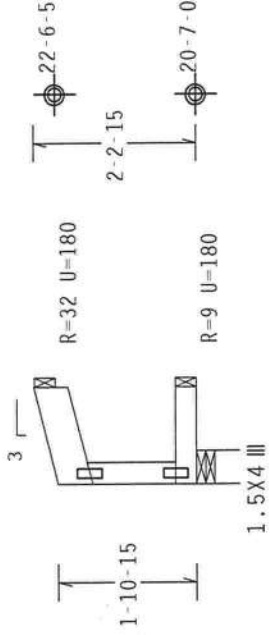
Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 22.66 ft mean htg, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

1.5X4 III



1-4-0 Over 3 Supports

R=40 U=180 W=5.375"

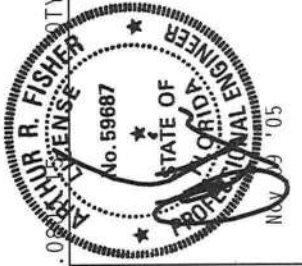
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

**ALPINE**  
 Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, BUILDING OR INSTALLING THE TRUSS IN ACCORDANCE WITH THE SPECIFICATIONS PROVIDED IN THIS DESIGN SPEC. BY ALPINE AND TPI OR OTHERWISE SHALL BE THE RESPONSIBILITY OF THE INSTALLER. UNLESS OTHERWISE INDICATED, ALL ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (2 JWS/8) ASTM A653 GRADE 40/60 (A, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AHEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487-- 35488	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313045	
TOT.LD.	45.0 PSF	HC-ENG JB/AF *	
DUR.FAC.	1.25	SEQN- 126952	
SPACING	16.0"	JREF- ISS0487_Z01	

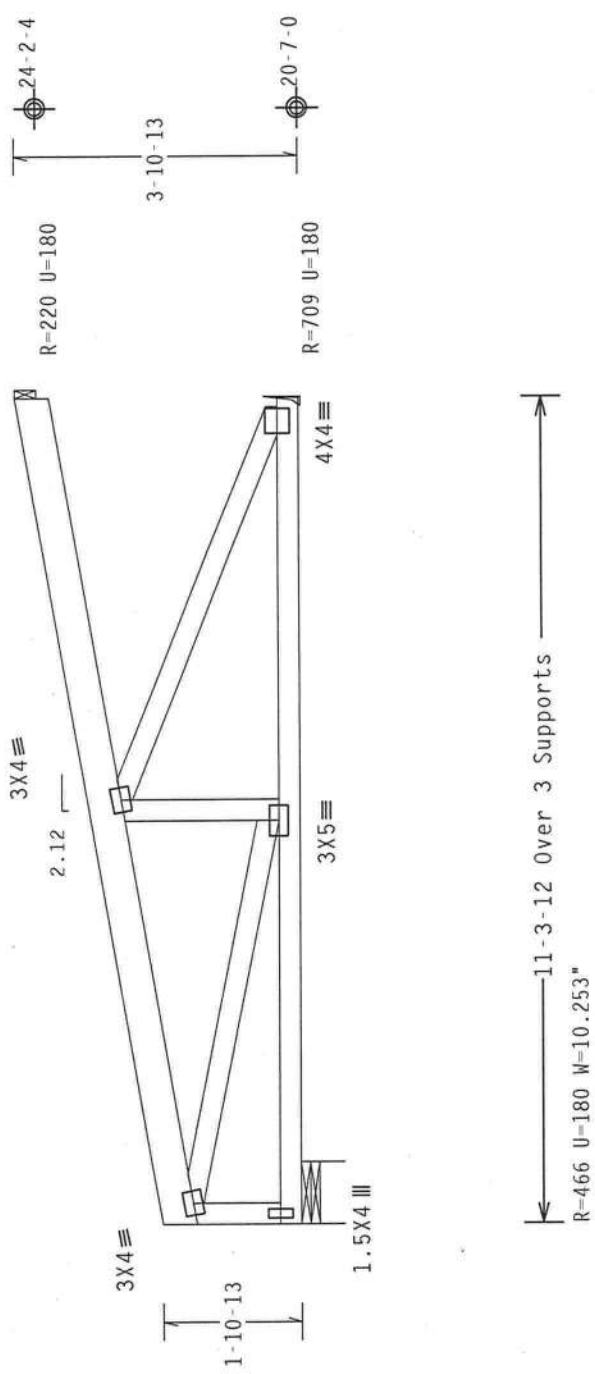


110 mph wind, 23.48 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 8-0-0 setback jacks with no webs.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.

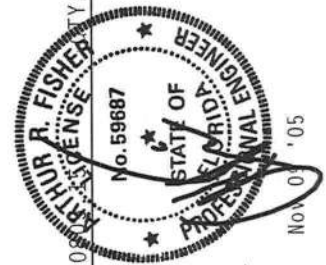
Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0.0.0.0

PLT TYP. Wave Scale = .375" / Ft.

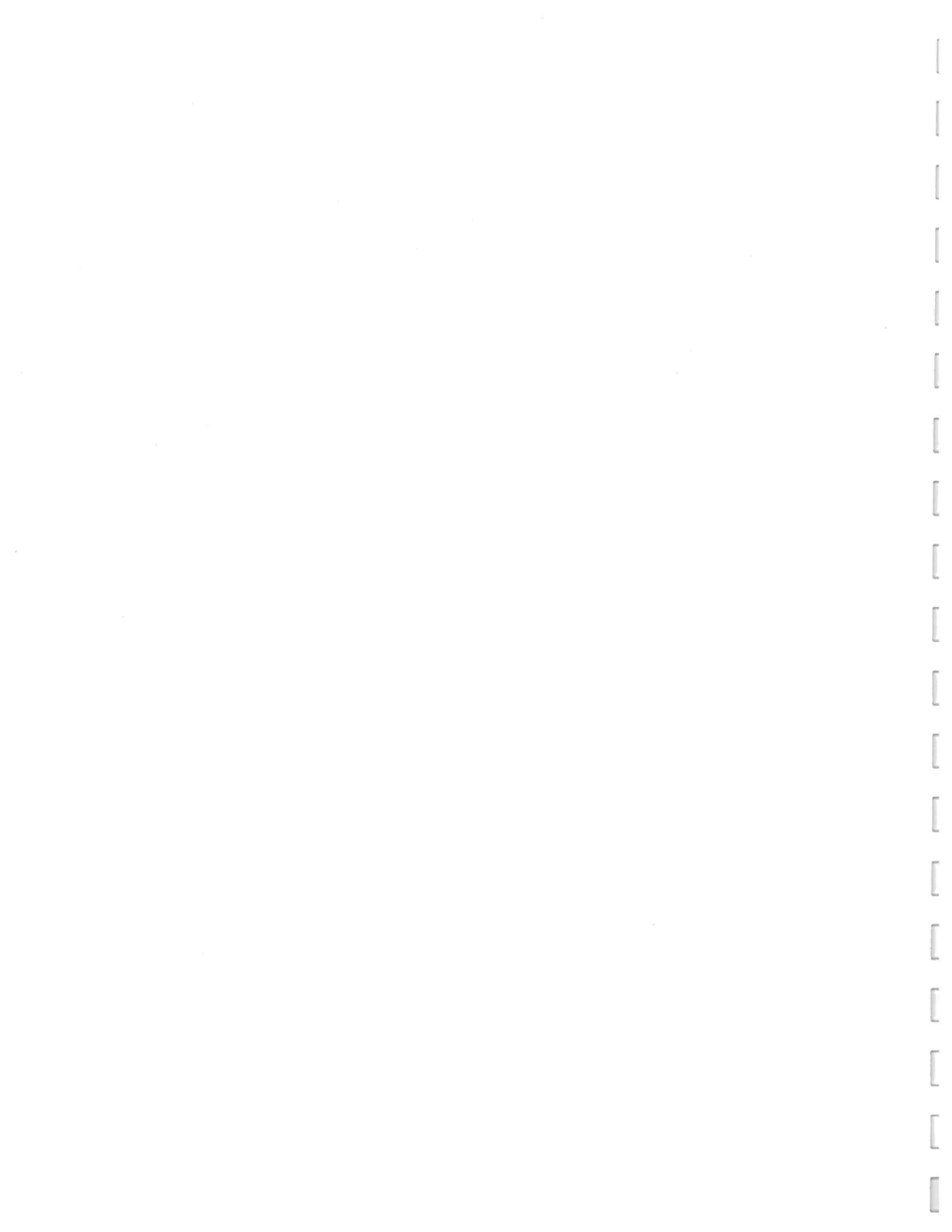
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TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313106
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126753	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BECS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFREO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THE INSTALLATION CONTRACTOR. ANY FABRICATED TRUSSES IN CONNECTION WITH THIS DESIGN SHALL BE SHIPPED AND BRACED IN ACCORDANCE WITH THE INSTALLATION AND BRACING OF TRUSSES DESIGN MANUALS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/15GA (W/3/5/8) ASTM A653 GRADE 40/60 (M, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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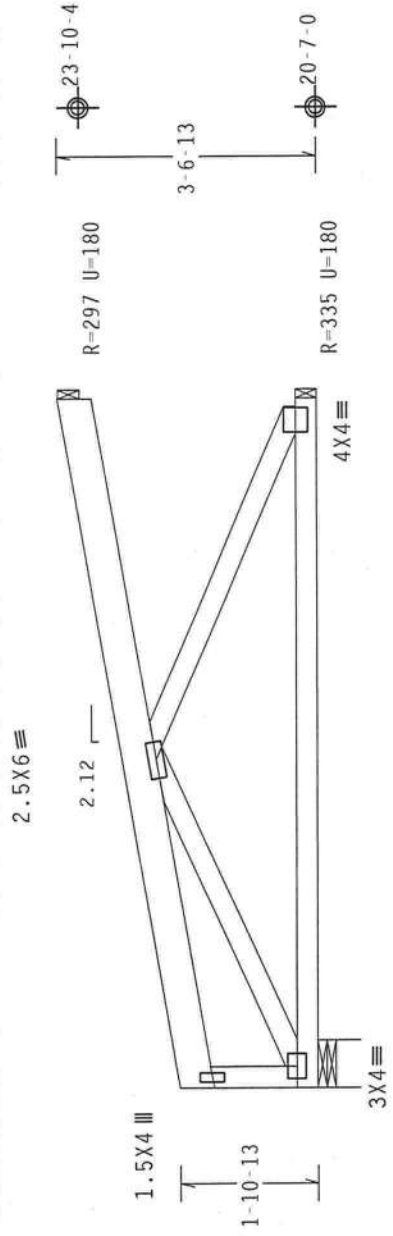


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

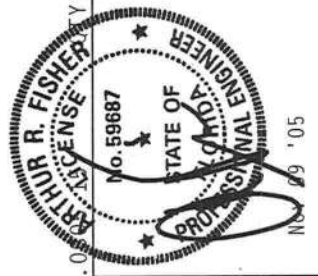
110 mph wind, 23.32 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 6-8-0 setback jacks with no webs.  
 Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



9-5-2 Over 3 Supports  
 R-335 U=180 W=7.602

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0)



FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL 20.0 PSF	REF R487-- 35490
TC DL 15.0 PSF	DATE 11/09/05
BC DL 10.0 PSF	DRW HCUSR487 05313107
BC LL 0.0 PSF	HC-ENG JB/AF
TOT.LD. 45.0 PSF	SEQN- 126947
DUR.FAC. 1.25	JREF- 1SS0487_Z01
SPACING 16.0"	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM DESIGNER'S OFFICE. ANY FABRICATOR OR SUBCONTRACTOR SHALL BE RESPONSIBLE FOR DESIGNING, ORDERING, SHIPPING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/19/18GA (W/1/5/3) ASTM A653 GRADE 40/60 (H, K/R/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 FL Certificate of Authorization # 567



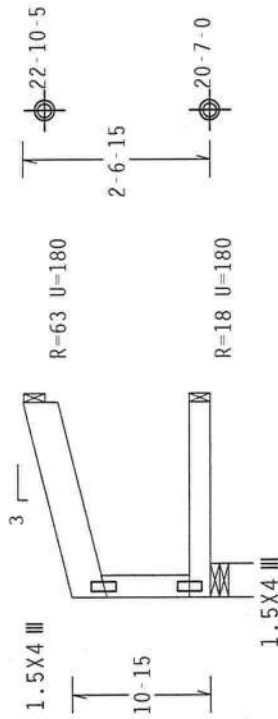
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 22.83 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

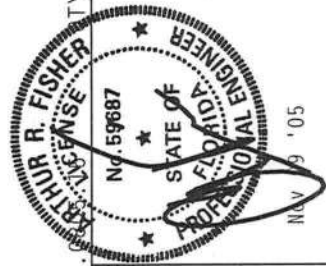
Cq/RT=1.00(1.25)/10(0)

7.04

TY:18 FL/-/3/-/-/R/-

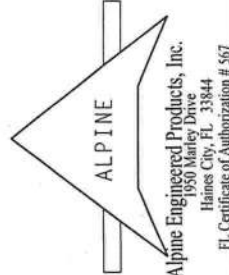
Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487--	35491
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313046
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	126959	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONOFRI DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE TRUSSES. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/1560 (M/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave

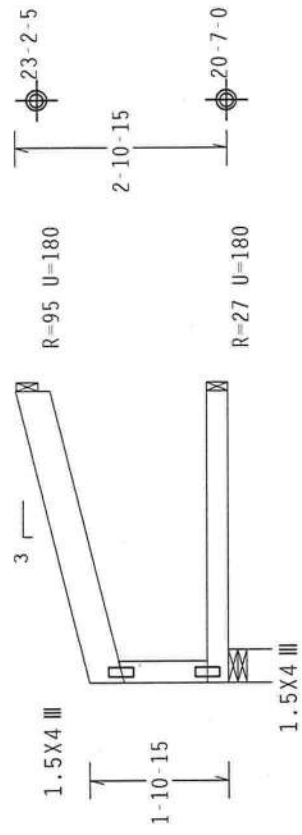
2-8-0 Over 3 Supports  
 R=81 U=180 W=5.375"



110 mph wind, 22.99 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



4-0-0 Over 3 Supports  
 R=121 U=180 W=5.375"

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

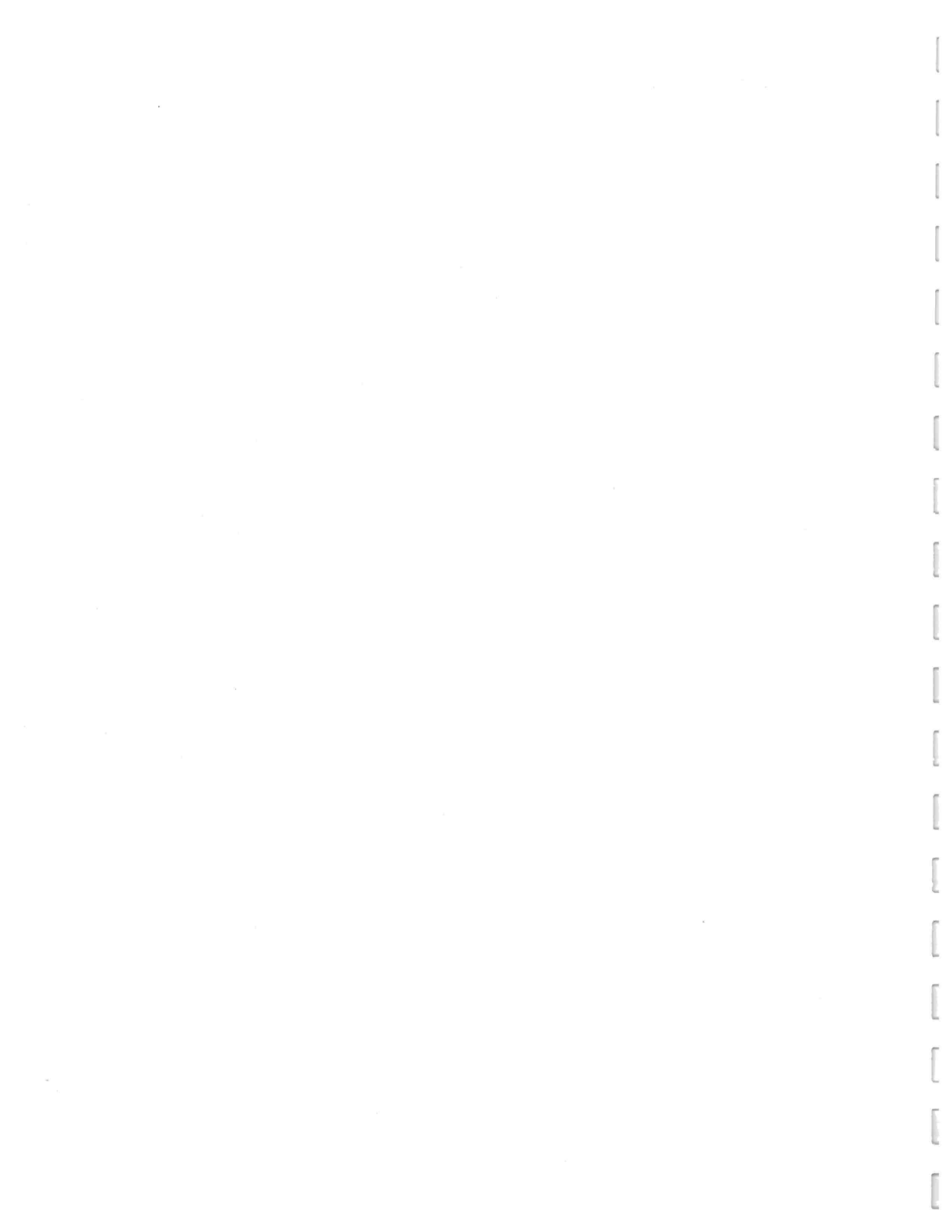


TY:13	FL/-/3/-/R/-	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487-- 35492
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313047
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEQN- 161055
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF BUILDING COMPONENTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGNER'S LIABILITY IS LIMITED TO THE DESIGN AND THE DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACP/A) AND TPI. ALPINE CORRELATOR PLATES ARE MADE OF 20/18/15GA (W/H/S/K) ASTM A653 GRADE 40/60 (H, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

**ALPINE**  
 Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

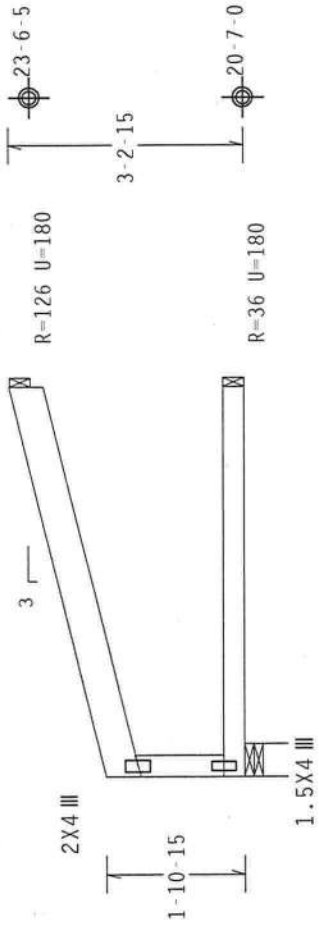


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 23.16 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



←5-4-0 Over 3 Supports →  
 R=162 U=180 W=5.375"

Design Crit: TPI-2002 (STD) /FBC

Cq/RT=1.00(1.25)/10(0)

7.04.0

FL/-/3/-/R/-

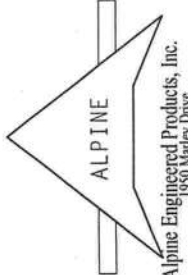
Scale = .375" /Ft.



Nov 05 '05

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRIO DR., SUITE 200, MADISON, WI 53719) AND NITCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF BUILDING TRUSSES IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROVISIONS OF NDS, NATIONAL DESIGN SPEC. BY ACAPA) AND TPI ALPINE CONNECTOR PLATES ARE MADE OF 20/10/15GA (R/R/S/S) ASTM A653 GRADE 40/60 (K, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER A900.3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



FL Certificate of Authorization # 587

TC LL	20.0 PSF	REF	R487--	35493
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313048
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	161063	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



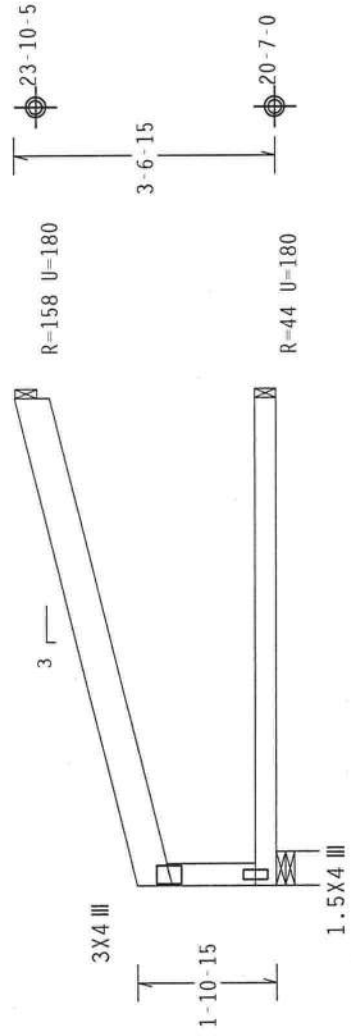
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

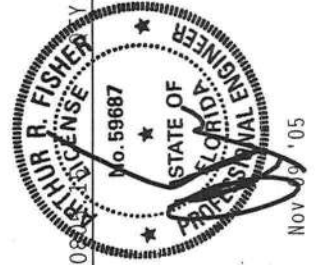
110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind IC DL=5.0 psf, wind BC DL=5.0 psf.



6-8-0 Over 3 Supports  
 R=202 U=180 W=5.375

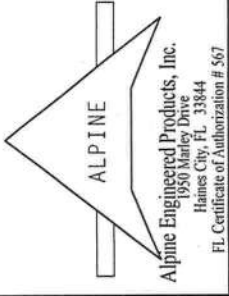
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave	Scale = .375" / Ft.
REF R487-- 35494	TC LL 20.0 PSF
DATE 11/09/05	TC DL 15.0 PSF
DRW HCUR487 05313049	BC DL 10.0 PSF
HC-ENG JB/AF *	BC LL 0.0 PSF
SEQN- 161071	TOT.LD. 45.0 PSF
DUR.FAC. 1.25	SPACING 16.0"
JREF- ISS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. PRODUCTS, THE INSTALLER SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS OR THE INSTALLATION OF THE TRUSS. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRETO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL NOT BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS OR THE INSTALLATION OF THE TRUSS. THE DESIGN CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE INSTALLATION OF THE TRUSS. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PAP) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/110/16GA (H/11/5/8) ASTM A653 GRADE 40/60 (H, K/N-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16DA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





Top chord 2x6 SP #1 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

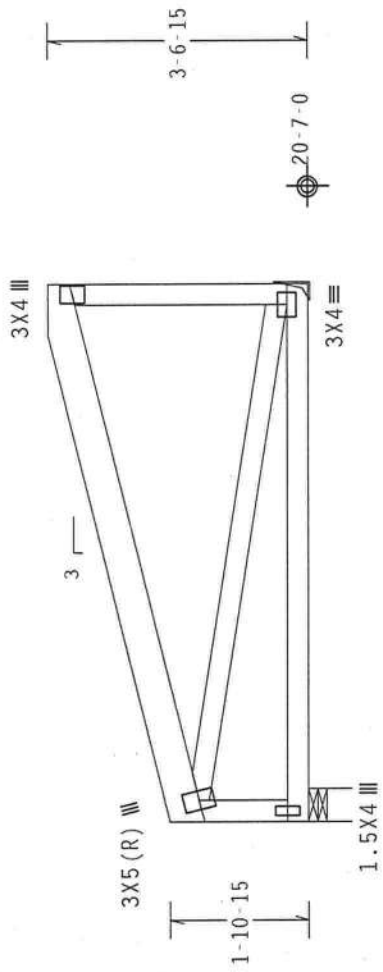
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

**SPECIAL LOADS**  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 6.67  
 BC - From 13 PLF at 0.00 to 13 PLF at 7.38  
 TC - 286 LB Conc. Load at 6.67  
 BC - 393 LB Conc. Load at 6.67

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.

#1 hip supports 6-8-0 jacks with no webs.

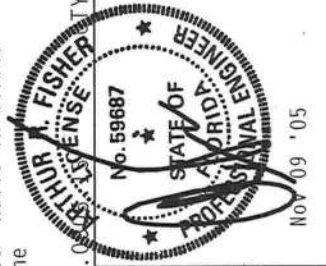


7-4-8 Over 2 Supports  
 R=281 U=180 W=5.375"

R=811 U=199 H=Simpson LUS26  
 w/ (4) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	FL / - / 3 / - / R / -	Scale = .375" / Ft.
	TC LL 20.0 PSF	REF R487 -- 35495
	TC DL 15.0 PSF	DATE 11/09/05
	BC DL 10.0 PSF	DRW HCUSR487 05313108
	BC LL 0.0 PSF	HC-ENG JB/AF
	TOT.LD. 45.0 PSF	SEQN- 126792
	DUR.FAC. 1.25	
	SPACING 16.0"	JREF- 1SS0487_201



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ALPINE ENGINEERED PRODUCTS, INC.  
 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



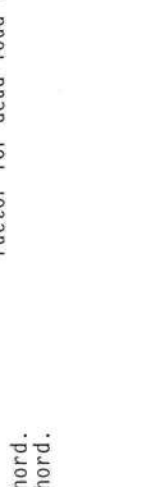
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 23.49 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

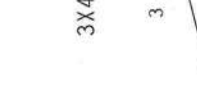


PLT TYP. Wave

Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .375" /Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 -- 35496
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313050
TOT.LD.	45.0 PSF	HC-ENG JB/AF *
DUR.FAC.	1.25	SEQN- 126747
SPACING	16.0"	JREF- 1SS0487_Z01



**ALPINE**

Alpine Engineered Products, Inc.  
 1950 Manley Drive  
 Haines City, FL 33844

FL Certificate of Authorization # 567

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Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.37 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

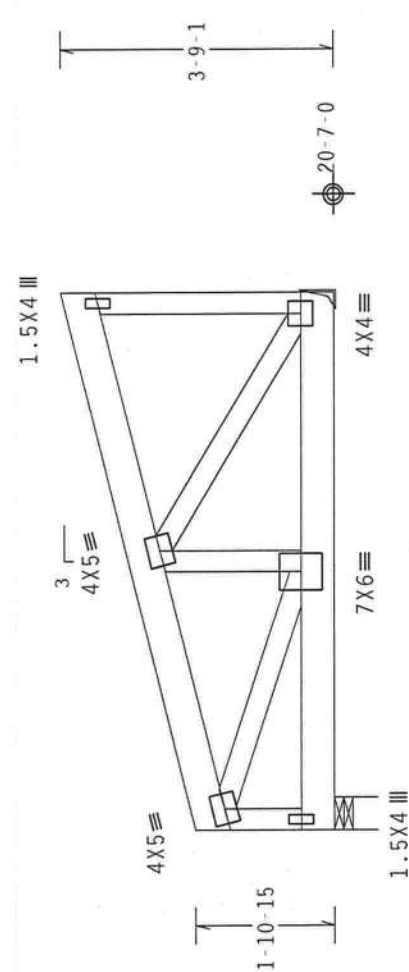
**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 7.38  
 BC - From 13 PLF at 0.00 to 13 PLF at 6.98  
 BC - From 60 PLF at 6.98 to 60 PLF at 7.38  
 BC - 597 LB Conc. Load at 0.77, 2.10, 3.44, 4.77, 6.10

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



7-4-8 Over 2 Supports  
 R=1817 U=537 W=5.375"

R=1635 U=478 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

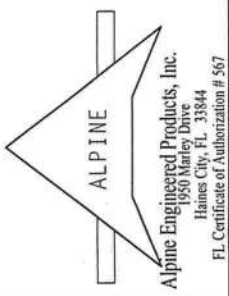
Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave	FL/3/-/R/-	Scale = .375" / Ft.
REF R487 -	20.0 PSF	35497
DATE 11/09/05	15.0 PSF	
DRW HCUSR487 05313109	10.0 PSF	
HC-ENG JB/AF	0.0 PSF	
SEQN- 126814	TOT.LD. 45.0 PSF	
JREF- 1SS0487_Z01	DUR.FAC. 1.25	
	SPACING 16.0"	



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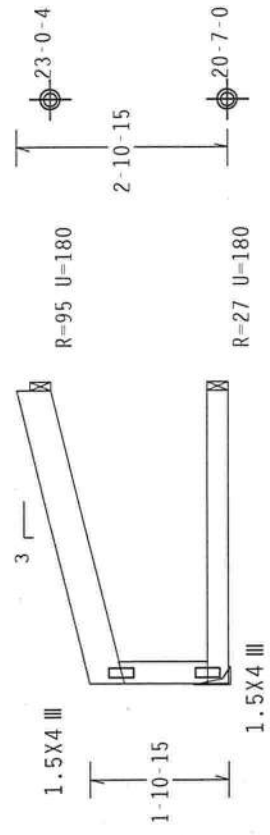
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 22.99 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



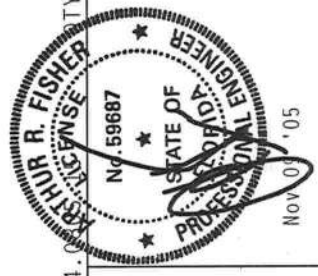
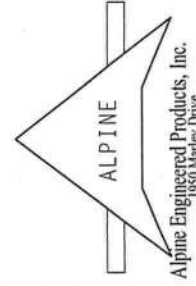
14-0-0 Over 3 Supports

R=121 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pi Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

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TY:6	FL/-/3/-/-/R/-	Scale = .375" /Ft.
TC LL	20.0 PSF	REF R487-- 35498
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUR487 05313051
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEQN- 161059
DUR.FAC.	1.25	JREF- 1SS0487_Z01
SPACING	16.0"	



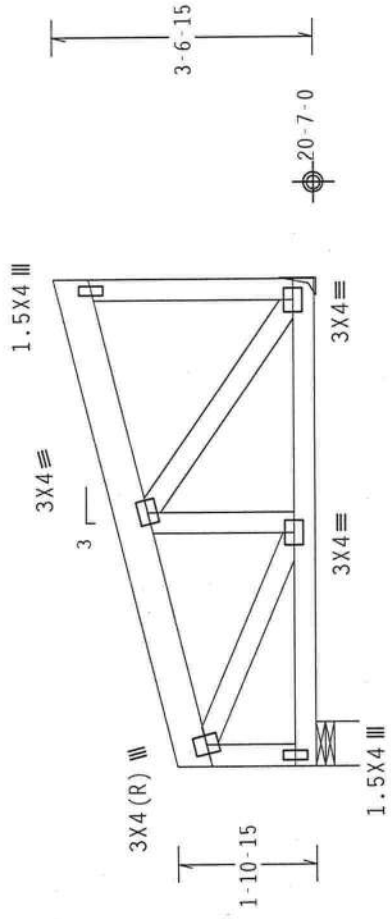
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 6.67  
 BC - From 13 PLF at 0.00 to 13 PLF at 6.67  
 BC - 121 LB Conc. Load at 0.42, 1.75, 3.08, 4.42, 5.75

Right end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

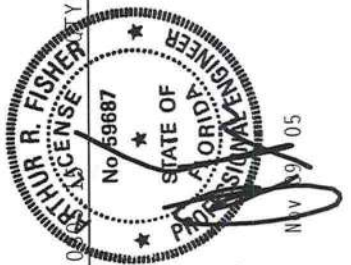


6-8-0 Over 2 Supports  
 R=527 U=180 W=7.25"

R=483 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .375" / Ft.	FL / - / 3 / - / - / R / -
REF R487 -- 35499	TC LL 20.0 PSF
DATE 11/09/05	TC DL 15.0 PSF
DRW HCUSR487 05313110	BC DL 10.0 PSF
HC-ENG JB/AF	BC LL 0.0 PSF
SEQN- 126875	TOT.LD. 45.0 PSF
JREF- 1SS0487_Z01	DUR.FAC. 1.25
	SPACING 16.0"



**ALPINE**

Alpine Engineered Products, Inc.  
 1950 Mantley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONDREO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AXPAS) AND TPI (TRUSS PLATE INSTITUTE) CONNECTOR PLATES ARE MADE OF 20/10/16GA (24/17/SP) ASTH A563 GRADE 40/60 (40/60) ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE INDICATED, TYPICAL CONNECTION PER DRAWINGS 160A-Z. ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

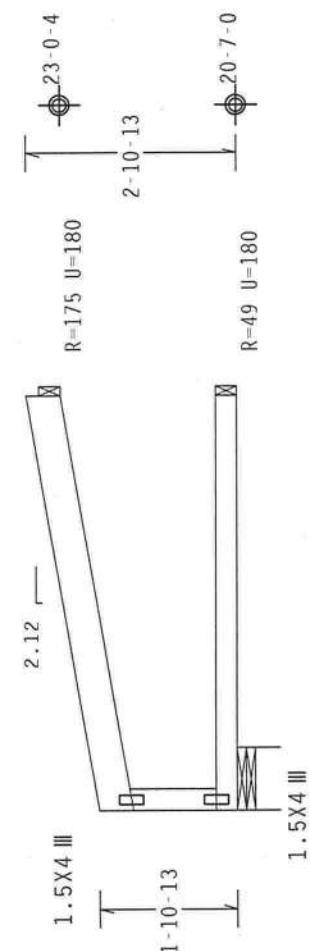


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 22.98 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

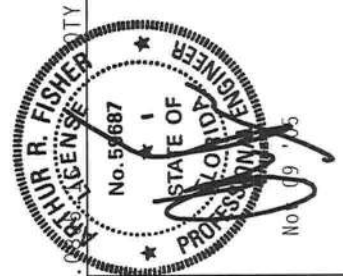
Hipjack supports 4-0-0 setback jacks with no webs.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



←5-7-14 Over 3 Supports →  
 R=112 U=180 W=10.253"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

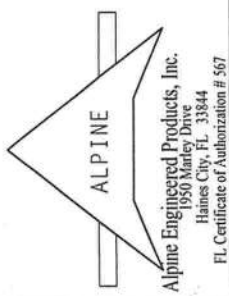
PLT TYP. Wave	TY:3 FL/-/3/-/-/R/-		Scale = .375" / Ft.
	TC LL	20.0 PSF	REF R487-- 35500
	TC DL	15.0 PSF	DATE 11/09/05
	BC DL	10.0 PSF	DRW HCUSR487 05313111
	BC LL	0.0 PSF	HC-ENG JB/AF
	TOT.LD.	45.0 PSF	SEQN- 126904
	DUR.FAC.	1.25	
	SPACING	16.0"	JREF- 1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002(STD) OR TPI-2002(STD) SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE TRUSS FABRICATOR SHALL PROVIDE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. THE TRUSS FABRICATOR SHALL PROVIDE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





110 mph wind, 23.15 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

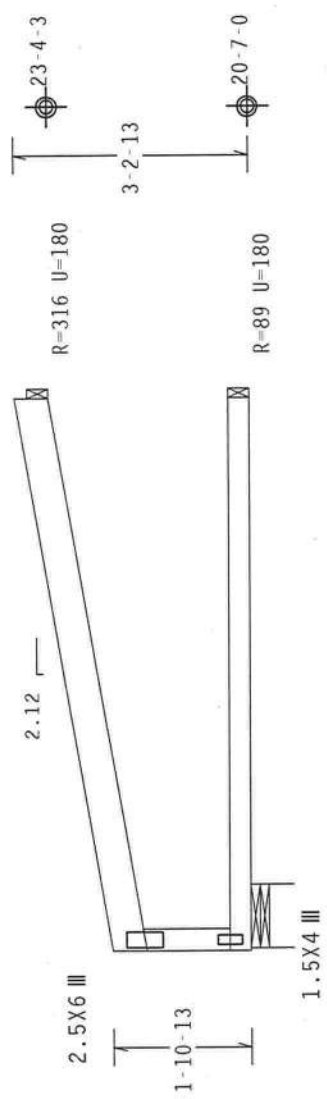
Hipjack supports 5-4-0 setback jacks with no webs.

Provide ( 3 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



0-0-6

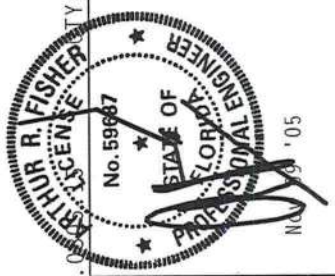
7-6-8 Over 3 Supports  
 R=201 U=180 W=10.253\*

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave

Scale = .375" /Ft.

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND HICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (N-J/S/R) ASTH A653 GRADE 40/60 (N, R/H/S) GALV. GALV. PER DRAWINGS 1604.2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, SHALL BE APPLIED TO EACH FACE OF TRUSS. THE DESIGNER ACCEPTS THE RESPONSIBILITY OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER PER ANSI/TPI 1 SEC. 2.

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Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

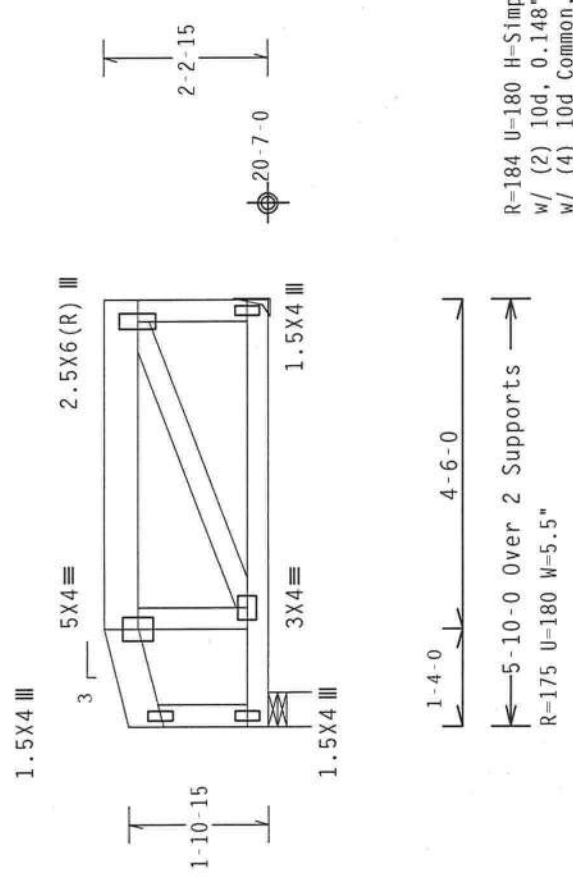
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

#1 hip supports 1-4-0 jacks with no webs.

Right end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 22.66 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

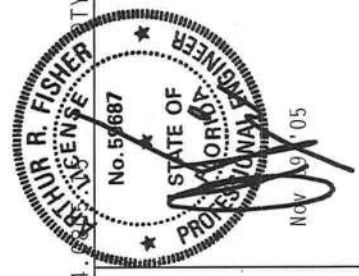


R=184 U=180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

R=175 U=180 W=5.5"  
 ← 5-10-0 Over 2 Supports →

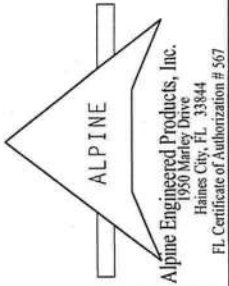
Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

PLT TYP. Wave	TY:1	FL/-/3/-/-/R/-	Scale = .375" /Ft.
	TC LL	20.0 PSF	REF R487-- 35502
	TC DL	15.0 PSF	DATE 11/09/05
	BC DL	10.0 PSF	DRW HCUR487 05313113
	BC LL	0.0 PSF	HC-ENG JB/AF
	TOT.LD.	45.0 PSF	SEQN- 161076
	DUR.FAC.	1.25	
	SPACING	16.0"	JREF- 1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D'ONDORF DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGN SHALL BE IN ACCORDANCE WITH THE APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/5/8) ASTM A653 GRADE 40/60 (Q. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





110 mph wind, 22.66 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

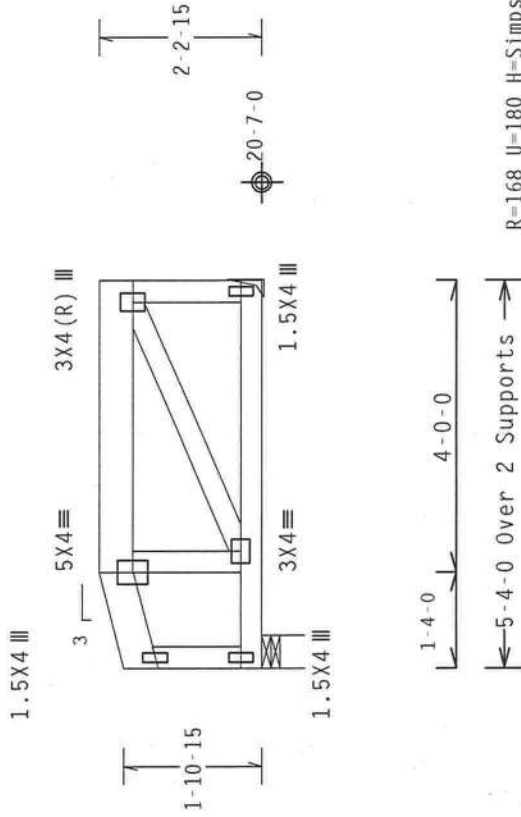
Right end vertical not exposed to wind pressure.  
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

#1 hip supports 1-4-0 jacks with no webs.

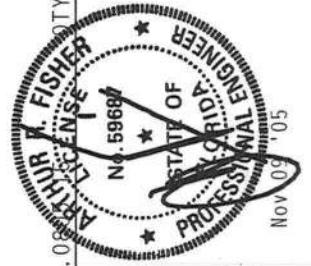
Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3



R=168 U=180 H=Simpson LU24  
w/ (2) 10d, 0.148"x1.5" nails in Truss  
w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

TC LL	20.0 PSF	REF	R487--	35503
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313114
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126972	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D-ONDORIO DR., SUITE 200, MADISON, WI 53719) AND UTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL MEMBERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS/1/1/1 SEC. 2.

ALPINE  
Alpine Engineered Products, Inc.  
1950 Mateo Drive  
Haines City, FL 33844  
FL Certificate of Authorization # 567

PLT TYP. Wave

Scale = .375" / Ft.



110 mph wind, 22.73 ft mean hgt., ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	71 PLF at 0.00 to	20 PLF at 5.83
BC - From	20 PLF at 0.00 to	20 PLF at 5.83
TC -	63 LB Conc. Load at	0.59
BC -	18 LB Conc. Load at	0.59
BC -	121 LB Conc. Load at	1.92
BC -	162 LB Conc. Load at	3.26
BC -	962 LB Conc. Load at	4.56



R=1188 U=361 H=Simpson HUS26  
W/ (6) 10d Common, 0.148"x3.0" nails in Truss  
W/ (14) 10d Common, 0.148"x3.0" nails in Girder  
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

Alpine Engineered Products, Inc.  
1950 Mateo Drive  
Haines City, FL 33844  
FL Certificate of Authorization # 567

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487--	35504
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313115
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	161098
SPACING	24.0"	JREF-	1SS0487_Z01

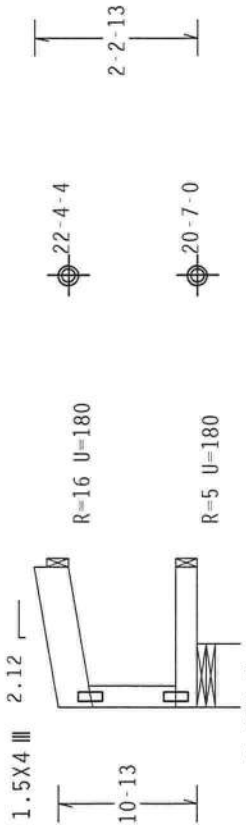
**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC61 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D-080RD DR., SUITE 200, HANSON, WI 53139) AND WICK (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, TOP CHURCH SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** \*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/H/S/A) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002-SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS/TP1 1 SEC. 2.



110 mph wind, 22.65 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
 Hipjack supports 1-4-0 setback jacks with no webs.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Top chord.  
 Provide ( 2 ) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3  
 Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



1-10-10 Over 3 Supports  
 R=11 U=180 W=10.253"

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08



TC LL	20.0	PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0	PSF	REF R487 - -	35505
BC DL	10.0	PSF	DATE	11/09/05
BC LL	0.0	PSF	DRW	HCUSR487 05313116
TOT.LD.	45.0	PSF	HC-ENG	JB/AF
DUR.FAC.	1.25		SEQN-	126965
SPACING	16.0"		JREF-	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DORRFIELD DR., SUITE 200, MADISON, WI 53719) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, WILSONVILLE, OR 97148) FOR ADDITIONAL INFORMATION. ALL TRUSSES MUST BE PROPERLY MANUFACTURED, TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS, AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/7/6GA (W, H/S/P) ASTM A653 GRADE 40/60 (W, R/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMBEX/TPI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567

PLT TYP. Wave



110 mph wind, 23.16 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

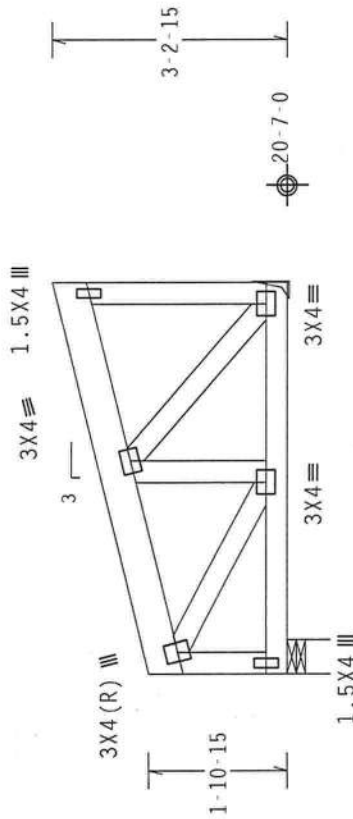
Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Meds 2x4 SP #3

**SPECIAL LOADS**  
 (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 5.33  
 BC - From 20 PLF at 0.00 to 20 PLF at 5.33  
 BC - 81 LB Conc. Load at 1.73  
 BC - 439 LB Conc. Load at 3.06  
 BC - 270 LB Conc. Load at 4.40



←-5-4-0 Over 2 Supports →  
R=525 U=180 W=5.5\*

R-749 U=228 H=Simpson LUS24  
 w/ (2) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

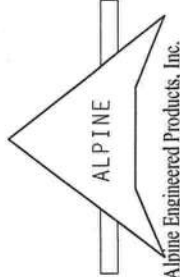
Design Crit: TPI-2002 (STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.04.08

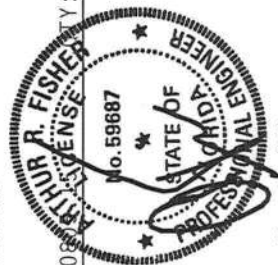
Scale = .375" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFELLO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, SUITE 100, WASHINGTON, DC 20004) PRIOR TO FABRICATING THESE TRUSSES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (W, W/S/W) ASTM A653 GRADE 40/60 (H, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMBEX/TPI 1 SEC. 2.



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 1950 Mayes Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



TC LL	20.0 PSF	REF	R487--	35506
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313117
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	126982	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1SS0487_Z01	



5-344-ISAAC CONST/muber - ROOF - HTG)  
 Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

110 mph wind, 22.80 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

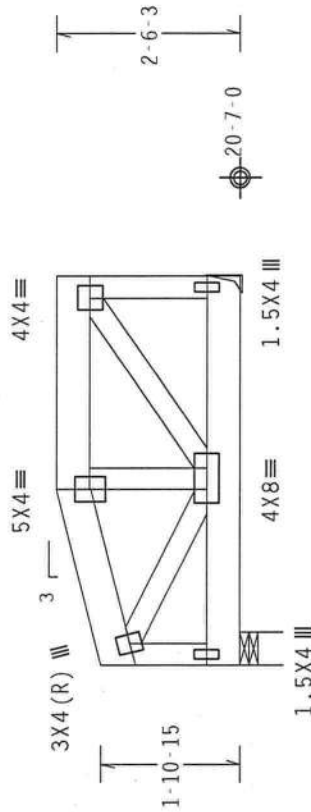
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 5.33  
 BC - From 20 PLF at 0.00 to 20 PLF at 5.33  
 BC - 530 LB Conc. Load at 0.77, 2.10, 3.44, 4.77

Right end vertical not exposed to wind pressure.

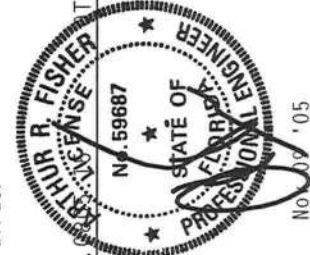
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1260 U=373 W=5.375"  
 R=1344 U=397 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

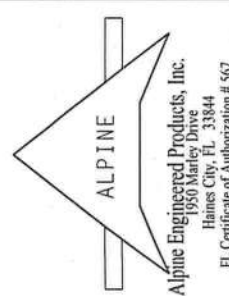
Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.

TC LL	20.0	PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0	PSF		REF R487 -- 35507
BC DL	10.0	PSF		DATE 11/09/05
BC LL	0.0	PSF		DRW HCUSR487 05313118
TOT.LD.	45.0	PSF		HC-ENG JB/AF
DUR.FAC.	1.25			SEQN- 161255
SPACING	24.0"			JREF- 1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TO AVOID COLLAPSE OR FAILURE OF THE TRUSS SYSTEM, THE USER SHALL BE RESPONSIBLE FOR THE CORRECT INSTALLATION OF THE TRUSS SYSTEM. THE USER SHALL BE RESPONSIBLE FOR THE CORRECT INSTALLATION OF THE TRUSS SYSTEM. THE USER SHALL BE RESPONSIBLE FOR THE CORRECT INSTALLATION OF THE TRUSS SYSTEM.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GGA (W,H/S/K) ASTM A653 GRADE 40/60 (H, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FABRICATED BY ALPINE ENGINEERED PRODUCTS, INC. FOR THE BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

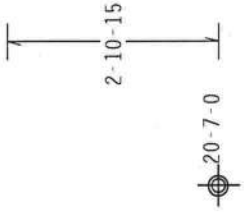
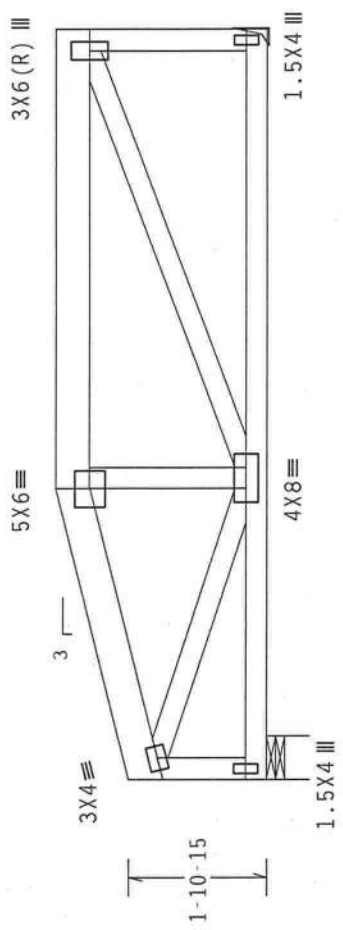
#1 hip supports 4-0-0 jacks with no webs.

110 mph wind, 22.99 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



4-0-0  
 6-3-12  
 10-3-12 Over 2 Supports  
 R=515 U=180 W=7.25"

R-620 U=183 H-Simpson LUS24  
 w/ (2) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave	7.04.00	FL/-/3/-/-/R/-	Scale = .375" / Ft.
REF	R487 --	20.0 PSF	35508
DATE	11/09/05	15.0 PSF	
DRW	HCUSR487 05313119	10.0 PSF	
HC-ENG	JB/AF	0.0 PSF	
SEON-	161220	TOT.LD. 45.0 PSF	
DUR.FAC.	1.25		
SPACING	16.0"		
JREF-	ISS0487_Z01		



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSEI 1.003, BUILDING COMMON SENSE, FOR THE LATEST INFORMATION ON THE PROVISIONS OF THE NATIONAL BUILDING CODE, D'ORFELLO DR., SUITE 200, MADISON, WI 53719, AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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Top chord 2x6 SP #2  
 Bot chord 2x6 SP #2  
 Webs 2x4 SP #3

110 mph wind, 23.12 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

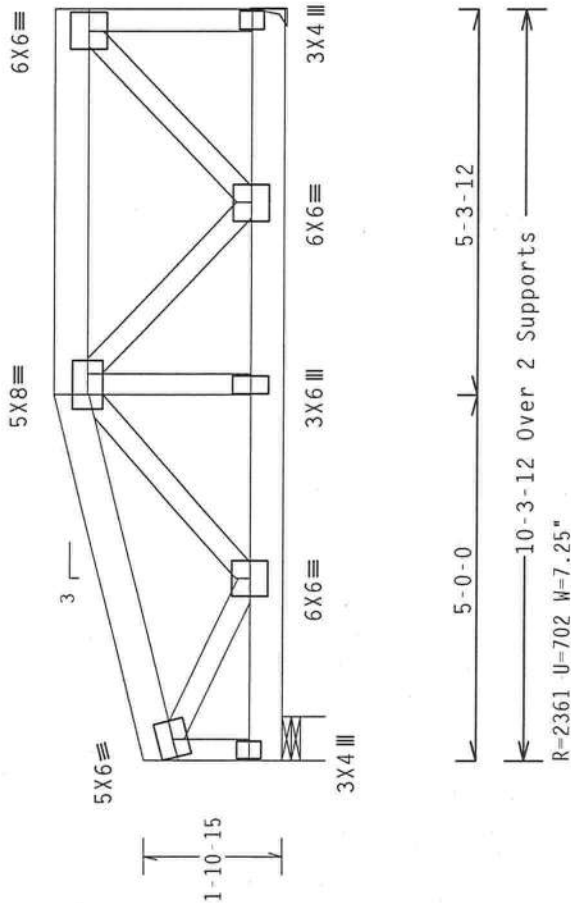
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 10.31  
 BC - From 20 PLF at 0.00 to 20 PLF at 10.31  
 BC - 530 LB Conc. Load at 1.04, 2.37, 3.71, 5.04  
 BC - 526 LB Conc. Load at 6.37, 7.71, 9.04

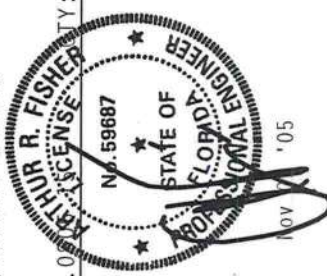
Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



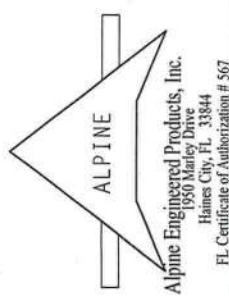
R-2273 U=683 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), BUILDING CODES, TPI-2002 (STEEL TRUSS DESIGN), D'ONOFRIO DR., SUITE 200, MADISON, WI 53719, AND IBCA (WOOD TRUSS COUNCIL OF AMERICA, 3300 ENTERPRISE BLVD., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 (STEEL TRUSS DESIGN) OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (HANS) DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/T606 (R, H/2/H) ASTM A653 GRADE 40/50 (H, K/R-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. DRAWING INDICATES PLATE LENGTH AND POSITION. SEE DRAWING 160A-2 FOR DIMENSIONS AND CONNECTIONS. A SEAL ON THIS DESIGN INDICATES THE DESIGNER'S RESPONSIBILITY. THE SEALING RESPONSIBILITY IS THE RESPONSIBILITY OF THE DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.



PLT TYP. Wave	Scale = .375" / Ft.
TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
REF	R487 -- 35509
DATE	11/09/05
DRW	HCUSR487 05313120
HC-ENG	JB/AF
SEQN-	161268
JREF-	1SS0487_Z01

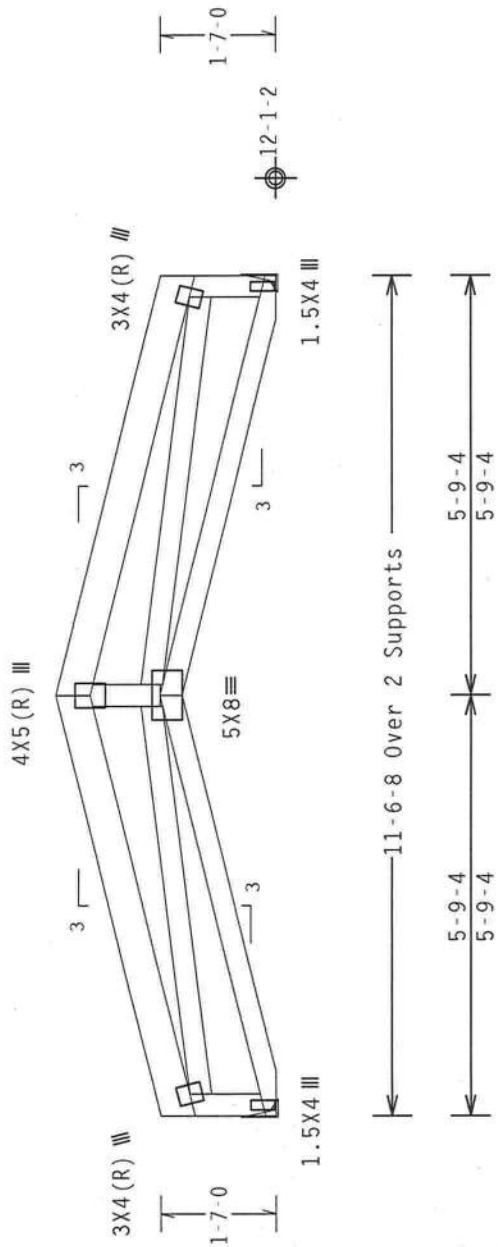


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.40 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=352 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=352 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.04.08

Scale = .375" / Ft.

TC LL	20.0 PSF	FL/-/3/-/-/R/-
TC DL	15.0 PSF	
BC DL	10.0 PSF	
BC LL	0.0 PSF	
TOT.LD.	45.0 PSF	
DUR.FAC.	1.25	
SPACING	16.0"	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF ALL TRUSSES. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/T16GA (N-H/S/K) ASTM A653 GRADE 40/60 (H. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PERFORMED AS PER TPI-2002, SEC. 3.1. SEAK OR HIRSH DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

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Nov 09 '05

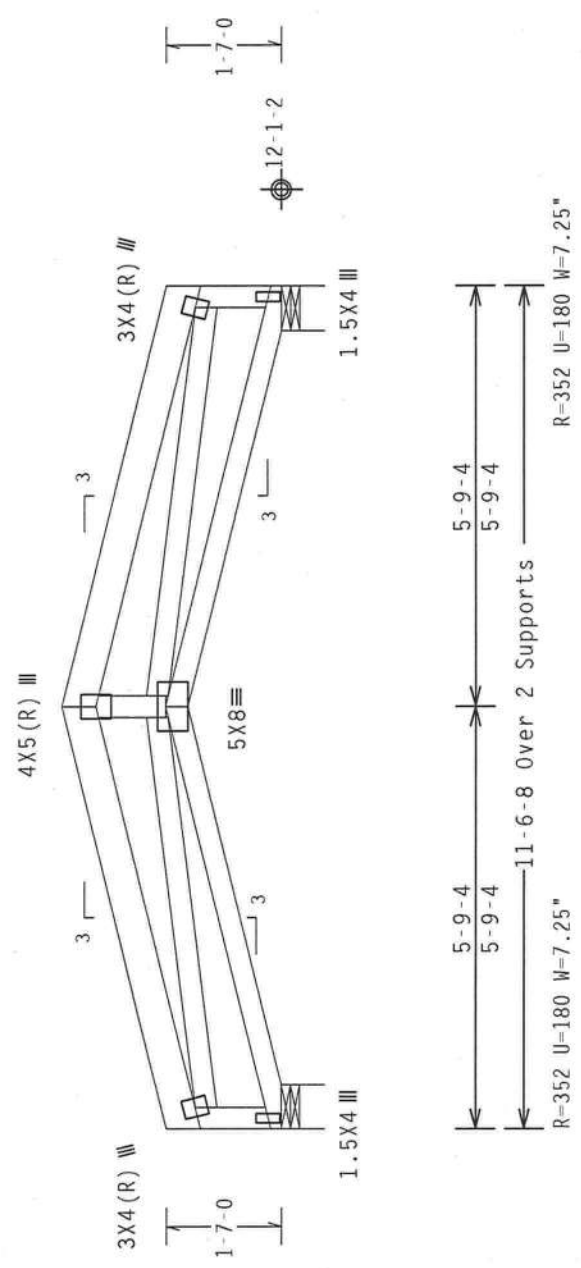
JREF- 1SS0487\_Z01



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

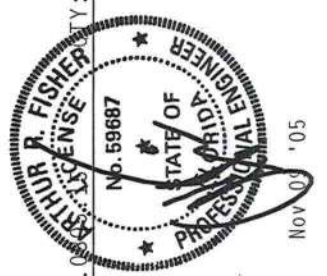
Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 14.40 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00 Scale = .375" /Ft.

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 503 D'ORFELD DR., SUITE 200, MADISON, WI 53719, AND NCA (GOOD TRUSS COUNCIL OF AMERICA, GOOD ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/S/P) ASTM A653 GRADE 40/60 (W. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10A-Z. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. AND IS TO BE USED ONLY FOR THE DESIGN AND CONSTRUCTION OF TRUSSES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSE/TPI 1 SEC. 2.

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 Haines City, FL 33844  
 FL Certificate of Authorization # 567



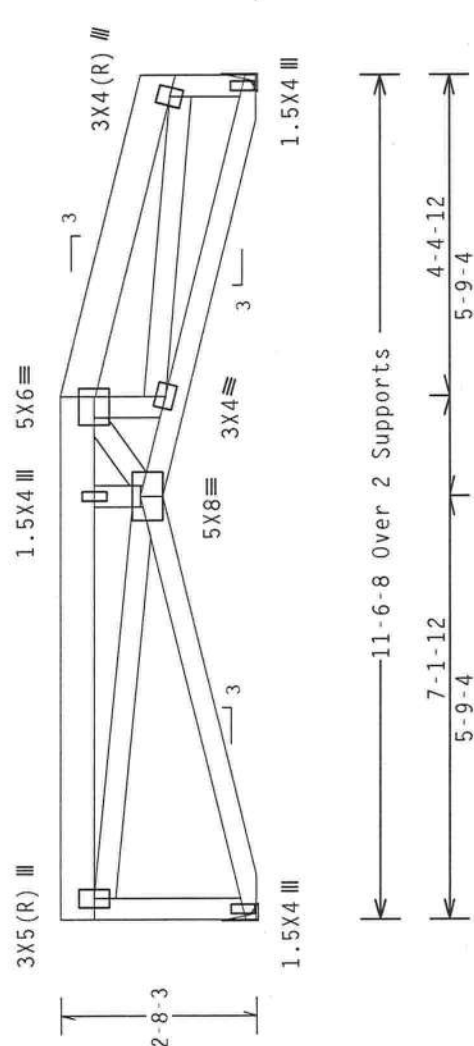
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 14.23 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4-50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

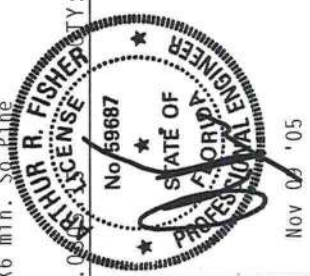


R-353 U=180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R-351 U=180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - -	35512
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313123
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126511
SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CROSS PLATE INSTITUTE, 593 D'ORFELIO DR., SUITE 200, MADISON, WI 53719) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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 FL Certificate of Authorization # 567

PLT TYP. Wave



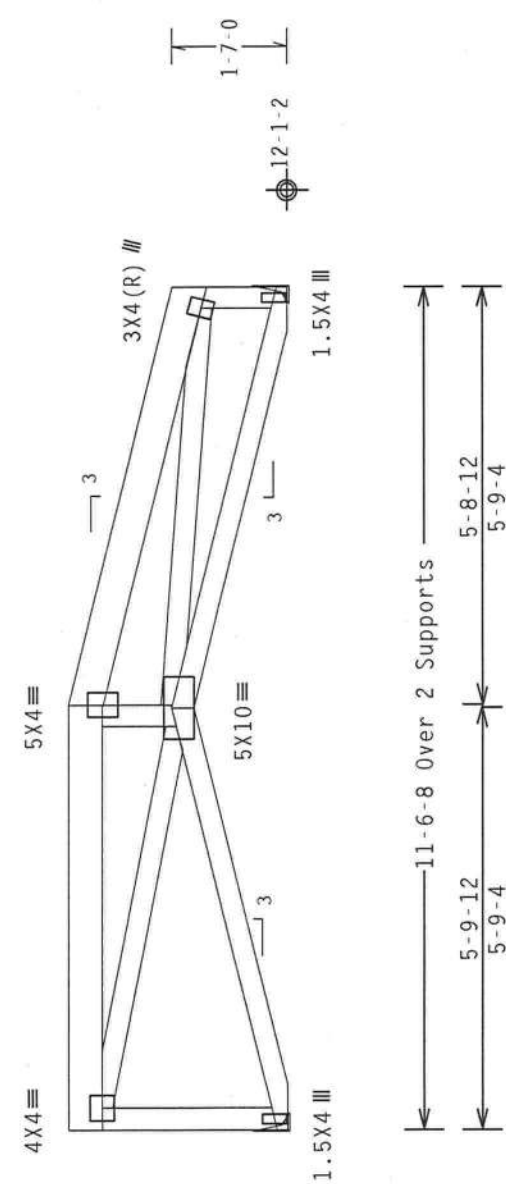
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.39 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Left end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

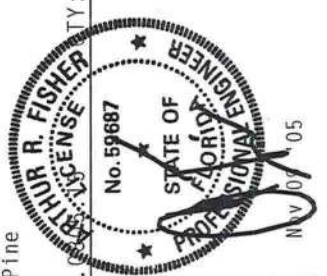


R=353 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=352 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 --	35513
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313124
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	126533
SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AYAP) AND TPI. ALPINE CONNECTION PLATES ARE MADE IN THE USA. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. ANY INSPECTION OF PLATES FOLLOWED BY THIS SHALL BE PER ANNEK 63 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS1/TPI 1 SEC. 2.

**ALPINE**  
 Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



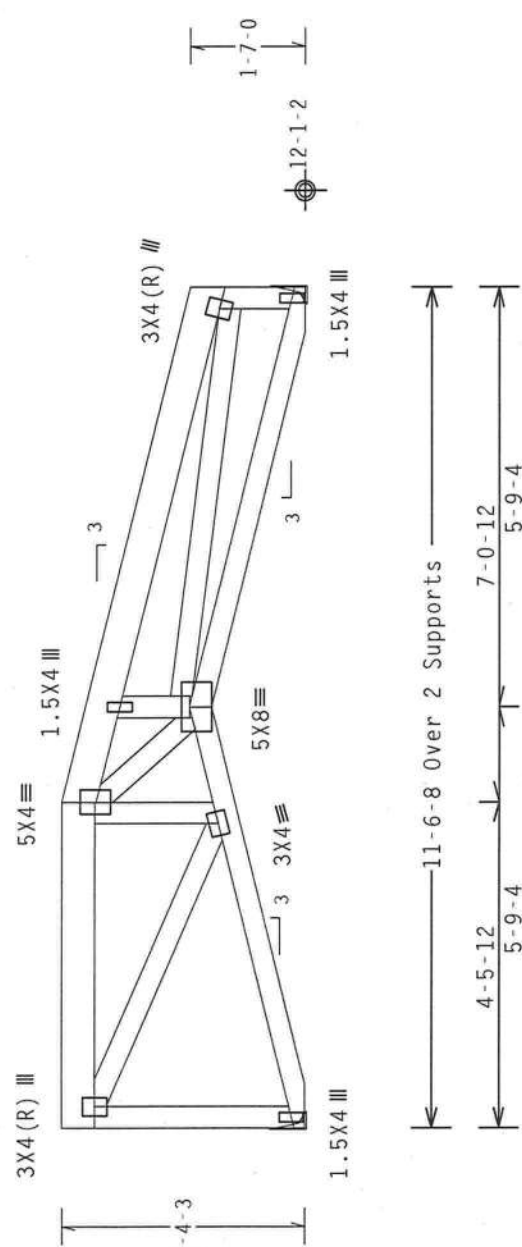
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 14.56 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Left end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=351 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=353 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

PLT TYP. Wave	Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 7.04.00		FL/-/3/-/-/R/-	Scale = .375" /Ft.
	TC LL	20.0 PSF	REF	R487 -- 35514
	TC DL	15.0 PSF	DATE	11/09/05
	BC DL	10.0 PSF	DRW	HCUSR487 05313125
	BC LL	0.0 PSF	HC-ENG	JB/AF
	TOT.LD.	45.0 PSF	SEQN-	126523
	DUR.FAC.	1.25		
	SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTION PLATES ARE MADE OF 20/18/10GA (W, H/S/F) ASTM A653 GRADE 40/80 (W, K/R/S) GALV. STEEL. APPLY PLATE PROTECTION TO ALL TRUSS AND CONNECTION SURFACES UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 100A, Z, 100B, 100C, 100D, 100E, 100F, 100G, 100H, 100I, 100J, 100K, 100L, 100M, 100N, 100O, 100P, 100Q, 100R, 100S, 100T, 100U, 100V, 100W, 100X, 100Y, 100Z. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

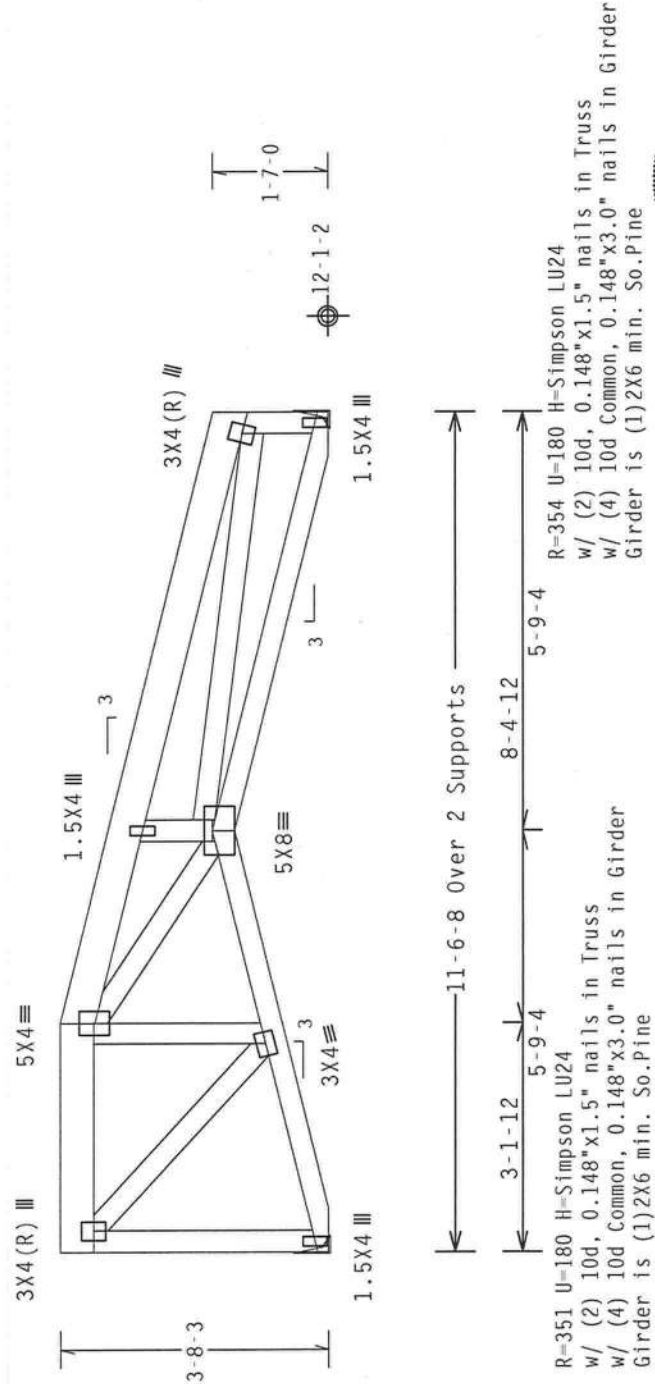
Left end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 14.73 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

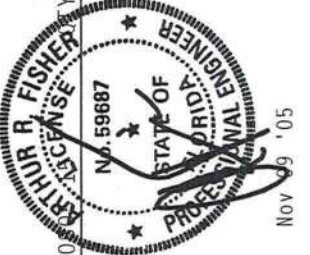
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave

Scale = .375" / Ft.



TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - 35515
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313126
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN- 126529
SPACING	16.0"	JREF- 1SS0487_Z01

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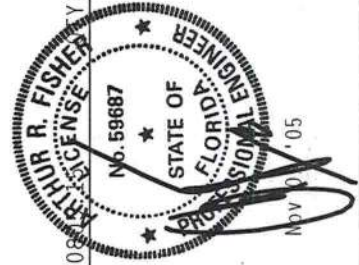
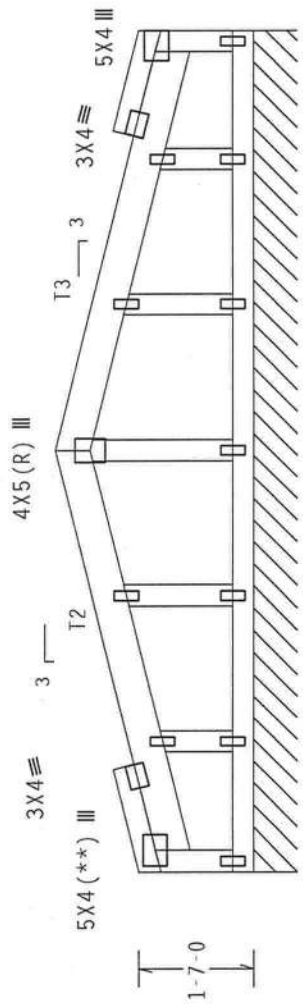
Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

SPECIAL LOADS  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 68 PLF at 0.00 to 68 PLF at 11.54  
 BC - From 13 PLF at 0.00 to 13 PLF at 11.54

Trusses to be spaced at 16.0" OC maximum.  
 Fasten rated sheathing to one face of this frame.  
 (\*\*) Plate relocated as shown.

See DWGS A11015EC1103 & GBLLETTIN0405 for more requirements.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 14.13 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



Note: All Plates Are 1.5X4 Except As Shown.  
 Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08  
 Scale = .375"/Ft.

PLT TYP. Wave

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MTGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

REF R487-- 35516  
 DATE 11/09/05  
 DRW HCUR487 05313127  
 HC-ENG JB/AF  
 SEQN- 126539  
 JREF- 1SS0487\_Z01

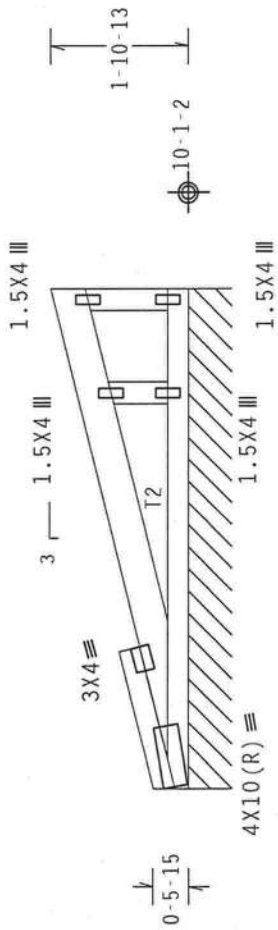


Top chord 2x4 SP #2 Dense : T2 2x6 SP #2:  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

**SPECIAL LOADS**  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 91 PLF at 0.00 to 91 PLF at 6.83  
 BC - From 20 PLF at 0.00 to 20 PLF at 6.83

Dead loads are stated on projected horizontal area basis.  
 See DMGS A11015EC1103 & GBLLETTIN0405 for more requirements.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

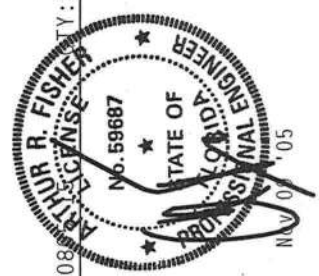
110 mph wind, 11.19 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



0-4-12  
 1-6-8  
 4-10-11  
 R-84 PLF U-28 PLF W-6-10-0

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25) / 10(0)

PLT TYP. Wave	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 - - 35517
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313128
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 126978
DUR.FAC.	1.25	JREF- 1SS0487_Z01
SPACING	24.0"	



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ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSI/TPI 1 SEC. 2.

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Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Dead loads are stated on projected horizontal area basis.

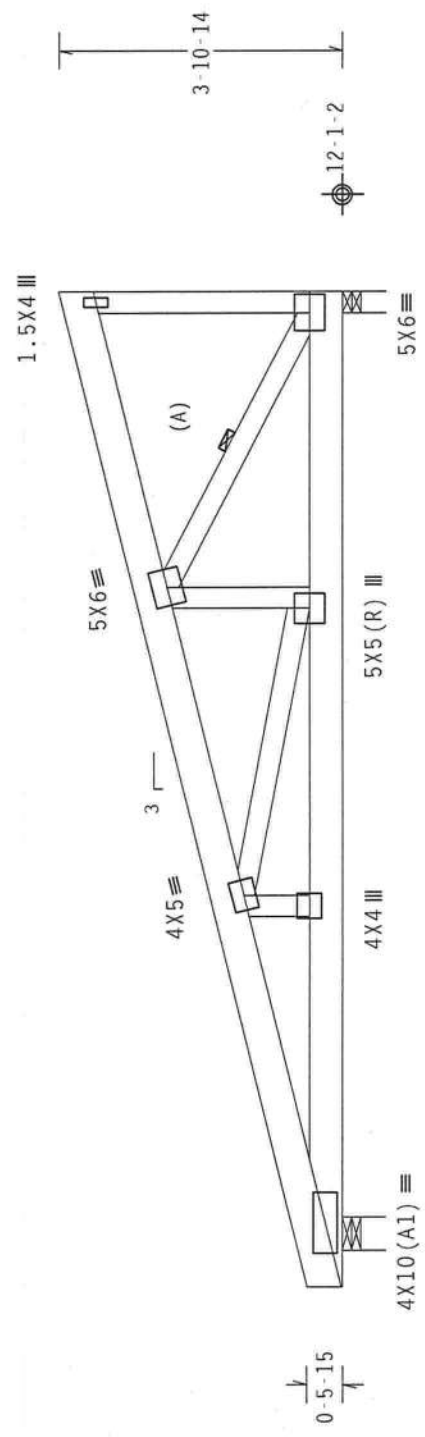
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 70 PLF at 0.00 to 70 PLF at 13.65  
 BC - From 20 PLF at 0.00 to 20 PLF at 13.65  
 BC - 519 LB Conc. Load at 1.48, 3.48, 5.48, 7.48, 9.48  
 11.48

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.



0-6-0

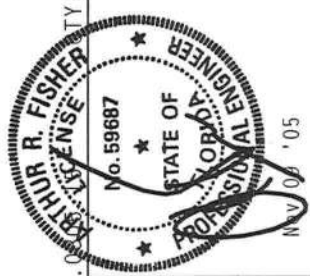
R-2327 U=698 W=5.5"

13-7-12 Over 2 Supports

R-2017 U=617 W=3.5"

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0)

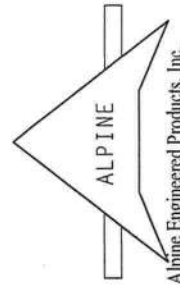
PLT TYP. Wave	TY:1	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
	TC LL	20.0 PSF	REF R487 - - 35518
	TC DL	15.0 PSF	DATE 11/09/05
	BC DL	10.0 PSF	DRW HCUSR487 05313129
	BC LL	0.0 PSF	HC-ENG JB/AF
	TOT.LD.	45.0 PSF	SEQN- 161387
	DUR.FAC.	1.25	JREF- 1SS0487_Z01
	SPACING	24.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC'S 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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110 mph wind, 12.59 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.  
 See DWGS A11015EC1103 & GBLLETIN0405 for more requirements.  
 Fasten rated sheathing to one face of this frame.

Top chord 2x4 SP #2 Dense : T2 2x6 SP #2:  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3 : C1 2x4 SP #2 Dense:

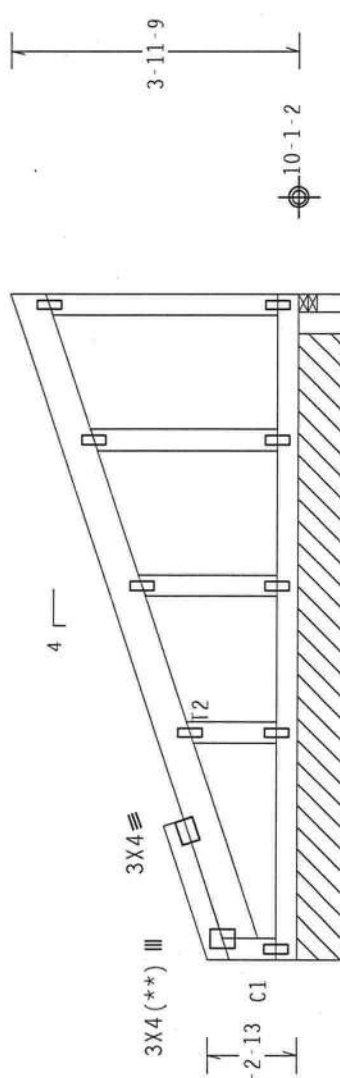
**SPECIAL LOADS**

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 91 PLF at 0.00 to 91 PLF at 9.10  
 BC - From 20 PLF at 0.00 to 20 PLF at 9.10

Dead loads are stated on projected horizontal area basis.

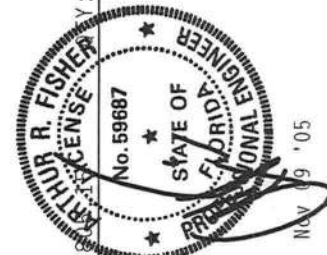
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

(\*\*) Plate relocated as shown.



0-3-8  
 1-7-4  
 7-2-8  
 9-1-4 Over 2 Supports  
 R=108 PLF U=33 PLF W=8-6-12  
 R=87 U=180 W=3"

Note: All Plates Are 1.5X4 Except As Shown.  
 Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08



Scale = .375" / Ft.	FL / - / 3 / - / R / -
TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
REF	R487 -- 35519
DATE	11/09/05
DRW	HCUSR487 05313130
HC-ENG	JB/AF
SEQN	161380
JREF	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1666A (H,H/S/E) ASTM A653 GRADE 40/50 (4, K/H,5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. SHOWN FOR INFORMATION ONLY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE  
 Alpine Engineered Products, Inc.  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



Top chord 2x4 SP #2 Dense : T2 2x6 SP #2:  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

SPECIAL LOADS  
 ----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 91 PLF at 0.00 to 91 PLF at 5.92  
 BC - From 20 PLF at 0.00 to 20 PLF at 5.92

Dead loads are stated on projected horizontal area basis.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 11.77 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

Scale = .375" / Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	REF R487 - - 35520
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUSR487 05313161
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN- 161372
SPACING	24.0"	JREF- 1SS0487_Z01

PLT TYP. Wave

ALPINE  
 Alpine Engineered Products, Inc.  
 1950 Marley Drive  
 Gaines City, FL 33844  
 FL Certificate of Authorization # 567

THOMAS R. FISHER  
 LICENSE  
 No. 99687  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 Nov 09 '05

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE TPI CROSS PLATE INSTITUTE, 583 D'AMORELO DR., SUITE 200, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.  
 \*\*IMPORTANT\*\* FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/1666 (W.H/S/K) ASTM A563 GRADE 40/60 (H. R/B.5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.2. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNED AND NOT THE USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



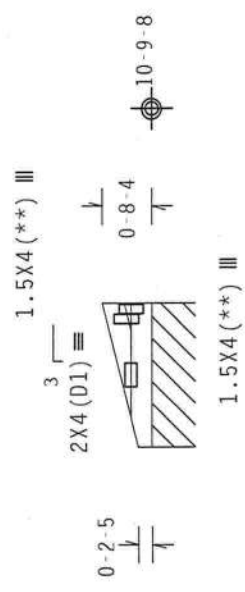
110 mph wind, 11.28 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Dead loads are stated on projected horizontal area basis.

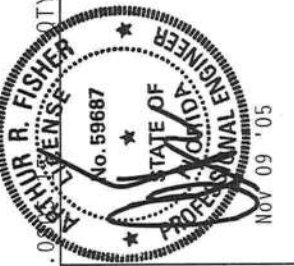
**SPECIAL LOADS**  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 91 PLF at 0.00 to 91 PLF at 1.98  
 BC - From 20 PLF at 0.00 to 20 PLF at 1.98

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

(\*\*) Plate relocated as shown.



1-11-12 Over Continuous Support  
 R=111 PLF U=91 PLF W=1-11-12



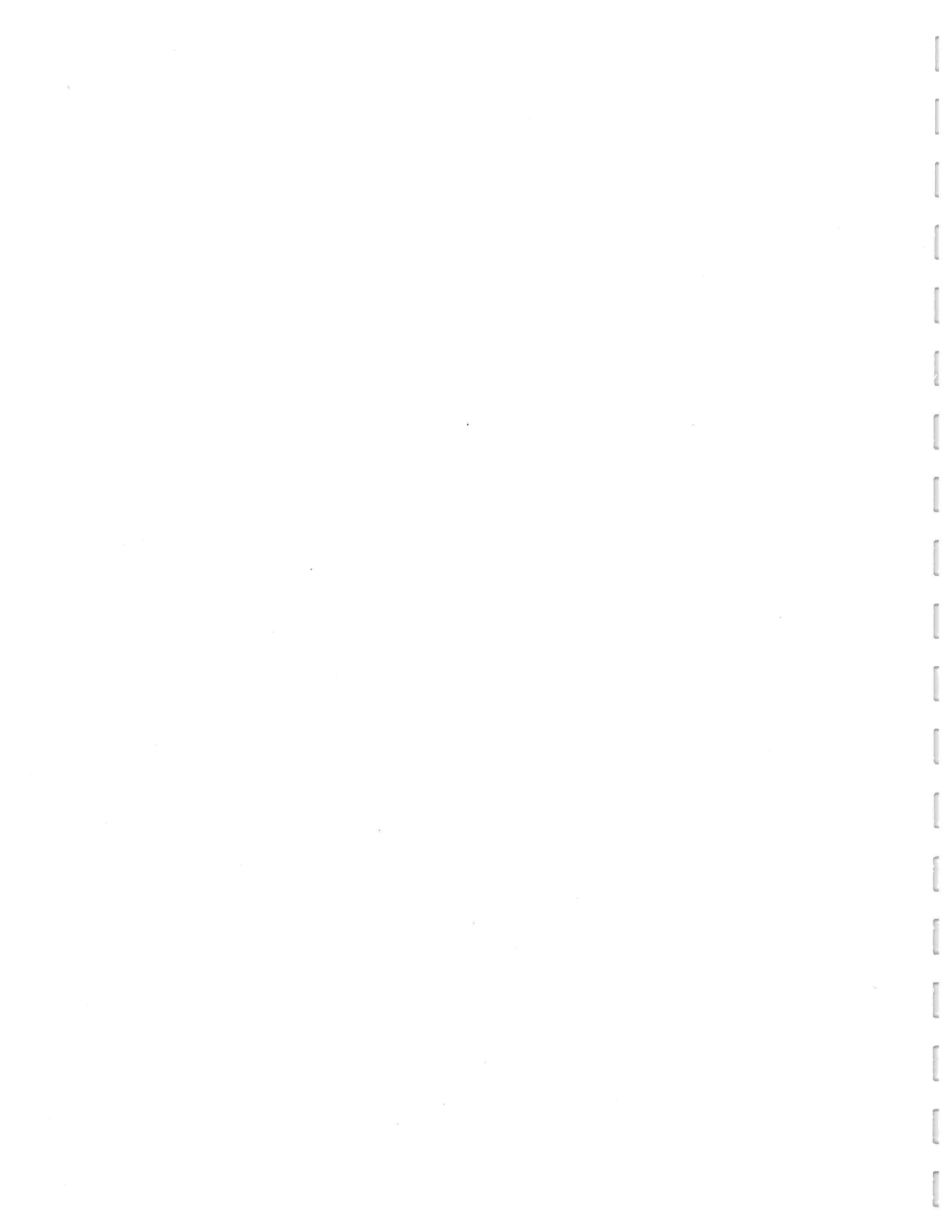
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487-- 35521	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313131	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 161368	
SPACING	24.0"	JREF- 1SS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND TRCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LB, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE RESPONSIBILITY TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING. HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/19/16GA (4-H/5/8) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

**Alpine Engineered Products, Inc.**  
 1350 Manley Drive  
 Gaines City, FL 32644  
 Fl. Certificate of Authorization # 567



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

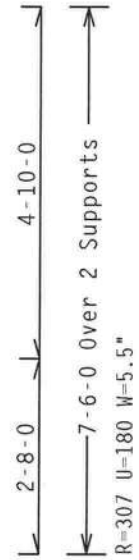
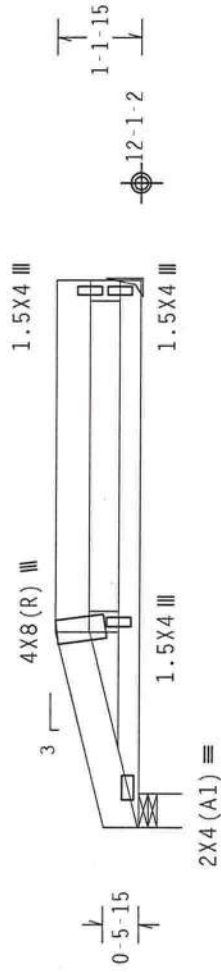
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

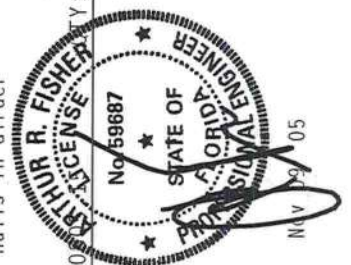
#1 hip supports 2-8-0 jacks with no webs.



R=322 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	FL/-/3/-/-R/-	Scale = .375" /Ft.
TC LL	20.0 PSF	REF R487-- 35522
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUR487 05313132
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 126181
DUR.FAC.	1.25	JREF- ISS0487_Z01
SPACING	16.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 983 D'ONOFREO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. CONNECTOR PLATES ARE MADE TO THE PROVISIONS OF MISC. (NATIONAL DESIGN SPEC. BY AIA/PVA) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, APPLY TO BOTH TOP AND BOTTOM CHORDS. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3 :W2 2x4 SP #2 Dense:

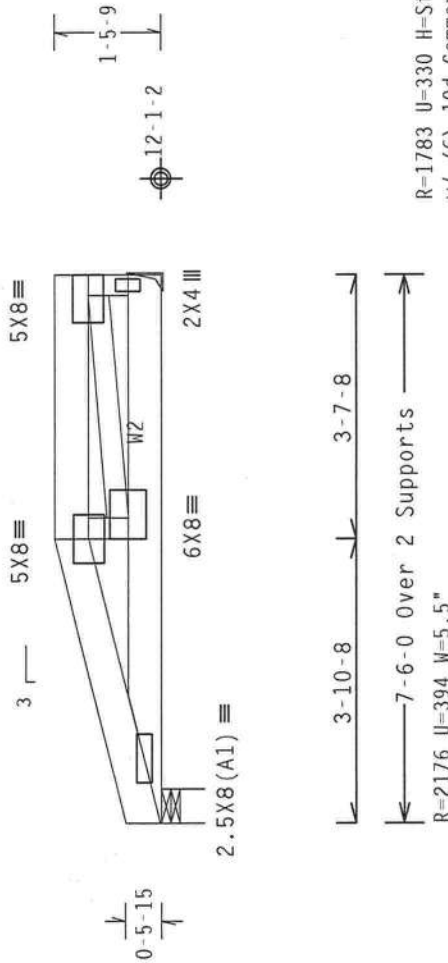
110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 20 PLF at 7.50  
 BC - From 20 PLF at 0.00 to 20 PLF at 7.50  
 BC - 663 LB Conc. Load at 0.90  
 BC - 821 LB Conc. Load at 2.23  
 BC - 685 LB Conc. Load at 3.56  
 BC - 555 LB Conc. Load at 4.90, 6.23

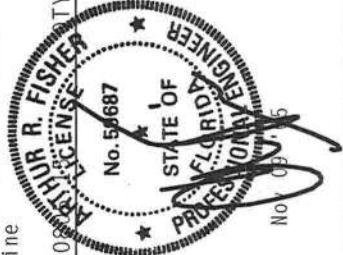
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1783 U=330 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

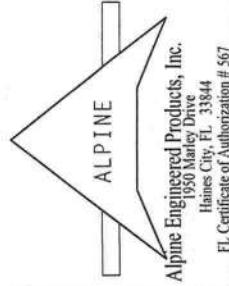
Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave	FL / - / 3 / - / R / -	Scale = .375" / Ft.
REF	R487--	35523
DATE	11/09/05	
DRW	HCUSR487	05313133
HC-ENG	JB/AF	
SEQN-	126258	
DUR.FAC.	1.25	
SPACING	24.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D-ONOFREO DR., SUITE 200, MADISON, WI 53719) AND METCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (A/H/S/K) ASTM A653 GRADE 40/60 (N. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





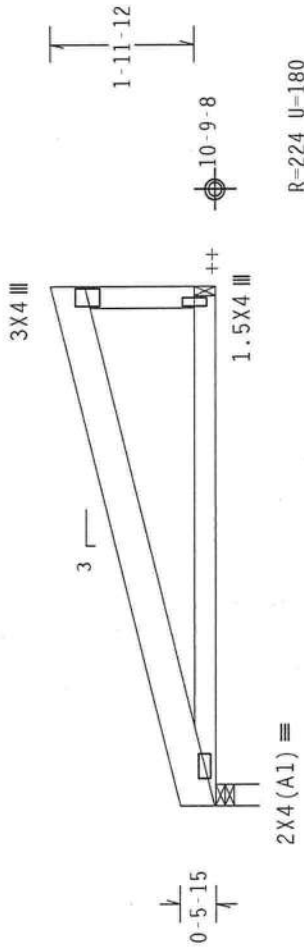
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

++ ANCHORAGE REQ'D TO PREVENT TRUSS FROM SLIPPING OFF BEARING.

110 mph wind, 12.17 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



0-1-8

← 7-1-4 Over 2 Supports →  
 R=207 U=180 W=3.5"

Design Crit: TPI-2002 (STD) / FBC

Cq/RT=1.00(1.25)/10(0)

7.04.08

FL/-/3/-/R/-

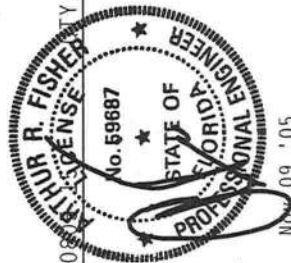
Scale = .375" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563 D'ONDRETT DR., SUITE 206, MADISON, MI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLER. THE ACCEPTANCE OF THE TRUSS DESIGN BY THE CONTRACTOR SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE ACCEPTANCE OF THE TRUSS DESIGN BY THE CONTRACTOR SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE ACCEPTANCE OF THE TRUSS DESIGN BY THE CONTRACTOR SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.



Alpine Engineered Products, Inc.  
 1950 Mainway Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



TC LL	20.0 PSF	REF	R487--	35524
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313052
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	45.0 PSF	SEQN-	160799	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	16.0"			



Top chord 2x6 SP #2  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

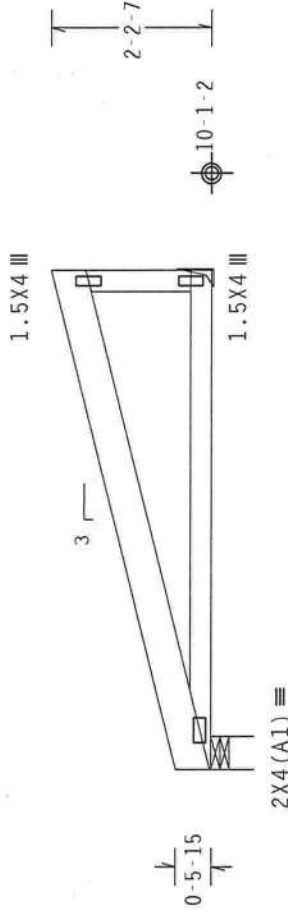
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 11.44 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 16.0" OC maximum.



6-10-0 Over 2 Supports  
R-204 U=180 W=5.5"

R-210 U=180 H=Simpson LU24  
w/ (2) 10d, 0.148"x1.5" nails in Truss  
w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave

Scale = .375" / Ft.

FL / - / 3 / - / - / R / -

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

REF R487 -- 35525

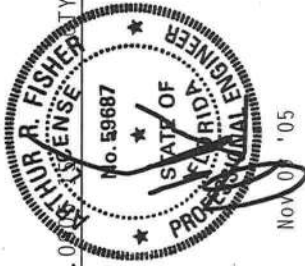
DATE 11/09/05

DRW HCUSR487 05313053

HC-ENG JB/AF \*

SEQN- 160810

JREF- 1SS0487\_Z01



ALPINE ENGINEERED PRODUCTS, INC. IS THE INSTALLATION CONTRACTOR. THE RESPONSIBILITY FOR THE DESIGN AND FABRICATION OF THIS TRUSS IS THE RESPONSIBILITY OF THE DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC.  
1950 W. Highway 1  
Haines City, FL 33844  
FL Certificate of Authorization # 567



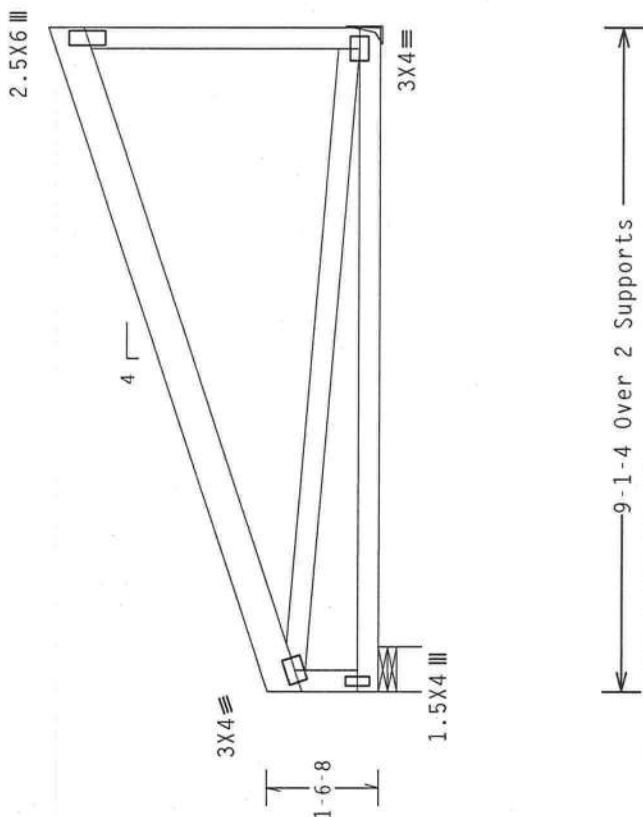
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 13.15 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.



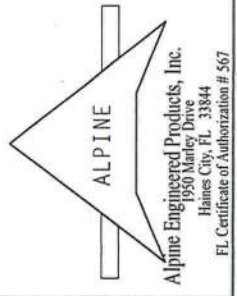
R-263 U=180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

PLT TYP. Wave	Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)	7.04.0	TY:5	FL/-/3/-/-/R/-	Scale = .375" /Ft.
REF R487 --	20.0 PSF	TC LL	11/09/05	DATE	35526
DRW HCUSR487 05313054	15.0 PSF	TC DL	11/09/05	DATE	11/09/05
HC-ENG JB/AF	10.0 PSF	BC DL	11/09/05	DATE	11/09/05
SEQN- 160815	0.0 PSF	BC LL	11/09/05	DATE	11/09/05
JREF- 1SS0487_Z01	45.0 PSF	TOT.LD.	11/09/05	DATE	11/09/05
	1.25	DUR.FAC.	11/09/05	DATE	11/09/05
	16.0"	SPACING	11/09/05	DATE	11/09/05



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSSES, 1000 ENTERPRISE LN, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/19/166GA (W, H/25/K) ASTM A653 GRADE 40/60 (4, K/P, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2, 100B-2, 100C-2, 100D-2, 100E-2, 100F-2, 100G-2, 100H-2, 100I-2, 100J-2, 100K-2, 100L-2, 100M-2, 100N-2, 100O-2, 100P-2, 100Q-2, 100R-2, 100S-2, 100T-2, 100U-2, 100V-2, 100W-2, 100X-2, 100Y-2, 100Z-2. A SEAL OR THIS DESIGN INDICATES ACCEPTANCE OF THE PROJECT AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.





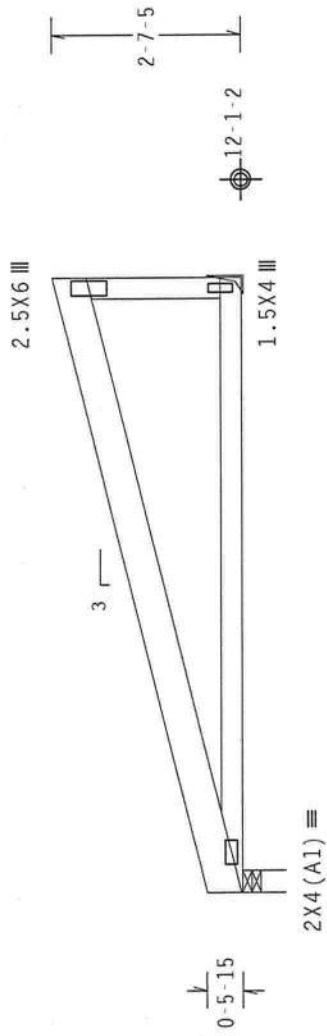
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 13.64 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind IC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

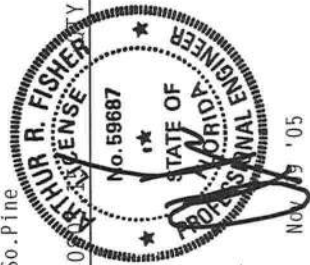


8-5-8 Over 2 Supports  
 R=249 U=180 W=3.5"

R=264 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	FL/-/3/-/-/R/-	Scale = .375" /Ft.
TC LL	20.0 PSF	REF R487-- 35527
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313055
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	45.0 PSF	SEQN- 126915
DUR.FAC.	1.25	
SPACING	16.0"	JREF- 1SS0487_Z01



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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (H, K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



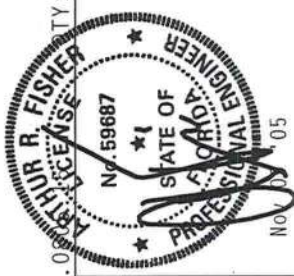
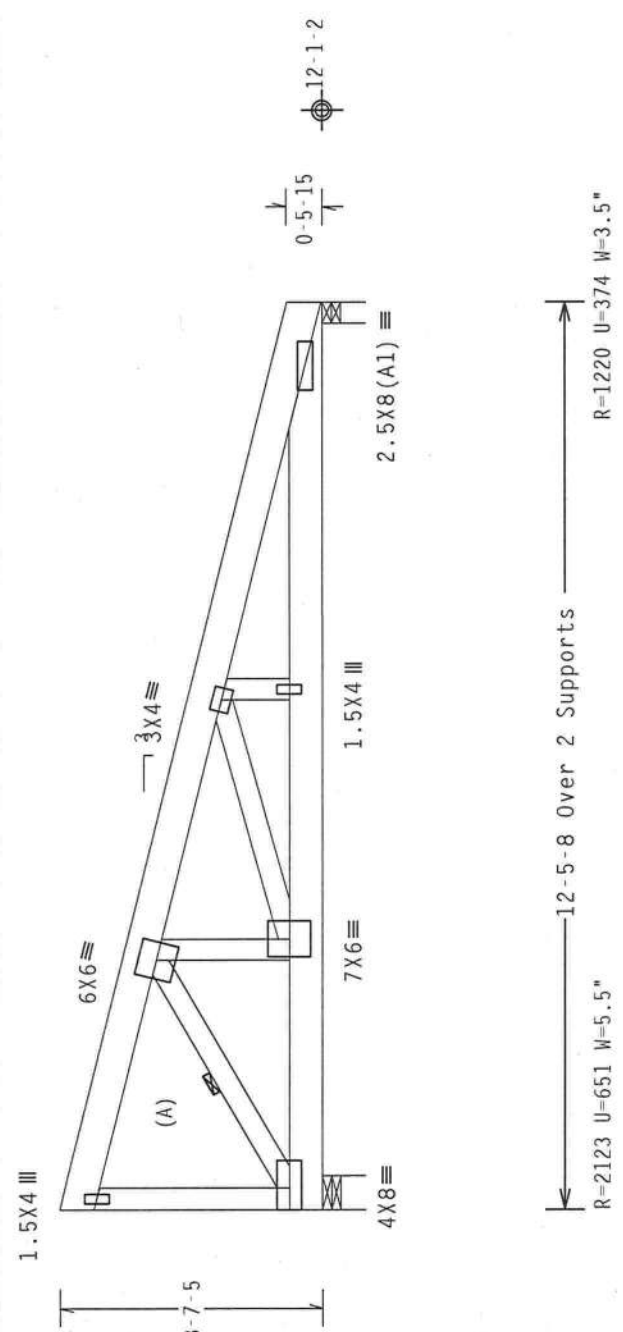
Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Left end vertical not exposed to wind pressure.

**SPECIAL LOADS**  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 12.46  
 BC - From 20 PLF at 0.00 to 20 PLF at 12.46  
 BC - 478 LB Conc. Load at 2.60  
 BC - 1733 LB Conc. Load at 3.94

(A) Continuous lateral bracing equally spaced on member.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave	Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)	7.04.00	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
			TC LL	20.0 PSF
			TC DL	15.0 PSF
			BC DL	10.0 PSF
			BC LL	0.0 PSF
			TOT.LD.	45.0 PSF
			DUR.FAC.	1.25
			SPACING	24.0"
			REF	R487 - - 35528
			DATE	11/09/05
			DRW	HCUSR487 05313134
			HC-ENG	JB/AF
			SEQN-	126941
			JREF-	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53719) AND NITCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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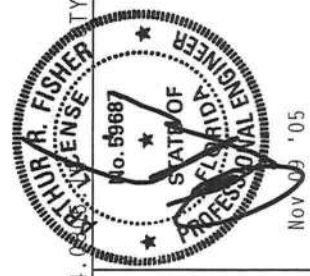
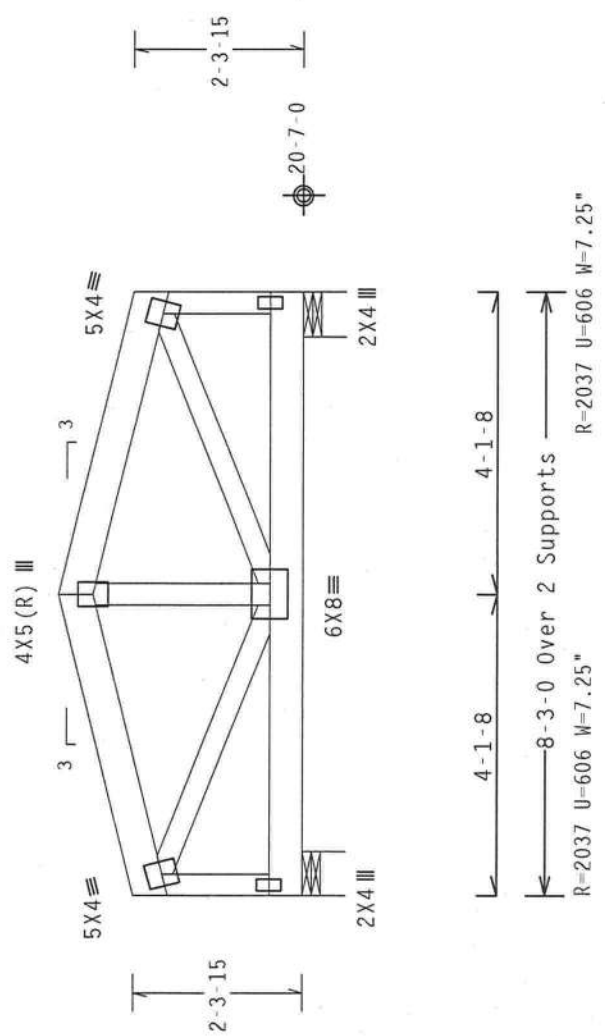


110 mph wind, 23.43 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Girder supports 20-0-4 span to BC one face and 2-0-0 span to TC/BC split opposite face.

End verticals not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487--	35529
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313135
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	161213	
DUR.FAC.	1.25	JREF-	1SS0487_Z01	
SPACING	24.0"			

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**\*\*IMPORTANT\*\*** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/F) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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110 mph wind, 23.16 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

End verticals not exposed to wind pressure.  
See DWGS A11030EC1103 & GBLLETIN0405 for more requirements.  
Fasten rated sheathing to one face of this frame.

Dead loads are stated on projected horizontal area basis.

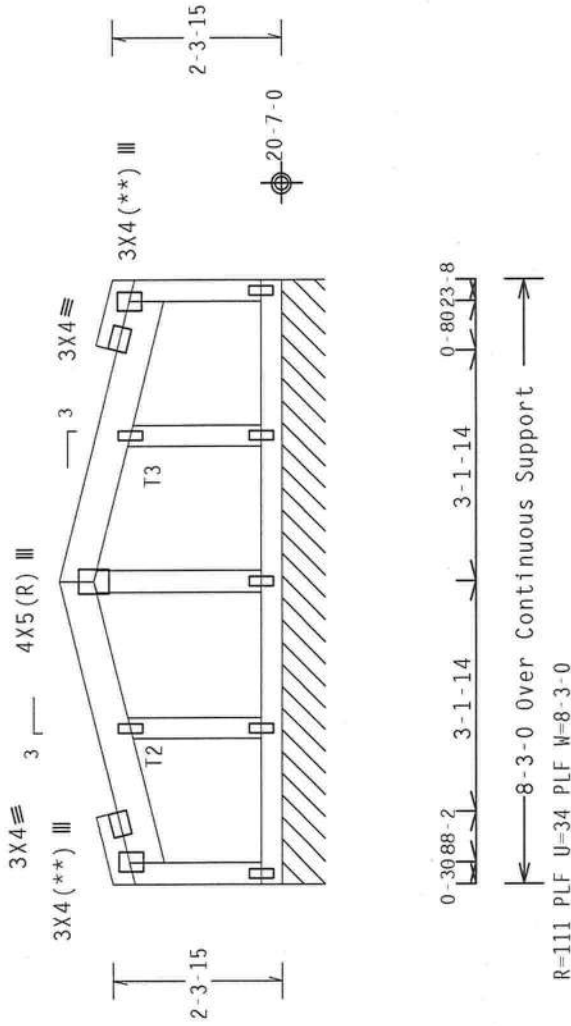
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

(\*\*) Plate relocated as shown.

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:  
Bot chord 2x4 SP #2 Dense  
Webs 2x4 SP #3

**SPECIAL LOADS**

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
TC - From 91 PLF at 0.00 to 91 PLF at 8.25  
BC - From 20 PLF at 0.00 to 20 PLF at 8.25



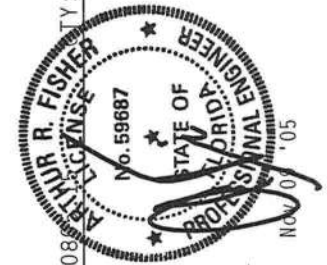
Note: All Plates Are 1.5x4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.04.08

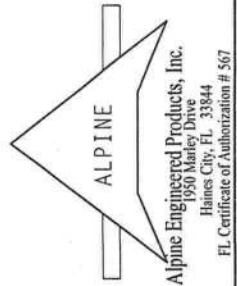
Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 -- 35530
TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313136
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	45.0 PSF	SEQN	161359
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1SS0487_Z01



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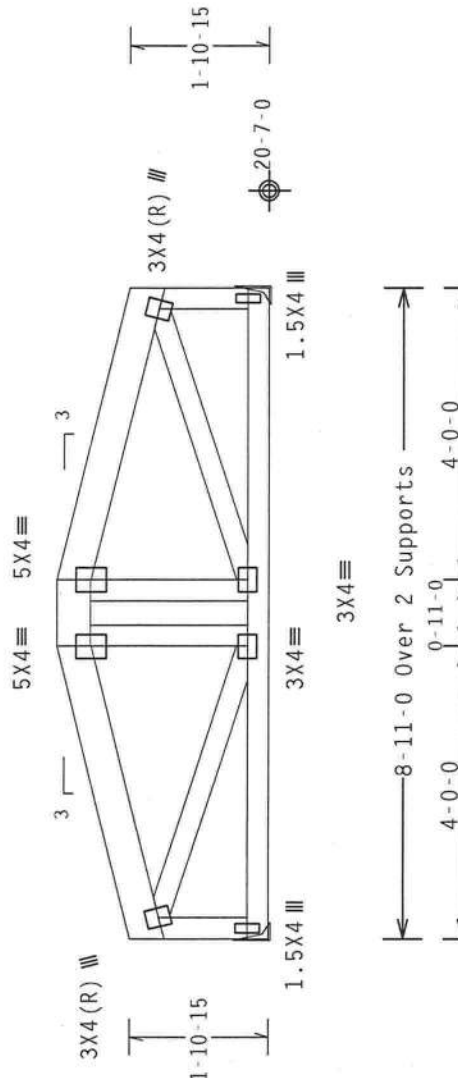
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.  
 #1 hip supports 4-0-0 jacks with no webs.

110 mph wind, 22.99 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

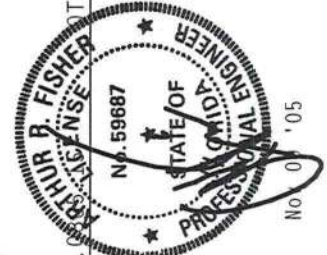


R-439 U-180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

R-439 U-180 H-Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375"/Ft.
TC DL	15.0 PSF	REF R487--	35531
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313137
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	161125
SPACING	16.0"	JREF-	1SS0487_701



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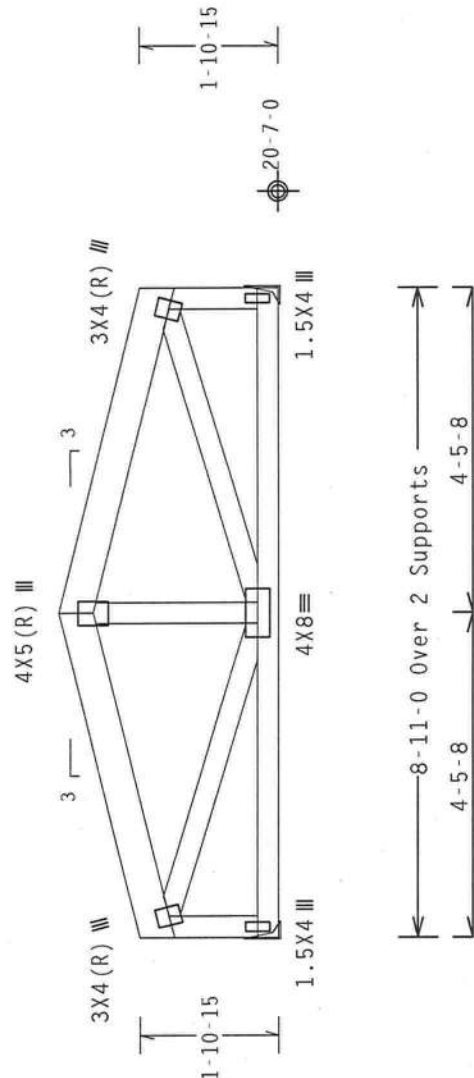
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 23.05 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 Live and L/240 total load. Creep increase factor for dead load is 1.50.



R=270 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

R=270 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X4 min. So.Pine

PLT TYP. Wave  
 Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 - -	35532
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313056
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	161132
SPACING	16.0"	JREF-	ISS0487_Z01



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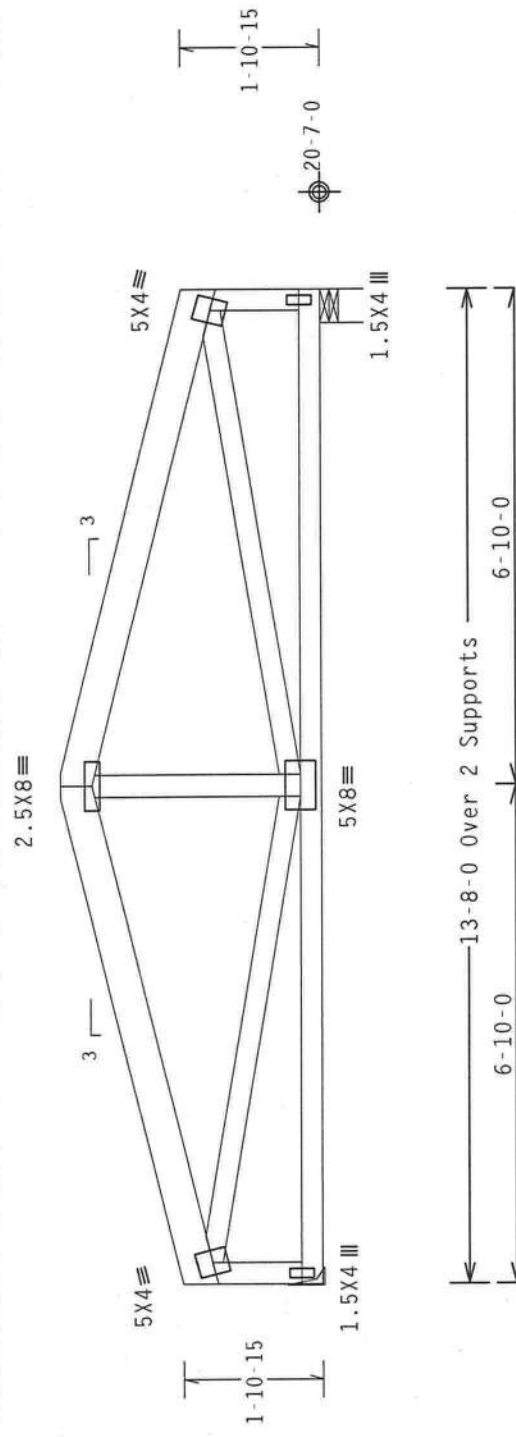
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.  
 #1 hip supports 6-8-0 jacks with no webs.

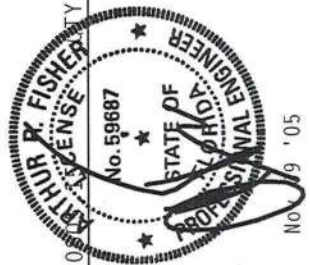
110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



R=962 U=286 H=Simpson LUS26  
 w/ (4) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

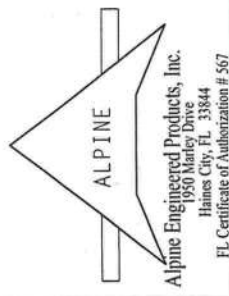
Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 -- 35533
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313138
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 161087
DUR.FAC.	1.25	JREF- 1SS0487_Z01
SPACING	16.0"	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIAA) ONLY. STEEL APPLY CONNECTOR PLATES PER DRAWING 200/10/10 UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

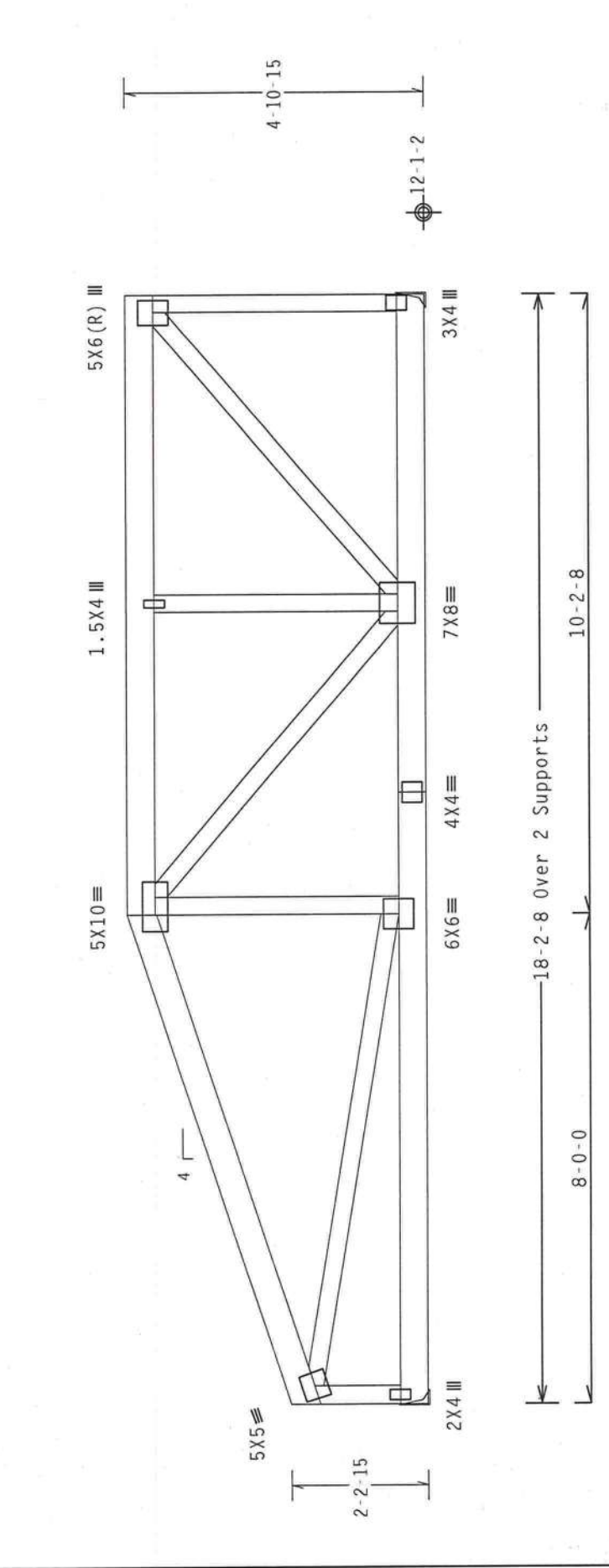




Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.  
 #1 hip supports 8-0-0 jacks with no webs.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1436 U=366 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=1986 U=505 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave

<p>Scale = .375" / Ft.</p> <p>REF R487 -- 35534</p> <p>DATE 11/09/05</p> <p>DRW HCUSR487 05313139</p> <p>HC-ENG JB/AF</p> <p>SEQN- 160948</p> <p>JREF- 1SS0487_Z01</p>	
TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RES 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDORF DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/10/16GA (4-N/S/S) ASH A563 GRADE 40/60 (N. K70/S) GALV. STEEL. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, THE DESIGN PER DRUGS SHALL APPLY. FOR MORE INFORMATION ON THE TPI TRUSS SYSTEM, VISIT US AT WWW.TPI.COM OR CALL 1-800-368-7263. THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

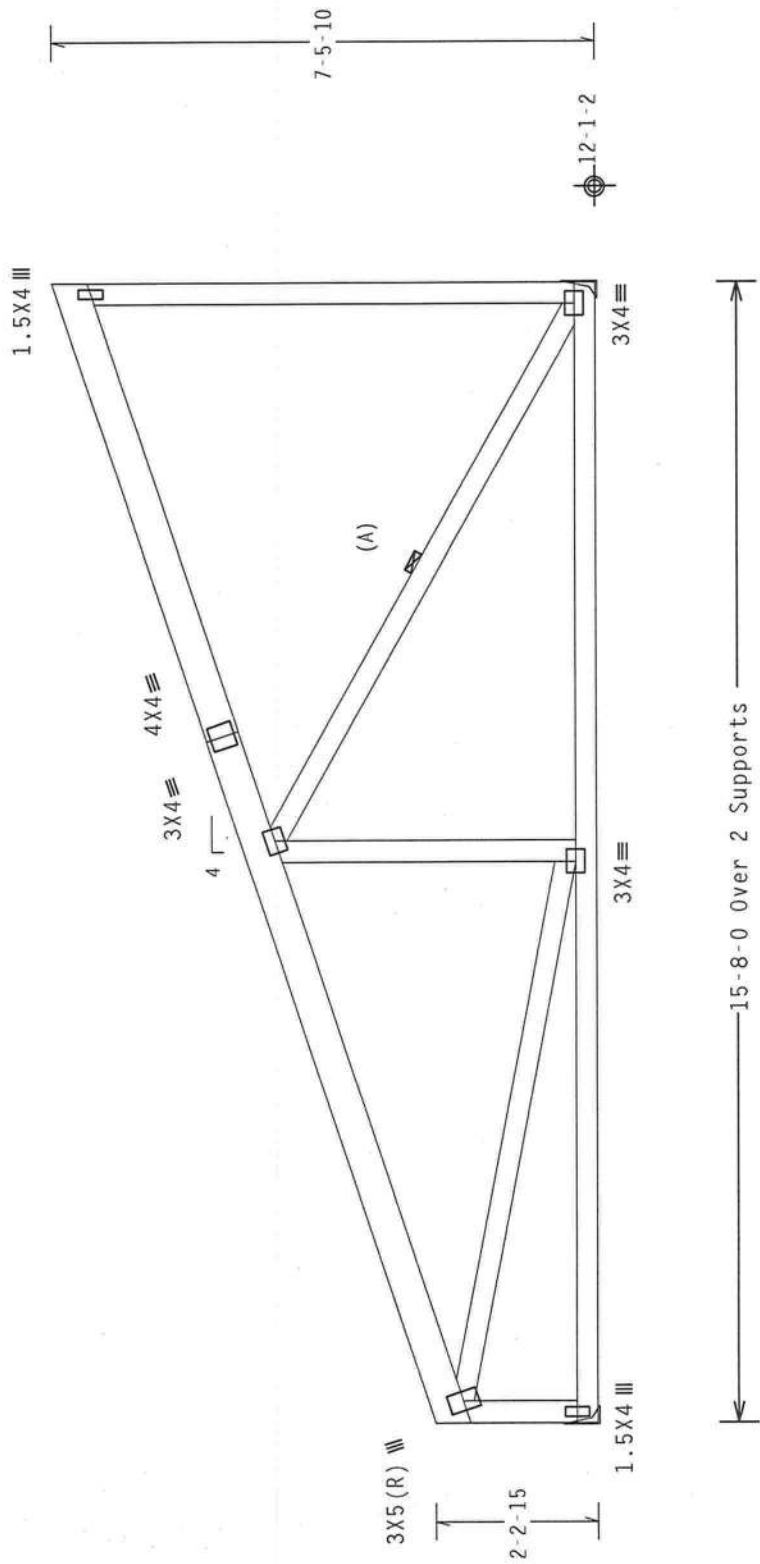
(A) Continuous lateral bracing equally spaced on member.

Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 16.95 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

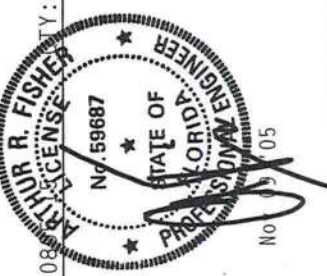
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=475 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=482 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine



TY:5	FL/-/3/-/3/-/R/-	Scale = .375" /Ft.
TC LL	20.0 PSF	REF R487-- 35535
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313140
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN- 160964
DUR.FAC.	1.25	JREF- 1SS0487_Z01
SPACING	16.0"	

PLT TYP. Wave Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 7.04.08

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE CODES AND STANDARDS. PROFESSIONALS MUST BE RESPONSIBLE FOR VERIFYING THAT THE TRUSS DESIGN CONFORMS WITH APPLICABLE CODES AND STANDARDS. UNLESS OTHERWISE INDICATED, APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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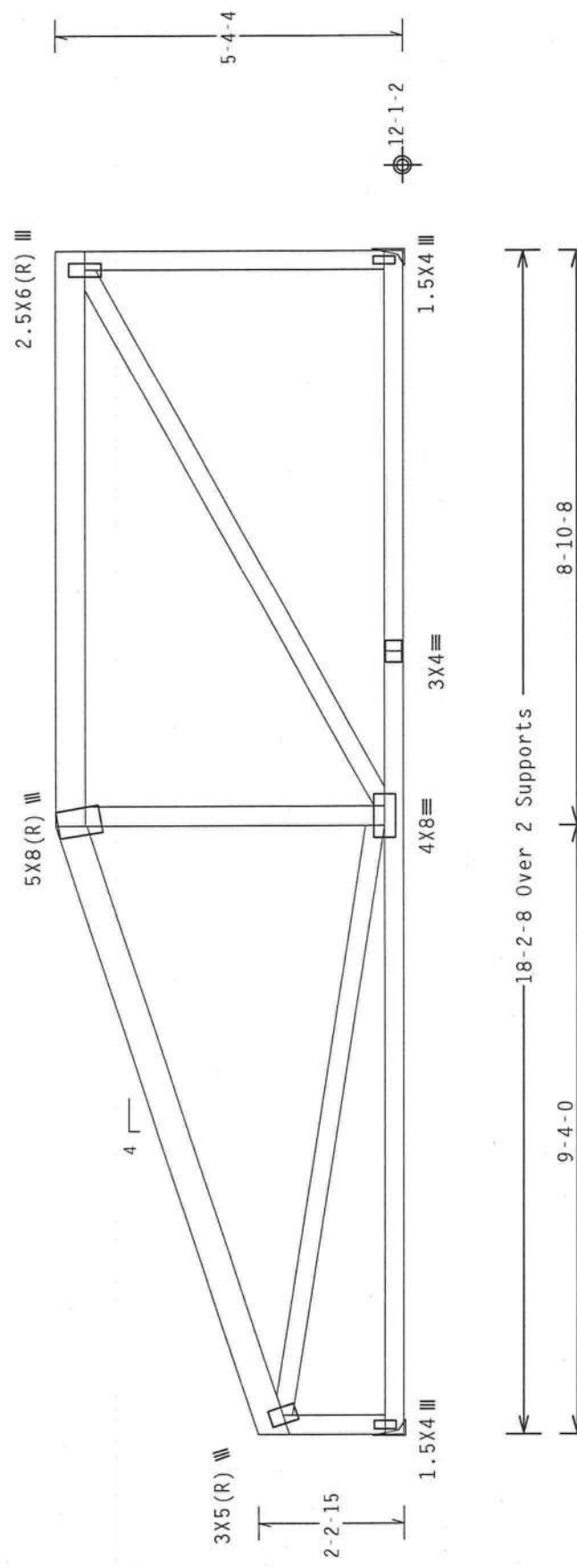
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.89 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4-50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

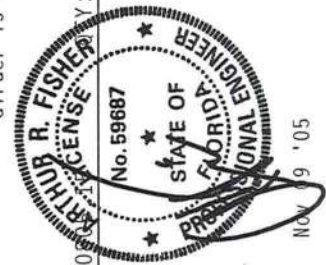


R=561 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=551 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF	REF	R487--	35536
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313141
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	160952	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND NCCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBC (NATIONAL DESIGN SPEC. BY ATAPA) AND TPI LEVEL 4 REQUIREMENTS. ALL TRUSSES AND TRUSS PLATES TO BE INSTALLED IN ACCORDANCE WITH THE POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

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PLT TYP. Wave

Scale = .375"/Ft.



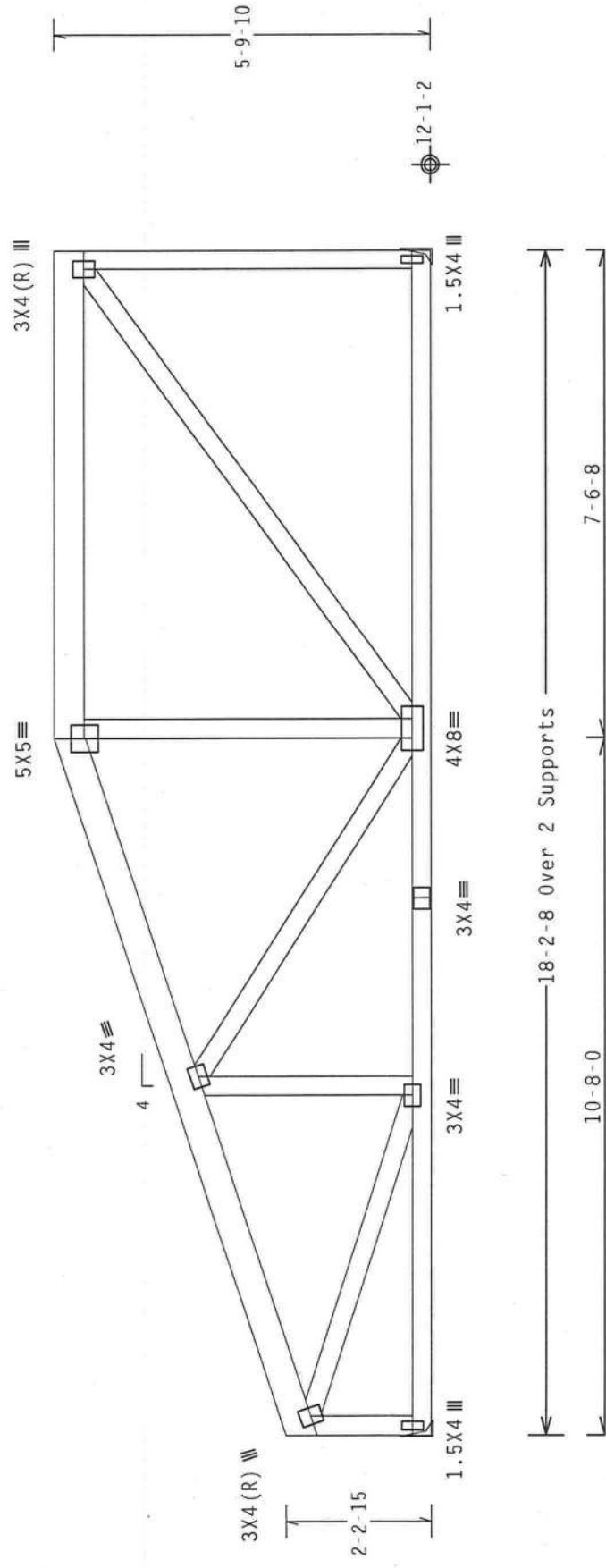
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 16.12 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

End verticals not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=558 U-180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=554 U-180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 --	35537
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313142
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	160956
SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ORFELD DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGNER CONFIRMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND THE AISC APPROVED CONNECTIONS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN, POSITION PER DRAWINGS 1004-2, PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1004-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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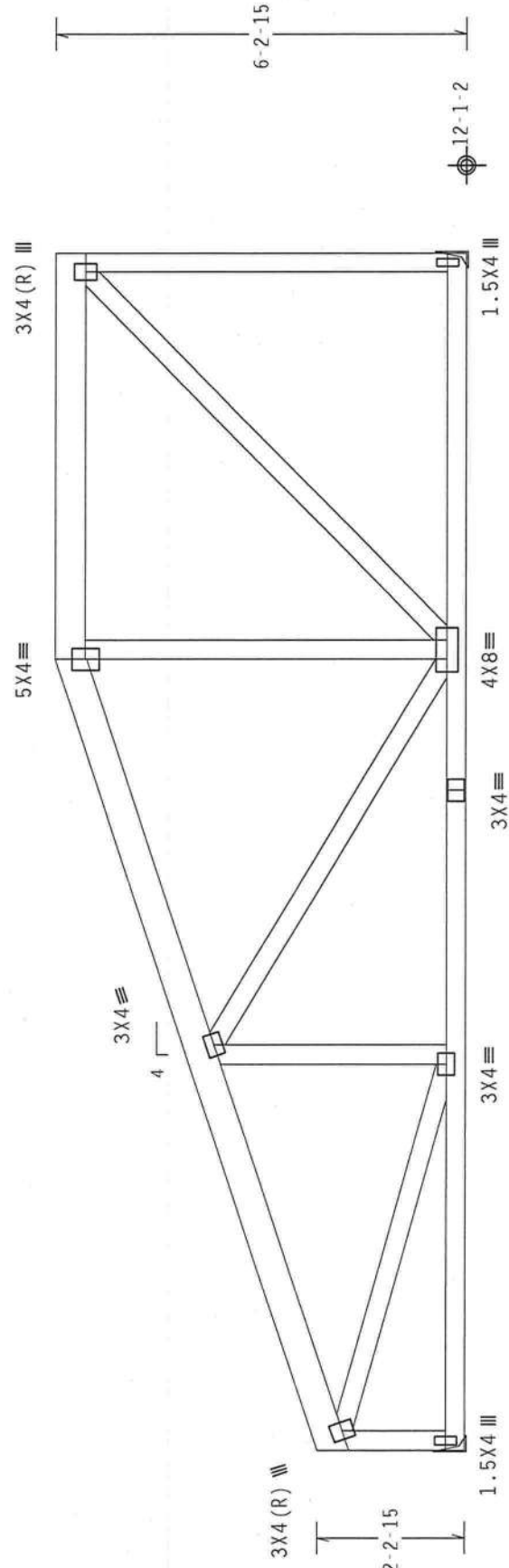
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.34 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



18-2-8 Over 2 Supports  
 12-0-0  
 6-2-8

R-558 U-180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R-554 U-180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 --	35538
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313143
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	160960
SPACING	16.0"	JREF-	1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 W. WASHINGTON AVENUE, SUITE 100, MILWAUKEE, WI 53219) FOR SAFETY PRACTICES. TOP CHORD BRACING AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/76GA (W, H/S/P) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.

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PLT TYP. Wave



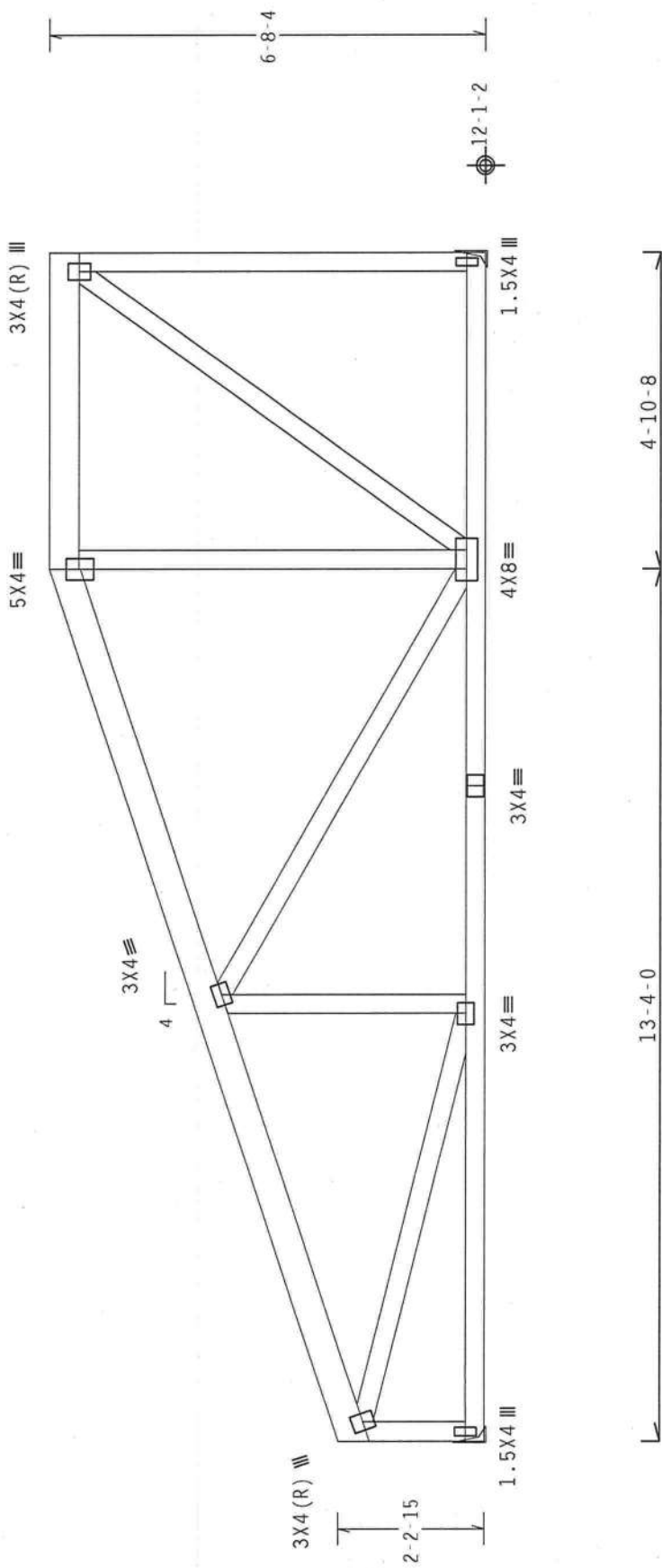
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.56 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



18-2-8 Over 2 Supports

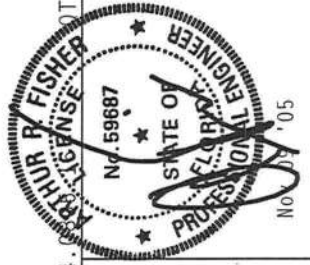
R-558 U=180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04

R-554 U=180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Scale = .375"/Ft.

PLT TYP. Wave



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE SPECIFICATIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CORRECTORS ARE MADE OF 2018/1606 (4x 1/2" X 5/8" ASTM A575 GRADE 40/50/60 K/18.5) GALV. STEEL. APPLY CORRECTOR PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487--	35539
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313144
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEQN-	160972	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	

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 1950 Manley Drive  
 Haines City, FL 33844  
 FL Certificate of Authorization # 567



Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

End verticals not exposed to wind pressure.

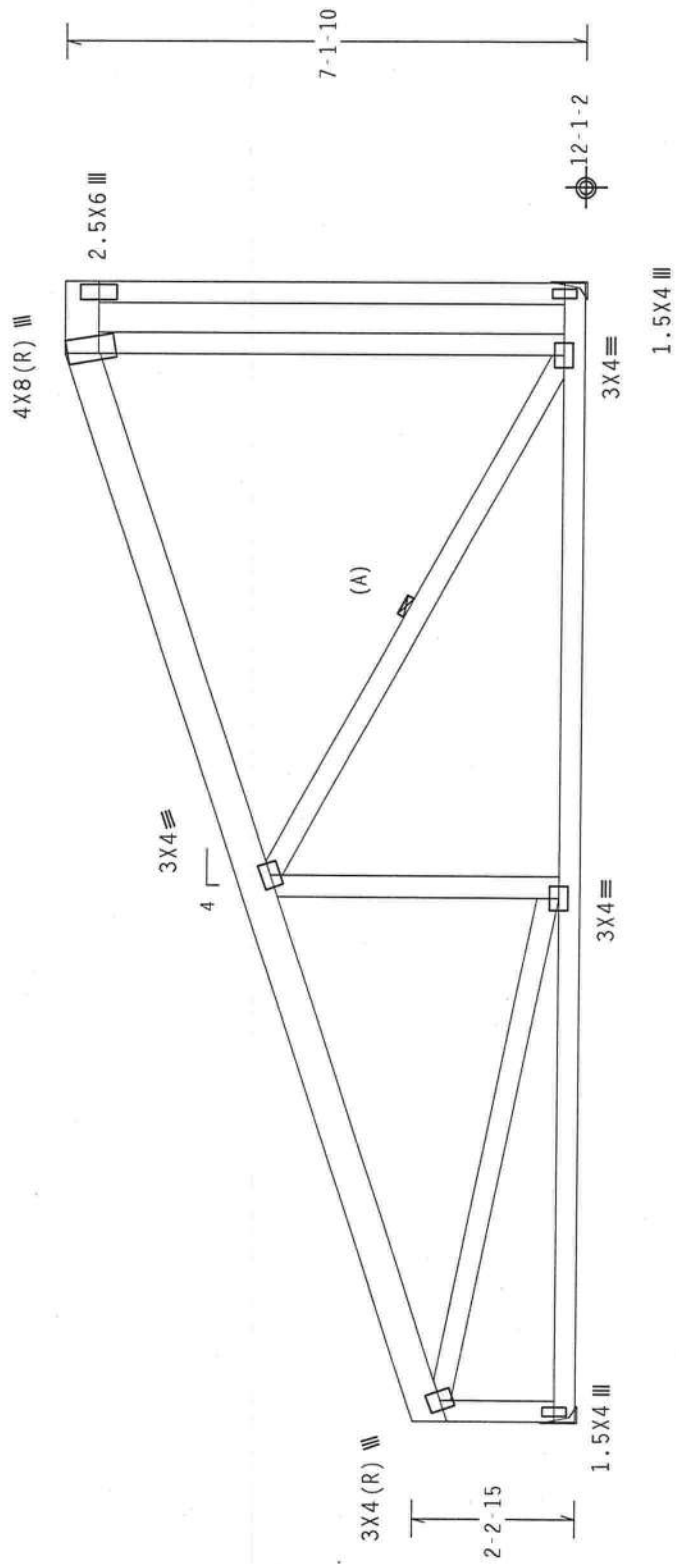
(A) Continuous lateral bracing equally spaced on member.

Trusses to be spaced at 16.0" OC maximum.

110 mph wind, 16.78 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=482 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

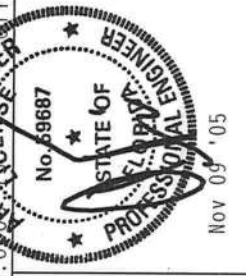
PLT TYP. Wave  
 Design Crit: TPI-2002(STD)/FBC  
 Cg/RT=1.00(1.25)/10(0) 7.04.0

R=475 U=180 H=Simpson LU24  
 w/ (2) 10d, 0.148"x1.5" nails in Truss  
 w/ (4) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

TC LL	20.0 PSF	REF	R487-- 35540
TC DL	15.0 PSF	DATE	11/09/05
BC DL	10.0 PSF	DRW	HCUSR487 05313145
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	45.0 PSF	SEQN-	160984
DUR.FAC.	1.25		
SPACING	16.0"	JREF-	1SS0487_201

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 BUILDING SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53761) OR THE NATIONAL COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (4-N/52K) ASTM A653 GRADE 40/60 (4, 4/M.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. DRAMA INSPECTOR OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE QUALITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



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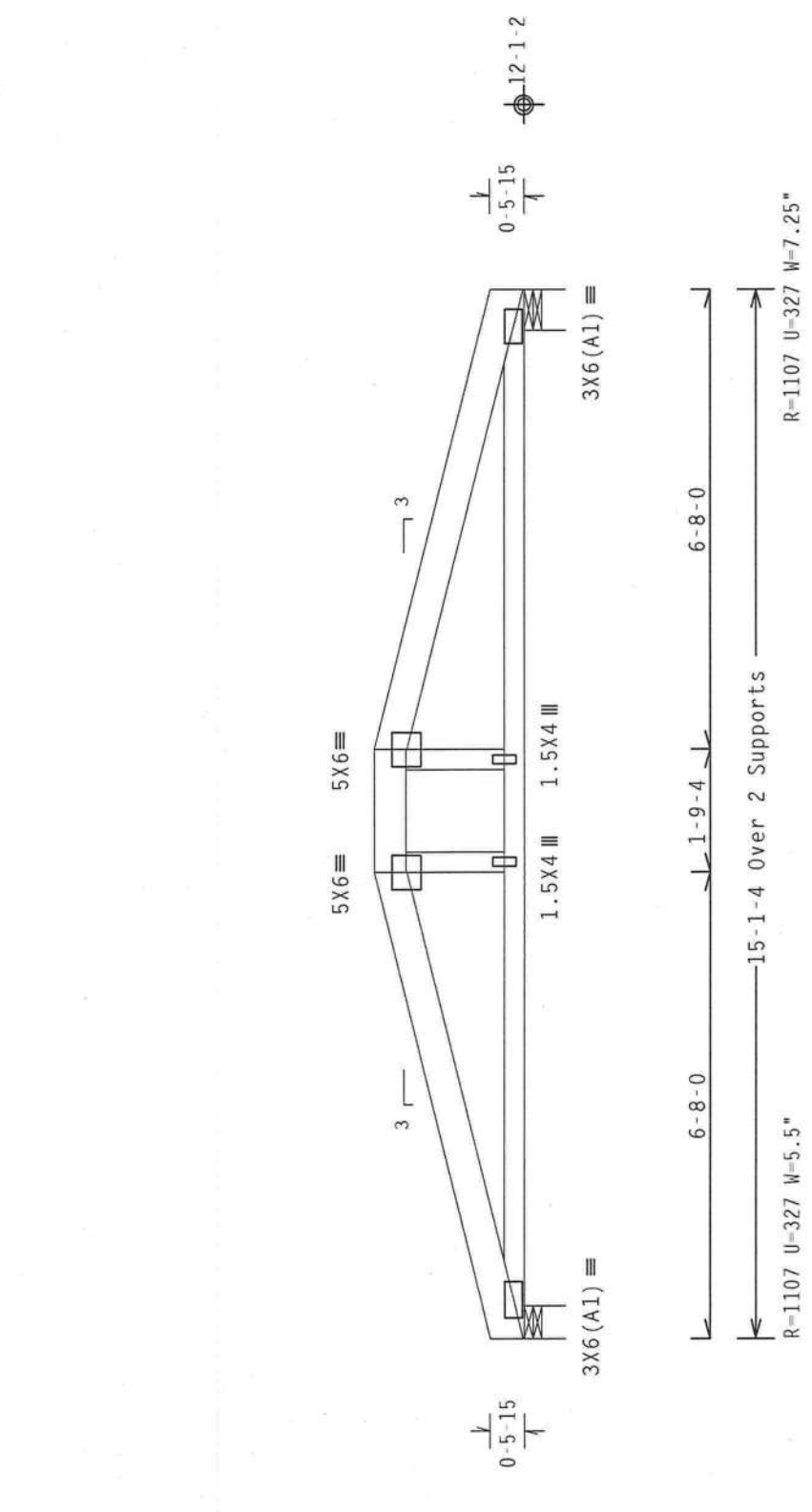
Top chord 2x6 SP #1 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, Located anywhere in roof, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 6-8-0 jacks with no webs.



PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.08  
 Scale = .375"/Ft.  
 REF R487-- 35541  
 DATE 11/09/05  
 DRW HCUSR487 05313146  
 HC-ENG JB/AF  
 SEQN- 126485  
 DUR.FAC. 1.25  
 SPACING 16.0"

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY ATAPPA) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND JOINTS. (SEE TPI/5724) FOR TRUSS AND JOINTS. ALPINE TRUSS PLATE POSITION PER DRAWINGS 1004-7. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	FL / - / 3 / - / - / R / -
TC DL	15.0 PSF	
BC DL	10.0 PSF	
BC LL	0.0 PSF	
TOT.LD.	45.0 PSF	
DUR.FAC.	1.25	
SPACING	16.0"	

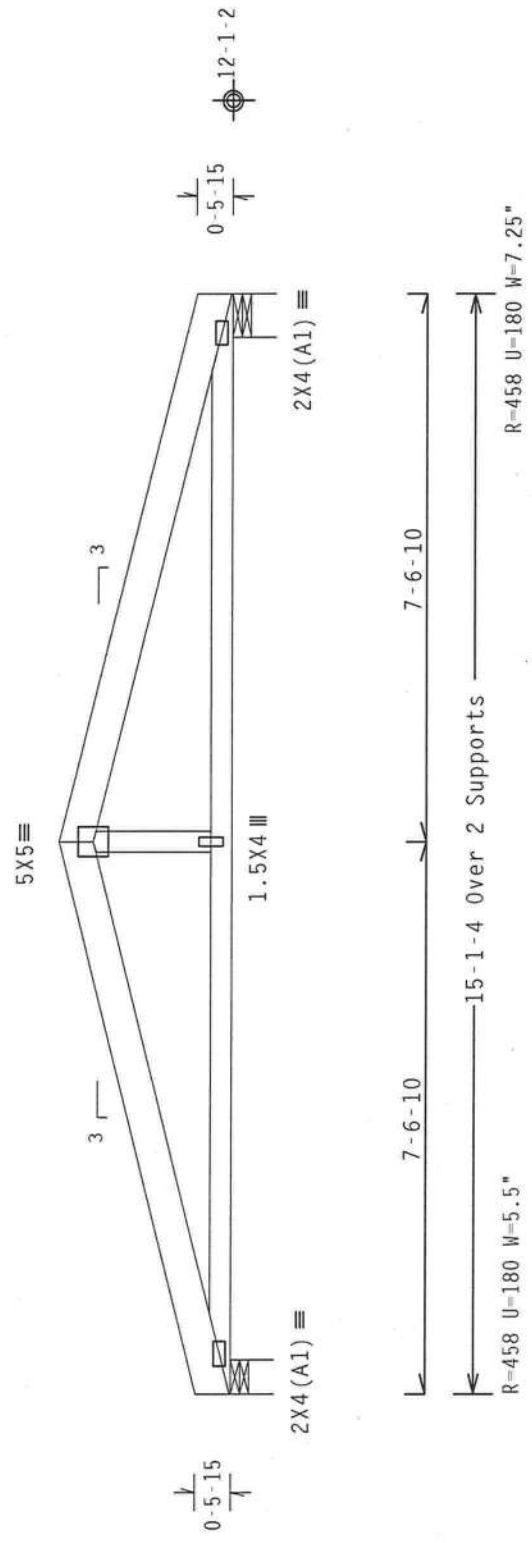


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 13.53 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase  
 factor for dead load is 1.50.



Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	Scale = .375" /Ft.
REF R487 --	35542
DATE	11/09/05
DRW	HCUSR487 05313057
HC-ENG JB/AF	*
SEQN-	126489
DUR.FAC.	1.25
SPACING	16.0"



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

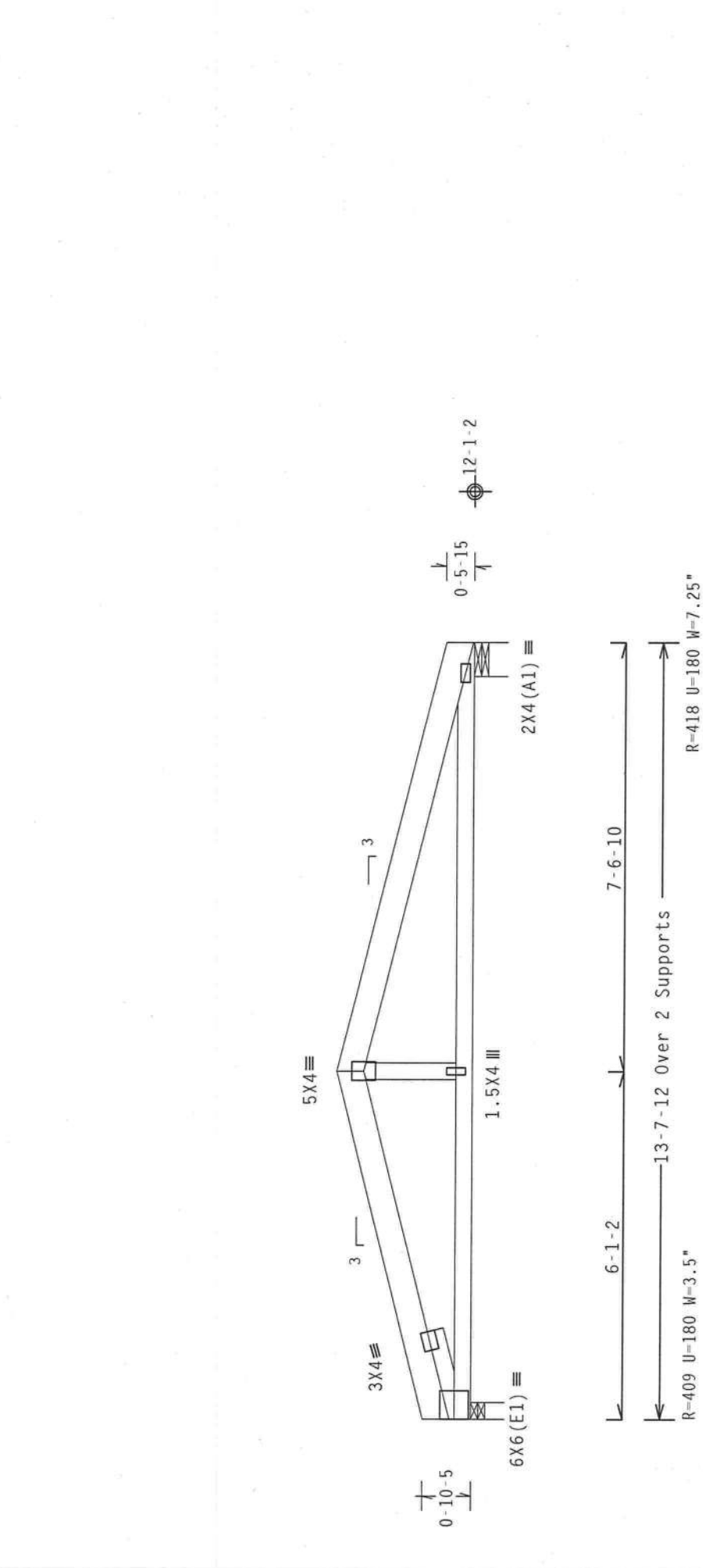
**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2019/1606A (W, H/S/S/P) ASTM A553 GRADE 40/60 (W, K/P/S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES, JOINTS, OR TRUSSES TO BE BUILT. UNLESS OTHERWISE NOTED, ALL DIMENSIONS ARE AS SHOWN. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3  
 :Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.606'  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 13.53 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
 Trusses to be spaced at 16.0" OC maximum.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

Scale = .375" / Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	REF	R487 - -	35543
TC DL	15.0 PSF		DATE	11/09/05	
BC DL	10.0 PSF		DRW	HCUSR487	05313058
BC LL	0.0 PSF		HC-ENG	JB/AF	*
TOT. LD.	45.0 PSF		SEQN	126493	
DUR. FAC.	1.25				
SPACING	16.0"		JREF	1SS0487_Z01	

ARTHUR R. FISHER  
 No. 59687  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 No. 09 '05

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**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (4-H/5/2) ASTM A653 GRADE 40/60 (4. V.H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BEING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANS/TP1 1 SEC. 2.



Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

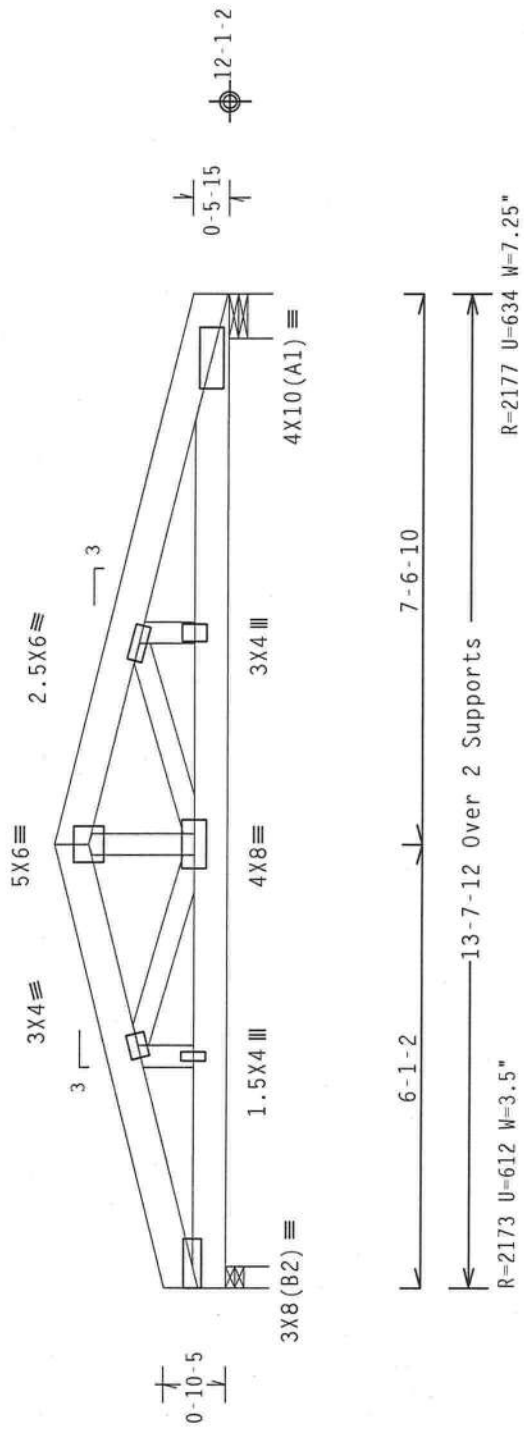
110 mph wind, 15.00 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

**SPECIAL LOADS**

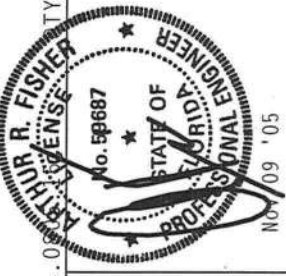
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 13.65  
 BC - From 13 PLF at 0.00 to 13 PLF at 13.65  
 BC - 354 LB Conc. Load at 0.83, 2.17  
 BC - 352 LB Conc. Load at 3.50, 4.83, 6.17, 7.50, 8.83, 10.17, 11.50, 12.83

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) / FBC  
 Cg/RT=1.00(1.25)/10(0) 7.04.0



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR SAFETY AND BRACING. THE TRUSS MANUFACTURER'S INSTRUCTIONS ARE PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'BORGOLO DR., SUITE 200, MADISON, WI 53716). THE TRUSS MANUFACTURER'S INSTRUCTIONS SHALL BE FOLLOWED UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	20.0	PSF	FL / - / 3 / - / - / R / -	Scale = .375" / Ft.
TC DL	15.0	PSF	REF R487 - -	35544
BC DL	10.0	PSF	DATE	11/09/05
BC LL	0.0	PSF	DRW	HCUSR487 05313147
TOT.LD.	45.0	PSF	HC-ENG	JB/AF
DUR.FAC.	1.25		SEQN-	126659
SPACING	16.0"		JREF-	1SS0487_Z01



Top chord 2x6 SP #1 Dense  
Bot chord 2x6 SP #1 Dense  
Webs 2x4 SP #3

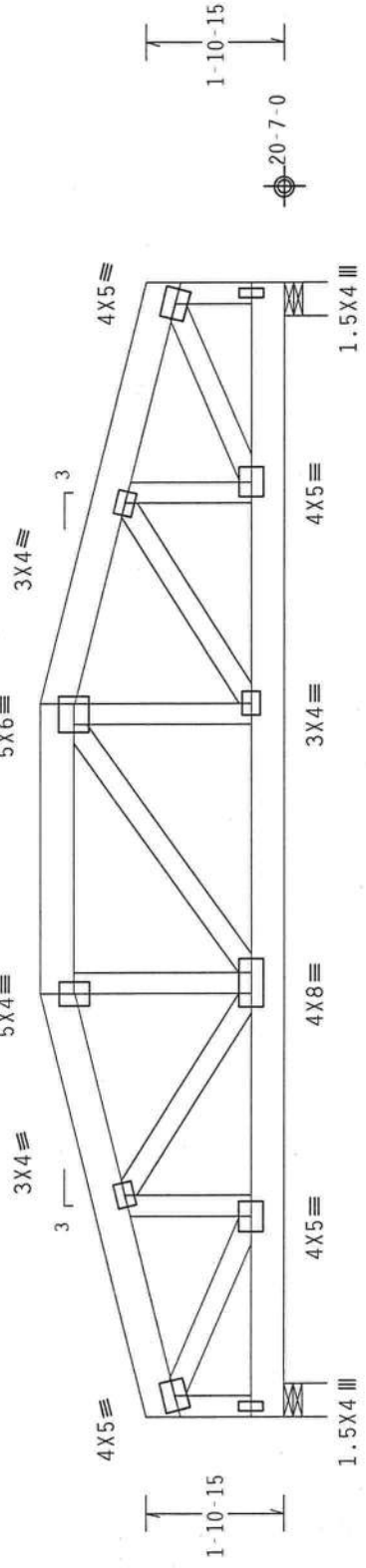
110 mph wind, 23.22 ft mean hgt, ASCE 7-98, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
TC - From 47 PLF at 0.00 to 47 PLF at 15.67  
BC - From 13 PLF at 0.00 to 13 PLF at 15.67  
BC - 184 LB Conc. Load at 1.40  
BC - 1188 LB Conc. Load at 1.94

Trusses to be spaced at 16.0" OC maximum.



5-10-0      4-0-0      5-10-0

15-8-0 Over 2 Supports

R=1685 U=509 W=5.5"

R=636 U=192 W=5.5"

Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/10(0) 7.04.08

PLT TYP. Wave

Scale = .375" / Ft.

TC LL	20.0 PSF
TC DL	15.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	45.0 PSF
DUR.FAC.	1.25
SPACING	16.0"

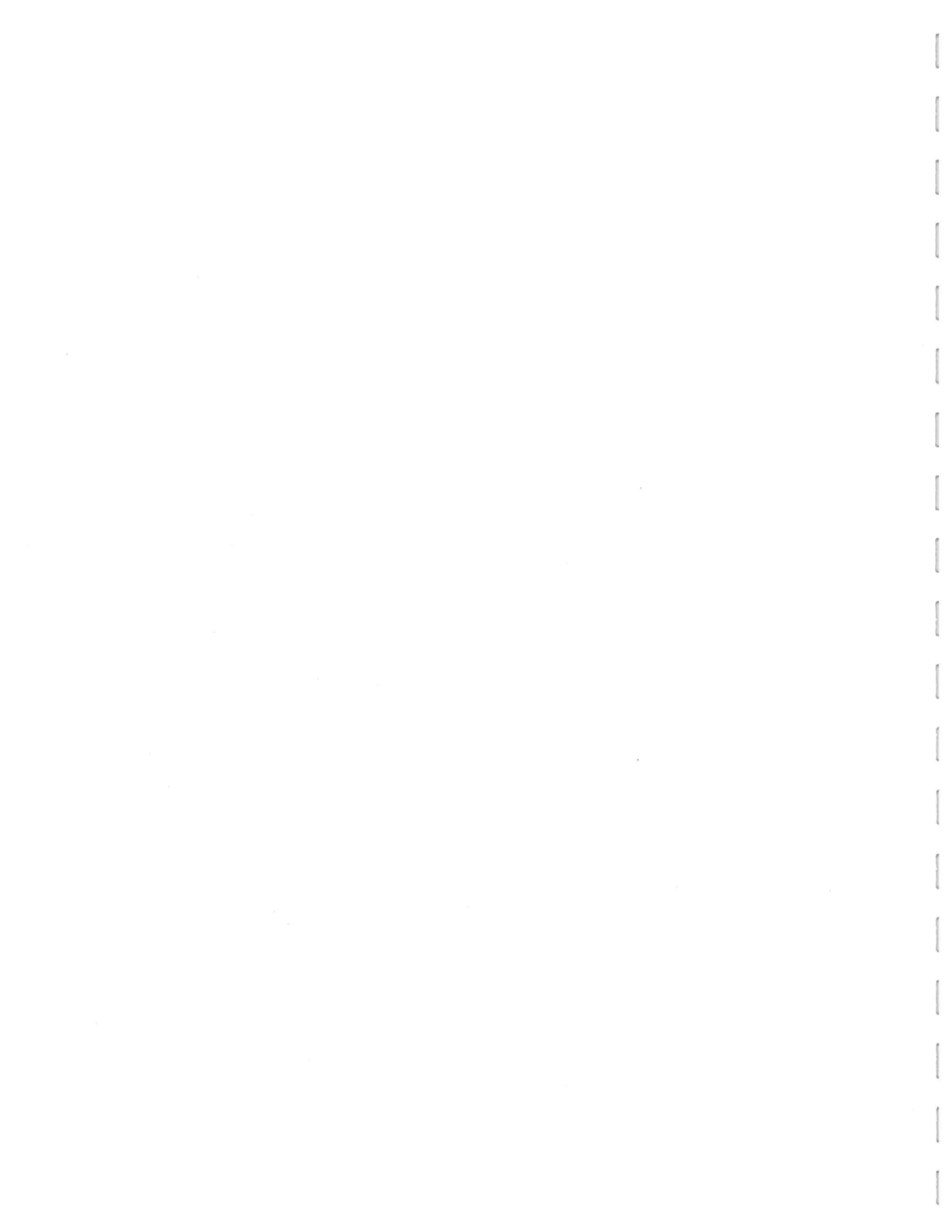


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FL Certificate of Authorization # 567

JREF - ISS0487\_Z01

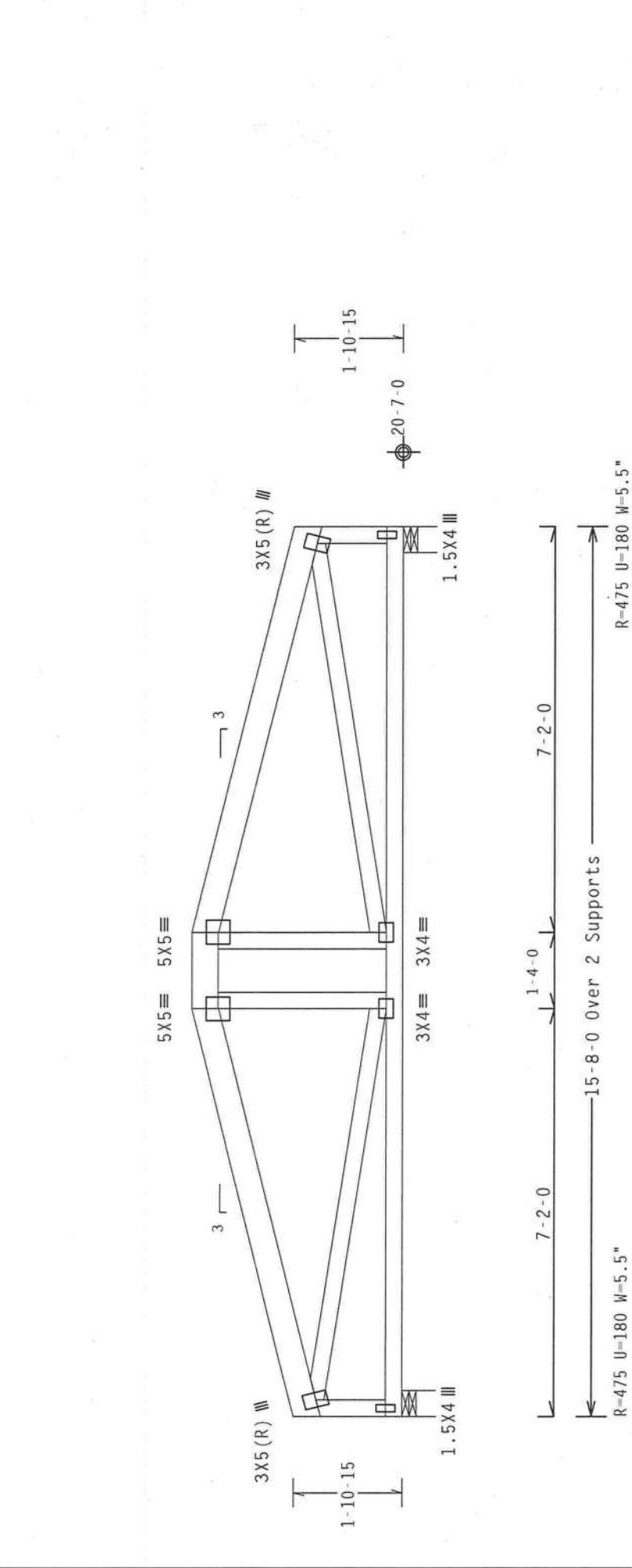


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

Trusses to be spaced at 16.0" OC maximum.

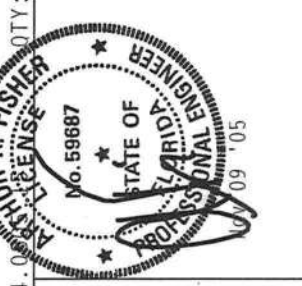
110 mph wind, 23.39 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

PLT TYP. Wave	FL / - / 3 / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 -- 35546
TC DL	15.0 PSF	DATE 11/09/05
BC DL	10.0 PSF	DRW HCUSR487 05313149
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	45.0 PSF	SEQN - 161113
DUR.FAC.	1.25	
SPACING	16.0"	JREF - 1SS0487_Z01



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**ALPINE**  
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## 2 COMPLETE TRUSSES REQUIRED

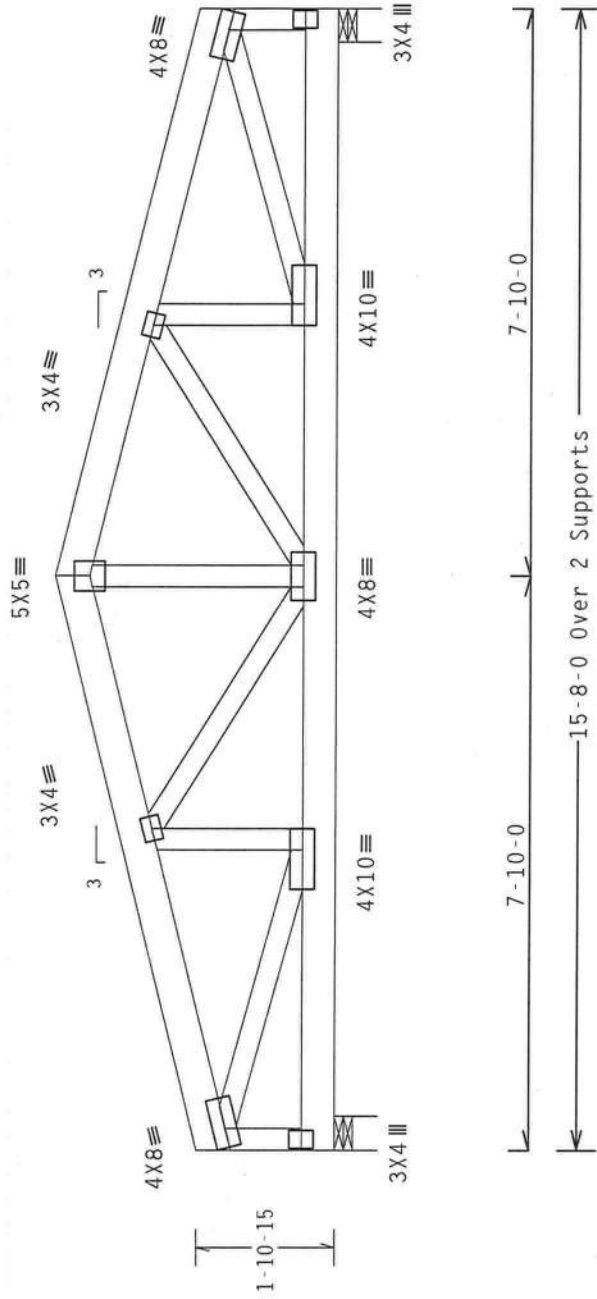
Nailing Schedule: (10d Common (0.148"x3",\_min.)\_nails)  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @ 4.75" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 23.47 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind IC DL=5.0 psf, wind BC DL=5.0 psf.

Top chord 2x6 SP #2  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

SPECIAL LOADS  
 -----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 15.67  
 BC - From 20 PLF at 0.00 to 20 PLF at 15.67  
 BC - 607 LB Conc. Load at 0.94, 2.27, 3.60, 4.94, 6.27  
 7.60, 8.94  
 BC - 2087 LB Conc. Load at 10.27

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=3416 U=1026 W=5.5"



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. BRACING MUST BE INSTALLED PRIOR TO THE TRUSS BEING LIFTED INTO POSITION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER BRACING OF THE TRUSSES.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/P) ASTH A653 GRADE 40/60 (H, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 James City, FL 33844  
 950 Marley Drive

FL Certificate of Authorization # 567

Scale = .375" / Ft.	REF R487 - - 35548
TC LL 20.0 PSF	DATE 11/09/05
TC DL 15.0 PSF	DRW HCUSR487 05313150
BC DL 10.0 PSF	HC-ENG JB/AF
BC LL 0.0 PSF	SEQN- 161209
TOT.LD. 45.0 PSF	DUR.FAC. 1.25
SPACING 24.0"	JREF- 1SS0487_Z01

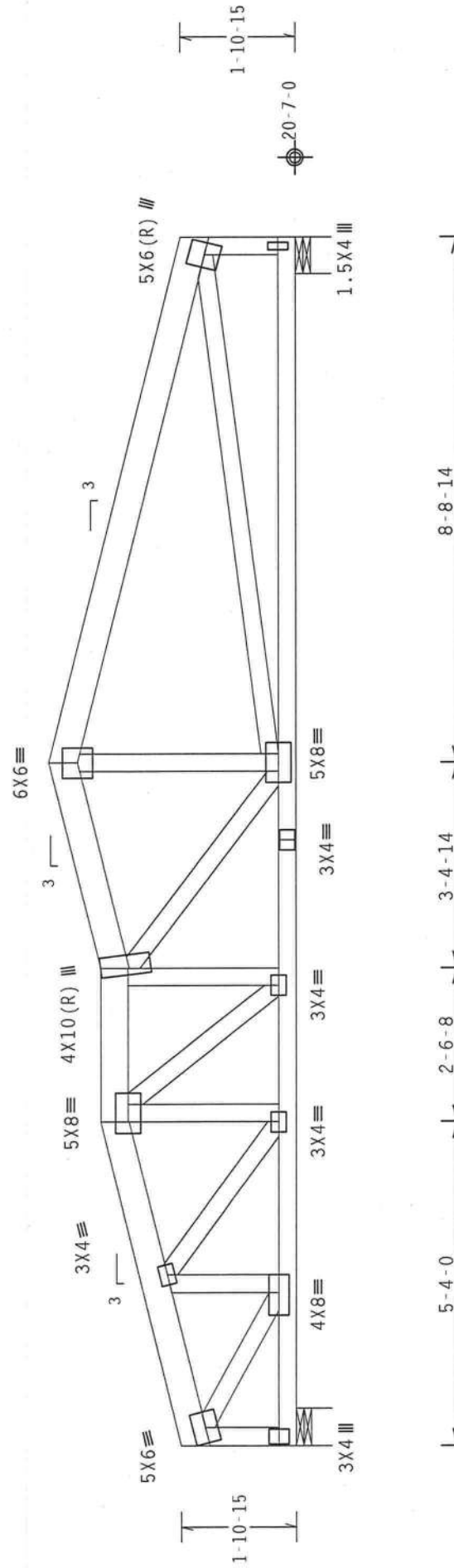
PLT TYP. Wave



**SPECIAL LOADS**  
 --- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 71 PLF at 0.00 to 71 PLF at 5.33  
 TC - From 71 PLF at 5.33 to 71 PLF at 11.28  
 TC - From 71 PLF at 11.28 to 71 PLF at 20.02  
 BC - From 20 PLF at 0.00 to 20 PLF at 20.02  
 BC - 168 LB Conc. Load at 1.40  
 BC - 1344 LB Conc. Load at 2.48

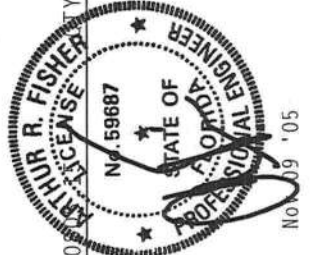
110 mph wind, 23.59 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #1 Dense  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3



R=2242 U=669 W=7.25" R=1090 U=326 W=7.25"

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 -- 35549	
BC DL	10.0 PSF	DATE 11/09/05	
BC LL	0.0 PSF	DRW HCUSR487 05313151	
TOT.LD.	45.0 PSF	HC-ENG JB/AF	
DUR.FAC.	1.25	SEQN- 161345	
SPACING	24.0"	JREF- ISS0487_Z01	

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSS SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI1. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1666A (W-H/S/K) ASTM A553 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T04A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) OR THIS DESIGNER'S REPRESENTATIVE. THE DESIGNER SHALL BE RESPONSIBLE FOR THE ASSESSMENT OF THIS DESIGN OR THIS DESIGNER'S REPRESENTATIVE. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave

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Top chord 2x6 SP #1 Dense  
 Bot chord 2x6 SP #1 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.16 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Left end vertical not exposed to wind pressure.

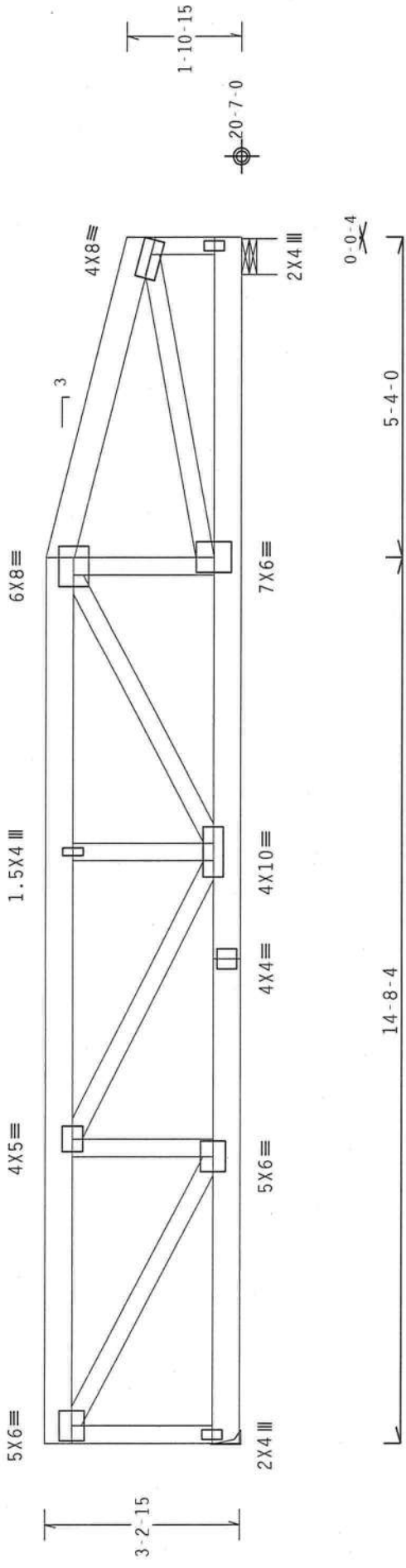
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

**SPECIAL LOADS**

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)  
 TC - From 47 PLF at 0.00 to 47 PLF at 20.02  
 BC - From 13 PLF at 0.00 to 13 PLF at 20.02  
 TC - 126 LB Conc. Load at 10.63, 11.96, 13.29  
 TC - 442 LB Conc. Load at 14.69  
 BC - 749 LB Conc. Load at 0.35, 9.40  
 BC - 36 LB Conc. Load at 10.63, 11.96, 13.29  
 BC - 125 LB Conc. Load at 14.69

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=2087 U=632 H=Simpson HUS26  
 w/ (6) 10d Common, 0.148"x3.0" nails in Truss  
 w/ (14) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine  
 Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0  
 R=1676 U=504 W=7.25"  
 20-0-4 Over 2 Supports  
 14-8-4  
 5-4-0  
 0-0-4  
 Scale = .375" / Ft.



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1-033 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 563 O'DONOGHUE DR., SUITE 200, MADISON, WI 53719, AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6100 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI1. ALPINE CONNECTOR PLATES ARE MADE OF 20/19/16GA (W./H./S./K) ASTM A653 GRADE 40/60 (W./K/H./S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10A-Z. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEER PRODUCTS, INC. IT SHALL BE THE USER'S RESPONSIBILITY TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THESE CONDITIONS OF USE SHALL BE ACCEPTED BY THE USER AT THE TIME OF ORDERING. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

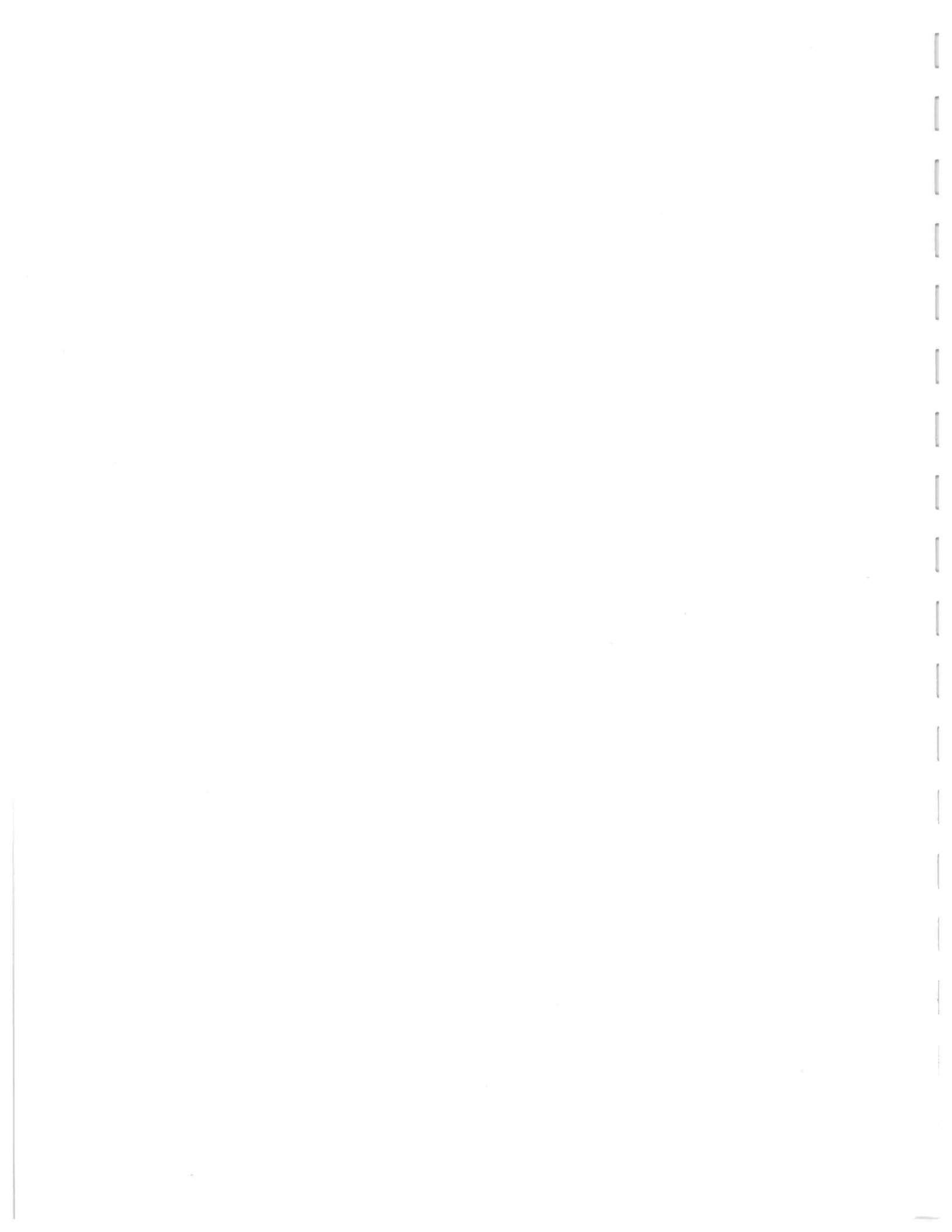
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PLT TYP. Wave	TC LL	20.0 PSF	FL/-/3/-/ -/R/-	Scale = .375" / Ft.
	TC DL	15.0 PSF	REF R487 --	35550
	BC DL	10.0 PSF	DATE	11/09/05
	BC LL	0.0 PSF	DRW	HCUSR487 05313152
	TOT.LD.	45.0 PSF	HC-ENG	JB/AF
	DUR.FAC.	1.25	SEQN-	161183
	SPACING	16.0"	JREF-	ISS0487_Z01





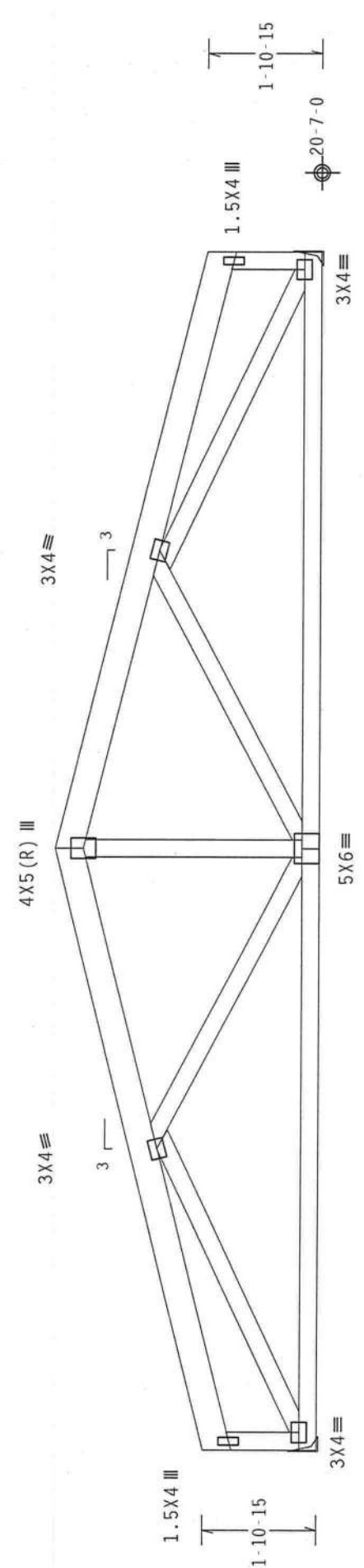


Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.75 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



10-0-2  
 20-0-4 Over 2 Supports  
 10-0-2

R=607 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

R=607 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00

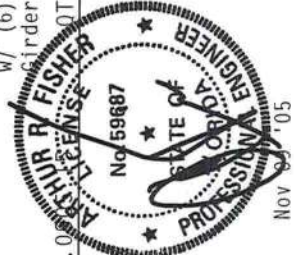
Scale = .375" / Ft.

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 500 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NTC (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGNER CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENTS. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED BY THE DESIGNER. THE DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENTS. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSII/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487 --	35552
TC DL	15.0 PSF	DATE	11/09/05	
BC DL	10.0 PSF	DRW	HCUSR487	05313153
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	45.0 PSF	SEON-	161162	
DUR.FAC.	1.25			
SPACING	16.0"	JREF-	1SS0487_Z01	

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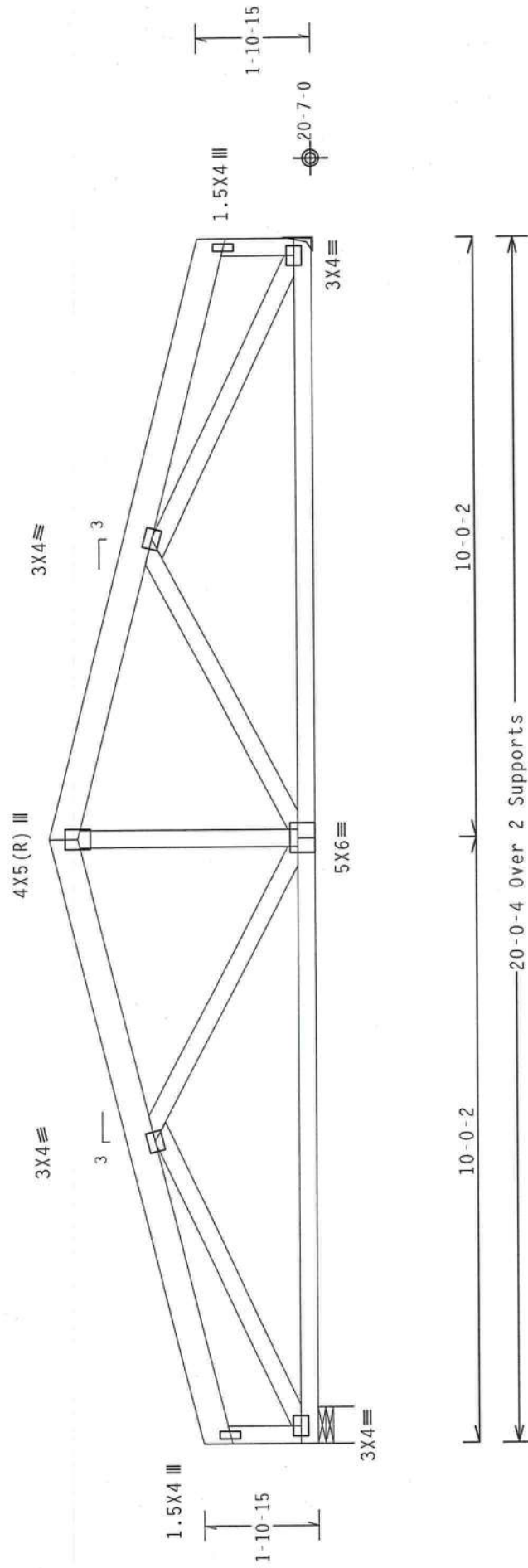


110 mph wind, 23.75 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



R=607 U=180 H-Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder



Design Crit: TPI-2002(STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.00  
 Scale = .375" / Ft.

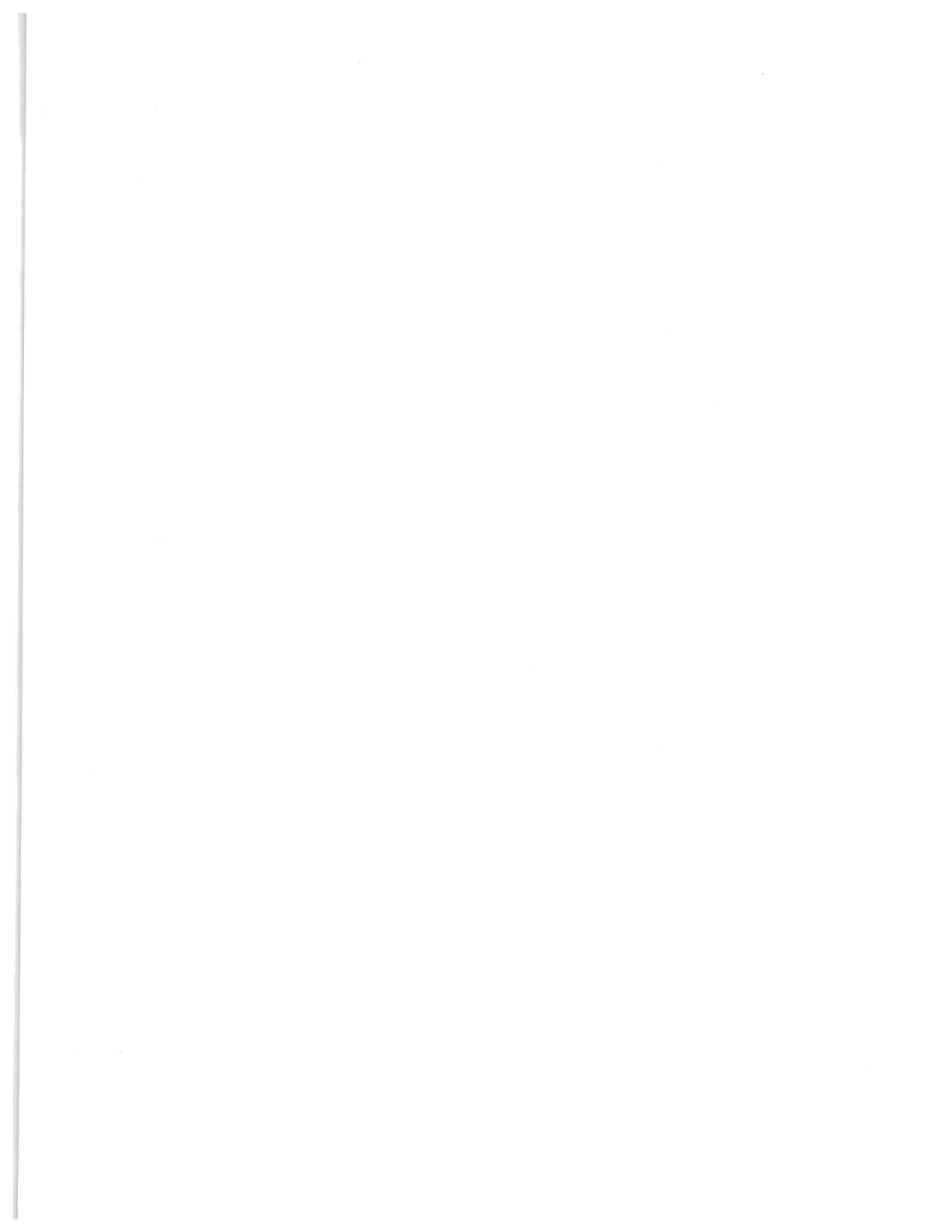
PLT TYP. Wave	TC LL	20.0 PSF	REF	R487--	35553
	TC DL	15.0 PSF	DATE	11/09/05	
	BC DL	10.0 PSF	DRW	HCUSR487	05313154
	BC LL	0.0 PSF	HC-ENG	JB/AF	
	TOT.LD.	45.0 PSF	SEQN-	161172	
	DUR.FAC.	1.25	JREF-	1SS0487_Z01	
	SPACING	16.0"			

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ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 13.2. DRAWING INDICATES ACCEPTANCE FOR INSPECTION. RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTER OF THIS DESIGN SHALL BE RESPONSIBLE FOR THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



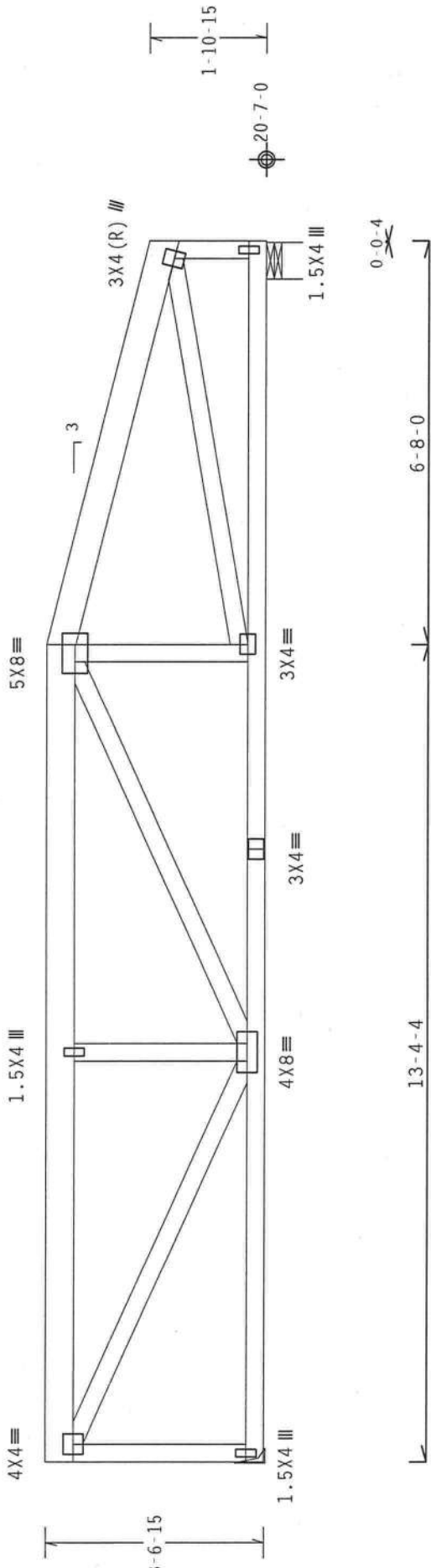
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, not located  
 within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind  
 BC DL=5.0 psf.

Left end vertical not exposed to wind pressure.  
 Trusses to be spaced at 16.0" OC maximum.

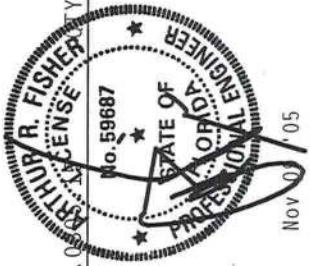
H = recommended connection based on manufacturer tested capacities and  
 calculations. Conditions may exist that require different connections  
 than indicated. Refer to manufacturer publication for additional  
 information.

Deflection meets L/360 live and L/240 total load. Creep increase factor  
 for dead load is 1.50.



R=607 U=188 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

PLT TYP. Wave  
 Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0



TC LL	20.0 PSF	Scale = .375" / Ft.
TC DL	15.0 PSF	REF R487 -- 35554
BC DL	10.0 PSF	DATE 11/09/05
BC LL	0.0 PSF	DRW HCUR487 05313155
TOT.LD.	45.0 PSF	HC-ENG JB/AF
DUR.FAC.	1.25	SEQN- 161189
SPACING	16.0"	JREF- 1SS0487_Z01

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-034 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSSES, 1000 W. WISCONSIN DR., SUITE 200, MADISON, WI 53719, AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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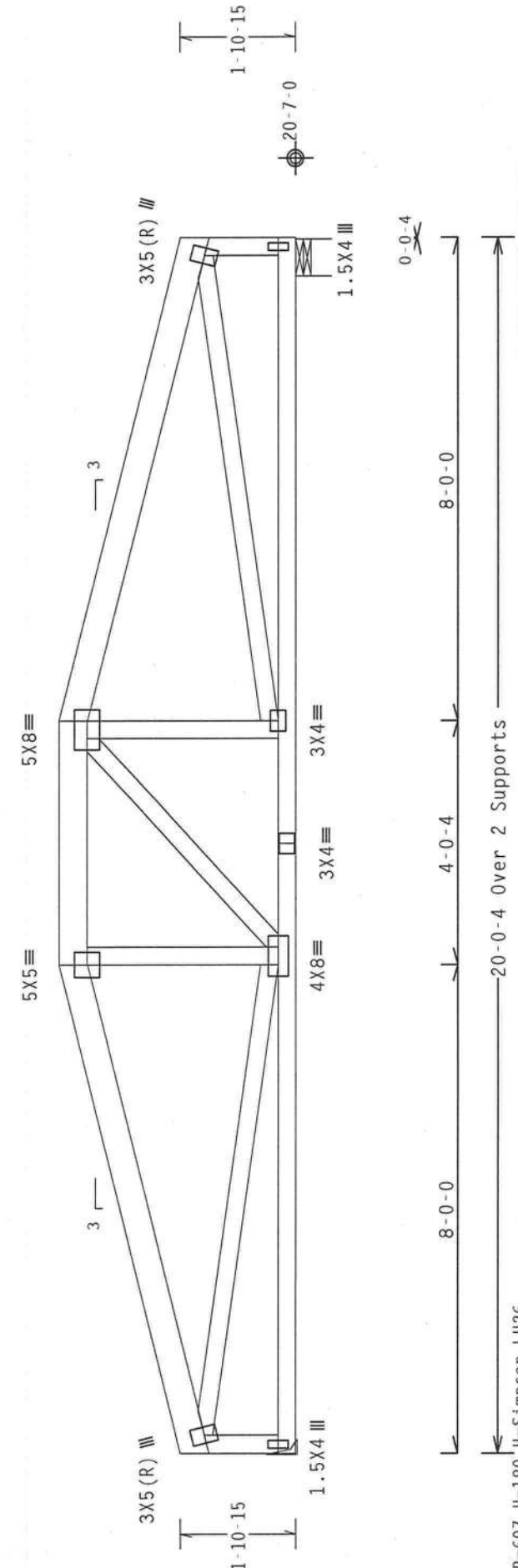
Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 23.49 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=607 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002 (STD) /FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.05

TC LL	20.0 PSF	FL/-/3/-/-/R/-	Scale = .375" /Ft.
TC DL	15.0 PSF	REF R487 --	35555
BC DL	10.0 PSF	DATE	11/09/05
BC LL	0.0 PSF	DRW	HCUSR487 05313156
TOT.LD.	45.0 PSF	HC-ENG	JB/AF
DUR.FAC.	1.25	SEQN-	161324
SPACING	16.0"	JREF-	1SS0487_Z01



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEER PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2019/166A (W/J/S/P) ASTH A553 GRADE 40/60 (R. K/M.S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE THE PER ANHE TO OF TPI 002 SECT. 1.1.1. ANY DESIGNATION INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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 Haines City, FL 33844  
 FL Certificate of Authorization # 567

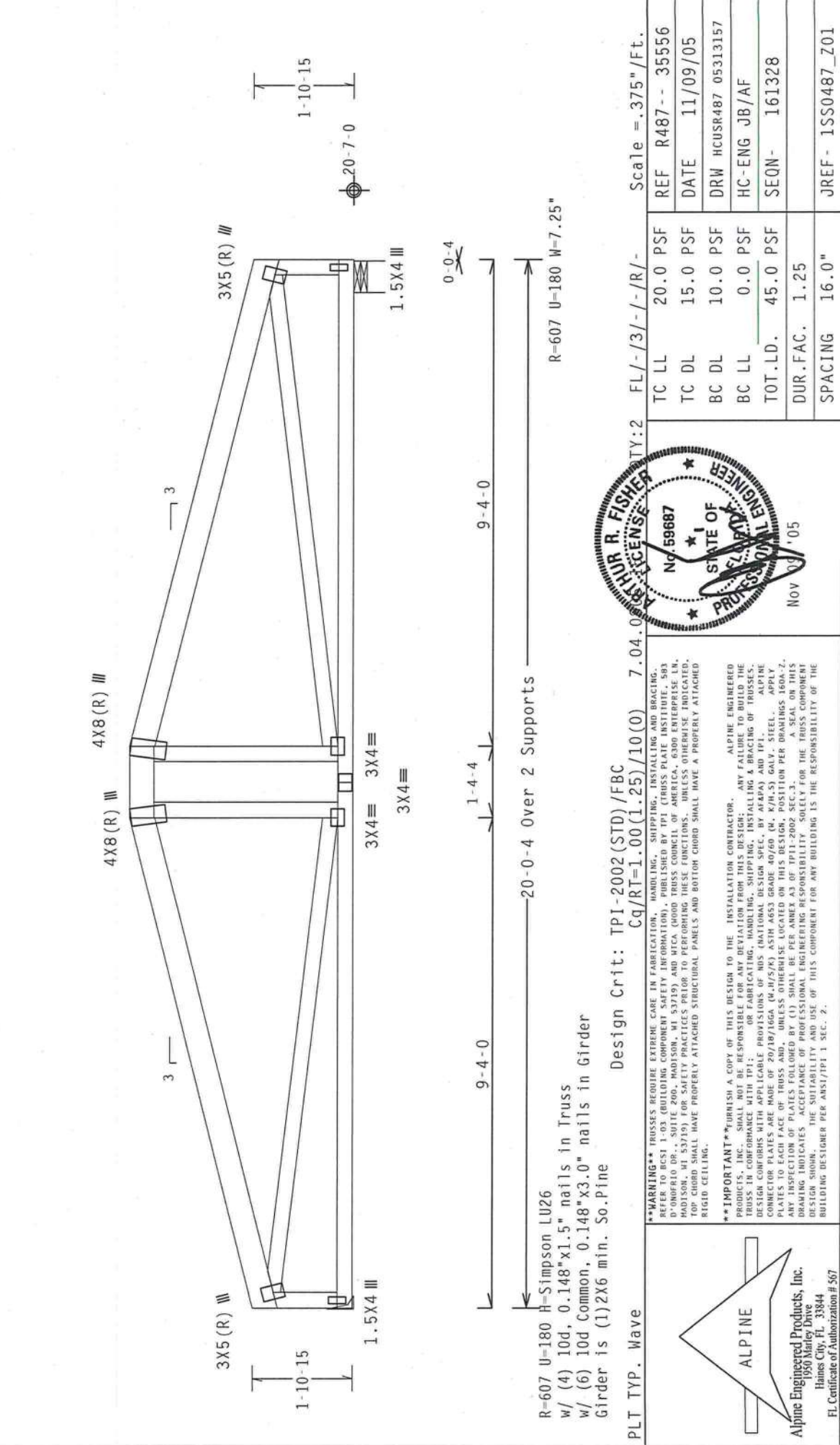


3-344-ISAAC CONST/muBR - KUVF - H9417  
 Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 23.66 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

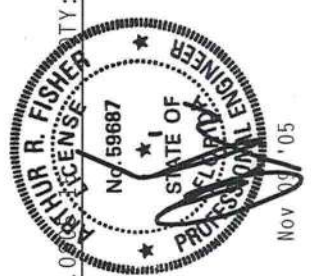
Trusses to be spaced at 16.0" OC maximum.  
 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=607 U=180 H=Simpson LU26  
 w/ (4) 10d, 0.148"x1.5" nails in Truss  
 w/ (6) 10d Common, 0.148"x3.0" nails in Girder  
 Girder is (1)2X6 min. So.Pine

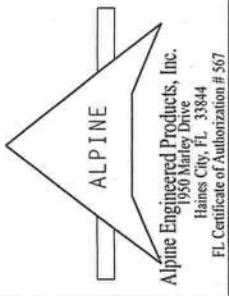
Design Crit: TPI-2002 (STD)/FBC  
 Cq/RT=1.00(1.25)/10(0) 7.04.0  
 Scale = .375" / Ft.

TC LL	20.0 PSF	FL / - / 3 / - / - / R / -	REF	R487 - -	35556
TC DL	15.0 PSF		DATE	11/09/05	
BC DL	10.0 PSF		DRW	HCUSR487	05313157
BC LL	0.0 PSF		HC-ENG	JB/AF	
TOT.LD.	45.0 PSF		SEQN-	161328	
DUR.FAC.	1.25				
SPACING	16.0"		JREF-	1SS0487_Z01	



**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 508 HUNTERS LANE, HUNTERSVILLE, NC 27640) AND THE MANUFACTURER'S INSTRUCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/S/P) ASTM A653 GRADE 40/60 (W. K/M.5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





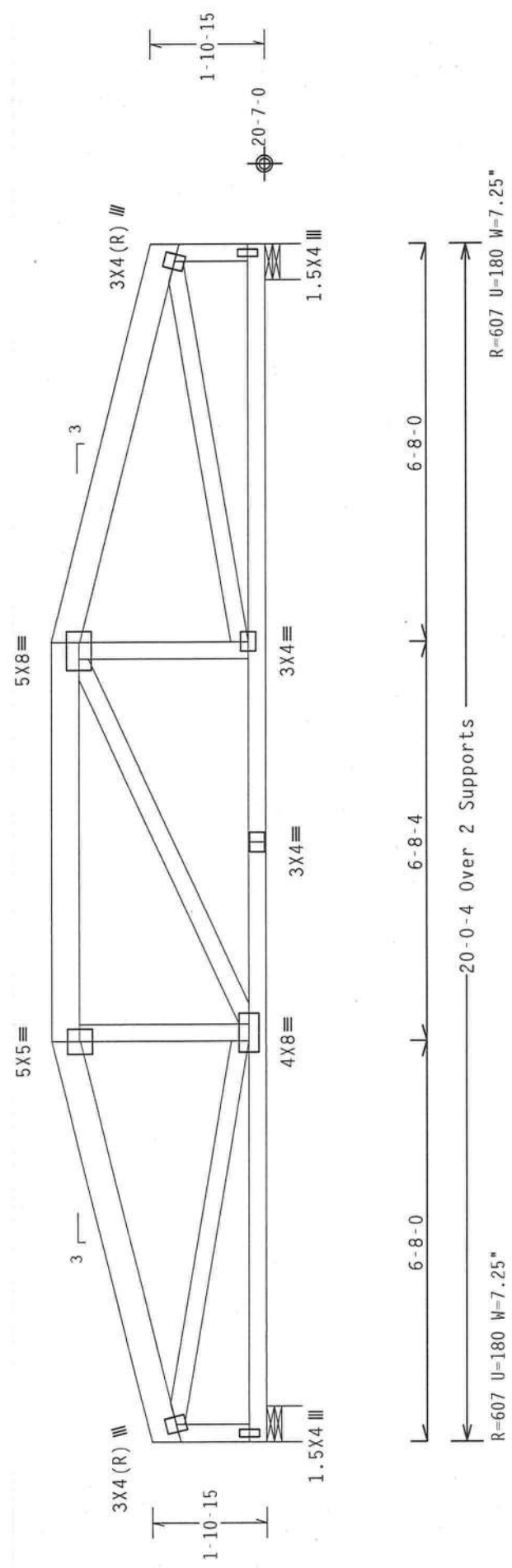
13-244-13AMC COR2/100DLN - NOV - 10001

Top chord 2x6 SP #2  
 Bot chord 2x4 SP #2 Dense  
 Webs 2x4 SP #3

110 mph wind, 23.33 ft mean hgt, ASCE 7-98, CLOSED bldg, not located within 4-50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

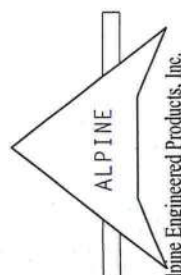
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

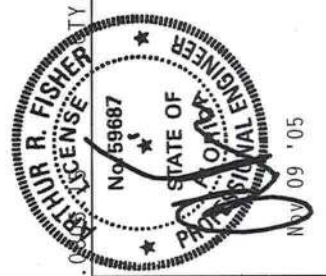


R=607 U=180 W=7.25"

R=607 U=180 W=7.25"

Design Crit: TPI-2002 (STD) / FBC  
 Cq/RT=1.00 (1.25) / 10(0) 7.04.0

PLT TYP. Wave	 <p>ALPINE          Alpine Engineered Products, Inc.          1950 Marley Drive          Haines City, FL 33844          FL Certificate of Authorization # 567</p>	<p><b>**WARNING**</b> TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONDRETO DR., SUITE 200, MADISON, MI 48719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, MI 48719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.</p> <p><b>**IMPORTANT**</b> FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING &amp; BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2010/10GA (24/25/27) ASTM A553 GRADE 40/60 (40-7/10/15) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS CHORDS UNLESS OTHERWISE SPECIFIED. PERFORM PER PER AIA SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.</p>	<p>Scale = .375" / Ft.</p>	
				<p>FL / - / 3 / - / - / R / -</p>
		TC LL	20.0 PSF	REF R487 - 35557
		TC DL	15.0 PSF	DATE 11/09/05
		BC DL	10.0 PSF	DRW HCUSR487 05313158
		BC LL	0.0 PSF	HC-ENG JB/AF
		TOT.LD.	45.0 PSF	SEQN- 161350
		DUR.FAC.	1.25	
		SPACING	16.0"	JREF- 1SS0487_Z01





# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

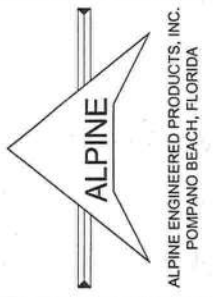
## NOTES:

- THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.
- ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING	SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4	2-2X4
2X6	1 ROW	2X4	1-2X6	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)	2-2X4(*)
2X8	1 ROW	2X6	1-2X8	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

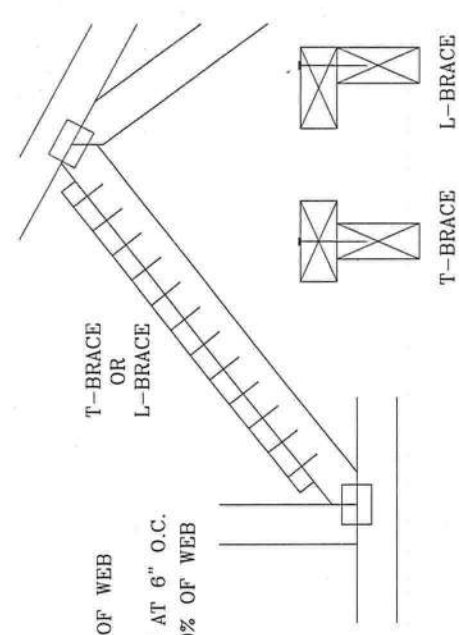


**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D'ONDRIO DR., SUITE 200, MADISON, WI 53719, AND VTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**\*\*IMPORTANT\*\*** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS, NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA CW/H/S/K3 ASTM A653 GRADE 40/60 CW/K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.

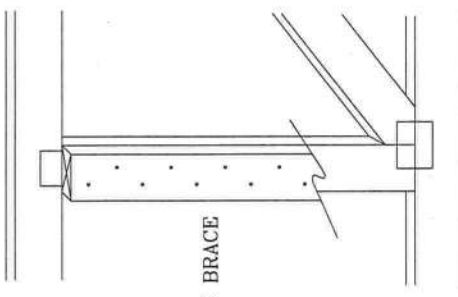
T-BRACING  
OR  
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE ATTACH WITH 16d NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			



ASCE 7-98: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH	2x4 GABLE VERTICAL SPACING		BRACE GRADE		NO BRACES		(1) 1x4 "L" BRACE *		(2) 2x4 "L" BRACE *		(1) 2x6 "L" BRACE **		(2) 2x6 "L" BRACE **				
	SPACING	SPECIES	#1 / #2	#3	STUD	STANDARD	#1	#2	#3	STUD	STANDARD	#1	#2	#3	STUD	STANDARD	
																	GROUP A
12" O.C.	SPF	HF	6' 4"	5' 5"	7' 2"	7' 1"	7' 8"	7' 2"	8' 11"	8' 11"	8' 11"	11' 2"	11' 2"	11' 2"	11' 2"	11' 2"	14' 0"
	SP	DFL	6' 4"	5' 5"	7' 2"	7' 1"	7' 8"	7' 2"	8' 11"	8' 11"	8' 11"	11' 2"	11' 2"	11' 2"	11' 2"	11' 2"	14' 0"
16" O.C.	SPF	HF	6' 8"	5' 8"	7' 6"	7' 6"	8' 7"	8' 7"	10' 3"	10' 3"	10' 3"	13' 5"	13' 5"	13' 5"	13' 5"	13' 5"	14' 0"
	SP	DFL	6' 8"	5' 8"	7' 6"	7' 6"	8' 7"	8' 7"	10' 3"	10' 3"	10' 3"	13' 5"	13' 5"	13' 5"	13' 5"	13' 5"	14' 0"
24" O.C.	SPF	HF	6' 9"	5' 9"	7' 7"	7' 7"	8' 11"	8' 11"	10' 3"	10' 3"	10' 3"	13' 5"	13' 5"	13' 5"	13' 5"	13' 5"	14' 0"
	SP	DFL	6' 9"	5' 9"	7' 7"	7' 7"	8' 11"	8' 11"	10' 3"	10' 3"	10' 3"	13' 5"	13' 5"	13' 5"	13' 5"	13' 5"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:	
SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#2
STUD	STUD
STANDARD	STANDARD
SOUTHERN PINE	
#3	#3
STUD	STUD
STANDARD	STANDARD
DOUGLAS FIR-LARCH	
#1	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

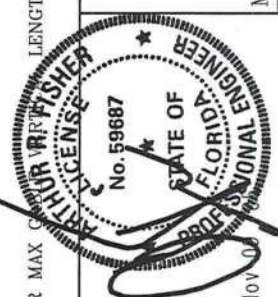
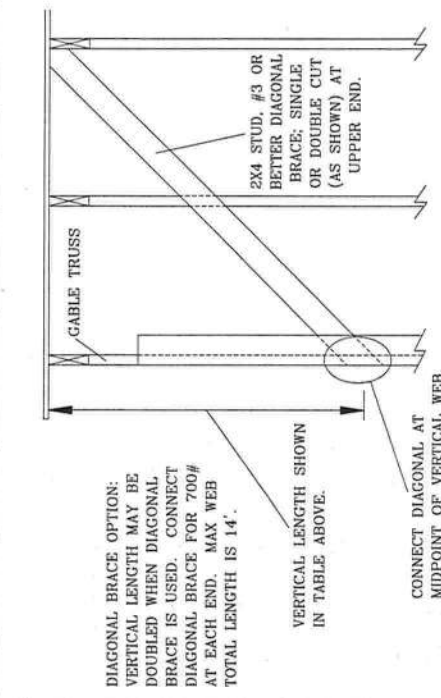
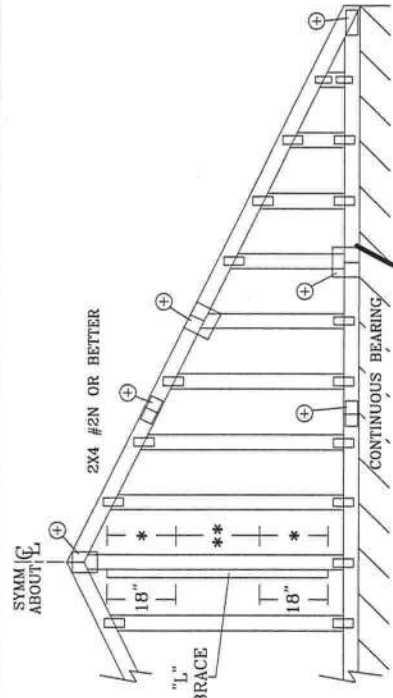
\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH LESS THAN 4' 0"	NO SPLICE
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	1X4 OR 2X3
GREATER THAN 11' 6"	2X4
	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

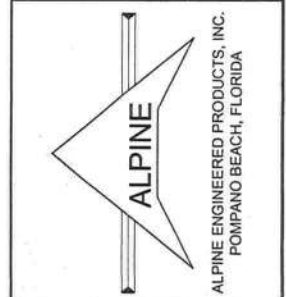


REF	ASCE7-98-GABI1030
DATE	11/26/03
DRWG	AI1030EC1103
	-ENG

MAX. TOT. LD.	60 PSF
MAX. SPACING	24' 0"

\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. BRACES MUST BE INSTALLED SAFELY AND CORRECTLY. PUBLISHED BY THE TRUSS BRACE INSTITUTE, 583 DUNDON RD., SUITE 201, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING PER THESE SPECIFICATIONS. APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. BY AISC AND AISC 360-05. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL TRUSS AND BRACE PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY GD SHALL BE PER ANNEA 3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSII/TPI 1 SEC. 2.






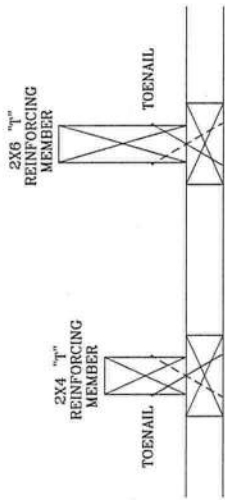
# GABLE DETAIL FOR LET-IN VERTICALS

GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS LESS THAN 4' 0"	PLATE SIZE	IF PLATES OVERLAP*
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LX4 OR 2X3	2X8
GREATER THAN 11' 6"	2X4	2X8
	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

\* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

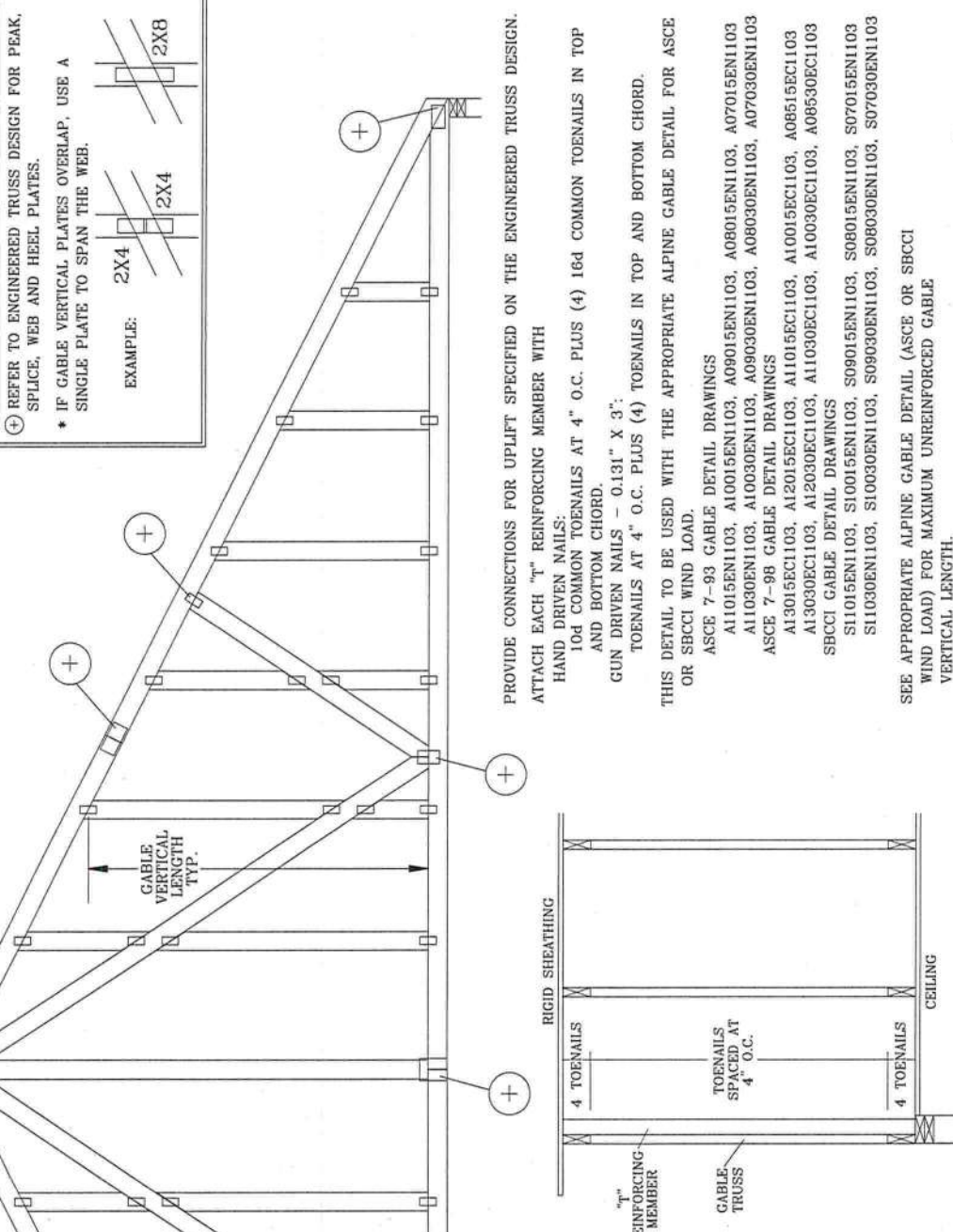
MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

## WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10%	10%
15 FT	2x6	40%	50%
110 MPH	2x4	10%	10%
30 FT	2x6	50%	50%
100 MPH	2x4	10%	10%
15 FT	2x6	30%	50%
100 MPH	2x4	10%	10%
30 FT	2x6	40%	40%
90 MPH	2x4	20%	10%
15 FT	2x6	20%	40%
90 MPH	2x4	10%	10%
30 FT	2x6	10%	50%
80 MPH	2x4	10%	20%
15 FT	2x6	10%	30%
80 MPH	2x4	20%	10%
30 FT	2x6	20%	40%
70 MPH	2x4	0%	20%
15 FT	2x6	0%	20%
70 MPH	2x4	10%	20%
30 FT	2x6	10%	30%

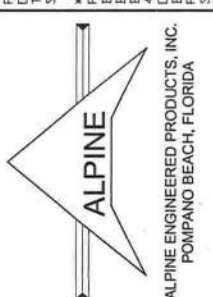
EXAMPLE:

ASCE WIND SPEED = 100 MPH  
MEAN ROOF HEIGHT = 30 FT  
GABLE VERTICAL = 24" O.C. SP #3  
"T" REINFORCING MEMBER SIZE = 2X4  
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10  
(1) 2X4 "L" BRACE LENGTH = 6' 7"  
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH  
1.10 x 6' 7" = 7' 3"



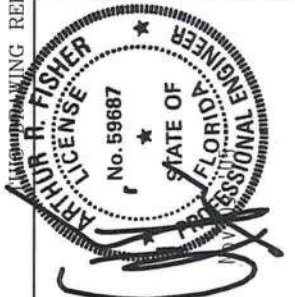
PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN. ATTACH EACH "T" REINFORCING MEMBER WITH HAND DRIVEN NAILS:  
10d COMMON TOENAILS AT 4" O.C. PLUS (4) 16d COMMON TOENAILS IN TOP AND BOTTOM CHORD.  
GUN DRIVEN NAILS - 0.131" X 3":  
TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.  
THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.  
SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

ASCE 7-93 GABLE DETAIL DRAWINGS  
A11015ENI103, A10015ENI103, A09015ENI103, A08015ENI103, A07015ENI103  
A1030ENI103, A10030ENI103, A09030ENI103, A08030ENI103, A07030ENI103  
ASCE 7-98 GABLE DETAIL DRAWINGS  
A13015ECI103, A12015ECI103, A11015ECI103, A10015ECI103, A08515ECI103  
A13030ECI103, A12030ECI103, A11030ECI103, A10030ECI103, A08530ECI103  
SBCCI GABLE DETAIL DRAWINGS  
S11015ENI103, S10015ENI103, S09015ENI103, S08015ENI103, S07015ENI103  
S11030ENI103, S10030ENI103, S09030ENI103, S08030ENI103, S07030ENI103



\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-93 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS COMPANY INSTITUTE), 585 W. WINDY RIDGE DR., SUITE 200, MADISON, WI 53719, AND AITCA CHORD TRUSS COUNCIL (TRUSS CHORDS) 10000 W. 11TH AVENUE, SUITE 100, DENVER, CO 80231. THESE TRUSSES ARE DESIGNED TO BE USED IN CONFORMANCE WITH THE SPECIFICATIONS AND DESIGN REQUIREMENTS OF THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS, AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD) AND AISC (STEEL CONSTRUCTION MANUAL) AND ALL APPLICABLE CODES AND REGULATIONS. (40/60 (L/K/H/S) GALV. STEEL) APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE SPECIFIED ON THIS DESIGN, POSITION PER DRAWINGS 1600A-2. ANY INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE BY (D) SHALL BE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

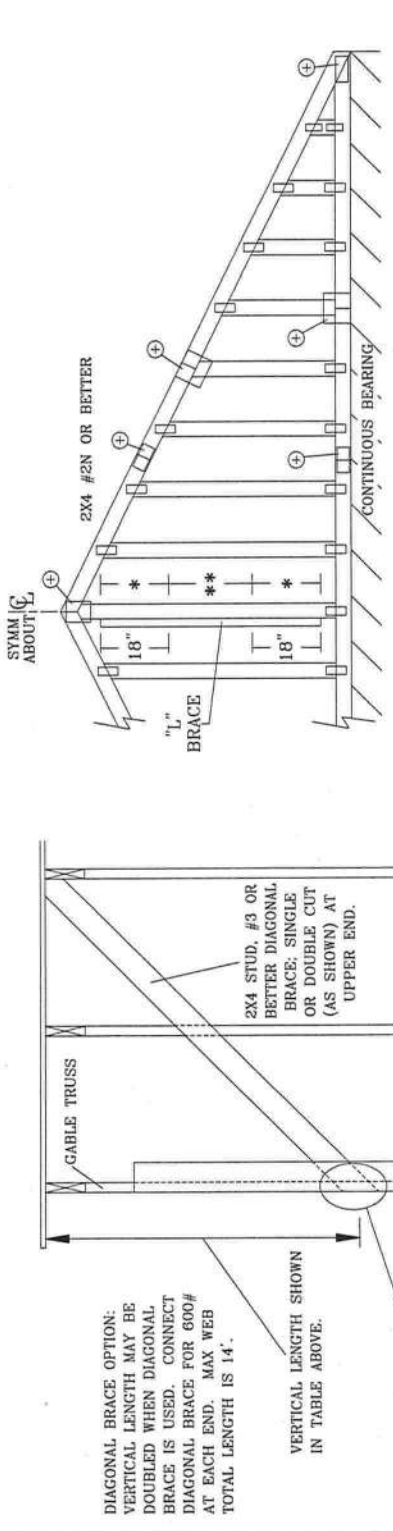


REF	LET-IN VERT
DATE	01/16/04
DRWG	GBLETTIN1103
	-ENG DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035



MAX GABLE VERTICAL LENGTH	GABLE VERTICAL SPACING	GABLE SPECIES	BRACE GRADE	(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE			
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
				SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	9' 8"	12' 5"	12' 9"	12' 5"	12' 9"
HF	#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	9' 5"	12' 4"	12' 4"	12' 4"	12' 4"	14' 0"	14' 0"		
SP	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	9' 1"	10' 7"	10' 7"	10' 7"	10' 7"	14' 0"	14' 0"		
DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	10' 2"	12' 5"	13' 5"	12' 5"	13' 5"	14' 0"	14' 0"		
SPF	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	9' 11"	12' 5"	12' 8"	12' 5"	12' 8"	14' 0"	14' 0"		
HF	STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	9' 11"	12' 5"	12' 6"	12' 5"	12' 6"	14' 0"	14' 0"		
SP	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SPF	STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	14' 0"	14' 0"		
HF	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SPF	STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	11' 1"	13' 3"	13' 3"	13' 3"	13' 3"	14' 0"	14' 0"		
HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SPF	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
HF	#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDDLE OF VERTICAL WEB.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

DESIGNER, PER ANSI/TPI 1 SEC. 2.

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

DESIGNER, PER ANSI/TPI 1 SEC. 2.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR #1 / #2	STANDARD	STUD	STANDARD
HEM-FIR #2	STUD	STANDARD	
HEM-FIR #3	STUD	STANDARD	

DOUGLAS FIR-LARCH #3

STUD	STANDARD
STUD	STANDARD

SOUTHERN PINE #3

STUD	STANDARD
STUD	STANDARD

GROUP B:

HEM-FIR #1 & BTR	#1
DOUGLAS FIR-LARCH #1	#2
DOUGLAS FIR-LARCH #2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

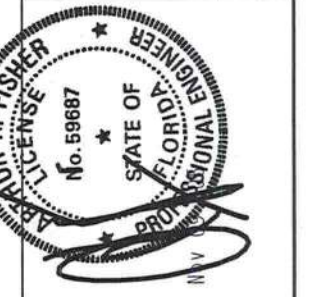
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH LESS THAN 4' 0"	NO SPLICE
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	1X4 OR 2X3
GREATER THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

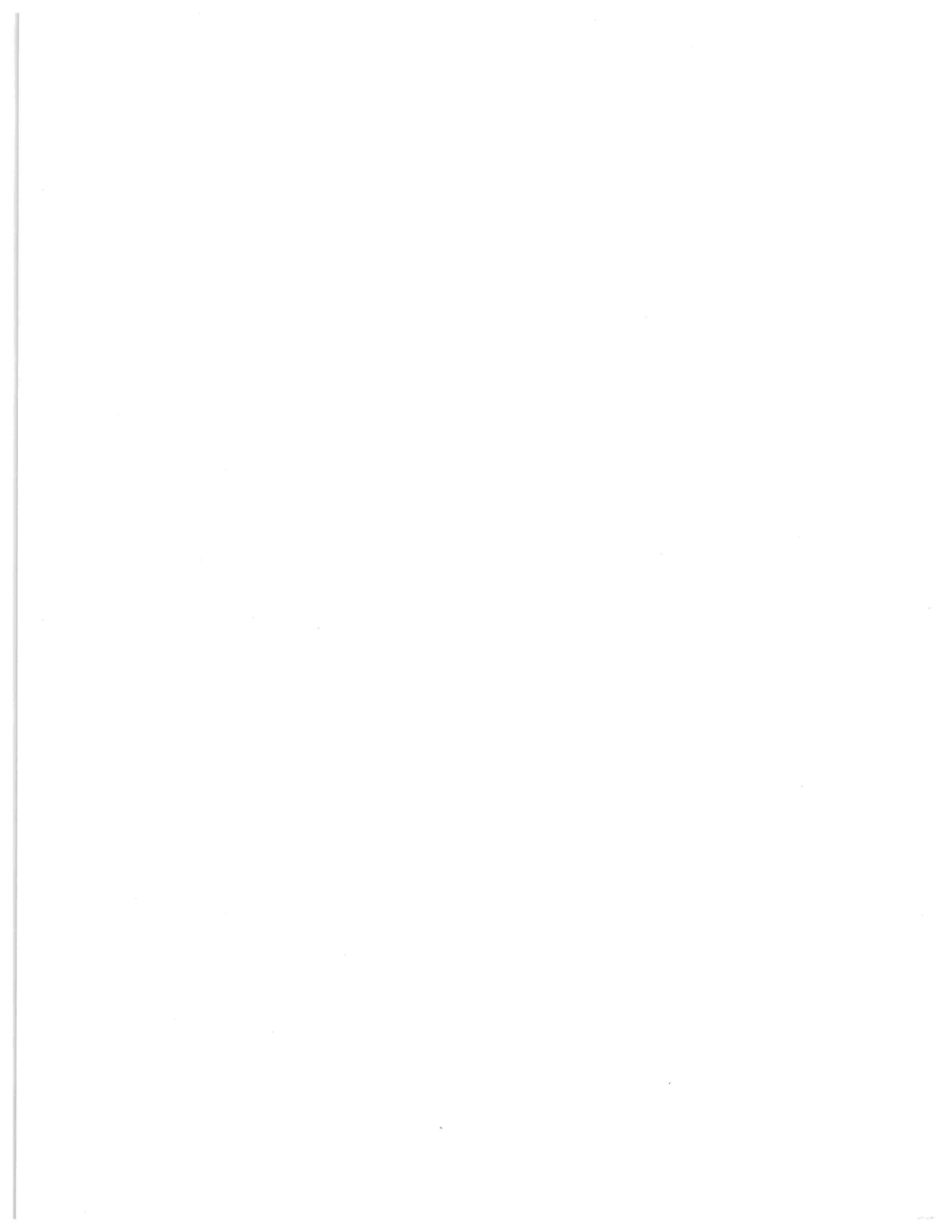
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DATE	11/26/03
DRWG	A11015EC1103
	-ENG

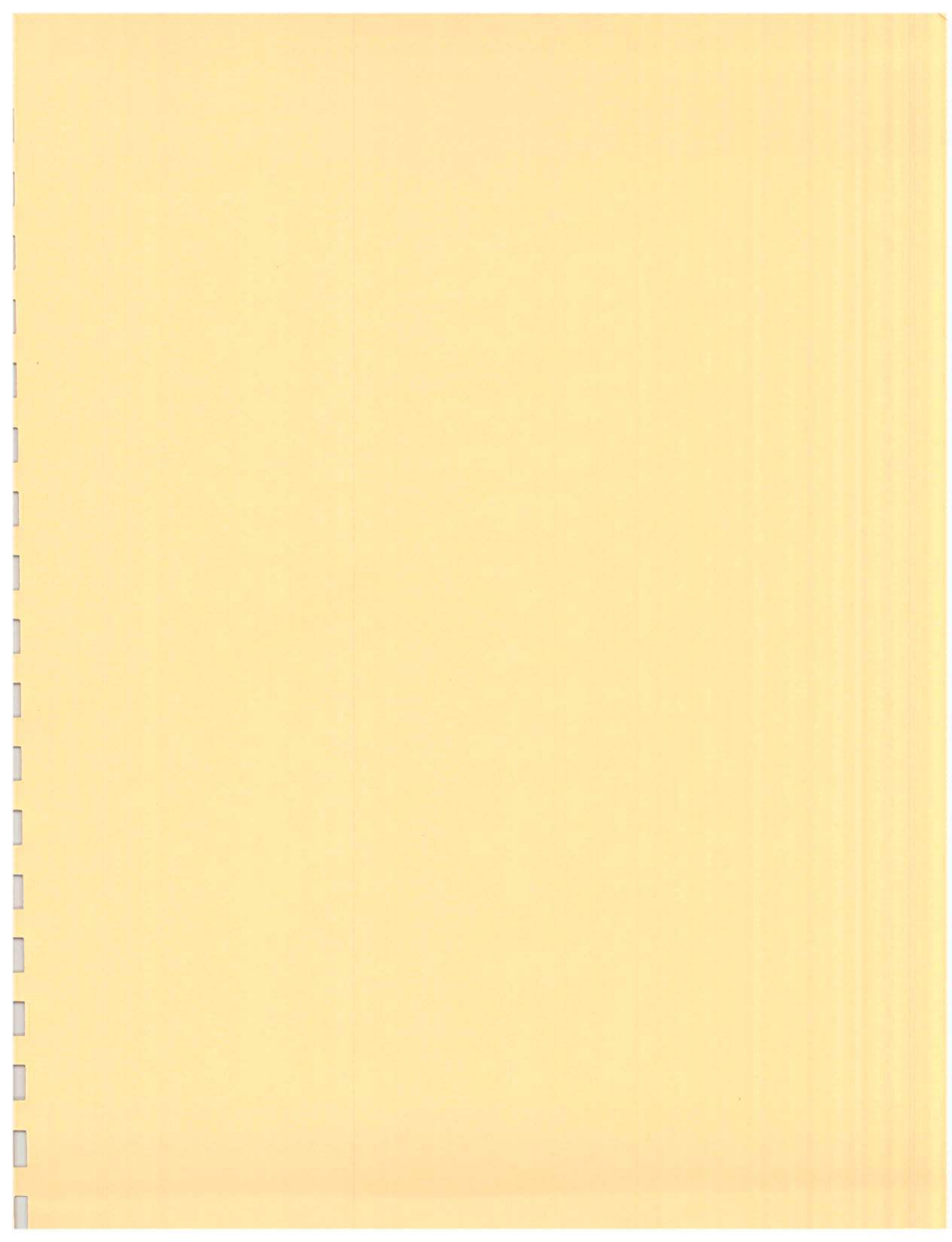
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"



\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 DUNDRIE DR., SUITE 200, MADISON, WI 53719 AND VICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA CW/H/S/K ASTM A653 GRADE 40/60 CW/K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.







For Office Use Only Application # 0510-71 Date Received 10-24-05 By AK Permit # 890/123867

Application Approved by - Zoning Official AK Date 06/10/05 Plans Examiner AK Date 11-16-05

Flood Zone X Development Permit N/A Zoning A3 Land Use Plan Map Category A.3

Comments Section 14.9 Special Family LT Permit

Applicants Name Linda or Melanie Roder Phone 752-2281

Address 387 S.W. Kemp Ct. Lake City, FL 32024

Owners Name Marion and Angela Huber Phone 758-1102 867-1463

911 Address 225 S.W. Finley Little Lane Lake City, FL 32024

Contractors Name Isaac Brathwaite of Isaac Construction Phone 79-7143

Address 1005 S.W. Water Ave Lake City, FL 32024

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Kevin Gray / 904-384-3265

Mortgage Lenders Name & Address Countrywide Mortgage

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 61-55-16-03387-012 Estimated Cost of Construction 220,000

Subdivision Name 475. L on Little Road (Soythwood Acres Entrance)

Driving Directions L on S.W. Finley Little, 3rd lot down on L

Type of Construction SPD Family lot

Total Acreage 2.05 Lot Size 0

Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 45' Side 38.75' Side 38.75' Rear 460.5'

Number of Stories 2 Heated Floor Area 4800 Roof Pitch 3-12

Porches 1264 Garage 761 Total 6825

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

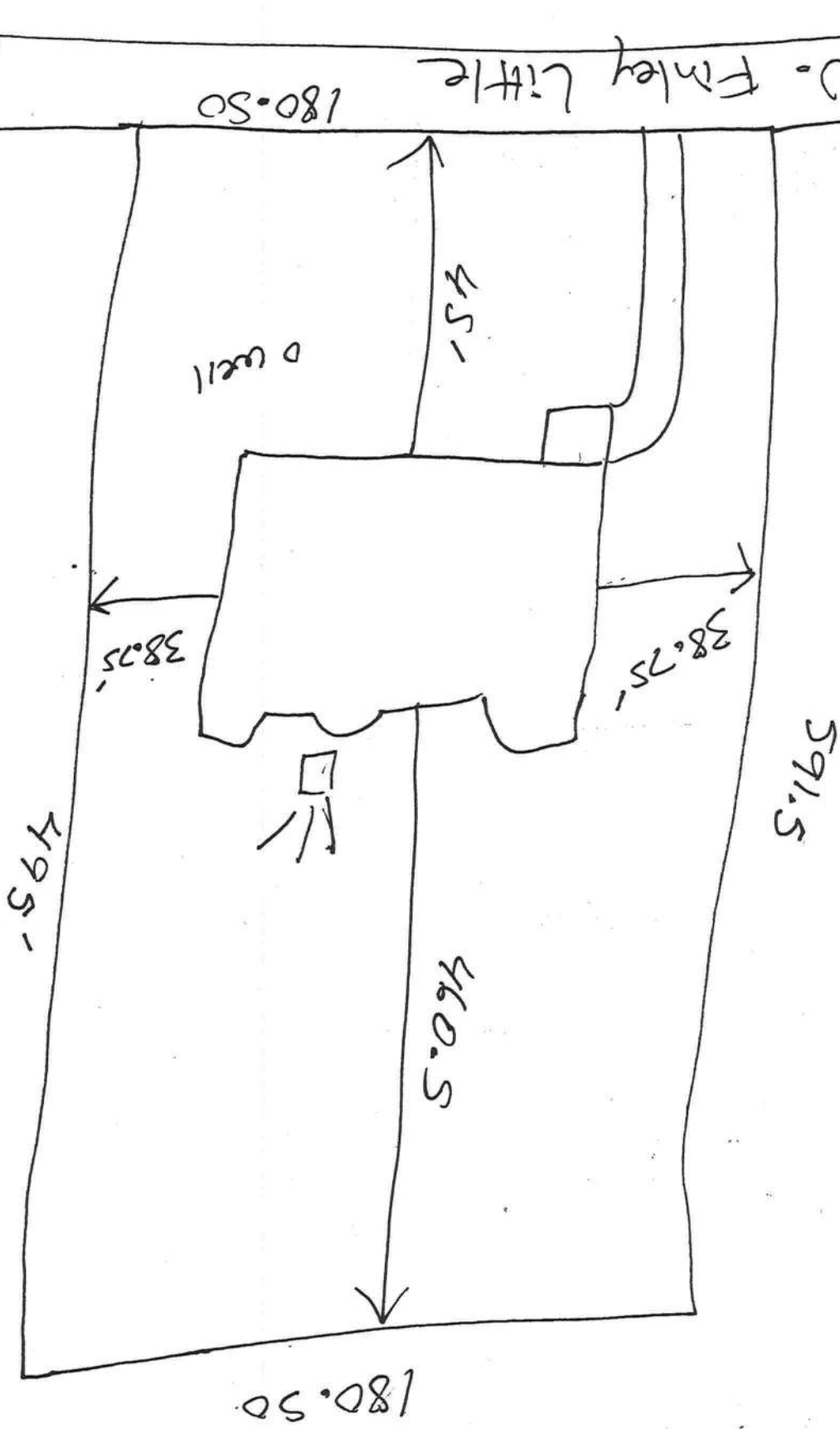
Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA  
COUNTY OF COLUMBIA  
Barbara C. Webster  
Commission # DD329279  
Expires July 2, 2008  
Notary Public  
Sworn to (or affirmed) and subscribed before me on this 30 day of Sept 2005  
Personally known X or Produced Identification

Contractor Signature [Signature]  
Contractors License Number CBC 059323  
Competency Card Number  
NOTARY STAMP/SEAL  
Notary Signature [Signature]



Site Plan  
Abraham Huber



S.W. Finley Little  
180.50

591.5

591.5

well

451



38.75

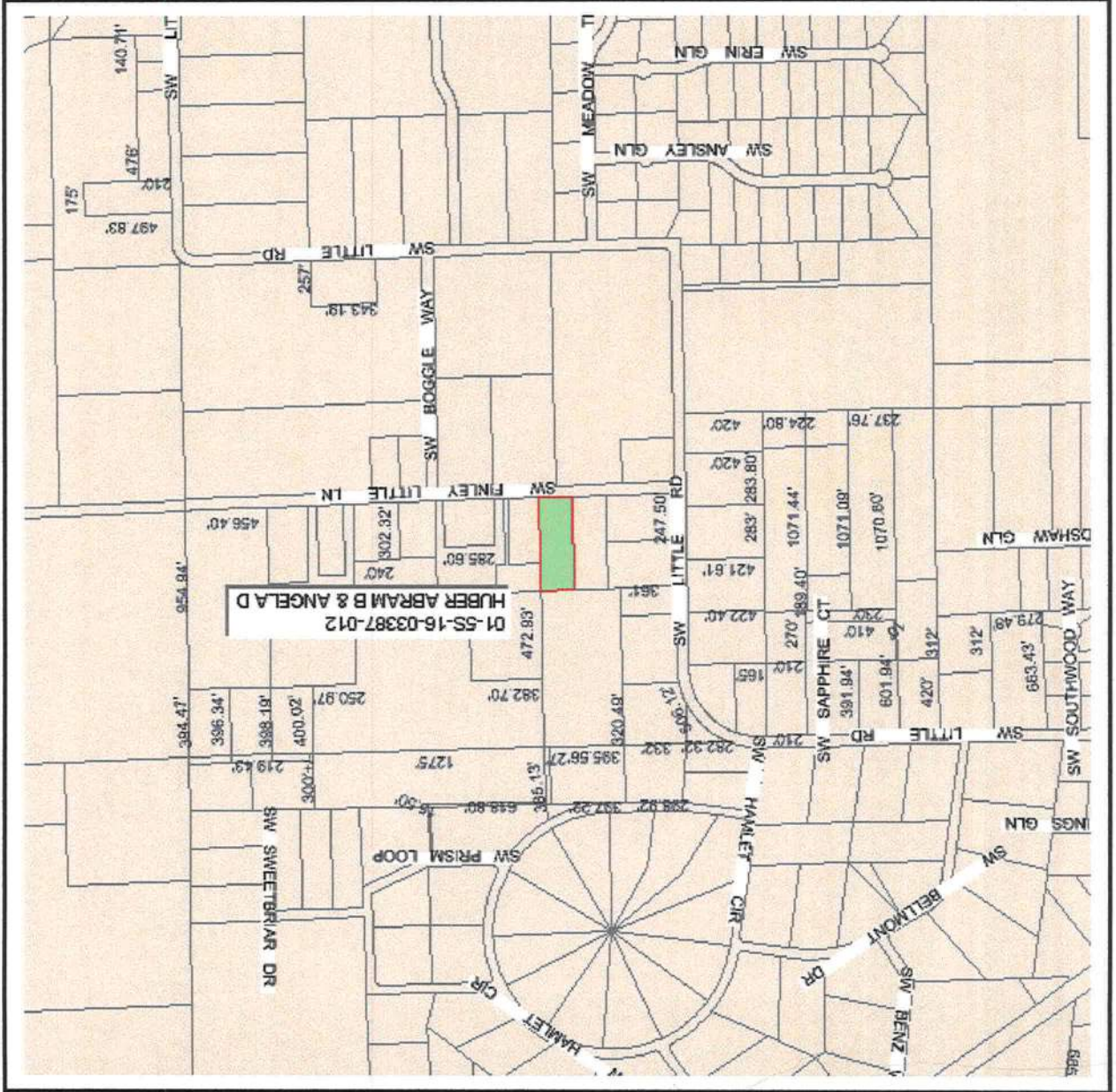
38.75

180.50



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

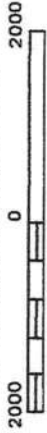
			<table border="1"> <tr><td>Landval</td><td>\$17,712.00</td></tr> <tr><td>BldgVal</td><td>\$0.00</td></tr> <tr><td>AppVal</td><td>\$17,712.00</td></tr> <tr><td>JustVal</td><td>\$17,712.00</td></tr> <tr><td>Assd</td><td>\$17,712.00</td></tr> <tr><td>Exmpt</td><td>\$0.00</td></tr> <tr><td>Taxable</td><td>\$17,712.00</td></tr> </table>	Landval	\$17,712.00	BldgVal	\$0.00	AppVal	\$17,712.00	JustVal	\$17,712.00	Assd	\$17,712.00	Exmpt	\$0.00	Taxable	\$17,712.00
Landval	\$17,712.00																
BldgVal	\$0.00																
AppVal	\$17,712.00																
JustVal	\$17,712.00																
Assd	\$17,712.00																
Exmpt	\$0.00																
Taxable	\$17,712.00																
<p><b>Parcel: 01-55-16-03387-012 - NO AG ACRE (009900)</b>                  COMM SW COR OF NW 1/4 OF NE 1/4, RUN E 591.5 FT FOR POB, CONT E 180.50 FT, N 495 FT.</p>																	
<p><b>Columbia County Property Appraiser</b>                  J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083</p>																	
<p>Name: HUBER ABRAM B &amp; ANGELA D                  Site: RT 9 BX 796                  Mail: P O BOX 932                  LAKE CITY, FL 32056</p>																	
<p>Info</p>																	







APPROXIMATE SCALE IN FEET



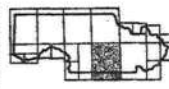
NATIONAL FLOOD INSURANCE PROGRAM

**FIRM**  
FLOOD INSURANCE RATE MAP

COLUMBIA  
COUNTY,  
FLORIDA  
(UNINCORPORATED AREAS)

PANEL 175 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER  
120070 0175 B

EFFECTIVE DATE:  
JANUARY 6, 1988



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Further information about National Flood Insurance Program flood hazard maps is available at [www.fema.gov/nif/isd](http://www.fema.gov/nif/isd).





*Huber*

**COLUMBIA COUNTY, FLORIDA  
LAND DEVELOPMENT REGULATION ADMINISTRATOR  
SPECIAL FAMILY LOT PERMIT APPLICATION**

A special family lot permit may be issued by the Land Development Regulation Administrator on land zoned Agricultural or Environmentally Sensitive Area within these land development regulations, for the purpose of conveying a lot or parcel to an individual who is the parent, grandparent, sibling, child or adopted child or grandchild of the person who conveyed the parcel to said individual, not to exceed two (2) dwelling units per one (1) acre and the lot complies with all other conditions from permitting development as set forth in these land development regulations. This provision is intended to promote the perpetuation of the family homestead in rural areas by making it possible for family members to reside on lots, which exceed maximum density for such areas, provided that the lot complies with the following conditions for permitting:

1. The division of lots shall be by recorded separate deed and meet all other applicable land development regulations; and
2. The lot split or subdivision is for the establishment of a homestead of that relative and the lot so conveyed is at least one-half (1/2) acre in size and the remaining lot is at least one-half (1/2) acre in size; and
3. The family lot permit shall only be issued once for each relative of the parent tract owner. However, for purposes of this provision, if a lot is permitted under this provision to a daughter, for example, and was to be returned to the ownership of the owner of the parent tract, then the original use of this provision to provide the lot to the daughter shall not be counted as one of the one permitted per relative.
4. The lot complies with all other conditions for permitting and development as set forth in these land development regulations.

**1. Name of Recipient Relative (Applicant)**

Address P.O. Box 932 City Lake City FL Zip Code 32056  
Phone (386) 758-6762

**2. Name of Title Holder(s)** Abram B. Huber and Angela D. Huber

Address P.O. Box 932 City Lake City FL Zip Code 32056  
Phone (386) 758-6762

**3. Recipient's Relationship to Title Holder** same

4. Size of Property 2.05 acres

5. Tax Parcel ID# 01-55-16-03387-012 (Attach a Copy of the Deed)

**No permit will be issued unless the deed is properly recorded in the Clerk of the Courts Office.**

I (we) hereby certify that all of the above statements and the statements contained in any papers or plans submitted herewith are true and correct to the best of my (our) knowledge and belief.

Applicants Name (Print or Type) Abram B. Huber and Angela D. Huber

*X* 

9/31/05

Applicant Signature Angela D. Huber

Date 10/02/05

OFFICIAL USE

Current Land Use Classification \_\_\_\_\_ Current Zoning District \_\_\_\_\_

Approved \_\_\_\_\_ Denial = Reason \_\_\_\_\_



THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID  
POST OFFICE BOX 1328  
LAKE CITY, FL 32056-1328

Inst: 2002016051 Date: 08/13/2002 Time: 12:55:01  
Doc Stamp-Deed : 0.70  
TICK DC, P. DeWitt Cason, Columbia County B:960 P:256

RETURN TO:

TERRY McDAVID  
POST OFFICE BOX 1328  
LAKE CITY, FL 32056-1328

WARRANTY DEED

THIS INDENTURE, made this 21st day of June, 2002, BETWEEN CHARLES S. SPARKS and SANDRA L. SPARKS, Husband and Wife, of the County of Columbia, State of Florida, Grantor\*, and ABRAM B. HUBER and ANGELA D. HUBER, Husband and Wife, whose post office address is Post Office Box 932, Lake City, FL 32056, of the County of Columbia, State of Florida, grantee\*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

SUBJECT TO: Restrictions, cements and outstanding mineral rights of record, if any, and taxes for the current year.

N.B.: The grantee, Angela D. Huber is the daughter of the grantors.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

\*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.



Inst:2002016051 Date:08/13/2005 Time:12:55:01

Doc Stamp-Dues: 0.70

TRK DC, P. Dewitt Cason, Columbia County B:963 P:25A**EXHIBIT "A"****TOWNSHIP 5 SOUTH - RANGE 16 EAST****SECTION 1:****PARCEL A:**

Commence at the Southwest corner of the NW 1/4 of the NE 1/4, Section 1, Township 5 South, Range 16 East, Columbia County, Florida and run N 89 deg. 29' E along the South line of said NW 1/4 of NE 1/4, 332 feet for a Point of Beginning, and run thence North parallel to the West line of said NW 1/4 of NE 1/4, 495 feet, thence N 89 deg. 29' E, 440 feet, thence South 495 feet, thence S 89 deg. 29' W, 440 feet to the Point of Beginning

**PARCEL B:**

Commence at the Southwest corner of the NW 1/4 of the NE 1/4, Section 1, Township 5 South, Range 16 East, Columbia County, Florida and run N 89 deg. 29' E along the South line of said NW 1/4 of the NE 1/4, a distance of 50 feet for a Point of Beginning; run thence North along the East boundary of graded county road a distance of 539 feet; thence N 89 deg. 29' E a distance of 282 feet; thence South parallel to the West boundary of said NW 1/4 of NE 1/4, a distance of 539 feet to the South boundary of said NW 1/4 of NE 1/4; thence S 89 deg. 29' W, a distance of 282 feet to the Point of Beginning.

**LESS AND EXCEPT:**

Commence at the Southwest corner of the NW 1/4 of NE 1/4 in Section 1, Township 5 South, Range 16 East, Columbia County, Florida and run thence N 89 deg. 29' E along the South line of said NW 1/4 of NE 1/4, 332 feet; run thence North parallel to the West line of said NW 1/4 of NE 1/4, 539 feet for a Point of Beginning; run thence S 89 deg. 29' W, 282 feet; run thence South parallel to said West line of said NW 1/4 of NE 1/4 along the East right-of-way line of a county road sometimes known as Little Road a distance of 44 feet; run thence N 89 deg. 29' E, 282 feet; run thence North parallel to the West line of said NW 1/4 of NE 1/4, 44 feet to the Point of Beginning.



Inst:2002016051 Date:06/13/2002 Time:12:55:01  
Doc Stamp-Deed : 0.70  
TRK DC, P. DeWitt Case, Columbia County B:960 P:257

Signed, sealed and delivered  
in our presence:

Lisa Hicks  
(Signature of First Witness)  
Lisa Hicks  
(Typed Name of First Witness)

Charles S. Sparks (SEAL)  
Grantor  
CHARLES S. SPARKS  
Printed Name

Mary Lynn  
(Signature of Second Witness)  
MARY LYNN  
(Typed Name of Second Witness)

Sandra L. Sparks (SEAL)  
Grantor  
SANDRA L. SPARKS

STATE OF FLORIDA  
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 21  
day of June, 2002, by CHARLES S. SPARKS and SANDRA L. SPARKS,  
Husband and Wife who is personally known to me or who have produced  
\_\_\_\_\_ as identification and who did not take an oath.

My Commission Expires: 8-23-02 Vera Lisa Hicks  
Notary Public  
Printed, typed, or stamped name:





ATS# 15092

Inst: 2005020103 Date: 08/18/2005 Time: 14:37  
DC, P. DeWitt Cason, Columbia County 3:1055 P:1585

This instrument Prepared By:  
Michael H. Harrell  
Abstract & Title Services, Inc.  
382 SW Baya Drive  
Lake City, Florida 32025

### NOTICE OF COMMENCEMENT

TO WHOM IT MAY CONCERN: The undersigned hereby give notice that improvements will be made to certain real property and in accordance with Chapter 713.13, Florida Statutes, the following is provided in this Notice of Commencement:

1. Construction of a single family dwelling.
2. The name and address of the undersigned owner is: Abram Huber, and his wife, Angela Huber, and the interest of the undersigned owner in the property is: FEE SIMPLE
3. Legal Description: See Exhibit "A" attached hereto and by this reference made a part hereof.

The name and address of the contractor is: Isaac Construction, Inc., 144 SW Waterford Court, Suite 101, Lake City, Florida 32025

4. LENDER: Countrywide Home Loans, 6400 Legacy Drive, Plano, Texas 76024
5. Persons within the State of Florida designated by Owner upon whom notices of other documents may be served as provided in Section 713.13(1)a.7., Florida Statutes: NONE
6. In addition to himself, Owner designates, to receive a copy of the Lienor's Notice as provided in Section 713.06(2)(b) Florida Statutes.
7. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

\*Owner is used for singular or plural as context requires.

Signed, sealed and delivered in the presence:

Megan Marable  
Witness Megan Marable

Abram Huber  
Abram Huber

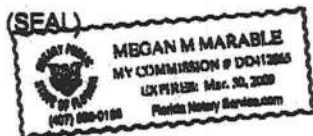
Traci Ladday  
Witness TRACI LADDAY

Angela D. Huber  
Angela D. Huber

STATE OF Florida  
COUNTY OF Columbia

Before me, personally appeared Abram Huber, and his wife, Angela Huber, known to be the person(s) described in and who executed the foregoing instrument, and they acknowledged to and before me that they executed said instrument for the purpose therein expressed.

Witness my hand and official seal this 9th day of August, 2005.



Megan M. Marable  
NOTARY PUBLIC

My Commission Expires:



Parcel A

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, 332 feet for a Point of Beginning, and run thence North parallel to the West line of said NW ¼ of the NE ¼, 495 feet, thence N 89°29' E, 440 feet; thence S 495 feet; thence S 89°29' W, 440 feet to the Point of Beginning.

Parcel B

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, a distance of 50 feet for a Point of Beginning; run thence North along the East boundary of graded county road a distance of 539 feet; thence N 89°29' E a distance of 282 feet; thence South parallel to the West boundary of said NW ¼ of the NE ¼, a distance of 539 feet to the South boundary of said NW ¼ of the NE ¼; thence S 89°29' W, a distance of 282 feet to the Point of Beginning.

LESS AND EXCEPT

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, 332 feet; run thence North parallel to the West line of said NW ¼ of the NE ¼, 539 feet for a Point of Beginning; run thence S 89°29' W, 282 feet; run thence South parallel to said West line of said NW ¼ of the NE ¼ along the East right-of-way line of a county road, sometimes known as Little Road, a distance of 44 feet; run thence N 89°29' E, 282 feet; run thence North parallel to the West line of said NW ¼ of the NE ¼, 44 feet to the Point of Beginning.

LESS AND EXCEPT

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, 50.00 feet to the Point of Beginning; thence continue N 89°29' E along said South line, 361.00 feet; thence due North 247.50 feet; thence S 89°29' W, 361.00 feet; thence due South, 247.50 feet to the Point of Beginning.

LESS AND EXCEPT

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, 411.00 feet to the Point of Beginning; thence continue N 89°29' E along said South line, 180.50 feet; thence due North, 495.00 feet; thence S 89°29' W, 180.50 feet; thence due South, 495.00 feet to the Point of Beginning.

LESS AND EXCEPT

Commence at the Southwest corner of the NW ¼ of the NE ¼, Section 1, Township 5 South, Range 16 East, Columbia County, Florida, and run thence N 89°29' E along the South line of said NW ¼ of NE ¼, 50.00 feet; thence run due North 247.50 feet to the Point of Beginning; thence continue due North, 247.50 feet; thence N 89°29' E, 361.00 feet; thence due South 247.50 feet; thence S 89°29' W, 361.00 feet to the Point of Beginning.

Inst:2005020403 Date:08/18/2005 Time:14:37

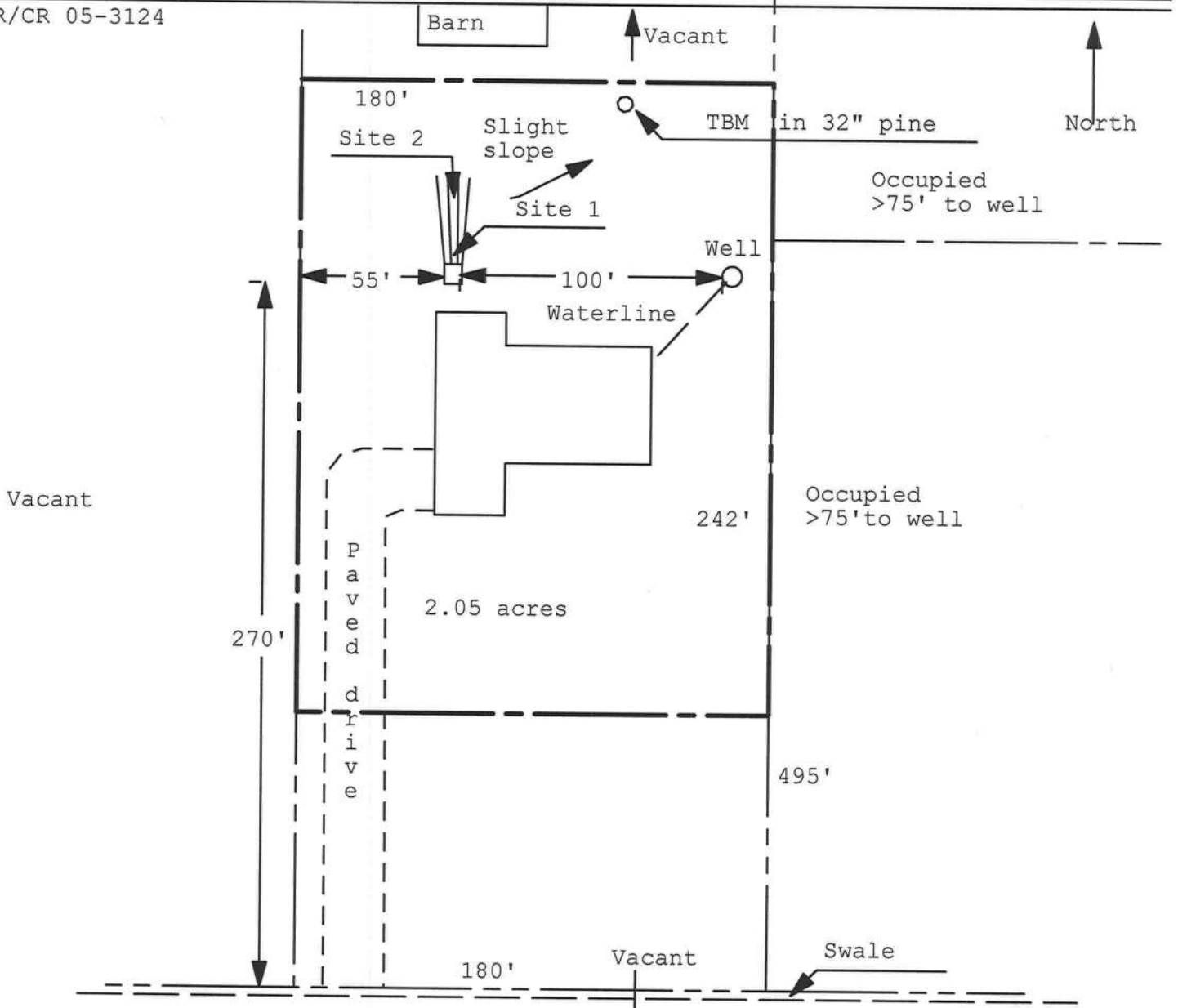
DC, P. DeWitt Cason, Columbia County B:1055 P:1596



**Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan**  
**Permit Application Number:** 05-1036N

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

HUBER/CR 05-3124



1 inch = 60 feet

Site Plan Submitted By Paul Lopez Date 10/3/05  
 Plan Approved  Not Approved  Date 10/14/05  
 By M. La Columbia CPHU

Notes: \_\_\_\_\_



FROM :

FAX NO. : 386-755-7022

Sep. 17 2002 01:52PM P1

# HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL  
OWNERS

PHONE (804) 752-1854  
FAX (804) 755-7022  
~~XXXXXXXXXXXXXXXXXXXX~~  
LAKE CITY, FLORIDA 32055  
904 NW Main Blvd.

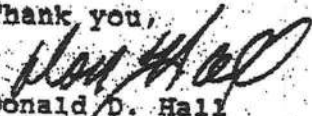
June 12, 2002

## NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

  
Donald D. Hall  
DDH/jk



Huber

FORM 600B-01

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION Residential Component Prescriptive Method B

NORTH 1 2 3

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME: AND ADDRESS: OWNER: ANGELA & ABRAHAM HUBER BUILDER: Isaac Coast PERMITTING OFFICE: Calumedia Co. PERMIT NO.: 23867 CLIMATE ZONE: 1 2 3 4 JURISDICTION NO.: 221000

GENERAL DIRECTIONS

- 1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass. 2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen. 3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels. 4. Complete page 1 based on the "To Be Installed" column information. 5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items. 6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

Please Print

CK

- 1. Compliance package chosen (A-F)
2. New construction or addition
3. Single family detached or Multifamily attached
4. If Multifamily—No. of units covered by this submission
5. Is this a worst case? (yes / no)
6. Conditioned floor area (sq. ft.)
7. Predominant eave overhang (ft.)
8. Glass type and area :
a. Clear glass
b. Tint, film or solar screen
9. Percentage of glass to floor area
10. Floor type, area or perimeter, and insulation:
a. Slab on grade (R-value)
b. Wood, raised (R-value)
c. Wood, common (R-value)
d. Concrete, raised (R-value)
e. Concrete, common (R-value)
11. Wall type, area and insulation:
a. Exterior: 1. Masonry (Insulation R-value)
2. Wood frame (Insulation R-value)
b. Adjacent: 1. Masonry (Insulation R-value)
2. Wood frame (Insulation R-value)
12. Ceiling type, area and insulation:
a. Under attic (Insulation R-value)
b. Single assembly (Insulation R-value)
13. Air Distribution System: Duct insulation, location
Test report (attach if required)
14. Cooling system
(Types: central, room unit, package terminal A.C., gas, none)
15. Heating system:
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)
16. Hot water system:
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)

1. A
2. New
3. Single family
4. 0
5. No
6. 4800
7. 2'
8a. Single Pane sq. ft. Double Pane 720 sq. ft.
8b. sq. ft. sq. ft.
9. 15 %
10a. R= 0 lin. ft.
10b. R= sq. ft.
10c. R= sq. ft.
10d. R= sq. ft.
10e. R= sq. ft.
11a-1 R= 13 3286 sq. ft.
11a-2 R= sq. ft.
11b-1 R= sq. ft.
11b-2 R= sq. ft.
12a. R= 30 3591 sq. ft.
12b. R= sq. ft.
13. R= 6
14a. Type: Central
14b. SEER/EER: 13.0
14c. Capacity: 8 Tons
15a. Type: Heat Pump
15b. HSPF/COP/AFUE:
15c. Capacity: 60 K
16a. Type: Elect.
16b. EF: 88

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code. PREPARED BY: DATE: 11-18-09

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: DATE:

DATE:



OCT 24 2005

\*\* LAMAR BOOZER \*\*  
 900 EAST PUTNAM STREET  
 LAKE CITY, FL 32055

PROJECT:  
 CLIENT: HUBER RES  
 DATE: 10 22 05  
 DESIGNER: LAMAR BOOZER

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

CLIENT INFORMATION:

NAME: HUBER RES  
 ADDRESS:  
 CITY, STATE: LAKE CITY, FLORIDA 32055

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	294	9,589	0	18,808	18,808
9-I FRENCH DOOR DBL CLR GLS METL FR	84	2,851	0	6,148	6,148
12-E WALL R-11 +1/2"EXTPOLY BD(R-2.5)	2,409	8,131	0	4,808	4,808
11-C DOOR METAL POLYSTYRENE CORE	57	1,206	0	713	713
16-G CEILING R-30 INSULATION	3,591	4,662	0	4,868	4,868
22-A SLAB ON GRADE NO EDGE INSUL	289	10,535	0	0	0
SUBTOTALS FOR STRUCTURE:		36,974	0	35,345	35,345
PEOPLE					
APPLIANCES	28	0	0	8,400	8,400
DUCTWORK	0	0	1,800	1,500	3,300
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	1,849	0	4,525	4,525
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				49,770	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		38,823	1,800	49,770	51,570

SUPPLY CFM AT 20 DEG DT: 2,262  
 SQUARE FT. OF ROOM AREA: 3,591  
 CFM PER SQUARE FOOT: 0.721  
 SQUARE FOOT PER TON: 730.425  
 TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 38.823 MBH  
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 6.298 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.  
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.



OCT 24 2005

\*\* LAMAR BOOZER \*\*  
 900 EAST PUTNAM STREET  
 LAKE CITY, FL 32055

PROJECT:  
 CLIENT: HUBER RES UP STAIRES  
 DATE: 10 22 05  
 DESIGNER: LAMAR BOOZER

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

CLIENT INFORMATION:

NAME: HUBER RES UP STAIRES  
 ADDRESS:  
 CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN	
3-C WINDOW DBL PANE CLR GLS METL FR	126	4,111	0	8,387	8,387	
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,186	4,268	0	2,334	2,334	
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462	462	
16-G CEILING R-30 INSULATION	1,246	1,850	0	1,850	1,850	
22-A SLAB ON GRADE NO EDGE INSUL	155	5,649	0	0	0	
SUBTOTALS FOR STRUCTURE:		2,753	16,724	0	13,033	13,033
PEOPLE		19	0	0	5,700	5,700
APPLIANCES		0	0	1,500	1,800	3,300
DUCTWORK		0	837	0	2,054	2,054
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0	0
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0	0
SENSIBLE GAIN TOTAL				22,587		
TEMP. SWING MULTIPLIER				X 1.00		
BUILDING LOAD TOTALS		17,561	1,500	22,587	24,087	

SUPPLY CFM AT 20 DEG DT: 1,027  
 SQUARE FT. OF ROOM AREA: 1,246  
 CFM PER SQUARE FOOT: 0.824  
 SQUARE FOOT PER TON: 620.750

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 17.561 MBH  
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.007 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.  
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.  
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.



# Columbia County Building Department Culvert Permit

## Culvert Permit No. 000000890

DATE 11/17/2005 PARCEL ID # 01-5S-16-03387-012

APPLICANT MELANIE RODER PHONE 752.2281

ADDRESS 387 SW KEMP CT LAKE CITY FL 32024

OWNER ABRAM & ANGELA HUBER PHONE 755.1102

ADDRESS 225 SW FINLEY LITTLE LN LAKE CITY FL 32024

CONTRACTOR ISAAC BRATKOVICH PHONE 719.714

LOCATION OF PROPERTY 47-S TO LITTLE RD(ENTRANCE OF SOUTHWOOD ACRES),TL ON FINLEY LITTLE AND  
IT'S THE 3RD LOT DOWN ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT \_\_\_\_\_

SIGNATURE Melanie Roder

### INSTALLATION REQUIREMENTS

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

Culvert installation shall conform to the approved site plan standards.

Department of Transportation Permit installation approved standards.

Other \_\_\_\_\_

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED  
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21  
Lake City, FL 32055  
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



tb 23867



# New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525  
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

**Public reporting burden** for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

## Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc. \* 23867  
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055  
Company Business License No. JB109476 Company Phone No. 386-755-3611  
FHAVA Case No. (if any) \_\_\_\_\_

## Section 2: Builder Information

Company Name: Truett Termite Company Phone No. \_\_\_\_\_

## Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 725 SW Folly LANE

Type of Construction (More than one box may be checked)  Slab  Basement  Crawl  Other \_\_\_\_\_  
Approximate Depth of Footing: Outside 17 Inside 24 Type of Fill DOT

## Section 4: Treatment Information

Date(s) of Treatment(s) 12-14-05  
Brand Name of Product(s) Used Proshield 77  
EPA Registration No. 100-1006  
Approximate Final Mix Solution % 0.25%  
Approximate Size of Treatment Area: Sq. ft. 4300 Linear ft. 374 Linear ft. of Masonry Voids 374  
Approximate Total Gallons of Solution Applied 997  
Was treatment completed on exterior?  Yes  No  
Service Agreement Available?  Yes  No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) \_\_\_\_\_

Comments \_\_\_\_\_

Name of Applicator(s) Steve Brance Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [Signature] Date \_\_\_\_\_

**Warning:** HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

# New Construction Subterranean Termites Soil Treatment Record

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number. Section 24 CFR 200.8 (b)(4) requires that the rules for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Authorized pest control companies, mortgage lenders, mortgage brokers, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This record is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is required by the builder, architect, or required by the lender, architect, FHA, or VA. All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

### Section 1: General Information (Treating Company Information)

Company Name: TERMITES  
Company Address: 12345 Main St  
Company Business License No: 123456789  
FHA/VA Case No. (if any): \_\_\_\_\_  
City: \_\_\_\_\_ State: \_\_\_\_\_ Zip: \_\_\_\_\_  
Company Phone No.: \_\_\_\_\_

### Section 2: Builder Information

Company Name: \_\_\_\_\_  
Company Phone No.: \_\_\_\_\_

### Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip): \_\_\_\_\_  
Approximate Depth of Footing: Outside \_\_\_\_\_ Inside \_\_\_\_\_  
Type of Construction (More than one box may be checked):  Slab  Basement  Crawl  Other \_\_\_\_\_  
Type of Fill: \_\_\_\_\_

### Section 4: Treatment Information

Date(s) of Treatment(s): \_\_\_\_\_  
Brand Name of Product(s) Used: \_\_\_\_\_  
EPA Registration No.: \_\_\_\_\_  
Approximate Final Mix Solution: \_\_\_\_\_  
Approximate Size of Treatment Area: Sq. ft. \_\_\_\_\_ Linear ft. of Masonry Voids: \_\_\_\_\_  
Approximate Total Gallons of Solution Applied: \_\_\_\_\_  
Was treatment completed on exterior?  Yes  No  
Service Agreement Available?  Yes  No  
Note: Some state laws require service agreement to be issued. This form does not prevent state law.

Attachments (List): \_\_\_\_\_  
Comments: \_\_\_\_\_

Name of Applicant(s): \_\_\_\_\_  
Certification No. (if required by State law): \_\_\_\_\_

The applicant has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature: \_\_\_\_\_  
Date: \_\_\_\_\_

FEAGLE & FEAGLE, ATTORNEYS, P.A.  
ATTORNEYS AT LAW  
153 NE MADISON STREET  
POST OFFICE BOX 1653  
LAKE CITY, FLORIDA 32056-1653  
(386) 752-7191  
Fax: (386) 758-0950

Marlin M. Feagle  
e-mail: leagle@bellsouth.net

Mark E. Feagle  
e-mail: mefeagle@bellsouth.net

June 14, 2006

**HAND DELIVERED**

Mr. John Kerce  
Planning and Zoning Department  
County Administrative Offices  
135 NE Hernando Avenue  
Lake City, Florida 32055

Re: Abram B. Huber

Dear John:

This will confirm our telephone conference of June 14, 2006 regarding the custom built window issue for Mr. Huber's residence here in Columbia County.

As stated in previous correspondence to Mr. Huber, Florida Statute 553.842(10) provides products which are custom fabricated or assembled shall not require separate approval under the section provided the component parts have been approved for the fabricated or assembled products used and the components meet the standards and requirements of the Florida Building Code which applies to the product's intended use. Section R613.3.1 provides that exterior windows shall be tested by an approved independent testing laboratory, etc. The determination of load resistance of glass for specified loads of products tested and certified in accordance with this section shall be designed to comply with ASTM E 1300. Section R613.4.2 provides for and allows exterior door assemblies (presumably including those containing glass) may be engineered in accordance with accepted engineering practices in lieu of testing by an approved testing laboratory.

Mr. Huber has obtained an engineering analysis and certification from Mark Disosway, P.E., as evidenced in his letter to Columbia County dated May 30, 2006. Engineer Disosway has given the opinion that in accordance with accepted engineering practices both the windows and the doors used in Mr. Huber's house meet the requirements of the 2004 Florida Building Code. I further discussed this with Mr. Disosway on June 14, 2006 at which time Mr. Disosway explained his letter and report. Attached to the Disosway report of May 30, 2006 is the ASTM E 1300 calculation for the largest window pane. This calculation shows the 110 MPH wind load pressure requirement as 30.0 per square feet (PSF). The calculated load resistance for the Huber



Mr. John Kerce  
Page 2  
June 14, 1006

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window panes is 43.8 PSF. Thus, the Huber window panes are 13.8 PSF greater than that required by the Florida Building Code according to Mr. Disosway's calculations. Mr. Disosway's letter of opinion further specifically states the window glass meets the 2004 Florida Building Code structural requirements. He did caution that our building inspector should make sure the installation is in compliance with Section 3CV of his letter. This section requires that the builder attach window jambs to wall studs with #8 by 3-inch wood screws at 12-inch OC maximum.

It appears the engineer's analysis letter and opinion meets the intent of Section 613 of the Florida Building Code regarding exterior windows. However, as an additional precaution of the county, I would suggest that the owner and contractor sign the enclosed indemnity and hold harmless agreement for the county. If you have any additional questions, please do not hesitate to give me a call.

Very truly yours,

  
Marlin M. Feagle

MMF:dse


Enclosure




**RELEASE AND HOLD HARMLESS AGREEMENT**

The undersigned, **ABRAM B. HUBER**, (“Owner”), and ISAAC CONSTRUCTION, (“Contractor”), each as to themselves, their heirs, successors and assigns, having been advised of the requirements of Section R613 of the Florida Building Code regarding exterior windows and door assemblies, and in consideration of the Columbia County Building Department approving the manufactured or custom-built doors and windows for the Owner’s residence located in Columbia County, Florida, do hereby release and agree to defend, reimburse, and hold harmless Columbia County, Florida, (“County”), regarding any claims, losses or damages against County as a result of the Owner’s residence not being constructed in full compliance with the provisions of the Florida Building Code; specifically as to Section R613 thereof. The undersigned represents and warrants that the residence has been and will be constructed in accordance with the Florida Building Code and other applicable rules and regulations.

DATED this 15<sup>th</sup> day of June, 2006.

  
\_\_\_\_\_  
**ABRAM B. HUBER**  
ABRAM HUBER  
Print Name: \_\_\_\_\_

“OWNER”

  
\_\_\_\_\_  
Print Name: ISAAC BRATKOVICH

“CONTRACTOR”



**Mark Disosway, P.E.**  
 POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4871

May 30, 2006

Building and Zoning, Columbia County, Florida

Re: Columbia County Building Permit # 23867  
 One Time Product Approval for Custom Teak Windows and Doors  
 Abram and Angela Huber Residence, 01-5S-16-03387-012 Columbia County, Florida

Dear Building Inspector:

Abram and Angela Huber are requesting one time local product approval for use of custom teak windows and doors for their new residence here in Columbia County. The owner's objective of using these custom products is to provide a more stable, rot and insect proof window and door to withstand the rigorous weather and tropical conditions here in Florida, and the desired aesthetic of which windows and doors are largely a part of in the house design.

- 1) Code basis for one time product approval of custom windows and doors.
  - a) Florida Statute 553.842(10) states: "Products, other than manufactured buildings, which are custom fabricated or assembled shall not require separate approval under this section provided the component parts have been approved for the fabricated or assembled product's use and the components meet the standards and requirements of the Florida Building Code which applies to the product's intended use." The windows and doors are custom fabricated of two basic materials, glass and teak wood. This letter will demonstrate that the glass and teak for the windows and doors used in this house meet the requirements of FBC2004.
  - b) FBC2004, R613.4.2 Custom doors states: "Custom (one of a kind) exterior door assemblies shall be tested by an approved testing laboratory or be engineered in accordance with accepted engineering practices." This letter will demonstrate with accepted engineering practices that the windows and doors used in this house meet the requirements of FBC2004. We are asking the building department to accept approval of the custom windows also even though the code language is for doors.
- 2) Window assembly description
  - a) Continuous flashing / bracket / flange 0.032 Aluminum attached to teak frame using 5/8" length 1/4" crown galv. staples fastened through polyurethane flashing glue into teak wood. This bracket extends 4" onto wall, and is fastened to wall every 12" using 1.75" #8 pan head wood screws.
  - b) Glass panels are double insulated low-e glass panels, each 1/8" thickness, with a 1/4" spacer gasket, for a total glass panel thickness of 1/2" all glass annealed except door glass panels which are tempered in accordance with building codes. Glass panels are attached using 1/16" polyethylene glazing tape and held in place at specified 25% compression with mechanically fastened muntins
  - c) Doors are attached using #8X3" wood screws
- 3) Engineering analysis
  - a) Design loads from Table R301.2(2) adjusted for height and exposure per Table R301.2(3) (Design wind speed = 100 mph, Exposure = B, Mean Roof Height = 30 ft, Coefficient = 1.00)

Wall Zone	Effective Wind Area (Ft <sup>2</sup> )	110 mph	110 mph
4	10	21.8	-23.6
4	20	20.8	-22.6
4	50	19.5	-21.3
4	100	18.5	-20.4
5	10	21.8	-29.1
5	20	20.8	-27.2
5	50	19.5	-24.6
5	100	18.5	-22.6



b) Doors

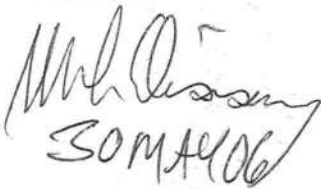
- i) Worst case wind pressure = -27.3 psf for 20 ft<sup>2</sup> door in Zone 5.
- ii) Worst case door structural element is the latch side stile which is simply supported from a latch mechanism to be installed by owner which latches at top and bottom of the stile. Due to the thin section of the stile deflection controls. Assume: 8' door height, latch at top and bottom of door, 2.0" x 5.0" stile, teak wood.
- iii) Max deflection =  $WL^3 / 76.8EI = 27.3 \text{ psf} * 8' * 1.5' * 96''^3 / 76.8 * 1700000 \text{ psi} * (5'' * 2''^3 / 12) = 0.666'' = L/144$  is greater than code limits of L/180 for a window but the deflection specified for a door is that after 10 seconds of testing at 1.5 times design wind pressure there will be "no permanent deformation of any main frame or panel member in excess of 0.4 percent of its span after the load is removed. After each specified loading, there shall be no glass breakage, permanent damage to fasteners, hardware parts, or any other damage which causes the door to be inoperable." Based on experience I would expect a wood door to have no permanent deflection after 10 seconds at deflection of  $1.5 * L/144 = L/96$ .
- iv) Attach door jambs to wall studs with #8 x 3" wood screws at 12"OC max. (123 lb per screw / (1.5 x 27.3 psf x 3') = 1.0')

c) Windows

- i) Worst case wind pressure = -28 psf for 15 ft<sup>2</sup> window in Zone 5.
- ii) Window glass as described above meets FBC2004 structural requirements for 30 psf design pressure as shown in the ASTM E1300 calculation for the largest pane, attached.
- iii) Worst case door structural element is the latch side or hinge side stile which is simply supported from a latch or hinge mechanism to be installed by owner with a maximum spacing between latches of 42". Due to the thin section of the stile deflection controls. Assume: 8' door height, latch at top and bottom of door, 2.0" x 2.5" stile, teak wood.
- iv) Max deflection =  $WL^3 / 76.8EI = 28 \text{ psf} * 3.5' * 1.5' * 42''^3 / 76.8 * 1700000 \text{ psi} * (2.5'' * 2''^3 / 12) = 0.05'' = L/840$  is less deflection than code limits of L/180
- v) Attach window jambs to wall studs with #8 x 3" wood screws at 12"OC max. (123 lb per screw / (1.5 x 27.3 psf x 3') = 1.0')

Please accept this letter as documentation that the custom one of a kind windows and doors used in this house meet the structural requirements of FBC2004 based on accepted engineering practices.

Sincerely,



Mark Disosway

Mark Disosway, PE



**ASTM E 1300 Design**

**Glass Construction**

Monolithic Single Glazing  
 Laminated Single Glazing  
 **Double Glazed Insulating Unit**

**System of Units**

**US Standard**  
 SI

**Glazing Position**

**Vertical**  
 Sloped

**Outboard Lite**

Check for Laminated  
 Glass Type: **Annealed**  
 Thickness Designation: **1/8** in.

**Inboard Lite**

Check for Laminated  
 Glass Type: **Annealed**  
 Thickness Designation: **1/8** in.

**Rectangular Dimensions**

Width: **30.25** in.  
 Height: **60.5** in.

**Loads**

Short Duration (<=60 sec): **30** psf  
 Long Duration (app 30 days): **0** psf

**Calculate**

**Results**

**Double Glazed Insulating Unit**

**Short Duration Load, Resistance and Deflection Data:**

Load (<=60 sec.):	30.0 psf
Load Resistance:	43.8 psf
Approximate Center of Glass Deflection under the Applied Load:	0.4 in.

Based on your design information, this glass configuration will resist the specified loading.

**Ok**



## SECTION R613

### EXTERIOR WINDOWS AND DOOR ASSEMBLIES

#### R613.1 General.

This section prescribes performance and construction requirements for exterior window systems installed in wall systems. Waterproofing, sealing and flashing systems are not included in the scope of this section.

#### R613.2 Performance.

Exterior windows and doors shall be designed to resist the design wind loads specified in Table R301.2(2) adjusted for height and exposure per Table R301.2(3).

#### R613.3 Exterior windows, sliding and patio glass doors.

##### R613.3.1 Testing and Labeling.

Exterior windows and glass doors shall be tested by an approved independent testing laboratory, and shall be labeled with an approved label identifying the manufacturer, performance characteristics and approved product certification agency, testing laboratory, evaluation entity or Miami-Dade notice of acceptance to indicate compliance with the requirements of one of the following specifications:

ANSI/AAMA/NWDA 101/I.S. 2-97 or 101/I.S. 2/NAFS or TAS 202 (HVHZ shall comply with TAS 202 utilizing ASTM E 1300-98 or ASTM E 1300-02).

Glass Strength: Determination of load resistance of glass for specified loads of products tested and certified in accordance with s. R613.3.1 shall be designed to comply with ASTM E 1300.

#### R613.4 Exterior door assemblies.

Exterior door assemblies not covered by R613.3 or R613.4.1 shall be tested for structural integrity in accordance with ASTM E 330 Procedure A at a load of 1.5 times the required design pressure load. The load shall be sustained for 10 seconds with no permanent deformation of any main frame or panel member in excess of 0.4 percent of its span after the load is removed. HVHZ shall comply with TAS 202. After each specified loading, there shall be no glass breakage, permanent damage to fasteners, hardware parts, or any other damage which causes the door to be inoperable.

The minimum test sizes and minimum design pressures shall be as indicated in Table R613.4

The unit size tested shall qualify all units smaller in width and/or height of the same operation type and be limited to cases where frame, panels and structural members maintain the same profile as tested.

#### R308.5 Site built windows.

Site built windows shall comply with Section 2404 of the Florida Building Code, Building.

##### 2404.1 Vertical glass.

Glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads for components and cladding. The load resistance of glass under uniform load shall be determined in accordance with ASTM E 1300. Design of exterior windows and glass doors in accordance with Section 2404.1 shall utilize the same edition of ASTM E 1300 used for testing in accordance with Section 1714.5. The design of vertical glazing shall be based on the following equation:

$$F_{gw} \leq F_{ga} \quad (\text{Equation 24-1})$$

where:

$F_{gw}$  is the wind load on the glass computed in accordance with Section 1609 and  $F_{ga}$  is the short duration load resistance of the glass as determined in accordance with ASTM E 1300.



553.842 Product evaluation and approval.--

(1) The commission shall adopt rules under ss. 120.536(1) and 120.54 to develop and implement a product evaluation and approval system that applies statewide to operate in coordination with the Florida Building Code. The commission may enter into contracts to provide for administration of the product evaluation and approval system. The product evaluation and approval system shall provide:

- (a) Appropriate promotion of innovation and new technologies.
- (b) Processing submittals of products from manufacturers in a timely manner.
- (c) Independent, third-party qualified and accredited testing and laboratory facilities, product evaluation entities, quality assurance agencies, certification agencies, and validation entities.
- (d) An easily accessible product acceptance list to entities subject to the Florida Building Code.
- (e) Development of stringent but reasonable testing criteria based upon existing consensus standards, when available, for products.
- (f) Long-term approvals, where feasible. State and local approvals will be valid until the requirements of the code on which the approval is based change, the product changes in a manner affecting its performance as required by the code, or the approval is revoked.
- (g) Criteria for revocation of a product approval.
- (h) Cost-effectiveness.

(2) The product evaluation and approval system shall rely on national and international consensus standards, whenever adopted by the Florida Building Code, for demonstrating compliance with code standards. Other standards which meet or exceed established state requirements shall also be considered.

(3) Products or methods or systems of construction that require approval under s. 553.77, that have standardized testing or comparative or rational analysis methods established by the code, and that are certified by an approved product evaluation entity, testing laboratory, or certification agency as complying with the standards specified by the code shall be approved for statewide use. Products required to be approved for statewide use shall be approved by one of the methods established in 1 subsection (6) without further evaluation.

(4) Products or methods or systems of construction requiring approval under s. 553.77 must be approved by one of the methods established in 2 subsection (5) or 1 subsection (6) before their use in construction in this state. Products may be approved by the commission for statewide use. Notwithstanding a local government's authority to amend the Florida Building Code as provided in this act, statewide approval shall preclude local jurisdictions from requiring further testing, evaluation, or submission of other evidence as a condition of using the product so long as the product is being used consistent with the conditions of its approval.

(5) Statewide approval of products, methods, or systems of construction may be achieved by one of the following methods. One of these methods must be used by the commission to approve the following categories of products: panel walls, exterior doors, roofing, skylights, windows, shutters, and structural components as established by the commission by rule.

(a) Products for which the code establishes standardized testing or comparative or rational analysis methods shall be approved by submittal and validation of one of the following reports or listings indicating that the product or method or system of construction was evaluated to be in compliance with the Florida Building Code and that the product or method or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code:

1. A certification mark or listing of an approved certification agency;
2. A test report from an approved testing laboratory;
3. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, from an approved product evaluation entity; or
4. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, developed and signed and sealed by a professional engineer or architect, licensed in this state.

A product evaluation report or a certification mark or listing of an approved certification agency which demonstrates that the product or method or system of construction complies with the Florida Building Code for the purpose intended shall be equivalent to a test report and test procedure as referenced in the Florida Building Code.

(b) Products, methods, or systems of construction for which there are no specific standardized testing or comparative or rational analysis methods established in the code may be approved by submittal and validation of one of the following:

1. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, from an approved product evaluation entity indicating that the product or method or system of construction was evaluated to be in compliance with the intent of the Florida Building Code and that the product or method or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code; or
2. A product evaluation report based upon testing or comparative or rational analysis, or a combination thereof, developed and signed and sealed by a professional engineer or architect, licensed in this state, who certifies that the product or method or system of construction is, for the purpose intended, at least equivalent to that required by the Florida Building Code.

(6) The commission shall ensure that product manufacturers that obtain statewide product approval operate quality assurance programs for all approved products. The commission shall adopt by rule criteria for operation of the quality assurance programs.

(7) For state approvals, validation shall be performed by validation entities approved by the commission. The commission shall adopt by rule criteria for approval of validation entities, which shall be third-party entities independent of the product's manufacturer and which shall certify to the commission the product's compliance with the code.

(8) The commission may adopt rules to approve the following types of entities that produce information on which product approvals are based. All of the following entities, including engineers and architects, must comply with a nationally recognized standard demonstrating independence or no conflict of interest:

(a) Evaluation entities that meet the criteria for approval adopted by the commission by rule. The commission shall specifically approve the National Evaluation Service, the International Conference of Building Officials Evaluation Services, the International Code Council Evaluation Services, the Building Officials and Code Administrators International Evaluation Services, the Southern Building Code Congress International Evaluation Services, and the Miami-Dade County Building Code Compliance Office Product Control. Architects and engineers licensed in this state are also approved to conduct product evaluations as provided in subsection (5).

(b) Testing laboratories accredited by national organizations, such as A2LA and the National Voluntary Laboratory Accreditation Program, laboratories accredited by evaluation entities approved under paragraph (a), and laboratories that comply with other guidelines for testing laboratories selected by the commission and adopted by rule.

(c) Quality assurance entities approved by evaluation entities approved under paragraph (a) and by certification agencies approved under paragraph (d) and other quality assurance entities that comply with guidelines selected by the commission and adopted by rule.

(d) Certification agencies accredited by nationally recognized accreditors and other certification agencies that comply with guidelines selected by the commission and adopted by rule.

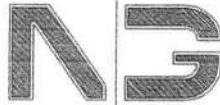
(e) Validation entities that comply with accreditation standards established by the commission by rule.

(9) A building official may deny the local application of a product or method or system of construction which has received statewide approval, based upon a written report signed by the official that concludes the product application is inconsistent with the statewide approval and that states the reasons the application is inconsistent. Such denial is subject to the provisions of s. 553.77 governing appeal of the building official's interpretation of the code.



(10) Products, other than manufactured buildings, which are custom fabricated or assembled shall not require separate approval under this section provided the component parts have been approved for the fabricated or assembled product's use and the components meet the standards and requirements of the Florida Building Code which applies to the product's intended use.





**NICHOLAS  
PAUL  
GEISLER**  
**ARCHITECT**  
N.C.A.R.B. Certified

■ 1758 NW Brown Road  
■ Lake City, FL 32055  
■ 386/755-9021

08 NOVEMBER 2005

JOE HALTIWANGER, PLANS REVIEW  
COLUMBIA COUNTY, BUILDING DEPT.  
COLUMBIA COUNTY COURTHOUSE ANNEX  
LAKE CITY, FLORIDA 32055

RE: HUBER RESIDENCE  
PLAN REVIEW Nr.: 0510-11

DEAR SIR:

WITH REGARD TO THE PLAN REVIEW NOTES YOU HAVE PROVIDED, THE FOLLOWING CLARIFICATIONS AND CORRECTIONS ARE PROVIDED. ITEM NUMBERS MATCH YOUR COMMENTS.

1. SEE ATTACHMENT.
2. SEE ATTACHMENT.
3. EGRESS FROM SLEEPING AREAS IS PROVIDED VIA DOORS TO THE OUTSIDE AND CASEMENT WINDOWS OF AT LEAST 26/40 SIZE WHICH WILL PROVIDE AN OPENING AREA GREATER THAN 5.7 SQFT. WITH A MINIMUM HEIGHT GREATER THAN 24" AND A MINIMUM WIDTH GREATER THAN 20".
4. GLASS AT TUBS AND SHOWERS IS MARKED WITH A SMALL CIRCLE, ENCLOSING A LETTER "T". THE INTENDED NOTATION IS TO REQUIRE TEMPERED GLASS AT SUCH LOCATIONS.
5. PLEASE REVIEW THE DETAIL OF THE STAIRS ON SHEET 6 OF 13.
6. SEE ATTACHMENT.
7. SEE ATTACHMENT.
8. FOUNDATION MODIFICATIONS ARE PENDING ENGINEERED FLOOR AND ROOF TRUSS SHOP DRAWINGS. FOLLOWING RECEIPT OF SAME, I WILL UPDATE THE FOUNDATION PLAN AS REQUIRED.
9. SEE ATTACHMENT.
10. SEE ATTACHMENT.
11. DESIGN LOADS FOR THE 2nd FLOOR SYSTEM SHALL BE A MINIMUM OF 30 PSF LL, 17 PSF DL.
12. SEE ATTACHMENT.



PAGE 2

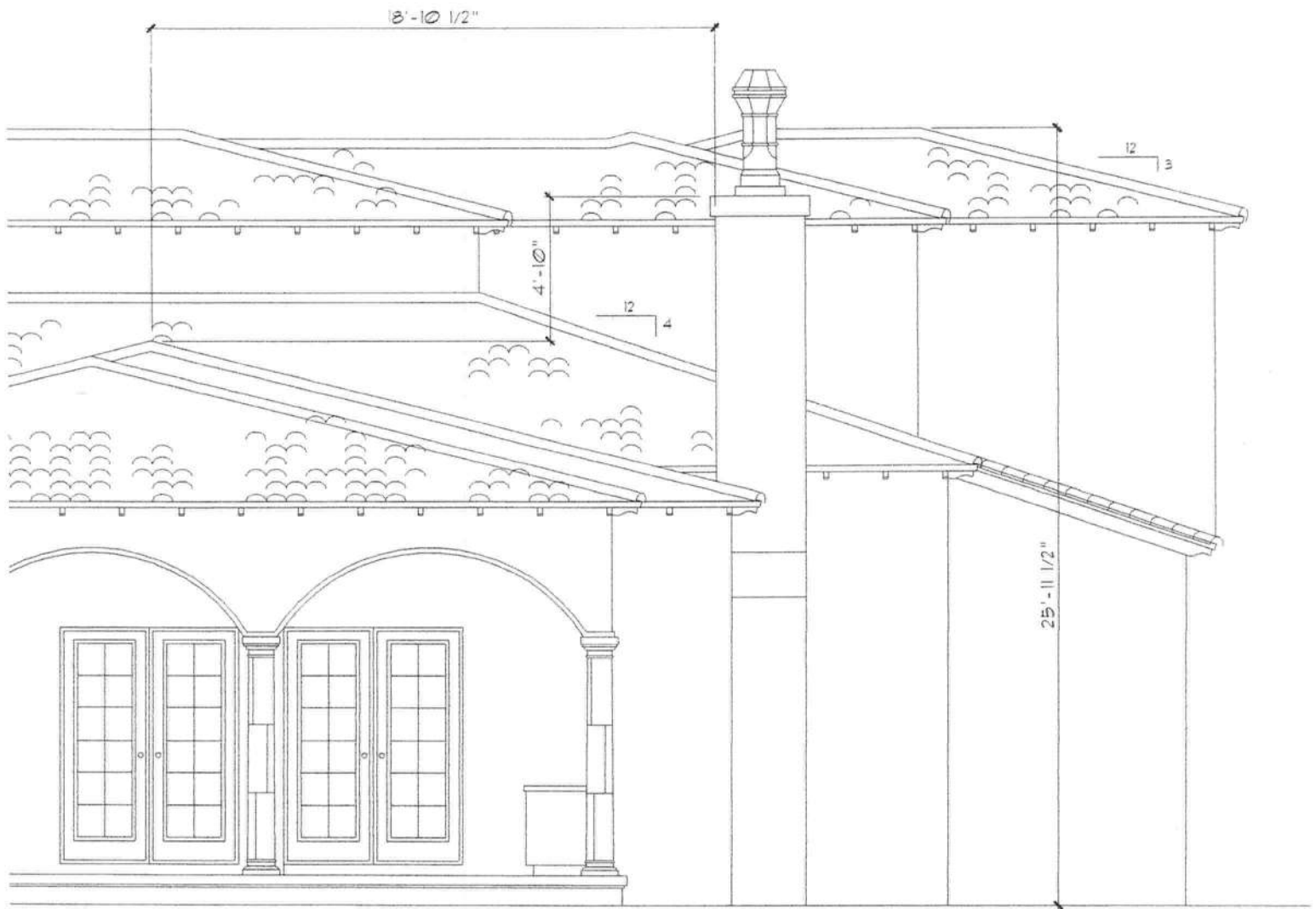
13. SEE ATTACHMENT.
14. REFER TO MANUFACTURERS SPECIFICATIONS, PROVIDED BY OTHERS.
15. SEE ATTACHMENT.
16. SEE ATTACHMENT.
17. PROVIDED BY OTHERS.
18. SEE ATTACHMENT.
19. AN EXTRA BLOCK APPEARED ON THE PLANS AND WAS NOT INTENDED TO BE PART OF THE FINAL DRAWINGS. THE EXTRA AHU INDICATED IN THE 2nd FLOOR CLOSET SHALL BE REMOVED.
20. PROVIDED BY OTHERS.

FOLLOWING REVIEW OF THE VARIOUS ATTACHMENTS, SHOULD YOU HAVE ANY FURTHER QUESTIONS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,  
NICHOLAS PAUL GEISLER, ARCHITECT ARO007005

A handwritten signature in black ink, appearing to read 'N. Geisler', with a long horizontal line extending to the right.





Items 1 & 2

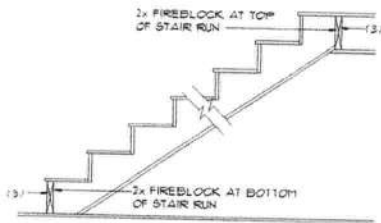
*AR 700T*  
*08 NOV 2005*



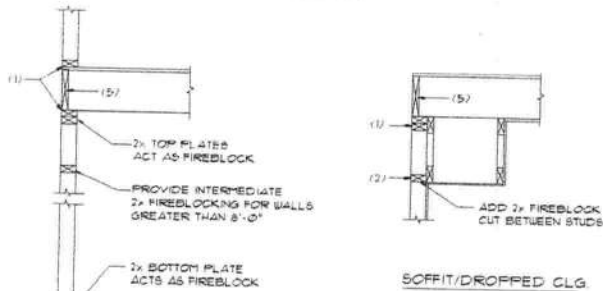
**FIREBLOCKING NOTES:**

FIREBLOCKING SHALL BE INSTALLED IN WOOD FRAME CONSTRUCTION IN THE FOLLOWING LOCATIONS:

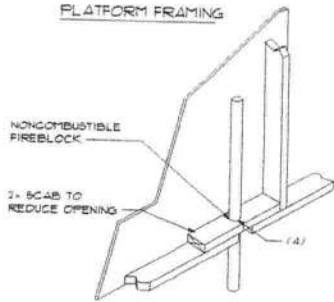
1. IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS INCLUDING FURRED SPACES AT CEILING AND FLOOR LEVELS.
2. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILINGS, COVE CEILINGS, ETC.
3. IN CONCEALED SPACES BETWEEN STAIR STRINGERS AT THE TOP AND BOTTOM OF THE RUN.
4. AT OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMNEYS AND FIREPLACES AT CEILING AND FLOOR LEVELS WITH "PYROPANEL MULTIFLEX SEALANT"
5. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL STUD WALL OR PARTITION SPACES AND CONCEALED SPACES CREATED BY AN ASSEMBLY OF FLOOR JOISTS, FIREBLOCKING SHALL BE PROVIDED FOR THE FULL DEPTH OF THE JOISTS AT THE ENDS AND OVER THE SUPPORTS.



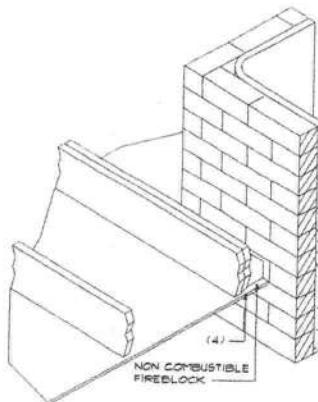
**BETWEEN STAIR STRINGERS**



**PLATFORM FRAMING**



**PENETRATIONS**



**FIREPLACE/CHIMNEY**

**Fire Stopping DETAILS**

SCALE: NONE

**NOTE**

ALL PENETRATIONS OF THE TOP PLATE OF ALL LOAD BEARING WALLS SHALL BE SEALED WITH FIRE RETARDANT CAULKING, INCLUDING WIRING, PLUMBING OR OTHER SUCH PENETRATIONS. WALLS OVER 8'-0" TALL SHALL HAVE CONTINUOUS BLOCKING TO LIMIT CAVITY HEIGHT TO 8'-0". PENETRATIONS THROUGH SUCH BLOCKING SHALL BE TREATED IN THE SAME MANNER AS TOP PLATES, NOTED ABOVE.

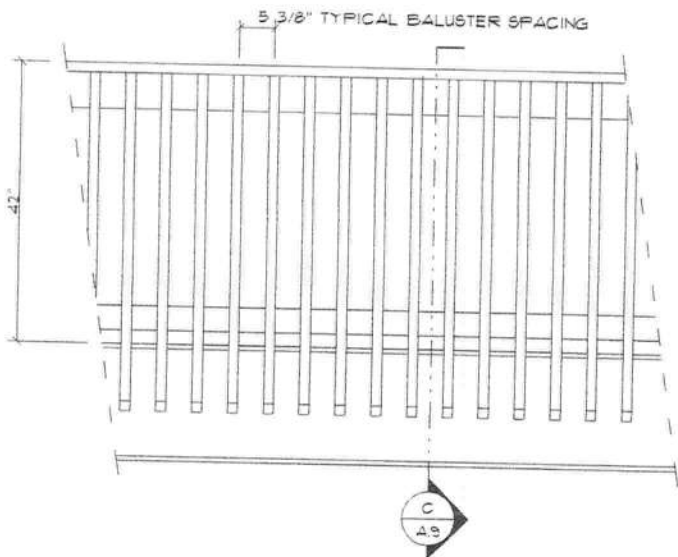
Items 6 & 16

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 MZ  
 08 NOV 2015



GENERAL NOTES:

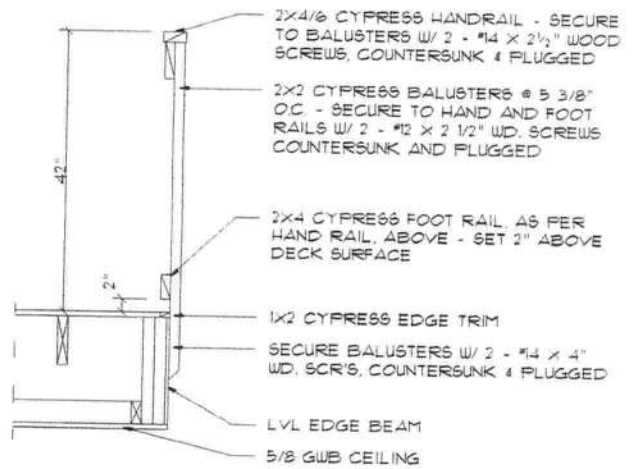
1. ALL LUMBER SHALL BE SOUTHERN BALD CYPRESS (RED OR YELLOW), GRADE 2 OR BETTER.
2. FASTENERS SHALL BE AS FOLLOWS:  
 \* 12d GALVANIZED FINISH NAILS  
 \* 14 X 4" GALV. WOOD SCREWS  
 \* 14 X 2 1/2" GALV. WOOD SCREWS  
 REFER TO DETAILS FOR APPLICATION.
3. ALL WOOD SHALL BE SMOOTH SANDED READY FOR APPLIED FINISH, AS DIRECTED BY THE OWNER.
4. ALL CUTS SHALL BE MADE PLUMB AND SQUARE.
5. FINISH NAILS SHALL BE SET BELOW SURFACE OF ATTACHED MEMBER.
6. ALL SCREW HEADS SHALL BE COUNTERSUNK AND PLUGGED, FLUSH WITH SURROUNDING WOOD SURFACE.
7. JOB CONDITIONS AS ENCOUNTERED SHALL BE TREATED PER THESE TYPICAL DETAILS - RAILINGS SHALL BE A MINIMUM OF 42" ABOVE WALKING SURFACE AND ALL OPENINGS SHALL BE LIMITED TO REJECT A 4" BALL.



Balcony Railing ELEV.

SCALE: 3/4" = 1'-0"

(B)



Balcony Railing SEC.

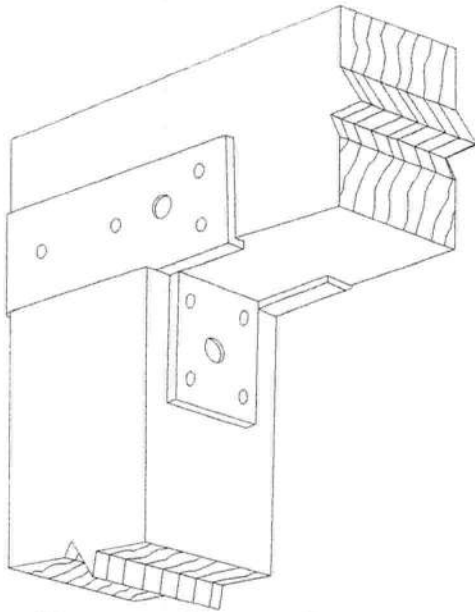
SCALE: 3/4" = 1'-0"

(C)

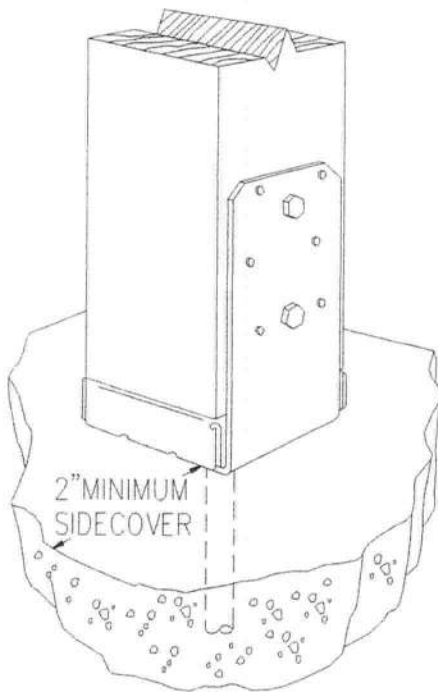
Item 7

*Handwritten signature and date:*  
 12/20/03  
 08 NOV 2003





Simpson Strong-Tie  
EPC66 - PC66 SIMILAR

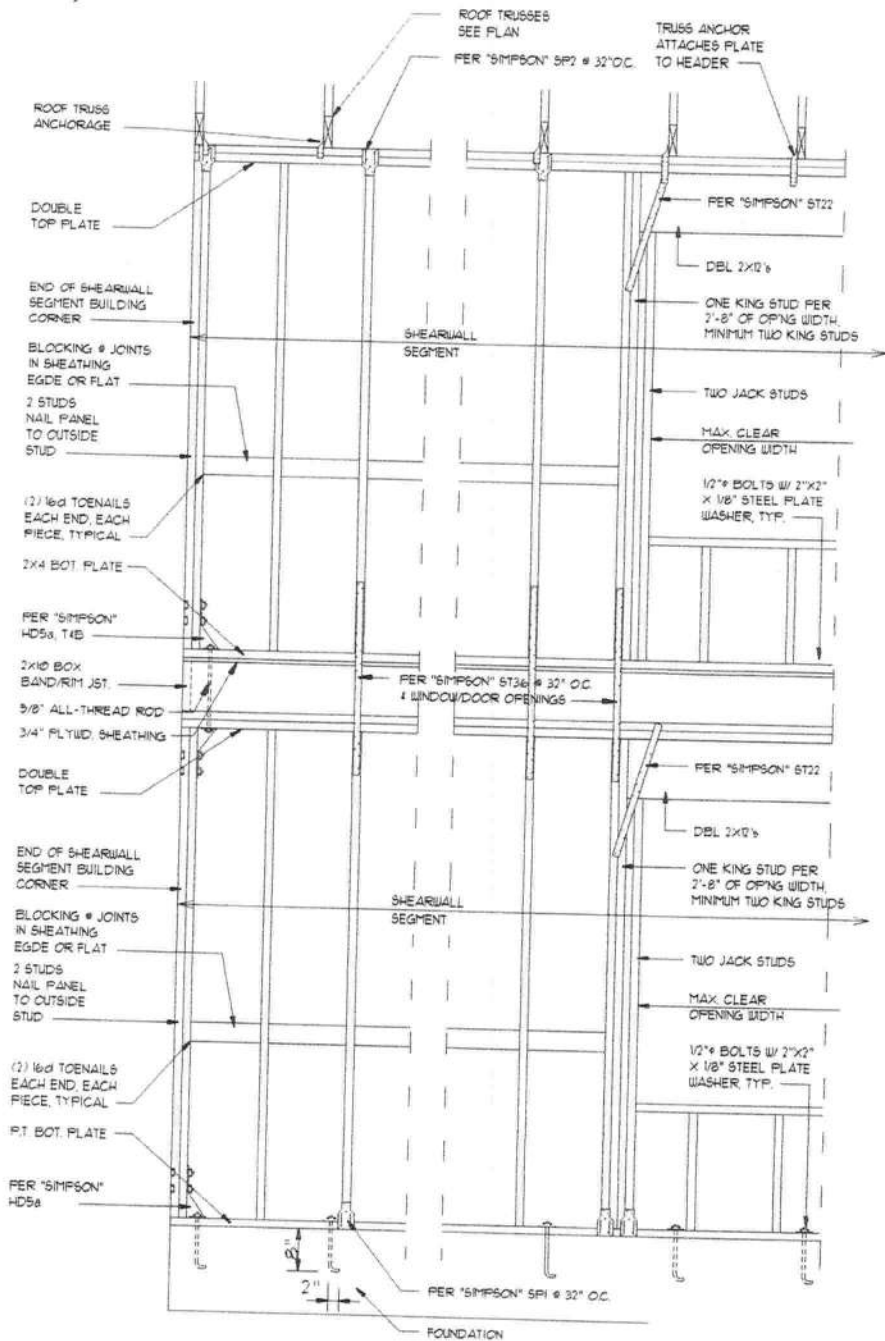


Simpson Strong-Tie  
ABU66

Item 9

*M*  
M27005  
& NEUZKE



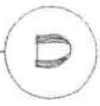


- SHEARWALL NOTES:**
1. ALL SHEARWALLS SHALL BE TYPE 2 SHEARWALLS AS DEFINED BY STD 10-97 SBC(1) 305.4.3
  2. THE WALL SHALL BE ENTIRELY SHEATHED WITH 7/16" OSB, INCLUDING AREAS ABOVE AND BELOW OPENINGS
  3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURRING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING
  4. NAIL SPACING SHALL BE 6" O.C. EDGES AND 12" O.C. IN THE FIELD.
  5. TYPE 2 SHEARWALLS ARE DESIGNED FOR THE OPENING IT CONTAINS. MAXIMUM HEIGHT OF OPENING SHALL BE 5/6 TIMES THE WALL HEIGHT. THE MINIMUM DISTANCE BETWEEN OPENINGS SHALL BE THE WALL HEIGHT/3.5 FOR 8'-0" WALLS (2'-3").

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6'-0"	(1) 2x4 OR (1) 2x6	1
6'-0" TO 9'-0"	(3) 2x4 OR (1) 2x6	3
9'-0" TO 12'-0"	(5) 2x4 OR (2) 2x6	3

# Shear Wall DETAILS

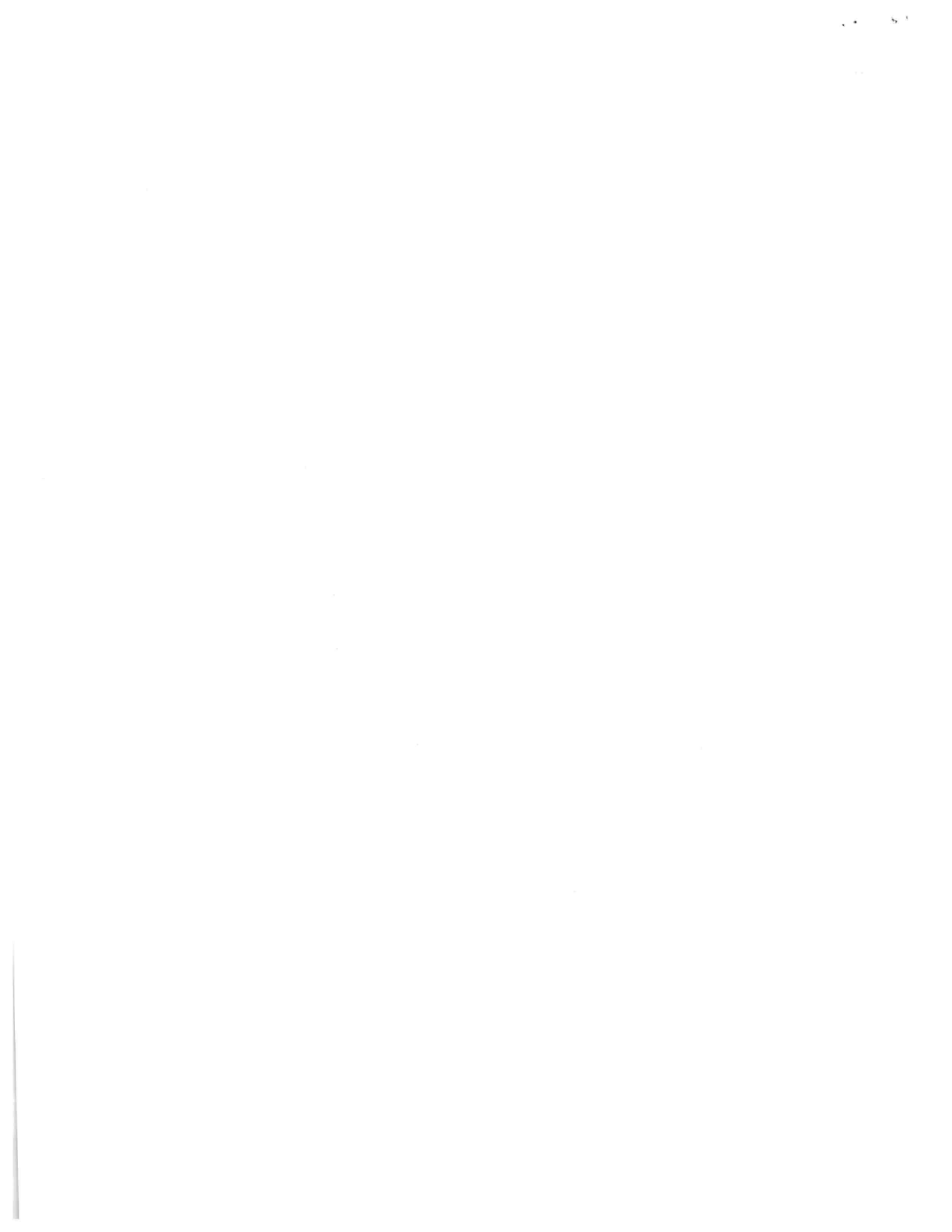
SCALE: NONE

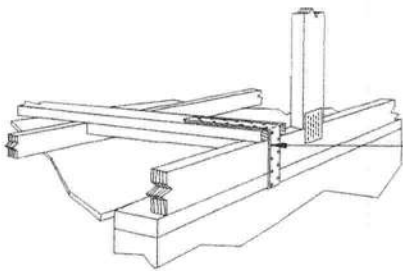


CONSTRUCT EXTERIOR WALLS W/ 2 TOP PLATES & 1 SILL PLATE, 2X4 STUDS @ 16" O.C., & "SIMPSON" SP2/SPI STUD/PLATE CONNECTORS @ 32" O.C. - SHEATH WALL W/ 7/16" OSB, APPLIED W/ 8d COMMON NAILS @ 4" O.C. ALONG EDGES & 8" O.C. ALONG INTERMEDIATE SUPPORTS

Items 10 & 13

*Handwritten signature and date:*  
 12/20/05  
 08 NOV 2005

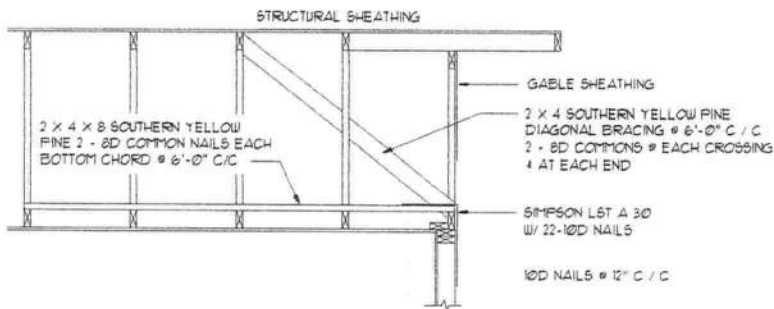




SIMPSON STRONG TIE LST30  
 WRAP END AROUND TRUSS ANCHOR  
 SEAT AND SECURE W/ 7-10d NAILS.  
 APPLY TO 2"x4" BRACE W/ 7-10d NAILS

## GABLE END GYPSUM DIAPHRAGM HOLDOWN CONNECTOR

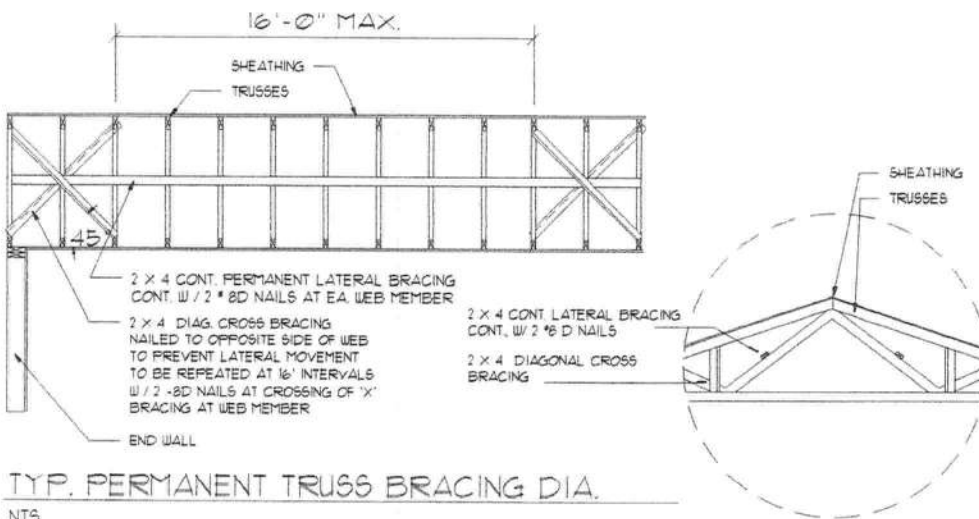
SCALE: NONE



## END WALL BRACING FOR CEILING DIAPHRAGM

NTS (ALTERNATIVE TO BALLOON FRAMING)

NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE



## TYP. PERMANENT TRUSS BRACING DIA.

NTS

NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE

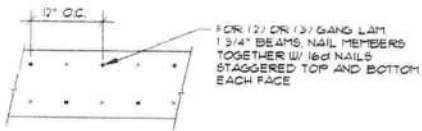
## Truss Bracing DETAILS

SCALE: AS NOTED

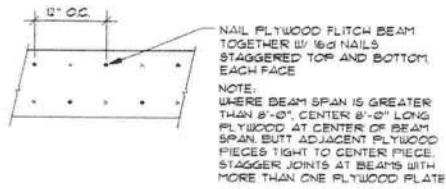
Item 12

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 AR7005  
 08 NOV 2015

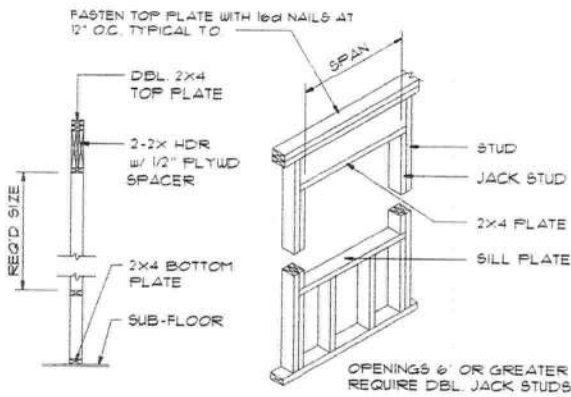




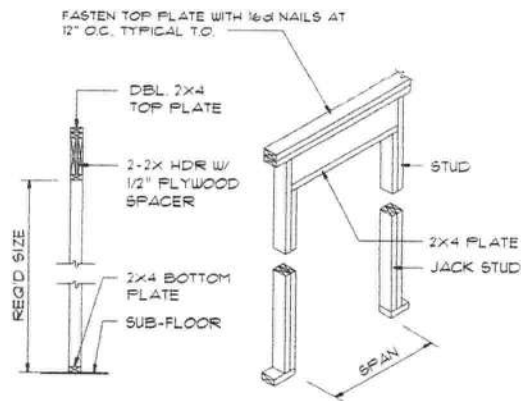
**MULTIPLE GANG LAM. DETAIL**  
NOT TO SCALE



**PLYWOOD FLITCH BEAM DETAIL**  
NOT TO SCALE



**TYPICAL WINDOW HEADER**



**BEARING WALL HEADER**

HEADERS SUPPORTING:		HEADER SPANS FOR EXTERIOR BEARING WALLS					
		BUILDING WIDTH (FT)					
HEADER SIZE		20'		28'		36'	
		SPAN	* JACKS	SPAN	* JACKS	SPAN	* JACKS
ROOF, CEILING	2-2x4	3'-6"	1	3'-2"	1	2'-10"	1
	2-2x6	5'-5"	1	4'-8"	1	4'-2"	1
	2-2x8	6'-10"	1	5'-11"	2	5'-4"	1
	2-2x10	8'-5"	2	7'-3"	2	6'-6"	2
	2-2x12	9'-9"	2	8'-5"	2	7'-6"	2
	3-2x8	8'-4"	1	7'-5"	1	6'-8"	1
	3-2x10	10'-6"	1	9'-1"	2	8'-2"	1
	3-2x12	12'-2"	2	10'-7"	2	9'-5"	2
	4-2x8	9'-2"	1	8'-4"	1	8'-2"	1
	4-2x10	11'-8"	1	10'-6"	1	9'-5"	1
	4-2x12	14'-1"	1	12'-2"	2	10'-11"	1

## Bearing Wall Header DETAILS

SCALE: NONE

NOTE ! FOR ALL SPAN CONDITIONS NOT LISTED ABOVE USE THE FOLLOWING BUILT-UP L.V.L BEAM ASSEMBLY

2 - 1 3/4" x 11 7/8" 2.0E MICRO=LAM L.V.L BEAM, EXTEND TOP PLY OF WALL PLATE FULL LENGTH, LAP MIN. 32" TO ADJOINING WALL, ASSEMBLE W/ 16d NAILS @ 12" O.C., STAGGERED TOP & BOTTOM OF BEAM, EACH SIDE

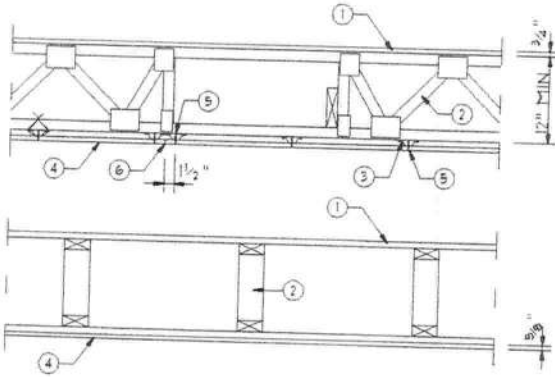
Item 15

*M. M. 08/2005 08/2005*



## Design No. L528

Unrestrained Assembly Rating-1 Hr.  
Finish Rating-22 Min.



1. Flooring System - Finish Flooring - 4 ft by 8 ft by 23/32 in. thick interior plywood with exterior glue and T & G edge detail along 8 ft sides. Plywood installed perpendicular to trusses with end joints staggered 4 ft. Plywood secured to trusses with construction adhesive and No. 6d ringed shank nails. Adhesive applied as 3/8 in. diam bead to top chord of trusses and groove edges of plywood. Nails spaced 12 in. O.C. along each truss. As an option, lightweight insulating concrete with Perlite or Vermiculite Aggregate or gypsum concrete may be placed on the flooring. The min thickness of insulating concrete shall be 3/4 in. The max thickness shall be determined by job site conditions. A thin plastic or paper vapor retarder may be placed on plywood prior to pouring the concrete. See Perlite Aggregate (IFFX) and Vermiculite Aggregate (CJZZ).

2. Trusses - Parallel chord trusses spaced a max 24 in. O.C. fabricated from nom 2 by 4 in. lumber with lumber orientated either vertically or horizontally. Truss members secured together with No. 20 MSG galv steel truss plates. Plates include 5/16 in. long teeth projecting perpendicular to the plane of the plate. The teeth are in pairs facing each other from the same punch creating a split tooth type plate. Each tooth has a chisel point on its outside edge, with these points being diagonally opposite from each other for each pair. The top half of each tooth has a twist for stiffness. The pairs are repeated on approx 7/8 in. centers with four rows of teeth per in. of plate width.

3. Furring Channels - Formed of No. 25 MSG galv steel spaced 24 in. O.C. perpendicular to trusses. Channels secured to trusses with double strand of No. 18 SWG galv steel wire spaced 48 in. O.C. Channels spliced with adjacent pieces overlapped 6 in. and tied with double strand of No. 18 SWG galv steel wire at each end of overlap.

3A. Resilient Channel - (Not shown) - As an alternate to item 3 - Formed from No. 26 MSG galv steel, spaced 16 in. O.C. perpendicular to trusses. Channels secured to trusses with Type 5, 1-1/4 in. long steel screws spaced 24 in. O.C. Channels overlapped at splice 4 in.

4. Wallboard Gypsum - 5/8 in. thick, 4 ft wide sheets of wallboard installed with long dimension perpendicular to furring or resilient channels with 1 in. long wallboard screws spaced 12 in. O.C. and located a min 1-1/2 in. from side and end joints. At end joints, two furring or resilient channels are used which extend a min of 6 in. beyond end of joint.

Canadian Gypsum Co., Ltd.-Type C.  
Celotex Corp.-Type FRP.  
Dontar Gypsum-Type 5  
Georgia-Pacific Corp. Gypsum Div.-Type GFF5-C.  
Gold Bond Building Products-Type F5W-G.  
United States Gypsum Co.-Types C, FCC, or IF-X2.

5. Screw, Wallboard - 1 in. long, Type 5, 3/64 in. diam, self-drilling and self-tapping, Bugle head.

6. Finishing System - (Not shown) - Paper tape embedded in cementitious compound over joints with edges of compound feathered out and exposed screw heads covered with compound. As an alternate, nom 3/32 in. thick veneer plaster may be applied to the entire surface of gypsum wallboard.

Bearing the UL Classification Marking

### NOTE 1

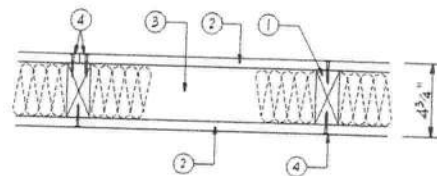
PROVIDE A FIRE RESISTIVE WALLS AROUND THE GARAGE AREA EQUAL TO UL Des. U333 - 1 HOUR

### NOTE 1

PROVIDE A FIRE RESISTIVE FLOOR/CEILING OVER THE GARAGE AREA EQUAL TO UL Des. L528 - 1 HOUR

## Design No. U333

Bearing Wall Rating-1 Hr.  
Finish Rating-23 Min.



1. Wood Studs - Nom 2 by 4 in. spaced 16 in. O.C. effectively cross-braced.

2. Gypsum Wallboard - 5/8 in. thick, 4 ft wide, applied either vertically or horizontally, screw attached to studs and plates with 1 1/4 in. long Type W steel screws, spaced 12 in. O.C.

Canadian Gypsum Co., Ltd.-Type C.  
Georgia-Pacific Corp. Gypsum Div.-Type GFF5-C.  
United States Gypsum Co.-Type C or IF-X2.

3. Bolts and Blankets - (Optional) - Mineral wool insulation, partially or completely filling stud cavity.

USG Interiors Inc.  
United States Gypsum Co.

4. Joints and Nailheads - Wallboard joints covered with paper tape and joint compound. Screwheads covered with joint compound.

Bearing the UL Classification Marking

Item 18

*Handwritten signature and date:*  
M27005  
08 NOV 2005

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From: The Columbia County Building Department  
Plans Review  
135 NE Hernando Av.  
P. O Box 1529  
Lake City Florida, 32056-1529

0510-71

Reference to: Build permit application Number:

ISAAC Construction Owner Abramand Huber 225 SW. Finley  
Little Lane

On the date of October 27, 2005 application 0510-71 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

**Please include application number 0510-71 when making reference to this application.**

1. On the elevation plans please show total height of the dwelling from finished grade.
2. On the elevation plans please show total height of the chimney above the roof line peak and also show the distance of the chimney flue from the nearest horizontal roof line.
3. Please show on the floor plans compliance with sections R310.1 of the FRC 2004 emergency escape and rescue requirements: R310.1.1 Minimum opening area. All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m<sup>2</sup>).  
Exception: Grade floor openings shall have a minimum net clear opening of 5 square feet (0.465 m<sup>2</sup>).  
R310.1.2 Minimum opening height. The minimum net clear opening height shall be 24 inches (610 mm).

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TEL: 734 763 1000  
WWW.LIBRARIES.UMICH.EDU

0310-71

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R310.1.3 Minimum opening width. The minimum net clear opening width shall be 20 inches (508 mm).

R310.1.4 Operational constraints. Emergency escape and rescue openings shall be operational from the inside of the room without the use of keys or tools.

R310.2 Window wells. The minimum horizontal area of the window well shall be 9 square feet (0.84 m<sup>2</sup>), with a minimum horizontal projection and width of 36 inches (914 mm). The area of the window well shall allow the emergency escape and rescue opening to be fully opened.

4. On the first and second story windows in the shower exterior walls show compliance with sections R308.4 of the FRC 2004 hazardous location.
5. Please provide a detail drawing to show compliance with section R311.5 and R311.6 Ramps. of the FRC 2004 Stairways: In occupancies in Group R-3, as applicable in Section 101.2, within dwelling units in occupancies in Group R-2, as applicable in Section 101.2, and in occupancies in Group U, which are accessory to an occupancy in Group R-3, as applicable in Section 101.2, the maximum riser height shall be 7.75 inches (197 mm) and the minimum tread depth, exclusive of nosing, shall be not less than 10 9 inches (254 mm), the minimum winder tread depth at the walk line shall be 10 inches (254 mm), and the minimum winder tread depth shall be 6 inches (152 mm). A nosing not less than 0.75 inch (19.1 mm) but not more than 1.25 inches (32 mm) shall be provided on stairways with solid risers where the tread dept is less than 11 inches (279 mm). Treads and risers of stairs shall be permitted to be so proportioned that the sum of two risers and a tread, exclusive of projection of nosing, is not less than 24 inches or



- more than 25 inches. Every tread less than 10 inches wide shall have a nosing, or effective projection, of approximately 1 inch over the level immediately below that tread.
6. Show the method of fireblocking for under the stairwell as required by sections R311.2.2 of the FRC 2004: Under stair protection: enclosed accessible space under stairs shall have walls, under stair surface and any soffits protected on the enclosed side with ½-inch (12.7 mm) gypsum board.
  7. Show compliance with sections R312.1 of the FRC 2004: Guards required on Porches, balconies or raised floor surfaces located more than 30 inches (762 mm) above the floor or grade below shall have guards not less than 36 inches (914 mm) in height. Open sides of stairs with a total rise of more than 30 inches (762 mm) above the floor or grade below shall have guards not less than 34 inches (864 mm) in height measured vertically from the nosing of the treads also show compliance with sections R301.5 of the FRC 2004 Live load. Guardrails and handrails minimum uniformly distributed live loads of 200(in pounds per square foot) and Guardrails in-fill components of minimum uniformly distributed live loads of 50 (in pounds per square foot).
  8. Show on the foundation plan the, supporting foundation design for interior load bearing wall which are created by the roof truss system.
  9. Show the foundation supporting and attachment method for the columns in the lanai and covered patio area include the attachment method of the columns to the supporting headers and beams.
  10. Provide the nailing pattern for the ½" structural sheathing on the typical exterior two story wall section.

more than 25 inches. Every tread less than 10 inches wide shall have a nosing or  
effective projection of approximately 1 inch over the foot line, extending below the tread

to show the method of fastening for treads. The stairwell as required by sections  
103.01.2.2 of the IRC 2004. Landing and platform surfaces shall be finished with a material  
which has a slip resistance that equals and any surface protection on the exterior side shall

be a minimum of 0.05 (5 mm) gypsum board

7. Show compliance with section 103.01.2.2 of the IRC 2004. Landings required on  
stairs shall be finished with a material which has a slip resistance that equals and any surface  
protection on the exterior side shall be a minimum of 0.05 (5 mm) gypsum board. Open  
sides of stairs with a total rise of more than 30 inches (762 mm) or with a total run  
of more than 48 inches (1219 mm) shall have a guard not less than 36 inches (914 mm) in height

and shall have a top rail. Guards shall have a top rail with a height of 36 inches (914 mm) in height  
measured vertically from the nosing of the treads. Also show compliance with section 103.01.2.2 of the  
IRC 2004. Live load (40 pounds per square foot) shall be uniformly distributed horizontally  
across the entire width of the floor. The floor shall be supported by a minimum

of 2000 pounds per square foot (95.7 kilopascals) in all components of the structure

8. Show on the foundation plan the supporting foundation design for the structure  
including all walls, columns and the roof mass system

9. Show the foundation supporting and attachment method for the columns, beams, girders  
and other parts and details. The attachment method of the columns to the supporting  
foundations shall be as follows:

10. Show the walling pattern for the 12" minimum exterior walls on the typical exterior wall  
section.

11. Please provide the pound per square foot loading for the 16" trimjoist on 24" centers.
12. On all gable end truss please provide a design method for the gable truss and wall hinge bracing.
13. Show the design method to provide for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) for the exterior wall section in which the 16" trimjoist are supported by the exterior wall section.
14. Please submit the manufacture specification with pound per square foot dead load weight along with wind speed rating of the barrel conc. roof tiles.
15. Please submit a supporting header callout were supporting headers will extend past the span table R502.5 of the FRC 2004.
16. Please show the method of complying with section R602.8 fireblocking: Fireblocking shall be provided to cut off all concealed draft openings (both vertical and horizontal) and to form an effective fire barrier between stories, and between a top story and the roof space. Fireblocking shall be provided in wood-frame construction in the following locations.
  1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs; as follows:
    - 1.1. Vertically at the ceiling and floor levels.
    - 1.2. Horizontally at intervals not exceeding 10 feet (3048 mm).
  2. At all interconnections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.
  3. In concealed spaces between stair stringers at the top and bottom of the run. Enclosed spaces under stairs shall comply with Section R311.2.2.
  4. At openings around vents, pipes, and ducts at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion.

1. The first part of the report is the general introduction for the project on the 1st floor.

2. The second part of the report is the detailed description of the building structure and its components.

3. The third part of the report is the detailed description of the building structure and its components.

4. The fourth part of the report is the detailed description of the building structure and its components.

5. The fifth part of the report is the detailed description of the building structure and its components.

6. The sixth part of the report is the detailed description of the building structure and its components.

7. The seventh part of the report is the detailed description of the building structure and its components.

8. The eighth part of the report is the detailed description of the building structure and its components.

9. The ninth part of the report is the detailed description of the building structure and its components.

10. The tenth part of the report is the detailed description of the building structure and its components.

11. The eleventh part of the report is the detailed description of the building structure and its components.

12. The twelfth part of the report is the detailed description of the building structure and its components.

13. The thirteenth part of the report is the detailed description of the building structure and its components.

14. The fourteenth part of the report is the detailed description of the building structure and its components.

15. The fifteenth part of the report is the detailed description of the building structure and its components.

16. The sixteenth part of the report is the detailed description of the building structure and its components.

17. The seventeenth part of the report is the detailed description of the building structure and its components.

18. The eighteenth part of the report is the detailed description of the building structure and its components.

19. The nineteenth part of the report is the detailed description of the building structure and its components.

20. The twentieth part of the report is the detailed description of the building structure and its components.

21. The twenty-first part of the report is the detailed description of the building structure and its components.

22. The twenty-second part of the report is the detailed description of the building structure and its components.

23. The twenty-third part of the report is the detailed description of the building structure and its components.

5. For the fireblocking of chimneys and fireplaces, see Section R1001.16.

17. Please submit a manual J and energy calculations for this dwelling.

18. Show compliance with section R309 of the FRC 2004: R309.1 Opening protection.

Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

R309.1.1 Duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

R309.2 Separation required.

The garage shall be separated from the residence and its attic area by not less than 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than 1/2-inch (12.7 mm) gypsum board or equivalent.

19. Show compliance with sections M1305 of the FMC 2004 as the code it relates to HVAC equipment for the dwelling.

20. Please submit a recorded notice of commencement with this department prior to requesting any inspections on this dwelling.



Thank you,

A handwritten signature in red ink, appearing to read "Joe Haltiwanger", with a stylized flourish at the end.

Joe Haltiwanger  
Plan Examiner  
Columbia County Building Department



**GENERAL CONTRACTORS**  
**OF FLORIDA**

**OCCUPANCY**

**COLUMBIA COUNTY, FLORIDA**

### Department of Building and Zoning Inspection

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 01-5S-16-03387-012

Building permit No. 000023867

Use Classification SFD/UTILITY

Fire: 44.64

Permit Holder ISAAC BRATKOVICH

Waste: 134.00

Owner of Building ABRAM & ANGELA HUBER

Total: 178.64

Location: 225 SW FINLEY LITTLE LANE, LAKE CITY, FL

Date: 02/23/2007

*Harry Dicks* by *td*

Building Inspector



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*(Business Places Only)*

