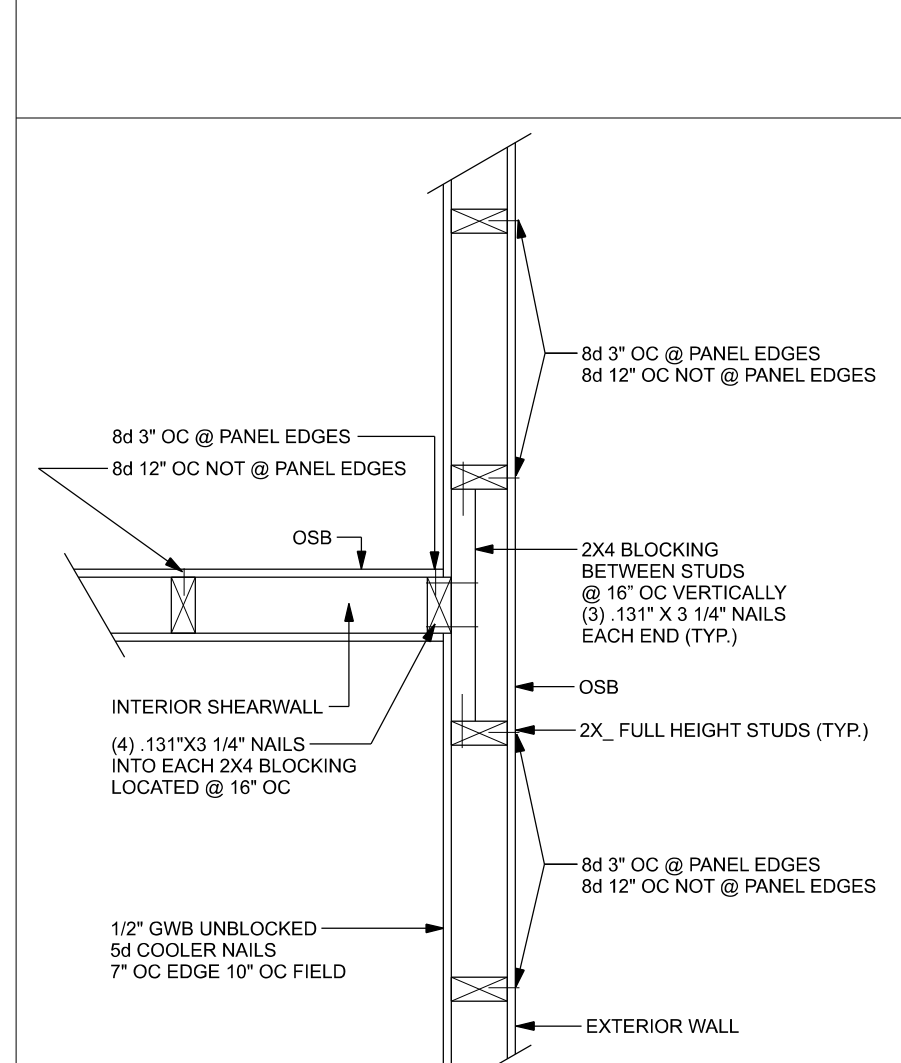
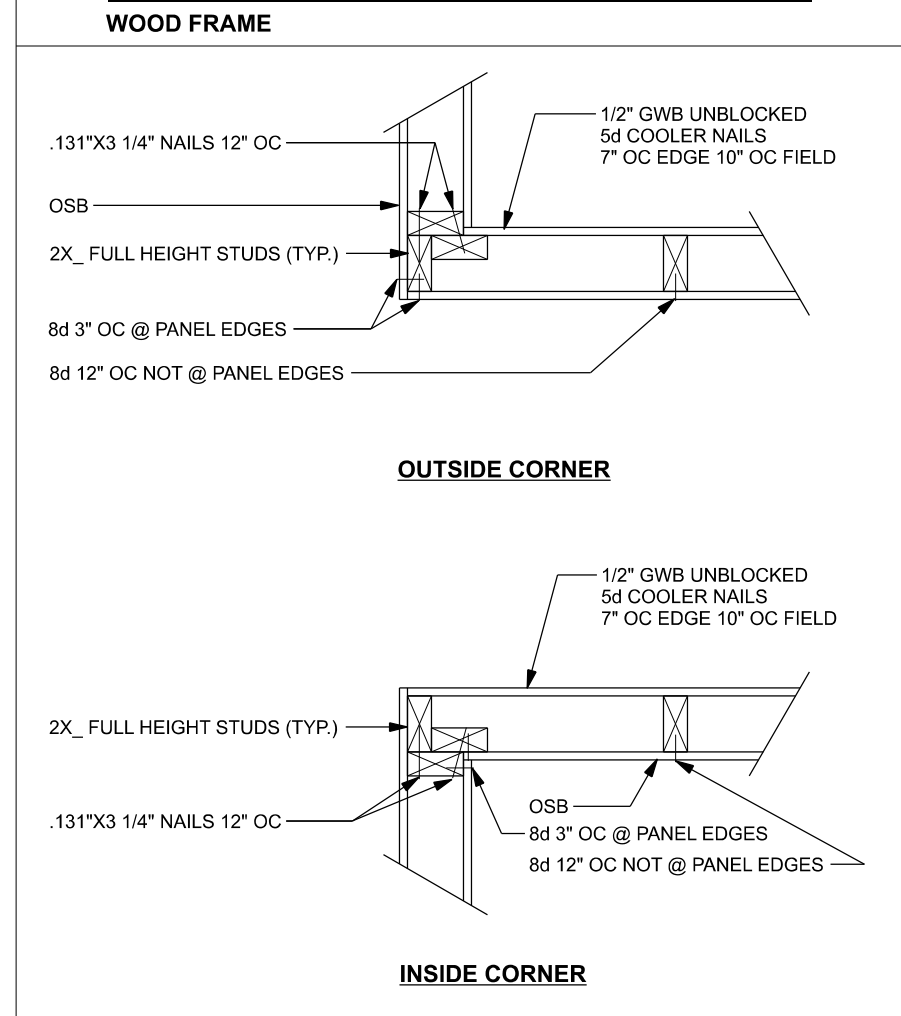


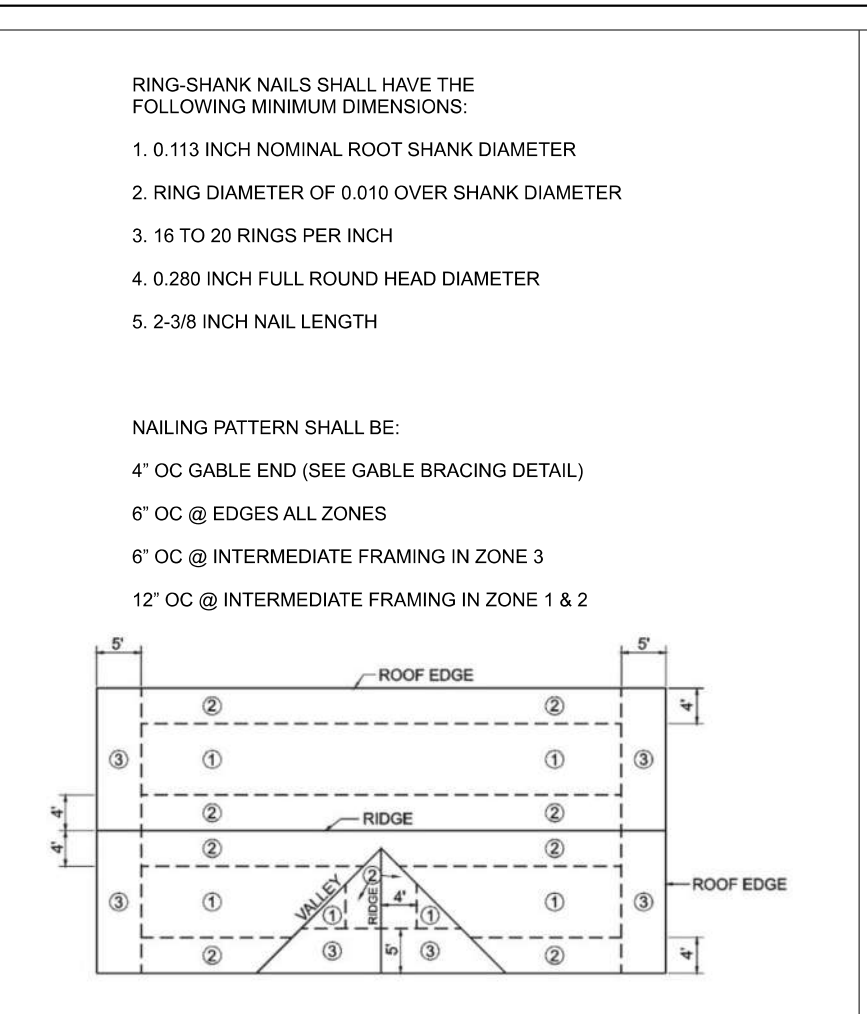
**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



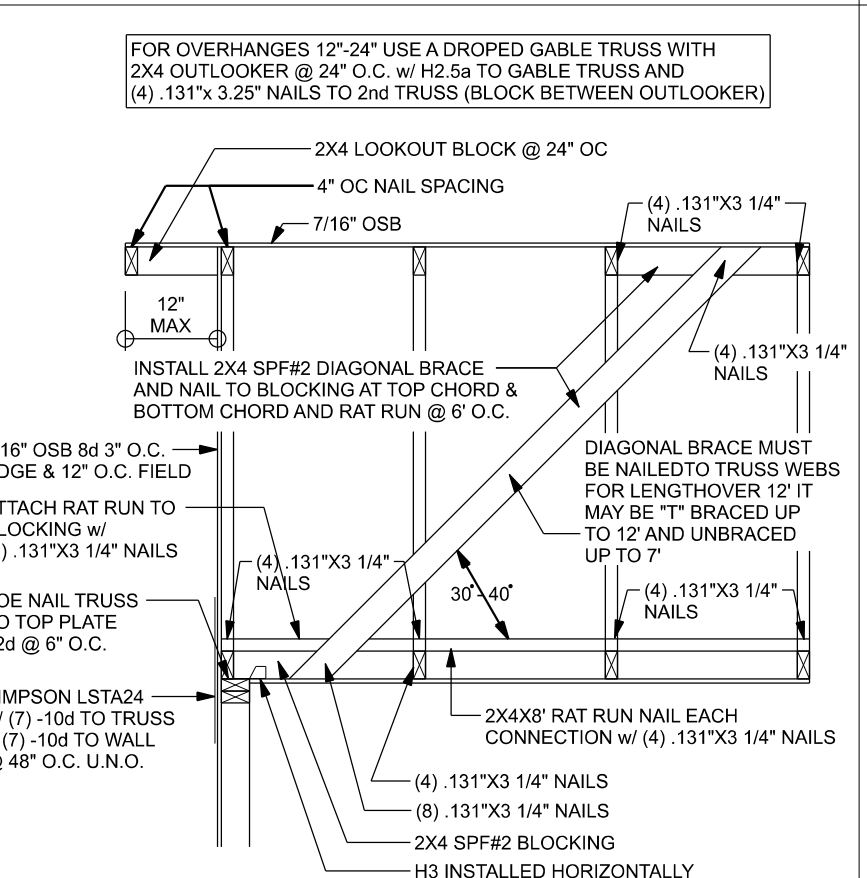
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



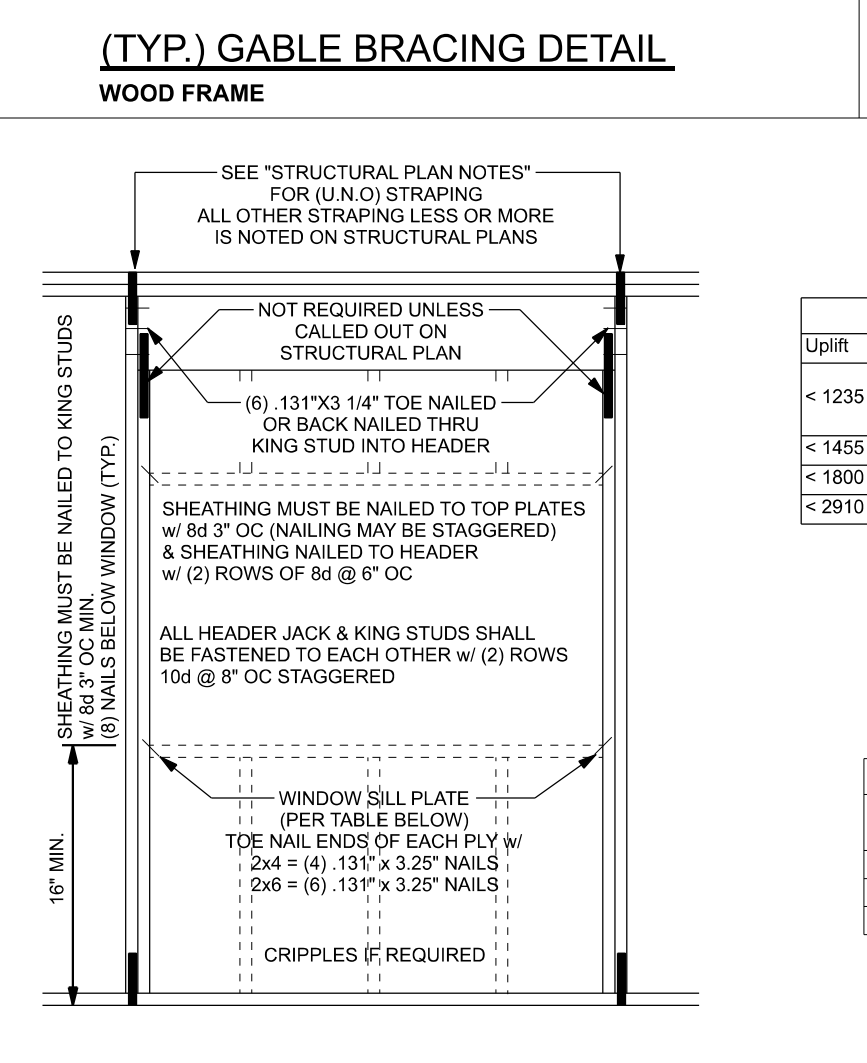
**(TYP.) CORNER FRAMING**  
WOOD FRAME



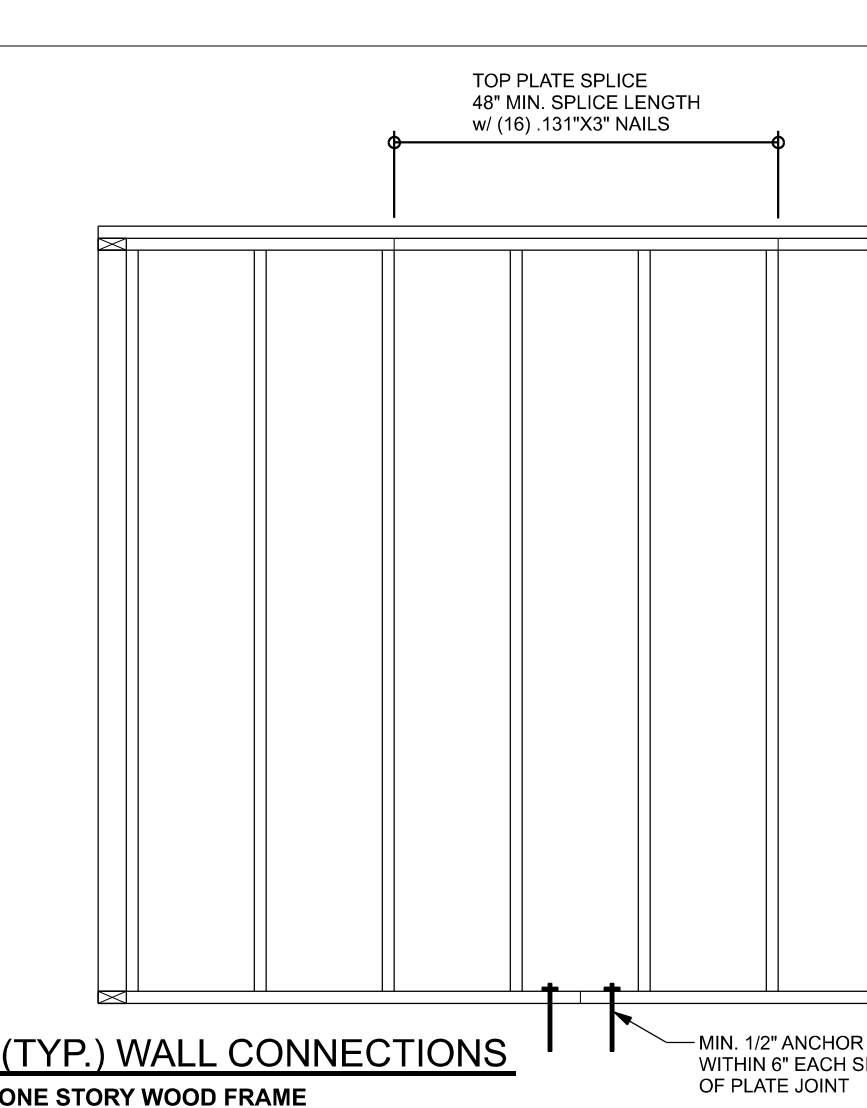
**ROOF SHEATHING FASTENING**



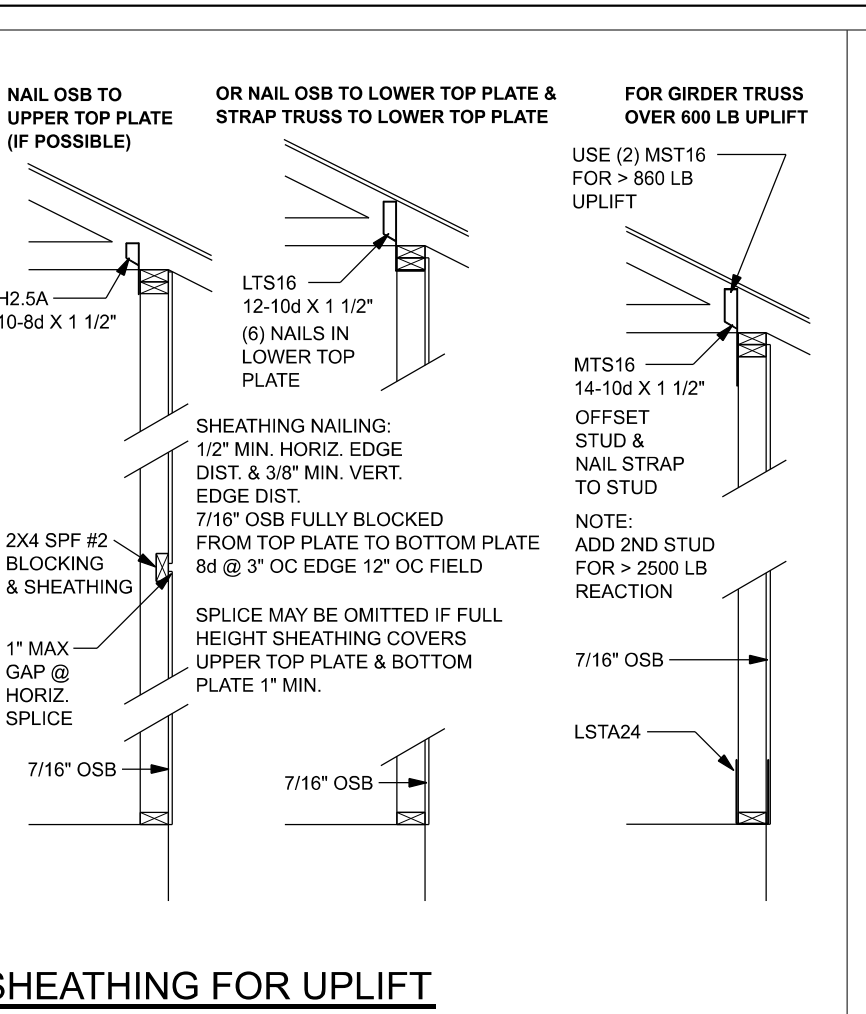
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



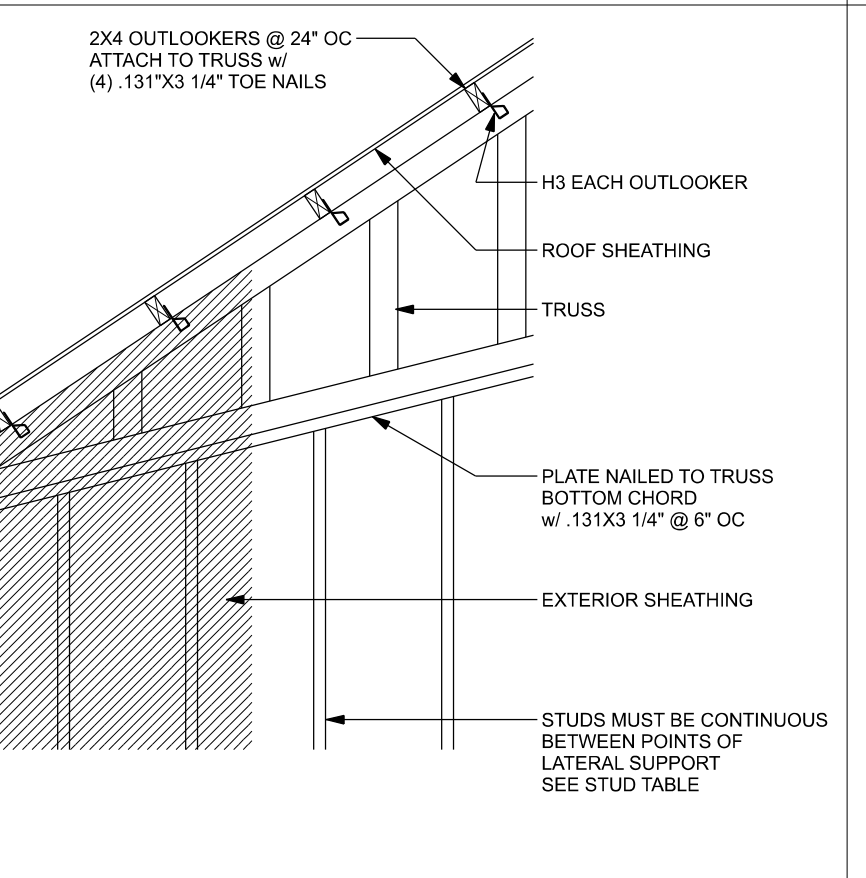
**TYPICAL HEADER STRAPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



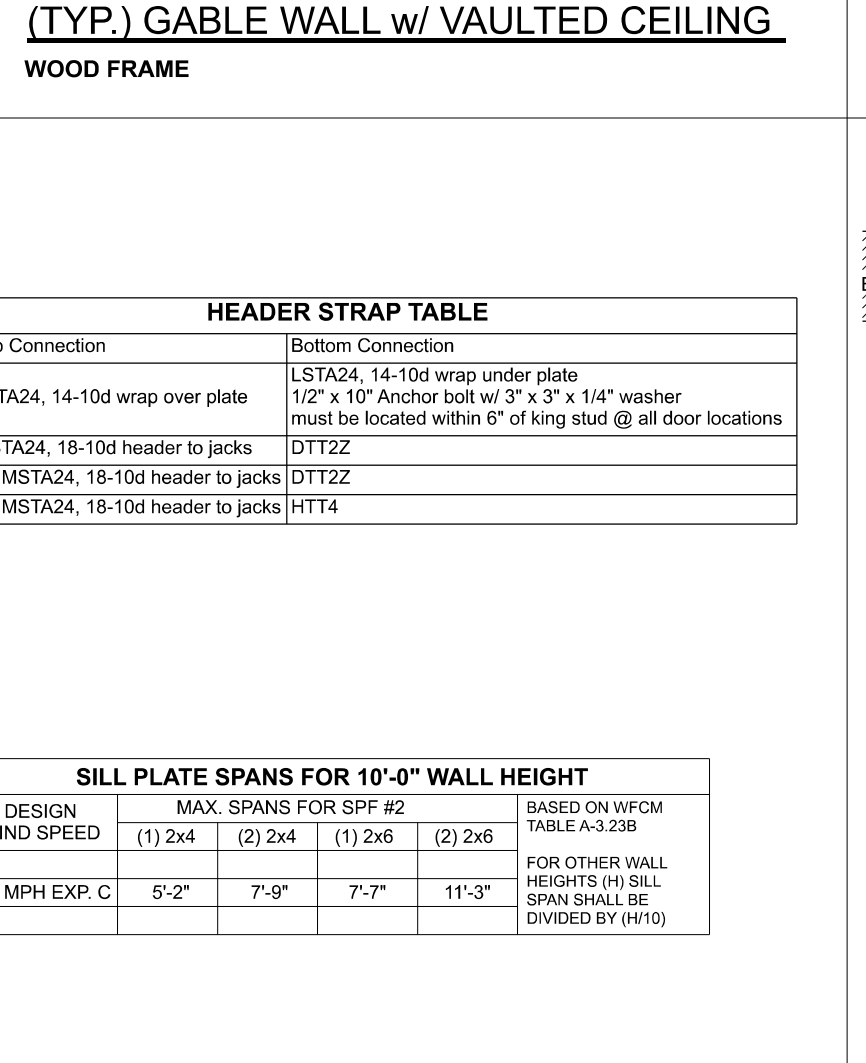
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



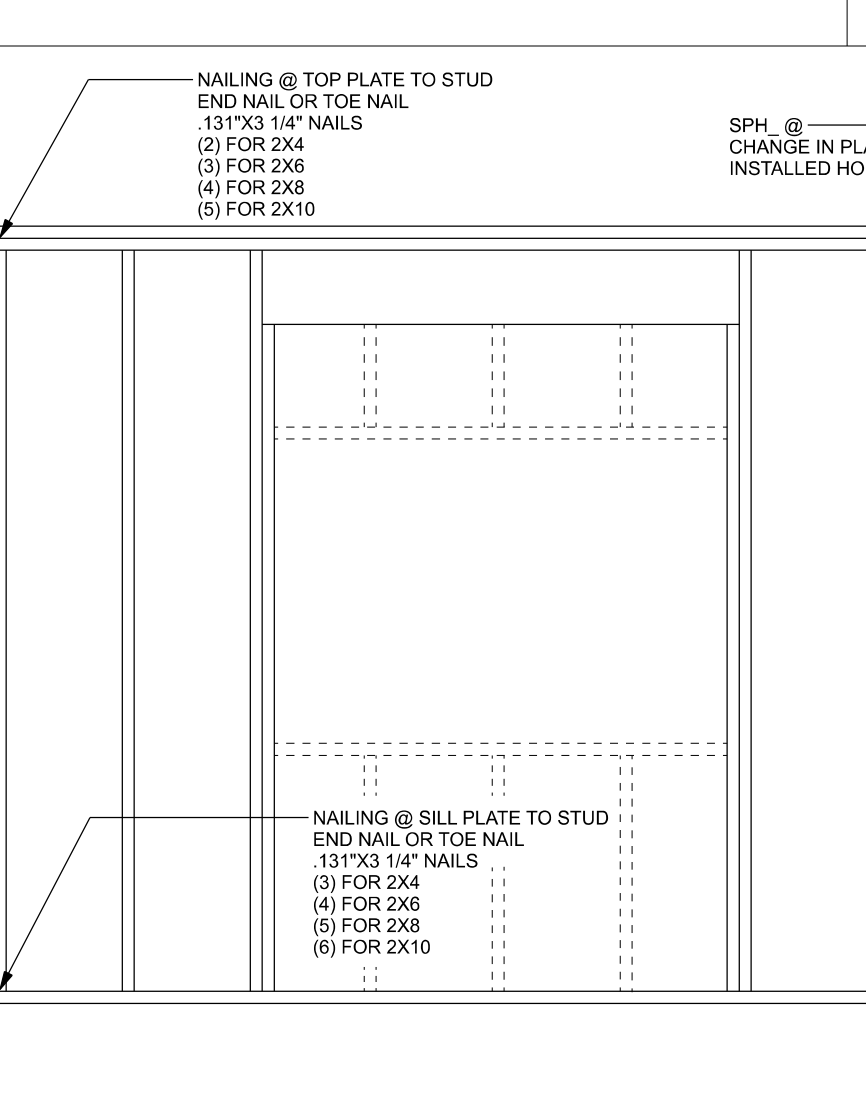
**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME



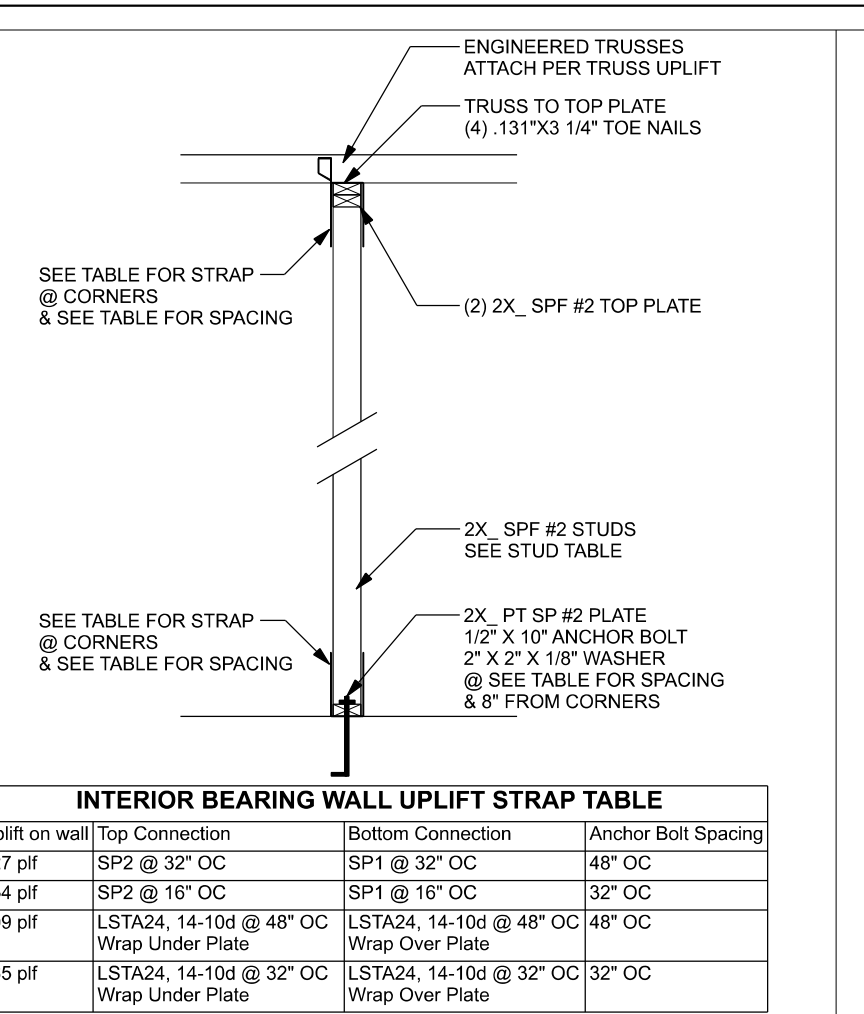
**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME



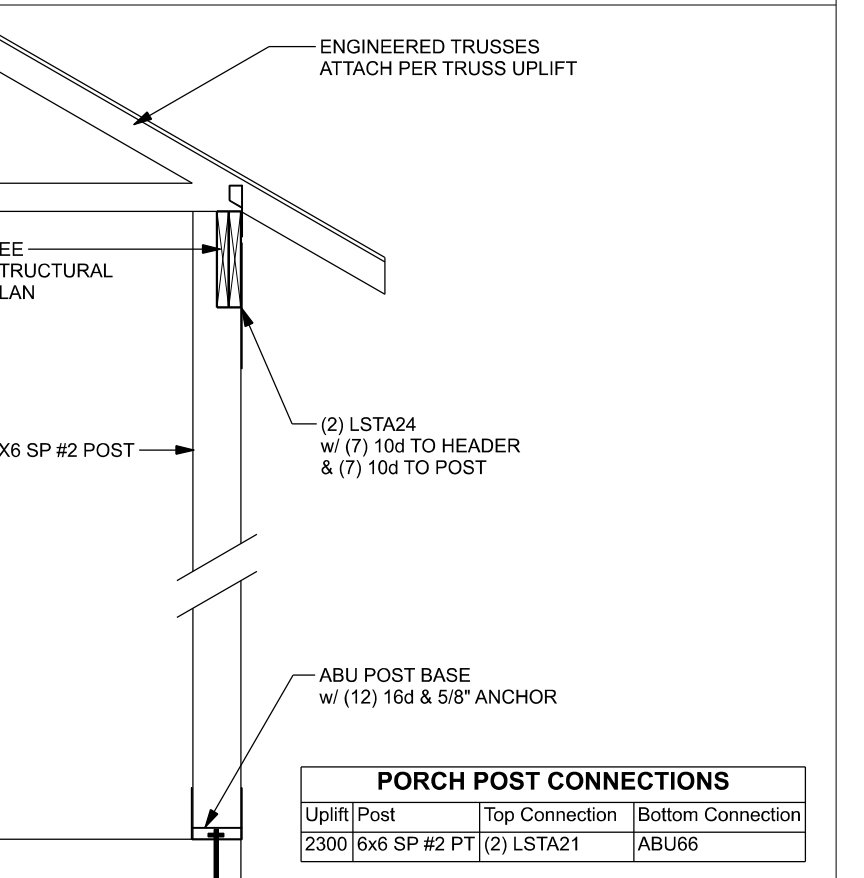
**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ STRAPS & ANCHORS



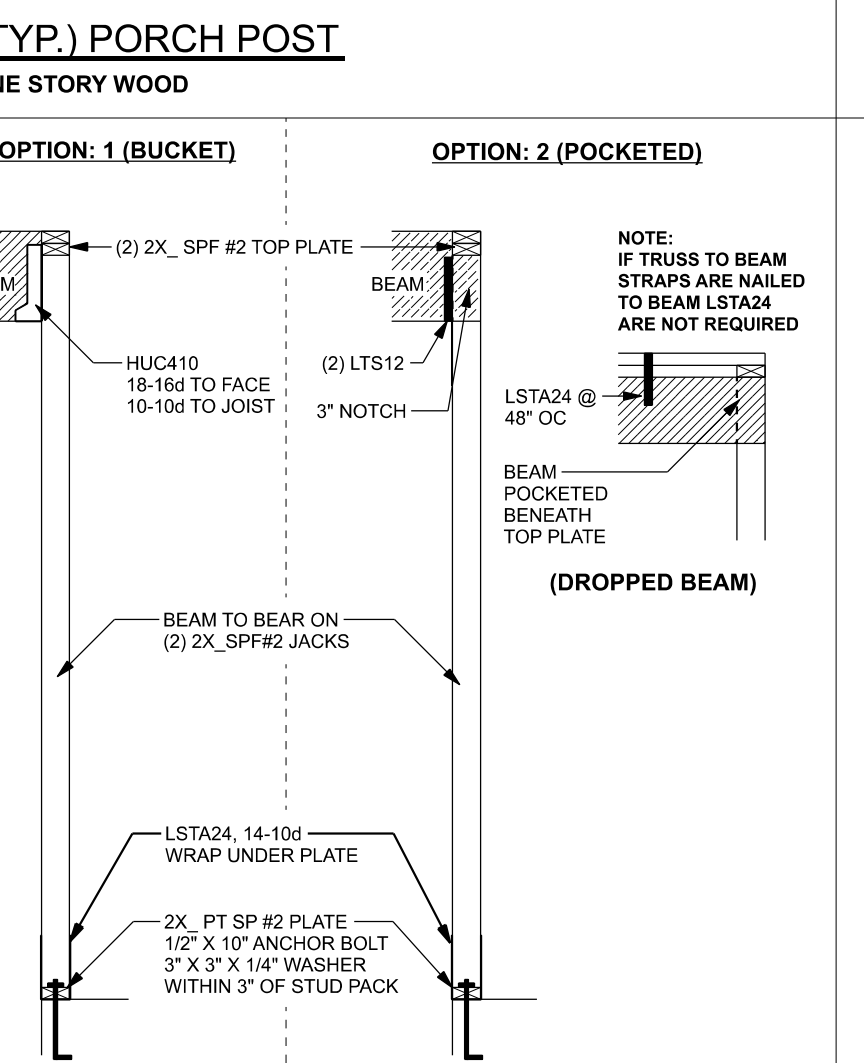
**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME



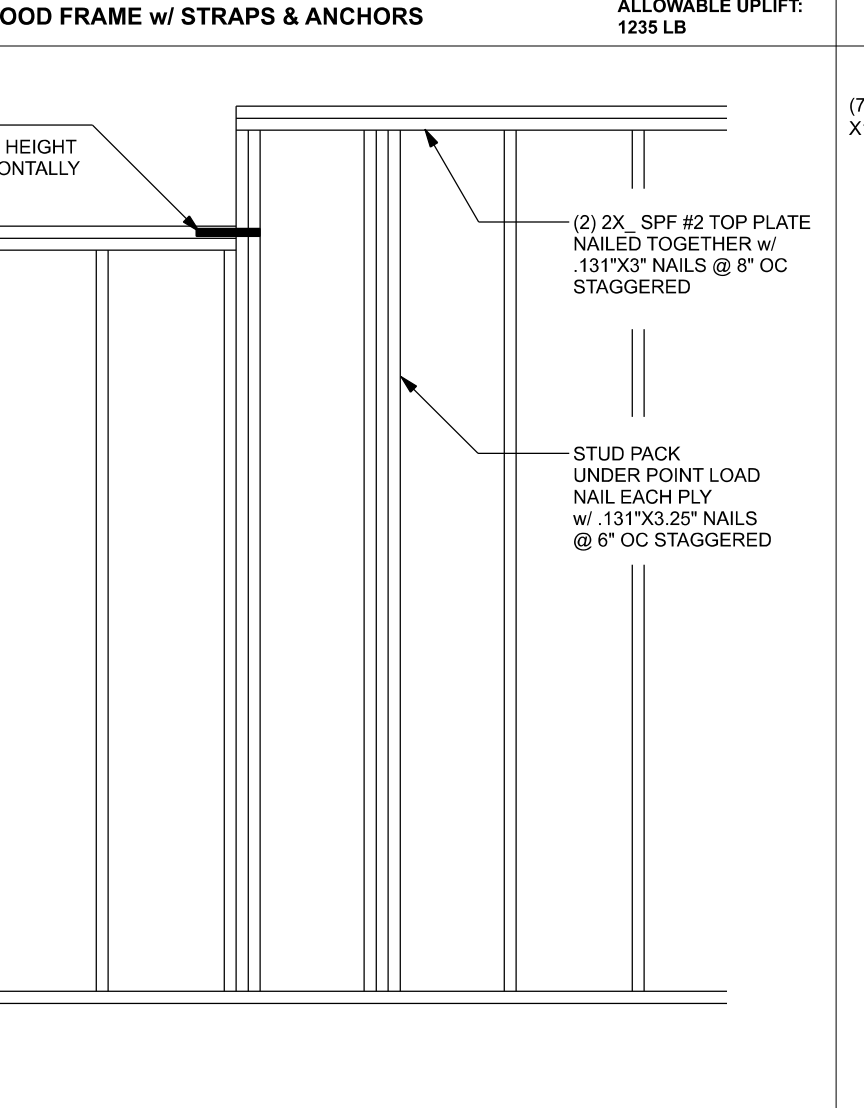
**INTERIOR BEARING WALL UPLIFT STRAP TABLE**  
(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



**PORCH POST CONNECTIONS**  
(TYP.) PORCH POST ONE STORY WOOD



**SILL PLATE SPANS FOR 10'-0\"/>**



**OPENING FORCE TRANSFER**  
WOOD FRAME

**CONNECTOR TABLE**

Uplift on Wall	Uplift on Truss	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8x1 1/2"	4-8x1 1/2"
415	290	H3	4-8x1 1/2"	4-8x1 1/2"
575	465	H2.5A	5-8x1 1/2"	5-8x1 1/2"
1340	1915	H10A	9-10x1 1/2"	9-10x1 1/2"
720	620	LTS12-20	6-10x1 1/2"	6-10x1 1/2"
1000	860	MTS12-30	7-10x1 1/2"	7-10x1 1/2"
1450	1245	HTS20-30	12-10x1 1/2"	12-10x1 1/2"
1230	1235	LSTA21	To One Member	To Other Member
1640	1455	MSTA24	9-10x1	9-10x1
1030	1030	CS20	7-10x1	7-10x1
Uplift SP	Uplift SP	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10x1	4-10x1
1065	665	SP2	6-10x1	6-10x1
771	771	LSTA24	10-10x1	wrap under or over plate
1235	1235	LSTA24	14-10x1	wrap under or over plate
Uplift SP	Uplift SP	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SP	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SP	Post Bases @ Stemwall	To Post	Anchor
2200	2200	ABU44	12-16x1	5/8"x12" Drill & Epoxy
2300	2300	ABU66	12-16x1	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SP	Post Bases @ Mono	To Post	Anchor
2200	2200	ABU44	12-16x1	5/8"x7" Drill & Epoxy
2300	2300	ABU66	12-16x1	5/8"x7" Drill & Epoxy

**EXTERIOR WALL STUD TABLE FOR SP# 2 STUDS:**  
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Uplift	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pft	SP2 @ 32" O.C.	SP1 @ 32" O.C.	48" O.C.
454 pft	SP2 @ 16" O.C.	SP1 @ 16" O.C.	32" O.C.
309 pft	LSTA24, 14-10x1 @ 48" O.C. Wrap Under Plate	LSTA24, 14-10x1 @ 48" O.C. Wrap Over Plate	48" O.C.
465 pft	LSTA24, 14-10x1 @ 32" O.C. Wrap Under Plate	LSTA24, 14-10x1 @ 32" O.C. Wrap Over Plate	32" O.C.

**GRADE & SPECIES TABLE**

Grade	Species	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:**  
TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, LAYOUT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS PROVIDED ALL ABOVE REQUIREMENTS AND TO VERIFY UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERS TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415 LB EACH END, 2X6 RAFTERS 700 LB EACH END.  
SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVIDES OTHERWISE).  
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI.  
WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, Fy = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.F.) CONFORMING TO ASTM A181, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.  
FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSEAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.  
CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH / WIDTH RATIOS OF SLAB REINERS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTRACTOR IS NOT INTENDING TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)  
REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 40 KSI, ALL LAP SPACED 40" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.C.  
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.  
STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.  
ANCHOR BOLTS: #307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.  
**BUILDER'S RESPONSIBILITY:**  
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBGCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN O.M.T.A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.  
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, LAYOUT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.  
**ROOF SYSTEM DESIGN:**  
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**DESIGN CRITERIA & LOADS:**

BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, SS GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

**COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)**

EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END + FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(VuIt) -46.2(VuIt)	+42.6(VuIt) -57(VuIt)
<b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)</b>		
9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)	
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)	

**JOB NUMBER:**  
200755  
**S-1**  
OF 3 SHEETS

**Amira Custom Homes**  
**Nicewonger Res.**  
PROJECT ADDRESS: 163 SW Midtown Place, Suite 103, Lake City, Florida 32025  
Parcel ID: 32-65-16-0401-002, Columbia County, FL

**DESIGN CRITERIA & LOADS:**

**BUILDING CODE:** 6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)

**CODE FOR DESIGN LOADS:** ASCE 7-10

**WINDLOADS:** BASIC WIND SPEED (ASCE 7-10, SS GUST) 130 MPH; WIND EXPOSURE (BUILDER MUST FIELD VERIFY) C; RISK CATEGORY II; ENCLOSURE CLASSIFICATION ENCLOSED; INTERNAL PRESSURE COEFFICIENT 0.18; ROOF ANGLE 7-45 DEGREES; MEAN ROOF HEIGHT 30 FT.

**C&C DESIGN PRESSURES:** SEE TABLE

**FLOOR LOADING:** ROOMS OTHER THAN SLEEPING ROOM 40 PSF LIVE LOAD; SLEEPING ROOMS 30 PSF LIVE LOAD

**ROOF LOADING:** FLAT OR < 4:12 20 PSF LIVE LOAD; 4:12 TO < 12:12 16 PSF LIVE LOAD; 12:12 & GREATER 12 PSF LIVE LOAD

**SOIL BEARING CAPACITY:** 1500 PSF

**FLOOD ZONE:** THIS BUILDING IS NOT IN THE FLOOD ZONE

**COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)**

**EFFECTIVE WIND AREA (FT<sup>2</sup>)**

ZONE 4 INTERIOR	ZONE 5 END + FROM ALL OUTSIDE CORNER
0 - 20: +25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20: +42.6(VuIt) -46.2(VuIt)	+42.6(VuIt) -57(VuIt)

**GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)**

9x7 GARAGE DOOR: +22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR: +21.7(Vasd) -24.1(Vasd)

**DIMENSIONS:** Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**MARK DISOWAY P.E. 53915**

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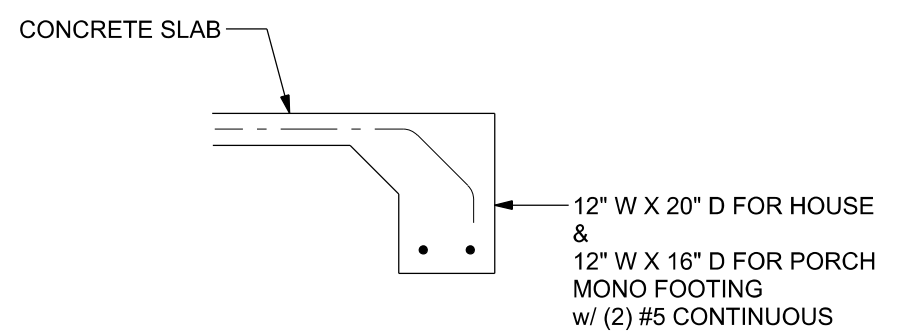
**Friday, July 17, 2020**

**Mark Disoway P.E.**  
163 SW Midtown Place  
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disowaydesign@gmail.com

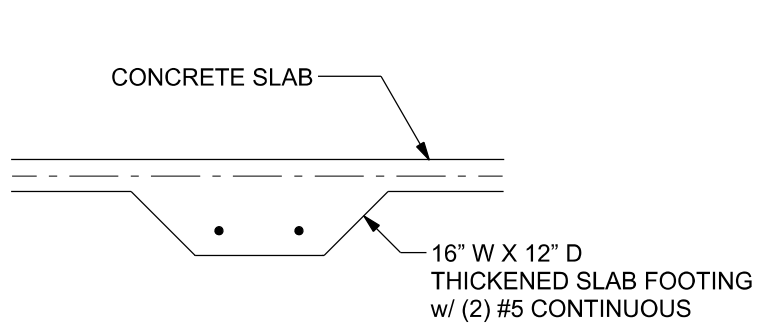
**JOB NUMBER:**  
200755  
**S-1**  
OF 3 SHEETS

**MASONRY NOTE:**  
 MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

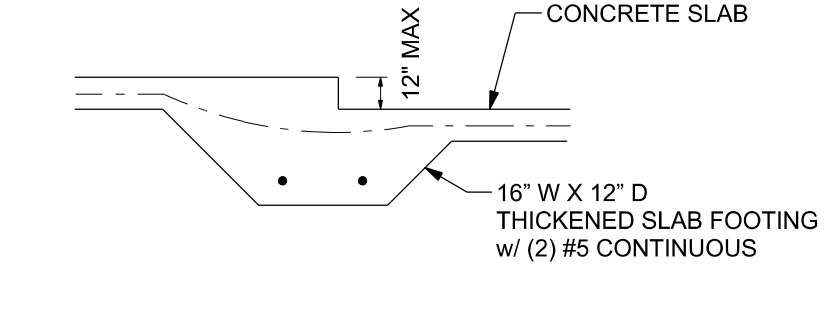
ACI/ASCE/TMS Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints



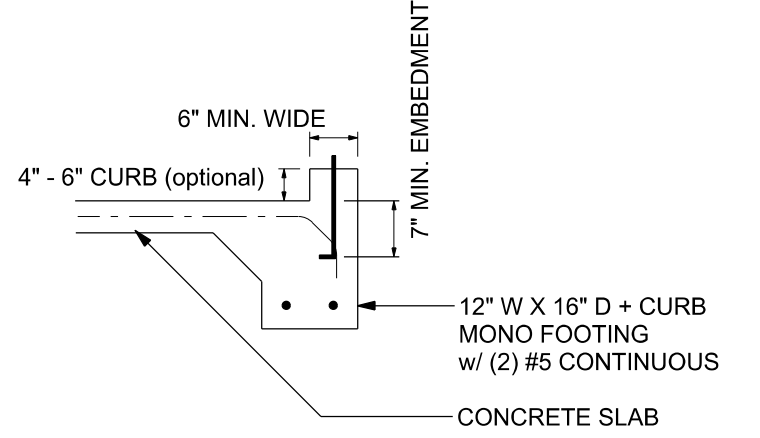
**F1 S-2** OPTIONAL MONOLITHIC FOOTING  
 SCALE: 1/2" = 1'-0"



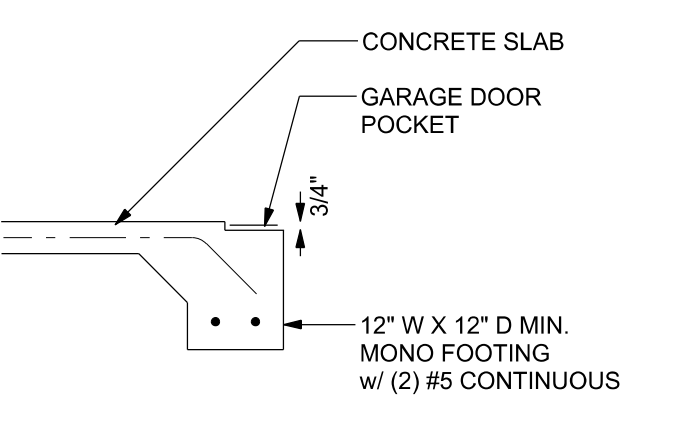
**F2 S-2** INTERIOR BEARING FOOTING  
 SCALE: 1/2" = 1'-0"



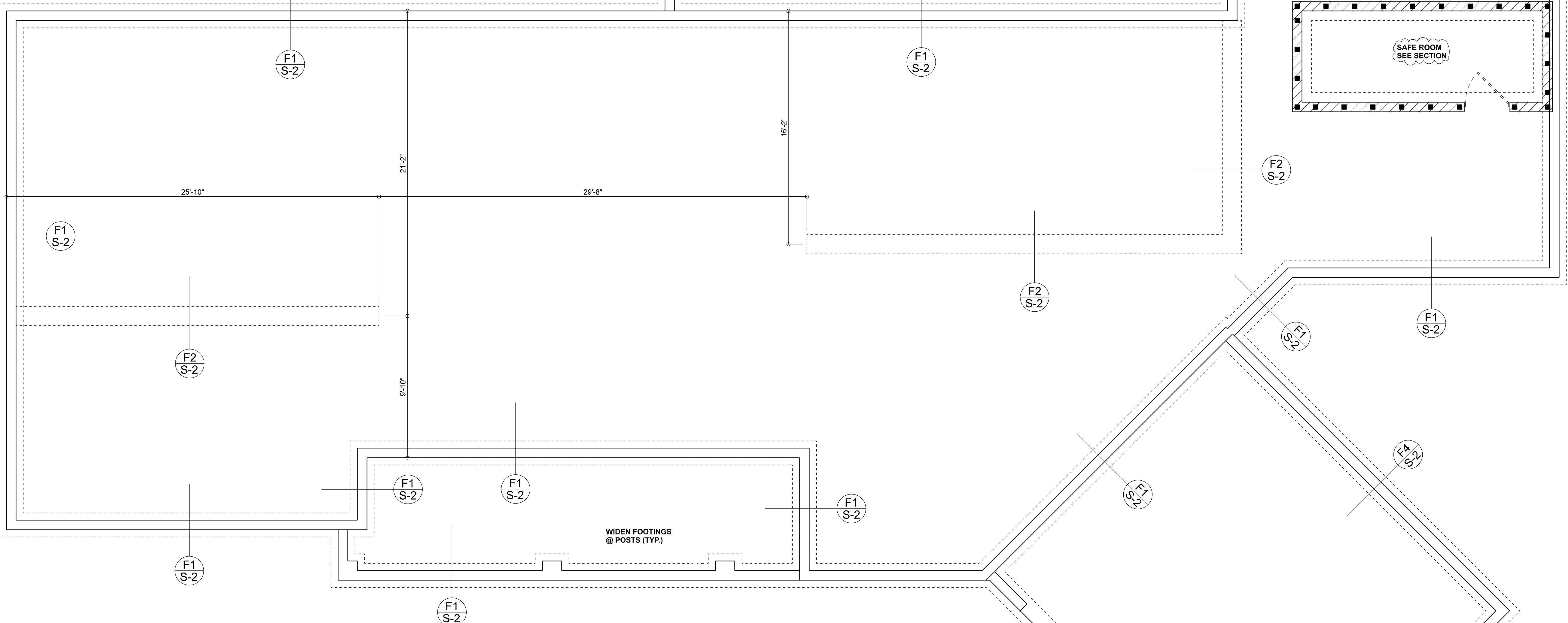
**F3 S-2** INTERIOR BEARING STEP FOOTING  
 SCALE: 1/2" = 1'-0"



**F4 S-2** OPTIONAL MONOLITHIC CURB FOOTING  
 SCALE: 1/2" = 1'-0"



**F5 S-2** GARAGE DOOR POCKET FOOTING  
 SCALE: 1/2" = 1'-0"

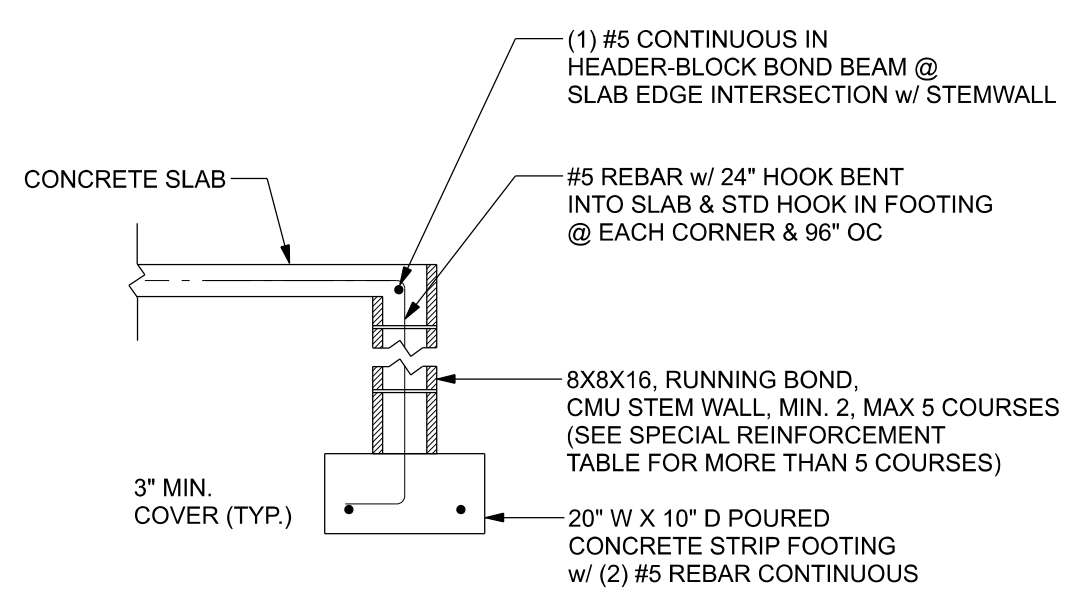


**FOUNDATION PLAN**

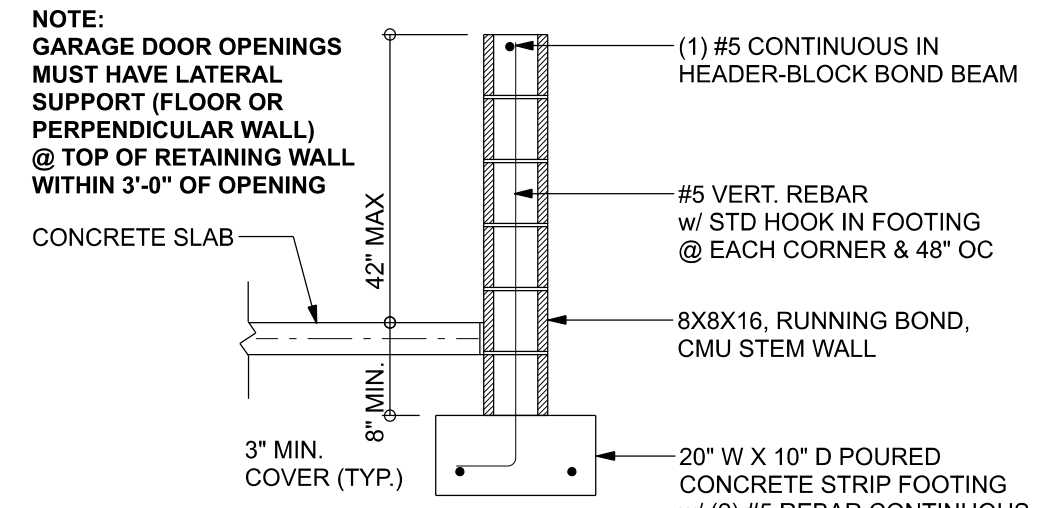
SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

- DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- THE SLAB SHALL BE 4\"/>



**F1 S-2** STEM WALL FOOTING  
 SCALE: 1/2" = 1'-0"



**F4 S-2** STEM WALL CURB FOOTING  
 SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12\"/>

**TALL STEM WALL TABLE:**  
 The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6\"/>

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8\"/> <th colspan="3">VERTICAL REINFORCEMENT FOR 12\"/&gt; </th>			VERTICAL REINFORCEMENT FOR 12\"/>		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

Amira Custom Homes  
 Nicewonger Res.  
 PROJECT ADDRESS:  
 Parcel ID: 32-65-16-0416-002  
 Columbia County, FL

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

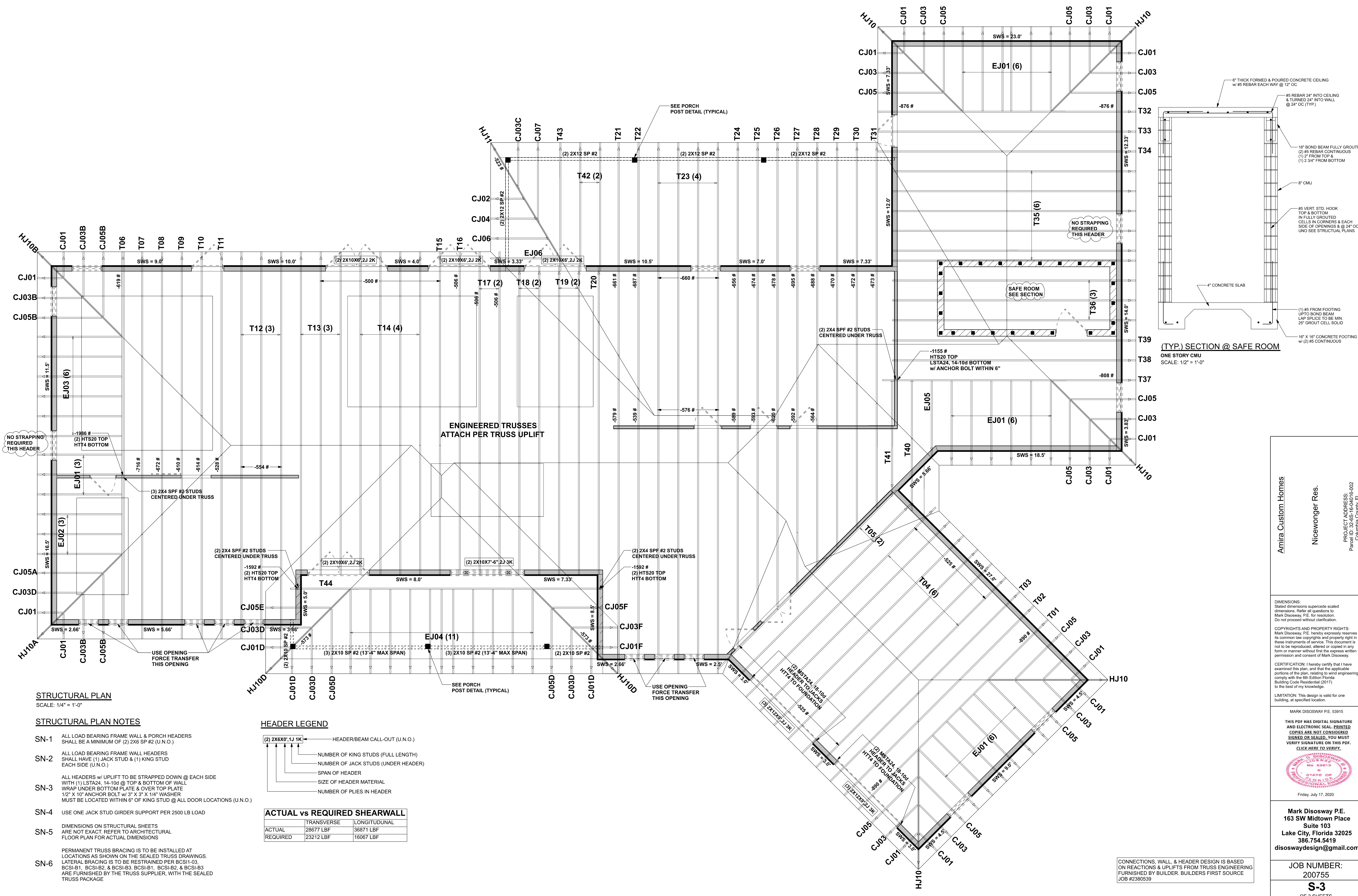
**LIMITATION:** This design is valid for one building, at specified location.

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Friday, July 17, 2020

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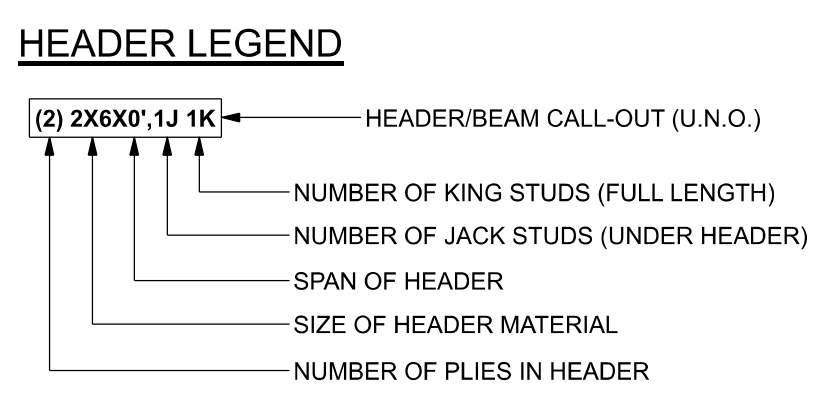
JOB NUMBER:  
 200755  
**S-2**  
 OF 3 SHEETS



**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

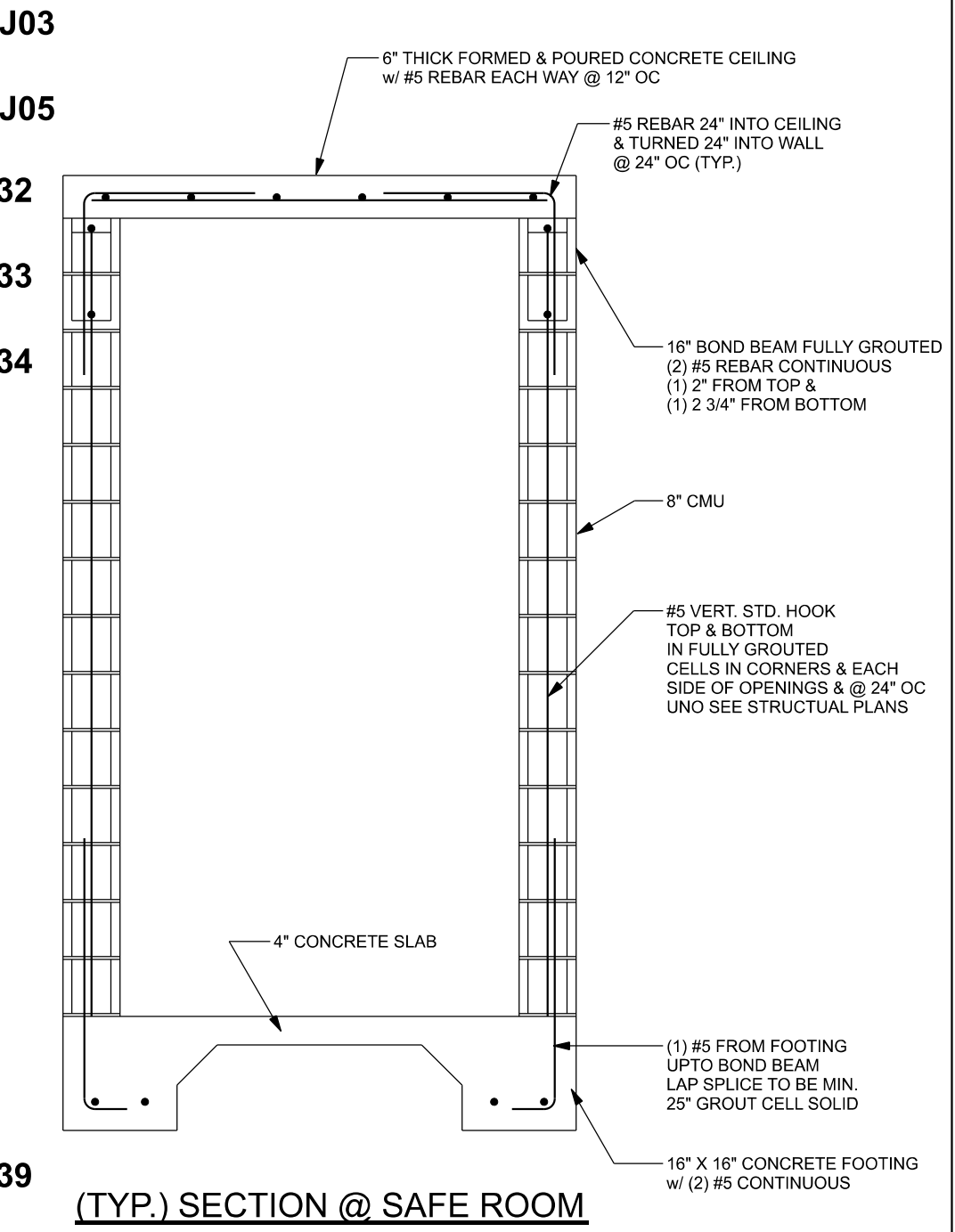
**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



**ACTUAL vs REQUIRED SHEARWALL**

	TRANSVERSE	LONGITUDINAL
ACTUAL	28677 LBF	36871 LBF
REQUIRED	23212 LBF	16067 LBF



Amira Custom Homes  
Nicewonger Res.  
PROJECT ADDRESS: 3246S-16-04015-02  
Columbia County, FL

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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JOB NUMBER:  
200755  
**S-3**  
OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2380539