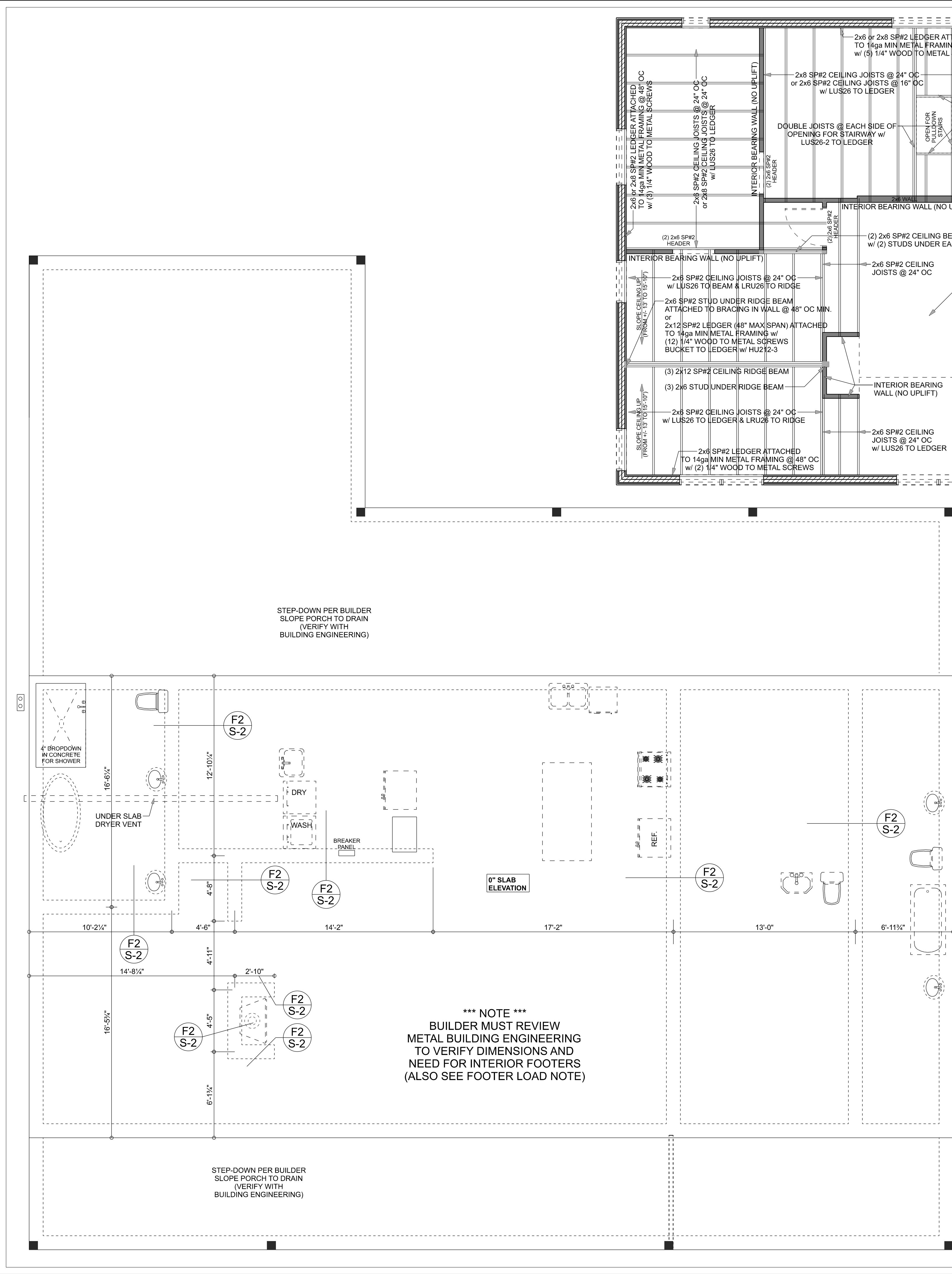




FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING

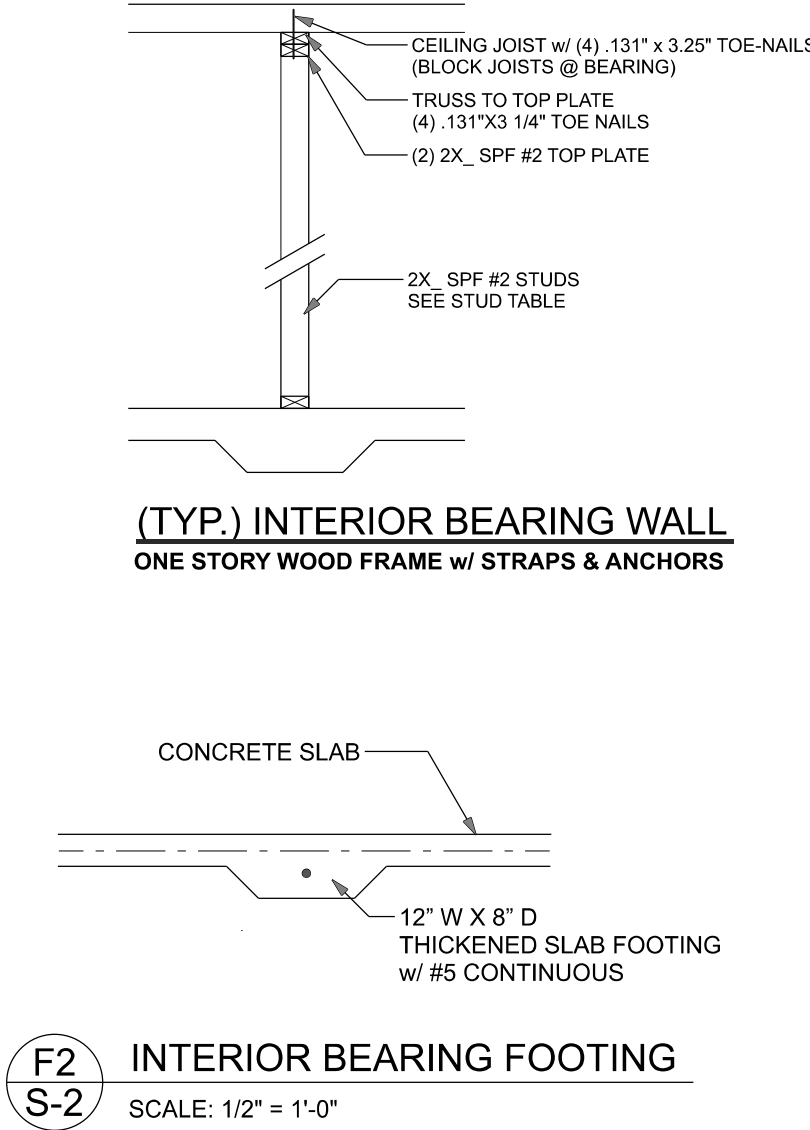
FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE-APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**IF FOUNDATION IS ON A STEEP SLOPE
CONTACT ENGINEER BEFORE CONSTRUCTION
FOR ADDITIONAL ENGINEERING**



INTERIOR BEARING FOOTING

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 3 INCHES, DISPERSE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM BEARING TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE	Fb	E
2x8 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2600	1.9
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2950	2.0
PSL PARALAM	2900	2.0

CONNECTOR TABLE	To Plate	To Truss/Rafter
Uplift SP Uplift SPF Truss Connector		
805 505 SDWC15600		
400 290 H3	4-131"x1 1/2"	4-131"x1 1/2"
625 540 H2.5A	5-131"x1 1/2"	5-131"x1 1/2"
1040 1015 H10A	9-148"x1 1/2"	9-148"x1 1/2"
645 515 L131X120	6-148"x1 1/2"	6-148"x1 1/2"
990 850 MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415 1215 HTS16-30	8-148"x1 1/2"	8-148"x1 1/2"
Uplift SP Uplift SPF Strap Ties		
1235 1235 LSTA21	8-148"x1 1/2"	8-148"x1 1/2"
1640 1460 MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030 1030 CS20	7-148"x1 1/2"	7-148"x1 1/2"
Uplift SP Uplift SPF Stud Plate Ties		
555 535 SP1	4-148"x3"	4-148"x3"
1010 605 SP2	6-148"x3"	6-148"x3"
1280 1100 SPH46	12-148"x1 1/2"	wrap under or over plate
771 771 LSTA24	10-148"x1 1/2"	wrap under or over plate
1235 1235 LSTA24	14-148"x1 1/2"	wrap under or over plate
Uplift SP Uplift SPF Holdowns @ Stemmwall		
2145 1835 DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235 3640 HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP Uplift SPF Holdowns @ Mono		
2145 1835 DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235 3640 HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP Uplift SPF Post Bases @ Stemmwall		
1900 ABU4Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475 ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP Uplift SPF Post Bases @ Mono		
1900 ABU4Z	12-162"x3 1/2"	5/8"x7" Drill & Epoxy
2475 ABU6Z	12-162"x3 1/2"	5/8"x7" Drill & Epoxy

DESIGN CRITERIA & LOADS:	
BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	SEE METAL BUILDING ENGINEERING
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ATTICS	20 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

Erkinger Construction Group

Kyle and Brandi Brownfield
Residence

PROJECT ADDRESS:
218 SW Jewel Court
PARCEL ID# 07-485-16-03767-0174724)
COLUMBIA COUNTY, FLORIDA

Mark Disoway FL PE 53915

This item has been digitally signed and sealed by Mark Disoway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

THIS PDF HAS DIGITAL SIGNATURE AND ELECTRONIC SEAL. PRINTED COPIES ARE NOT CONSIDERED SIGNED OR SEALED. YOU MUST VERIFY SIGNATURE ON THIS PDF. CLICK HERE TO VERIFY.

INTERIOR STRUCTURE ONLY

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
260577

S-1
OF 1 SHEET