

SBA Communications Corporation
8051 Congress Avenue
Boca Raton, FL 33487-1307

T + 561 995 7670
F + 561 995 7626

sbasite.com



Structural Analysis Report

Client: AT&T

By AMeek at 11:19:46 AM, 8/31/2026

Client Site ID / Name: 10152022 / SW LAKE CITY
Application #: 320784, v1

SBA Site ID / Name: FL10961-A / Bass

250 ft Self Supporting Tower

4741 SW Birley Avenue
Lake City, Florida 32024
Lat: 30.1260, Long: -82.7329

Project number: FL10961-ATT-081026
SBA Contact: Samuel Apaez

Analysis Results

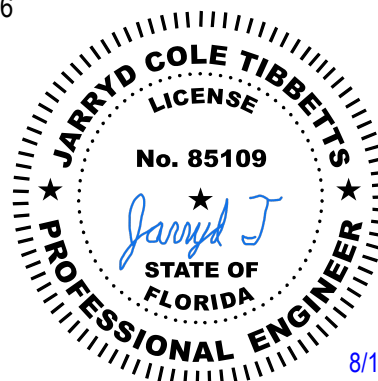
Tower	81.5%	Pass
Foundation	68.0%	Pass

Change in tower stress due to mount modification	0.0%
--	------

August 13, 2026

Jarryd
Tibbetts

Digitally signed
by Jarryd
Tibbetts
Date: 2026.08.13
13:53:26 -05'00'



8/13/2026

This item has been digitally signed and sealed by Jarryd Tibbetts on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

TOWER ENGINEERING
SOLUTIONS, LLC
1320 Greenway Drive Ste 600
Irving, TX 75038
CA#30690

Table of Contents

Introduction..... 3

Analysis Criteria 3

Appurtenance Loading 4

 Existing Loading: 4

 Proposed Loading: 5

Analysis Results 6

 Tower..... 6

 Foundation..... 6

Conclusions..... 7

Installation Requirements..... 7

Assumptions and Limitations 8

 Assumptions 8

 Limitations..... 8

Appendix 9

 Tower Geometry.....

 Coax Layout.....

 TESTower Report.....

 Foundation Analysis Report.....



Introduction

The purpose of this report is to summarize the analysis results on the 250 ft Self Supporting Tower to support the proposed antennas and transmissions lines in addition to those currently installed.

Table 1 List of Documents Used

Item	Document
Tower design/drawings	CellXion, Dwg # TNXPO2122, dated 4/27/2005
Foundation drawings	CellXion, Dwg # TNXPO2122, dated 4/27/2005
Geotechnical report	Terracon, Project # 49048313G, dated 3/18/2005
Modification drawings	N/A
Latest SA	TES, Project # 153858, dated 11/21/2024

Analysis Criteria

Table 2 Code Related Data

Jurisdiction (State/County/City)	Florida/Columbia/Lake City
Governing Codes	ANSI/TIA 222-H, 2021 IBC with ASCE 7-22, Florida Building Code 8th Edition (2023)
Ultimate Wind Speed (3-Sec gust)	119.0 mph
Wind Speed with Ice (3-Sec gust)	N/A
Service Wind Speed (3-Sec gust)	60 mph
Ice Thickness	No Ice
Risk Category	II
Exposure Category	C
Topographic Category	1
Crest Height	0 ft
Ground Elevation	104.59 ft.
Seismic Parameter S_s	0.12
Seismic Parameter S_1	0.058

This structural analysis is based upon the tower being classified as a risk category II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run.

Appurtenance Loading

Existing Loading:

Table 3 Existing Appurtenances

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
1	244	3	Ericsson 840590966 - Panel	(3) Sector Frames	(1) 0.275" DC (1) 0.315" DC Power (2) 1.996" Hybrid (2) 55 mm Fiber	T-Mobile
2		3	Ericsson AIR 6419 B41 - Panel			
3		3	Ericsson 4460 B25 + B66 - RRU			
4		3	Ericsson 4480 B71 + B85 - RRU			
5		1	Commscope (Andrew) VHLP3-11W/A - Dish			
6		2	Ceragon IP20D-HP - ODU			
-	228	3	Andrew DBXNH-8585B-R2M - Panel	(3) Sector Frames [Sabre C10857007C] @228'	(18) 1 5/8" (3) 3/8" Fiber (3) 3/8" RET (6) 7/8" DC	AT&T
-		9	Andrew SBNHH-1D85B - Panel			
-		9	Andrew E15Z01P13 - TMA			
-		6	Ericsson RRUS 11			
-		3	Ericsson RRUS 32			
-		3	Ericsson RRUS 4478 B5			
-		2	Ericsson RRUS 4478 B14			
-		3	Ericsson RRUS 4426 B66			
-		3	Ericsson RRUS A2			
-		2	Raycap DC6-48-60-18-8F - OVP			
-		4	Raycap DC2-48-60-0-9E - OVP			
-		1	Raycap DC6-48-60-18-8C - OVP			
16	200	10	Ericsson AIR6419 - Panel	(3) Sector Frames	(12) 1 5/8" Hybrid	Verizon
17		10	JMA Wireless MX06FHG865-HG - Panel			
18		12	Commscope CBC426T-DS-43 -Diplexer			
19		8	Ericsson 4490 - RRU			
20		8	Ericsson 4890 - RRU			
21		6	Raycap RCMDC-6627-PF-48 - OVP			

Note: AT&T loading includes FirstNET equipment

Proposed Loading:

Information pertaining to proposed antennas and transmission lines were based upon the Application #: 320784, v1 from AT&T and is listed in Table 4.

Table 4 Proposed Appurtenances

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
7	227	3	Ericsson AIR 6472 B77G (3450) + B77M (3840-3980) - Panel	(3) Sector Frames [Sabre C10857007C] + Site Pro Crossover Plate Kit #SCX2-K + Mount Pipes @228' As built mount model C10857001C total weight with mods 2,446.5 lbs.	(12) 1 5/8" (3) 3/8" Fiber (3) 3/8" RET (3) 3/4" DC Power Rosenberger WR-VG86ST-BRD (3) 0.957" DC Power Rosenberger WR-VG86ST-BRD	AT&T
8		6	Andrew / Commscope NNH4-85B-R6 - Panel			
9		3	Ericsson RRU 4490 B5 + B12A			
10		3	Ericsson RRUS 32 B30			
11		3	Ericsson RRU 4426 B66			
12		2	Ericsson RRU 4478 B14 w/ fan			
13		3	Ericsson RRU 4890 B25 + B66			
14		1	Ericsson RRU 4494 B14 + B29			
15		3	Raycap DC6-48-60-18-8F - OVP			

Note: AT&T loading includes FirstNET equipment

Analysis Results

Tower

The results of the structural analysis are shown below in table 5. Additional information for the tower analysis is provided within the Appendix.

Table 5 Tower Analysis Summary

Tower Component	Legs	Diagonals	Horizontals	Anchor Rods
Max. Usage:	68.4%	81.5%	7.9%	68.2%
Pass/Fail	Pass	Pass	Pass	Pass

Foundation

The results of the foundation analysis are shown below in table 6. Additional information for the foundation analysis is provided within the Appendix.

Table 6 Foundation Analysis Summary

Structural Component	Max Usage (%)	Analysis Result
Foundation	68.0%	Pass

Conclusions

Based on the analysis results, the existing tower and foundation were found to be **sufficient** to safely support the equipment listed in this analysis. No modification to the tower and foundation is needed at this time.

Installation Requirements

This analysis was performed under the assumption that the carrier will place the proposed equipment and feed lines at the installation height listed in Table 4 and in accordance with the coax layout shown. TMAs and RRUs are to be installed on existing mounts behind tenant's antennas unless otherwise noted. No equipment is to be installed directly in the climbing path. All equipment is to be installed per mount manufacturer specifications. In case site conditions do not allow for the required installation parameters to be met the carrier must notify SBA Communications Corporation engineers for approval of an alternative placement.

Assumptions and Limitations

Assumptions

This analysis was completed based on the following assumptions:

- Tower and foundation were built in accordance to manufacturer specifications.
- Tower and foundation has been properly maintained in accordance with the manufacturer's specifications
- All existing structural members were assumed to be in good condition with no physical damage or deterioration associated with corrosion
- Welds and bolts are assumed able to carry their intended original design loads.
- The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Table 3 and 4.
- This analysis may be affected if any assumptions are not valid or have been made in error. SBA should be notified to determine the effect on the structural integrity of the tower.

Limitations

The computer generated analysis performed by the tower software is limited to theoretical capacities of the towers structural members and does not account for any missing or damaged members or connections. The tower and foundation are assumed to have been properly designed, fabricated, installed and maintained, barring any conflicting findings from the most recent inspection.

SBA Communications Corporation has used its due diligence to verify the information provided to perform this analysis. It is unreasonable to perform a more detailed inspection of a tower and its components. This report is not a condition assessment of the tower or foundation.

Appendix

Structure: FL10961-A

Site Name: Bass **Code:** TIA-222-H 8/13/2026
Type: Self Support **Base Shape:** Triangle **Basic WS:** 119.00
Height: 250.00 (ft) **Base Width:** 24.30 **Basic Ice WS:** 0.00
Base Elev: 0.00 (ft) **Top Width:** 4.50 **Operational WS:** 60.00
Ice Thickness: 0.00 Page: 1



Section Properties

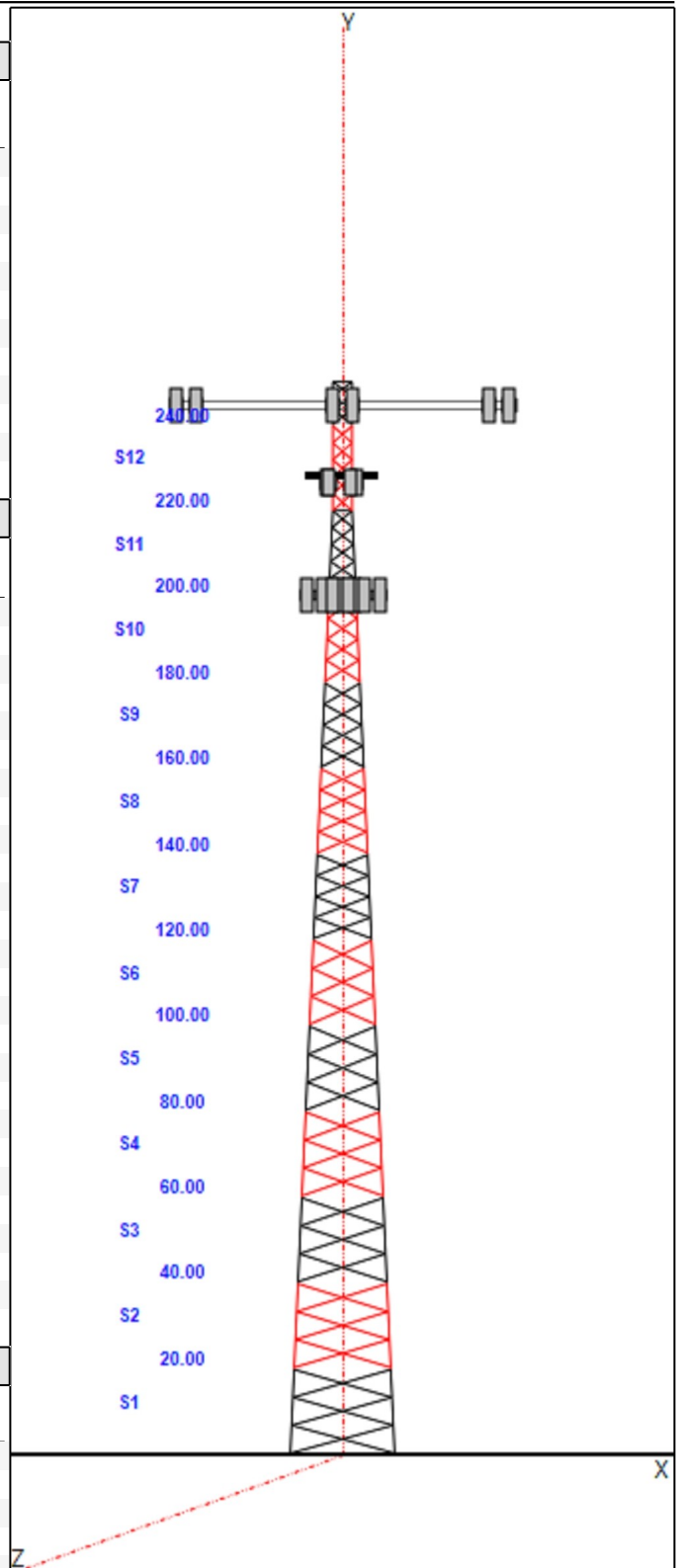
Sect	Leg Members	Diagonal Members	Horizontal Members
1	SOL 5 1/2" SOLID	SAE 4X4X0.375	
2-3	SOL 5 1/4" SOLID	SAE 4X4X0.25	
4	SOL 5" SOLID	SAE 3.5X3.5X0.25	
5	SOL 4 3/4" SOLID	SAE 3.5X3.5X0.25	
6	SOL 4 1/2" SOLID	SAE 3X3X0.25	
7	SOL 4 1/4" SOLID	SAE 3X3X0.1875	
8	SOL 4" SOLID	SAE 2.5X2.5X0.25	
9	SOL 3 3/4" SOLID	SAE 2.5X2.5X0.1875	
10	SOL 3 1/4" SOLID	SAE 2.5X2.5X0.1875	
11	SOL 2 3/4" SOLID	SAE 2.5X2.5X0.1875	SAE 2.5X2.5X0.1875
12	SOL 2 1/4" SOLID	SAE 2.5X2.5X0.1875	
13	SOL 1 3/4" SOLID	SAE 2X2X0.1875	SAE 2X2X0.1875

Discrete Appurtenances

Attach Elev (ft)	Force Elev (ft)	Qty	Description
250.00	250.00	1	Beacon
244.00	244.00	3	Sector Frame Site Pro1 VFA12HD
244.00	244.00	3	Ericsson 840590966
244.00	244.00	3	Ericsson AIR 6419 B41
244.00	244.00	3	Ericsson 4460 B25 + B66
244.00	244.00	3	Ericsson 4480 B71 + B85
244.00	244.00	1	Commscope (Andrew) VHLP3-11W/A
244.00	244.00	2	Ceragon IP20D-HP
228.00	228.00	12	Mount Pipes
228.00	228.00	1	(3) Sabre C10857007C
227.00	226.00	3	Ericsson AIR 6472 B77G
227.00	226.00	3	Ericsson RRUS 32 B30
227.00	226.00	6	Andrew NNH4-85B-R6
227.00	226.00	3	Ericsson RRU 4490 B5 + B12A
227.00	226.00	3	Ericsson RRU 4426 B66
227.00	226.00	2	Ericsson RRU 4478 B14
227.00	226.00	3	Ericsson RRU 4890 B25
227.00	226.00	1	Ericsson RRU 4494 B14 + B29
227.00	226.00	3	Raycap DC6-48-60-18-8F
200.00	200.00	10	Ericsson AIR6419
200.00	200.00	10	JMA Wireless MX06FHG865-HG
200.00	200.00	1	(3) SitePro1 VFA10-HD-S w/pipes
200.00	200.00	12	Commscope CBC426T-DS-43
200.00	200.00	8	Ericsson 4490
200.00	200.00	8	Ericsson 4890
200.00	200.00	6	RCMDC-6627-PF-48

Linear Appurtenances

Elev From (ft)	Elev To (ft)	Qty	Description
0.00	250.00	1	Climbing Ladder
0.00	250.00	1	Safety Cable
0.00	244.00	1	0.275" DC
0.00	244.00	1	0.315" DC Power
0.00	244.00	2	1.996" Hybrid
0.00	244.00	2	55 mm Fiber
0.00	244.00	1	W/G Ladder
0.00	228.00	3	0.96" DC



Structure: FL10961-A

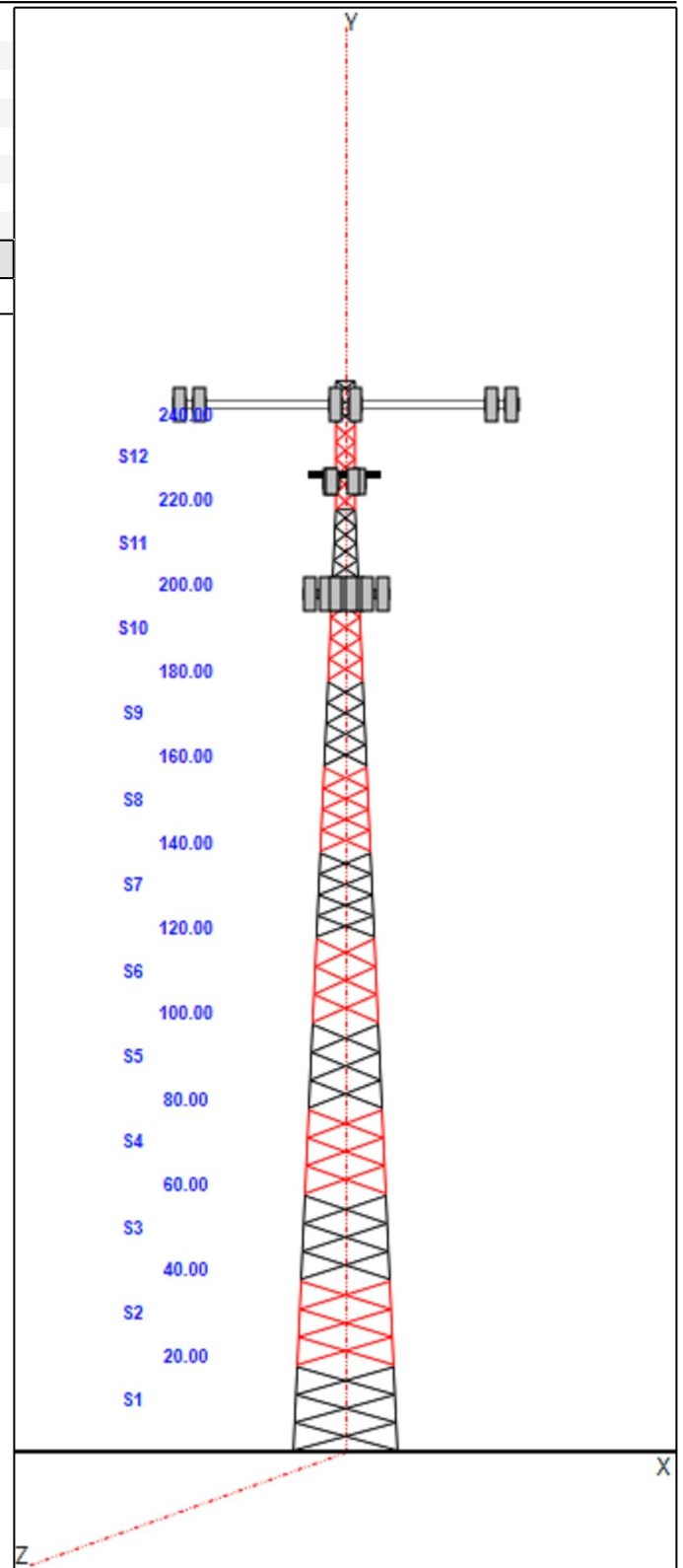
Site Name: Bass	Code: TIA-222-H	8/13/2026
Type: Self Support	Base Shape: Triangle	Basic WS: 119.00
Height: 250.00 (ft)	Base Width: 24.30	Basic Ice WS: 0.00
Base Elev: 0.00 (ft)	Top Width: 4.50	Operational WS: 60.00
	Ice Thickness: 0.00	Page: 2



0.00	228.00	12	1 5/8" Coax
0.00	228.00	3	3/4" DC
0.00	228.00	3	3/8" Fiber
0.00	228.00	3	3/8" RET
0.00	228.00	1	W/G Ladder
0.00	200.00	12	1 5/8" Hybrid
0.00	200.00	1	W/G Ladder

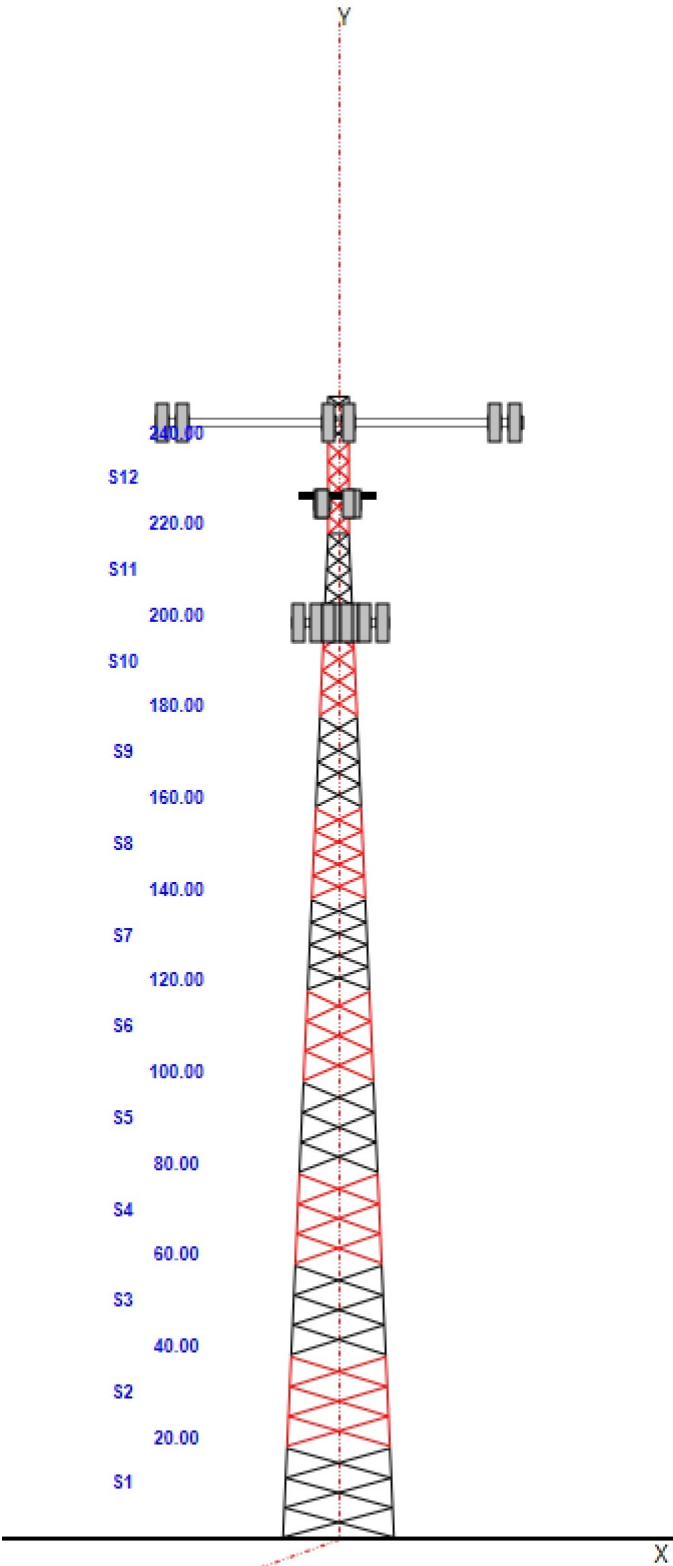
Base Reactions

Leg	Overturning
Max Uplift: -479.53 (kips)	Moment: 11200.69 (ft-kips)
Max Down: 564.20 (kips)	Total Down: 96.01 (kips)
Max Shear: 49.56 (kips)	Total Shear: 82.43 (kips)



Structure: FL10961-A

Site Name: Bass				Code: TIA-222-H		8/13/2026	
Type: Self Support		Base Shape: Triangle		Basic WS: 119.00			
Height: 250.00 (ft)		Base Width: 24.30		Basic Ice WS: 0.00			
Base Elev: 0.00 (ft)		Top Width: 4.50		Operational WS: 60.00		Page: 3	



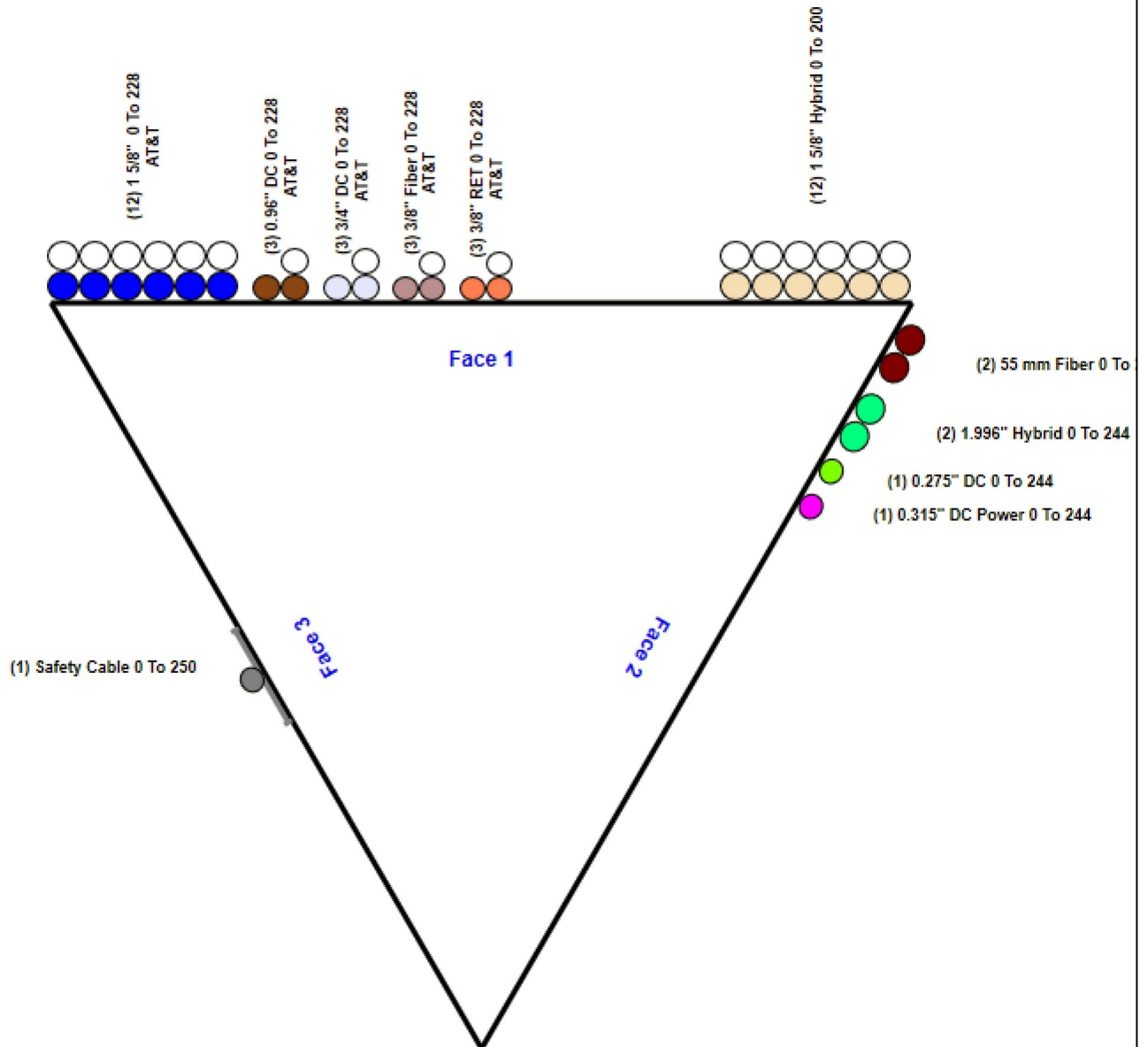
Structure: FL10961-A - Coax Line Placement

Type: Self Support
Site Name: Bass
Height: 250.00 (ft)

8/13/2026



Page: 4



Loading Summary

Structure: FL10961-A	Code: TIA-222-H	8/13/2026
Site Name: Bass	Exposure: C	
Height: 250.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II
		Page: 5



Discrete Appurtenances Properties

Attach Elev (ft)	Description	No Ice			Ice						Orientation Factor	Vert Ecc (ft)
		Qty	Weight (lb)	CaAa (sf)	Weight (lb)	CaAa (sf)	Len (in)	Width (in)	Depth (in)	Ka		
250.00	Beacon	1	36.00	2.720	75.00	2.720	75.000	17.500	17.500	1.00	1.00	0.000
244.00	Sector Frame Site Pro1 VFA12HD	3	630.79	18.900	900.00	18.900	300.000	0.000	0.000	0.75	0.75	0.000
244.00	Ericsson 840590966	3	68.50	19.880	68.50	19.880	95.900	23.500	7.100	0.80	0.70	0.000
244.00	Ericsson AIR 6419 B41	3	68.50	5.750	68.50	5.750	34.500	20.000	8.000	0.80	0.71	0.000
244.00	Ericsson 4460 B25 + B66	3	104.00	2.140	104.00	2.140	15.100	17.000	11.900	0.80	0.67	0.000
244.00	Ericsson 4480 B71 + B85	3	93.00	2.420	93.00	2.420	19.200	15.100	7.500	0.80	0.67	0.000
244.00	Commscope (Andrew)	1	110.00	8.920	110.00	8.920	36.000	36.000	0.000	1.00	1.00	0.000
244.00	Ceragon IP20D-HP	2	26.50	2.140	26.50	2.140	12.500	11.200	4.210	0.80	0.50	0.000
228.00	Mount Pipes	12	50.00	2.280	50.00	2.280	0.000	0.000	0.000	0.80	1.00	0.000
228.00	(3) Sabre C10857007C	1	1386.0	17.930	1386.00	17.930	0.000	0.000	0.000	1.00	1.00	0.000
227.00	Ericsson AIR 6472 B77G	3	77.17	4.790	77.17	4.790	36.300	15.830	9.290	0.80	0.81	-1.000
227.00	Ericsson RRUS 32 B30	3	53.00	2.740	53.00	2.740	27.200	12.100	7.000	0.80	0.80	-1.000
227.00	Andrew NNH4-85B-R6	6	82.23	12.270	82.23	12.270	71.970	19.600	7.760	0.80	0.73	-1.000
227.00	Ericsson RRU 4490 B5 + B12A	3	65.00	2.680	65.00	2.680	20.600	15.600	7.000	0.80	0.73	-1.000
227.00	Ericsson RRU 4426 B66	3	48.40	1.860	48.40	1.860	16.540	13.500	6.450	0.80	0.74	-1.000
227.00	Ericsson RRU 4478 B14	2	59.40	2.030	59.40	2.030	18.110	13.460	8.270	0.80	0.90	-1.000
227.00	Ericsson RRU 4890 B25	3	69.50	2.690	69.50	2.690	20.590	15.670	7.000	0.80	0.73	-1.000
227.00	Ericsson RRU 4494 B14 + B29	1	57.30	2.680	57.30	2.680	20.550	15.630	5.870	0.80	1.00	-1.000
227.00	Raycap DC6-48-60-18-8F	3	14.88	2.690	14.88	2.690	25.970	12.430	9.660	0.80	0.89	-1.000
200.00	Ericsson AIR6419	10	66.10	3.800	66.10	3.800	28.300	16.100	7.900	0.80	0.76	0.000
200.00	JMA Wireless MX06FHG865-HG	10	50.00	11.610	50.00	11.610	95.900	12.200	10.700	0.80	0.95	0.000
200.00	(3) SitePro1 VFA10-HD-S w/pipes	1	2007.0	49.050	2007.00	49.050	0.000	0.000	0.000	0.75	1.00	0.000
200.00	Commscope CBC426T-DS-43	12	7.90	0.320	7.90	0.320	4.600	8.300	3.300	0.80	0.00	0.000
200.00	Ericsson 4490	8	55.00	2.940	55.00	2.940	17.800	17.000	7.200	0.80	0.67	0.000
200.00	Ericsson 4890	8	69.50	2.700	69.50	2.700	20.600	15.700	7.200	0.80	0.67	0.000
200.00	RCMDC-6627-PF-48	6	32.00	4.060	32.00	4.060	29.500	16.500	12.600	0.80	0.67	0.000
Totals:		114	11,183.50		12,030.13		Number of Appurtenances :					26

Loading Summary

Structure: FL10961-A	Code: TIA-222-H	8/13/2026
Site Name: Bass	Exposure: C	
Height: 250.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II
		Page: 6

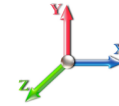


Linear Appurtenances Properties

Elev. From (ft)	Elev. To (ft)	Description	Qty	Width (in)	Weight (lb/ft)	Pct In Block	Spread On Faces	Bundling Arrangement	Cluster Dia (in)	Out of Zone	Spacing (in)	Orientation Factor	Ka Override
0.00	250.00	Climbing Ladder	1	3.00	6.90	100.00	3	Individual NR		N	1.00	1.00	
0.00	250.00	Safety Cable	1	0.38	0.27	100.00	3	Individual NR		N	1.00	1.00	
0.00	244.00	0.275" DC	1	0.27	0.30	100.00	2	Individual NR		N	1.00	1.00	
0.00	244.00	0.315" DC Power	1	0.32	0.03	100.00	2	Individual NR		N	1.00	1.00	
0.00	244.00	1.996" Hybrid	2	1.99	1.99	100.00	2	Individual NR		N	1.00	1.00	
0.00	244.00	55 mm Fiber	2	2.16	0.50	100.00	2	Individual NR		N	1.00	1.00	
0.00	244.00	W/G Ladder	1	3.00	6.00	100.00	2	Individual NR		N	1.00	1.00	
0.00	228.00	0.96" DC	3	0.96	0.40	66.60	1	Block		N	1.00	1.00	
0.00	228.00	1 5/8" Coax	12	1.98	1.04	50.00	1	Block		N	0.50	1.00	
0.00	228.00	3/4" DC	3	0.75	0.40	66.60	1	Block		N	1.00	1.00	
0.00	228.00	3/8" Fiber	3	0.38	0.06	50.00	1	Block		N	1.00	1.00	
0.00	228.00	3/8" RET	3	0.44	0.08	50.00	1	Block		N	1.00	1.00	
0.00	228.00	W/G Ladder	1	2.00	6.00	100.00	1	Individual NR		N	1.00	1.00	
0.00	200.00	1 5/8" Hybrid	12	1.98	1.04	50.00	1	Block		N	0.50	1.00	
0.00	200.00	W/G Ladder	1	2.00	6.00	100.00	1	Individual NR		N	1.00	1.00	

Section Forces

Structure: FL10961-A	Code: EIA_H	8/13/2026
Site Name: Bass	Exposure: C	
Height: 250.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Topography: 1	Struct Class: II



Page: 7

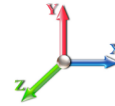
Wind Loads

Wind Load Factor: 1.00	Wind Speed: 119.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.20	Ice Thickness: 0.00	
Ice Dead Load Factor: 0.00		Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	26.09	47.659	18.36	0.00	0.14	2.81	1.00	1.00	0.00	57.25	92.68	0.00	12,322.	0.0	3574.13	1848.86	5,422.99
								0.85	1.00	0.00	50.10	92.68	0.00	12,322.	0.0	3127.82	1848.86	4,976.68
								0.80	1.00	0.00	47.72	92.68	0.00	12,322.	0.0	2979.05	1848.86	4,827.91
2	30.0	30.15	44.239	17.52	0.00	0.14	2.81	1.00	1.00	0.00	53.31	92.68	0.00	9,879.2	0.0	3836.81	2136.53	5,973.34
								0.85	1.00	0.00	46.67	92.68	0.00	9,879.2	0.0	3359.21	2136.53	5,495.74
								0.80	1.00	0.00	44.46	92.68	0.00	9,879.2	0.0	3200.02	2136.53	5,336.54
3	50.0	33.58	40.806	17.52	0.00	0.14	2.79	1.00	1.00	0.00	49.70	92.68	0.00	9,634.4	0.0	3961.96	2379.10	6,341.06
								0.85	1.00	0.00	43.58	92.68	0.00	9,634.4	0.0	3474.00	2379.10	5,853.10
								0.80	1.00	0.00	41.54	92.68	0.00	9,634.4	0.0	3311.35	2379.10	5,690.45
4	70.0	36.04	32.764	16.69	0.00	0.13	2.83	1.00	1.00	0.00	41.24	92.68	0.00	8,567.3	0.0	3575.79	2553.74	6,129.54
								0.85	1.00	0.00	36.33	92.68	0.00	8,567.3	0.0	3149.67	2553.74	5,703.41
								0.80	1.00	0.00	34.69	92.68	0.00	8,567.3	0.0	3007.63	2553.74	5,561.37
5	90.0	38.00	29.858	15.85	0.00	0.14	2.82	1.00	1.00	0.00	38.01	92.68	0.00	7,887.3	0.0	3458.32	2692.50	6,150.82
								0.85	1.00	0.00	33.53	92.68	0.00	7,887.3	0.0	3050.79	2692.50	5,743.28
								0.80	1.00	0.00	32.03	92.68	0.00	7,887.3	0.0	2914.94	2692.50	5,607.44
6	110.0	39.64	23.141	15.02	0.00	0.13	2.85	1.00	1.00	0.00	30.93	92.68	0.00	6,927.2	0.0	2970.29	2808.68	5,778.97
								0.85	1.00	0.00	27.46	92.68	0.00	6,927.2	0.0	2637.00	2808.68	5,445.68
								0.80	1.00	0.00	26.31	92.68	0.00	6,927.2	0.0	2525.90	2808.68	5,334.58
7	130.0	41.06	26.314	14.19	0.00	0.16	2.75	1.00	1.00	0.00	33.86	92.68	0.00	6,276.5	0.0	3247.79	2909.22	6,157.01
								0.85	1.00	0.00	29.91	92.68	0.00	6,276.5	0.0	2869.20	2909.22	5,778.42
								0.80	1.00	0.00	28.60	92.68	0.00	6,276.5	0.0	2743.01	2909.22	5,652.23
8	150.0	42.31	19.196	13.35	0.00	0.15	2.79	1.00	1.00	0.00	26.38	92.68	0.00	5,835.0	0.0	2643.36	2998.20	5,641.55
								0.85	1.00	0.00	23.50	92.68	0.00	5,835.0	0.0	2354.87	2998.20	5,353.06
								0.80	1.00	0.00	22.54	92.68	0.00	5,835.0	0.0	2258.70	2998.20	5,256.90
9	170.0	43.44	16.544	12.52	0.00	0.16	2.75	1.00	1.00	0.00	23.41	92.68	0.00	4,967.5	0.0	2377.16	3078.25	5,455.41
								0.85	1.00	0.00	20.93	92.68	0.00	4,967.5	0.0	2125.21	3078.25	5,203.46
								0.80	1.00	0.00	20.10	92.68	0.00	4,967.5	0.0	2041.22	3078.25	5,119.47
10	190.0	44.47	14.050	10.85	0.00	0.17	2.71	1.00	1.00	0.00	20.23	92.68	0.00	4,157.3	0.0	2073.12	3151.18	5,224.30
								0.85	1.00	0.00	18.12	92.68	0.00	4,157.3	0.0	1857.12	3151.18	5,008.30
								0.80	1.00	0.00	17.42	92.68	0.00	4,157.3	0.0	1785.12	3151.18	4,936.30
11	210.0	45.42	14.349	9.18	0.00	0.21	2.57	1.00	1.00	0.00	19.64	65.38	0.00	3,155.5	0.0	1945.66	2231.10	4,176.76
								0.85	1.00	0.00	17.48	65.38	0.00	3,155.5	0.0	1732.41	2231.10	3,963.51
								0.80	1.00	0.00	16.77	65.38	0.00	3,155.5	0.0	1661.32	2231.10	3,892.42
12	230.0	46.30	12.075	7.50	0.00	0.21	2.57	1.00	1.00	0.00	16.40	41.41	0.00	2,238.4	0.0	1656.21	1455.42	3,111.63
								0.85	1.00	0.00	14.58	41.41	0.00	2,238.4	0.0	1473.25	1455.42	2,928.67
								0.80	1.00	0.00	13.98	41.41	0.00	2,238.4	0.0	1412.26	1455.42	2,867.68
13	245.0	46.92	6.131	2.92	0.00	0.19	2.61	1.00	1.00	0.00	7.80	6.77	0.00	744.3	0.0	813.41	261.51	1,074.92
								0.85	1.00	0.00	6.88	6.77	0.00	744.3	0.0	717.55	261.51	979.07
								0.80	1.00	0.00	6.58	6.77	0.00	744.3	0.0	685.60	261.51	947.11

Section Forces

Structure: FL10961-A	Code: EIA_H	8/13/2026	
Site Name: Bass	Exposure: C		
Height: 250.00 (ft)	Crest Height: 0.00		
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil		
Gh: 0.85	Topography: 1	Struct Class: II	Page: 8



Service Loads

Wind Load Factor: 1.00	Wind Speed: 60.00	Wind Importance Factor: 1.00
Dead Load Factor: 1.00	Ice Thickness: 0.00	
Ice Dead Load Factor: 0.00		Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	6.63	47.659	18.36	0.00	0.14	2.81	1.00	1.00	0.00	58.06	92.68	0.00	10,268.	0.0	921.45	470.02	1,391.47
								0.85	1.00	0.00	50.91	92.68	0.00	10,268.	0.0	807.99	470.02	1,278.01
								0.80	1.00	0.00	48.53	92.68	0.00	10,268.	0.0	770.17	470.02	1,240.19
2	30.0	7.67	44.239	17.52	0.00	0.14	2.81	1.00	1.00	0.00	54.17	92.68	0.00	8,232.6	0.0	991.10	543.15	1,534.25
								0.85	1.00	0.00	47.53	92.68	0.00	8,232.6	0.0	869.69	543.15	1,412.83
								0.80	1.00	0.00	45.32	92.68	0.00	8,232.6	0.0	829.22	543.15	1,372.36
3	50.0	8.54	40.806	17.52	0.00	0.14	2.79	1.00	1.00	0.00	50.74	92.68	0.00	8,028.7	0.0	1028.34	604.81	1,633.15
								0.85	1.00	0.00	44.62	92.68	0.00	8,028.7	0.0	904.29	604.81	1,509.11
								0.80	1.00	0.00	42.58	92.68	0.00	8,028.7	0.0	862.94	604.81	1,467.76
4	70.0	9.16	32.764	16.69	0.00	0.13	2.83	1.00	1.00	0.00	42.21	92.68	0.00	7,139.4	0.0	930.45	649.21	1,579.66
								0.85	1.00	0.00	37.30	92.68	0.00	7,139.4	0.0	822.12	649.21	1,471.33
								0.80	1.00	0.00	35.66	92.68	0.00	7,139.4	0.0	786.01	649.21	1,435.22
5	90.0	9.66	29.858	15.85	0.00	0.14	2.82	1.00	1.00	0.00	38.84	92.68	0.00	6,572.7	0.0	898.43	684.48	1,582.91
								0.85	1.00	0.00	34.36	92.68	0.00	6,572.7	0.0	794.82	684.48	1,479.31
								0.80	1.00	0.00	32.87	92.68	0.00	6,572.7	0.0	760.29	684.48	1,444.77
6	110.0	10.08	23.141	15.02	0.00	0.13	2.85	1.00	1.00	0.00	31.64	92.68	0.00	5,772.6	0.0	772.28	714.02	1,486.31
								0.85	1.00	0.00	28.17	92.68	0.00	5,772.6	0.0	687.56	714.02	1,401.58
								0.80	1.00	0.00	27.01	92.68	0.00	5,772.6	0.0	659.31	714.02	1,373.33
7	130.0	10.44	26.314	14.19	0.00	0.16	2.75	1.00	1.00	0.00	34.37	92.68	0.00	5,230.4	0.0	838.17	739.58	1,577.75
								0.85	1.00	0.00	30.43	92.68	0.00	5,230.4	0.0	741.93	739.58	1,481.51
								0.80	1.00	0.00	29.11	92.68	0.00	5,230.4	0.0	709.84	739.58	1,449.42
8	150.0	10.76	19.196	13.35	0.00	0.15	2.79	1.00	1.00	0.00	26.77	92.68	0.00	4,862.5	0.0	681.81	762.20	1,444.00
								0.85	1.00	0.00	23.89	92.68	0.00	4,862.5	0.0	608.46	762.20	1,370.66
								0.80	1.00	0.00	22.93	92.68	0.00	4,862.5	0.0	584.02	762.20	1,346.22
9	170.0	11.04	16.544	12.52	0.00	0.16	2.75	1.00	1.00	0.00	23.66	92.68	0.00	4,139.6	0.0	610.57	782.55	1,393.12
								0.85	1.00	0.00	21.17	92.68	0.00	4,139.6	0.0	546.52	782.55	1,329.07
								0.80	1.00	0.00	20.35	92.68	0.00	4,139.6	0.0	525.17	782.55	1,307.72
10	190.0	11.31	14.050	10.85	0.00	0.17	2.71	1.00	1.00	0.00	20.23	92.68	0.00	3,464.5	0.0	527.03	801.09	1,328.12
								0.85	1.00	0.00	18.12	92.68	0.00	3,464.5	0.0	472.11	801.09	1,273.21
								0.80	1.00	0.00	17.42	92.68	0.00	3,464.5	0.0	453.81	801.09	1,254.90
11	210.0	11.55	14.349	9.18	0.00	0.21	2.57	1.00	1.00	0.00	19.64	65.38	0.00	2,629.6	0.0	494.63	567.19	1,061.81
								0.85	1.00	0.00	17.48	65.38	0.00	2,629.6	0.0	440.41	567.19	1,007.60
								0.80	1.00	0.00	16.77	65.38	0.00	2,629.6	0.0	422.34	567.19	989.53
12	230.0	11.77	12.075	7.50	0.00	0.21	2.57	1.00	1.00	0.00	16.40	41.41	0.00	1,865.4	0.0	421.04	370.00	791.04
								0.85	1.00	0.00	14.58	41.41	0.00	1,865.4	0.0	374.53	370.00	744.52
								0.80	1.00	0.00	13.98	41.41	0.00	1,865.4	0.0	359.02	370.00	729.02
13	245.0	11.93	6.131	2.92	0.00	0.19	2.61	1.00	1.00	0.00	7.80	6.77	0.00	620.3	0.0	206.78	66.48	273.27
								0.85	1.00	0.00	6.88	6.77	0.00	620.3	0.0	182.42	66.48	248.90
								0.80	1.00	0.00	6.58	6.77	0.00	620.3	0.0	174.29	66.48	240.77

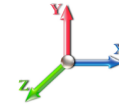
Force/Stress Compression Summary

Structure: FL10961-A
Site Name: Bass
Height: 250.00 (ft)
Base Elev: 0.000 (ft)
Gh: 0.85

Topography: 1

Code: EIA/TIA-222-H
Exposure: C
Crest Height: 0.00
Site Class: D - Stiff Soil
Struct Class: II

8/13/2026



Page: 9

LEG MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X Y Z	KL/R	Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls
1	20	SOL - 5 1/2" SOLID	-557.33	1.2D + 1.0W 240° Wind	6.51	100 100 100	56.80	50.00	844.42	66.0	Member X
2	40	SOL - 5 1/4" SOLID	-514.69	1.2D + 1.0W 240° Wind	6.51	100 100 100	59.51	50.00	751.93	68.4	Member X
3	60	SOL - 5 1/4" SOLID	-470.80	1.2D + 1.0W 240° Wind	6.51	100 100 100	59.51	50.00	751.93	62.6	Member X
4	80	SOL - 5" SOLID	-426.66	1.2D + 1.0W 240° Wind	6.51	100 100 100	62.48	50.00	664.15	64.2	Member X
5	100	SOL - 4 3/4" SOLID	-382.72	1.2D + 1.0W 240° Wind	6.51	100 100 100	65.77	50.00	581.21	65.8	Member X
6	120	SOL - 4 1/2" SOLID	-338.45	1.2D + 1.0W 240° Wind	6.51	100 100 100	69.43	50.00	503.11	67.3	Member X
7	140	SOL - 4 1/4" SOLID	-295.62	1.2D + 1.0W 240° Wind	4.88	100 100 100	55.13	50.00	511.15	57.8	Member X
8	160	SOL - 4" SOLID	-249.96	1.2D + 1.0W 240° Wind	4.88	100 100 100	58.58	50.00	439.99	56.8	Member X
9	180	SOL - 3 3/4" SOLID	-202.22	1.2D + 1.0W 240° Wind	4.88	100 100 100	62.48	50.00	373.59	54.1	Member X
10	200	SOL - 3 1/4" SOLID	-150.26	1.2D + 1.0W 240° Wind	4.88	100 100 100	72.10	50.00	255.28	58.9	Member X
11	220	SOL - 2 3/4" SOLID	-91.41	1.2D + 1.0W 240° Wind	3.96	100 100 100	69.04	50.00	188.63	48.5	Member X
12	240	SOL - 2 1/4" SOLID	-47.29	1.2D + 1.0W 240° Wind	3.95	100 100 100	84.27	50.00	106.46	44.4	Member X
13	250	SOL - 1 3/4" SOLID	-4.43	1.2D + 1.0W 240° Wind	3.25	100 100 100	89.14	50.00	60.54	7.3	Member X

HORIZONTAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X Y Z	KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	Use %	Controls
1	20								0.00	0	0				
2	40								0.00	0	0				
3	60								0.00	0	0				
4	80								0.00	0	0				
5	100								0.00	0	0				
6	120								0.00	0	0				
7	140								0.00	0	0				
8	160								0.00	0	0				
9	180								0.00	0	0				
10	200								0.00	0	0				
11	220	SAE - 2.5X2.5X0.1875	-0.37	1.2D + 1.0W 300° Wind	4.50	100 100 100	109.09	36.00	20.32	1	1	27.05	18.27	2.0	Bolt Bear
12	240								0.00	0	0				
13	250	SAE - 2X2X0.1875	-0.67	1.2D + 1.0W 300° Wind	4.50	100 100 100	137.06	36.00	10.82	1	1	13.81	13.05	6.2	Member Z

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X Y Z	KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	Use %	Controls
1	20	SAE - 4X4X0.375	-13.9	1.2D + 1.0W 30° Wind	23.72	48 48 48	173.41	36.00	27.22	1	1	27.05	36.5	51.4	Bolt Shear
2	40	SAE - 4X4X0.25	-13.0	0.9D + 1.0W 210° Wind	23.12	48 48 48	167.50	36.00	19.79	1	1	27.05	24.3	65.8	Member Z
3	60	SAE - 4X4X0.25	-12.3	1.2D + 1.0W 210° Wind	21.40	48 48 48	155.02	36.00	23.11	1	1	27.05	24.3	53.4	Member Z
4	80	SAE - 3.5X3.5X0.25	-11.3	1.2D + 1.0W 30° Wind	19.69	49 49 49	166.83	36.00	17.38	1	1	27.05	24.3	65.2	Member Z
5	100	SAE - 3.5X3.5X0.25	-10.5	1.2D + 1.0W 30° Wind	18.00	49 49 49	152.51	36.00	20.80	1	1	27.05	24.3	50.6	Member Z
6	120	SAE - 3X3X0.25	-9.68	1.2D + 1.0W 210° Wind	16.34	49 49 49	162.26	36.00	15.65	1	1	27.05	24.3	61.8	Member Z
7	140	SAE - 3X3X0.1875	-8.71	1.2D + 1.0W 210° Wind	14.13	49 49 49	139.37	36.00	16.06	1	1	27.05	18.2	54.2	Member Z
8	160	SAE - 2.5X2.5X0.25	-7.99	1.2D + 1.0W 210° Wind	12.45	49 49 49	149.11	36.00	15.32	1	1	27.05	24.3	52.1	Member Z
9	180	SAE - 2.5X2.5X0.1875	-7.43	1.2D + 1.0W 30° Wind	10.82	49 49 49	128.53	36.00	15.63	1	1	27.05	18.2	47.6	Member Z
10	200	SAE - 2.5X2.5X0.1875	-8.03	1.2D + 1.0W 30° Wind	8.16	49 49 49	96.94	36.00	22.88	1	1	27.05	18.2	43.9	Bolt Bear
11	220	SAE - 2.5X2.5X0.1875	-4.20	1.2D + 1.0W 210° Wind	7.27	49 49 49	86.35	36.00	24.86	1	1	27.05	18.2	23.0	Bolt Bear
12	240	SAE - 2.5X2.5X0.1875	-5.84	1.2D + 1.0W 30° Wind	5.99	49 49 49	71.13	36.00	27.31	1	1	27.05	18.2	32.0	Bolt Bear
13	250	SAE - 2X2X0.1875	-2.14	1.2D + 1.0W 30° Wind	5.55	49 49 49	82.84	36.00	20.04	1	1	13.81	13.0	16.4	Bolt Bear

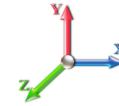
Force/Stress Tension Summary

Structure: FL10961-A
Site Name: Bass
Height: 250.00 (ft)
Base Elev: 0.000 (ft)
Gh: 0.85

Topography: 1

Code: EIA/TIA-222-H
Exposure: C
Crest Height: 0.00
Site Class: D - Stiff Soil
Struct Class: II

8/13/2026



Page: 10

LEG MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls
1	20	SOL - 5 1/2" SOLID	481.68	0.9D + 1.0W 300° Wind	50	1069.1	45.1	Member
2	40	SOL - 5 1/4" SOLID	448.78	0.9D + 1.0W 300° Wind	50	974.16	46.1	Member
3	60	SOL - 5 1/4" SOLID	413.94	0.9D + 1.0W 300° Wind	50	974.16	42.5	Member
4	80	SOL - 5" SOLID	378.28	0.9D + 1.0W 300° Wind	50	883.58	42.8	Member
5	100	SOL - 4 3/4" SOLID	342.15	0.9D + 1.0W 300° Wind	50	797.45	42.9	Member
6	120	SOL - 4 1/2" SOLID	305.10	0.9D + 1.0W 300° Wind	50	715.68	42.6	Member
7	140	SOL - 4 1/4" SOLID	266.96	0.9D + 1.0W 300° Wind	50	638.37	41.8	Member
8	160	SOL - 4" SOLID	227.36	0.9D + 1.0W 300° Wind	50	565.47	40.2	Member
9	180	SOL - 3 3/4" SOLID	185.34	0.9D + 1.0W 300° Wind	50	497.03	37.3	Member
10	200	SOL - 3 1/4" SOLID	138.82	0.9D + 1.0W 300° Wind	50	373.32	37.2	Member
11	220	SOL - 2 3/4" SOLID	84.74	0.9D + 1.0W 300° Wind	50	267.28	31.7	Member
12	240	SOL - 2 1/4" SOLID	39.48	0.9D + 1.0W 300° Wind	50	178.92	22.1	Member
13	250	SOL - 1 3/4" SOLID	3.75	0.9D + 1.0W 180° Wind	50	108.24	3.5	Member

HORIZONTAL MEMBERS

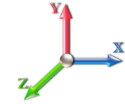
Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	-			36	0.00	0	0					
2	40	-			36	0.00	0	0					
3	60	-			36	0.00	0	0					
4	80	-			36	0.00	0	0					
5	100	-			36	0.00	0	0					
6	120	-			36	0.00	0	0					
7	140	-			36	0.00	0	0					
8	160	-			36	0.00	0	0					
9	180	-			36	0.00	0	0					
10	200	-			36	0.00	0	0					
11	220	SAE - 2.5X2.5X0.1875	0.25	0.9D + 1.0W 120° Wind	36	23.31	1	1	27.05	13.05	10.67	2.4	Blck Shear
12	240	-			36	0.00	0	0					
13	250	SAE - 2X2X0.1875	0.67	1.2D + 1.0W 120° Wind	36	18.58	1	1	13.81	9.79	8.51	7.9	Blck Shear

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	SAE - 4X4X0.375	13.55	0.9D + 1.0W 210° Wind	36	81.07	1	1	27.05	26.10	25.42	53.3	Blck Shear
2	40	SAE - 4X4X0.25	12.85	0.9D + 1.0W 30° Wind	36	55.14	1	1	27.05	17.40	16.95	75.8	Blck Shear
3	60	SAE - 4X4X0.25	12.25	0.9D + 1.0W 30° Wind	36	55.14	1	1	27.05	17.40	16.95	72.3	Blck Shear
4	80	SAE - 3.5X3.5X0.25	11.22	0.9D + 1.0W 210° Wind	36	46.98	1	1	27.05	17.40	16.95	66.2	Blck Shear
5	100	SAE - 3.5X3.5X0.25	10.43	0.9D + 1.0W 210° Wind	36	46.98	1	1	27.05	17.40	16.95	61.5	Blck Shear
6	120	SAE - 3X3X0.25	9.60	1.2D + 1.0W 210° Wind	36	38.82	1	1	27.05	17.40	14.23	67.5	Blck Shear
7	140	SAE - 3X3X0.1875	8.61	1.2D + 1.0W 30° Wind	36	29.44	1	1	27.05	13.05	10.67	80.7	Blck Shear
8	160	SAE - 2.5X2.5X0.25	8.01	1.2D + 1.0W 210° Wind	36	30.67	1	1	27.05	17.40	12.87	62.3	Blck Shear
9	180	SAE - 2.5X2.5X0.1875	7.77	1.2D + 1.0W 30° Wind	36	23.31	1	1	27.05	13.05	9.65	80.5	Blck Shear
10	200	SAE - 2.5X2.5X0.1875	7.87	1.2D + 1.0W 210° Wind	36	23.31	1	1	27.05	13.05	9.65	81.5	Blck Shear
11	220	SAE - 2.5X2.5X0.1875	4.17	1.2D + 1.0W 30° Wind	36	23.31	1	1	27.05	13.05	9.65	43.2	Blck Shear
12	240	SAE - 2.5X2.5X0.1875	5.74	1.2D + 1.0W 210° Wind	36	23.31	1	1	27.05	13.05	9.65	59.5	Blck Shear
13	250	SAE - 2X2X0.1875	2.15	1.2D + 1.0W 210° Wind	36	18.58	1	1	13.81	9.79	7.50	28.6	Blck Shear

Seismic Section Forces

Structure: FL10961-A	Code: TIA-222-H	8/13/2026
Site Name: Bass	Exposure: C	
Height: 250.00 (ft)	Crest Height: 0.00	
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil	
Gh: 0.85	Struct Class: II	Page: 11
Topography: 1		



Load Case: 1.2D + 1.0Ev + 1.0Eh

Dead Load Factor	1.20	Sds	0.128	Ss	0.1200	Fa	1.6000	Ke	1.2978	TL	8.0000
Seismic Load Factor	1.00	Sd1	0.092	S1	0.0580	Fv	2.4000	Kg	0.0000	Cs	0.0300
Seismic Importance Factor	1.00	W1	35.65	R	3.0000	Vs	2.4003	T	1.0955	f1	0.9128

Sect #	Elev (ft)	Wz (lb)	Lateral Fsz (lbs)	Vertical Ev (lbs)
1	10.00	10268.	15.09	263.01
2	30.00	8232.6	47.12	210.86
3	50.00	8028.6	88.51	205.64
4	70.00	7139.4	117.62	182.86
5	90.00	6572.7	146.39	168.35
6	110.00	5772.6	160.49	147.85
7	130.00	5230.4	175.39	133.97
8	150.00	4862.5	192.11	124.54
9	170.00	4139.5	183.39	106.03
10	190.00	7915.2	491.36	202.73
11	210.00	2629.5	133.88	67.35
12	230.00	5504.7	392.99	140.99
13	245.00	3713.6	255.95	95.12

Load Case: 0.9D + 1.0Ev + 1.0Eh

Dead Load Factor	0.90	Sds	0.128	Ss	0.1200	Fa	1.6000	Ke	1.2978	TL	8.0000
Seismic Load Factor	1.00	Sd1	0.092	S1	0.0580	Fv	2.4000	Kg	0.0000	Cs	0.0300
Seismic Importance Factor	1.00	W1	35.65	R	3.0000	Vs	2.4003	T	1.0955	f1	0.9128

Sect #	Elev (ft)	Wz (lb)	Lateral Fsz (lbs)	Vertical Ev (lbs)
1	10.00	10268.	15.09	263.01
2	30.00	8232.6	47.12	210.86
3	50.00	8028.6	88.51	205.64
4	70.00	7139.4	117.62	182.86
5	90.00	6572.7	146.39	168.35
6	110.00	5772.6	160.49	147.85
7	130.00	5230.4	175.39	133.97
8	150.00	4862.5	192.11	124.54
9	170.00	4139.5	183.39	106.03
10	190.00	7915.2	491.36	202.73
11	210.00	2629.5	133.88	67.35
12	230.00	5504.7	392.99	140.99
13	245.00	3713.6	255.95	95.12

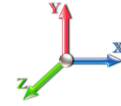
Support Forces Summary

Structure: FL10961-A
Site Name: Bass
Height: 250.00 (ft)
Base Elev: 0.000 (ft)
Gh: 0.85

Topography: 1

Code: TIA-222-H
Exposure: C
Crest Height: 0.00
Site Class: D - Stiff Soil
Struct Class: II

8/13/2026



Page: 12

Load Case	Node	FX (kips)	FY (kips)	FZ (kips)	(-) = Uplift (+) = Down
1.2D + 1.0W Normal Wind	1	-0.02	564.19	-49.56	
	1a	16.47	-234.10	-16.42	
	1b	-16.46	-234.08	-16.45	
1.2D + 1.0W 30° Wind	1	-3.05	474.75	-41.17	
	1a	-4.62	32.00	-4.62	
	1b	-31.43	-410.74	-21.96	
1.2D + 1.0W 60° Wind	1	-5.24	284.14	-24.14	
	1a	-23.52	284.13	7.54	
	1b	-37.77	-472.25	-21.81	
1.2D + 1.0W 90° Wind	1	-6.32	32.01	-1.69	
	1a	-37.18	474.74	17.95	
	1b	-34.73	-410.74	-16.26	
1.2D + 1.0W 120° Wind	1	-5.98	-234.09	22.48	
	1a	-42.93	564.18	24.77	
	1b	-22.48	-234.08	-6.04	
1.2D + 1.0W 150° Wind	1	-3.26	-410.74	38.20	
	1a	-34.14	474.74	23.20	
	1b	-1.71	32.01	6.34	
1.2D + 1.0W 180° Wind	1	0.02	-472.25	43.61	
	1a	-18.30	284.12	16.59	
	1b	18.28	284.14	16.62	
1.2D + 1.0W 210° Wind	1	3.29	-410.74	38.20	
	1a	1.70	32.00	6.31	
	1b	34.12	474.76	23.23	
1.2D + 1.0W 240° Wind	1	6.00	-234.09	22.48	
	1a	22.47	-234.10	-6.05	
	1b	42.92	564.20	24.79	
1.2D + 1.0W 270° Wind	1	6.32	32.01	-1.69	
	1a	34.73	-410.76	-16.26	
	1b	37.18	474.76	17.95	
1.2D + 1.0W 300° Wind	1	5.22	284.14	-24.14	
	1a	37.78	-472.27	-21.80	
	1b	23.53	284.14	7.53	
1.2D + 1.0W 330° Wind	1	3.02	474.75	-41.17	
	1a	31.45	-410.76	-21.93	
	1b	4.64	32.02	-4.65	
0.9D + 1.0W Normal Wind	1	-0.02	555.43	-49.12	
	1a	16.83	-241.72	-16.64	
	1b	-16.82	-241.70	-16.67	
0.9D + 1.0W 30° Wind	1	-3.06	466.11	-40.73	
	1a	-4.26	24.00	-4.85	
	1b	-31.79	-418.10	-22.17	

0.9D + 1.0W 60° Wind	1	-5.25	275.77	-23.71
	1a	-23.15	275.76	7.32
	1b	-38.13	-479.52	-22.02
0.9D + 1.0W 90° Wind	1	-6.33	24.01	-1.26
	1a	-36.80	466.10	17.72
	1b	-35.09	-418.10	-16.46
0.9D + 1.0W 120° Wind	1	-5.99	-241.71	22.90
	1a	-42.54	555.42	24.55
	1b	-22.85	-241.70	-6.24
0.9D + 1.0W 150° Wind	1	-3.27	-418.10	38.62
	1a	-33.75	466.10	22.99
	1b	-2.09	24.01	6.13
0.9D + 1.0W 180° Wind	1	0.02	-479.52	44.03
	1a	-17.91	275.76	16.38
	1b	17.90	275.77	16.41
0.9D + 1.0W 210° Wind	1	3.30	-418.10	38.62
	1a	2.07	24.00	6.11
	1b	33.74	466.11	23.01
0.9D + 1.0W 240° Wind	1	6.01	-241.71	22.90
	1a	22.84	-241.72	-6.25
	1b	42.54	555.43	24.57
0.9D + 1.0W 270° Wind	1	6.33	24.01	-1.26
	1a	35.09	-418.11	-16.46
	1b	36.80	466.12	17.72
0.9D + 1.0W 300° Wind	1	5.23	275.77	-23.71
	1a	38.14	-479.53	-22.01
	1b	23.16	275.77	7.30
0.9D + 1.0W 330° Wind	1	3.03	466.11	-40.73
	1a	31.81	-418.11	-22.14
	1b	4.27	24.01	-4.87
1.0D + 1.0W Normal Wind	1	0.00	162.22	-13.72
	1a	3.34	-41.11	-3.67
	1b	-3.34	-41.10	-3.68
1.0D + 1.0W 30° Wind	1	-0.81	139.04	-11.53
	1a	-2.05	26.66	-0.69
	1b	-7.12	-85.70	-5.06
1.0D + 1.0W 60° Wind	1	-1.37	90.82	-7.18
	1a	-6.90	90.81	2.41
	1b	-8.74	-101.62	-5.05
1.0D + 1.0W 90° Wind	1	-1.62	26.67	-1.43
	1a	-10.41	139.30	5.08
	1b	-7.96	-85.96	-3.65
1.0D + 1.0W 120° Wind	1	-1.51	-41.10	4.73
	1a	-11.88	162.21	6.86
	1b	-4.86	-41.10	-1.05
1.0D + 1.0W 150° Wind	1	-0.82	-85.97	8.72
	1a	-9.60	139.30	6.47
	1b	0.42	26.68	2.13
1.0D + 1.0W 180° Wind	1	0.00	-101.79	10.10
	1a	-5.54	90.89	4.77
	1b	5.53	90.91	4.78
1.0D + 1.0W 210° Wind	1	0.82	-85.97	8.72
	1a	-0.43	26.66	2.12
	1b	9.60	139.32	6.47

1.0D + 1.0W 240° Wind	1	1.51	-41.02	4.72
	1a	4.85	-41.03	-1.05
	1b	11.87	162.06	6.86

1.0D + 1.0W 270° Wind	1	1.62	26.67	-1.43
	1a	7.96	-85.98	-3.65
	1b	10.41	139.32	5.08

1.0D + 1.0W 300° Wind	1	1.36	90.82	-7.18
	1a	8.74	-101.63	-5.04
	1b	6.90	90.83	2.40

1.0D + 1.0W 330° Wind	1	0.80	139.45	-11.56
	1a	7.15	-86.12	-5.07
	1b	2.05	26.68	-0.70

1.2D + 1.0Ev + 1.0Eh	1	0.00	51.75	9.39
	1a	9.97	23.16	-5.87
	1b	-9.97	23.16	-5.87

0.9D + 1.0Ev + 1.0Eh	1	0.00	43.72	9.84
	1a	10.36	15.17	-6.10
	1b	-10.36	15.17	-6.10

Max Reactions

Leg			Overturning		
Max Uplift:	-479.53	(kips)	Moment:	11200.69	(ft-kips)
Max Down:	564.20	(kips)	Total Down:	96.01	(kips)
Max Shear:	49.56	(kips)	Total Shear:	82.43	(kips)

Analysis Summary

Structure: FL10961-A	Code: TIA-222-H	8/13/2026	
Site Name: Bass	Exposure: C		
Height: 250.00 (ft)	Crest Height: 0.00		
Base Elev: 0.000 (ft)	Site Class: D - Stiff Soil		
Gh: 0.85	Topography: 1	Struct Class: II	Page: 15



Max Reactions

Leg		Overturning	
Max Uplift:	-479.53 (kips)	Moment:	11200.69 (ft-kips)
Max Down:	564.20 (kips)	Total Down:	96.01 (kips)
Max Shear:	49.56 (kips)	Total Shear:	82.43 (kips)

Anchor Bolts

Bolt Size (in.): 2.00	Number Bolts: 6	Type: UnGrouted
Yield Strength (Ksi): 50.00	Tensile Strength (Ksi): 65.00	
	Length: 1.50	

Interaction Ratios:

Tensile: **0.656** Compression: **0.682** Shear: **0.130**

Max Usages

Max Leg: 68.4% (1.2D + 1.0W 240° Wind - Sect 2)
 Max Diag: 81.5% (1.2D + 1.0W 210° Wind - Sect 10)
 Max Horiz: 7.9% (1.2D + 1.0W 120° Wind - Sect 13)

Max Deflection, Twist and Sway

Load Case	Elevation (ft)	Deflection (ft)	Twist (deg)	Sway (deg)
0.9D + 1.0Ev + 1.0Eh - Normal To Face	200.00	0.0579	-0.0007	0.0428
	227.90	0.0788	-0.0001	0.0498
	243.50	0.0927	-0.0001	0.0491
	250.00	0.0985	0.0000	0.0509
0.9D + 1.0W 119 mph Wind at 120° From Face	200.00	1.4557	-0.0256	1.0704
	227.90	1.9411	-0.0203	0.9568
	243.50	2.2456	-0.0242	1.0892
	250.00	2.3709	-0.0266	1.1298
0.9D + 1.0W 119 mph Wind at 150° From Face	200.00	1.4085	-0.0209	1.0177
	227.90	1.8804	-0.0234	0.9280
	243.50	2.1767	-0.0293	1.0688
	250.00	2.2981	-0.0333	1.1065
0.9D + 1.0W 119 mph Wind at 180° From Face	200.00	1.3923	-0.0250	1.0229
	227.90	1.8597	-0.0187	0.9161
	243.50	2.1529	-0.0274	1.0486
	250.00	2.2736	-0.0338	1.0857
0.9D + 1.0W 119 mph Wind at 210° From Face	200.00	1.4078	-0.0266	1.0198
	227.90	1.8796	-0.0160	0.9272
	243.50	2.1750	-0.0205	1.0700
	250.00	2.2977	-0.0243	1.1025
0.9D + 1.0W 119 mph Wind at 240° From Face	200.00	1.4556	-0.0215	1.0705
	227.90	1.9410	-0.0110	0.9571
	243.50	2.2450	-0.0102	1.0875
	250.00	2.3706	-0.0088	1.1250

0.9D + 1.0W 119 mph Wind at 270° From Face	200.00	1.4078	0.0213	1.0193
	227.90	1.8794	0.0086	0.9242
	243.50	2.1749	0.0124	1.0703
	250.00	2.2976	0.0156	1.1029
0.9D + 1.0W 119 mph Wind at 30° From Face	200.00	1.4078	0.0266	1.0201
	227.90	1.8795	0.0160	0.9271
	243.50	2.1751	0.0203	1.0696
	250.00	2.2975	0.0240	1.1026
0.9D + 1.0W 119 mph Wind at 300° From Face	200.00	1.3923	0.0219	1.0226
	227.90	1.8596	0.0144	0.9161
	243.50	2.1529	0.0227	1.0489
	250.00	2.2736	0.0288	1.0861
0.9D + 1.0W 119 mph Wind at 330° From Face	200.00	1.4086	0.0210	1.0181
	227.90	1.8805	0.0234	0.9283
	243.50	2.1769	0.0293	1.0691
	250.00	2.2984	0.0333	1.1068
0.9D + 1.0W 119 mph Wind at 60° From Face	200.00	1.3919	0.0178	1.0225
	227.90	1.8591	0.0051	0.9157
	243.50	2.1517	0.0085	1.0459
	250.00	2.2729	0.0105	1.0807
0.9D + 1.0W 119 mph Wind at 90° From Face	200.00	1.4077	-0.0213	1.0193
	227.90	1.8792	-0.0086	0.9231
	243.50	2.1748	-0.0122	1.0696
	250.00	2.2972	-0.0154	1.1027
0.9D + 1.0W 119 mph Wind at Normal To Face	200.00	1.4560	0.0287	1.0709
	227.90	1.9415	0.0246	0.9574
	243.50	2.2460	0.0289	1.0894
	250.00	2.3714	0.0317	1.1300
1.0D + 1.0W 60 mph Wind at 120° From Face	200.00	0.3706	-0.0064	0.2708
	227.90	0.4940	-0.0044	0.2435
	243.50	0.5714	-0.0052	0.2769
	250.00	0.6033	-0.0059	0.2868
1.0D + 1.0W 60 mph Wind at 150° From Face	200.00	0.3581	-0.0051	0.2579
	227.90	0.4779	-0.0053	0.2353
	243.50	0.5530	-0.0066	0.2711
	250.00	0.5838	-0.0079	0.2807
1.0D + 1.0W 60 mph Wind at 180° From Face	200.00	0.3546	-0.0068	0.2614
	227.90	0.4734	-0.0050	0.2326
	243.50	0.5480	-0.0061	0.2665
	250.00	0.5786	-0.0078	0.2763
1.0D + 1.0W 60 mph Wind at 210° From Face	200.00	0.3579	-0.0070	0.2584
	227.90	0.4777	-0.0041	0.2353
	243.50	0.5527	-0.0043	0.2718
	250.00	0.5839	-0.0054	0.2801
1.0D + 1.0W 60 mph Wind at 240° From Face	200.00	0.3701	-0.0054	0.2704
	227.90	0.4934	-0.0021	0.2433
	243.50	0.5706	-0.0017	0.2765
	250.00	0.6025	-0.0013	0.2857
1.0D + 1.0W 60 mph Wind at 270° From Face	200.00	0.3580	0.0057	0.2583
	227.90	0.4778	0.0022	0.2352
	243.50	0.5529	0.0023	0.2720
	250.00	0.5840	0.0032	0.2803
1.0D + 1.0W 60 mph Wind at 30° From Face	200.00	0.3569	0.0071	0.2575
	227.90	0.4762	0.0041	0.2342
	243.50	0.5509	0.0043	0.2703
	250.00	0.5818	0.0053	0.2786
1.0D + 1.0W 60 mph Wind at 300° From Face	200.00	0.3541	0.0061	0.2610
	227.90	0.4728	0.0040	0.2323
	243.50	0.5473	0.0049	0.2663
	250.00	0.5779	0.0065	0.2761

1.0D + 1.0W 60 mph Wind at 330° From Face	200.00	0.3587	0.0051	0.2583
	227.90	0.4788	0.0053	0.2362
	243.50	0.5542	0.0066	0.2720
	250.00	0.5851	0.0079	0.2816
1.0D + 1.0W 60 mph Wind at 60° From Face	200.00	0.3539	0.0051	0.2609
	227.90	0.4724	0.0016	0.2319
	243.50	0.5466	0.0013	0.2649
	250.00	0.5773	0.0019	0.2741
1.0D + 1.0W 60 mph Wind at 90° From Face	200.00	0.3579	-0.0057	0.2582
	227.90	0.4775	-0.0022	0.2345
	243.50	0.5524	-0.0023	0.2711
	250.00	0.5834	-0.0031	0.2795
1.0D + 1.0W 60 mph Wind at Normal To Face	200.00	0.3707	0.0071	0.2709
	227.90	0.4942	0.0055	0.2438
	243.50	0.5717	0.0064	0.2772
	250.00	0.6036	0.0072	0.2871
1.2D + 1.0Ev + 1.0Eh - Normal To Face	200.00	0.0580	0.0007	0.0429
	227.90	0.0790	0.0001	0.0500
	243.50	0.0929	0.0001	0.0493
	250.00	0.0987	0.0000	0.0511
1.2D + 1.0W 119 mph Wind at 120° From Face	200.00	1.4588	-0.0256	1.0733
	227.90	1.9456	-0.0203	0.9601
	243.50	2.2510	-0.0243	1.0924
	250.00	2.3766	-0.0267	1.1332
1.2D + 1.0W 119 mph Wind at 150° From Face	200.00	1.4115	-0.0209	1.0204
	227.90	1.8847	-0.0235	0.9311
	243.50	2.1818	-0.0294	1.0719
	250.00	2.3037	-0.0334	1.1098
1.2D + 1.0W 119 mph Wind at 180° From Face	200.00	1.3953	-0.0251	1.0259
	227.90	1.8639	-0.0187	0.9191
	243.50	2.1581	-0.0275	1.0517
	250.00	2.2791	-0.0339	1.0890
1.2D + 1.0W 119 mph Wind at 210° From Face	200.00	1.4108	-0.0266	1.0225
	227.90	1.8839	-0.0160	0.9304
	243.50	2.1803	-0.0205	1.0733
	250.00	2.3033	-0.0244	1.1059
1.2D + 1.0W 119 mph Wind at 240° From Face	200.00	1.4587	-0.0216	1.0734
	227.90	1.9455	-0.0111	0.9605
	243.50	2.2504	-0.0103	1.0909
	250.00	2.3764	-0.0088	1.1286
1.2D + 1.0W 119 mph Wind at 270° From Face	200.00	1.4108	0.0214	1.0221
	227.90	1.8838	0.0086	0.9276
	243.50	2.1802	0.0124	1.0736
	250.00	2.3033	0.0157	1.1064
1.2D + 1.0W 119 mph Wind at 30° From Face	200.00	1.4108	0.0266	1.0228
	227.90	1.8838	0.0161	0.9302
	243.50	2.1803	0.0204	1.0728
	250.00	2.3031	0.0241	1.1059
1.2D + 1.0W 119 mph Wind at 300° From Face	200.00	1.3952	0.0219	1.0257
	227.90	1.8639	0.0144	0.9192
	243.50	2.1582	0.0228	1.0521
	250.00	2.2792	0.0288	1.0895
1.2D + 1.0W 119 mph Wind at 330° From Face	200.00	1.4116	0.0211	1.0208
	227.90	1.8849	0.0235	0.9315
	243.50	2.1822	0.0294	1.0723
	250.00	2.3040	0.0334	1.1102
1.2D + 1.0W 119 mph Wind at 60° From Face	200.00	1.3949	0.0178	1.0256
	227.90	1.8633	0.0051	0.9186
	243.50	2.1568	0.0086	1.0488
	250.00	2.2783	0.0106	1.0838

1.2D + 1.0W 119 mph Wind at 90° From Face	200.00	1.4107	-0.0214	1.0220
	227.90	1.8835	-0.0086	0.9263
	243.50	2.1799	-0.0122	1.0727
	250.00	2.3027	-0.0154	1.1060

1.2D + 1.0W 119 mph Wind at Normal To Face	200.00	1.4590	0.0288	1.0737
	227.90	1.9459	0.0246	0.9608
	243.50	2.2514	0.0290	1.0927
	250.00	2.3771	0.0317	1.1334

	Mat Foundation Design for Self Supporting Tower			
	Customer Name:	AT&T	TIA Standard:	TIA-222-H
	Site Name:		Structure Height (Ft.):	250
	Site Number:	FL10961-A	Engineer Name:	
	Engr. Number:		Date:	8/10/2026

Foundation Info Obtained from:

Analysis or Design?

Number of Tower Legs:

Base Reactions (Factored 1.2D + 1.0W):

(1). Individual Leg:

Axial Load (Kips):	564.2	Uplift Force (Kips):	479.5
Shear w/ Max Leg Comp. (Kips):	49.6	Shear w/ Max tension. (Kips):	49.6

(2). Tower Base:

Total Vertical Load (Kips):	96.0	Total Shear Force (Kips):	82.4
Moment (Kips-ft):	11200.7		

Foundation Geometries:

Leg distance (Center-to-Center ft.):	24.3	Mods required -Yes/No ?:	No
Diameter of Pier (ft.):	Round 4.0	Pier Height A. G. (ft.):	0.50
Tower center to mat center (L) (ft):	3.5	Depth of Base BG (ft.):	6.0
Length of Pad (ft.):	38.5	Width of Pad (ft.):	38.5
Thickness of Pad (ft):	2.50		

Material Properties and Rebar Info:

Concrete Strength (psi):	3000	Steel Elastic Modulus:	29000	ksi
Vertical bar yield (ksi)	60	Tie steel yield (ksi):	60	
Vertical Rebar Size #:	9	Tie / Stirrup Size #:	4	
Qty. of Vertical Rebars:	24	Tie Spacing (in):	12.0	
Pad Rebar Yield (Ksi):	60	Pad Steel Rebar Size (#):	9	
Concrete Cover (in.):	3	Unit Weight of Concrete:	150.0	pcf

Rebar at the bottom of the concrete pad:

Qty. of Rebar in Pad (L):	39	Qty. of Rebar in Pad (W):	39
---------------------------	----	---------------------------	----

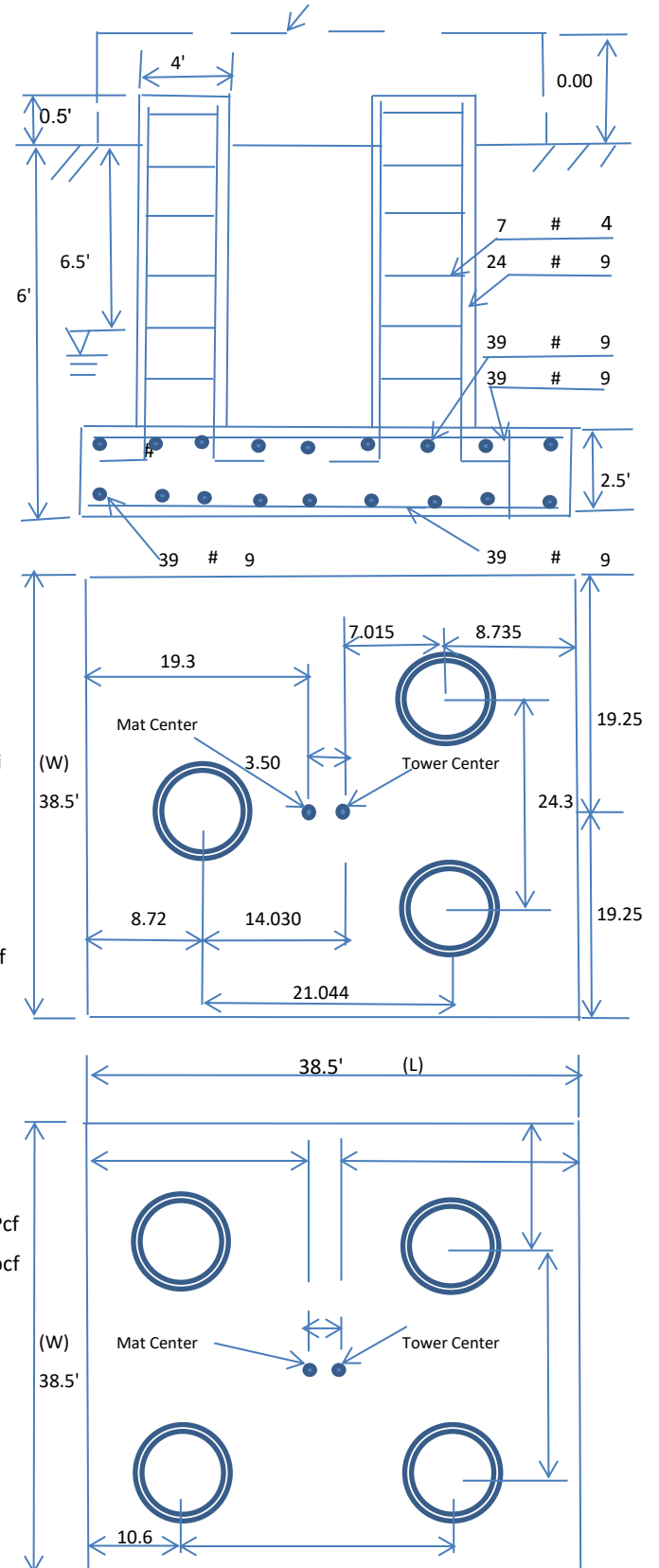
Rebar at the top of the concrete pad

Qty. of Rebar in Pad (L):	39	Qty. of Rebar in Pad (W):	39
---------------------------	----	---------------------------	----

Soil Design Parameters:

Soil Unit Weight (pcf):	110.0	Soil Buoyant Weight:	47.6	Pcf
Water Table B.G.S. (ft):	6.5	Unit Weight of Water:	62.4	pcf
Ultimate Bearing Pressure (psf):	9000			
Consider Soil Lateral Resistance ?	No			

Soil Wedge Angle From Top of Pad:	15	Ultimate Skin Friction (psf):	0
Final Length of pad (L) (ft)	38.5	Final width of pad (W) (ft):	38.5



Allowable overstress %: 5.00%
1

TES Engr. Number: 0

Page 2/2 Date: 8/10/2026

<u>Foundation Analysis and Design:</u>	Uplift Strength Reduction Factor:	0.75	Compression Strength Reduction Factor:	0.75
Total Dry Soil Volume (cu. Ft.):	5055.93	Total Dry Soil Weight (Kips):	556.15	
Total Buoyant Soil Volume (cu. Ft.):	0.00	Total Buoyant Soil Weight (Kips):	0.00	
Total Effective Soil Weight (Kips):	556.15	Weight from the Concrete Block at Top (K):	0.00	
Total Dry Concrete Volume (cu. Ft.):	3856.42	Total Dry Concrete Weight (Kips):	578.46	
Total Buoyant Concrete Volume (cu. Ft.):	0.00	Total Buoyant Concrete Weight (Kips):	0.00	
Total Effective Concrete Weight (Kips):	578.46	Total Vertical Load on Base (Kips):	1230.63	

Check Soil Capacities:

Calculated Maximum Net Soil Pressure under the base (psf):	1032	<	Allowable Factored Soil Bearing (psf):	6750	0.15	OK!
Foundation Overturning Resistance (kips-ft.):	20921	>	Design Factored Momont (kips-ft):	11736	0.68	OK!

Check the capacities of Reinforcing Concrete:

Strength reduction factor (Flexure and axial tension):	0.90	Strength reduction factor (Shear):	0.75
Strength reduction factor (Axial compression):	0.65	Wind Load Factor on Concrete Design:	1.00

(1) Concrete Pier:

Vertical Steel Rebar Area (sq. in./each):	1.00	Tie / Stirrup Area (sq. in./each):	0.20		
Calculated Moment Capacity (Mn,Kips-Ft):	1315.5	>	Design Factored Moment (Mu, Kips-Ft)	198.2	0.15 OK!
Calculated Shear Capacity (Kips):	128.8	>	Design Factored Shear (Kips):	49.6	0.38 OK!
Calculated Tension Capacity (Tn, Kips):	1296.0	>	Design Factored Tension (Tu Kips):	479.5	0.37 OK!
Calculated Compression Capacity (Pn, Kips):	2367.6	>	Design Factored Axial Load (Pu Kips):	564.2	0.24 OK!
Moment & Tension Strength Combination:	0.15	OK!	Check Tie Spacing (Design/Req'd):	1.00	
Pier Reinforcement Ratio:	0.013	Reinforcement Ratio is satisfied per ACI			

(2).Concrete Pad:

One-Way Design Shear Capacity (L or W Direction, Kips):	982.1	>	One-Way Factored Shear (L/W-Dir Kips	192.8	0.20	OK!
One-Way Design Shear Capacity (Diagonal Dir., Kips):	994.2	>	One-Way Factored Shear (Dia. Dir, Kips	331.6	0.33	OK!
Lower Steel Pad Reinforcement Ratio (L or W-Direct.):	0.0028		Lower Steel Reinf. Ratio (Dia. Dir.):	0.0028		
Lower Steel Pad Moment Capacity (L or W-Dir. Kips-ft):	4366.8	>	Moment at Bottom (L-Direct. K-Ft):	735.0	0.17	OK!
Lower Steel Pad Moment Capacity (Dia. Direction, K-ft):	4985.0	>	Moment at Bottom (Dia. Dir. K-Ft):	2472.1	0.50	OK!
Upper Steel Pad Reinforcement Ratio (L or W -Direction):	0.0028		Upper Steel Reinf. Ratio (Dia. Dir.):	0.0028		
Upper Steel Pad Moment Capacity (L or W-Dir., Kips-ft):	4366.8	>	Moment at the top (L-Dir Kips-Ft):	960.9	0.22	OK!
Upper Steel Pad Moment Capacity (Dia. Direction, K-ft):	4985.0	>	Moment at the top (Dia. Dir., K-Ft):	2419.6	0.49	OK!
Punching Failure Capacity (Kips):	986.8	>	Punch. Failure Factored Shear (K):	564.2	0.57	OK!
Punching Failure Capacity From Uplift (Kips):	891.6	>	Punch. Failure Factored Shear (K):	479.5	0.54	OK!

The maximum eccentricity of Loading:	9.66	Allowable eccentricity (0.45 W, ft.):	17.325	OK!
--------------------------------------	------	---------------------------------------	--------	-----