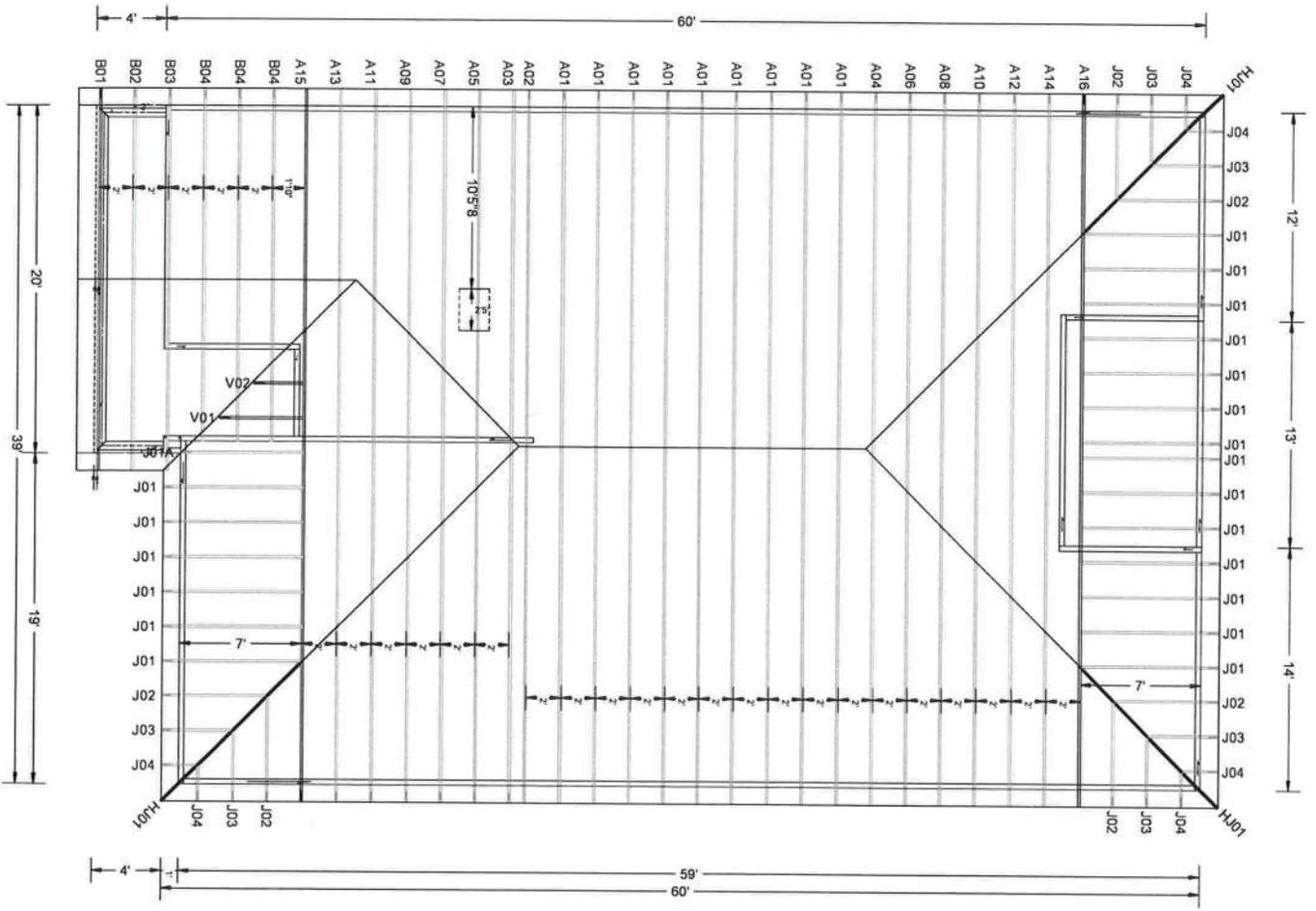


73



W.B. Howland Truss Co.
 610 11th St. SW
 Live Oak, FL 32064
 (386) 362-1235
 (386) 362-7124 (Fax)
 howlandtruss@gmail.com

ROOF PITCH: 6/12
 OVERHANG: 12"
 CEILING: FLAT
 EXT. WALLS: FRAME
 LOADING: 40 TL
 WIND LOAD: 130
 CATEGORY: II
 EXPOSURE: C
 DEFLECTION: 360/240
 DATE: 8/6/21



Reviewed for Code Compliance
 Universal Engineering Sciences

Chris McCall
 EXAMINER - LICENSE NO. 042767

<p>JOB #:</p> <p>21-5888</p>	<p>Job Name: Reserve at Jewel Lake 42 Customer: Century Complete Designer: Chris McCall ADDRESS: SALESMAN: Fill in later : <Not Found></p>	<p>JOB NO: 21-5888</p> <p>PAGE NO: 1 OF 1</p>
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This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.



Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Reviewed for Code Compliance
Universal Engineering Sciences

John P. Lee PE 2707
EXAMINER - LICENSE NO.

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-5888
Job Description: Reserve at Jewel Lake 42 - Radford B - GR	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res	IntelliVIEW Version: 21.01.01A JRef #: 1X7V2150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 28 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	223.21.1145.08801	A01	2	223.21.1145.08614	A02
3	223.21.1145.08910	A03	4	223.21.1145.09426	A04
5	223.21.1145.08769	A05	6	223.21.1145.08615	A06
7	223.21.1145.08613	A07	8	223.21.1145.08941	A08
9	223.21.1145.09067	A09	10	223.21.1145.09302	A10
11	223.21.1145.09316	A11	12	223.21.1145.09129	A12
13	223.21.1145.09114	A13	14	223.21.1145.09347	A14
15	223.21.1145.09066	A15	16	223.21.1145.08786	A16
17	223.21.1145.08176	B01	18	223.21.1145.09177	B02
19	223.21.1145.09505	B03	20	223.21.1145.08192	B04
21	223.21.1145.09473	HJ01	22	223.21.1145.09191	J01
23	223.21.1145.08238	J01A	24	223.21.1145.08895	J02
25	223.21.1145.09239	J03	26	223.21.1145.09270	J04
27	223.21.1145.08911	V01	28	223.21.1145.09380	V02
29	A14015ENC160118		30	BRCLBSUB0119	
31	CNNAILSP1014		32	GBLLETIN0118	
33	VAL180160118		34	VALTN160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

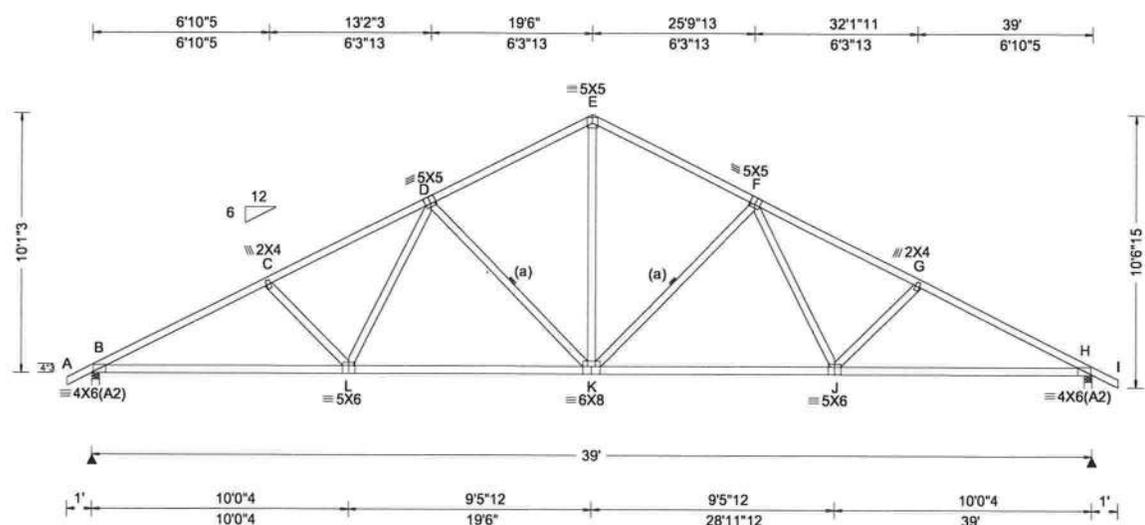
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 387889 / COMN Ply: 1 Job Number: 21-5888 Cust: R215 JRef: 1X7V2150005 T9
 FROM: CDM Qty: 9 Reserve at Jewel Lake 42 - Radford B - GR DrwNo: 223.21.1145.08801
 Truss Label: A01 / YK 08/11/2021



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.90 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.190 K 999 360 VERT(CL): 0.357 K 999 240 HORZ(LL): 0.064 J - - HORZ(TL): 0.120 J - - Creep Factor: 2.0 Max TC CSI: 0.569 Max BC CSI: 0.484 Max Web CSI: 0.549 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																																																																																										
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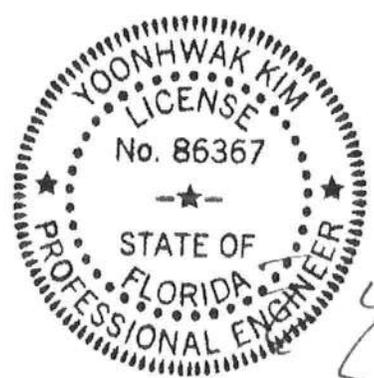
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

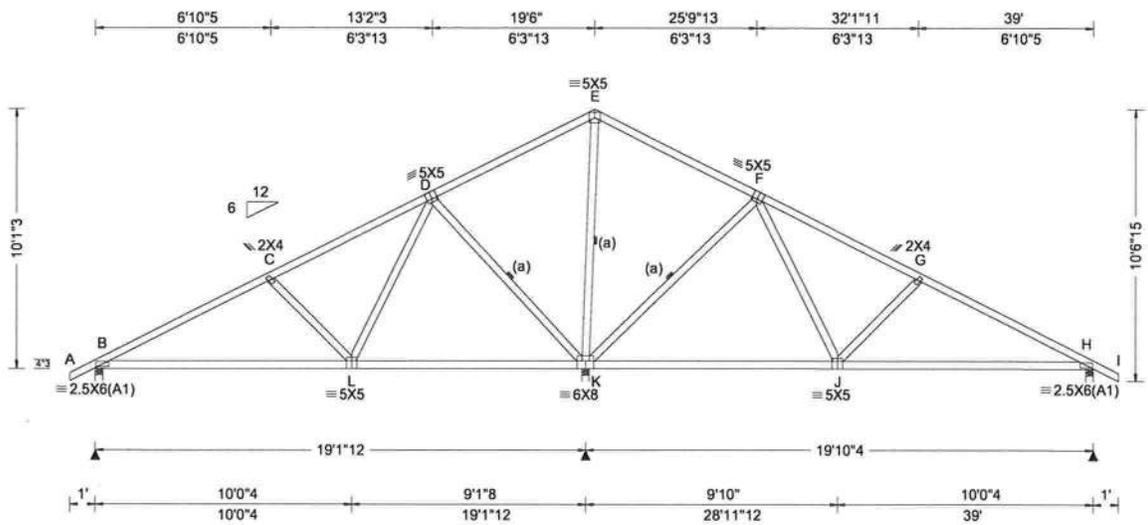
Additional Notes
 The overall height of this truss excluding overhang is 10'-1-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 08/11/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.90 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.020 J 999 360 VERT(CL): 0.045 L 999 240 HORZ(LL): 0.011 L - - HORZ(TL): 0.026 L - - Creep Factor: 2.0 Max TC CSI: 0.636 Max BC CSI: 0.881 Max Web CSI: 0.433 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																	
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H	717	-	-	499	1106	-																															

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;		Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
		B - C	156	-825	E - F	655	-6
		C - D	130	-536	F - G	149	-631
		D - E	690	-17	G - H	177	-919

Bracing (a) Continuous lateral restraint equally spaced on member.		Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
		B - L	671	-240	J - H	754	-53

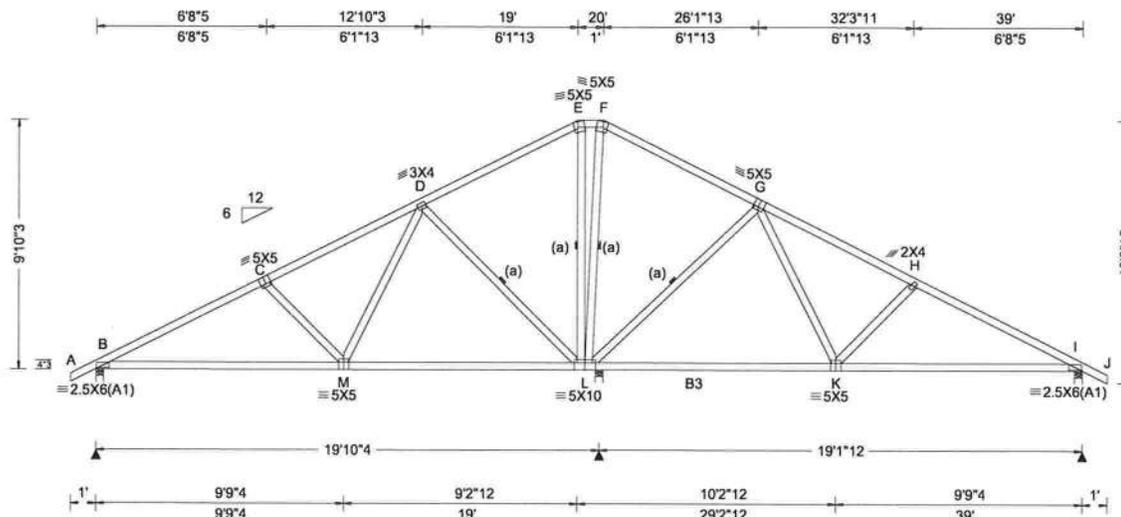
Loading Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.		Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.					
		C - L	209	-397	K - F	298	-851
		L - D	643	-66	F - J	668	-60
		D - K	297	-820	J - G	209	-393
		K - E	153	-859			



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.065 D 999 360 VERT(CL): 0.133 D 999 240 HORZ(LL): 0.029 K - - HORZ(TL): 0.060 K - - Creep Factor: 2.0 Max TC CSI: 0.481 Max BC CSI: 0.957 Max Web CSI: 0.330 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1009 /- /- /634 /35 /281 L 1352 /- /- /752 /17 /- I 984 /- /- /655 /49 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 L Brg Width = 3.5 Min Req = 1.5 I Brg Width = 3.5 Min Req = 1.5 Bearings B, L, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 385 -1555 F - G 255 -409 C - D 360 -1276 G - H 334 -1187 D - E 260 -406 H - I 361 -1471
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

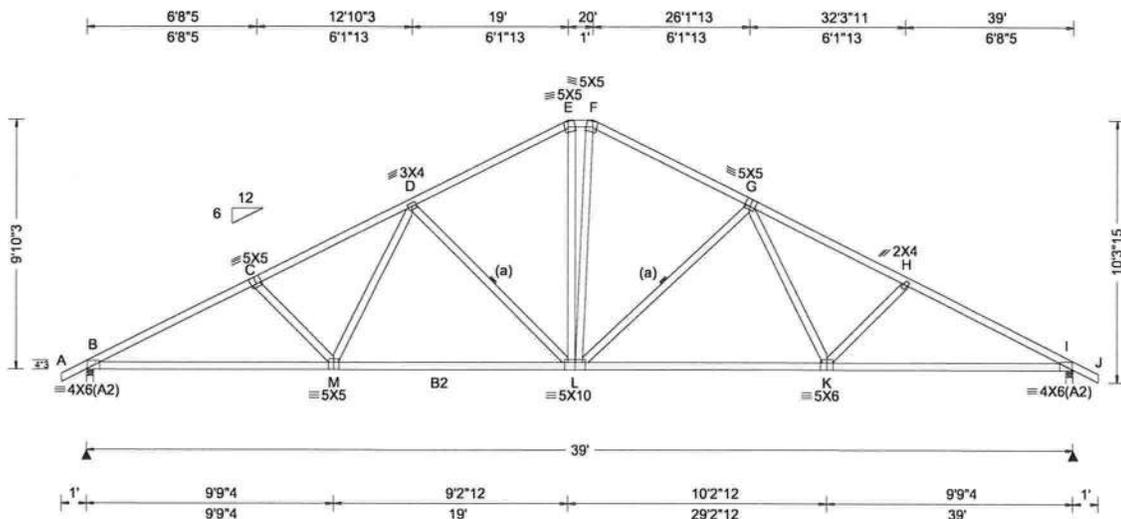
Additional Notes
The overall height of this truss excluding overhang is 9'-10-3/8."



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

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Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0"

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: h to 2h
 C&C Dist a: 3.90 ft
 Loc. from endwall: not in 9.00 ft
 GCpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg, Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT: 20(0)/10(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/def L/#
 VERT(LL): 0.152 L 999 360
 VERT(CL): 0.310 L 999 240
 HORZ(LL): 0.057 K - -
 HORZ(TL): 0.116 K - -
 Creep Factor: 2.0
 Max TC CSI: 0.531
 Max BC CSI: 0.955
 Max Web CSI: 0.333

VIEW Ver: 21.01.01A.0521.20

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1673	-	-	/1003	/33	/281
I	1673	-	-	/1003	/33	-

Wind reactions based on MWFRS
 B Brg Width = 3.5 Min Req = 1.5
 I Brg Width = 3.5 Min Req = 1.5
 Bearings B & I are a rigid surface.
 Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	687 -2972	F - G	574 -1879
C - D	663 -2699	G - H	662 -2702
D - E	580 -1907	H - I	687 -2976
E - F	567 -1641		

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 9-10-3.



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Maximum Bot Chord Forces Per Ply (lbs)

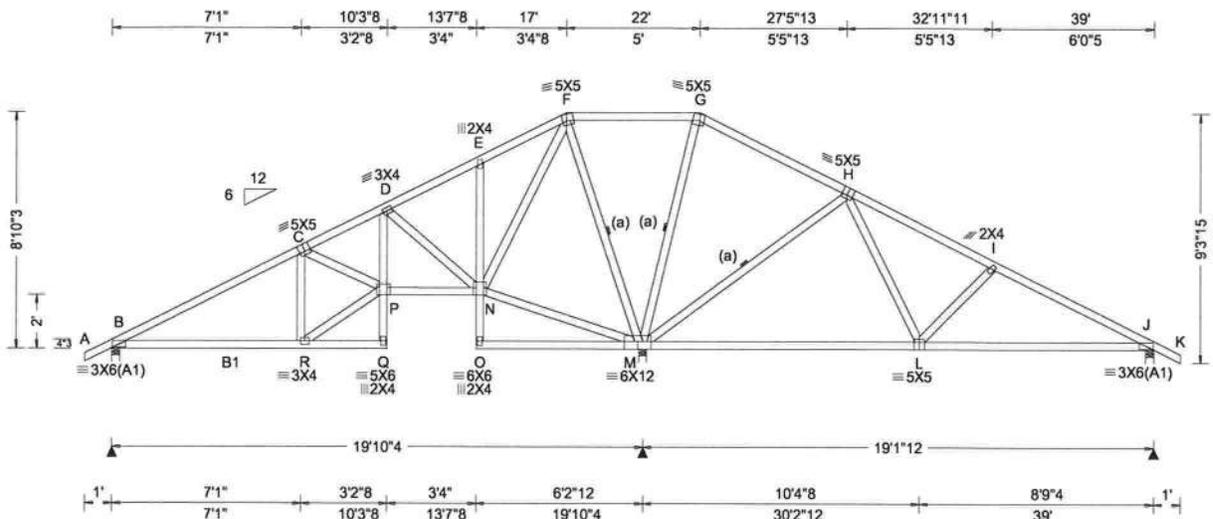
Chords	Tens.Comp.	Chords	Tens. Comp.
B - M	2584 -523	L - K	2125 -359
M - L	2128 -373	K - I	2588 -510

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
M - D	545 -52	L - F	587 -183
D - L	267 -710	L - G	266 -720
E - L	603 -189	G - K	560 -50

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf Soffit: 2.00 MWFRS Parallel Dist: h to 2h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.018 P 999 360 VERT(CL): 0.047 P 999 240 HORZ(LL): 0.011 L - - HORZ(TL): 0.025 M - - Creep Factor: 2.0 Max TC CSI: 0.626 Max BC CSI: 0.437 Max Web CSI: 0.392 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																			
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>618</td> <td>-</td> <td>-</td> <td>/376</td> <td>/30</td> <td>/254</td> </tr> <tr> <td>M</td> <td>2229</td> <td>-</td> <td>-</td> <td>/1270</td> <td>/35</td> <td>-</td> </tr> <tr> <td>J</td> <td>653</td> <td>-</td> <td>-</td> <td>/468</td> <td>/117</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 M Brg Width = 3.5 Min Req = 1.8 J Brg Width = 3.5 Min Req = 1.5 Bearings B, M, & J are a rigid surface. Members not listed have forces less than 375#</p>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	618	-	-	/376	/30	/254	M	2229	-	-	/1270	/35	-	J	653	-
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3;		Maximum Top Chord Forces Per Ply (lbs)			
		Chords Tens.Comp.		Chords Tens. Comp.	
		B - C	167 -711	G - H	709 -24
		C - D	122 -545	H - I	243 -571
		F - G	730 -3	I - J	245 -819

Bracing (a) Continuous lateral restraint equally spaced on member.		Maximum Bot Chord Forces Per Ply (lbs)			
		Chords Tens.Comp.		Chords Tens. Comp.	
		B - R	560 -175	L - J	673 -157
		P - N	434 -233		

Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.		Maximum Web Forces Per Ply (lbs)			
		Webs Tens.Comp.		Webs Tens. Comp.	
		R - P	689 -211	F - M	273 -900
		P - D	509 -78	M - G	211 -605
		D - N	131 -609	M - H	272 -736
		N - F	658 -163	H - L	573 -39
		N - M	287 -481		

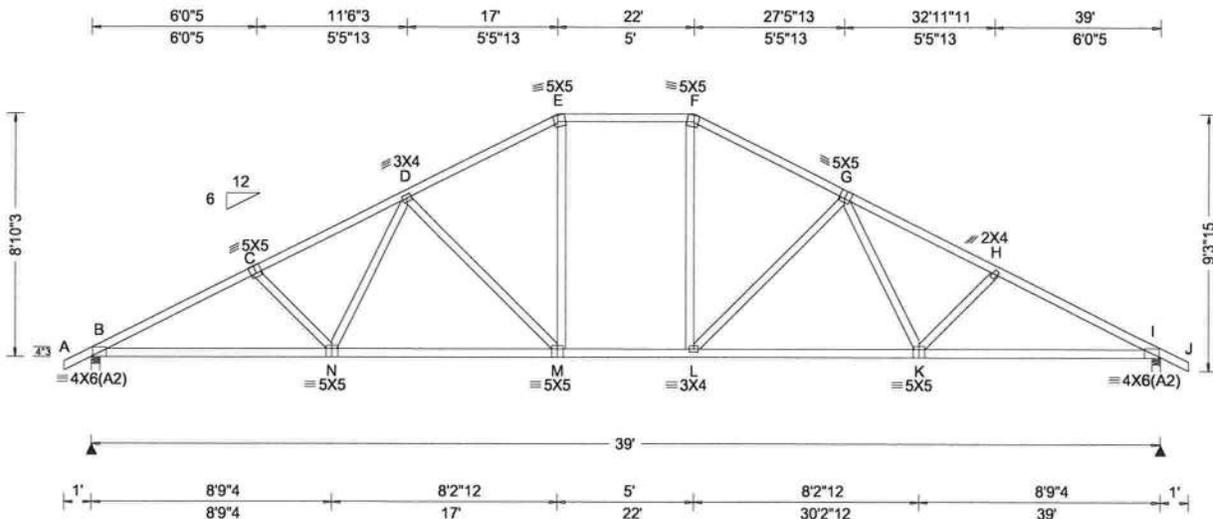
Additional Notes
 The overall height of this truss excluding overhang is 8-10-3.



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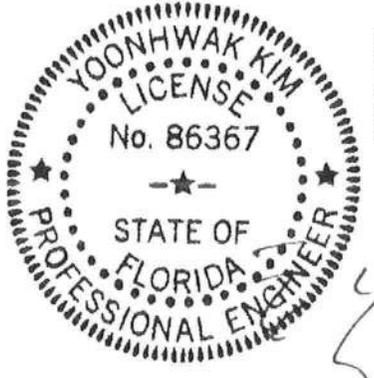
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.223 L 999 360 VERT(CL): 0.623 L 745 240 HORZ(LL): 0.083 E - - HORZ(TL): 0.233 E - - Creep Factor: 2.0 Max TC CSI: 0.786 Max BC CSI: 0.905 Max Web CSI: 0.876 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1673 /- /- /1002 /39 /254 I 1673 /- /- /1002 /39 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 2.0 I Brg Width = 3.5 Min Req = 2.0 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 789 -3005 F - G 708 -2086 C - D 771 -2765 G - H 772 -2766 D - E 708 -2087 H - I 790 -3007 E - F 675 -1793
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
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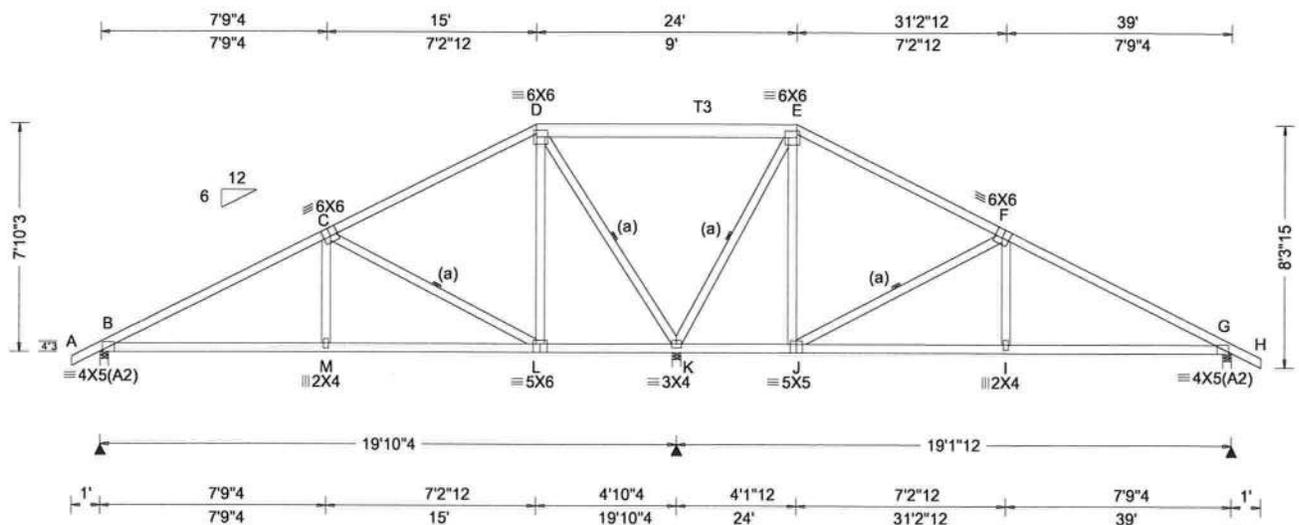
Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - N	2618 -628	L - K	2224 -488
N - M	2225 -502	K - I	2619 -616
M - L	1793 -330		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
N - D	486 -38	F - L	576 -99
D - M	248 -697	L - G	247 -686
E - M	577 -101	G - K	486 -40

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D - K	401	-1154	J - F	276	-784																																												

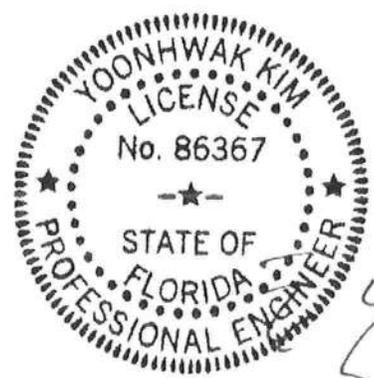
Lumber
 Top chord: 2x4 SP #2; T3 2x6 SP 2400F-2.0E;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 7'-10-3/8."



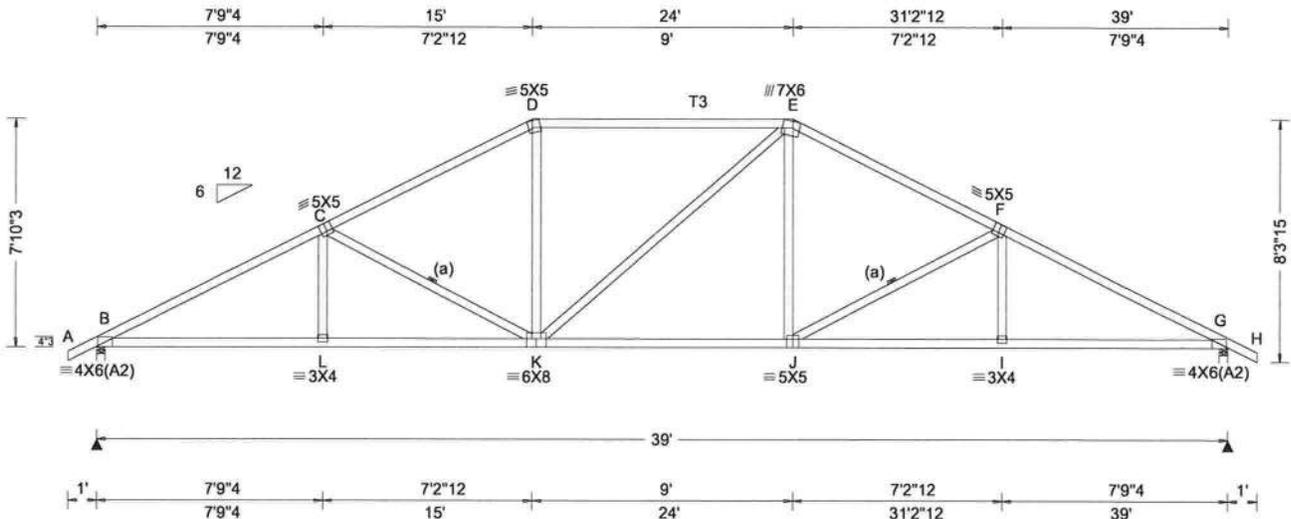
FL REG# 278, Yoonhwak Kim, FL PE #86367
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.159 J 999 360 VERT(CL): 0.325 J 999 240 HORZ(LL): 0.073 I - - HORZ(TL): 0.149 I - - Creep Factor: 2.0 Max TC CSI: 0.886 Max BC CSI: 0.855 Max Web CSI: 0.399 VIEW Ver: 21.01.01A.0521.20

▲ Maximum Reactions (lbs)					
Gravity			Non-Gravity		
Loc	R+	/R-	/Rh	/Rw	/U /RL
B	1669	-	-	/998	/47 /227
G	1669	-	-	/998	/47 -
Wind reactions based on MWFRS					
B	Brg Width = 3.5		Min Req = 2.0		
G	Brg Width = 3.5		Min Req = 2.0		
Bearings B & G are a rigid surface. Members not listed have forces less than 375#					
Maximum Top Chord Forces Per Ply (lbs)					
Chords		Tens.Comp.		Chords Tens. Comp.	
B - C	873	-2948	E - F	823	-2311
C - D	821	-2303	F - G	874	-2948
D - E	811	-1983			

Lumber
Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
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Additional Notes
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FL REG# 278, Yoonhwak Kim, FL PE #86367
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Maximum Bot Chord Forces Per Ply (lbs)					
Chords		Tens.Comp.		Chords Tens. Comp.	
B - L	2547	-686	J - I	2544	-675
L - K	2544	-688	I - G	2547	-673
K - J	1980	-491			
Maximum Web Forces Per Ply (lbs)					
Webs		Tens.Comp.		Webs Tens. Comp.	
C - K	210	-649	E - J	566	-20
D - K	566	-35	J - F	210	-645

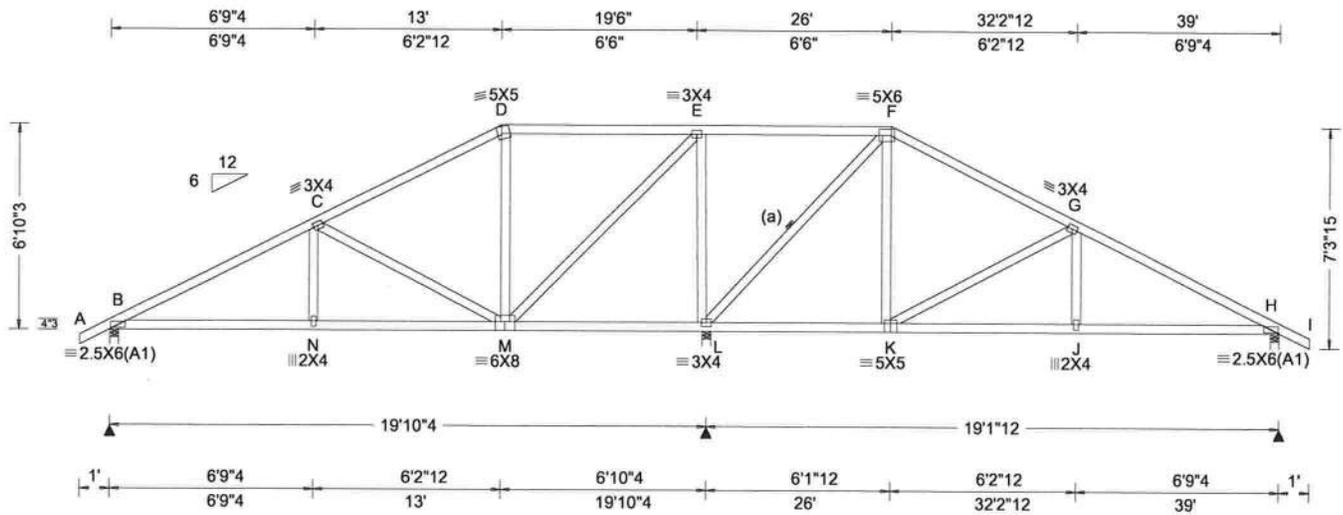
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.024 N 999 360 VERT(CL): 0.054 N 999 240 HORZ(LL): 0.011 J - - HORZ(TL): 0.023 J - - Creep Factor: 2.0 Max TC CSI: 0.699 Max BC CSI: 0.430 Max Web CSI: 0.976 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																																																																																																	
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Lumber
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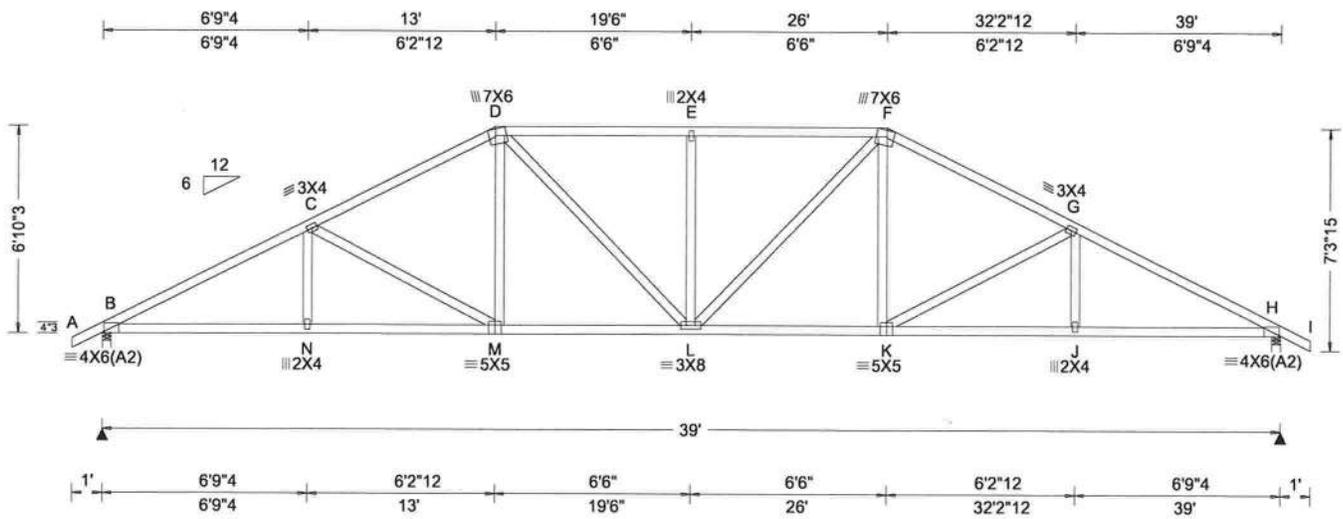
Additional Notes
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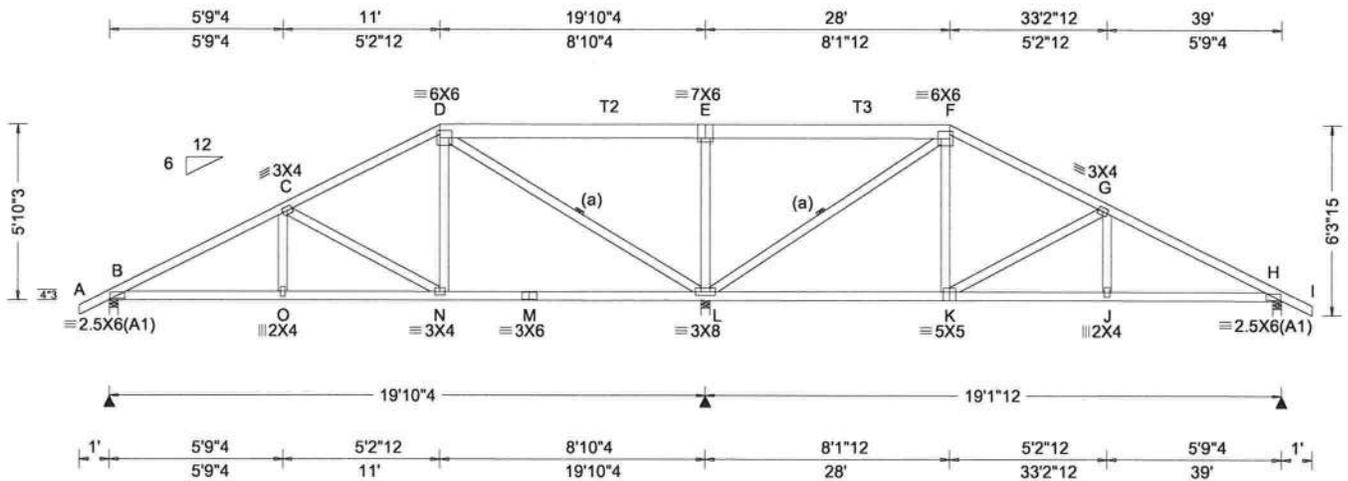
Additional Notes
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.025 O 999 360	B	726	-	-	/458	/36	/173
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.057 O 999 240	L	2039	-	-	/1061	/131	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 J - -	H	692	-	-	/466	/57	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.031 J - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B	Brg Width = 3.5		Min Req = 1.5			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.365	L	Brg Width = 3.5		Min Req = 2.0			
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.619	H	Brg Width = 3.5		Min Req = 1.5			
Spacing: 24.0"	MWFRS Parallel Dist: h to 2h	FT/RT: 20(0)/10(0)	Max Web CSI: 0.607	Bearings B, L, & H are a rigid surface.						
	C&C Dist a: 3.90 ft	Plate Type(s):	VIEW Ver: 21.01.01A.0521.20	Members not listed have forces less than 375#						
	Loc. from endwall: not in 9.00 ft	WAVE		Maximum Top Chord Forces Per Ply (lbs)						
	GCpi: 0.18			Chords	Tens.Comp.	Chords	Tens. Comp.			
	Wind Duration: 1.60			B - C	295	-1021	E - F	662	-66	

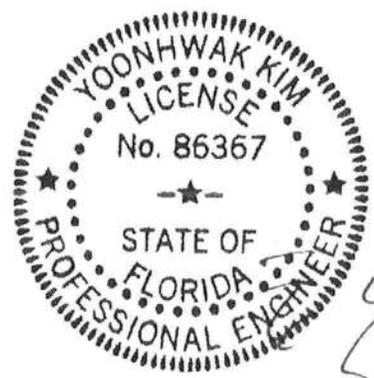
Lumber
 Top chord: 2x4 SP #2; T2,T3 2x6 SP 2400f-2.0E;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 5-10-3.



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Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.	Chords	Tens. Comp.		
B - O	854	-195	M - L	403	-72
O - N	852	-197	K - J	790	-159
N - M	403	-72	J - H	793	-158

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.	Webs	Tens. Comp.		
C - N	217	-523	L - F	336	-1062
D - N	473	-33	F - K	449	-40
D - L	363	-1128	K - G	215	-533
E - L	520	-661			

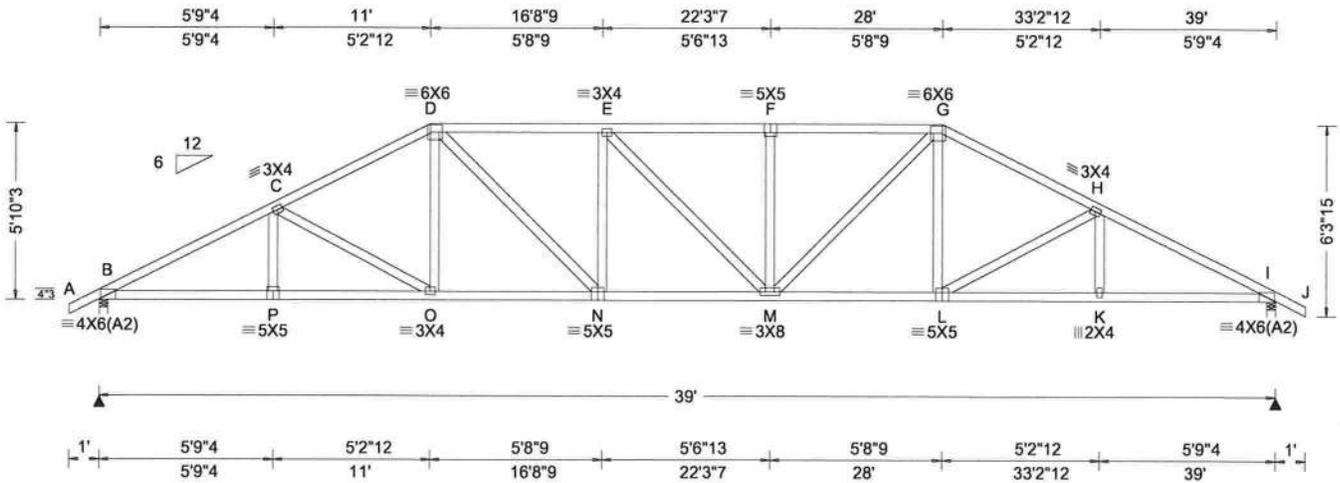
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.208 F 999 360 VERT(CL): 0.424 F 999 240 HORZ(LL): 0.078 K - - HORZ(TL): 0.159 K - - Max TC CSI: 0.495 Max BC CSI: 0.744 Max Web CSI: 0.396 Creep Factor: 2.0 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1673</td> <td>-</td> <td>-</td> <td>/979</td> <td>/303</td> <td>/174</td> </tr> <tr> <td>I</td> <td>1673</td> <td>-</td> <td>-</td> <td>/979</td> <td>/303</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 2.0 I Brg Width = 3.5 Min Req = 2.0 Bearings B & I are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1098 -3029</td> <td>F - G</td> <td>1221 -2746</td> </tr> <tr> <td>C - D</td> <td>1072 -2611</td> <td>G - H</td> <td>1073 -2611</td> </tr> <tr> <td>D - E</td> <td>1215 -2733</td> <td>H - I</td> <td>1098 -3029</td> </tr> <tr> <td>E - F</td> <td>1221 -2746</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1673	-	-	/979	/303	/174	I	1673	-	-	/979	/303	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1098 -3029	F - G	1221 -2746	C - D	1072 -2611	G - H	1073 -2611	D - E	1215 -2733	H - I	1098 -3029	E - F	1221 -2746		
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 5-10-3.



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 08/11/2021

Maximum Bot Chord Forces Per Ply (lbs)

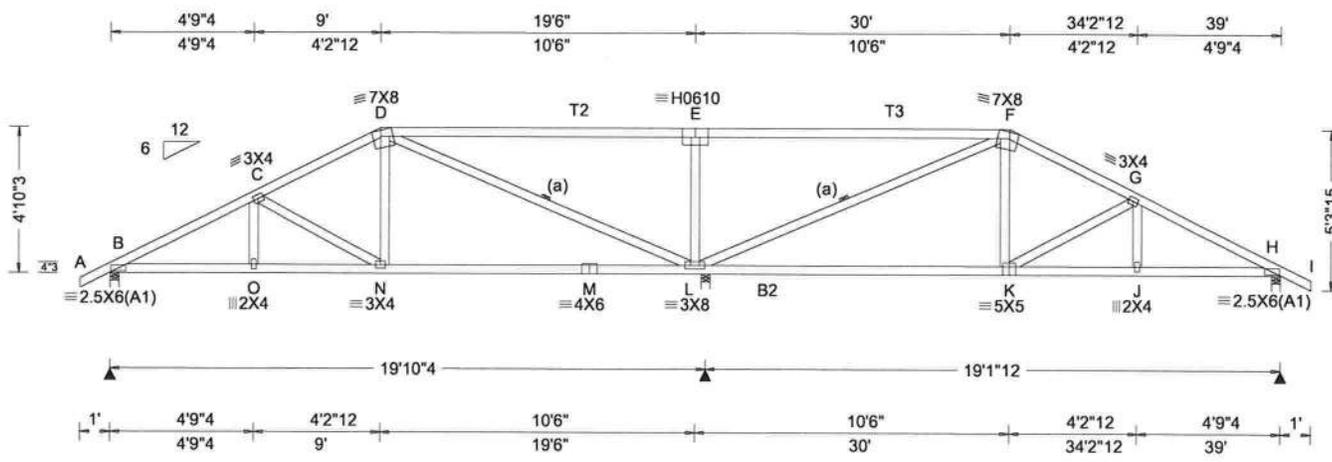
Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	2637 -908	M - L	2272 -767
P - O	2635 -910	L - K	2636 -896
O - N	2272 -781	K - I	2638 -894
N - M	2751 -994		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - O	149 -419	M - G	661 -343
D - O	384 -26	G - L	384 -27
D - N	651 -337	L - H	149 -420

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.052 N 999 360 VERT(CL): 0.099 N 999 240 HORZ(LL): 0.028 J - - HORZ(TL): 0.057 J - - Creep Factor: 2.0 Max TC CSI: 0.718 Max BC CSI: 0.978 Max Web CSI: 0.598 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 879 /- /- /556 /160 /147 L 1616 /- /- /832 /295 /- H 855 /- /- /554 /155 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 L Brg Width = 3.5 Min Req = 1.5 H Brg Width = 3.5 Min Req = 1.5 Bearings B, L, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 490 -1346 F - G 442 -1004 C - D 484 -1095 G - H 474 -1310 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - O 1145 -373 L - K 1760 -502 O - N 1145 -374 K - J 1107 -346 N - M 961 -302 J - H 1110 -345 M - L 961 -302 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. D - N 459 0 L - F 283 -841 D - L 329 -922 F - K 382 0 E - L 573 -727
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Lumber
Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
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Wind
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Wind loading based on both gable and hip roof types.

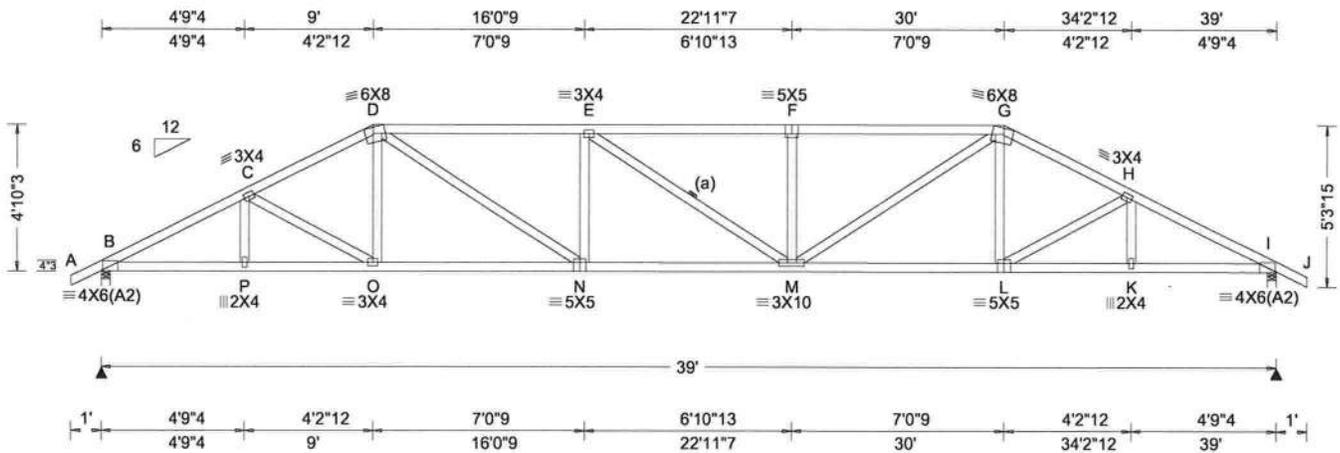
Additional Notes
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.90 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.261 F 999 360 VERT(CL): 0.532 F 872 240 HORZ(LL): 0.083 K - - HORZ(TL): 0.168 K - - Creep Factor: 2.0 Max TC CSI: 0.924 Max BC CSI: 0.847 Max Web CSI: 0.686 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL					
				B 1673 /- /- /964 /305 /147 I 1673 /- /- /964 /305 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 2.0 I Brg Width = 3.5 Min Req = 2.0 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1196 -3035 F - G 1529 -3337 C - D 1196 -2770 G - H 1196 -2769 D - E 1519 -3317 H - I 1196 -3036 E - F 1529 -3336					

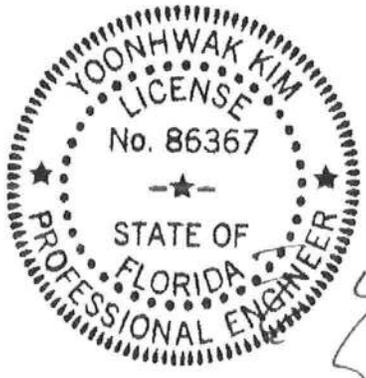
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Purlins
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Wind
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 Wind loading based on both gable and hip roof types.

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Maximum Bot Chord Forces Per Ply (lbs)

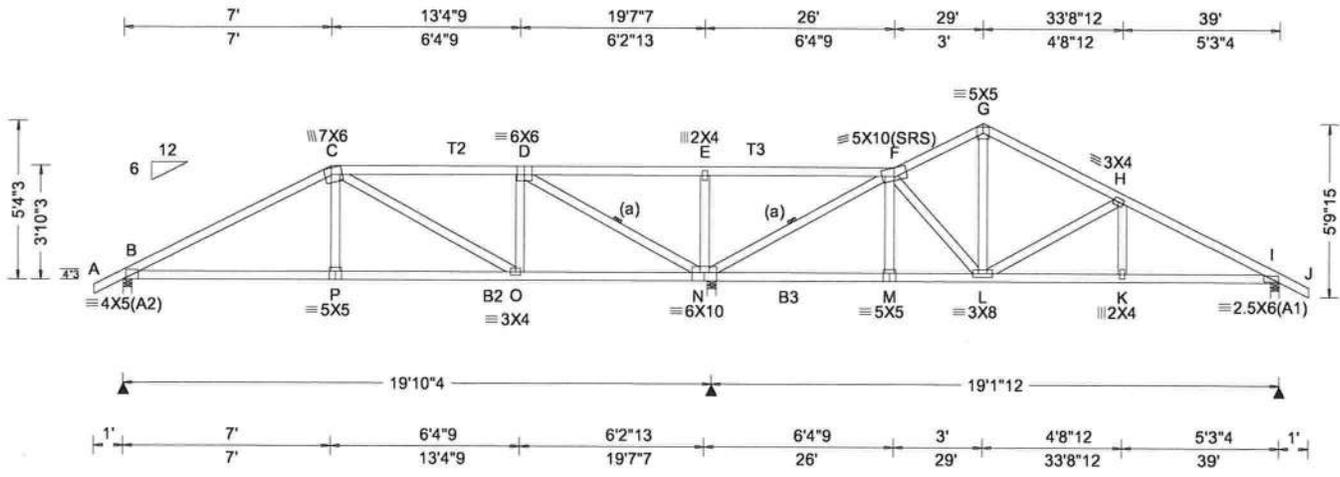
Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	2648 -1003	M - L	2438 -913
P - O	2647 -1004	L - K	2648 -992
O - N	2439 -927	K - I	2649 -990
N - M	3345 -1342		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - N	1051 -525	M - G	1069 -536
N - E	362 -440	F - M	345 -432

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				B 1604 /- /- /- /344 /- N 3329 /- /- /- /671 /- I 656 /- /- /- /114 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.9 N Brg Width = 3.5 Min Req = 2.8 I Brg Width = 3.5 Min Req = 1.5 Bearings B, N, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					

Lumber
 Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
 Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31;
 Webs: 2x4 SP #3;

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

B - C	629	-2854	F - G	55	-440
C - D	439	-2101	G - H	66	-462
D - E	966	-217	H - I	137	-893
E - F	966	-218			

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Additional Notes
 The overall height of this truss excluding overhang is 5-4-3.

Maximum Bot Chord Forces Per Ply (lbs)
 Chords Tens.Comp. Chords Tens. Comp.

B - P	2475	-533	N - M	701	-476
P - O	2504	-533	L - K	741	-102
O - N	2034	-440	K - I	744	-101

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.00 to	62 plf at	7.00
TC: From	31 plf at	7.00 to	31 plf at	19.06
TC: From	62 plf at	19.06 to	62 plf at	40.00
BC: From	4 plf at	-1.00 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	7.03
BC: From	10 plf at	7.03 to	10 plf at	19.06
BC: From	20 plf at	19.06 to	20 plf at	39.00
BC: From	4 plf at	39.00 to	4 plf at	40.00
TC:	266 lb Conc. Load at	7.03		
TC:	191 lb Conc. Load at	9.06,11.06,13.06,15.06		
17.06				
TC:	194 lb Conc. Load at	19.06		
BC:	499 lb Conc. Load at	7.03		
BC:	130 lb Conc. Load at	9.06,11.06,13.06,15.06		
17.06				
BC:	131 lb Conc. Load at	19.06		



Maximum Web Forces Per Ply (lbs)
 Webs Tens.Comp. Webs Tens. Comp.

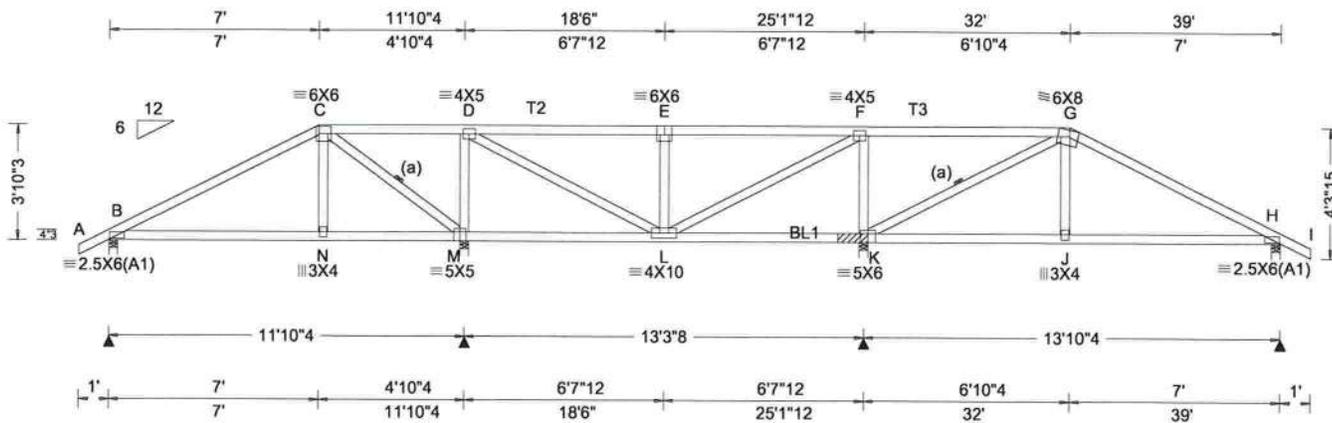
C - P	700	0	E - N	258	-679
C - O	108	-537	N - F	219	-1212
O - D	846	0	F - L	407	-87
D - N	761	-3322	L - H	79	-454

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

FL REG# 278, Yoonhwak Kim, FL PE #86367
 08/11/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.90 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/H VERT(LL): 0.035 E 999 360 VERT(CL): 0.070 E 999 240 HORZ(LL): 0.012 J - - HORZ(TL): 0.025 J - - Creep Factor: 2.0 Max TC CSI: 0.901 Max BC CSI: 0.958 Max Web CSI: 0.635 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 719 /- /- /- /149 /- M 2892 /- /- /- /659 /- K 3171 /- /- /- /721 /- H 932 /- /- /- /198 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 M Brg Width = 3.5 Min Req = 3.4 K Brg Width = 3.5 Min Req = - H Brg Width = 3.5 Min Req = 1.5 Bearings B, M, K, & H are a rigid surface. Members not listed have forces less than 375#

Lumber
 Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 62 plf at -1.00 to 62 plf at 7.00
 TC: From 31 plf at 7.00 to 31 plf at 32.00
 TC: From 62 plf at 32.00 to 62 plf at 40.00
 BC: From 4 plf at -1.00 to 4 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 7.03
 BC: From 10 plf at 7.03 to 10 plf at 31.97
 BC: From 20 plf at 31.97 to 20 plf at 39.00
 BC: From 4 plf at 39.00 to 4 plf at 40.00
 TC: 266 lb Conc. Load at 7.03,31.97
 TC: 191 lb Conc. Load at 9.06,11.06,13.06,15.06,17.06,19.06,19.94,21.94,23.94,25.94,27.94,29.94
 BC: 499 lb Conc. Load at 7.03,31.97
 BC: 130 lb Conc. Load at 9.06,11.06,13.06,15.06,17.06,19.06,19.94,21.94,23.94,25.94,27.94,29.94

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Bearing Block(s)
 Brg blocks:0.131"x3", min. nails
 brg x-loc #blocks length/blk #nails/blk wall plate
 3 25.000' 1 12" 4 Rigid Surface
 Brg block to be same size and species as chord.
 Refer to drawing CNNAILSP1014 for more information.

Additional Notes
 The overall height of this truss excluding overhang is 3-10-3.



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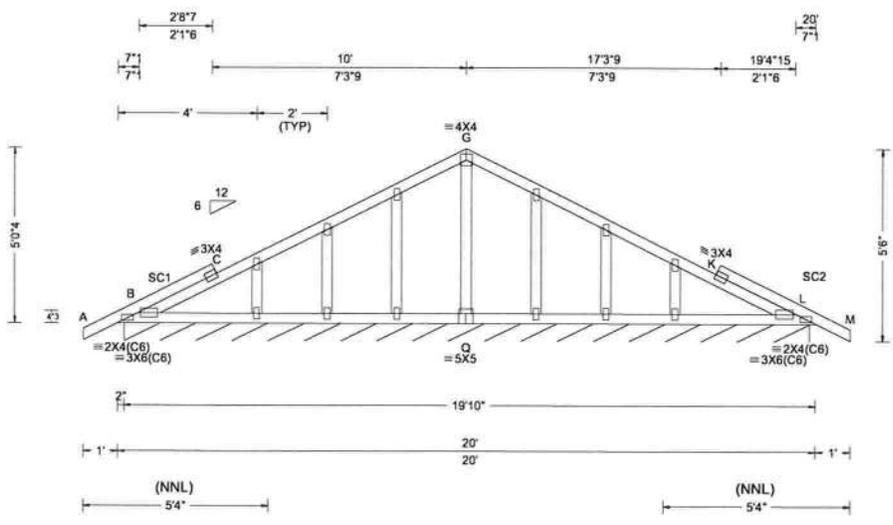
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SEQN: 387905 / FROM: CDM	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 21-5888 Reserve at Jewel Lake 42 - Radford B - GR Truss Label: B01	Cust: R 215 JRef: 1X7V2150005 T19 / DrwNo: 223.21.1145.08176 / YK 08/11/2021
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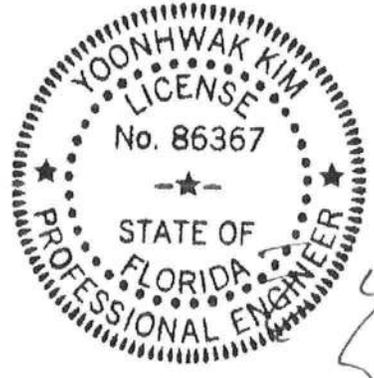
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 360 VERT(CL): 0.010 C 999 240 HORZ(LL): 0.002 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.136 Max BC CSI: 0.071 Max Web CSI: 0.060 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 91 /- /- /46 /- /3 Wind reactions based on MWFRS B Brg Width = 236 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 5'-0-4."

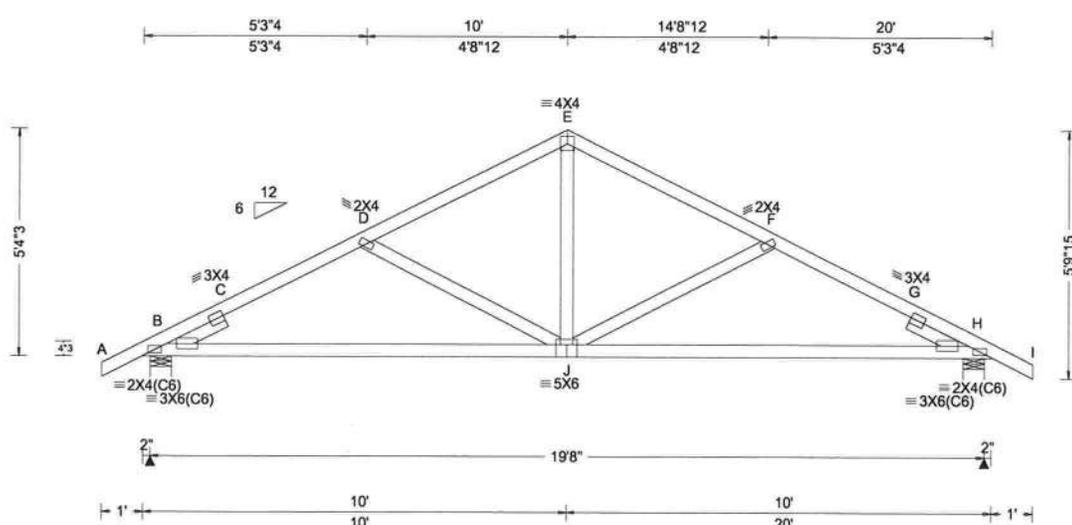


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SEQN: 387908 / COMN Ply: 1 Job Number: 21-5888 Cust: R215 JRef: 1X7V2150005 T24 /
 FROM: CDM Qty: 1 Reserve at Jewel Lake 42 - Radford B - GR DrwNo: 223.21.1145.09177
 Truss Label: B02 / YK 08/11/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.036 J 999 360	B	890	/-	/-	/535	/157	/154
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.072 J 999 240	H	890	/-	/-	/535	/157	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.016 J - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.033 J - -	B	Brg Width = 6.0		Min Req = 1.5			
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	H	Brg Width = 6.0		Min Req = 1.5			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.232	Bearings B & H are a rigid surface.						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.824	Members not listed have forces less than 375#						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.244	Maximum Top Chord Forces Per Ply (lbs)						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.01.01A.0521.20	Chords	Tens.Comp.	Chords	Tens. Comp.			
	Loc. from endwall: Any	WAVE		B - C	0	-834	E - F	483	-947	
	GCpi: 0.18			C - D	603	-1212	F - G	603	-1212	
	Wind Duration: 1.60			D - E	483	-947	G - H	0	-834	

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Lt Slider: 2x4 SP #3; block length = 1.500'
 Rt Slider: 2x4 SP #3; block length = 1.500'

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 5-4-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 08/11/2021

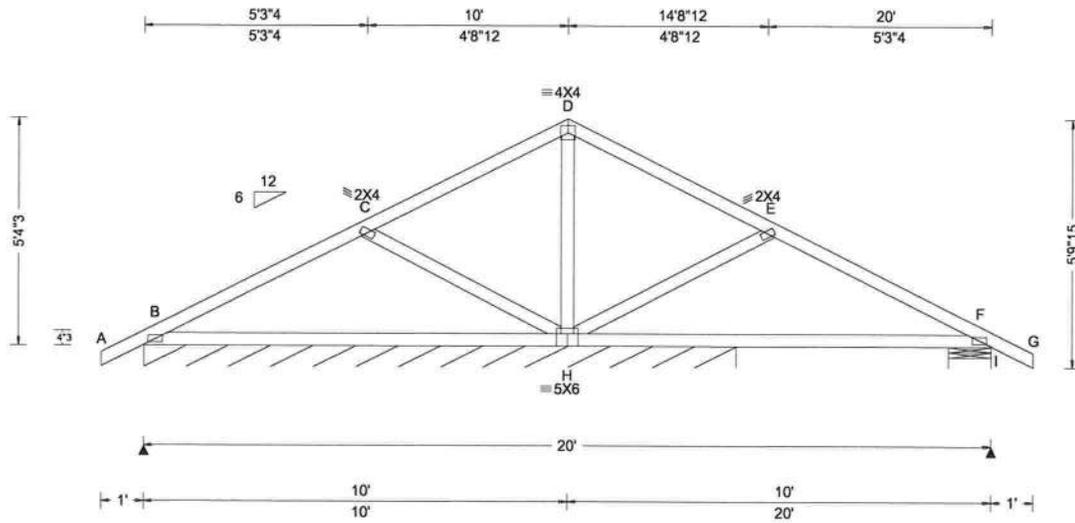
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SEQN: 387911 / FROM: CDM	COMN Ply: 1 Qty: 1	Job Number: 21-5888 Reserve at Jewel Lake 42 - Radford B - GR Truss Label: B03	Cust: R215 JRef: 1X7V2150005 T26 / DrwNo: 223.21.1145.09505 / YK 08/11/2021
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.012 H 999 360 VERT(CL): 0.039 H 999 240 HORZ(LL): 0.006 H - - HORZ(TL): 0.021 H - - Creep Factor: 2.0 Max TC CSI: 0.479 Max BC CSI: 0.711 Max Web CSI: 0.246 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B* 99 /- /- /56 /18 /11 I 393 /- /- /278 /67 /- Wind reactions based on MWFRS B Brg Width = 168 Min Req = - I Brg Width = 12.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. H - F 494 -16
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4(A1) except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

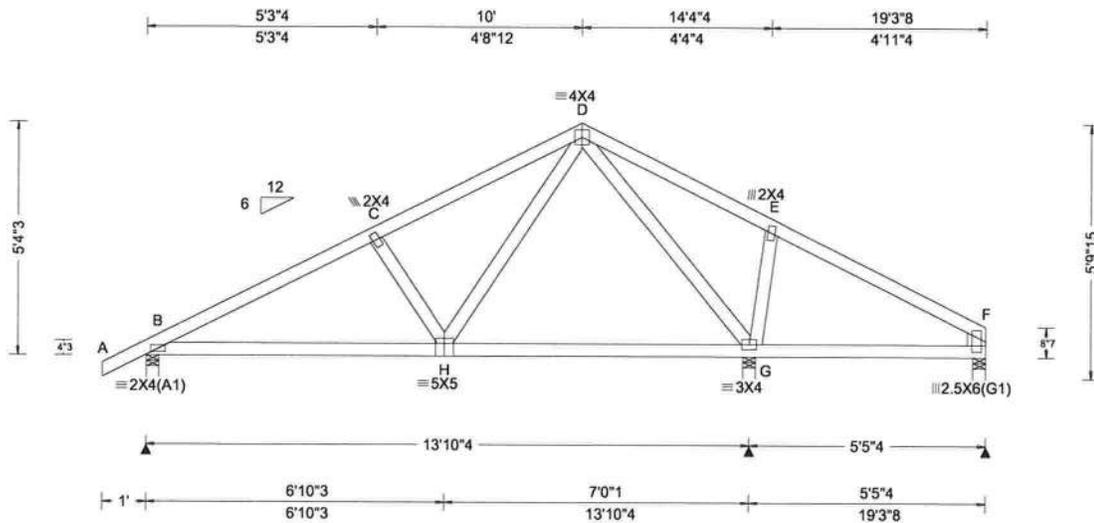
Additional Notes
The overall height of this truss excluding overhang is 5-4-3.



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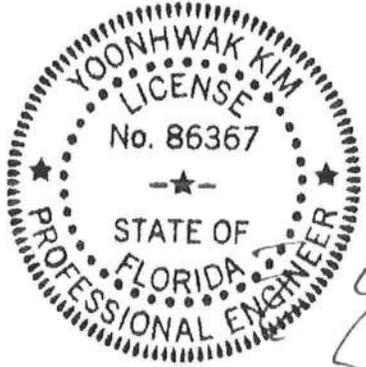


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.015 C 999 360 VERT(CL): 0.030 C 999 240 HORZ(LL): 0.005 G - - HORZ(TL): 0.010 G - - Creep Factor: 2.0 Max TC CSI: 0.308 Max BC CSI: 0.424 Max Web CSI: 0.421 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 633 /- /- /397 /113 /138 G 832 /- /- /437 /143 /- F 234 /- /- /143 /31 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 G Brg Width = 3.5 Min Req = 1.5 F Brg Width = 3.5 Min Req = 1.5 Bearings B, G, & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 283 -836 C - D 285 -675
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

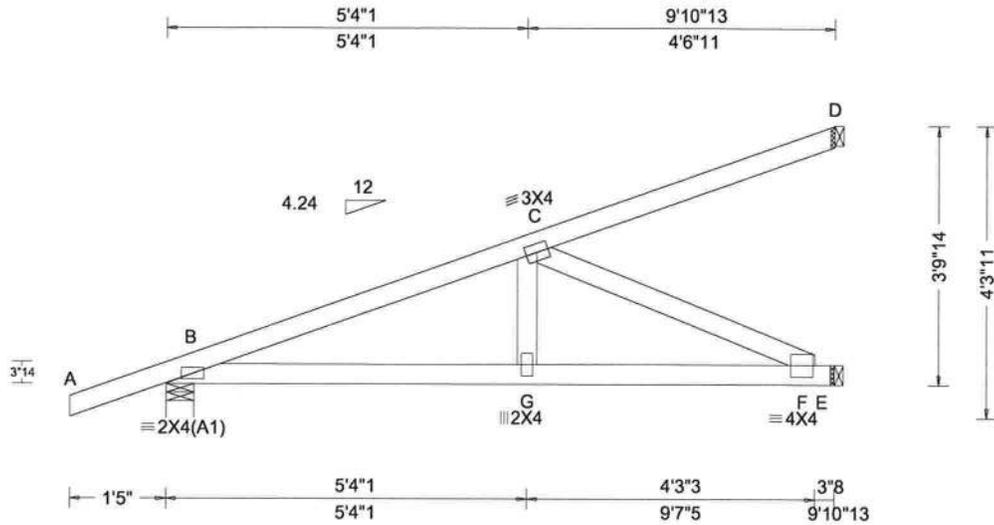
Additional Notes
The overall height of this truss excluding overhang is 5-4-3.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.023 G 999 360 VERT(CL): 0.047 G 999 240 HORZ(LL): 0.006 F - - HORZ(TL): 0.011 F - - Creep Factor: 2.0 Max TC CSI: 0.549 Max BC CSI: 0.662 Max Web CSI: 0.361 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 347 /- /- /- /118 /- E 369 /- /- /- /70 /- D 75 /- /- /- /30 /- Wind reactions based on MWFRS B Brg Width = 4.9 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 202 -786 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - G 738 -179 G - F 725 -181 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 199 -800
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From -0 plf at -1.41 to 61 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 9.90
BC: From 0 plf at -1.41 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 9.90
TC: -9 lb Conc. Load at 1.48
TC: 143 lb Conc. Load at 4.31
TC: 265 lb Conc. Load at 7.13
BC: 20 lb Conc. Load at 1.48
BC: 104 lb Conc. Load at 4.31
BC: 182 lb Conc. Load at 7.13

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

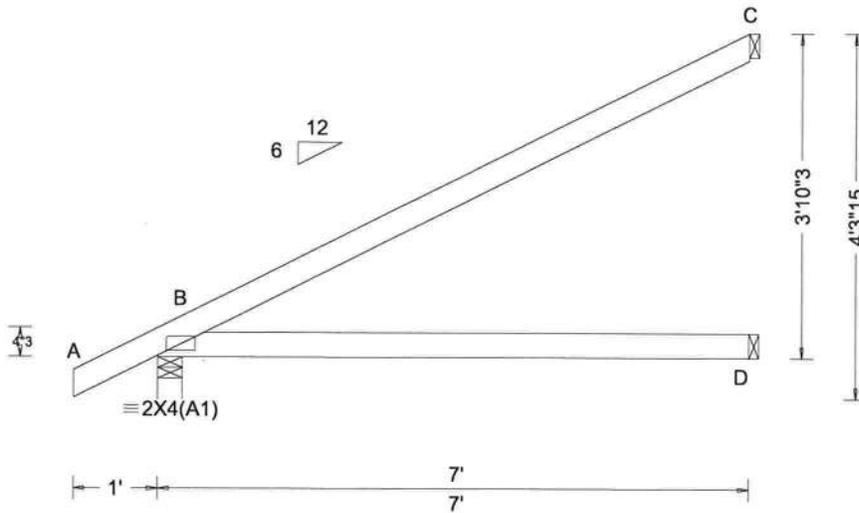
Additional Notes
The overall height of this truss excluding overhang is 3-9-14.



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)																																		
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 D - - HORZ(TL): 0.031 D - - Creep Factor: 2.0 Max TC CSI: 0.740 Max BC CSI: 0.522 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>368</td> <td>-</td> <td>-</td> <td>/245</td> <td>/36</td> <td>/137</td> </tr> <tr> <td>D</td> <td>130</td> <td>-</td> <td>-</td> <td>/75</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>191</td> <td>-</td> <td>-</td> <td>/121</td> <td>/95</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	368	-	-	/245	/36	/137	D	130	-	-	/75	-	-	C	191	-	-	/121	/95	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	368	-	-	/245	/36	/137																																
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C	191	-	-	/121	/95	-																																

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-10-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

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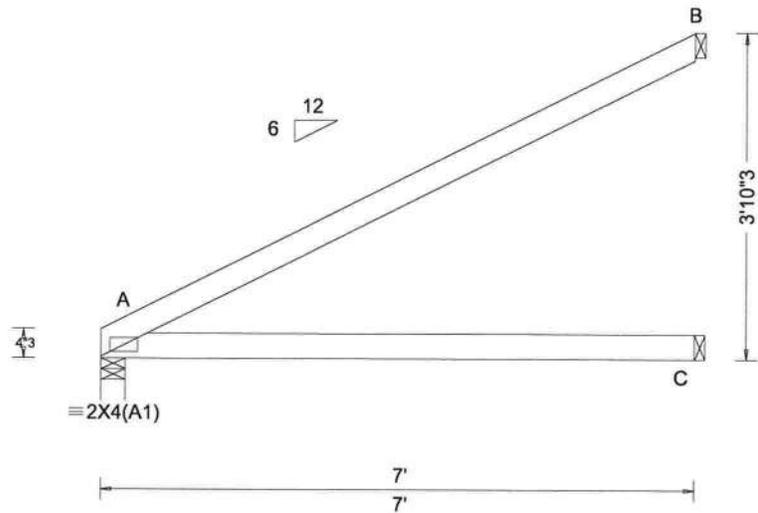
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6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.016 C - - HORZ(TL): 0.034 C - - Creep Factor: 2.0 Max TC CSI: 0.766 Max BC CSI: 0.532 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>294</td> <td>/-</td> <td>/-</td> <td>/186</td> <td>/-</td> <td>/90</td> </tr> <tr> <td>C</td> <td>131</td> <td>/-</td> <td>/-</td> <td>/78</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>B</td> <td>194</td> <td>/-</td> <td>/-</td> <td>/124</td> <td>/60</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	294	/-	/-	/186	/-	/90	C	131	/-	/-	/78	/-	/-	B	194	/-	/-	/124	/60	/-
				Loc		Gravity			Non-Gravity																													
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Wind reactions based on MWFRS A Brg Width = 3.5 Min Req = 1.5 C Brg Width = 1.5 Min Req = - B Brg Width = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#																																						

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 3-10-3.

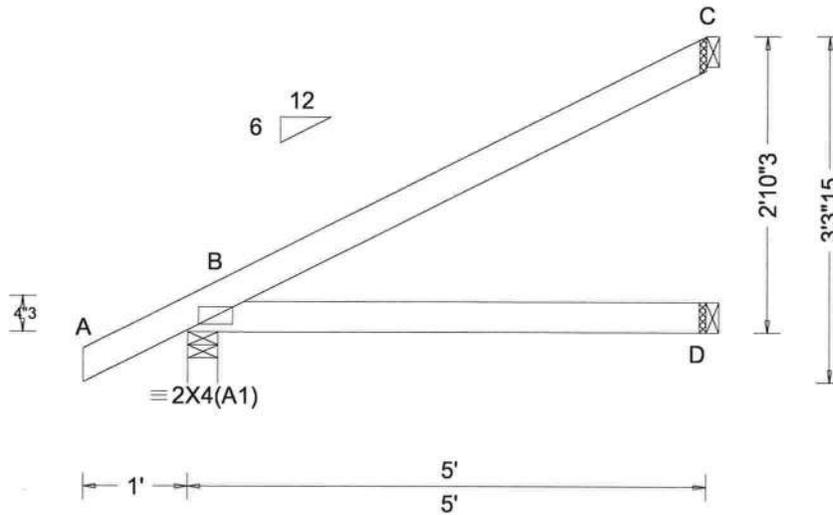


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SEQN: 387862 / FROM: CDM	JACK Qty: 6	Ply: 1	Job Number: 21-5888 Reserve at Jewel Lake 42 - Radford B - GR Truss Label: J02	Cust: R215 JRef: 1X7V2150005 T11 / DrwNo: 223.21.1145.08895 / YK 08/11/2021
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 D - - HORZ(TL): 0.010 D - - Creep Factor: 2.0 Max TC CSI: 0.336 Max BC CSI: 0.243 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																															
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>288</td> <td>/-</td> <td>/-</td> <td>/195</td> <td>/31</td> <td>/102</td> </tr> <tr> <td>D</td> <td>91</td> <td>/-</td> <td>/-</td> <td>/52</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>133</td> <td>/-</td> <td>/-</td> <td>/84</td> <td>/66</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 D Brg Width = 1.5 Min Req = - C Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	288	/-	/-	/195	/31	/102	D	91	/-	/-	/52	/-	/-	C	133	/-
Loc	Gravity			Non-Gravity																															
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Lumber

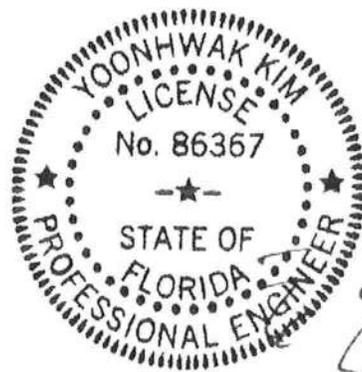
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

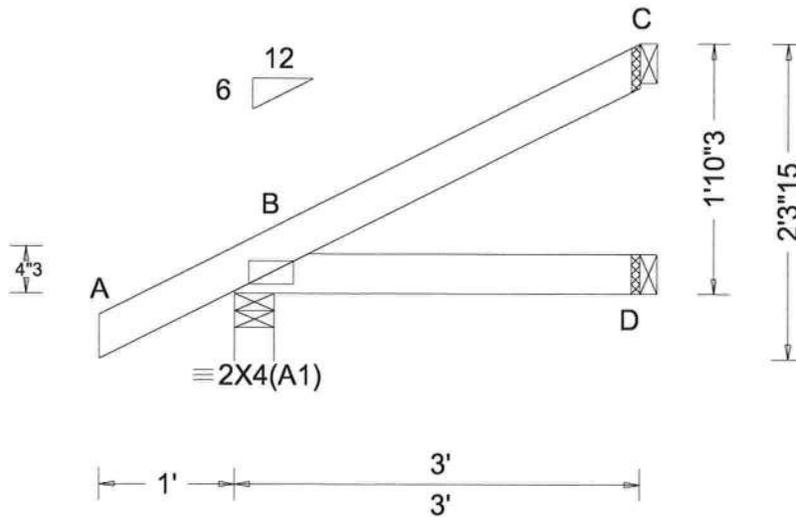
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 AN ITW COMPANY
 6750 Forum Drive
 Suite 305
 Orlando FL, 32821



Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defll/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.123 Max BC CSI: 0.071 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																														
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Loc	Gravity			Non-Gravity																														
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

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Additional Notes

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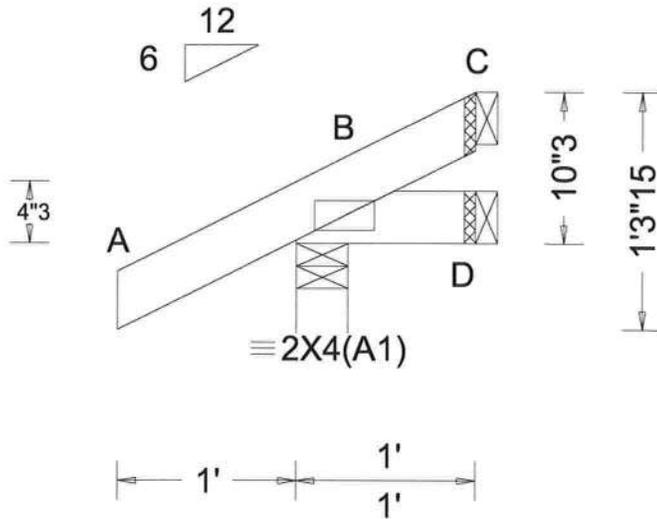
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Loc	Gravity			Non-Gravity																																		
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Lumber
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Wind
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Additional Notes
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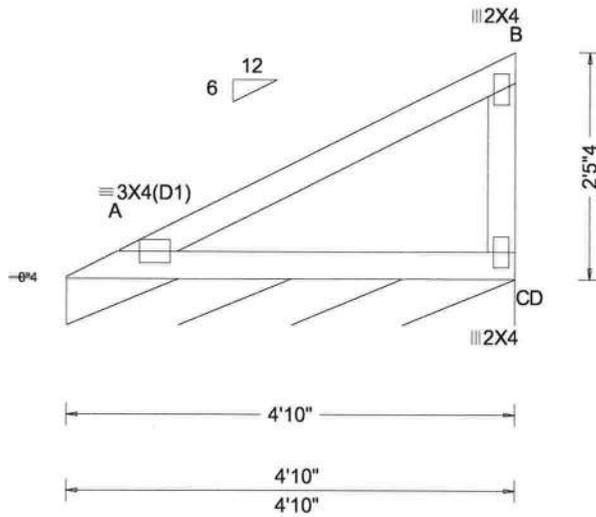
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 C - - HORZ(TL): 0.011 C - - Creep Factor: 2.0 Max TC CSI: 0.278 Max BC CSI: 0.264 Max Web CSI: 0.107 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL D* 82 /- /- /51 /2 /11 Wind reactions based on MWFRS D Brg Width = 58.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.
The overall height of this truss excluding overhang is 2-5-4.



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

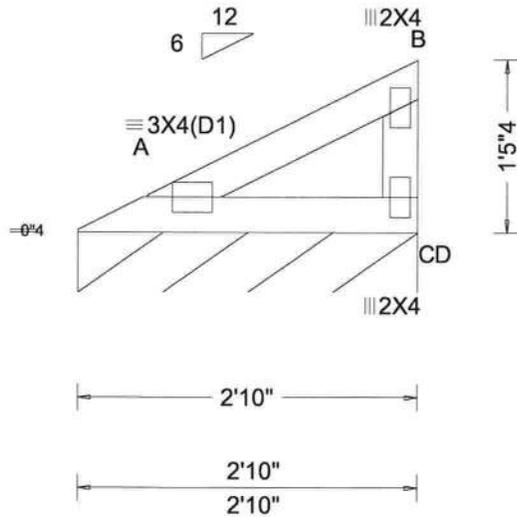
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.085 Max BC CSI: 0.077 Max Web CSI: 0.044 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL D* 82 /- /- /48 /1 /10 Wind reactions based on MWFRS D Brg Width = 34.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.
The overall height of this truss excluding overhang is 1'-5-4.



FL REG# 278, Yoonhwak Kim, FL PE #86367
08/11/2021

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4 $\text{\textcircled{X}}$
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6 $\text{\textcircled{X}}$

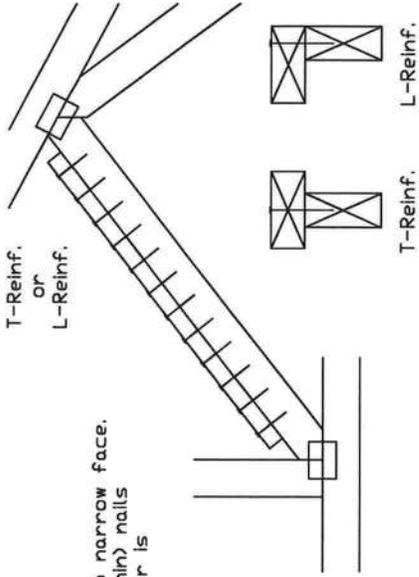
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

$\text{\textcircled{X}}$ Center scab on wide face of web. Apply (1) scab to each face of web.

T-Reinforcement

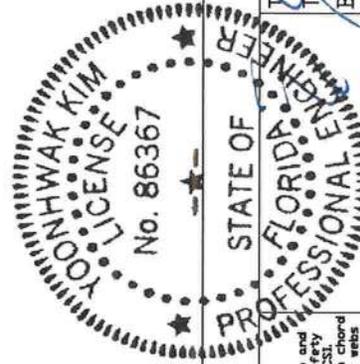
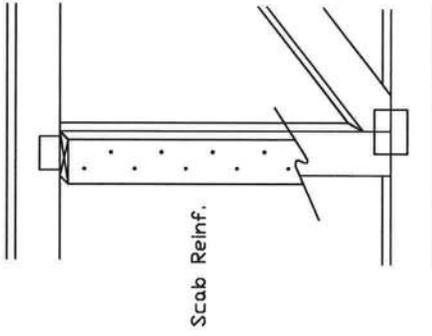
OR
L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0"/min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0"/min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING AND FOLLOW ALL NOTES ON THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of AISC Building Component Safety Information, by TPI and SBCA for safety instructions. Trusses shall be installed in accordance with the manufacturer's instructions. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per AISC sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the manufacturer's instructions for the installation, bracing of trusses in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses. A seal on this drawing or cover page listing the drafter, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpine.com TPI: www.tpi.com SBCA: www.sbcainc.com

ALPINE
 AN ITW COMPANY
 514 Earth City Expressway
 Suite 242
 Earth City, MO 63045

IC LL	PSF	REF	CLR Subst.
1/2 DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Yoonhwak Kim, EIT, PE #86367

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

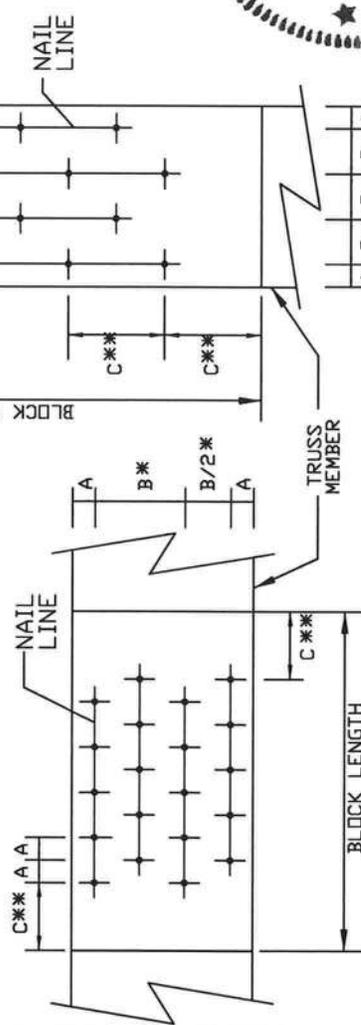
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

* SPACING MAY BE REDUCED BY 50%.

** SPACING MAY BE REDUCED BY 33%.

DIRECTION OF LOAD AND NAIL ROWS



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113" X 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128" X 3", MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128" X 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135" X 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148" X 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148" X 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148" X 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162" X 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120" X 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120" X 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 3", MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLER'S.
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SCAQ for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI requirements. Trusses shall be braced in accordance with the bracing details shown on this drawing. Trusses shall have bracing installed per BCSI sections 32, 37 or 310, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the installation of any truss to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this job's general notes page and these web sites: www.alpine.com, www.tpi.com, www.scaq.com, www.bcsi.com, www.sbcindustry.org, www.icb.com

ALPINE
 AN ITW COMPANY
 514 Earth City Expressway
 Earth City, MO 63045

REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Yoonhwak Kim, FL PE #86367

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or
 equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

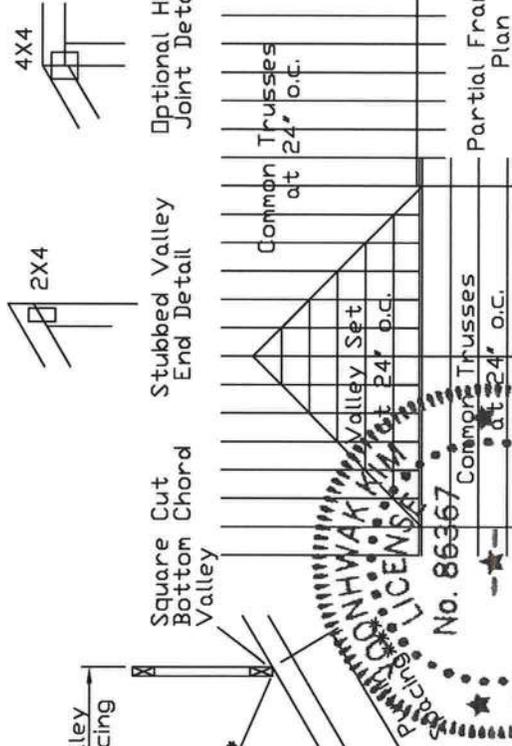
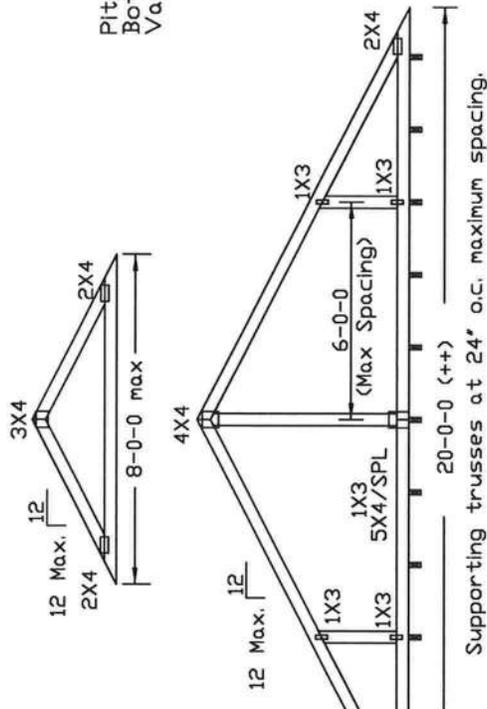
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9' apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6' o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.
 Or
 Purlins at 24' o.c. or as otherwise specified on engineer's sealed design

Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

** Larger spans may be built as long as the vertical height does not exceed 14'-0".



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 Suite 242
 Earth City, MO 63045

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the design shown unless the truss is fabricated in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2.

or more information see this job's general notes page and these web sites: <http://www.alpine.com>, <http://www.tpi.com>, <http://www.sbcasafety.com>, <http://www.bcs1.com>

REF	VALLEY DETAIL	40PSF	30	30	15	7 PSF	DATE	01/26/2018
TC	LL	30	30	15	7 PSF	DATE	01/26/2018	
TC	DL	20	20	10	10 PSF	DRWG	VAL180160118	
BC	DL	10	10	0	0 PSF			
BC	LL	0	0	55	57PSF			
TOT.	L.D.	60	60	115	115			
DUR.FAC.	1.25/1.33	1.15	1.15					
SPACING				24.0'				

