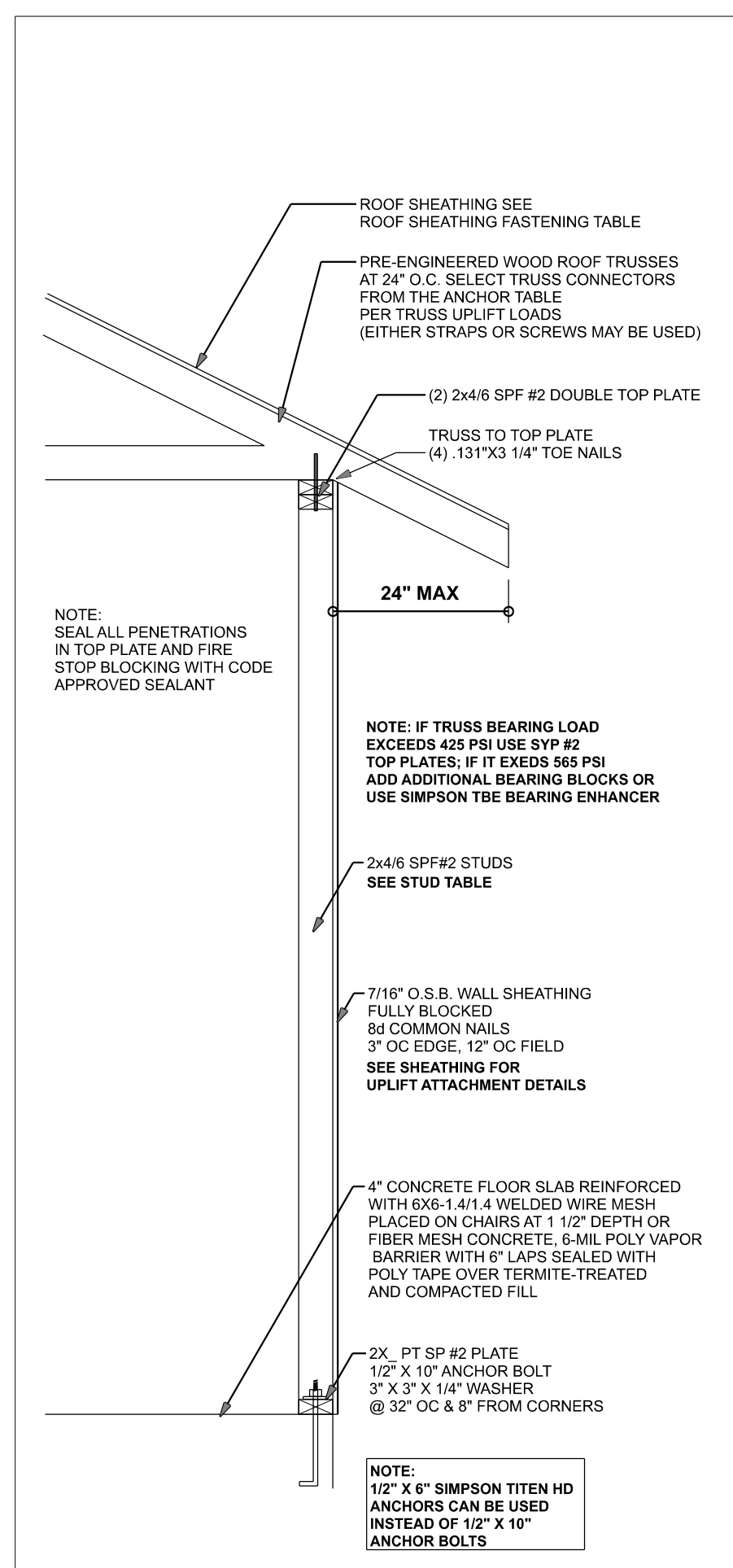
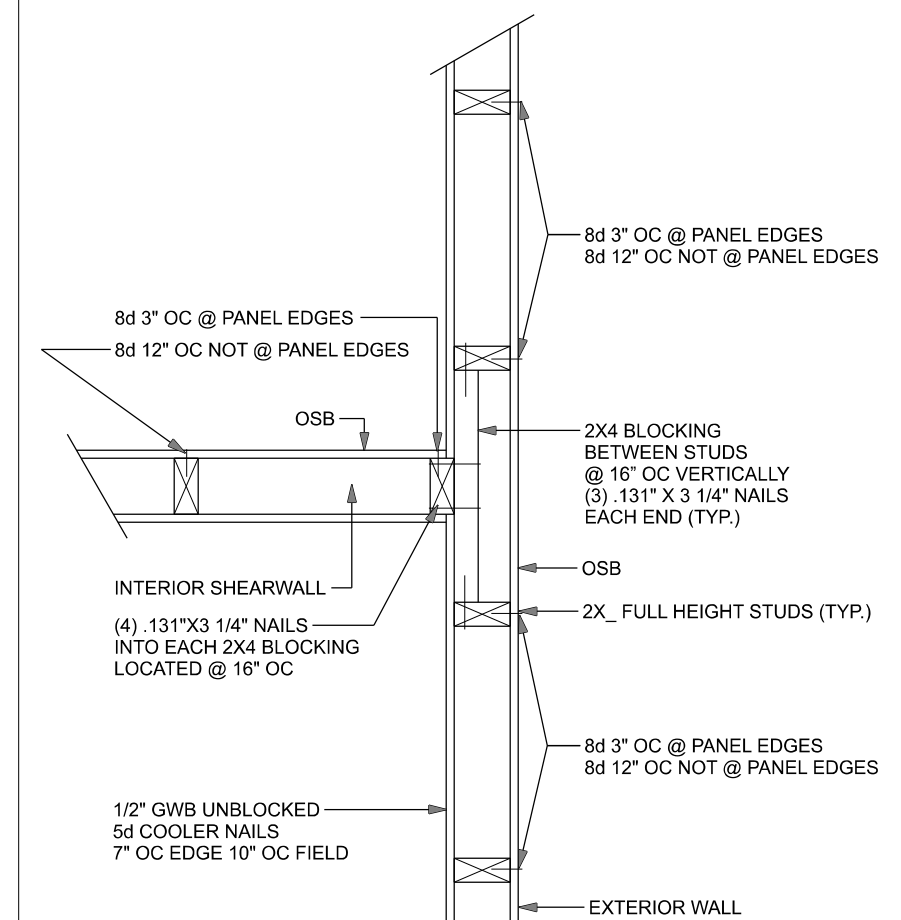
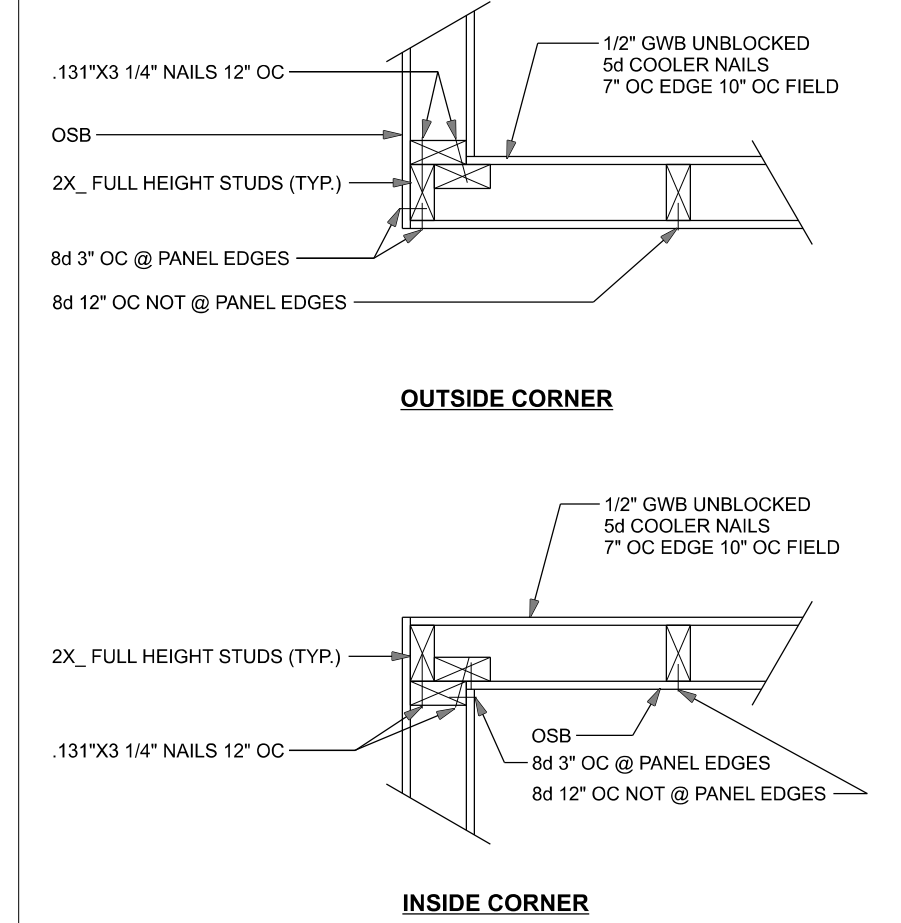




ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



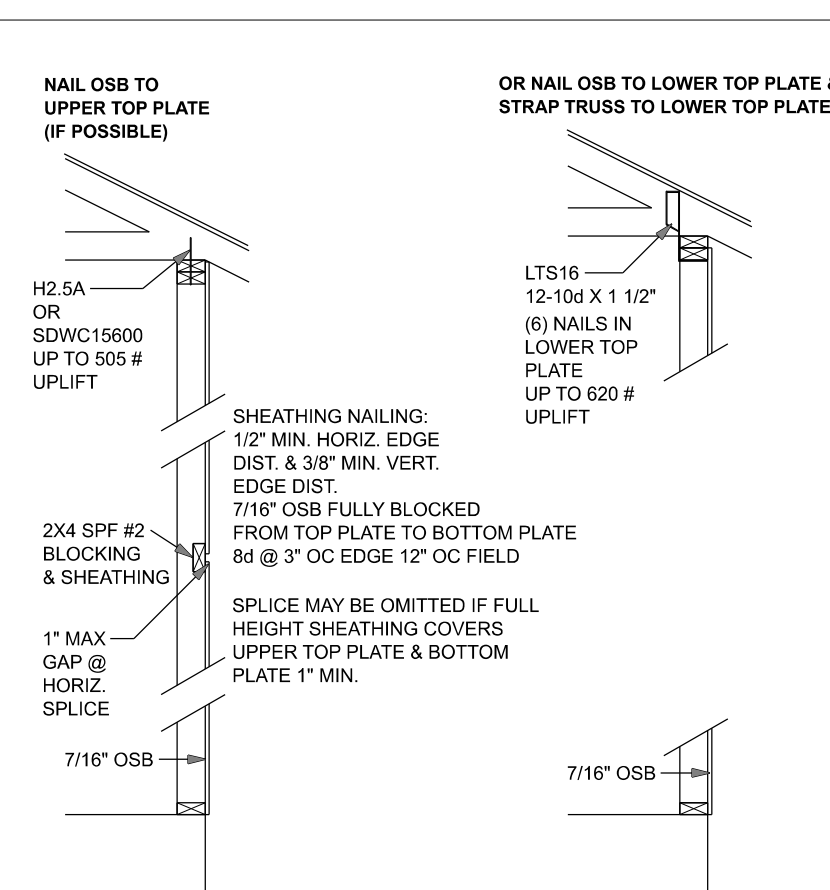
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



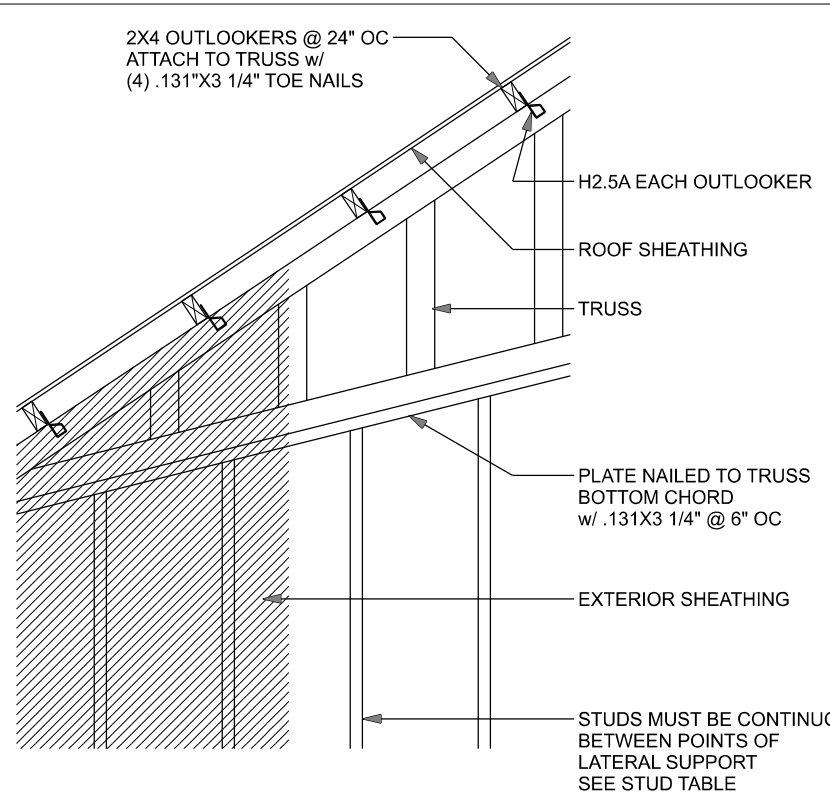
(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" OC	12" OC
120 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" OC	6" OC
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" OC	6" OC
130 mph Exp. C	15/32"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" OC	6" OC
130 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" OC	6" OC
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" OC	6" OC
140 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" OC	6" OC
140 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" OC	6" OC
150 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" OC	6" OC
150 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.12" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	4" OC	4" OC

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

Option/Uplift	Top Connection	Bottom Connection
#1	< 510 Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2	< 895 Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

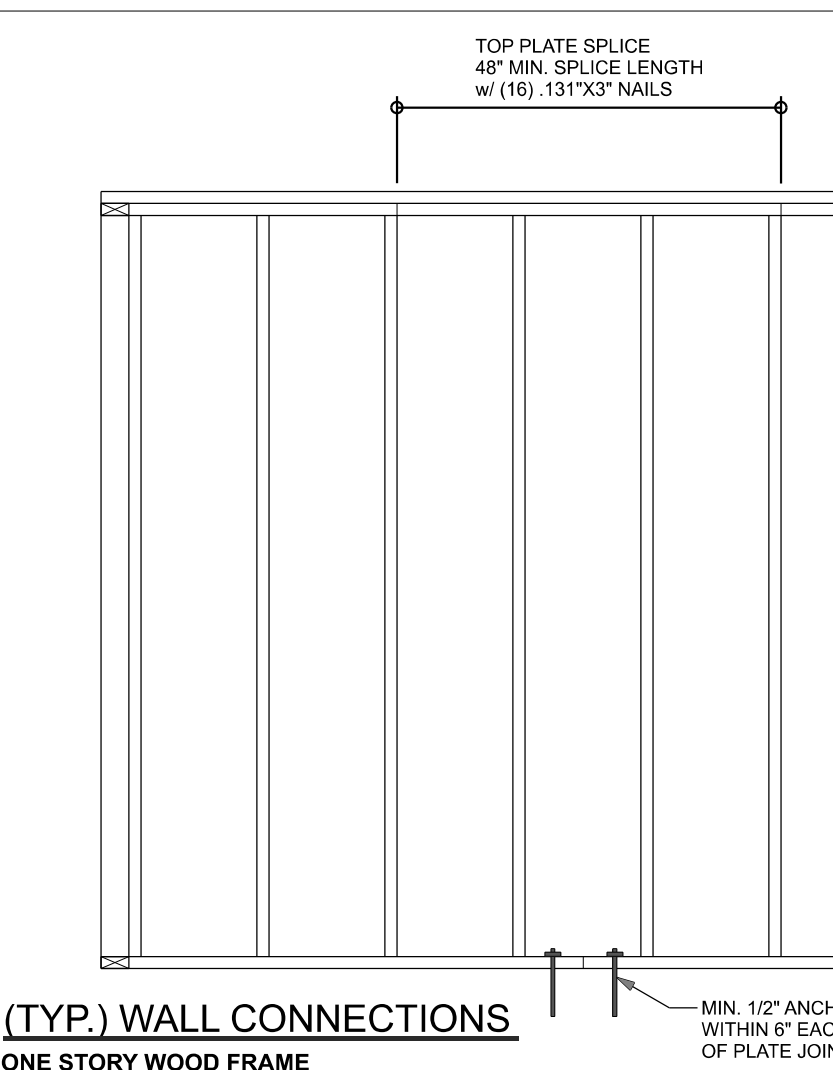
Option/Uplift	Top Connection	Bottom Connection
#3	< 1235 LSTA24, 14-10d x 1 1/2" wrap over plate	LSTA24, 14-10d x 1 1/2" wrap under plate 12" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4	< 1455 MSTA24, 18-148x1 1/2" header to jacks	DTT22
#5	< 1800 (2) MSTA24, 18-148x1 1/2" header to jacks	DTT22
#6	< 2910 (2) MSTA24, 18-148x1 1/2" header to jacks	HTT4

Option/Uplift	Top Connection	Bottom Connection
#7	< 1056 2" x 2" x 1/8" washer & nut @ top plate	Simpson THD37634RC 3/8" full height A307 all threaded rod
#8	< 1575 2" x 2" x 1/8" washer & nut @ top plate	Simpson THD37634RC 1/2" full height A307 all threaded rod
#9	< 2250 2" x 2" x 1/8" washer & nut @ top plate	Simpson SET-XP epoxy (or equal) 3/8" full height A307 all threaded rod

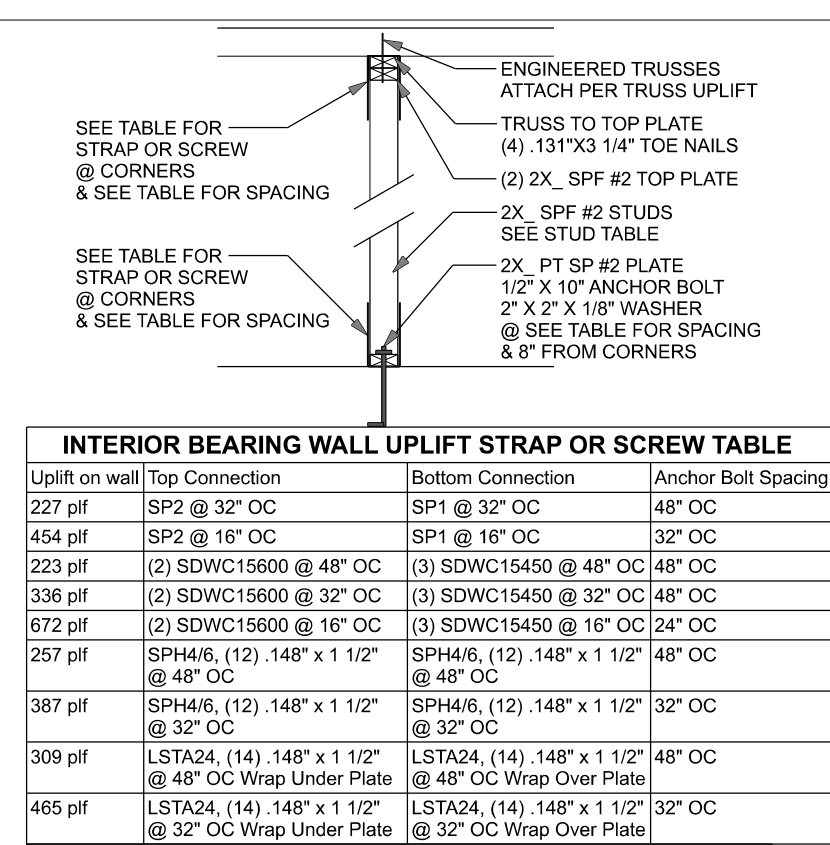
DESIGN WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

BASED ON WFCM TABLE A-3.2.8B
FOR OTHER WALL HEIGHTS OR SILL SPAN SHALL BE DIVIDED BY (H/10)

TYPICAL HEADER STRAPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

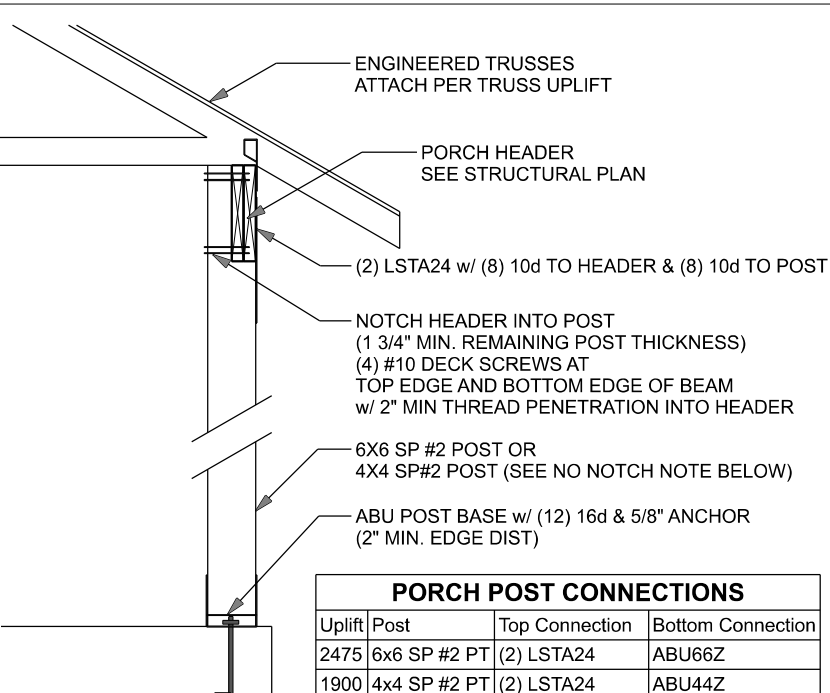


(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



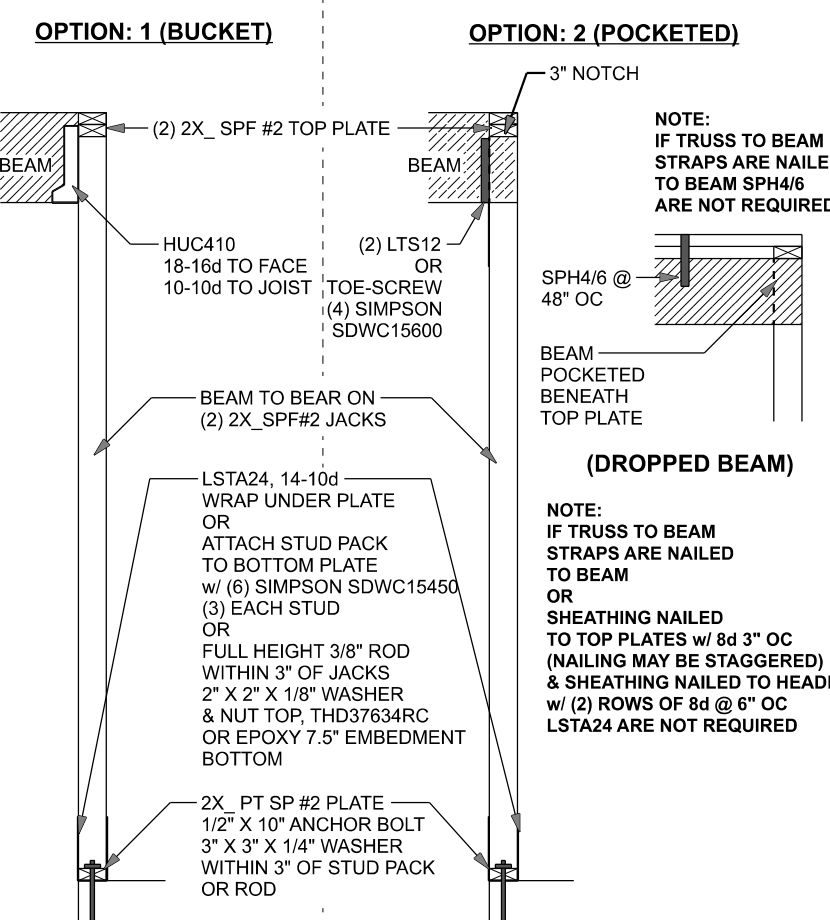
INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

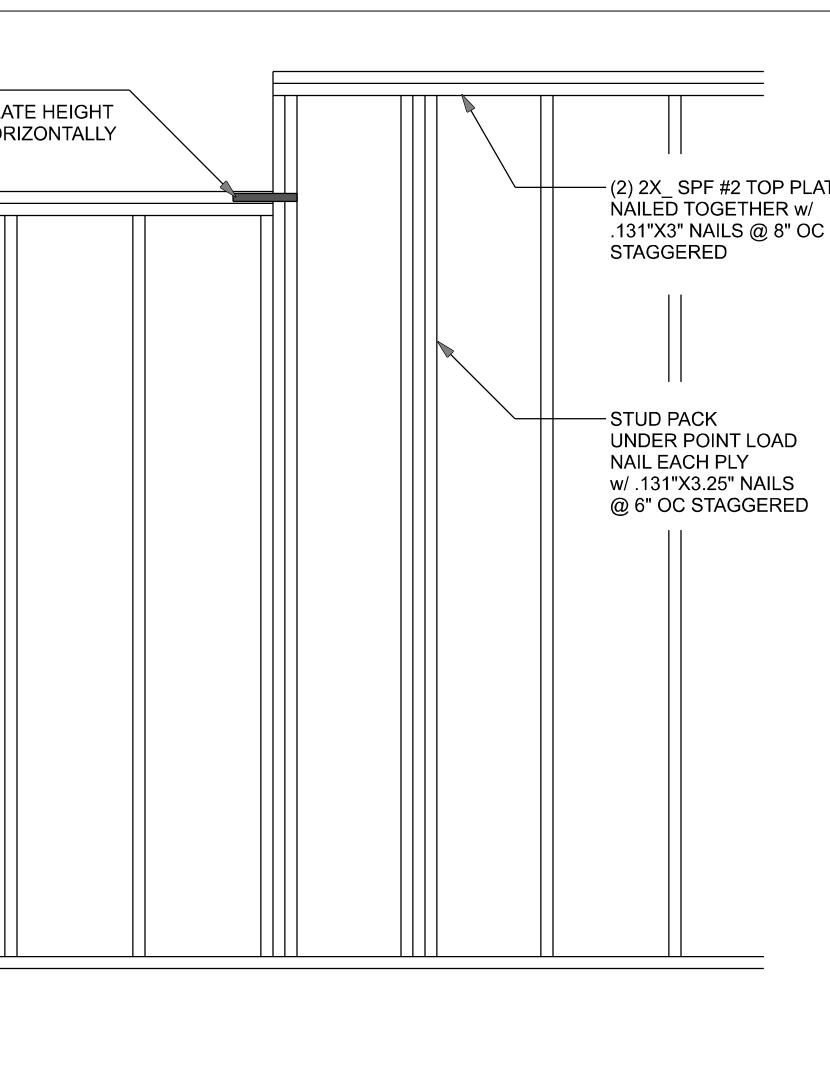


PORCH POST CONNECTIONS

(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



Uplift	Top Connection	Bottom Connection
up to 905 lb	(2) SDWC15600	(3) SDWC15450 w anchor bolt within 6"
up to 1215 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within 6"
up to 2250 lb	(2) HTS16	3/8" full height rod within 6" 2"x2"x1/8" washer & nut @ top plate Simpson SET-XP epoxy (or equal) w 7.5" min. embedment @ foundation (install per mfg. specs)
up to 2430 lb	(2) HTS16	HTT4

Uplift	Top Connection	Bottom Connection
up to 1215 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within 6"
up to 1810 lb	(4) SDWC15600 (2) Each Ply	HTT4
up to 2250 lb	(2) HTS16	3/8" full height rod within 6" 2"x2"x1/8" washer & nut @ top plate Simpson SET-XP epoxy (or equal) w 7.5" min. embedment @ foundation (install per mfg. specs)
up to 2430 lb	(2) HTS16	HTT4
up to 3645 lb	(3) HTS16	HTT4

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
400	290	H3	4-131"x1 1/2"	4-131"x1 1/2"
625	540	H2.5A	5-131"x1 1/2"	5-131"x1 1/2"
1040	1015	H10A	9-148"x1 1/2"	9-148"x1 1/2"
645	515	LTS12-20	6-148"x1 1/2"	6-148"x1 1/2"
990	850	MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415	1215	HTS16-30	8-148"x1 1/2"	8-148"x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-148"x1 1/2"	8-148"x1 1/2"
1640	1460	MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030	1030	CS20	7-148"x1 1/2"	7-148"x1 1/2"
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4-148"x3"	4-148"x3"
1010	905	SP2	6-148"x3"	6-148"x3"
1280	1100	SPH46	12-148"x1 1/2"	wrap under or over plate
771	771	LSTA24	10-148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148"x1 1/2"	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
1900	ABU44Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
2475	ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
1900	ABU44Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
2475	ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

BEARING SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCE, IS BASED ON REACTION UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCE REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCE. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TRUSS LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED WITH THE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W/ 4" x W/ 4" FB + 8KSI, WELED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.15 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM 1116, SUPPLIER TO PROVIDE ASTM 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

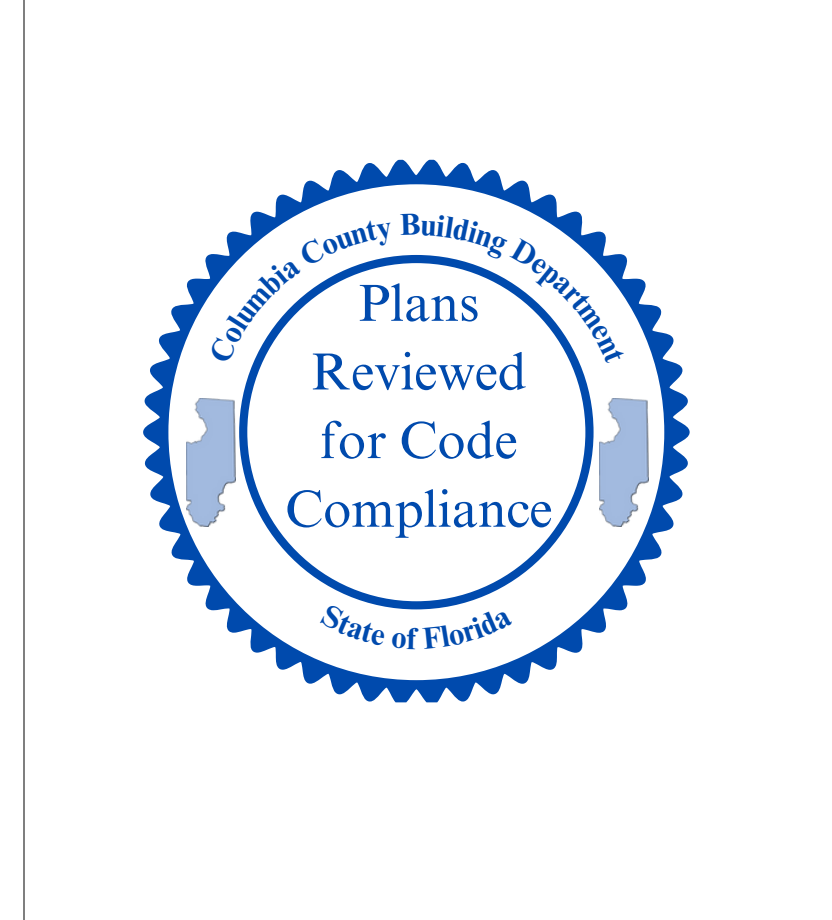
CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS NOT TO EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/ W/ OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR AS BARS). UNLO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-05, UNLO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE REQUIREMENTS FOR THE SITED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.



BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

Bryan Zecher Homes, Inc.

DONAL PARNELL
RESIDENCE

PROJECT ADDRESS:
1880 NW JAKE GLN
LAKE CITY, FL 32805
COLUMBIA COUNTY

FL PE 53915
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DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
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BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
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TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasdt) -27.8(Vasdt)	+25.6(Vasdt) -34.2(Vasdt)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57.1(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.6(Vasdt) -25.5(Vasdt)
16x7 GARAGE DOOR	+21.7(Vasdt) -24.1(Vasdt)

JOB NUMBER:
260555
S-1
OF 2 SHEETS

